GENERAL NOTES

- ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST STANDARDS AND SPECIFICATIONS OF HOWARD COUNTY PLUS MSHA STANDARDS AND SPECIFICATIONS IF APPLICABLE.
- THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS/BUREAU OF ENGINEERING/CONSTRUCTION INSPECTION DIVISION AT (410) 313-1880 AT LEAST FIVE (5) WORKING DAYS PRIOR TO START OF WORK.
- THE CONTRACTOR SHALL NOTIFY "MISS UTILITY" AT 1-800-257-7777 AT LEAST 48 HOURS PRIOR TO ANY EXCAVATION WORK BEING DONE.
 TRAFFIC CONTROL DEVICES, MARKINGS AND SIGNING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). ALL STREET AND REGULATORY SIGNS SHALL BE IN PLACE PRIOR TO THE PLACEMENT OF ANY PAYING.
- ALL PLAN DIMENSIONS ARE TO FACE OF CURB UNLESS OTHERWISE NOTED.
 THE EXISTING SITE TOPOGRAPHY IS TAKEN FROM FIELD RUN SURVEY WITH 2' CONTOUR INTERVALS PREPARED BY MORRIS & RITCHIE ASSOCIATES, INC. DATED JANUARY 21, 2006.
 COORDINATES, COURSES AND ELEVATIONS SHOWN HEREON ARE REFERRED TO THE MERIDIAN OF THE MARYLAND STATE PLANE COORDINATE SYSTEM (NAD 83/91, NAVD 88) AS DETERMINED FROM THE FOLLOWING
- HOWARD COUNTY CONTROL STATIONS (US SURVEY FEET): 37GD AND 37GC SHOWN HEREON.

 8. WATER IS PUBLIC.
- 9. SEWER IS BOTH PRIVATE AND PUBLIC.
 10. ALL SWM DEVICES WILL BE PRIVATE.
 11. EXISTING UTILITIES ARE BASED ON CONTRACT DRAWINGS FOR WATER & SEWER (CONTRACT #24-4284-D) AND
- CAD DRAWINGS FROM PATTON, HARRIS, RUST & ASSOCIATES, PC.

 12. THE 100-YEAR FLOODPLAIN LIMITS FROM PATTON, HARRIS, RUST & ASSOCIATES, PC FINAL PLAN F-05-058, DATED DECEMBER 12, 2005 WERE REVISED BY MORRIS & RITCHIE ASSOCIATES INC. FLOODPLAIN STUDY

 DATED 7/31/06 AND THE PENASED FLOODPLAIN IS SHOWN HEPEON.
- DATED 7/31/06 AND THE REVISED FLOODPLAIN IS SHOWN HEREON.

 13. THE WETLANDS AND STREAMS FOR THIS PROJECT WERE TAKEN FROM PATTON, HARRIS, RUST & ASSOCIATES, PC FINAL PLAN F-05-058, DATED DECEMBER 12, 2005.
- 14. THE TRAFFIC STUDY FOR THIS PROJECT WAS PREPARED BY WELLS & ASSOCIATES, LLC, DATED OCTOBER 7, 2002, LAST REVISED APRIL 13, 2005.
- 15. CONTRACTOR IS RESPONSIBLE FOR ALL SITE CONDITIONS, CONSTRUCTION REQUIREMENTS, AND SHALL CONFORM TO ALL STATE, FEDERAL, AND COUNTY CONSTRUCTION REGULATIONS. THE CONTRACTOR IS NOT RELIEVED OF RESPONSIBILITY SHOULD ANY REQUIRED ITEMS PERTAINING TO SITE CONSTRUCTION NOT BE INCLUDED ON THESE PLANS. CONTRACTOR IS RESPONSIBLE FOR ALL ITEMS NECESSARY TO COMPLETE THE SITE IMPROVEMENTS AS SHOWN ON THESE PLANS.
- 16. ANY DAMAGE TO EXISTING UTILITIES, PAVEMENT, OR CURB AND GUTTER DUE TO CONSTRUCTION ACTIVITY OUTSIDE THE LIMITS OF DISTURBANCE IS TO BE REPLACED BY THE CONTRACTOR AT HIS OWN EXPENSE.
- 17. WHERE NECESSARY, THE CONTRACTOR SHALL TEST PIT ALL EXISTING UTILITIES AT LEAST FIVE (5) DAYS PRIOR TO STARTING ANY WORK SHOWN ON THESE DRAWINGS.
- TO STARTING ANY WORK SHOWN ON THESE DRAWINGS.

 18. CONTRACTOR IS RESPONSIBLE FOR REPLACING ANY PROPERTY MONUMENTS, MARKERS, SIGNS, LIGHTS, OR ANY OTHER EXISTING SITE FEATURES DISTURBED DURING CONSTRUCTION.
- 19. ALL WORK SHALL COMPLY WITH ALL APPLICABLE PROVISIONS OF THE "1994 STANDARDS AND S SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROLS" PUBLISHED JOINTLY BY THE WATER RESOURCES ADMINISTRATION, SOIL CONSERVATION SERVICE, AND STATE SOIL CONSERVATION COMMITTEE.
- 20. THE SUBJECT PROPERTY IS ZONED NT PER THE FEBRUARY 2, 2004 COMPREHENSIVE ZONING PLAN.
 21. THIS PROJECT IS EXEMPT FROM THE REQUIREMENTS OF SECTION 16.1200 OF THE HOWARD COUNTY CODE FOR FOREST CONSERVATION BECAUSE THE PROPERTY IS ZONED NT. AND HAS PRELIMINARY PLAN APPROVAL
- PRIOR TO DECEMBER 31, 1992 PER SECTION 16.1202 (b)(1)(w).

 22. A NOISE STUDY FOR THIS PROJECT WAS PREPARED BY THE MARS GROUP, DATED APRIL, 2006. A NOISE MITIGATION STUDY WAS PREPARED BY THE MARS GROUP, DATED JULY, 2006.

 23. A GEO—TECHNICAL STUDY WAS CONDUCTED FOR THIS PROJECT BY GEO—TECHNOLOGY ASSOCIATES, INC. DATED
- NOVEMBER, 2005 & MARCH, 2006.

 24. TRENCH COMPACTION FOR STORM DRAINS WITHIN ROADWAYS AND PARKING AREAS SHALL BE IN ACCORDANCE WITH LOWARD COLLECT DESIGN MANUAL VOL. AT STANDARD NO. C—2.01
- WITH HOWARD COUNTY DESIGN MANUAL, VOL. IV, STANDARD NO. G-2.01.

 25. ALL COMPACTED FILL SHALL BE IN ACCORDANCE WITH AASHTO T-180 REQUIREMENTS.

 26. ALL EXTERIOR LIGHTING SHALL COMPLY WITH ZONING REGULATIONS SECTION 134. SITE LIGHTING TO BE
- 26. ALL EXTERIOR LIGHTING SHALL COMPLY WITH ZONING REGULATIONS SECTION 134. SITE LIGHTING TO BE PROVIDED BY BGE.

 A) SIGHT LIGHT LOCATIONS SHOWN ON SHEETS 7—8 AND DETAIL ON SHEET 4 PROVIDED BY
- CONSTELLATION ENERGY. THE LIGHT INTENSITY AT THE PROPERTY LINE WILL NOT EXCEED 0.1 FOOT CANDLES. SITE LIGHTS WILL BE 150 WATT W/ 16,000 LUMENS. CONSTELLATION ENERGY WILL INSTALL LIGHT SHEILDS AS NECESSARY.

 B) EACH ENTRANCE AND EXIT POINT ONTO OLD WATERLOO ROAD SHALL HAVE A STREET LIGHT ON THE
- OPPOSITE CORNER OF THE STOP SIGN. THE REQUIRED STREET LIGHTS SHALL BE 150-WATT HPS VAPOR "PREMIER" POST TOP FIXTURES MOUNTED ON A 14-FOOT BLACK FIBERGLASS POLE.

