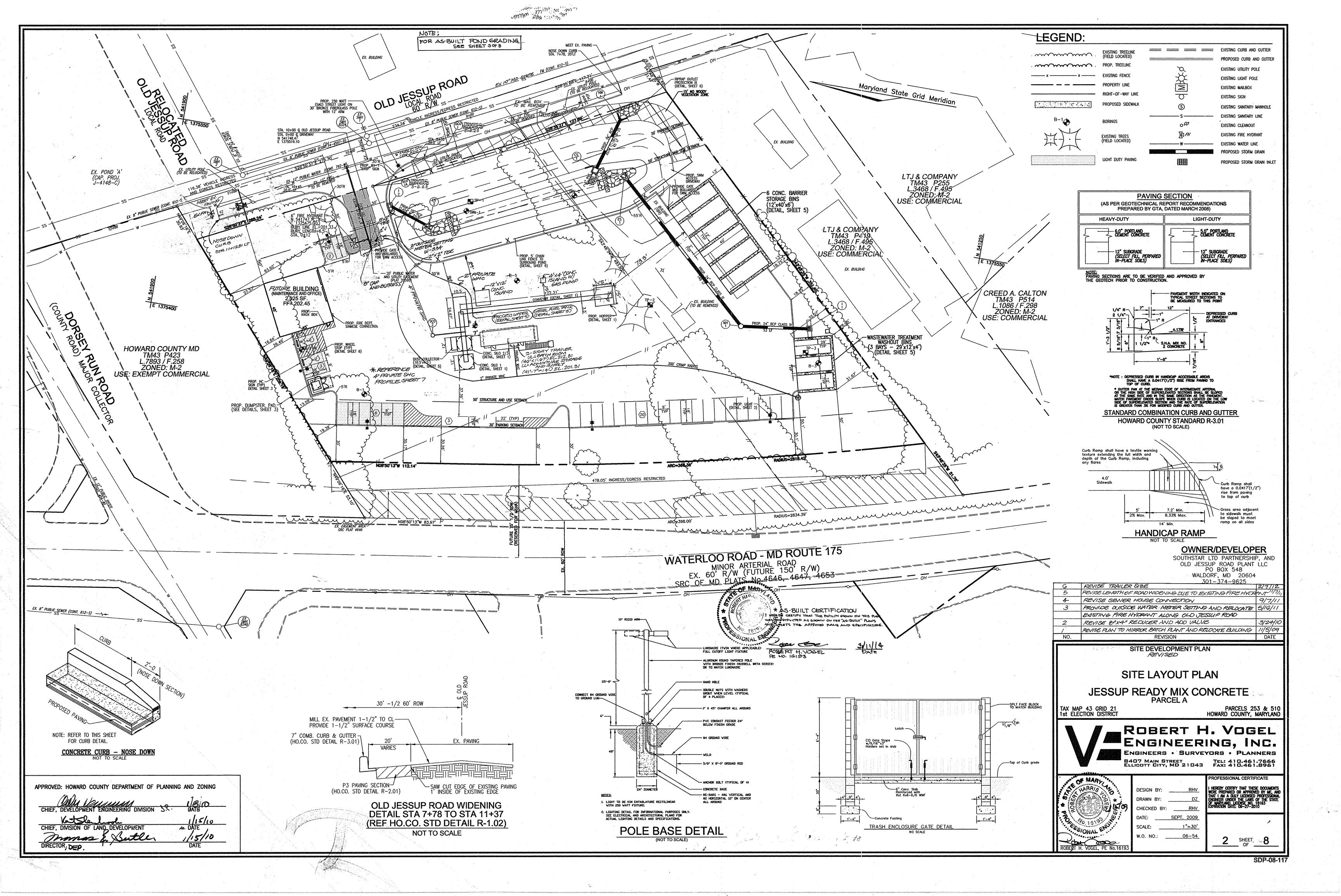
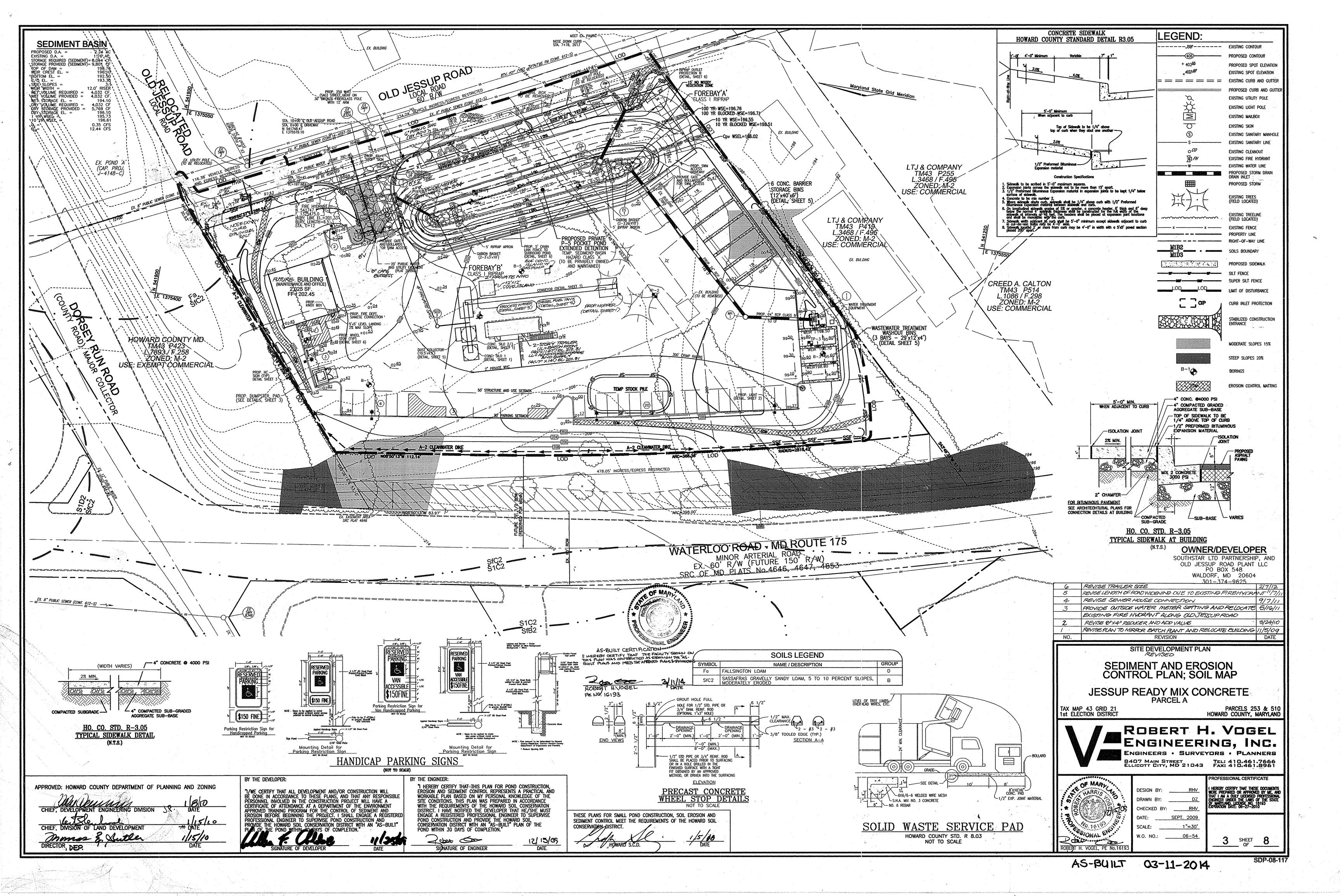
