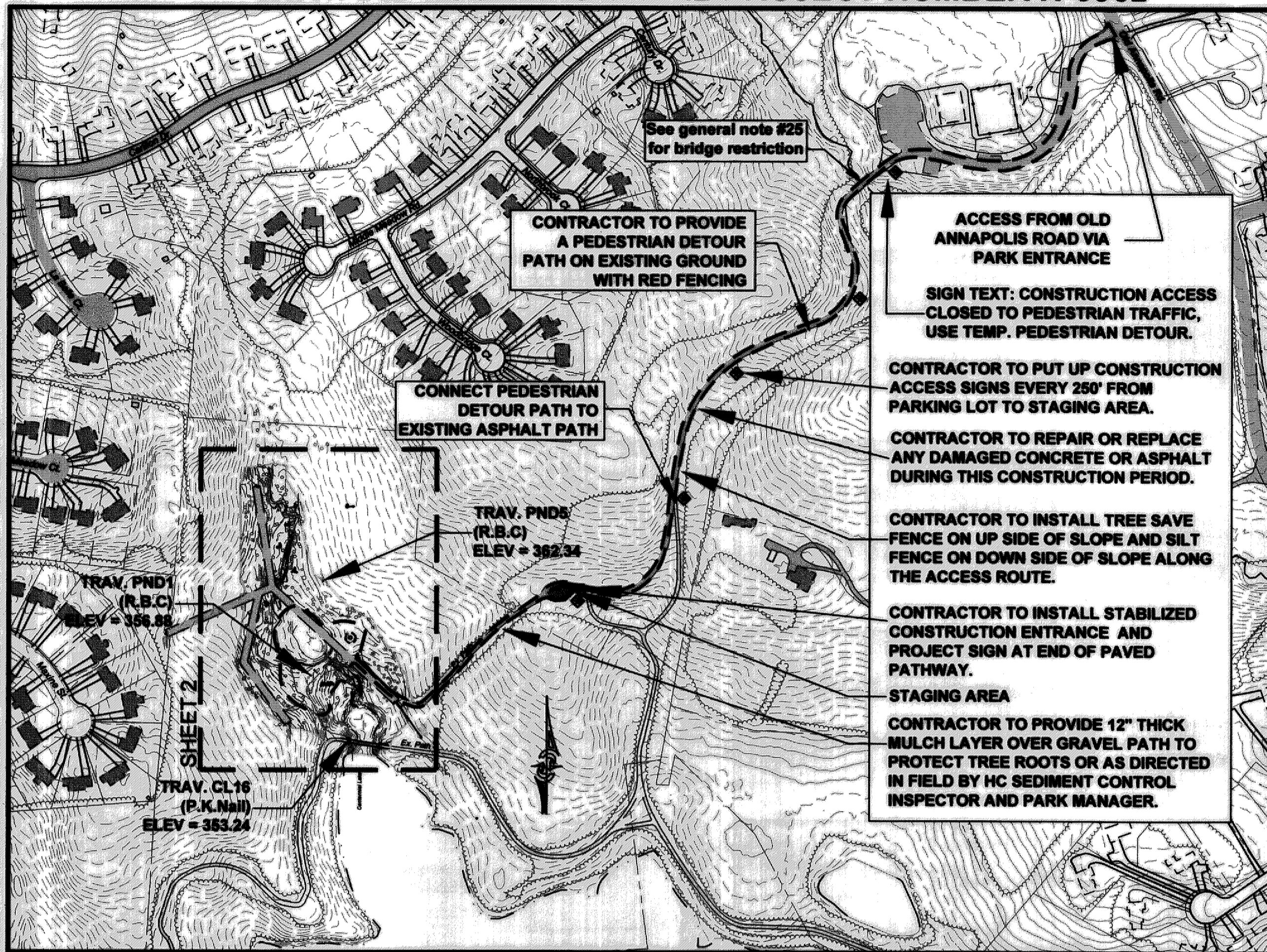
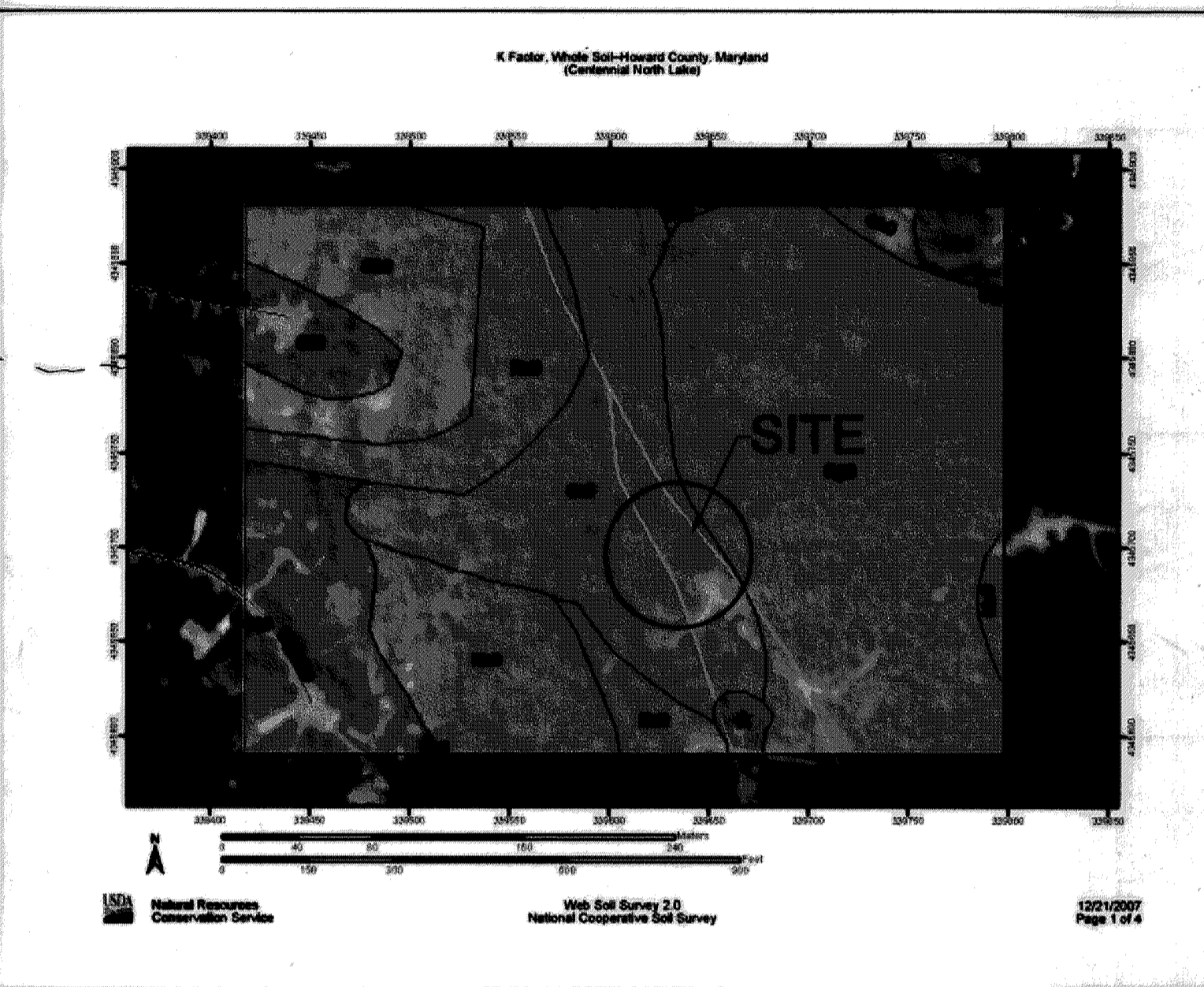


# CENTENNIAL PARK NORTH POND RETROFIT

HOWARD COUNTY CAPITAL PROJECT NUMBER N-3962



## SHEET INDEX

- Title Sheet
- Plan View and Profiles
- Details
- Notes and Details
- Phase I - Sediment Control
- Phase II - Sediment Control
- Sediment Control Notes and Details
- Sediment Control Notes and Details

## OPERATION MAINTENANCE AND INSPECTION

Inspection of the pond(s) shown hereon shall be performed at least annually, in accordance with the checklist and requirements contained within the USDA, NRCS "Standards and Specifications for Ponds" (MD-378). The pond owner(s) and any heirs, successors, or assigns shall be responsible for the safety of the pond and the continued operation, surveillance, inspection, and maintenance thereof. The pond owner(s) shall promptly notify the Soil Conservation District of any unusual observations that may be indication of distress such as excessive seepage, turbid seepage, sliding or slumping.

## GENERAL NOTES

- All construction shall be in accordance with the latest standards and specifications of Howard County plus MSHA Standards and specifications if applicable.
- The contractor shall notify the Department of Public Works/Bureau of Engineering/Construction Inspection Division at (410) 313-1880 at least five (5) working days prior to the start of work.
- The contractor shall notify "MISS Utility" at 1-800-257-7777 at least 48 hours prior to any excavation work being done.
- Traffic control devices, markings and signing shall be in accordance with the latest edition of the Manual of Uniform Traffic Control Devices (MUTCD). All street and regulatory signs shall be in place prior to the placement of any asphalt.
- All plan dimensions are to face of curb unless otherwise noted.
- The existing topography is taken from field run survey with two foot contour intervals provided by Howard County field survey dated 08/28/2007.

## SURVEY CONTROL

Point	Northings	Eastings	Elevation	Description
BM1 Trv. CL	181574787.35	1352536.30	353.24	(P.K. Nail)
BM2 Trv. PND1	574865.50	1352438.28	356.88	(R.B.C)
BM3 Trv. PND5	575214.19	1352468.15	362.34	(R.B.C)

All horizontal control is NAD 83/91.  
All vertical control is NGVD 86.

- Water: 522-D
- Sewer: 522-D
- There is no existing or proposed stormwater management control required for this project.
- Existing utilities are based on the survey by HC and only include utilities visible at surface (i.e., manholes).
- The floodplain study for this project was taken from a HEC-RAS study by CPJ and Hazen & Sawyer dated March 2012.
- Wetland shown hereon were field-delineated by CPJ/MCE on July 27, 2007.
- No traffic study is required for this project.
- No clearing, grading or construction is permitted within the delineated stream except as shown hereon. No work can be done within the stream until a permit from the Maryland Department of the Environment is secured. The stream and stream buffer disturbance shown on these plans have been determined "necessary" in accordance with Section 16.116.c of the Subdivision Regulations.
- Centennial Lake is a 50-acre impoundment on the Centennial Branch of the Little Patuxent River. These are Maryland Use Class IV-P Waters. Stream use closure dates are March 1 - May 31 inclusive.
- All material removed from this site shall be taken to a site with an active grading permit.
- These plans were prepared with the field information at the time of construction vary from these plans and it is the contractor's responsibility to verify field conditions such as elevations, depths, etc. prior to proceeding with work. It is the contractor's responsibility to verify with the supplier/manufacturer of any proprietary product that their product will function per the design for the given field conditions. The design engineer should be notified immediately if any deviations from the design plan are found.
- All specified and/or proprietary products shown hereon may be subject to substitution with other products recommended by the contractor, subject to written review and approval of the design engineer.
- The average estimated dry weather base flow for this project was estimated at 0.01 cfs for pump-around purposes. This information is provided for conceptual use by the contractor but should not be considered binding to this design as distant storm events, weather patterns, groundwater discharge, upstream man-induced releases, snow melt, etc. are incalculable factors which can increase or decrease dry weather flow. The contractor is responsible to carry out a site reconnaissance to determine the size and number of pumps/he/she will need to bid and complete work.
- All quantities hereon are estimates only, the contractor is responsible for verifying quantities through a field visit and his own quantity takeoffs.
- This project is conditionally exempt from the forest conservation requirements of Section 16.1200 of Howard County Code for Forest Conservation: (Forest conservation Waiver Petition WF-08-082 was approved on 04/01/2008.)
- This plan has been prepared in accordance with the provisions of Section 16.124 of the Howard County Code and the Landscape Manual.
- This plan was prepared with the best available records which did not include geotechnical testing analysis. It is the Contractor's responsibility to determine geotechnical field conditions prior to bid.
- The maximum gross vehicle weight (GVW) for frequent traffic over the Low Profile Arch SUPER-SPAN bridge at the construction access should not exceed 36.0 tons per CBC Engineers & Associates, Ltd., report to Howard County in August 2009.
- The MDE wetland and waterway tracking permit number for this project is #201061252 dated October 26, 2010.
- Centennial Park Lake was built in 1985.

## SOILS NOTE:

The Glenville soil series (Howard County soil designation "GnB") are moderately drained, strongly acidic to very strongly acidic soils that have a fragipan. Taxonomic class is fine-loamy, mixed, active, mesic Aquic Fragiuults. They are found on flats, in depressions, at the foot of slopes, and around the heads of drains. Slopes range from 0 to 3 percent. Glenville silt loam is not acknowledged as a hydric soil type.

## SITE ANALYSIS CHART

A	Total project area is 104.36 acres
B	Area of plan submission is the same as the limits of disturbance.
C	Limit of disturbed area (LOD) is 97,400 square feet or 2.2 acres.
D	Present zoning is R-20 County Park (open space).
E	Proposed use of site is to remain open space.
F	Floor space/number of units/employees/parking is not applicable.
G	Open space on this site is assumed to be the same as the LOD or 2.2 acres.
H	Required open space is not applicable.
I	Building coverage is not applicable.
J	This project is for the replacement of existing pond embankment.

## DAM HAZARD SUMMARY

Dam Hazard Summary		low hazard class "A"
Dam Normal Depth Foot:	4.5 feet	
Dam Height (downstream barrel invert to e.s.):	8.5 feet	
Dam Height (d/s invert to top of settled dam):	11.2 feet	
Surface Area (normal pool at WSE = 353.5):	0.42 acres	
Storage (normal pool at WSE = 353.5):	1.05 ac-ft ("dead" storage)	
Surface Area (at WSE = 355.5 for e.s. flow to begin):	0.65 acres	
Storage (at WSE = 355.5 for e.s. flow to begin):	1.08 ac-ft ("live" storage)	
Surface Area (when WSE = 357.1 for 100-year):	0.90 acres	
Storage (when WSE = 357.1 for 100-year):	2.25 ac-ft ("live" storage)	
Surface Area (when WSE is at settled top of dam):	1.28 acres	
Storage (top of settled dam):	4.41 ac-ft ("live" storage)	
G100 release from barrel:	140 cfs	
G100 release from e.s.:	203 cfs	
G100 (total):	343 cfs	
Velocity in e.s. at 100-year WSE:	5.4 fps	

## TREE IMPACTS TABLE

ID	Common Name	DBH	Trunk	Health	CRZ	Reason
1	Honey Locust	5	Single	Fair	7.5	Grading
2	Red Maple	5	Single	Fair	7.5	Grading
3	Sassafras	5	Single	Fair	7.5	Grading
4	Spice Bush	1	Single	Fair	1.5	Grading
5	Norway Maple	6	Single	Fair	9	Grading
132	Tulip Poplar	20	Single	Fair	30	Grading
133	Black Walnut	8	Single	Poor	12	Grading
134	Black Walnut	18	Single	Fair	27	Grading
160	Tulip Poplar	15	Single	Fair	22.5	Grading
162	Red Maple	10	Single	Fair	15	Grading
163	Tulip Poplar	11	Single	Fair	16.5	Grading
164	Tulip Poplar	13.5	Single	Fair	20.25	Grading
165	Red Maple	8	Single	Fair	12	Grading
166	Black Walnut	9	Double	Fair	13.5	Grading
167	Red Maple	7	Single	Fair	10.5	Grading
168	Honey Locust	9	Single	Fair	13.5	Grading
A	Tree (Unknown)	3.5	Single	NA	5.25	
Total = 17						

## PERMIT INFORMATION CHART

Subdivision Name	Section/Area	Lot/Parcel
Centennial Park	N/A	P. 10

Plat # or L/F	Grid #	Zone	Tax/Zone Map	Elec. Dist.	Census Tract
L885/F76	7	R-20	30	5	602304
L607/F437		County Park			

## ADDRESS CHART

Lot Number	Street Address
N/A	9801 Old Annapolis Road Ellicott City, MD 21042

## SUMMARY OF ENVIRONMENTAL IMPACTS

Restoration Design Area	Tree Removal (# of trees)	Stream Disturbance (ft)	Permanent Wetland Disturbance (sq ft)	Temp. Wetland Disturbance (sq ft)	Temp. Wetland Buffer Disturbance (sq ft)	Total LOD (sq ft) Includes Access	Total LOD (acres) Includes Access	Phase 1 LOD (sq ft) Excludes Access	Phase 1 LOD (acres) Excludes Access	Phase 2 LOD (sq ft) Excludes Access	Phase 2 LOD (acres) Excludes Access	CUT (cy)	FILL (cy)	NET (cy)
Total	17	0	480	2,000	4,860	98,590	2.3	19,900	0.46	47,850	1.1	1,640	1,930	290

APPROVED: HC DEPARTMENT OF PUBLIC WORKS  
 Director of Public Works  
 Chief, Bureau of Environmental Services  
 Chief, Stormwater Management Division

THESE PLANS FOR SMALL POND CONSTRUCTION SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD COUNTY SOIL CONSERVATION DISTRICT.