 27. THIS PLAN HAS BEEN PREPARED IN ACCORDANCE WITH THE PROVISIONS OF SECTION 16.124 OF THE HOWARD COUNTY CODE AND THE LANDSCAPE MANUAL. FINANCIAL SURETY FOR THE REQUIRED LANDSCAPING IN THE AMOUNT OF \$61,440 (76 SHADE TREES © \$300.00 EA., 178 FLOWERING/ EVERGREEN TREES ©
- \$150.00 EA, 398 SHRUBS @ \$30.00 EA) IS PART OF THE DEVELOPER'S AGREEMENT.

 28. SEE HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS CONTRACT DRAWING 24-4376-D FOR PUBLIC WATER & SANITARY SEWER DESIGN.

 29. SEE FINAL PLAN F-06-203 FOR PUBLIC ROAD AND STORM DRAIN DESIGN.
- 30. WAIVER PETITION WP-06-110 HAS BEEN WITHDRAWN. THE SITE PLAN HAS BEEN REVISED SO THAT NO WAIVER PETITION WILL BE REQUIRED.
- 31. FOR DRIVEWAY ENTRANCE DETAIL REFER TO HOWARD COUNTY DESIGN MANUAL, VOLUME IV, STANDARD DETAIL R-6.01.

 32. CURRENT IMPROVEMENTS BEING DONE TO WATERLOO ROAD FOR ROAD WIDENING, NEW CURB & GUTTER,
- STORM DRAIN AND SIDEWALKS ARE BEING DONE UNDER F-05-058 ARE SHOWN ON OUR PLAN AS EXISTING.

 33. THERE ARE NO STEEP SLOPES (25% OR >), WITHIN OUR LOD.

 34. IN ACCORDANCE WITH FDP PHASE 240, BAY WINDOWS OR CHIMNEYS NOT MORE THAN 10 FEET IN WIDTH MAY PROJECT NOT MORE THAN 4 FEET INTO ANY SETBACKS, PORCHES OR DECKS MAY PROJECTS NOT MORE
- THAN 3 FEET INTO THE FRONT OR REAR YARD SETBACKS. EXTERIOR BASEMENT AREAWAYS/ STAIRWAYS MAY NOT ENCROACH INTO ANY BRL.

 35. NO GRADING, REMOVAL OF VEGETATIVE COVER OR TREES, PAVING AND NEW STRUCTURES SHALL BE PERMITTED WITHIN THE REQUIRED WETLANDS, STREAM(S) OR THEIR BUFFERS, FOREST CONSERVATION
- EASEMENT AREAS AND 100-YEAR FLOODPLAIN. NO CLEARING, GRADING OR CONSTRUCTION IS PERMITTED WITHIN THE WETLANDS, WETLAND BUFFERS, STREAM OR STREAM BUFFERS AND FLOODPLAINS.

 36. DRIVEWAYS SHALL BE PROVIDED PRIOR TO ISSUANCE OF A USE AND OCCUPANCY PERMIT FOR ANY NEW DWFLLINGS TO INSURE SAFE ACCESS FOR FIRE AND EMFRGENCY VEHICLES PER THE FOIL OWING MINIMUM.
- 36. DRIVEWAYS SHALL BE PROVIDED PRIOR TO ISSUANCE OF A USE AND OCCUPANCY PERMIT FOR ANY NEW DWELLINGS TO INSURE SAFE ACCESS FOR FIRE AND EMERGENCY VEHICLES PER THE FOLLOWING MINIMUM REQUIREMENTS:

 WIDTH: 12' (14' SERVING MORE THAN ONE RESIDENCE).
- SURFACE: 6" OF COMPACTED CRUSHER RUN BASE W/ TAR AND CHIP COATING (1-1/2" MIN.).
 GEOMETRY: MAX. 15% GRADE, MAX. 10% GRADE CHANGE AND MIN. 45' TURNING RADIUS.
 STRUCTURES (CULVERTS/ BRIDGES): CAPABLE OF SUPPORTING 25 GROSS TONS (H25 LOADING).
- DRAINAGE ELEMENTS: CAPABLE OF SAFELY PASSING 100—YEAR FLOOD WITH NO MORE THAN 1' DEPTH OVER DRIVEWAY SURFACE.
 MAINTENANCE: SUFFICIENT TO INSURE ALL WEATHER USE.
 THE 65 dBA NOISE CONTOUR LINE DRAWN ON THIS DEVELOPMENT PLAN IS ADVISORY AS REQUIRED BY THE HOWARD COUNTY DESIGN MANUAL, CHAPTER 5, REVISED FEBRUARY, 1992, AND CAN NOT BE CONSIDERED TO EXACTLY LOCATE THE 65 dBA NOISE EXPOSURE. THE 65 dBA NOISE LINE WAS ESTABLISHED BY HOWARD
- COUNTY TO ALERT DEVELOPERS, BUILDERS AND FUTURE RESIDENTS THAT AREAS BEYOND THIS THRESHOLD MAY EXCEED GENERALLY ACCEPTED NOISE LEVELS ESTABLISHED BY THE U.S. DEPARTMENT OF HOUSING AND URBAN DEVELOPMENT.

 38. THERE ARE NO EXISTING STRUCTURES IN J, K, L OR Q.
- 39. ADVISORY NOTE: PER FDP-240, "WHENEVER AN ATTACHED LAND USE, SINGLE FAMILY DWELLING IS CONSTRUCTED, A MAINTENANCE AGREEMENT FOR THE PARTY WALL SIDE OF THE STRUCTURE, AS WELL AS ANY APPROVED OVERHANGS PROTRUDING ONTO THE ADJACENT LOT OR LOTS MUST BE INCLUDED IN THE DEED OF CONVEYANCE AND RECORDING REFERENCE OF SAME FURNISHED TO THE DEPARTMENT OF PLANNING
- AND ZONING."

 40. PROPOSED ROADS WILL BE PRIVATE AND WILL BE CLASSIFIED AS LOCAL ROADS.

 41. FIRE LANES SHOWN ON SHEETS 7—8 TO BE PAINTED AND MARKED PER HOWARD COUNTY FIRE DEPARTMENT
- STANDARDS.

 42 TWO DESIGN MANUAL WAIVERS HAVE BEEN REQUESTED FOR THIS SDP:

 A. VOLUME I SECTION 5.2.4.1: REQUEST FOR RELIEF OF THE REQUIRED 25 FOOT STORMWATER

 MANAGEMENT POND SETBACK FROM ADJACENT PROPERTIES AND RIGHT—OF—WAYS. DED HAS DEFERRED THIS
- 43. A reduction of the 30' setback from the right-of-way of old Waterloo Road to 10' for Parcels "J", "k" and "L" was granted by the Planning Board on May 10, 2007.

 The Planning Board approved SDP-06-128 on May 10, 2007.

B. VOLUME II - SECTION 2.3.2.A: REQUEST FOR RELIEF OF THE REQUIRED INTERSECTION SPACING. DED

AFFROVED PLANNING BOARD of HOWARD COUNTY

APPROVED THIS WAIVER REQUEST ON 10/30/06.