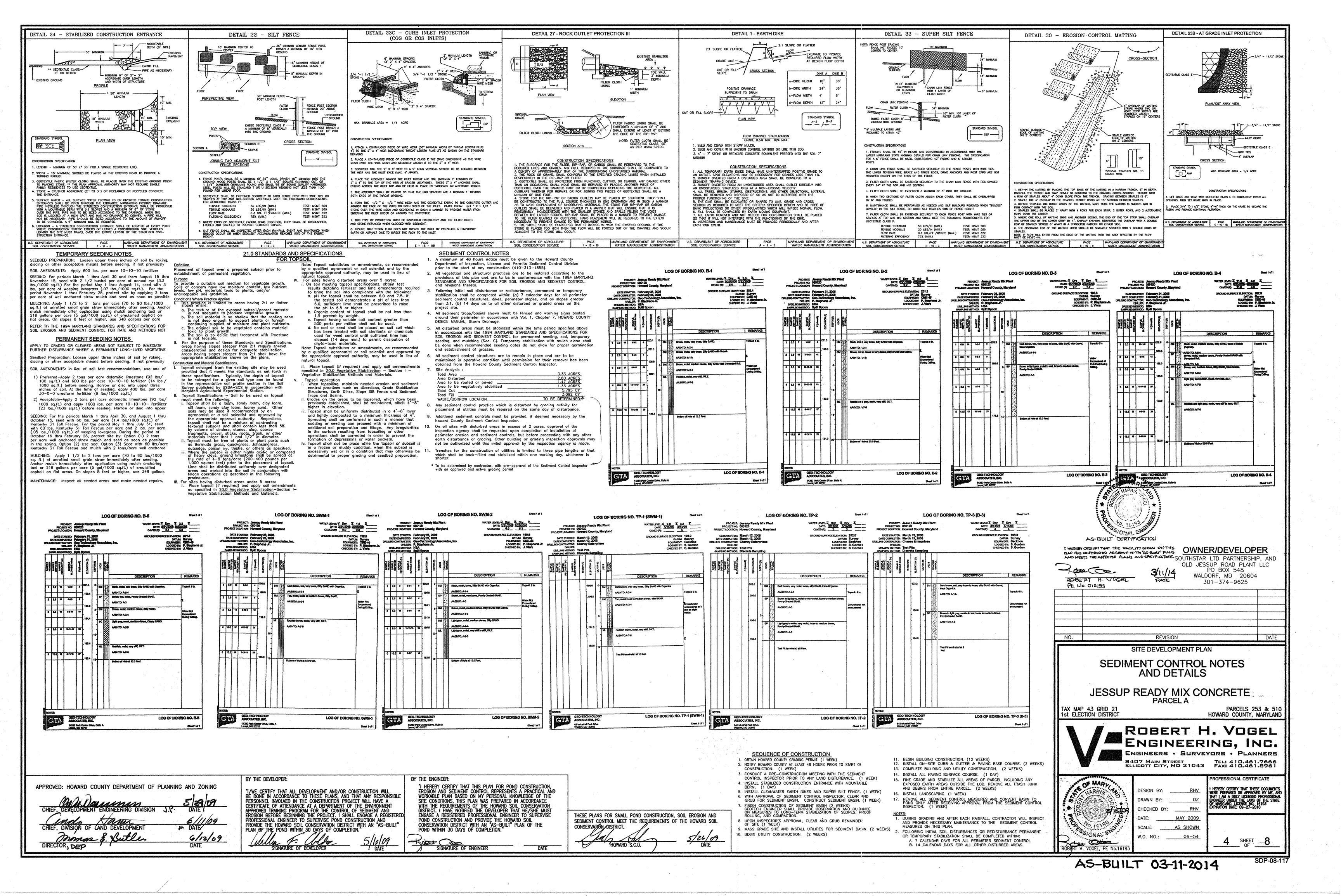