"I HAVE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE ACCORDING TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD COUNTY SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTIONS BY THE HOWARD COUNTY SOIL CONSERVATION DISTRICT."

"This facility has been evaluated for downstream flooding hazard by a study entitled Centennial Lake Pond Repair Computations prepared by Hazen & Sawyer and Charles P. Johnson & Associates, Inc. in March of 2012. According to that study, this facility is a Class A facility with classification of low hazard."

Call "Miss Utility" at 1-800-257-7777, 48 hours prior to the start of work. The excavator must notify all public utility companies with underground facilities in the area of proposed excavation and have those facilities located by the utility companies prior to commencing excavation.

"I CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD COUNTY SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD COUNTY SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

OWNER/DEVELOPER  
 Howard County Recreation and Parks  
 PRINTED NAME

SIGNATURE  
 TIMOTHY SCHUELER  
 PRINTED NAME  
 20207  
 MD LICENSE #  
 10/18/2015  
 EXPIRATION DATE

HOWARD COUNTY DPW - ENVIRONMENTAL SERVICES  
 6751 COLUMBIA GATEWAY DRIVE, SUITE 514  
 COLUMBIA, MD 21046  
 PHONE: (410) 313-6413  
 ATTN: MARK RICHMOND

9801 Old Annapolis Road  
 HOWARD COUNTY, MD  
 ELECTION DISTRICT 5  
 MAP 30, GRID 7, PARCEL 10

## CENTENNIAL PARK NORTH POND RETROFIT

Howard County Project # N-3962  
 Title Sheet

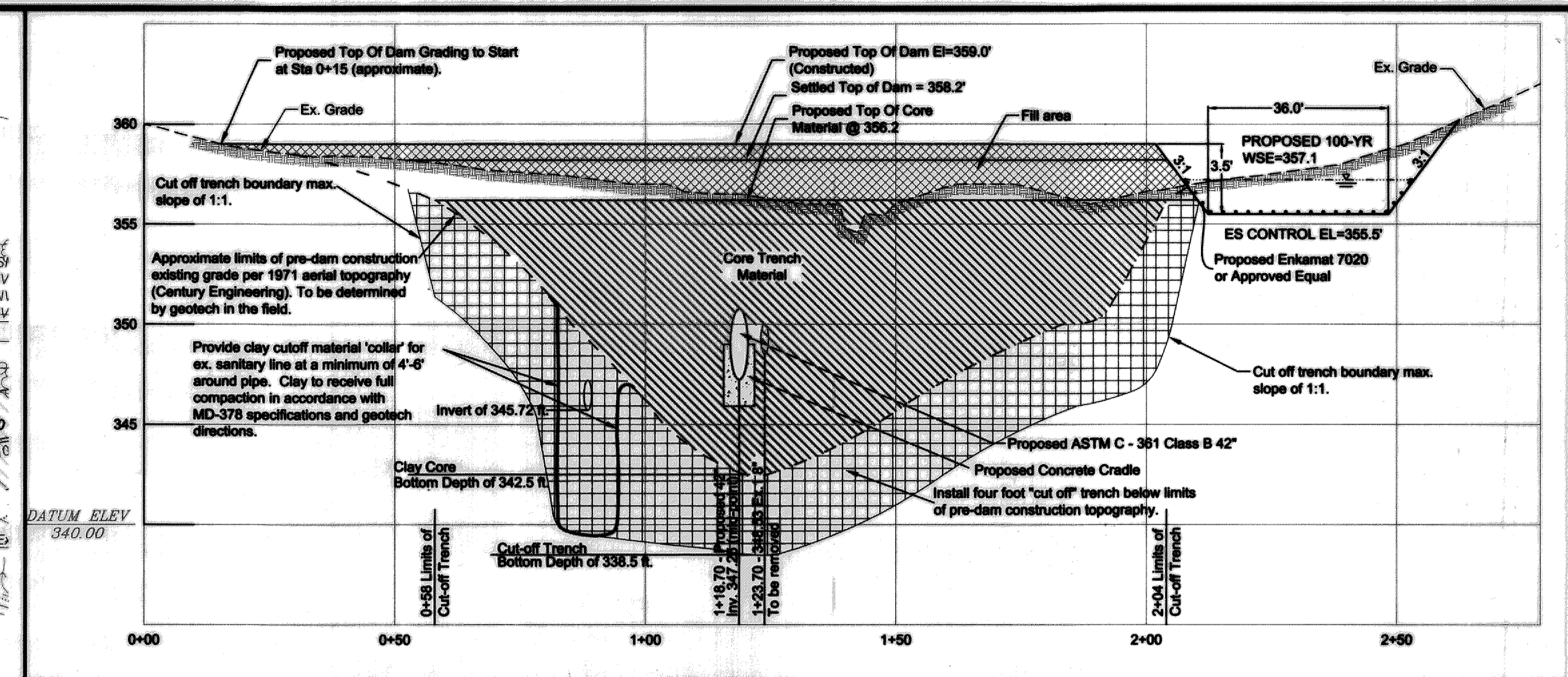
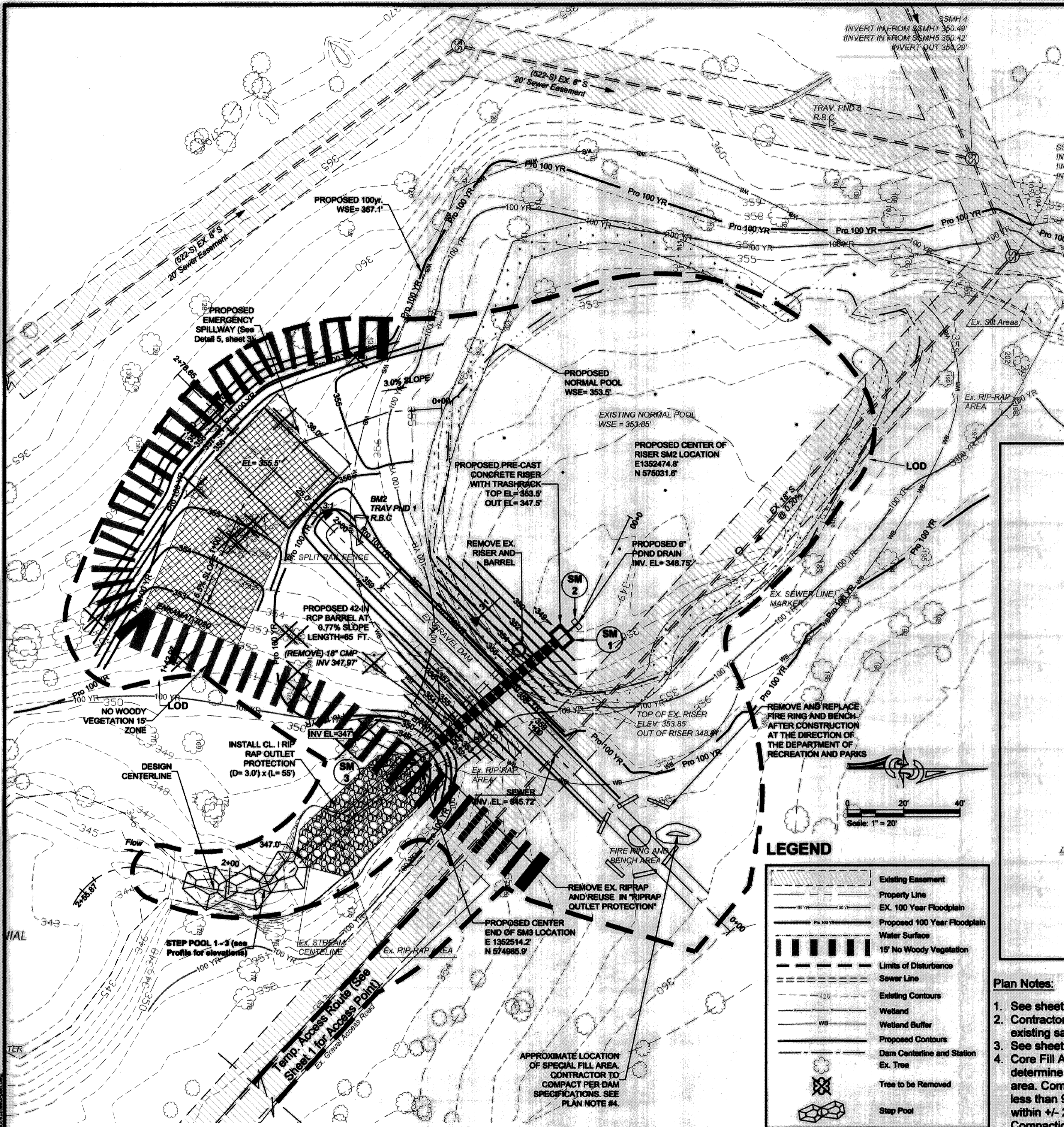
DATE:	DESIGNED:	DRAFTED:	CHECKED:	BASE DATA:	NO.	REVISIONS	BY	DATE
11/2013	TCS	HT/JM	CW	HC				



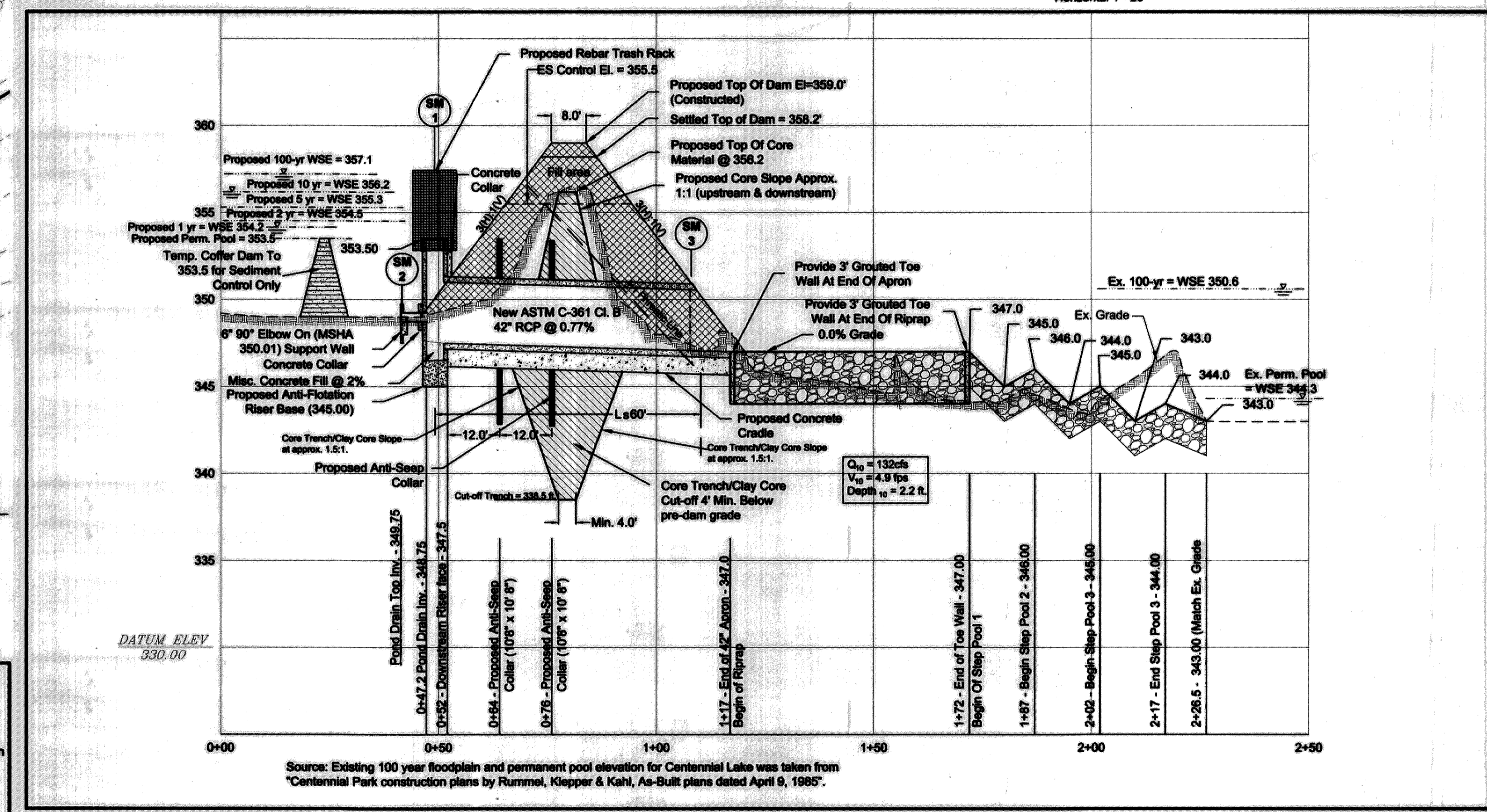
CPJ Associates  
 CPJ Environmental Services Division  
 STREAM RESTORATION • STORMWATER MANAGEMENT • INSPECTION  
 910 CLOFFER ROAD, STE 2618 GAITHERSBURG MARYLAND 20878  
 Phone: (301) 208-9573 E-mail: envcpj@a.com Fax: (301) 208-4551  
 SILVER SPRING, MD FREDERICK, MD FAIRFAX, VA

SCALE AS SHOWN  
 SHEET 1 OF 8 SHEETS  
 JOB NO. 37-556

HAZEN AND SAWYER  
 Environmental Engineers & Scientists  
 4011 WestChase Blvd, Raleigh, North Carolina 27607



1 PROFILE THROUGH TOP OF DAM AND E.S.  
(View Downstream)  
Scale: Vertical 1"=5'  
Horizontal 1"=20'



2 PROFILE THROUGH PRINCIPAL SPILLWAY  
Scale: Vertical 1"=5'  
Horizontal 1"=20'

**LEGEND**

	Existing Easement
	Property Line
	EX. 100 Year Floodplain
	Proposed 100 Year Floodplain
	Water Surface
	15' No Woody Vegetation
	Limits of Disturbance
	Sewer Line
	Existing Contours
	Wetland
	Wetland Buffer
	Proposed Contours
	Dam Centerline and Station
	Ex. Tree
	Tree to be Removed
	Step Pool

- Plan Notes:**
- See sheet 1 for entire LOD with access point and SCE.
  - Contractor to use extreme care when working near the existing sanitary sewer pipe.
  - See sheet 1 for bench mark locations.
  - Core Fill Area: contractor to perform test pitting to determine subsurface conditions and extent of core fill area. Compacted fill to meet dam fill specifications (not less than 95% of max. dry density with moisture content within +/- 2% of optimum; see MD-378 notes under Compaction on Sheet 4).

APPROVED: HC DEPARTMENT OF PUBLIC WORKS  
*Mark Richmond* 12/19/13  
 CHIEF, BUREAU OF ENVIRONMENTAL SERVICES DATE

THESE PLANS FOR SMALL POND CONSTRUCTION SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.