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

CHIEF, DEVELOPMENT ENGINEERING DIVISION

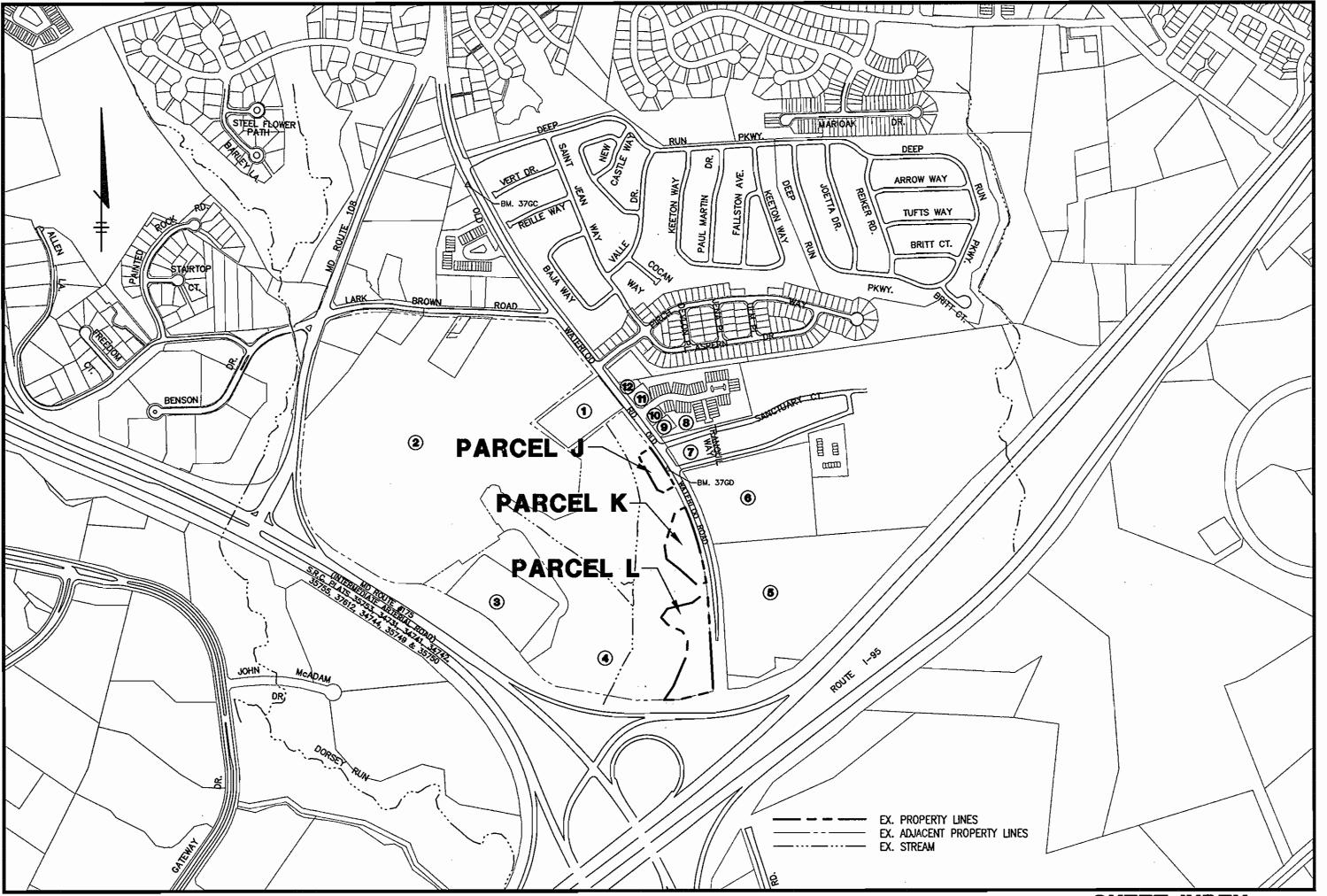
CHIEF, DIVISION OF LAND DEVELOPMENT THE DATE

DIRECTOR, DEPARTMENT OF PLANNING AND ZONING

DATE

SITE DEVELOPMENT PLAN OLD WATERLOO ROAD JATEWAY OVERLOOK AT BENSON EAST PARCEL "382"

6th ELECTION DISTRICT HOWARD COUNTY, MARYLAND SDP-06-128



LOCATION MAP

SCALE: 1" = 600'

SITE ANALYSIS DATA CHART

- A. TOTAL PROJECT AREA:

 PARCEL 382/LOT J

 K

 2.11 ACRES OR 35,840.09 SQ. FT.

 L

 3.67 ACRES OR 159,709.24 SQ. FT.

 (PORTION OF) Q

 2.76 ACRES OR 120,232.82 SQ. FT.

 TOTAL:

 9.36 ACRES OR 407,507.02 SQ. FT.

 B. LIMIT OF DISTURBED AREA:
 PARCEL 382/LOT J

 K

 0.82 ACRES OR 35,754.64 SQ. FT.

 2.11 ACRES OR 91,724.87 SQ. FT.
- (PORTION OF) Q
- D. PROPOSED USE: SINGLE FAMILY ATTACHED CONDOMINIUM UNITS
- TOTAL NUMBER OF UNITS ALLOWED PER PLAT F-05-058: 66 UNITS
- F. TOTAL NUMBER ON UNITS PROPOSED: 66 UNITS
- G. PARKING SPACES REQUIRED: 2 SPACES PER D.U. = 2 SPACES/ UNIT X 60 UNITS = 132 SPACES

3.37 ACRES OR 146,972.22 SQ. FT.

- H. PARKING PROVIDED: 2 INSIDE GARAGE/ UNIT (132 SPACES) & 2 IN DRIVEWAY/ UNIT (132 SPACES) + 36 SPACES = 300 SPACES
- BUILDING COVERAGE PERCENTAGE (LOTS J, K & L ONLY): LOT J 9,156 SQ. FT. = 25.5%
- LOT K 20,370 SQ. FT. = 22.2% LOT L 29,484 SQ. FT. = 18.5%
- J. DPZ FILE REFERENCES: F-05-058, FDP-240, PB-360, 24-4284-D, S-03-05, WP-04-113, WP-04-135, SDP-04-163, F-06-102, F-06-203, WP-06-110, 24-4376-D, F-07-119.

SHEET INDEX

	_ •
1	COVER SHEET
2	SITE DETAILS
3	SITE DETAILS
4	SITE DETAILS
5	EXISTING CONDITIONS/ DEMOLITION PLAN
6	EXISTING CONDITIONS/ DEMOLITION PLAN
7	SITE LAYOUT PLAN
8	SITE LAYOUT PLAN
9	GRADING PLAN
10	GRADING PLAN
11	UTILITY PLAN
12	UTILITY PLAN
13	STORM DRAIN PROFILES
14	SANITARY SEWER PROFILES
15	EXISTING DRAINAGE AREA MAP
16	PROPOSED DRAINAGE AREA MAP
17	STORM DRAIN DRAINAGE AREA MAP
18	SWM POND #1 PLAN, PROFILE & DETAILS
19	SWM POND #1 PROFILES & DETAILS
20	SWM POND #2 PLAN, PROFILE & DETAILS
21	SWM POND #2 PROFILES & DETAILS
22	EROSION & SEDIMENT CONTROL PLAN
23	EROSION & SEDIMENT CONTROL PLAN
24	ESC NOTES & DETAILS
25	ESC DETAILS
26	MD-378 POND NOTES
27	LANDSCAPE PLAN & DETAILS
28	LANDSCAPE PLAN & DETAILS
29	GEO-TECHNICAL REPORT INFORMATION
30	STRUCTURAL RETAINING WALL PLAN & DETAILS
31	SEGMENTAL RETAINING WALL PLAN & GENERAL NOTES
32	SEGMENTAL RETAINING WALL PROFILE & SECTION
33	SEGMENTAL RETAINING WALL TYPICAL DETAILS

A BENCHMARKS

LOT: PARCEL: UNIT:

GEODETIC SURVEY CONTROL: 37GD
N 553,237.204
E 1,372,353.605
ELEVATION: 290.931
LOCATED 30' EAST OF FIRE HYDRANT ACROSS
FROM DEEP RUN ELEMENTARY SCHOOL