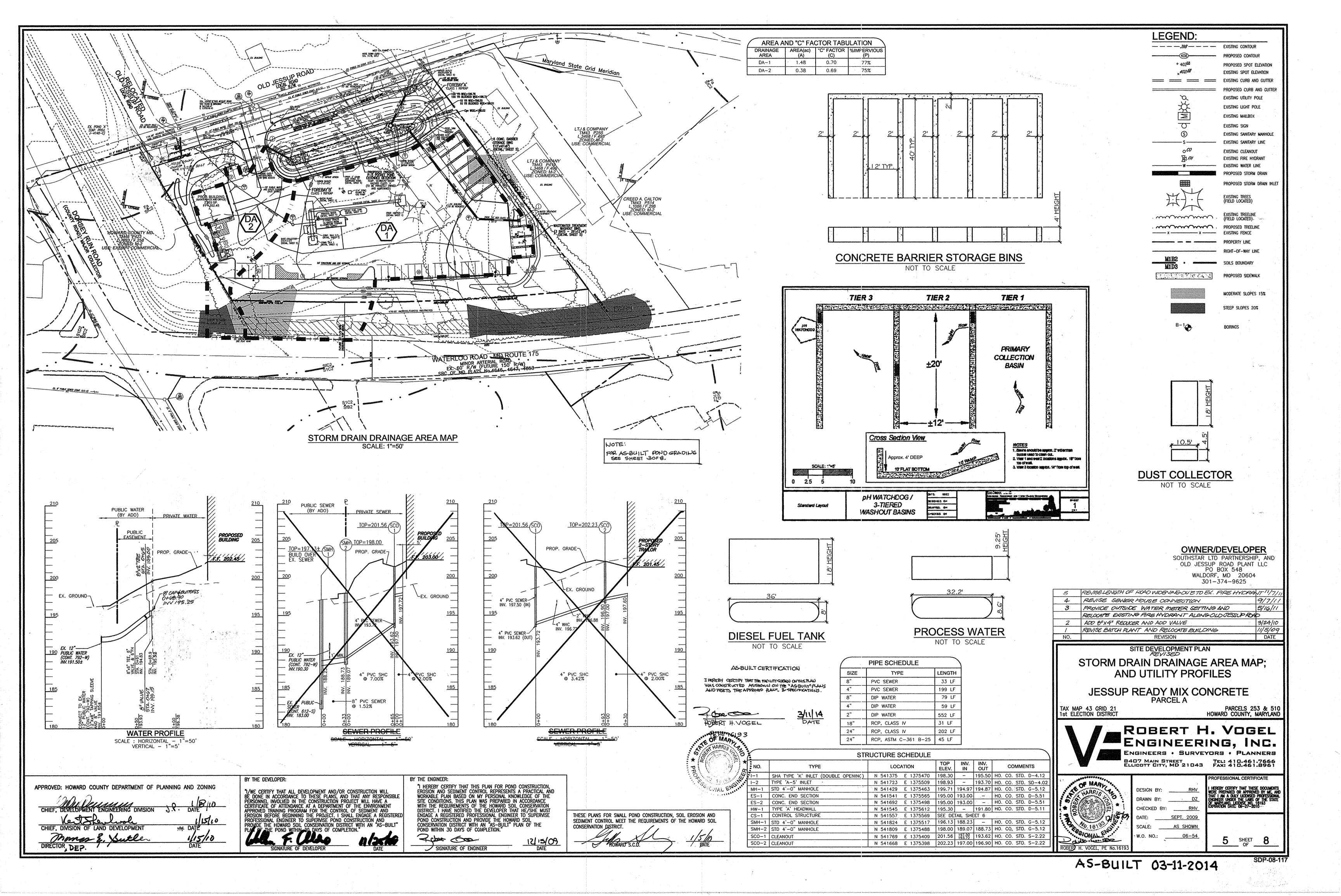
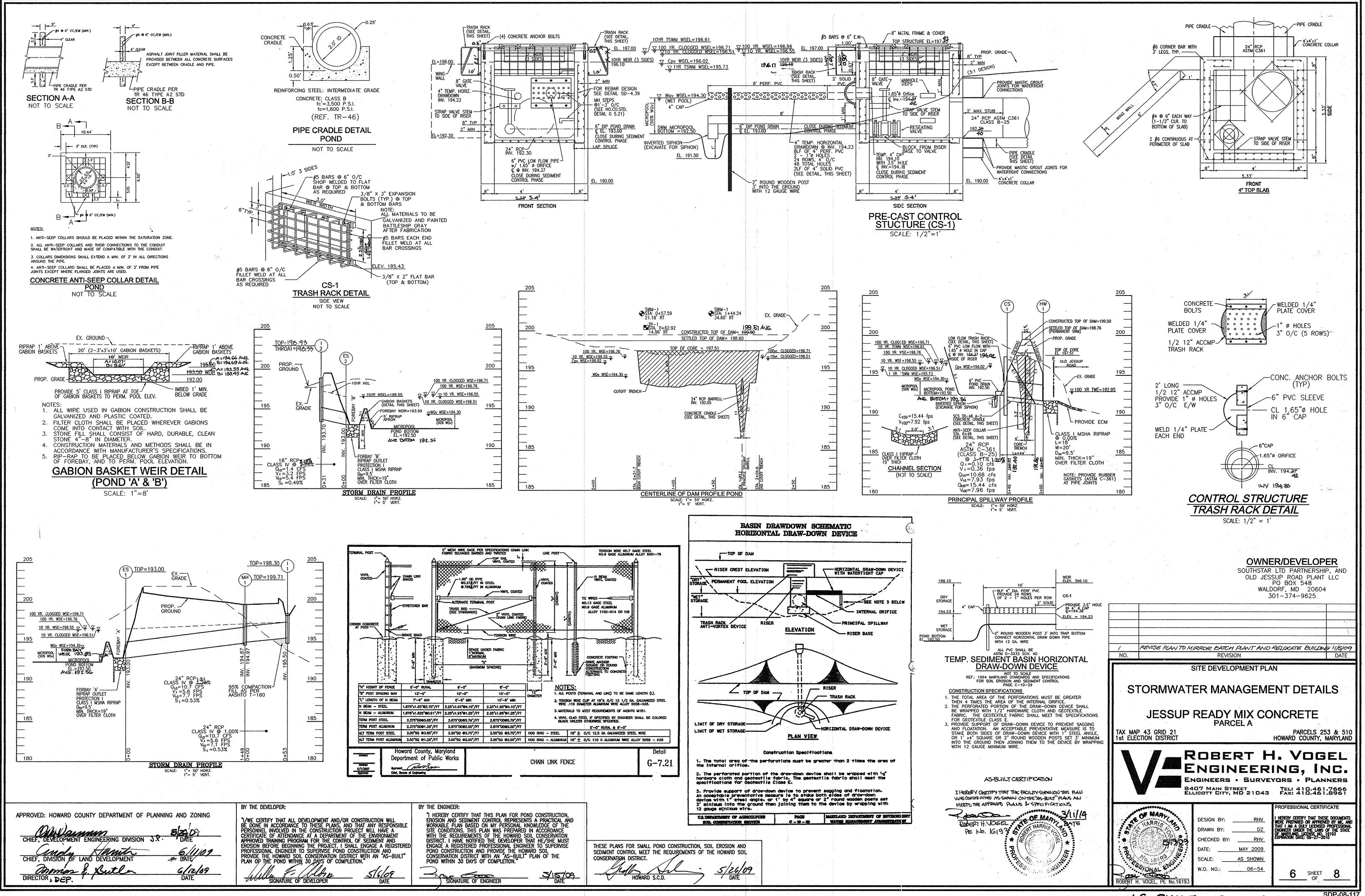
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MARYLAND 378