*John P. Robertson* 1/2/14  
 HOWARD SOIL CONSERVATION DISTRICT DATE

**HAZEN AND SAWYER**  
 Environmental Engineers & Scientists  
 4011 WestChase Blvd, Raleigh, North Carolina 27607

HOWARD COUNTY DPW - ENVIRONMENTAL SERVICES  
 6751 COLUMBIA GATEWAY DRIVE, SUITE 514  
 COLUMBIA, MD 21046  
 PHONE: (410) 313-6413  
 ATTN: MARK RICHMOND

9801 Old Annapolis Road  
 HOWARD COUNTY, MD  
 ELECTION DISTRICT 5  
 MAP 30, GRID 7, PARCEL 10

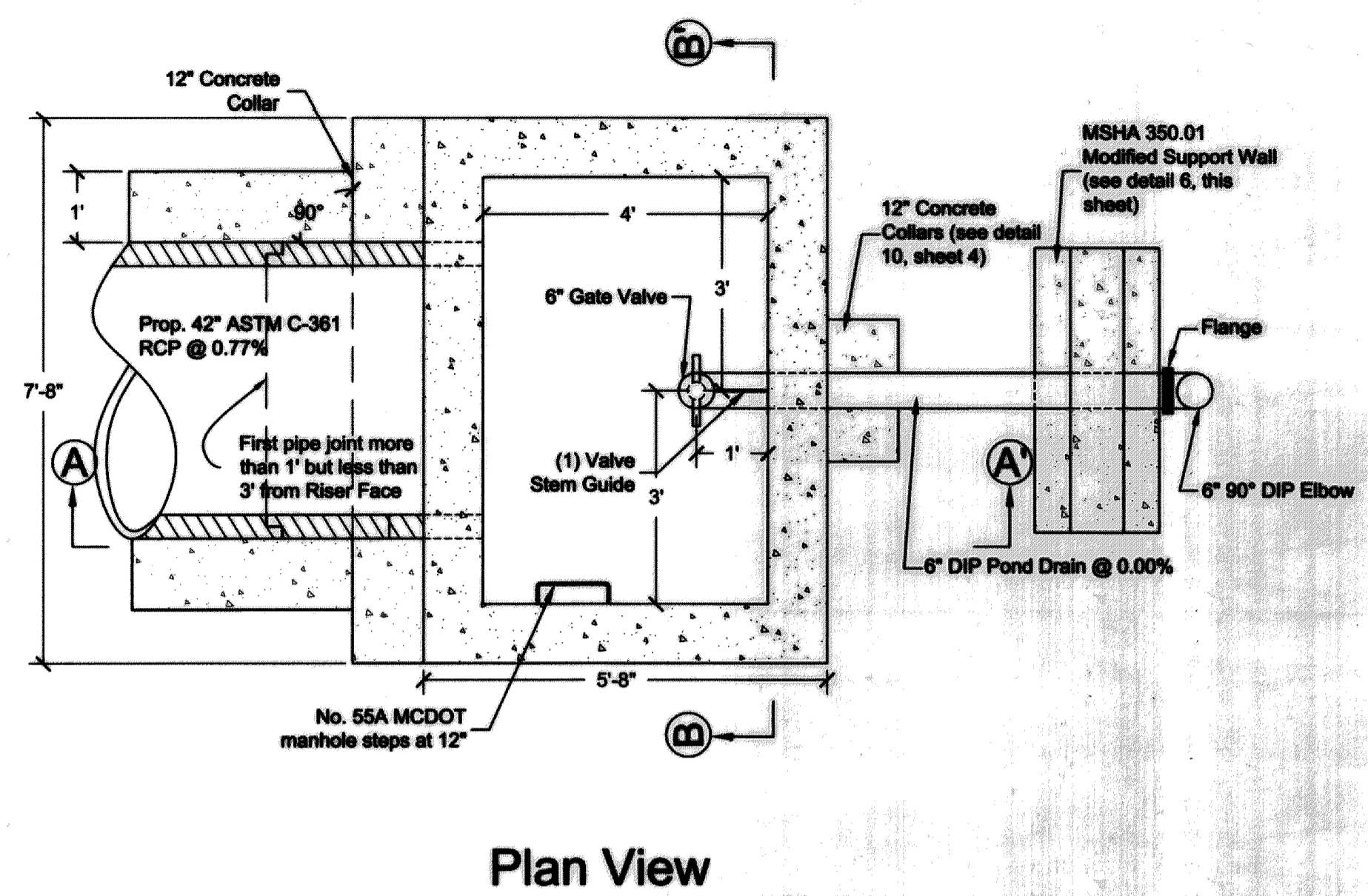
**CENTENNIAL PARK  
 NORTH POND RETROFIT**  
 Howard County Project # N-3962  
 Plan View and Profiles

DATE:	11/2013		
DESIGNED:	TCS		
DRAFTED:	HT/JJM		
CHECKED:	CW		
BASE DATA:	HC		
NO.	REVISIONS	BY	DATE

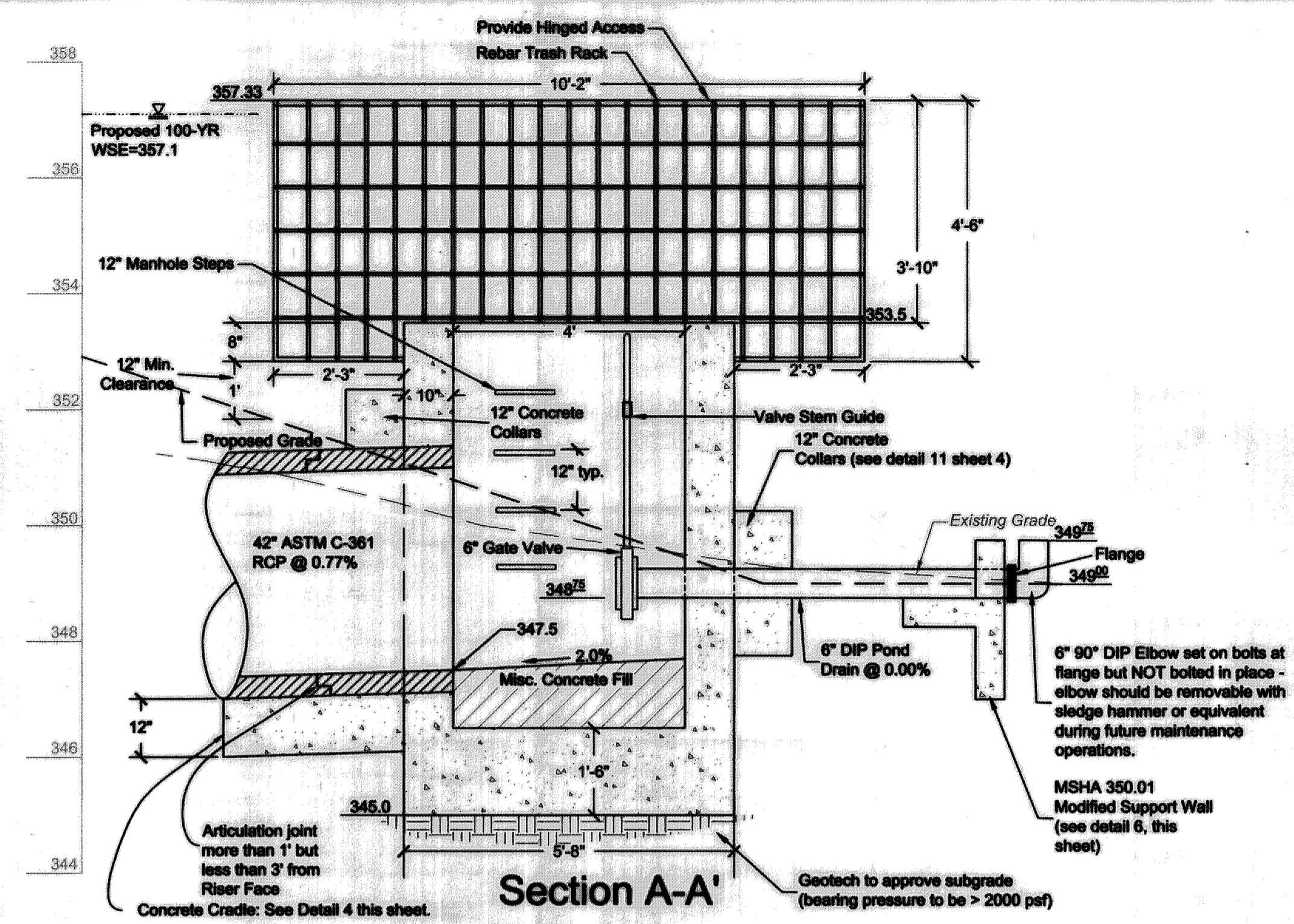


**CPJ Associates**  
 CPJ Environmental Services Division  
 STREAM RESTORATION - STORMWATER MANAGEMENT - INSPECTION  
 910 CLOPPER ROAD, STE 25N GANTHERSBURG MARYLAND 20878  
 Phone: (301) 208-9675 E-mail: cove@cpj.com Fax: (301) 928-4551  
 SILVER SPRING, MD FREDERICK, MD FAIRFAX, VA

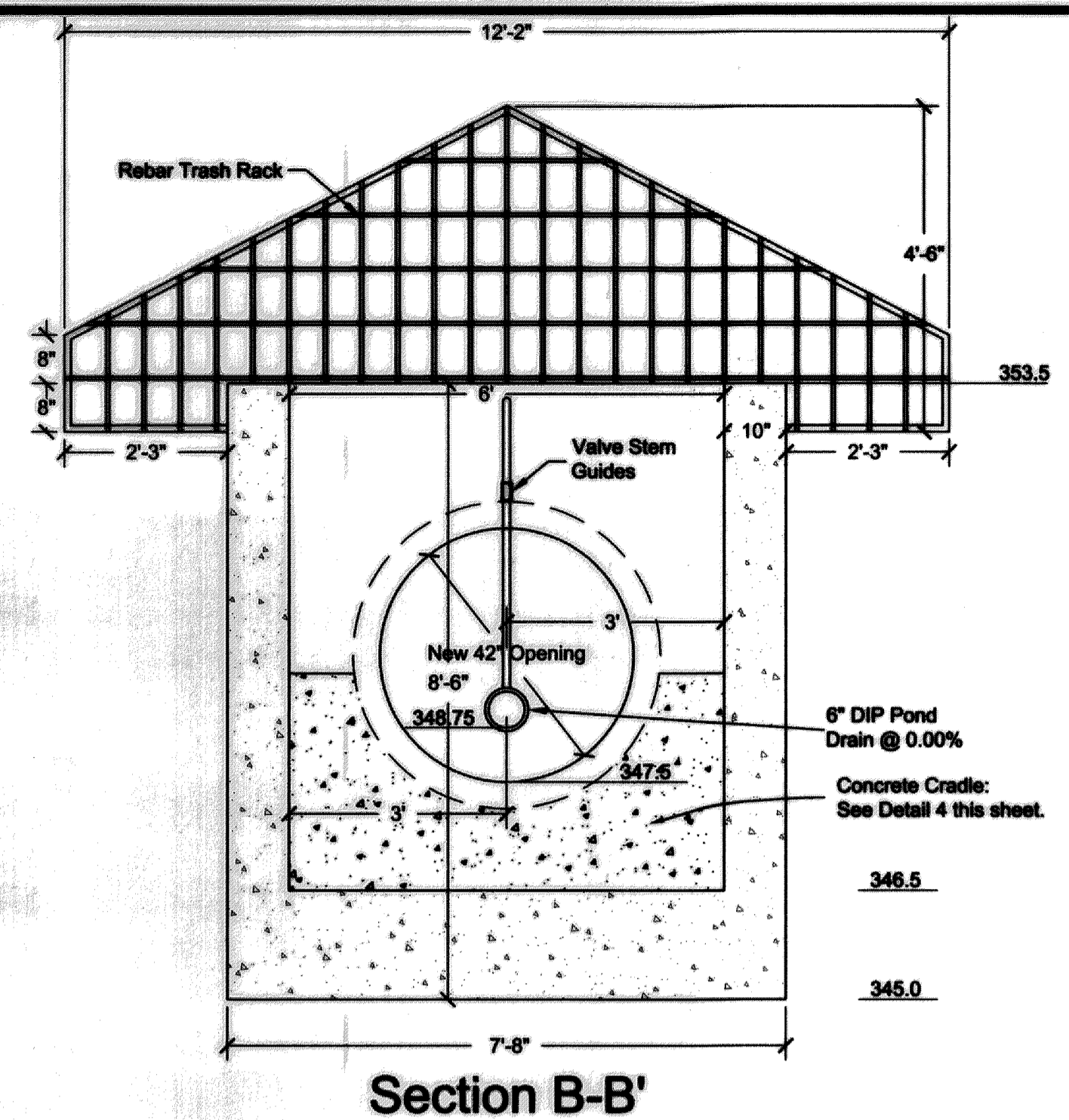
SCALE AS SHOWN  
 SHEET 2 OF 8 SHEETS  
 JOB NO. 37-556



**SM1 - RISER DETAILS**  
 Scale: Vertical 1"=2'  
 Horizontal 1"=2'

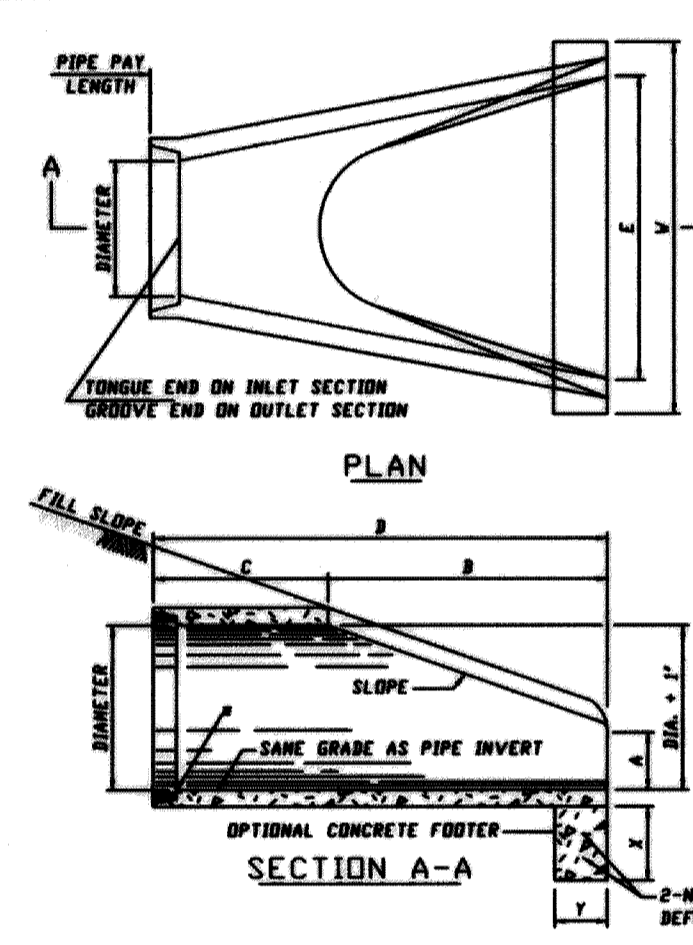


**Section A-A**  
 Geotech to approve subgrade (bearing pressure to be > 2000 psf)



**Section B-B'**

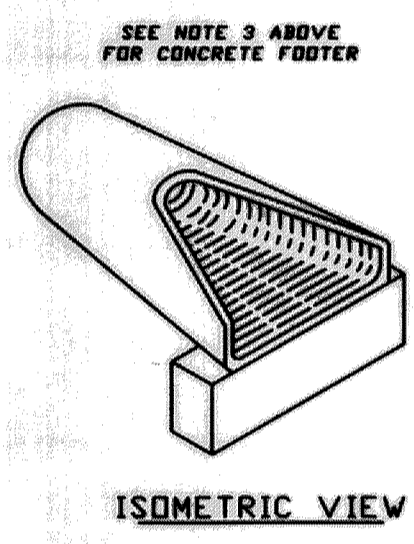
**Notes:**  
 1. If riser is one piece pre-cast, shop drawings will need to be submitted to and approved by Howard County and the Design Engineer.  
 2. If riser is not one piece pre-cast, shop drawings must provide details for joint including watertight and straps will need to be submitted to and approved by Howard County and the Design Engineer. Use of mastic or similarly approved sealant between joints is required.