GEODETIC SURVEY CONTROL: 37GC N 555,250.791 E 1,370,946.348 ELEVATION: 331.855 LOCATED 30' EAST ON THE SOUTH SIDE OF OLD

WATERLOO ROAD ACROSS FROM DEEP RUN PARKWAY

THE WAY COLUMBIA

GATEWAY

GATEWAY

COMMERCE

GATEWAY

COMMERCE

GATEWAY

COMMERCE

GATEWAY

COLUMBIA

COMMERCE

GATEWAY

COLUMBIA

COMMERCE

GATEWAY

COLUMBIA

COMMERCE

GATEWAY

COLUMBIA

COLUMB

VICINITY MA

***** ADJACENT PROPERTY CHART

NO.:	OWNER:	TAX MAP:	PARCEL:	PREMISES ADDRESS:
1	FRANK E. RHODES & WIFE	37	487	6686 WATERLOO ROAD, ELKRIDGE, MD 21075
2	HOWARD RESEARCH & DEVELOPMENT	37	382-F	LARK BROWN ROAD ELKRIDGE, MD 21075
3	HOWARD RESEARCH & DEVELOPMENT	37	382-I	RT 175 ELKRIDGE, MD 21075
4	HOWARD RESEARCH & DEVELOPMENT	37	382-Q	OLD WATERLOO RD ELKRIDGE, MD 21075
5	HOWARD COUNTY MD PUBLIC RECREATION	43	1	6951 WATERLOO RD ELKRIDGE, MD 21075
6	WATERLOO MIDDLE SCHOOL C/O HOWARD COUNTY BOARD OF EDUCATION	37	168	6925 E OLD WATERLOO ROAD ELKRIDGE, MD 21075
7	SHERWOOD CROSSING APARTMENTS LLC	37	637	E OLD WATERLOO ROAD ELKRIDGE, MD 21075
8	SANCTUARY HOMEOWNERS ASSOCIATION, INC.	37 .	700-59	S SANCTUARY COURT ELKRIDGE, MD 21075
9	PATRICK ALEY	37	700-58	6719 OLD WATERLOO ROAD ELKRIDGE, MD 21075
10	CHARLOTTE POWELL & JARED HEALY	37	700-57	6715 OLD WATERLOO ROAD ELKRIDGE, MD 21075
11	LINDA BELLOFATTO SOPER	37	700-2	6693 OLD WATERLOO ROAD ELKRIDGE, MD 21075
12	GLENN & CHERYL HAYES	37	700-1	6691 OLD WATERLOO ROAD ELKRIDGE, MD 21075

PREMISES ADDRESS:

SITE ADDRESS CHART

LOT: | PARCEL: | UNIT: |

L	382	1	8896 PURPLE IRIS LANE, ELKRIDGE, MD 21075	L	382	34	8857 PURPLE IRIS LANE, ELKRIDGE, MD 21075
L	382	2	8894 PURPLE IRIS LANE, ELKRIDGE, MD 21075	L	382	35	8855 PURPLE IRIS LANE, ELKRIDGE, MD 21075
L	382	3	8892 PURPLE IRIS LANE, ELKRIDGE, MD 21075	L	382	36	8853 PURPLE IRIS LANE, ELKRIDGE, MD 21075
L	382	4	8890 PURPLE IRIS LANE, ELKRIDGE, MD 21075	L	382	37	8851 PURPLE IRIS LANE, ELKRIDGE, MD 21075
L	382	5	8888 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	38	8754 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	6	8884 PURPLE IRIS LANE, ELKRIDGE, MD 21075	K	382	39	8756 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	7	8882 PURPLE IRIS LANE, ELKRIDGE, MD 21075	K	382	40	8758 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	8	8880 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	41	8759 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	9	8878 PURPLE IRIS LANE, ELKRIDGE, MD 21075	K	382	42	8757 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	10	8866 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	43	8755 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	11	8864 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	44	8753 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	12	8862 PURPLE IRIS LANE, ELKRIDGE, MD 21075	K	382	45	8749 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	13	8860 PURPLE IRIS LANE, ELKRIDGE, MD 21075	K	382	46	8747 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	14	8856 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	47	8745 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	15	8854 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	48	8743 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	16	8852 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	49	8739 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	17	8850 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	50	8737 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	18	8893 PURPLE IRIS LANE, ELKRIDGE, MD 21075	K	382	51	8735 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	19	8891 PURPLE IRIS LANE, ELKRIDGE, MD 21075	K	382	52	8733 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	20	8889 PURPLE IRIS LANE, ELKRIDGE, MD 21075	K	382	53	8731 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	21	8887 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	54	8732 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	22	8885 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	55	8734 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	23	8883 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	56	8736 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	24	8881 PURPLE IRIS LANE, ELKRIDGE, MD 21075	К	382	57	8738 BLAZING BROOK WAY, ELKRIDGE, MD 21075
L	382	25	8879 PURPLE IRIS LANE, ELKRIDGE, MD 21075	J	382	58	8621 BLUE SMOKE COURT, ELKRIDGE, MD 21075
L	382	26	8875 PURPLE IRIS LANE, ELKRIDGE, MD 21075	J	382	59	8619 BLUE SMOKE COURT, ELKRIDGE, MD 21075
L	382	27	8873 PURPLE IRIS LANE, ELKRIDGE, MD 21075	J	382	60	8617 BLUE SMOKE COURT, ELKRIDGE, MD 21075
L	382	28	8871 PURPLE IRIS LANE, ELKRIDGE, MD 21075	J	382	61	8615 BLUE SMOKE COURT, ELKRIDGE, MD 21075
L	382	29	8869 PURPLE IRIS LANE, ELKRIDGE, MD 21075	J	382	62	8609 BLUE SMOKE COURT, ELKRIDGE, MD 21075
L	382	30	8865 PURPLE IRIS LANE, ELKRIDGE, MD 21075	J	382	63	8607 BLUE SMOKE COURT, ELKRIDGE, MD 21075
L	382	31	8863 PURPLE IRIS LANE, ELKRIDGE, MD 21075	J	382	64	8605 BLUE SMOKE COURT, ELKRIDGE, MD 21075
L	382	32	8861 PURPLE IRIS LANE, ELKRIDGE, MD 21075	J	382	65	8603 BLUE SMOKE COURT, ELKRIDGE, MD 21075
L	382	33	8859 PURPLE IRIS LANE, ELKRIDGE, MD 21075	J	382	66	8601 BLUE SMOKE COURT, ELKRIDGE, MD 21075

PERMIT INFORMATION CHART

ŀ			LOTS		PARCEL	NO.			
GATEWAY OVE	PARCEL 3 ERLOOK AT			J,	K & L		PARCEL	382	
LAT # OR L/F	LAT # OR L/F GRID # ZONE TAX MAP NO. ELECT. DIST. CENSUS TRAC							TRACT	
1888518887 20, 21 NT 31				& 4.	3	6		6067	03
WATER CODE:						SEWE	R CO	DE:	

WATER CODE: D-06

MORRIS & RITCHIE ASSOCIATES, INC.