STORMWATER MANAGEMENT POND CONSTRUCTION SPECIFICATIONS

CONSTRUCTION SPECIFICATIONS

These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent version.

Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the toe of the

Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet structure shall be cleared.

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

Earth Fill

Material - The fill material shall be taken from approved designated borrow areas. It shall be free of roots, stumps, wood, rubbish, stones greater than 6", frozen or other objectionable materials. Fill material for the center of the embankment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer. Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment.

Placement - Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment.

Compaction - The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not be so wet that water can be squeezed out.

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within $\pm -2\%$ of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

Cut Off Trench - The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability.

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed concurrently with the outer shell of the embankment.

Structure Backfill

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operated closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials. Section 313 as modified. The mixture shall have a 100-200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2,000 ohm-cm. Material shall be placed such that minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sides of the pipe. it only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of the structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24" or greater over the structure or pipe. Backfill (flowable fill)zone shall be of the type and quality conforming to that specified for the core of the embankment or other embankment materials.

Pipe Conduits

All pipes shall be circular in cross section.

Corrugated Metal Pipe - All of the following criteria shall apply for corrugated metal pipe:

Materials - (Polymer Coated steel pipe) - Steel pipes with polymeric coating shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 & M-246 with watertight coupling bands or flanges.

Materials — (Aluminum Coated Steel Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.

Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-196 or M-211 with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized balts

may be used for connections. The pH of the surrounding soils shall be between 4 and 9. 2. Coupling, bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at lease 24 mils in thickness.

3. Connections - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.

All connections shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be rerolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches diameter: flonges on both ends of the pipe with a circular 3/8 inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by a 24 inch long annular corrugated band using a minimum of 4(four) rods and lugs. 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8 inch closed cell gaskets the full width of the flange is also acceptable.

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or a neoprene bead.

4. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

Backfilling shall conform to "Structure Backfill."

exceed ASTM C-361.

Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete pipe: 1. Materials - Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets, and shall equal or

2. Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in the "Structure Backfill" section of this standard. Gravel bedding is not permitted.

3. Laying pipe - Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

4. Backfilling shall conform to "Structure Backfill."

5. Other details (anti-seep collars, valves, etc.) shall be shown on the drawings. Plastic Pipe - The following criteria shall apply for plastic pipe:

1. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" -10" inch pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" inch shall meet the requirements of AASHTO M294 Type S.

2. Joints and connections to anti-seep collars shall be completely watertight.

Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

4. Backfilling shall conform to "Structure Backfill."

5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Drainage Diaphragms — When a drainage diaphragm is used, a registered professional engineer will supervise the design and construction inspection.

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3.

Rock Riprop Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction Materials, Section 311.

Geotexile shall be placed under all riprap and shall meet requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

Care of Water during Construction

All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the excavations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall be pumped.

Stabilization All borrow areas shall be graded to provide proper drainage and left I a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures.

DEWATERING STRATEGY

Dewatering refers to the act of removing and discharging water from excavated areas on construction sites or from sediment trops or basins on construction sites. Standards and specifications for dewatering practices follow:

These standards apply to removal and discharge of water from any excavated area or sediment trap or basic at any construction site. Given the unique conditions at any particular construction site, any or off of the practices may apply. Regardless of the applicability of the practices listed herein, operators are required use acceptable procedures for maintenance and dewatering. In all cases, every effort shall be made to eliminate sediment pollution associated with dewatering.

Designers shall specify the preferred procedures for dewatering on plans. In particular, designers should identify procedures for dewatering sediment trops and basins prior to elimination of the last sediment control facility on the site or prior to conversion of sediment control facilities to stammwater management ocilities. Recommended procedures shall be consistent with these standards. Alypical site conditions may equire innovative dewatering designs. Dewatering measures not referenced in this standard may be used with the consent of the approval authority.

Dewatering of Excovoted Areas

- A Designers shall specify on plans, and in sequences of construction included on plans, practices for dewatering of excavated areas. Plan reviewers shall check to see that pracedures for dewatering
- B. In all cases, water removed from excavated areas shall be discharged such that it shall pass through a sediment control device prior to entering receiving waters. Sediment control devices include sediment traps and basins, in addition to the practices in this section.

Approved Practices for Dewatering of Excavated Areas

- 1. Pumping of water to an existing sediment basin or trop in which the entire valume of water from the area to be dewatered can be contained without discharge to receiving waters. 2. Pumping of water to an existing sediment basin or trop such that the entire volume of water from
- 3. Removable Pumping Station? Standards and specifications for Removable Pumping Station are
- Use of a Sump Pit ? Standards and specifications for a sump pit are on Detail 208.
 Sediment Tank ? Standards and specifications for a sump pit are on Detail 21.