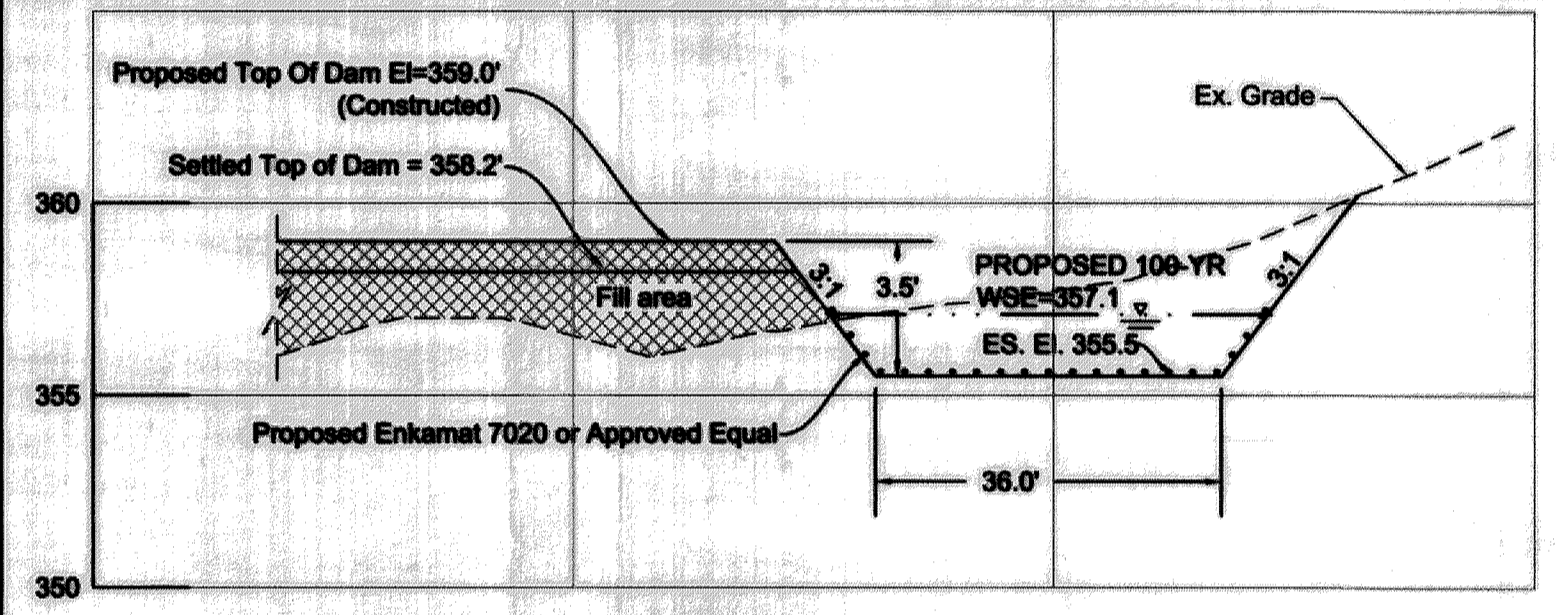
**NOTES**  
 1. CONTRACTOR HAS OPTION OF FURNISHING END SECTIONS CONFORMING TO DETAILS ON THIS SHEET OR END SECTIONS CONFORMING TO DETAILS ON STANDARD NO. 368.01.  
 2. END SECTIONS MUST BE REINFORCED TO CONFORM TO CLASS III PIPE.  
 3. CONCRETE FOOTER SHALL BE USED WHEN SPECIFIED ON THE PLANS. COST OF CONCRETE FOOTER TO BE INCLUDED IN PRICE OF END SECTION. CONCRETE TO BE HDL. HDL. REINFORCEMENT TO BE NO.3 BARS.  
 \* INVERT ELEVATION TO BE AT THE PIPE END OF THE STANDARD END SECTION ELEVATIONS TO BE NOTED ON THE CONSTRUCTION PLANS.

QUANTITIES FOR ESTIMATING PURPOSES ONLY

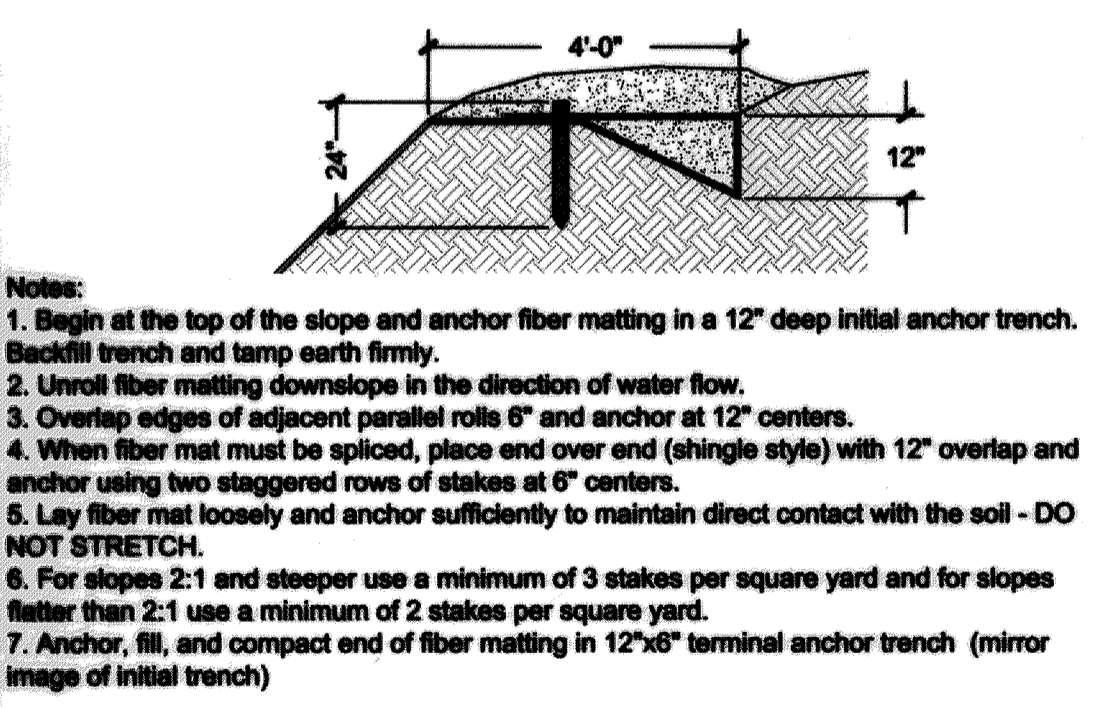
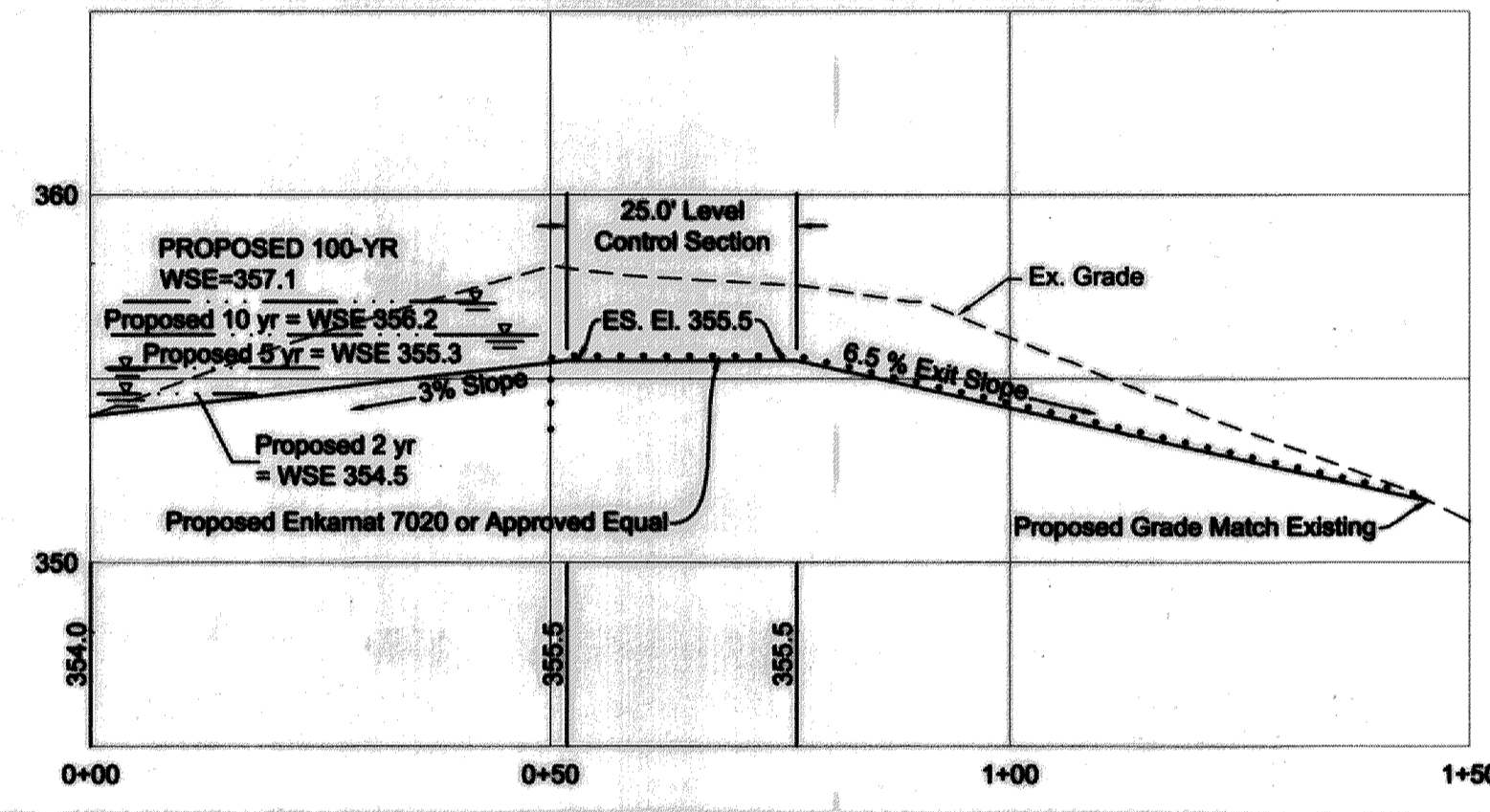
DIA.	SLOPE	CONCRETE END SECTION					CONCRETE FOOTER				
		A	B	C	D	E	V	X	Y	CONC.	STEEL
12"	3:4	4"	2'-8"	4'-0"	6'-0"	2'-0"	3'-0"	12"	9"	0.85	24.00
15"	3:4	5"	3'-0"	4'-6"	6'-6"	2'-0"	3'-0"	12"	9"	0.98	29.50
18"	3:4	6"	2'-6"	4'-0"	6'-0"	2'-0"	3'-0"	12"	9"	1.11	33.00
21"	3:4	7"	2'-0"	3'-6"	6'-0"	2'-0"	3'-0"	12"	9"	1.23	37.50
24"	3:4	8"	1'-6"	3'-0"	6'-0"	2'-0"	3'-0"	12"	9"	1.37	42.00
27"	3:4	10"	1'-0"	2'-6"	6'-0"	2'-0"	3'-0"	12"	9"	1.51	46.50
30"	3:4	12"	0'-6"	2'-0"	6'-0"	2'-0"	3'-0"	12"	9"	1.65	51.00
36"	3:4	1'-0"	0'-0"	1'-6"	6'-0"	2'-0"	3'-0"	12"	9"	1.83	55.50
42"	3:4	1'-6"	0'-0"	1'-0"	6'-0"	2'-0"	3'-0"	12"	9"	2.01	60.00
48"	3:4	2'-0"	0'-0"	0'-0"	6'-0"	2'-0"	3'-0"	12"	9"	2.23	64.50
54"	2:4	2'-6"	0'-0"	0'-0"	6'-0"	2'-0"	3'-0"	12"	9"	2.50	70.00
60"	2:4	3'-0"	0'-0"	0'-0"	6'-0"	2'-0"	3'-0"	12"	9"	2.83	76.50
66"	2:4	3'-6"	0'-0"	0'-0"	6'-0"	2'-0"	3'-0"	12"	9"	3.26	84.00
72"	2:4	4'-0"	0'-0"	0'-0"	6'-0"	2'-0"	3'-0"	12"	9"	3.79	93.75



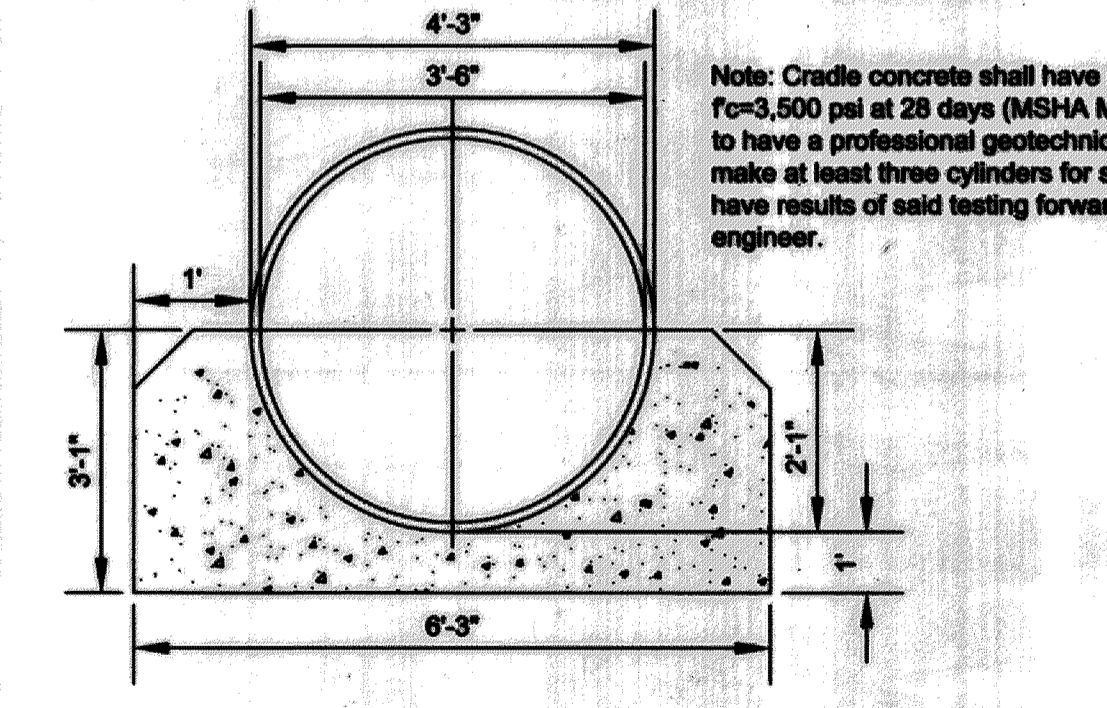
**MD-368.01 CONCRETE END SECTION**  
 Scale: N.T.S.