MRA

14280 PARK CENTER DRIVE, SUITE A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690 FAX (410) 792-7395

ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

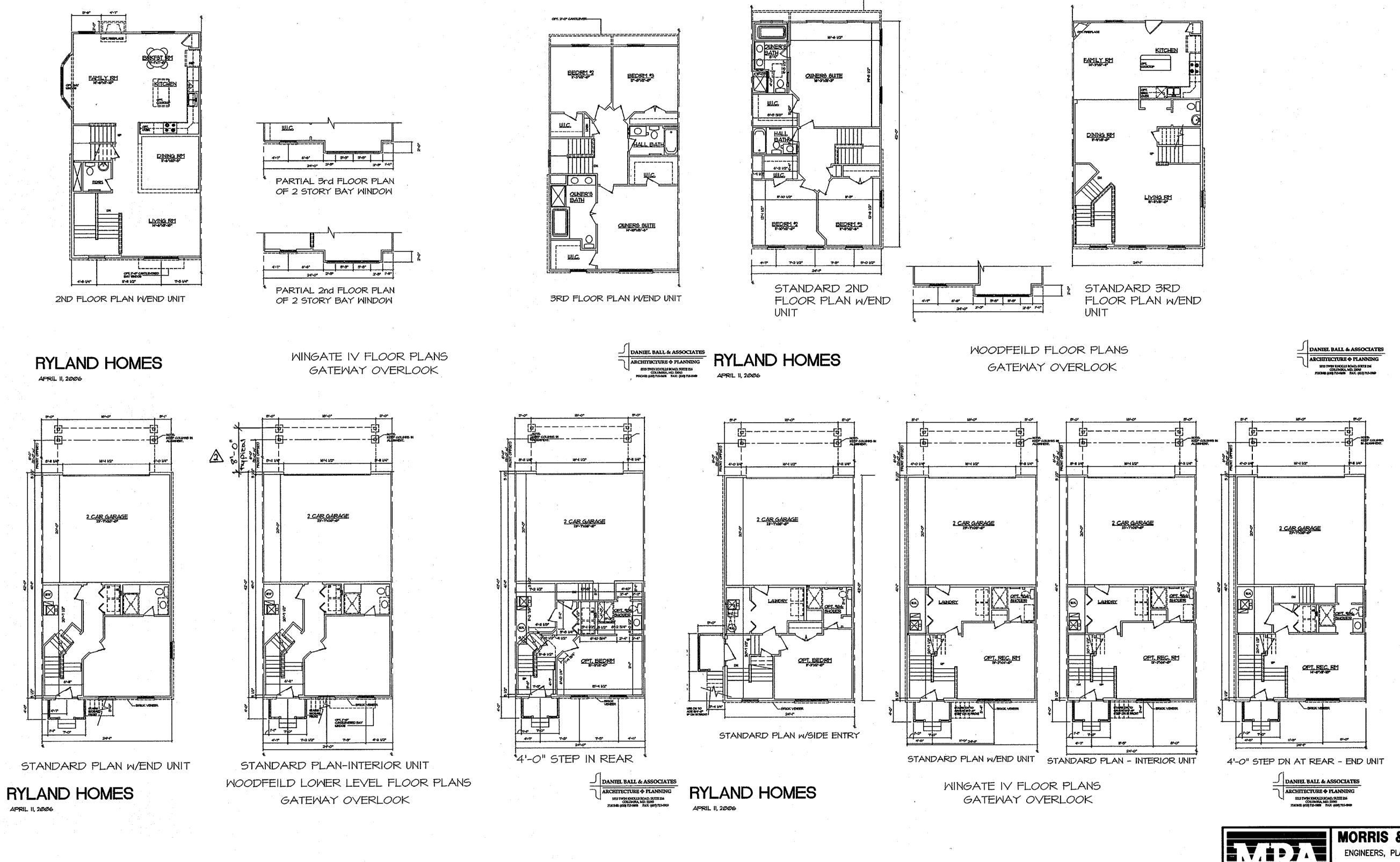
COVER SHEET

LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382

ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3

OTS: J. K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY. MARYLAND

	_	The state of the second st	Wate 00011111;	THE UTIL UTIL
	DATE	REVISIONS	JOB NO.:	14719
OWNER/DEVELOPER/APPLICANT	:		SCALE:	AS SHOWN
			DATE:	4/24/07
NAME: MR. BRIAN KNAUFF COMPANY: THE RYLAND GROUP			DRAWN BY:	MSP
6011 UNIVERSITY BOULEVARD, SUITE 260			DESIGN BY:	MSP/KSK
ELLICOTT CITY, MARYLAND 21043 PHONE: 410-480-2467			REVIEW BY:	KSK
FAX: 410-480-0543			SHEET:	1 OF 33





APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

CHIEF, DEVELOPMENT ENGINEERING DIVISION

CHIEF, DIVISION OF LAND DEVELOPMENT

DATE

DIRECTOR, DEPARTMENT OF PLANNING AND ZONING

DATE

G OF MARIAN G

MORRIS & RITCHIE ASSOCIATES, INC.
ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

14280 PARK CENTER DRIVE, SUITE A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690 FAX (410) 792-7395

SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

SITE DETAILS

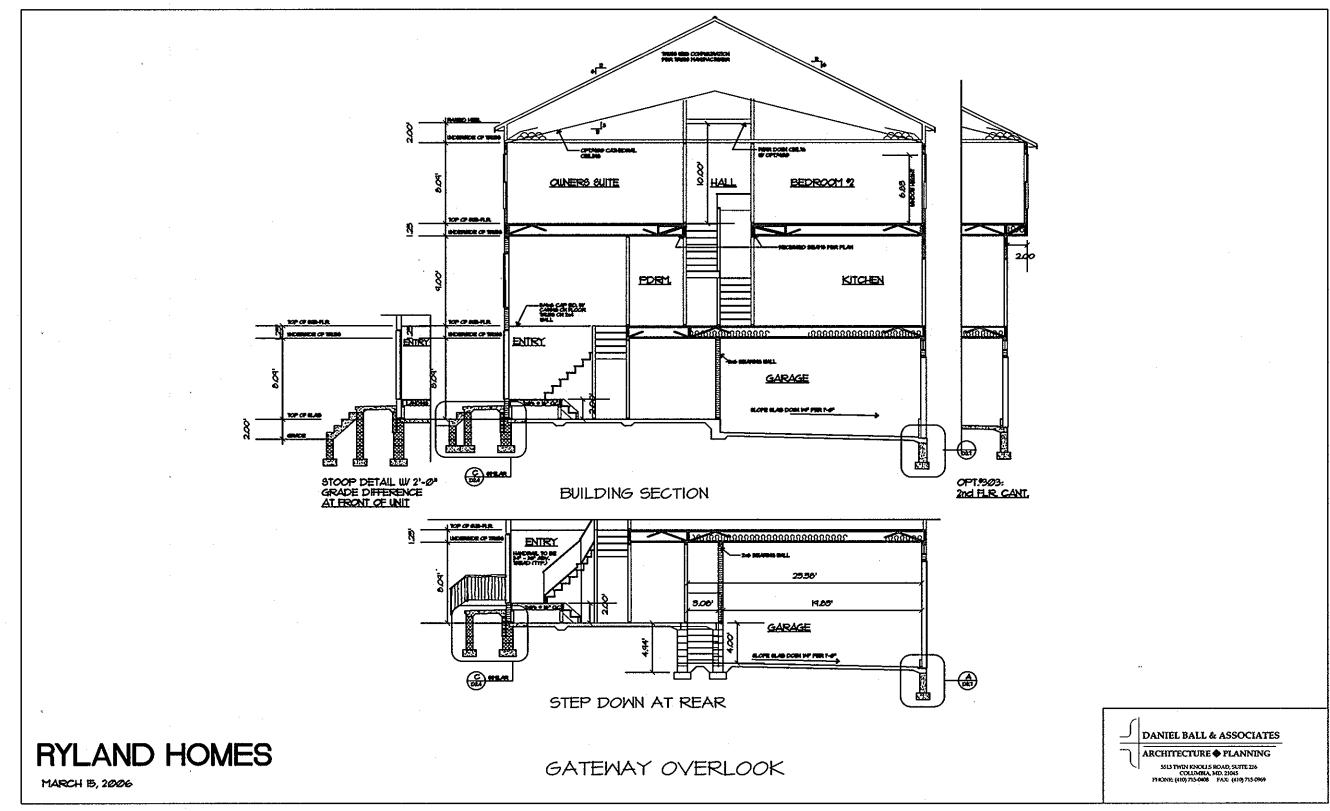
LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382

ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3

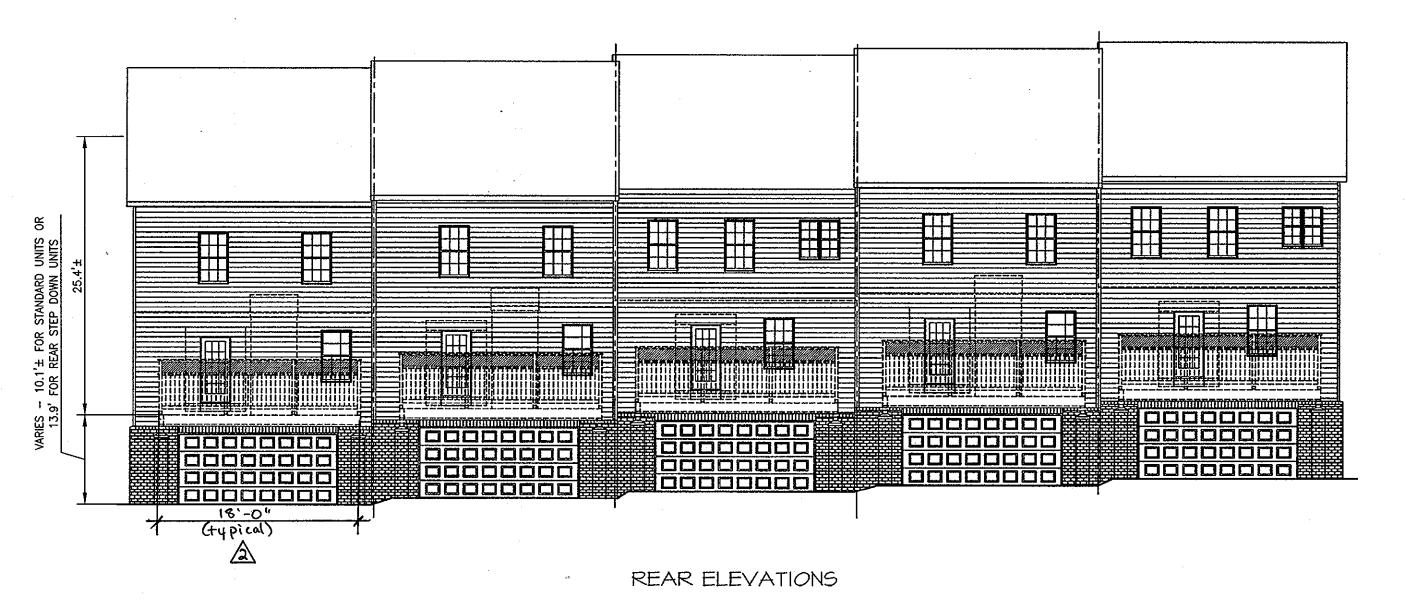
OTS: J, K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

	DATE	REVISIONS	JOB NO.:	14719
OWNER/DEVELOPER/APPLICANT	12/3/07	ADDED DECK DIMENSIONS	SCALE:	N.T.S.
			DATE:	4/24/07
NAME: MR. BRIAN KNAUFF COMPANY: THE RYLAND GROUP			DRAWN BY:	MM/MSP
6011 UNIVERSITY BOULEVARD, SUITE 260			DESIGN BY:	KSK
ELLICOTT CITY, MARYLAND 21043 PHONE: 410-480-2467			REVIEW BY:	KSK
FAX: 410-480-0543			SHEET:	2 OF 33









RYLAND HOMES

GATEWAY OVERLOOK

DANIEL BALL & ASSOCIATES

ARCHITECTURE & PLANNING

SUST TWIN NOULS ROAD, SUITE 216
COLUMNA NO. 2005
PHONE (410) 715-0400

1AX (410) 715-0400

RYLAND HOMES
MARCH 15, 2006

GATEWAY OVERLOOK

DANIEL BALL & ASSOCIATES

ARCHITECTURE & PLANNING

SID TWN INDIA BOAD SITE 246
COLUMNIA BOAD 2804

NOTE: DIMENSIONS SHOWN ARE FOR INFORMATIONAL PURPOSES ONLY. SEE ARCHITECTURAL PLANS FOR EXACT UNIT DIMENSIONS.



APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

HIEF, DEVELOPMENT ENGINEERING DIVISION

HIEF, DIVISION OF LAND DEVELOPMENT

DATE

JEFCTOR, DEPARTMENT OF PLANNING AND ZONING

DATE



MORRIS & RITCHIE ASSOCIATES, INC.
ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

14280 PARK CENTER DRIVE, SUITE A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690 FAX (410) 792-7395

OF MAA 2 SANA 3 SANA

SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

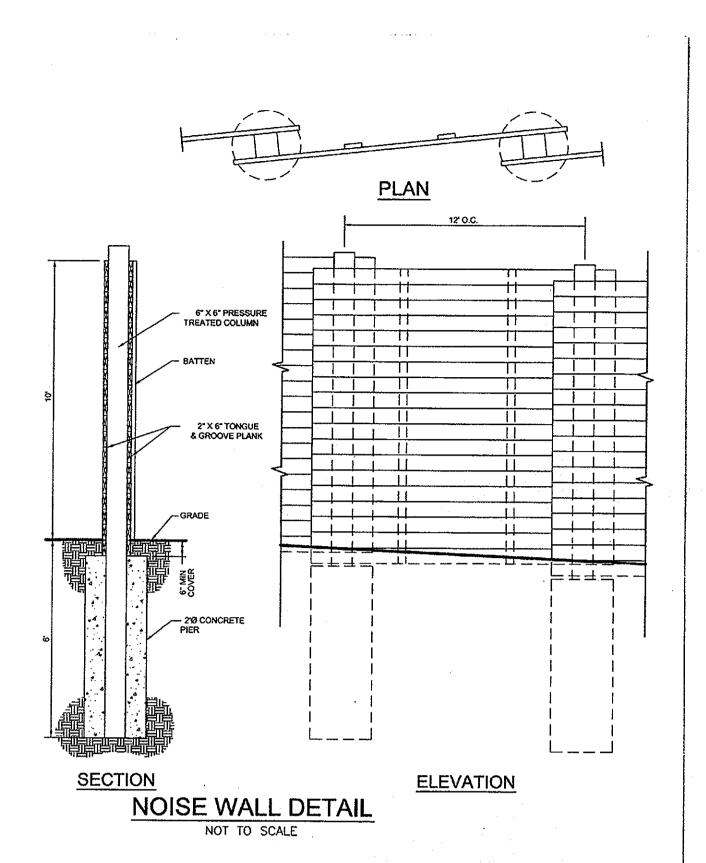
SITE DETAILS

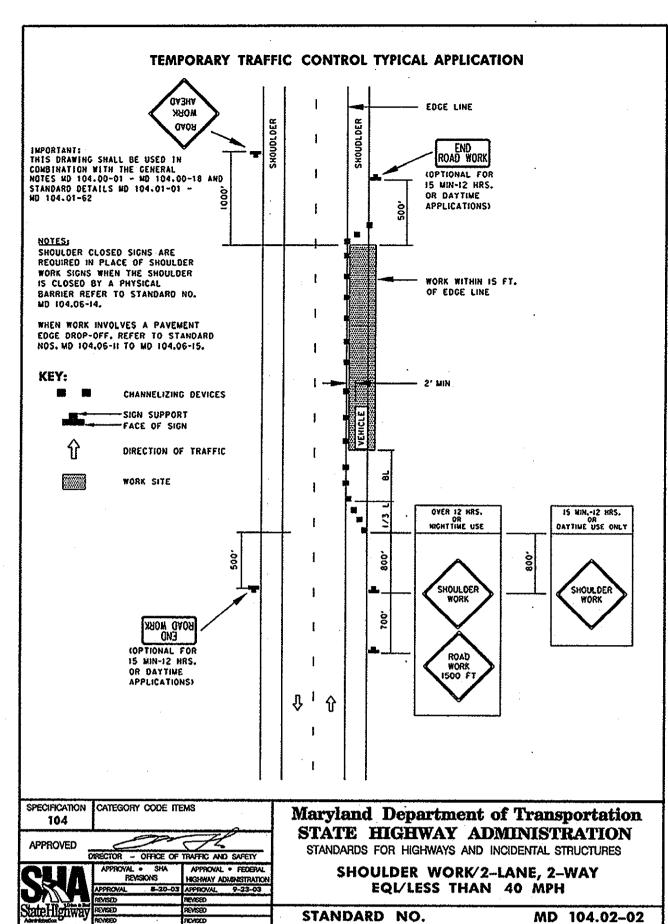
LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382

ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3

LOTS: J, K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

	DATE	REVISIONS	JOB NO.:	14719
OWNER/DEVELOPER/APPLICANT	12/3/07	ABDED DECK DIMENSIONS	SCALE:	N.T.S.
NAME NO COUNTY (ANALYSE	•		DATE:	4/24/07
NAME: MR. BRIAN KNAUFF COMPANY: THE RYLAND GROUP			DRAWN BY:	MM/MSP
6011 UNIVERSITY BOULEVARD, SUITE 260			DESIGN BY:	KSK
ELLICOTT CITY, MARYLAND 21043 PHONE: 410-480-2467			REVIEW BY:	KSK
FAX: 410-480-0543			SHEET:	3 OF 33





-6"x6" x W2.0 MESH

CRUSHED GRAVEL (CR-6)

APPROVED SUBGRADE

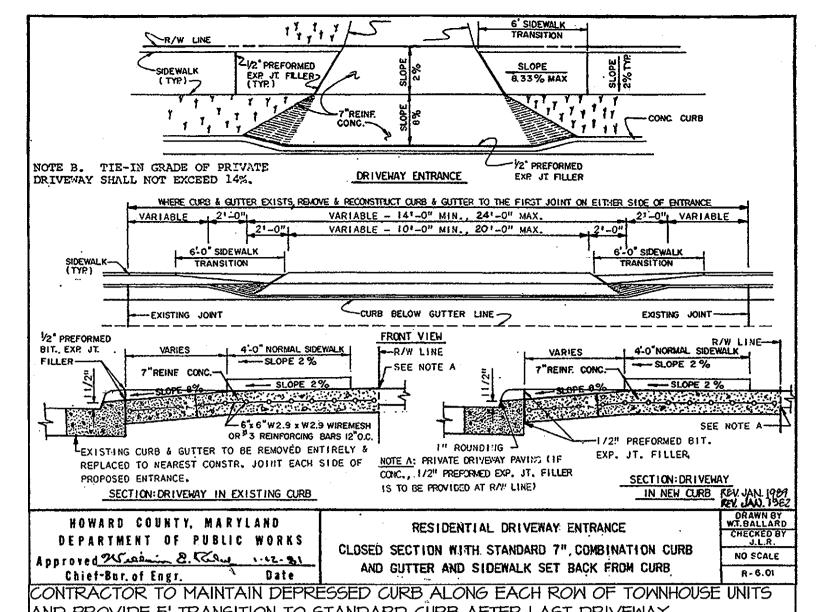
SIDEWALK JOINTS SHALL BE ARRANGED SUCH THAT SQUARE PANELS ARE FORMED AND NO PANEL IS LARGER OR SMALLER THAN 4' UNLESS OTHERWISE NOTED.

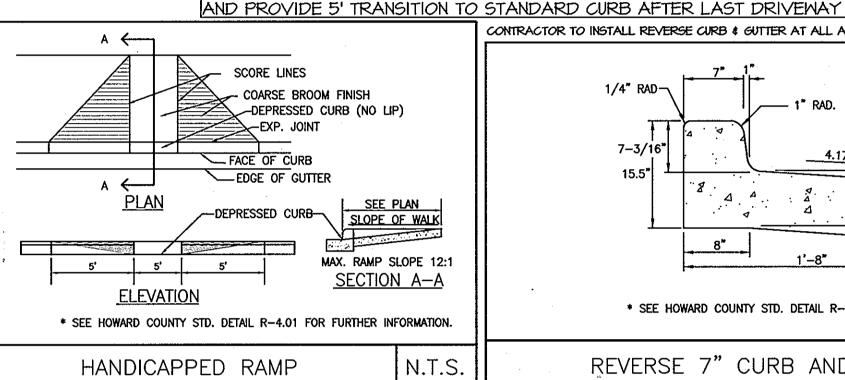
EXPANSION JOINTS TO BE PROVIDED EVERY 50'

SIDEWALK SECTION

3. SEE HOWARD COUNTY STD. DETAIL R-3.05 FOR FURTHER

CONCRETE





BARRIER CURB, SEE HOWARD COUNTY

CONCRETE CURB

DETAIL, R-3.03

CURB OPENING DETAIL

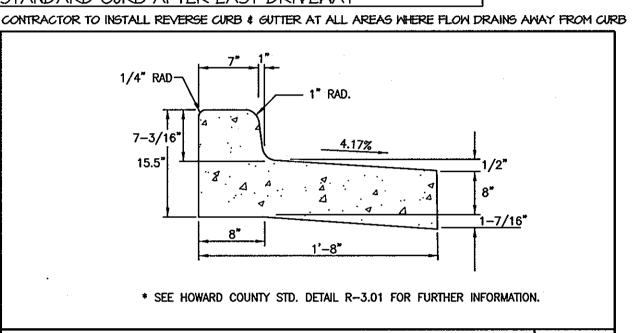
8" THICK CONCRETE CHANNEL

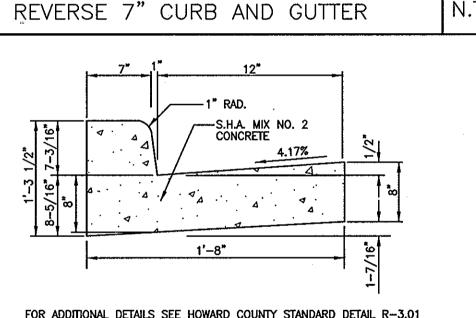
EXPANSION

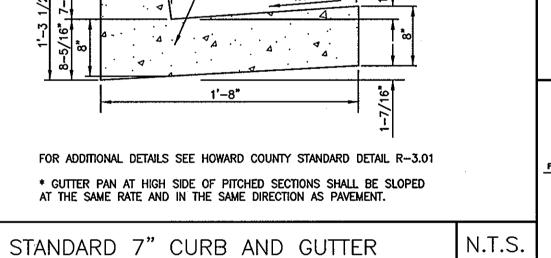
N.T.S.