Dewatering of Sediment Trops and Basins

Designers shall specify on plans, and in sequences of construction included on plans, the practices for dewatering of traps and basins. Plan reviewers shall check to see that procedures for dewatering to be used are included on plans. In all cases, water removed from trops and basins shall be discharged so that it posses through a sediment control device prior to entering receiving waters

Approved Practices for Dewatering of Trops and Basins

- Removable pumping station
- . Use of a floating suction hose to pump the cleaner water from the top of the pond. As the cleane water is pumped the suction hose will lower and eventually encounter sediment laden water. When this hoppens the pumping operation will cease. Provisions shall be made to filter water

GEOTECHNICAL CONCLUSIONS AND RECOMMENDATIONS FOR STORMWATER MANEMENT

Based upon the results of this exploration, it is GTA's opinion that construction of the stormwater management pond is feasible in the areas explored, yet infiltration is not feasible because of relatively shallow groundwater observed across the site and the presence of clay within the infiltration zone. GTA understands that Vogel plans to design the SWM facility as extended detention. Due to the water perched in the 1 and layers during the wet seasons, the SWM pond will likely hold water during the wet seasons. Because storage is required in the pond as a detention facility, GTA recommends placing a trench drain at the lowest portion of the pond, with a positive gravity slope to the outfall structure to maintain the pond's storage capacity. GTA's preliminary recommendations regarding SWM facilities are provided in the following paragraphs.

. Material Requirements

GTA understands that the Soil Conservation Service of Maryland (SCS), Specification 378 (MD 378) governs design and construction of the ponds. MD 378 specifies that soils for use in cutoff trench construction meet USCS Classification CL (low plasticity clay), CH (high plasticity clay), SC (clayey sand), or GC (clayey gravel). Furthermore, GTA recommends that similar materials be used for brokfill adjacent to the outfall structure. The use of the fine-grained or plastic material adjacent to the pipe should decrease the potential for 'piping' crosional failures.

The majority of the on-site soils encountered near the existing ground surface in the vicinity of the SWM ponds are generally classified as USCS SM or SP, with ML classified soils found at depths of 7 to 12 feet below existing grade. Borrow materials will likely be required. Borrow materials should meet the classifications required by MD 378, and be approved by GTA prior to placement as cutoff trench fill.

MD 378 specifies that all of the referenced classifications suitable for cutoff trench construction are suitable for embankment construction as well. Furthermore, it is GTA's opinion that select USCS ML soils, low plasticity silt, are also suitable for embankment construction. Sand and gravel are not recommended as embankment material

2. Basin Excavation and Embankmer t Construction

Based on the results of the test bosings, it is likely that the majority of the excavations required for the ponds can be performed by standard excavation methods. The groundwater levels observed within the vicinity of the proposed SWM Pond generally correspond to levels above the approximate basin elevations (see soil profiles in Appendix A). In addition due to the granular nature of the surficial soils and potential for interbedded clay and silt layers in this area, perched water may be encountered during pond construction. If groundwater is encountered, then temporary dewatering of the basin and the outfall structure subgrades will be required during the construction phase to facilitate excavation and construction. Due to the potential for high water conditions, water seepage may occur along

PRIVATE SEWER

SCO TOP=196.94 IA TRAFFIC BEARING

.EX. GROUND

4"PVC SHC

@5.00%

SEWER PROFILE

VERTICAL - /=5'

SCALE: HORIZONTAL - 1"=50"

TOP-202.23 (SCO

2.00%

PROP. GRADE-

GTA understands that the Soil Conservation Service of Maryland (SCS), Specification 378 (MD 378) governs design and construction of the pends. MD 378 specifies that soils for use in cutoff trench construction meet USCS Classification CL (low plasticity clay), CH (high plasticity clay), SC (clayey sand), or GC (clayey gravel). Furthermore, GTA recommends that similar materials be used for backfill adjacent to the outfall structure. The use of the fine-grained or plastic material adjacent to the pipe should decrease the potential for 'piping' erosional failures.

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the cut slopes planned for the pond, which may result in surface sloughing and instability of the proposed slopes. Depending on the groundwater conditions encountered during construction, mitigating measures such as field or blanket drains may be required to improve long term stability of the slope. GTA can review field conditions and provide recommendations for such measures during construction.

Prior to the placement of compacted fill or the construction of the outfall cradle and structures, areas supporting the proposed pond embankment and structures should be stripped and grubbed to remove all topsoil and other organic matter. GTA anticipates a stripping thickness on the order of 8 inches within the proposed pend locations. However, the topsoil/rootmat thickness is variable and may be encountered to greater depths between boring locations and in unexplored areas of the site, or in swales, where it could extend to several feet below grade. The actual stripping thickness will be dependant on topsoil development, soil moisture, construction traffic disturbance, and contractor care. After stripping, the subgrade should be proof rolled as directed by a geotechnical engineer or his qualified representative. Unstable soils identified by proof rolling should be removed from

Fills for cutoff trench and embankment construction should be constructed in eight-inch loose lifts, and compacted to within 95 percent of the maximum dry density in accordance with the Standard Proctor, ASTM D-698. Fills around the outfall works should be placed in 4-inch lifts and compacted to the same standard with hand equipment. Compactive effort should be monitored with in-place density testing as performed by a qualified representative under the direction of a professional engineer. Fills should generally be placed within 2 to 4% of the optimum moisture content. However, more restrictive moisture control may be required by the County specifications. Earthwork should be monitored by engineering technicians under the direct supervision of a registered professional engineer and all compactive effort should be verified by in-place density testing. The majority of the on-site soils are likely to be wet of their optimum moisture contents and significant moisture conditioning may be required prior to using on-site materials as fill. GTA recommends that a contingency be established for drying and/or over-excavating and replacing wet on-site