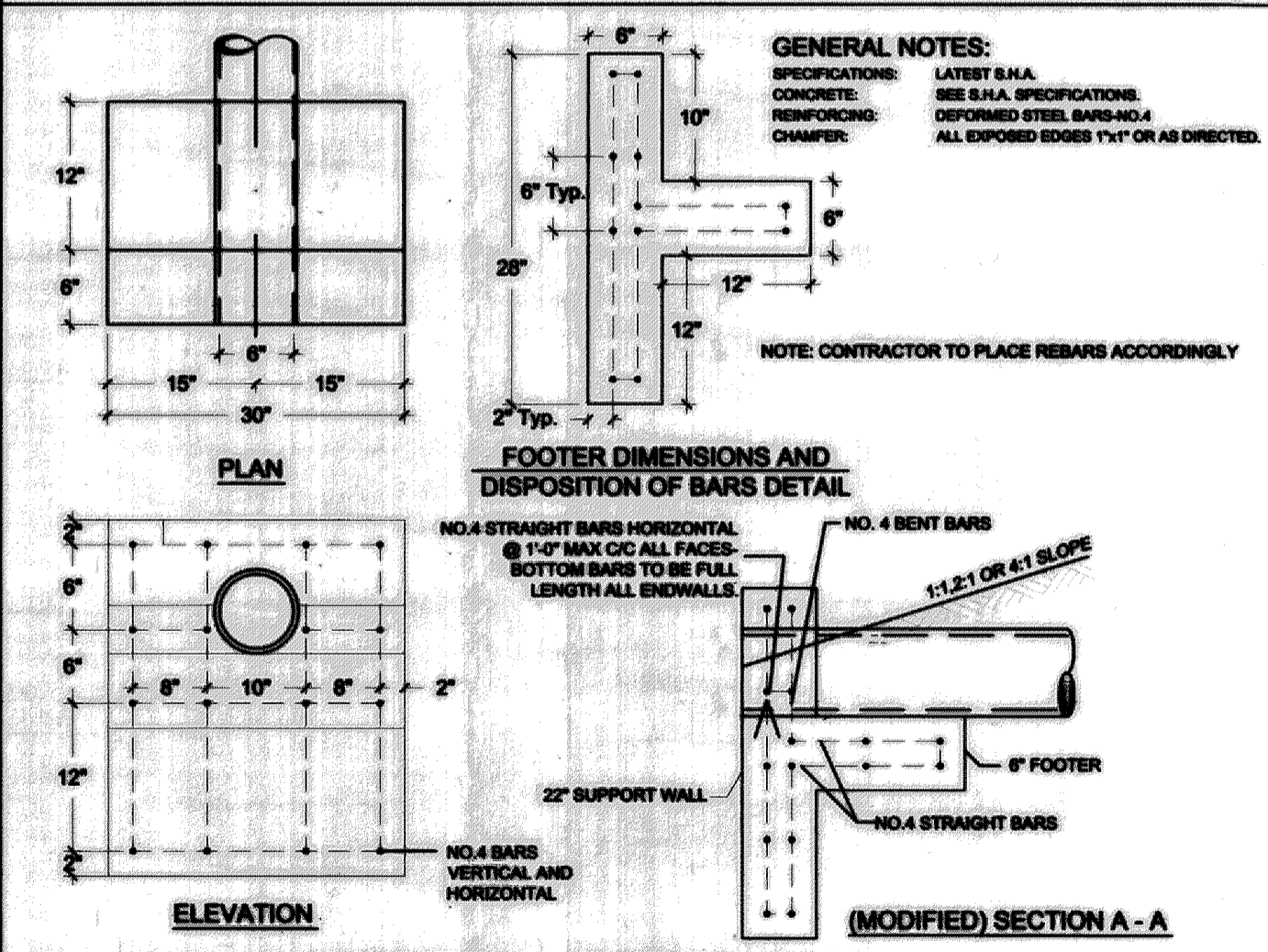
**EMERGENCY SPILLWAY PROFILES**  
 Scale: Vertical 1"=5'  
 Horizontal 1"=20'



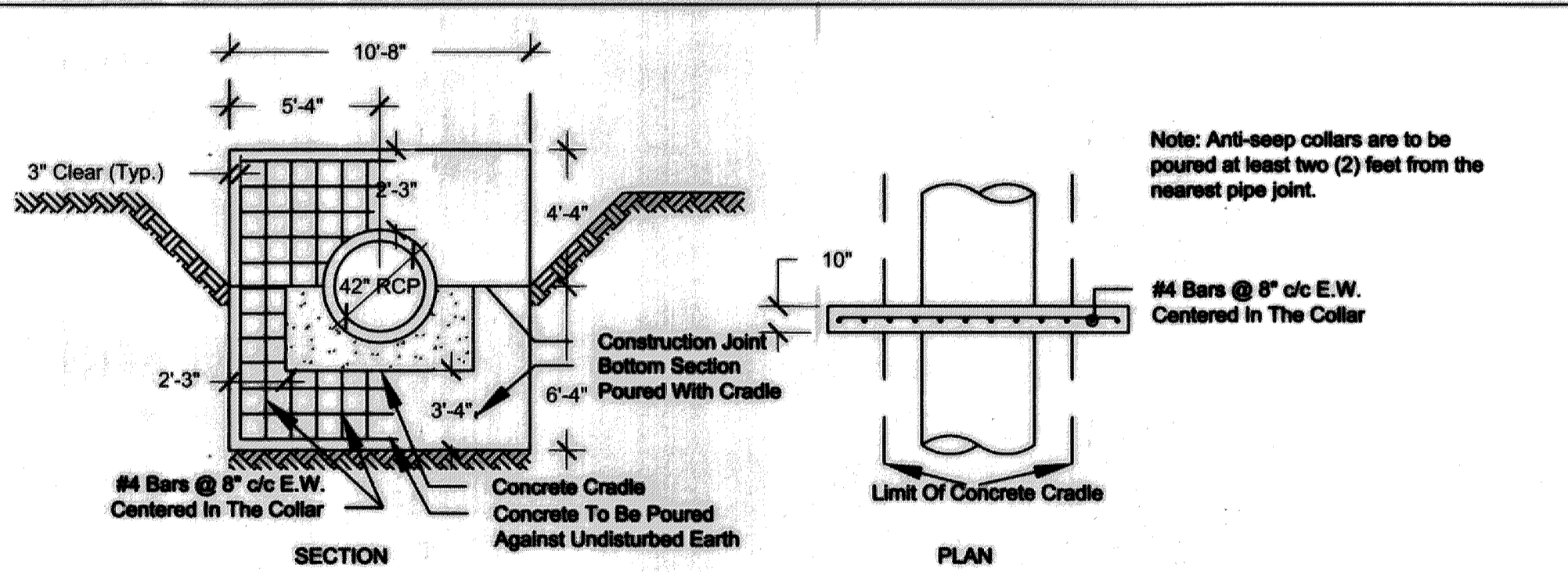
**EROSION CONTROL MAT KEYING**  
 Scale: N.T.S.



**CONCRETE CRADLE**  
 Scale: N.T.S.



**SM2 (MODIFIED) STANDARD END SUPPORT WALL**  
 METAL OR CONCRETE ROUND PIPE  
 STANDARD NO. MD 350.01  
 Scale: N.T.S.



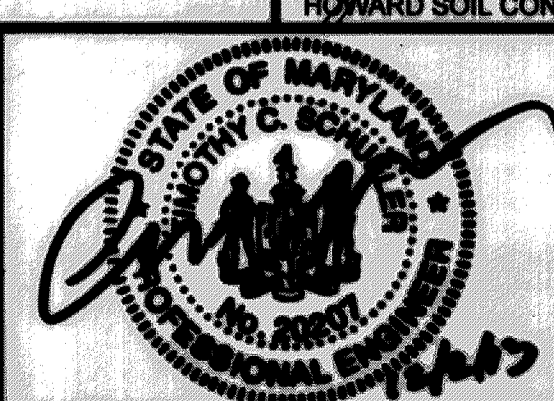
**ANTI-SEEP COLLAR**  
 Scale: N.T.S.

HOWARD COUNTY DPW - ENVIRONMENTAL SERVICES  
 6751 COLUMBIA GATEWAY DRIVE, SUITE 514  
 COLUMBIA, MD 21046  
 PHONE: (410) 313-6413  
 ATTN: MARK RICHMOND

9801 Old Annapolis Road  
 HOWARD COUNTY, MD  
 ELECTION DISTRICT 5  
 MAP 30, GRID 7, PARCEL 10

**CENTENNIAL PARK  
 NORTH POND RETROFIT**  
 Howard County Project # N-3962  
 Details

DATE:	11/2013		
DESIGNED:	TCS		
DRAFTED:	HT/JM		
CHECKED:	CW		
BASE DATA:	HC		
NO.		REVISIONS	BY DATE



**CPJ Associates**  
 CPJ Environmental Services Division  
 STREAM RESTORATION - STORMWATER MANAGEMENT - INSPECTION  
 90 CLOPPER ROAD, STE 259N GAITHERSBURG MARYLAND 20878  
 Phone: (301) 208-9673 E-mail: env@cpj.com Fax: (301) 920-4551  
 SILVER SPRING, MD FREDERICK, MD FAIRFAX, VA

SCALE AS SHOWN  
 SHEET 3 OF 8 SHEETS  
 JOB NO. 37-556

**HAZEN AND SAWYER**  
 Environmental Engineers & Scientists  
 4011 WestChase Blvd, Raleigh, North Carolina 27607

APPROVED: HC DEPARTMENT OF PUBLIC WORKS  
 Mark Richmond 12/19/13  
 CHIEF, BUREAU OF ENVIRONMENTAL SERVICES DATE  
 John P. Roberts 1/2/14  
 HOWARD SOIL CONSERVATION DISTRICT DATE

THESE PLANS FOR SMALL POND CONSTRUCTION SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.

**MD-378 NOTES**

**CONSTRUCTION SPECIFICATIONS**  
 These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent version.

**Site Preparation**  
 Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the toe of the embankment. Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry temporary management ponds, a minimum of a 25-foot radius around the inlet structure shall be cleared. All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

**Earth Fill**  
 The fill material shall be taken from approved designated borrow areas. It shall be free of roots, stumps, wood, rubbish, stones greater than 6" frozen or other objectionable materials. Fill material for the center of the embankment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer. Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required for prevention of erosion of the embankment.

**Placement**  
 Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the embankment. The maximum thickness of any layer shall be 8 inches. The maximum thickness of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment.

**Compaction**  
 The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each fill shall be traversed by not less than one track of heavy equipment or achieved by a minimum of four complete passes of a sheepsfoot, pneumatic tire, or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not so wet that water can be squeezed out. When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within +2% of the optimum. Each layer of fill shall be compacted to the required density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

**Cut Off Trench**  
 The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability.

**Embankment Core**  
 The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed concurrently with the outer shell of the embankment.

**Structure Backfill**  
 Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe. Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi, 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2,000 ohm-cm. Material shall be placed such that a minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over, and on the sides of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24" or greater over the structure or pipe. Backfill material outside the structural backfill (flowable fill) zone shall be of the type and quality conforming to that specified for the core of the embankment or other embankment materials.

**Pipe Conduits**  
 All pipes shall be circular in cross section.

**Corrugated Metal Pipe** - All of the following criteria shall apply for corrugated metal pipe:  
 1. Materials - (Polymer Coated steel pipe) - Steel pipes with polymeric coatings shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 & M-246 with watertight coupling bands or flanges.  
 Materials - (Aluminum Coated Steel Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-274 with watertight coupling bands or flanges.  
 Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound.  
 Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.  
 Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-196 or M-211 with watertight coupling bands or flanges. Aluminum pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be between 4 and 9.  
 2. Coupling bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at least 24 mils in thickness.  
 3. Connections - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.  
 All connections shall use a rubber or neoprene gasket when joining pipe sections.  
 The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the bandwidth. The following pipe connections are acceptable for pipes less than 24 inches in diameter: flanges on both ends of the pipe with a circular 3/8 inch closed cell neoprene gasket, pre-punched to the flange bolt circle, sandwiched between adjacent flanges; a 12-inch wide standard lap type band with 12-inch wide by 3/8-inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with oring gaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by a 24 inch long annular corrugated band using a minimum of 4 (four) rods and lugs, 2 on each connecting pipe end.  
 A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8 inch closed cell gaskets the full width of the flange is also acceptable. Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or a neoprene bead.  
 4. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.  
 5. Backfilling shall conform to "Structure Backfill".  
 6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.  
**Reinforced Concrete Pipe** - All of the following criteria shall apply for reinforced concrete pipe:  
 1. Materials - Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C-361.  
 2. Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding / cradle for their entire length. This bedding / cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in the "Structure Backfill" section of this standard. Gravel bedding is not permitted.  
 3. Laying pipe - Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.  
 4. Backfilling shall conform to "Structure Backfill".  
 5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.  
**Plastic Pipe** - The following criteria shall apply for plastic pipe:  
 1. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" - 10" inch pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" inch shall meet the requirements of AASHTO M294 Type S.  
 2. Joints and connections to anti-seep collars shall be completely watertight.  
 3. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.  
 4. Backfilling shall conform to "Structure Backfill".  
 5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

**Drainage Diaphragms** - When a drainage diaphragm is used, a registered professional engineer will supervise the design and construction inspection.

**Concrete**  
 Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 314, Mix No. 3.

**Rock Riprap**  
 Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 311.  
 Geotextile shall be placed under all riprap and shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.  
**Care of Water Surface Construction**  
 All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect the areas to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the excavations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall be pumped.  
**Stabilization**  
 All borrow areas shall be graded to provide proper drainage and left in a slightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