SUMMARY TABLE AREA A (SITE)

O (1919) (1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				
REQUIREMENT	VOLUME REQUIREMENT W/O CREDITS	CREDITS	VOLUME REQUIREMENT AFTER CREDITS	NOTES
WATER QUALITY VOLUME WQV	0.1745 AC	0	0.1745 AC	MICROPOOL EXTENDED DETENTION POCKET POND
RECHARGE VOLUME REV	0.0454 AC	0	0.0454	Rev NOT PROVIDED DUE TO "HOT SPOT" DESIGNATION
CHANNEL PROTECTION VOLUME CPV	0.1929 AC	N/A	0.1929 AC	MICROPOOL EXTENDED DETENTION POCKET POND
OVERHEAD FLOOD PROTECTION, Q10P	N/A	N/A	N/A	
EXTREME FLOOD VOLUME, Q100P	N/A	N/A	N/A	
	WATER QUALITY VOLUME WQV RECHARGE VOLUME REV CHANNEL PROTECTION VOLUME CPV OVERHEAD FLOOD PROTECTION, Q10P EXTREME FLOOD	REQUIREMENT WOLUME REQUIREMENT W/O CREDITS WATER QUALITY VOLUME WQV O.1745 AC RECHARGE VOLUME REV CHANNEL PROTECTION VOLUME CPV OVERHEAD FLOOD PROTECTION, Q10P EXTREME FLOOD	REQUIREMENT W/O CREDITS WATER QUALITY VOLUME WQV 0.1745 AC 0 RECHARGE VOLUME REV 0.0454 AC 0 CHANNEL PROTECTION VOLUME CPV 0.1929 AC N/A N/A EXTREME FLOOD	REQUIREMENT W/O CREDITS WATER QUALITY VOLUME WQV 0.1745 AC 0.1745 AC 0.1745 AC 0.1745 AC RECHARGE VOLUME REV 0.0454 AC 0.1929 AC OVERHEAD FLOOD PROTECTION, Q10P N/A N/A N/A N/A N/A N/A N/A N/

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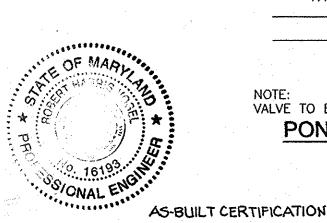
TRAFFIC BEARING

REF 5-223

BUILD CLEAN OUT OVER EX. 6"SHC

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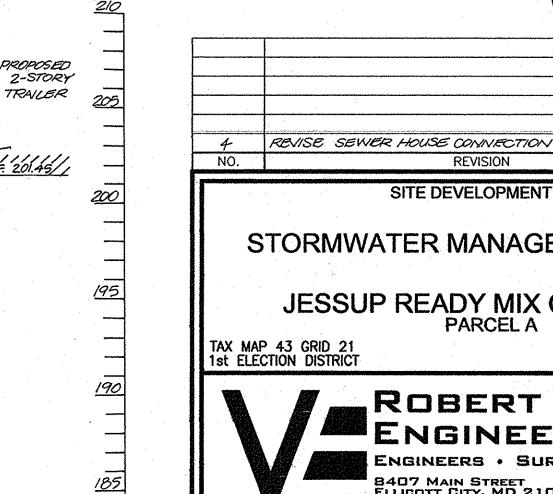


6" P.V.C VALVE TO BE PLACED UNDER COVER OPENING. POND DRAIN DETAIL NOT TO SCALE

I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN WAS CONSTRUCTED AS SHOWN ON THE "AS-BUILT PLANS

AND MEETS THE APPROVED PLANE AND SPECIFICATIONS PE. No. 16193

OWNER/DEVELOPER SOUTHSTAR LTD PARTNERSHIP, AND OLD JESSUP ROAD PLANT LLC PO BOX 548 WALDORF, MD 20604



301-374-9625

Engineers • Surveyors • Planners

8407 Main Street Tel: 410.461.7666 ELLICOTT CITY, MD 21043 FAX: 410.461.8961

REVISION SITE DEVELOPMENT PLAN STORMWATER MANAGEMENT NOTES JESSUP READY MIX CONCRETE

HOWARD COUNTY, MARYLAND ROBERT H. VOGEL ENGINEERING, INC.

OF MAD.

CHECKED BY: DATE: 1"=30' SCALE: 06-54

ROFESSIONAL CERTIFICATE SHEET _ OF .