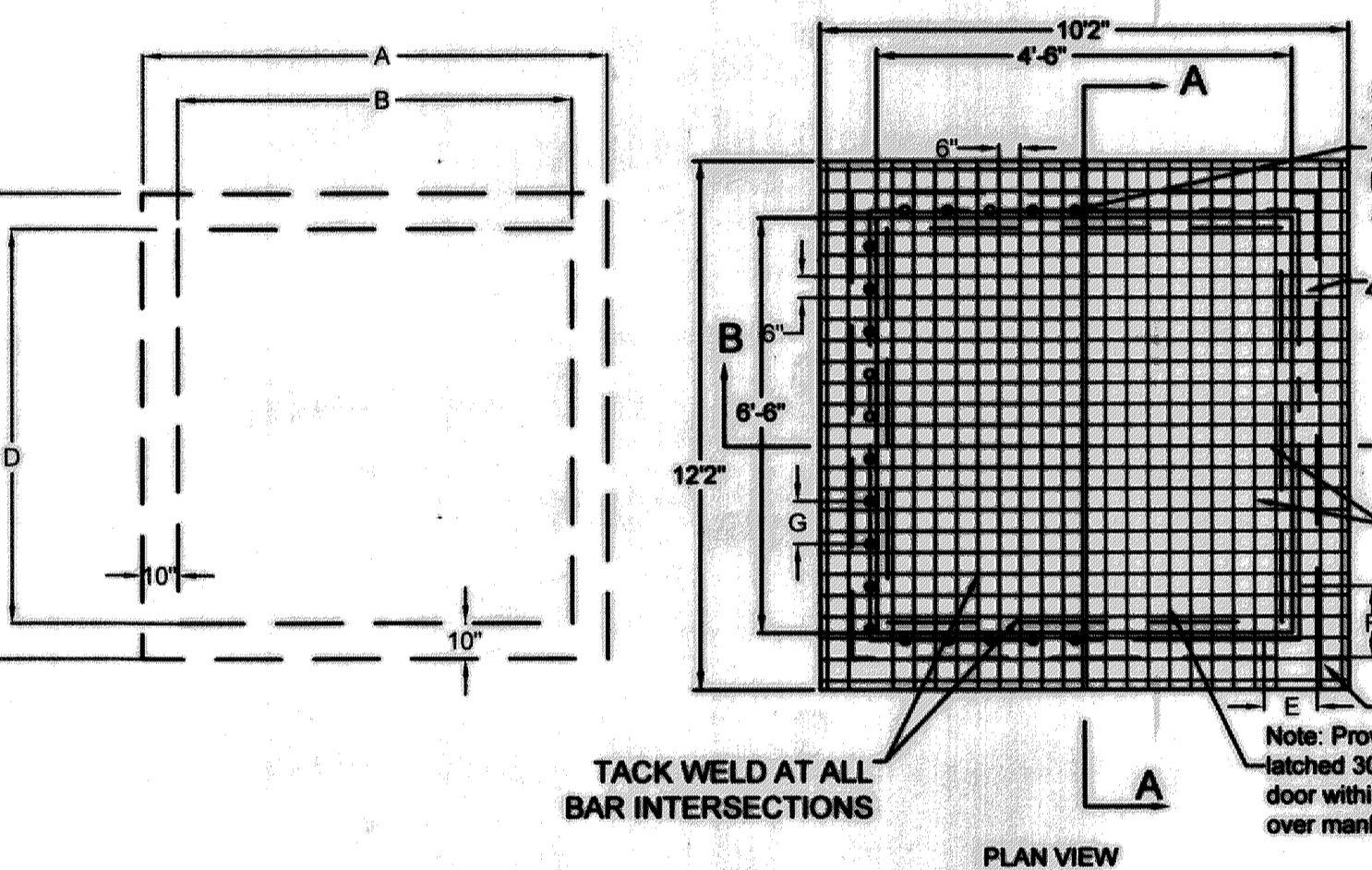
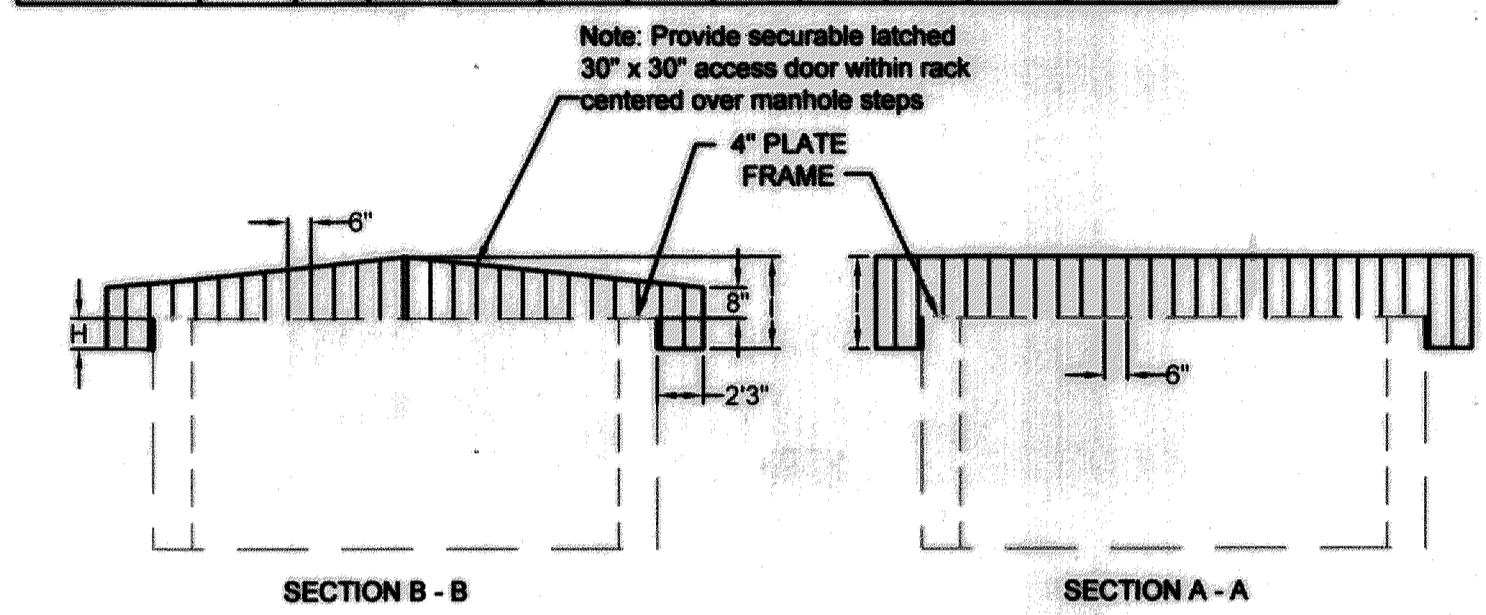
**B-4-8: STANDARDS AND SPECIFICATIONS FOR STOCKPILE AREA**  
 (Source: 2011 Maryland Standards and Specifications for Soil Erosion and Sediment Control)  
**Definition:**  
 A mound or pile of soil protected by appropriately designed erosion and sediment control measures.  
**Purpose:**  
 To provide a designated location for the temporary storage of soil that controls the potential for erosion, sedimentation, and changes to drainage patterns.  
**Conditions Where Practice Applies:**  
 Stockpile areas are utilized when it is necessary to salvage and store soil for later use.  
**Criteria:**  
 1. The stockpile location and all related sediment control practices must be clearly indicated on the erosion and sediment control plan.  
 2. The footprint of the stockpile must be sized to accommodate the anticipated volume of material and based on a side slope no steeper than 2:1. Benching must be provided in accordance with Section B-3 Land Grading.  
 3. Runoff from the stockpile area must drain to a suitable sediment control practice.  
 4. Access the stockpile area from the upgrade side.  
 5. Clear water runoff into the stockpile area must be minimized by use of a diversion device such as an earth dike, temporary swale or diversion fence. Provisions must be made for discharging concentrated flow in a non-erosive manner.  
 6. Where runoff concentrates along the toe of the stockpile fill, an appropriate erosion/sediment control practice must be used to intercept the discharge.  
 7. Stockpiles must be stabilized in accordance with the 3/7 day stabilization requirement as well as Standard B-4-1 Incremental Stabilization and Standard B-4-4 Temporary Stabilization.  
 8. If the stockpile is located on an impervious surface, a liner should be provided below the stockpile to facilitate cleanup. Stockpiles containing contaminated material must be covered with impermeable sheeting.  
**Maintenance:**  
 The stockpile area must continuously meet the requirements for Adequate Vegetative Establishment in accordance with Section B-4 Vegetative Stabilization. Side slopes must be maintained at no steeper than a 2:1 ratio. The stockpile area must be kept free of erosion. If the vertical height of a stockpile exceeds 20 feet for 2:1 slopes, 30 feet for 3:1 slopes, or 40 feet for 4:1 slopes, benching must be provided in accordance with Section B-3 Land Grading.

**SPECIFICATIONS**

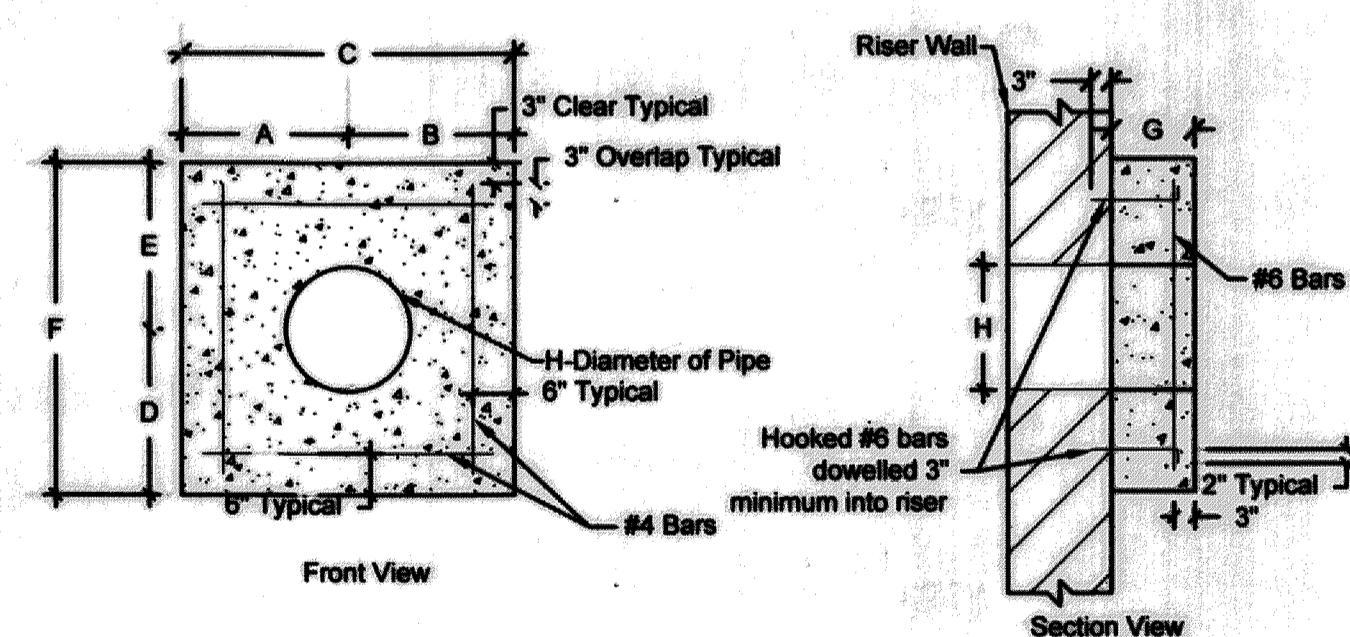
- 1.0 ROCKSTONE PLACEMENT FOR RIP RAP CHANNELS AND STEP POOLS**
- 1.1 All stone shall be from quarries within 50 miles of site. Stone shall be angular, similar in color, texture and density to the native rock onsite. The dry unit weight of the stone shall be 160 lb / cu ft or greater. Concrete shall not be acceptable.
  - 1.2 The contractor shall supply to the design engineer certification from the source quarry that the stone meets (1) the proper rock classification, (2) weight per cubic foot, and (3) sizing and quantities as detailed below.
  - 1.3 Placement for rock toe protection and/or grade control: Stones to be uniformly placed (non-segregated) with large stones (D<sub>50</sub> = 16 inches) buried or "push-placed" at least one-half their diameter. Place on erosion control matting/geotextile as specified herein. Stones shall not prohibit or retard flow over what is called for on plans. Voids shall be chinked with smaller select stone (D<sub>50</sub> = 2"). Placement tolerance shall be +/- three (3) inches for grade checks at center, beginning and end of step pool.
  - 1.4 If excavation is required for placement, follow excavation specifications. No excavation is required if base material is solid or decomposing bedrock. However, if design grades and tolerances cannot be achieved, immediately contact the Design Engineer for a possible field modification.
  - 1.5 If fill is required for placement, follow suitable backfill specifications. The transition of layers (from bottom to top): sub base (scarified if not bedrock), compacted suitable subgrade, filter cloth, transitional gravel, cobble layer, (Tensar BX1100 or approved equivalent), then stone.

**FABRICATED TRASH RACK DETAIL**  
**TRASH RACK DETAIL**  
**TRASH RACK DETAIL**  
**TRASH RACK DETAIL**

LOCATION	QTY	"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H"	BAR SPACING	NOTES
SM1	1	10'-2"	8'	4'-6"	4'-0"	12'-2"	6'-0"	2'-6"	2'-6"	1'-0"	6" OC



**10 TRASH RACK DETAIL N.T.S.**



**Concrete Collar Dimension Schedule**

Location	"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H"	Notes
42" RCP	27"	27"	54"	27"	27"	54"	12"	42"	
6" DIP	6"	6"	12"	6"	6"	12"	12"	6"	
6" DIP	6"	6"	12"	6"	6"	12"	12"	6"	

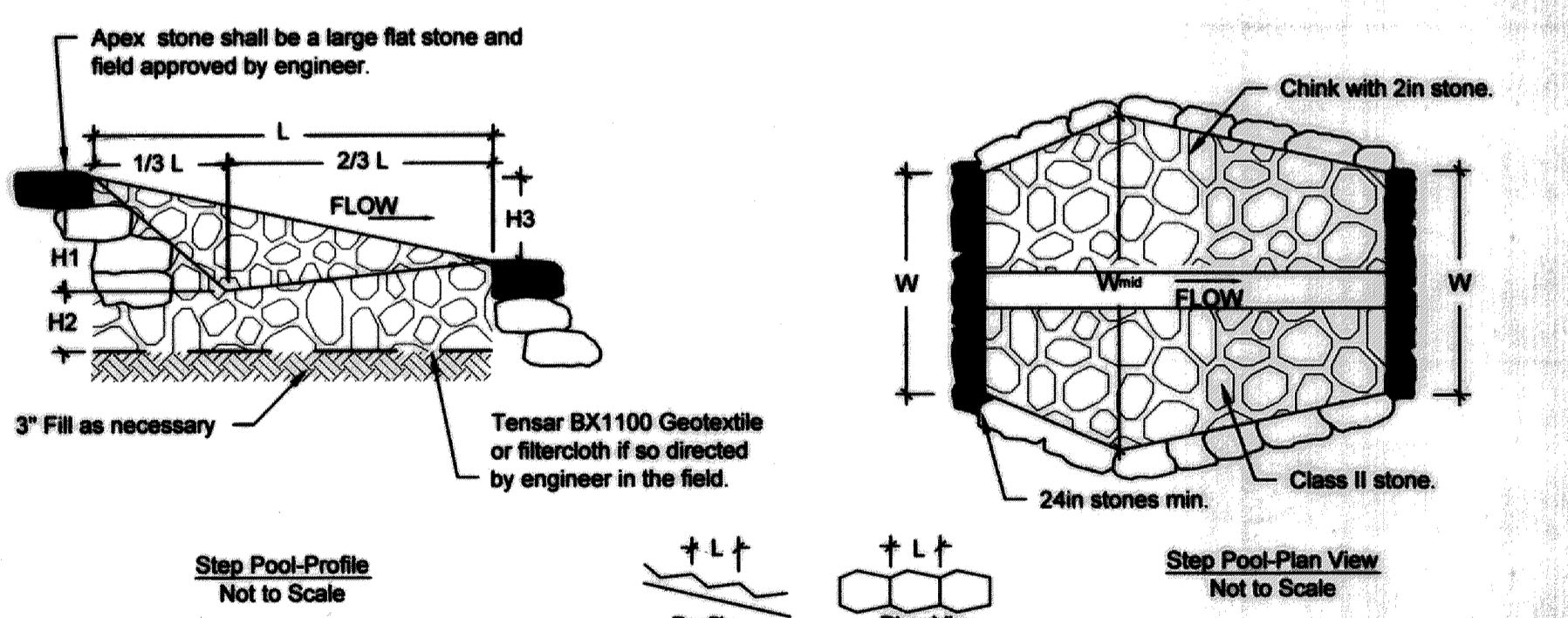
Use MSHA mix # 3 concrete (f<sub>c</sub> = 3500 psi)

**11 CONCRETE COLLAR DETAIL N.T.S.**

APPROVED: HC DEPARTMENT OF PUBLIC WORKS  
 [Signature] 12/19/13  
 CHIEF, BUREAU OF ENVIRONMENTAL SERVICES DATE

THESE PLANS FOR SMALL POND CONSTRUCTION SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.  
 [Signature] 1/2/14  
 HOWARD SOIL CONSERVATION DISTRICT DATE

**HAZEN AND SAWYER**  
 Environmental Engineers & Scientists  
 4011 WestChase Blvd, Raleigh, North Carolina 27607



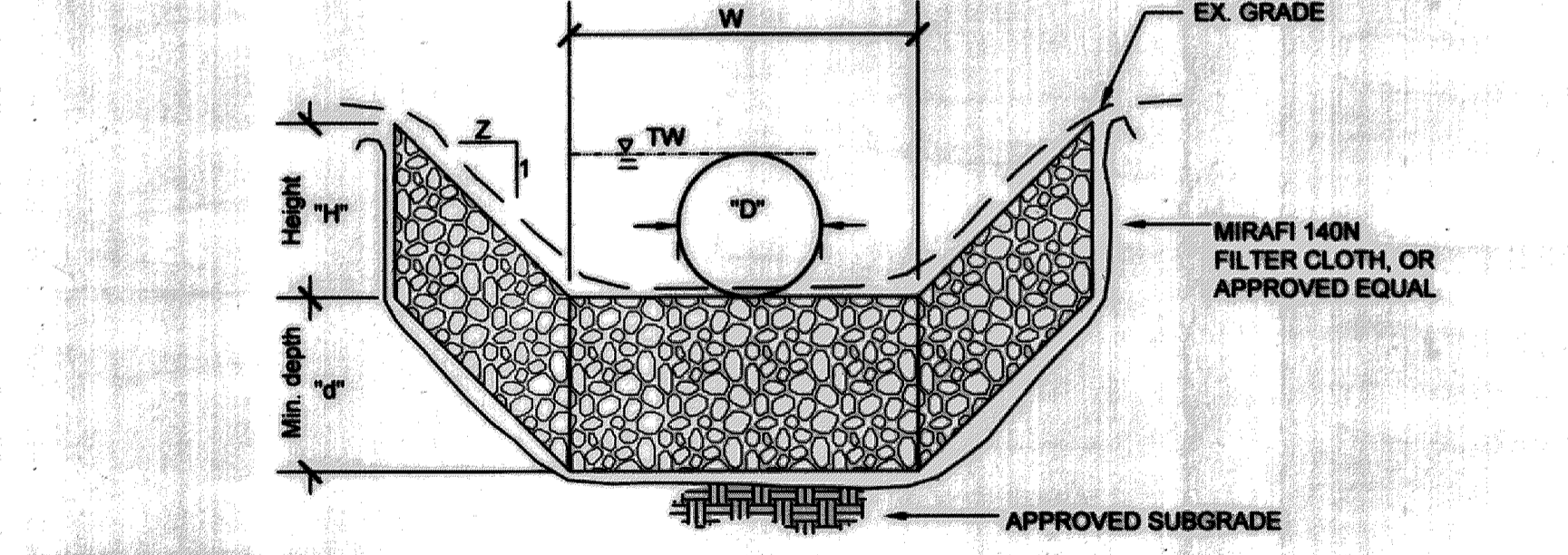
**STEP POOL PROFILE**  
 Not to Scale

**Step Pools**

Station	L	W	W <sub>ms</sub>	H <sub>1</sub>	H <sub>2</sub>	H <sub>3</sub>
1+72	15	6	16	2.0	1.0	1.0
1+87	15	6	16	2.0	1.0	1.0
2+02	15	6	16	2.0	1.0	1.0
2+17	15	6	16	2.0	1.0	1.0

NOTES:  
 1. D<sub>50</sub> = 16" (MSHA II). Chink with D<sub>50</sub> = 2" stone.  
 2. Field tolerance for field heights (H<sub>1</sub>, H<sub>2</sub>) to be 2" +/-.  
 3. Stone to be replaced by hand or with small machinery.

**8 STEP POOL DETAIL N.T.S.**



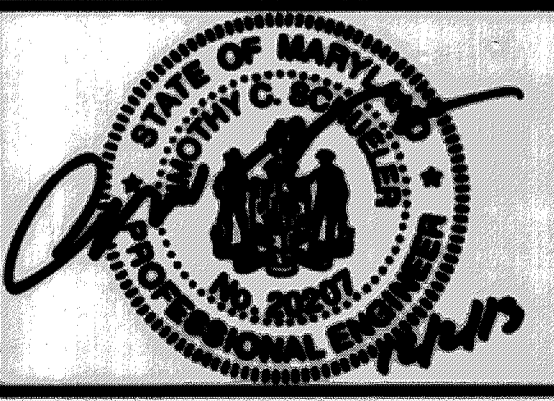
**RIPRAP OUTFALL TABLE**

Location	Pipe Diam., "D"	Invert of pipe	Tailwater "TW" Q10	Flow "Q"	Velocity "V"	Type of riprap	D <sub>50</sub>	D <sub>100</sub>	Length of Riprap	Slope "Z"	Width at end of Pipe	Width at end of Riprap "W"	H	d
SWMS	42	347.0	349.2	132	4.9	MSHA I	12	18	55	3	8	6	Varies	3.0

**9 RIP-RAP OUTFALL SECTION NOT TO SCALE**

**CENTENNIAL PARK NORTH POND RETROFIT**  
 Howard County Project # N-3962  
 NOTES AND DETAILS

DATE:	11/2013				
DESIGNED:	TCS				
DRAFTED:	HT/JM				
CHECKED:	CW				
BASE DATA:	HC				
NO.		REVISIONS		BY	DATE

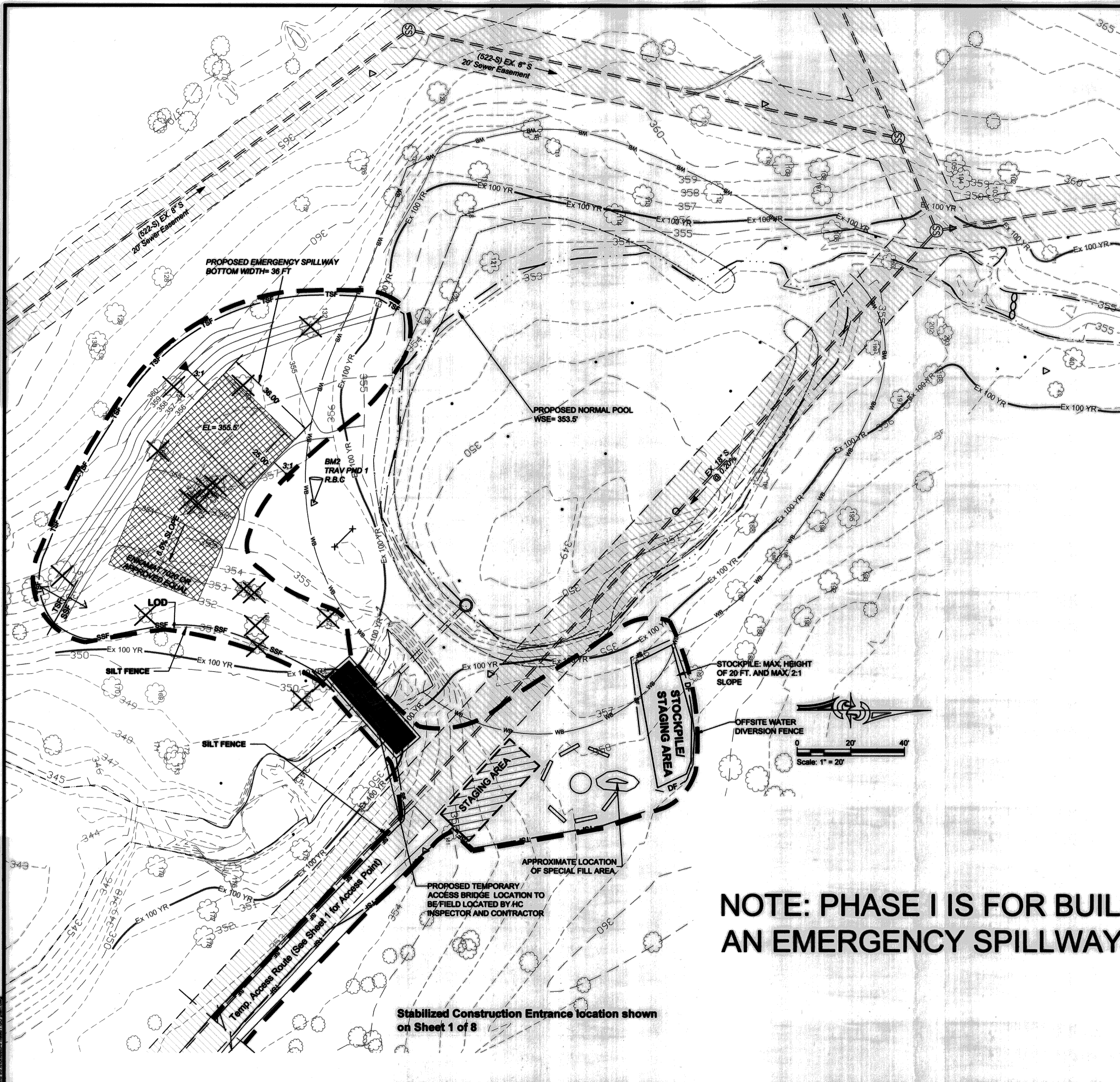


**CPJ Associates**  
 CPJ Environmental Services Division  
 STREAM RESTORATION • STORMWATER MANAGEMENT • INSPECTION  
 90 CLOFFER ROAD, STE 265N GAITHERSBURG MARYLAND 20878  
 Phone: (301) 208-9573 E-mail: cove@cpj.com Fax: (301) 208-4551  
 SILVER SPRING, MD FREDERICK, MD FAIRFAX, VA

SCALE AS SHOWN  
 SHEET 4 OF 8 SHEETS  
 JOB NO. 37-556

HOWARD COUNTY DPW - ENVIRONMENTAL SERVICES  
 6751 COLUMBIA GATEWAY DRIVE, SUITE 514  
 COLUMBIA, MD 21046  
 PHONE: (410) 313-6413  
 ATTN: MARK RICHMOND

9801 Old Annapolis Road  
 HOWARD COUNTY, MD  
 ELECTION DISTRICT 5  
 MAP 30, GRID 7, PARCEL 10



**LEGEND**

	Easement
	Property Line
	EX 100 YR Floodplain
	Water Surface
	Limits of Disturbance
	Sewer Line
	Existing Contours
	Wetland
	Wetland Buffer
	Proposed Contours
	Ex. Tree
	Tree to be Removed
	Diversion Fence
	Super Silt Fence (Detail 11, Sheet 8)
	Silt Fence (Detail 2, Sheet 7)
	Tree Save Fence (Detail 3, Sheet 7)
	Temporary Access Bridge (Detail 7, Sheet 7)

**Centennial Pond North - PHASE I Sequence of Construction**

1. Obtain Howard County grading permit prior to construction. The MDE Wetland and Waterways tracking permit number for this project is #2010081252 dated October 28, 2010; permit expires October 2013. There can be no in-stream construction between March 1st and May 31st of any year. Obtain MDE Permit for Stormwater Associated with Construction Activity prior to the start of any construction activity. 1 day
  2. All erosion and sediment control practices shall be installed and maintained according to the criteria contained in the most current version (2011) of the Maryland Standards and Specifications for Soil Erosion and Sediment Control. 1 day
  3. All construction shall be in accordance with the latest standards and specifications of Howard County plus MSHA Standards and specifications if applicable. The contractor shall notify "MISS Utility" at 1-800-257-7777 at least 48 hours prior to any excavation work being done. 1 day
  4. Prior to clearing of trees, installing sediment control measures, or grading, the Contractor will schedule a pre-construction meeting on-site with the Sediment Control Inspector, the County Project Manager, Parks Project Manager, and the Design Engineer. All parties require 48 hour notice. 1 day
  5. With Sediment Control Inspector's permission, install stabilized construction entrance. 1 day
  6. Clear and grub for installation of sediment control features. 2 days
  7. Install silt fence, pedestrian fence and tree protection measures as shown on plan. Development activities shall not impair any drainage, create an erosion hazard, or create a source of sediment to any adjacent watercourse, wetland, or property. Maximum height of stockpile shall be 20 ft.; maximum stockpile slope shall be 2:1 (horizontal to vertical) per Detail B-3-1 of 2011 Soil Erosion & SC Standards. (For other stockpile specifications, see Spec B-4-8 on Sheet 6). 2 days
  8. Install temporary construction access bridge per detail 1 this sheet. 1 day
  9. Grade emergency spillway and install Enkamat per plan. 5 days
  10. Provide written grade check to Design Engineer indicating all spot elevations are within tolerance. 1 day
  11. Permanently stabilize all newly graded area including disturbed areas per permanent seeding specifications. 1 day
  12. With permission of the Sediment Control Inspector, remove any remaining sediment control features and stabilize all areas disturbed by this process. 2 days
- Total 17 days

**NOTE: PHASE I IS FOR BUILDING AN EMERGENCY SPILLWAY ONLY**

APPROVED: HC DEPARTMENT OF PUBLIC WORKS  
*Mark Richmond* 12/19/13  
 CHIEF, BUREAU OF ENVIRONMENTAL SERVICES DATE

THESE PLANS FOR SMALL POND CONSTRUCTION SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.  
*John K. Neuberger* 1/2/14  
 HOWARD SOIL CONSERVATION DISTRICT DATE

**HAZEN AND SAWYER**  
 Environmental Engineers & Scientists  
 4011 West Chase Blvd., Raleigh, North Carolina 27607

HOWARD COUNTY DPW - ENVIRONMENTAL SERVICES  
 6751 COLUMBIA GATEWAY DRIVE, SUITE 514  
 COLUMBIA, MD 21046  
 PHONE: (410) 313-6413  
 ATTN: MARK RICHMOND