PARCELS 253 & 510

OPERATION AND MAINTENANCE SCHEDULE FOR STORMWATER MANAGEMENT EXTENDED DETENTION FACILITY

ROUTINE MAINTENANCE

1. FACILITY WILL BE INSPECTED ANNUALLY AND AFTER MAJOR STORMS. INSPECTIONS SHOULD BE PERFORMED DURING WET WEATHER TO DETERMINE IS FUNCTIONING PROPERLY. . TOP AND SIDE SLOPES OF THE EMBANKMENT SHALL BE MOWED A MINIMUM OF TWO (2) TIMES A YEAR, ONCE IN JUNE AND ONCE IN SEPTEMBER.
OTHER SIDE SLOPES AND MAINTENANCE ACCESS SHOULD BE MOWED AS

. DEBRIS AND LITTER NEXT TO THE OUTLET STRUCTURE SHALL BE REMOVED DURING REGULAR MOWING OPERATIONS AND AS NEEDED 4. VISIBLE SIGNS OF EROSION IN THE POND AS WELL AS RIPRAP OUTLET AREAS SHALL BE REPAIRED AS SOON AS IT IS NOTICED.

1. STRUCTURAL COMPONENTS OF THE POND SUCH AS THE DAM, THE RISER, AND THE PIPES SHALL BE REPAIRED UPON DETECTION OF ANY DAMAGE. THE COMPONENTS SHOULD BE INSPECTED DURING ROUTINE MAINTENANCE

2. SEDIMENT SHOULD BE REMOVED WHEN ITS ACCUMULATION SIGNIFICANTLY

DEEMED NECESSARY BY THE HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

6/12/09

REDUCES THE DESIGN STORAGE, INTERFERE WITH THE FUNCTION OF THE

RISER, WHEN DEEMED NECESSARY FOR AESTHETIC REASONS, OR WHEN

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

If broken rock fragments are encountered at finished pond bottom, under cut a minimum of 12" below basin grade and to a horizontal distance of at least 18" beyond each edge of the broken rock and backfill with fine—grained ML or CL soils compacted to a firm condition. This procedure should be performed under the supervision of the project Geotechnical Engineer

OPERATION, MAINTENANCE AND INSPECTION

POND BOTTOM SOIL CONDITIONS

INSPECTION OF THE POND(S) SHOWN HEREON SHALL BE PERFORMED AT LEAST ANNUALLY, IN ACCORDANCE WITH THE CHECKLIST AND REQUIREMENTS CONTAINED WITHIN USDA, SCS "STANDARDS AND SPECIFICATIONS FOR PONDS" (MD-378), THE POND OWNER(S) AND ANY HEIRS, SUCCESSORS, OR ASSIGNS SHALL BE RESPONSIBLE FOR THE SAFETY OF THE POND AND THE CONTINUED OPERATION, SURVEILLANCE, INSPECTION, AND MAINTENANCE THEREOF. THE POND OWNER(S) SHALL PROMPTLY NOTIFY THE SOIL CONSERVATION DISTRICT OF ANY UNUSUAL OBSERVATIONS THAT MAY BE INDICATIONS OF DISTRESS SUCH AS EXCESSIVE SEEPAGE, TURBID SEEPAGE, SLIDING OR SLUMPING.

OPERATION AND MAINTENANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED

STORMWATER INFILTRATION TRENCHES 1. THE MONITORING WELLS AND STRUCTURES SHALL BE INSPECTED ON A QUARTERLY BASIS AND AFTER EVERY LARGE STORM EVENT. . WATER LEVELS AND SEDIMENT BUILD UP IN THE MONITORING WELLS SHALL BE RECORDED OVER A PERIOD OF SEVERAL DAYS TO INSURE TRENCH DRAINAGE. 5. A LOGBOOK SHALL BE MAINTAINED TO DETERMINE THE RATE AT WHICH THE 4. WHEN THE FACILITY BECOMES CLOGGED SO THAT IT DOES NOT DRAIN DOWN WITHIN THE XXX HOUR TIME PERIOD, CORRECTIVE ACTION SHALL BE TAKEN. 5. THE MAINTENANCE LOGBOOK SHALL BE AVAILABLE TO HOWARD COUNTY FOR INSPECTION TO INSURE COMPLIANCE WITH OPERATION AND MAINTENANCE

6. ONCE THE PERFORMANCE CHARACTERISTICS OF THE INFILTRATION FACILITY

HAVE BEEN VERIFIED. THE MONITORING SCHEDULE CAN BE REDUCED TO AN

ANNUAL BASIS UNLESS THE PERFORMANCE DATA INDICATES THAT A MORE

BY THE DEVELOPER:

"I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE IN ACCORDANCE TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

SIGNATURE OF DEVELOPER

BY THE ENGINEER: "I HEREBY CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENCACE A RECUESTED PROFESSIONAL ENCINEER TO SUBSPICIO ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION." SIGNATURE OF ENGINEER SISON

THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND

SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.

AS-BUILT 03-11-2014

DATE