9801 Old Annapolis Road  
 HOWARD COUNTY, MD  
 ELECTION DISTRICT 5  
 MAP 30, GRID 7, PARCEL 10

**CENTENNIAL PARK  
 NORTH POND RETROFIT**  
 Howard County Project # N-3962  
**PHASE I - Sediment Control**

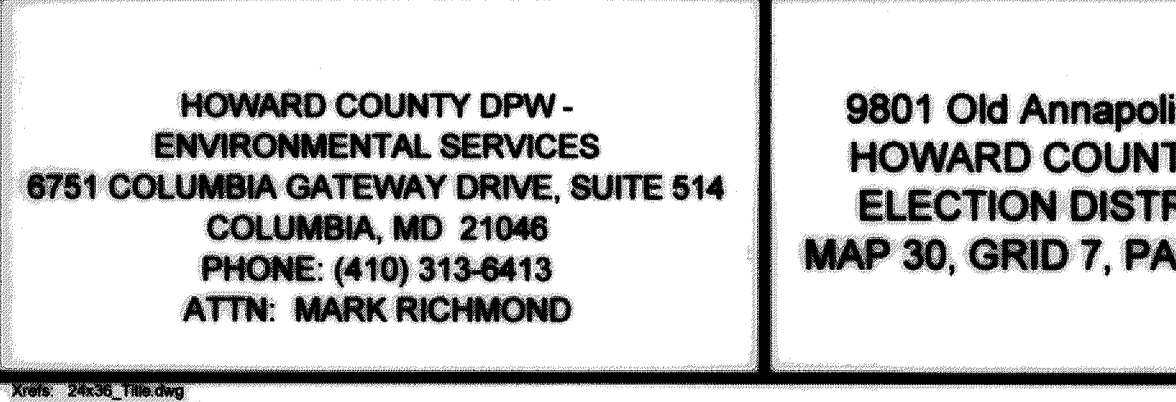
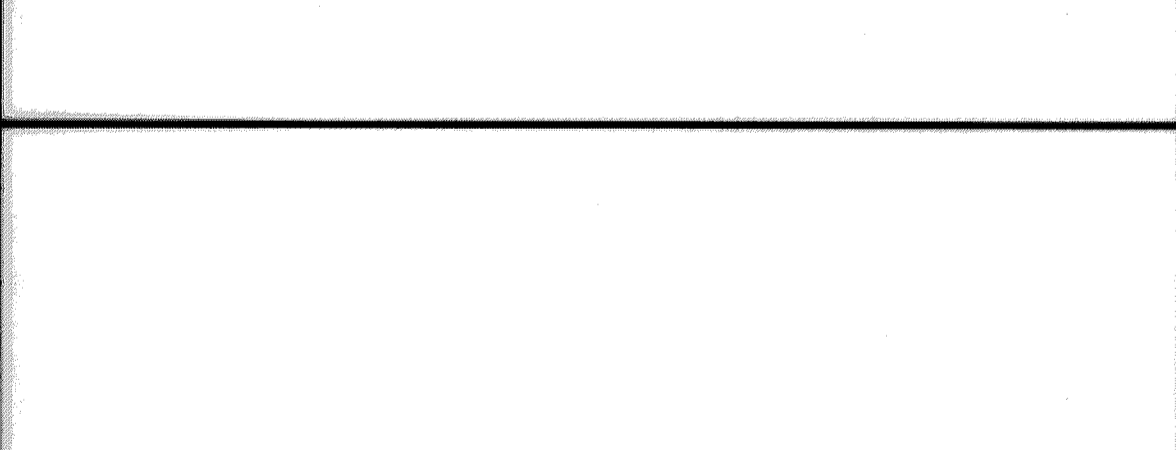
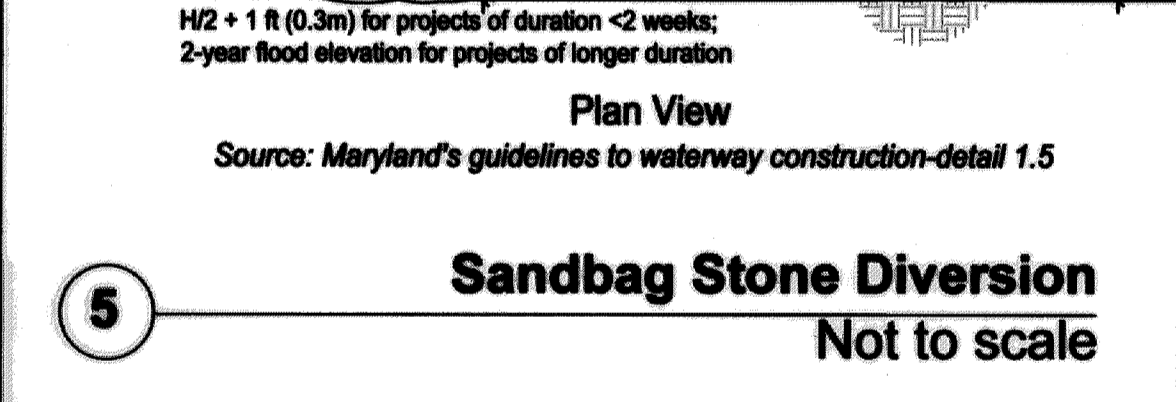
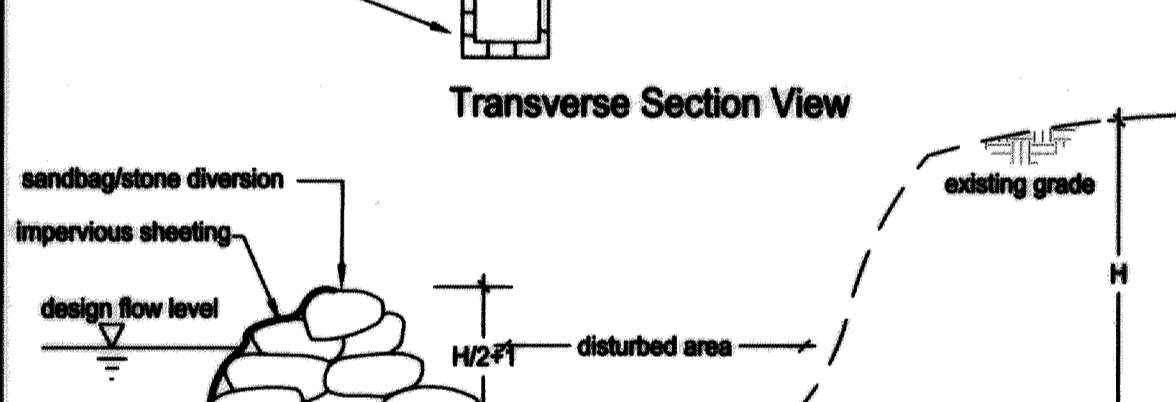
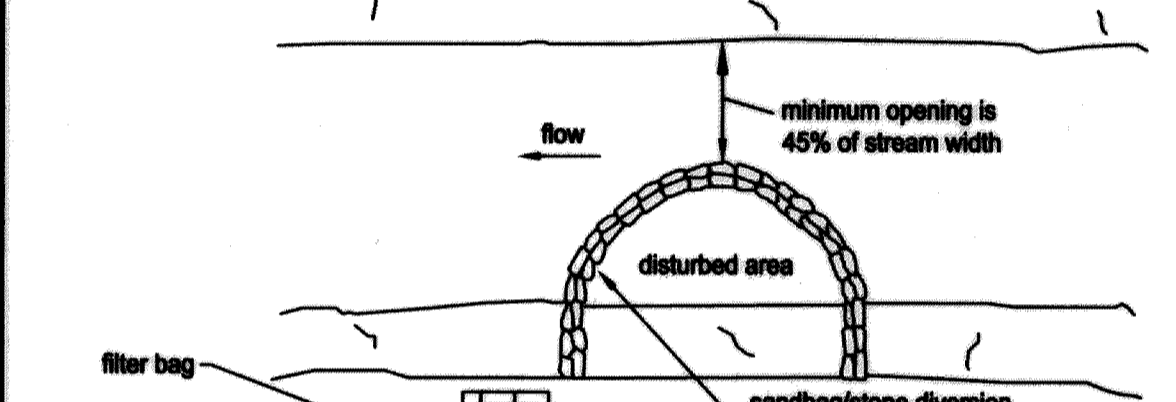
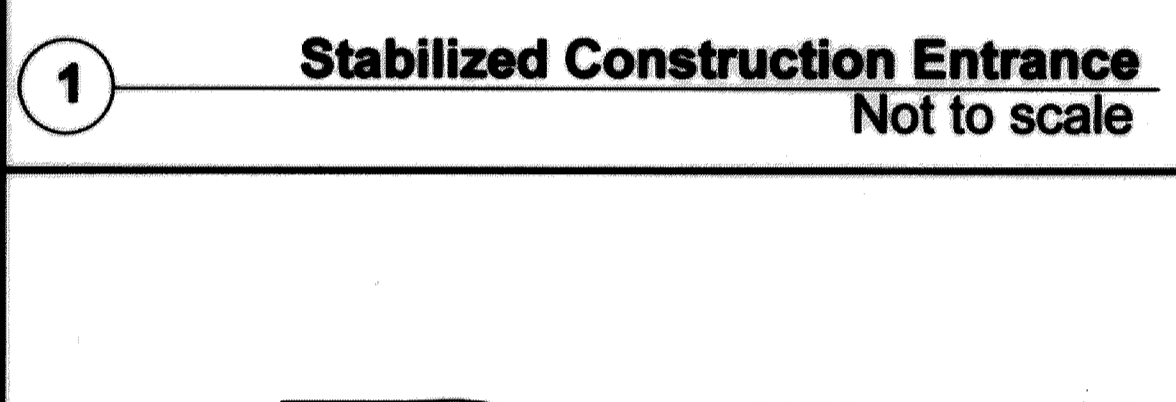
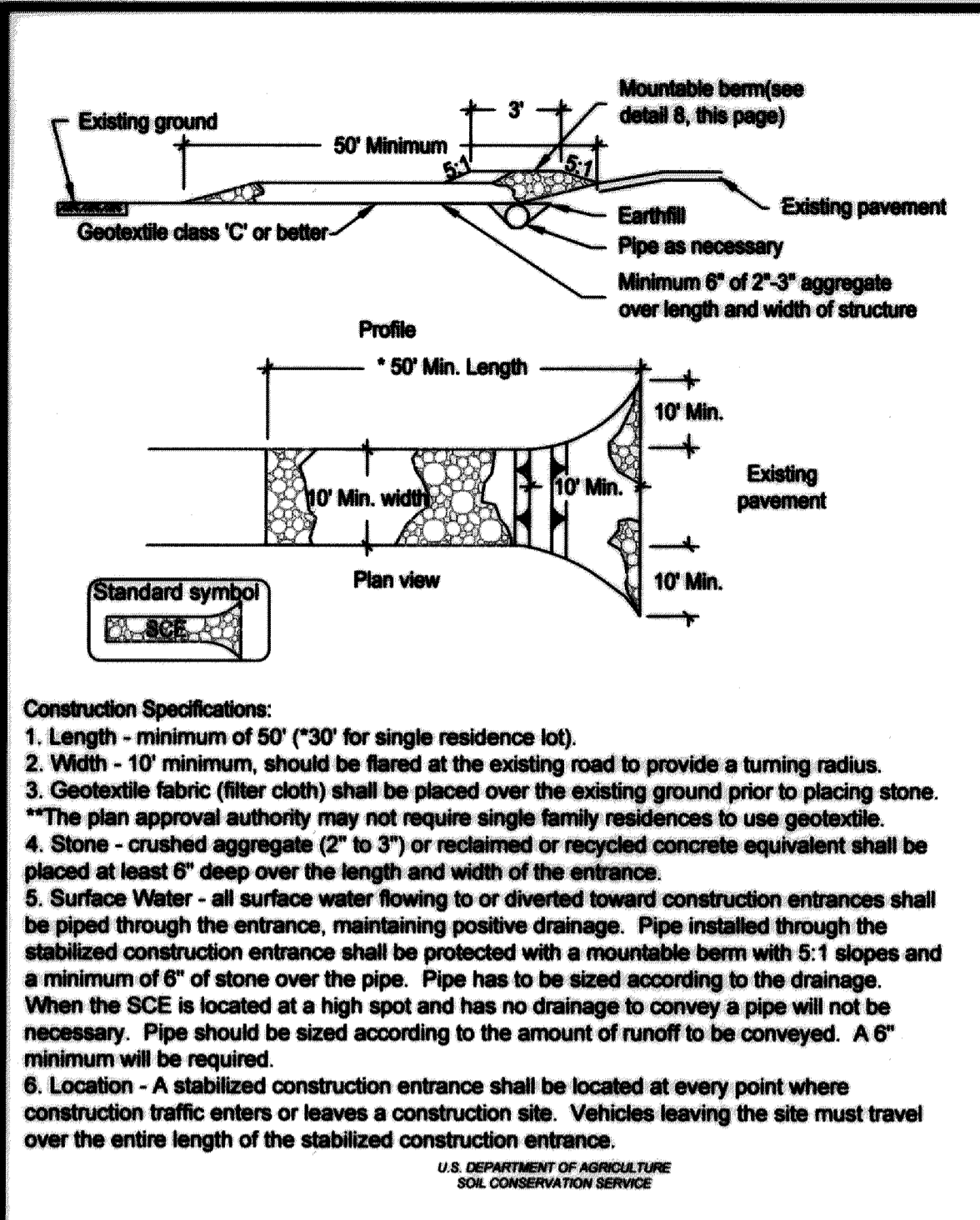
DATE:	11/2013				
DESIGNED:	TCS				
DRAFTED:	HT/JM				
CHECKED:	CW				
BASE DATA:	HC	NO.	REVISIONS	BY	DATE



**CPJ Associates**  
 CPJ Environmental Services Division  
 STREAM RESTORATION - STORMWATER MANAGEMENT - INSPECTION  
 910 CLOPPER ROAD, STE 215N GAITHERSBURG MARYLAND 20878  
 Phone: (301) 208-9673 E-mail: env@cpja.com Fax: (301) 926-4551  
 SILVER SPRING, MD FREDERICK, MD FAIRFAX, VA

SCALE AS SHOWN  
 SHEET 5 OF 8 SHEETS  
 JOB NO. 37-556





**Silt Fence Design Criteria**

Slope Steepness	(Maximum) Slope Length	(Maximum) Silt Fence Length
Flatter than 50:1	unlimited	unlimited
50:1 to 10:1	125 feet	1,000 feet
10:1 to 5:1	100 feet	750 feet
5:1 to 3:1	60 feet	500 feet
3:1 to 2:1	40 feet	250 feet
2:1 and steeper	20 feet	125 feet

Note: In areas of less than 2% slope and sandy soils (USDA general classification system, soil class A) maximum slope length and silt fence length will be unlimited. In these areas a silt fence may be the only perimeter control required.

**Construction Specifications**

1. Fence posts shall be a minimum of 36" long driven 16" minimum into the ground. Wood posts shall be 1 1/2" x 1 1/2" square (minimum) cut, or 1 3/4" diameter (minimum) round and shall be of sound quality hardwood. Steel posts will be standard T or U section weighting not less than 1.00 pound per linear foot.

2. Geotextile shall be fastened securely to each fence post with wire ties or staples at top and mid-section and shall meet the following requirements for Geotextile Class F:  
 Tensile Strength 50 lbs/in (min.) Test: MSMT 509  
 Tensile Modulus 20 lbs/in (min.) Test: MSMT 609  
 Flow Rate 0.3 gal / ft / minute (max.) Test: MSMT 322  
 Filtering Efficiency 75% (min.) Test: MSMT 322

3. Where ends of geotextile fabric come together, they shall be overlapped, folded and stapled to prevent sediment bypass.

4. Silt Fence shall be inspected after each rainfall event and maintained when bulges occur or when sediment accumulation reached 50% of the fabric height.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Restriction - Construction, use, or removal of a temporary access bridge will not normally have any time of year restrictions since construction, use, or removal should not affect the stream or its banks. Unless the bridge is built with a pier(s) in the water.  
 2. Bridge Placement - A temporary bridge structure shall be constructed at or above the bank elevation to prevent the entrapment of floating materials and debris.  
 3. Abutments - Abutments shall be placed parallel to and on stable banks.  
 4. Bridge Span - Bridges shall be constructed to span the entire channel. If the channel width exceeds 8 feet. (as measured from top-of-bank to top-of-bank), then a footing, pier, or bridge support will be permitted within the channel for waterways less than 8 feet wide.  
 5. Stringers - Stringers shall either be logs, sawn timber, prestressed concrete beams, metal beams, or other approved materials.  
 6. Deck Material - Decking materials shall be of sufficient strength to support the anticipated load. All decking members shall be placed perpendicular to the stringers, butted tightly, and securely fastened to the stringers. Decking materials must be butted tightly to the stringers. Decking materials must be butted tightly to prevent any soil material tracked onto the bridge from falling into the waterway below.  
 7. Run Planks (optional) - Run planks shall be securely fastened to the length of the span. One run plank shall be provided for each track of the equipment wheels. Although run planks are optional, they may be necessary to properly distribute loads.  
 8. Curbs or fenders - Curbs or fenders may be installed along the outer sides of the deck. Curbs or fenders are on option which will provide additional safety.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Howard Soil Conservation District Standard Sediment Control Notes**

- A minimum of 48 hours notice must be given to the Howard County Department of Inspections, Licenses and Permits, Sediment Control Division prior to the start of any construction (313-1855).
- All vegetative and structural practices are to be installed according to the provisions of this plan and are to be in conformance with the most current MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL and revisions thereto.
- Following initial soil disturbance or re-disturbance, permanent or temporary stabilization shall be completed within: a) 3 calendar days for all perimeter sediment control structures, dikes, perimeter slopes and all slopes greater than 3:1, b) 7 days as to all other disturbed or graded areas on the project site.
- All disturbed areas must be stabilized within the time period specified above in accordance with the 2011 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL for permanent seeding (Sec. B-4-5), temporary seeding (Sec. B-4-4) and mulching (Sec. B-4-3). Temporary stabilization with mulch alone can only be done when recommended seeding dates do not allow for proper germination and establishment of grasses.
- All sediment control structures are to be maintained in place and are to be maintained in operative condition until permission for their removal has been obtained from the Howard County Sediment Control Inspector.

6. Site Analysis:  
 Total Area of site 104.36 acres.  
 Area Disturbed 2.1 acres.  
 Area to be roofed or paved 0 acres.  
 Area to be vegetatively stabilized 0.25 acres.  
 Total Cut 1,840 Cu. Yds.  
 Total Fill 1,930 Cu. Yds.  
 To be Provided by the Contractor for Approval by the Project Manager, site must have a current open grading permit.

7. Any sediment control practice that is disturbed by grading activity for placement of utilities must be repaired on the same day of disturbance.  
 8. Additional sediment control must be provided, if deemed necessary by the Howard County Sediment Control Inspector.  
 9. On all sites with disturbed areas in excess of 2 acres, approval of the inspection agency shall be requested upon completion of installation of perimeter erosion and sediment controls, but before proceeding with any other earth disturbance or grading. Other building or grading inspection approvals may not be authorized until this initial approval by the inspection agency is made.  
 10. Trenches for the construction of utilities is limited to three pipe lengths or that which shall be back-filled and stabilized by the end of each work day, whichever is shorter.  
 11. Any changes or revisions to the sequence of construction must be reviewed and approved by the plan approval authority prior to proceeding with construction.  
 12. A project is to be sequenced so that grading activities begin on one grading unit (maximum acreage of 20 ac. per grading unit) at a time. Work may proceed to a subsequent grading unit when at least 50 percent of the disturbed area in the preceding grading unit has been stabilized and approved by the enforcement authority. Unless otherwise specified and approved by the approval authority, no more than 30 acres cumulatively may be disturbed at a given time.

Rev. 4/2013

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.  
 4. The standpipe should extend 12" to 18" above the lip of the pit or the riser crest elevation (basin dewatering only) and the filter material should extend 3" minimum above the anticipated standing water elevation.

**Construction Specifications**

1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.  
 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1" diameter holes.  
 3. A base of filter material consisting of clean