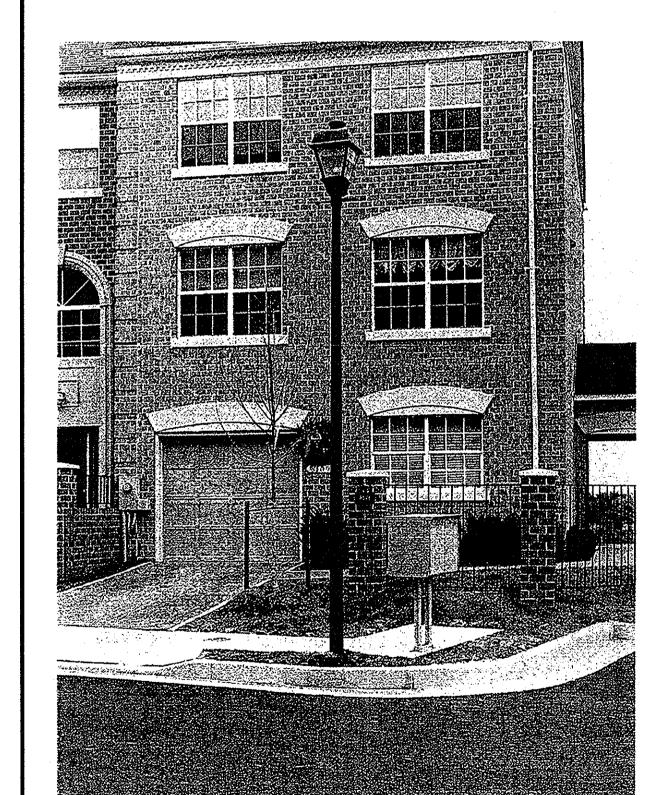
EXPANSION

JOINT











ON-SITE LIGHTING DETAIL *ON-SITE LIGHTING TO BE PROVIDED BY CONSTELLATION ENERGY/BGE. *LIGHTING AT ENTRANCES ON OLD WATERLOO ROAD TO BE SHALL BE 150-WATT HPS VAPOR "PREMIER" POST TOP FIXTURES MOUNTED ON A 14-FOOT BLACK FIBERGLASS POLE.

PLANNING PORED

of HOWARD COUNTY

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

1000 10 A C - 200 - -Colonied GE LIGHTING SYSTEMS - ISO-ILLUMINATION DIAGRAM
SALEM TR1R
QTY: 1 7679
Lamp: HPS 50-150
Mounting Height: 14.0
Scale: I Inch = 40.0 Pole is Located at X=0, Y=0
Photometric Axis is Aimed Out 0.0 from the Pole Base. 12000 M-S-I) Photometric Axis is Aimed Out --90 -130 APPROVED

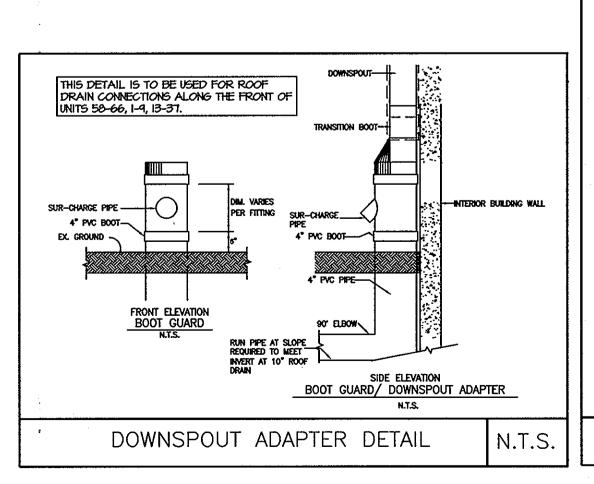
BITUMINOUS CONCRÉTE SURFACE COURSE (SC)

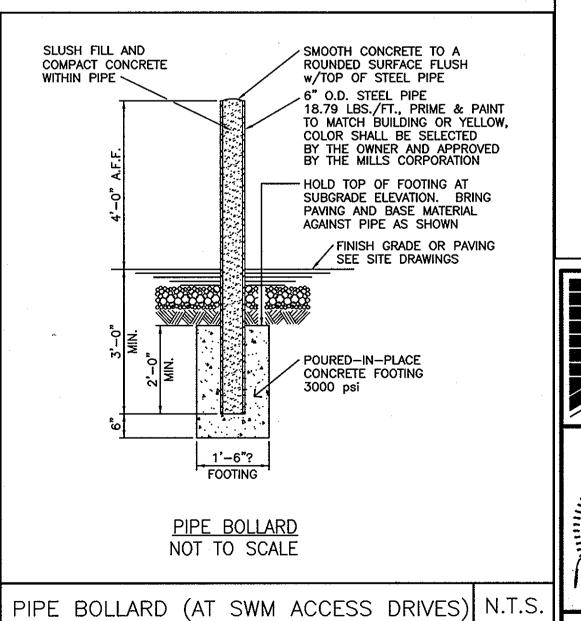
SEE HOWARD COUNTY STD. DETAIL R-2.01 FOR ADDITIONAL INFORMATION.

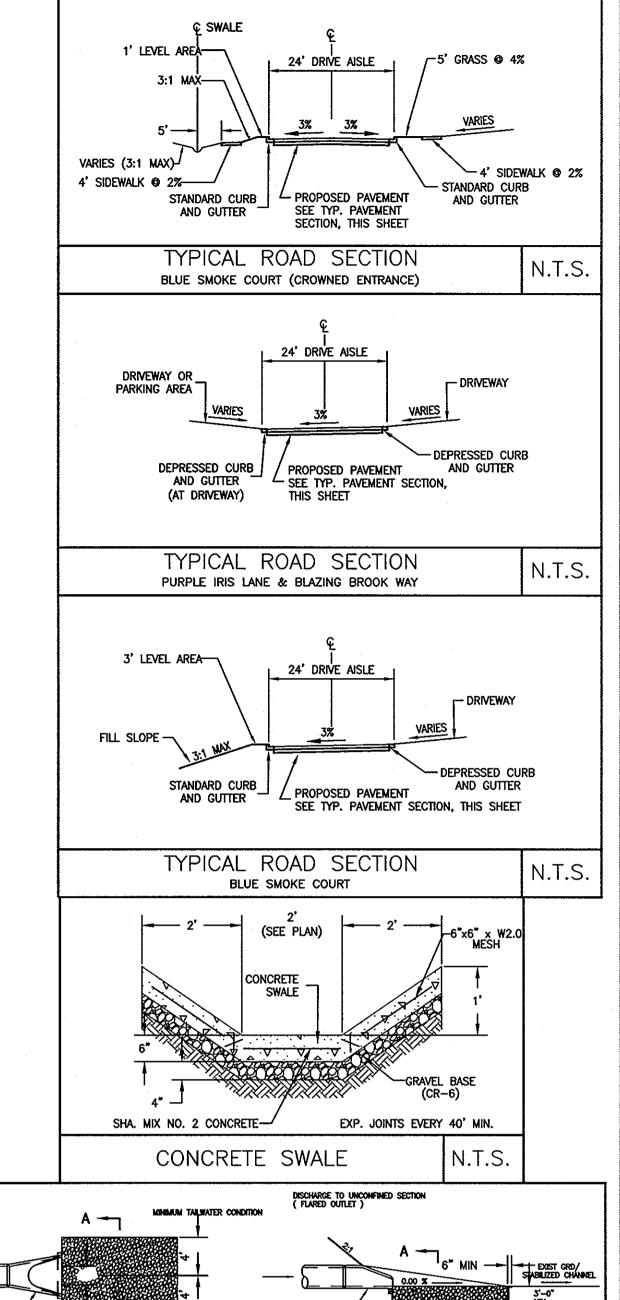
P-2 PAVING SECTION

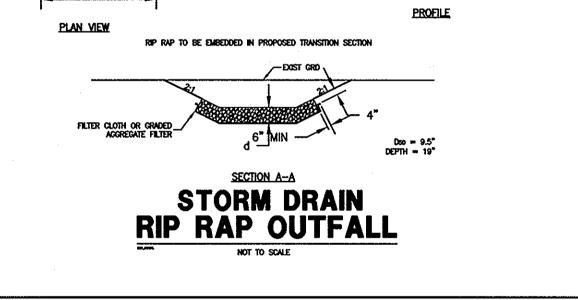
BRUMINOUS CONCRETE BASE COURSE (80)

DENSE GRADED AGGREGATE BASE (GAB) APPROVED SUBGRADE











MORRIS & RITCHIE ASSOCIATES, INC.

ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

14280 PARK CENTER DRIVE, SUITE A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690 FAX (410) 792-7395

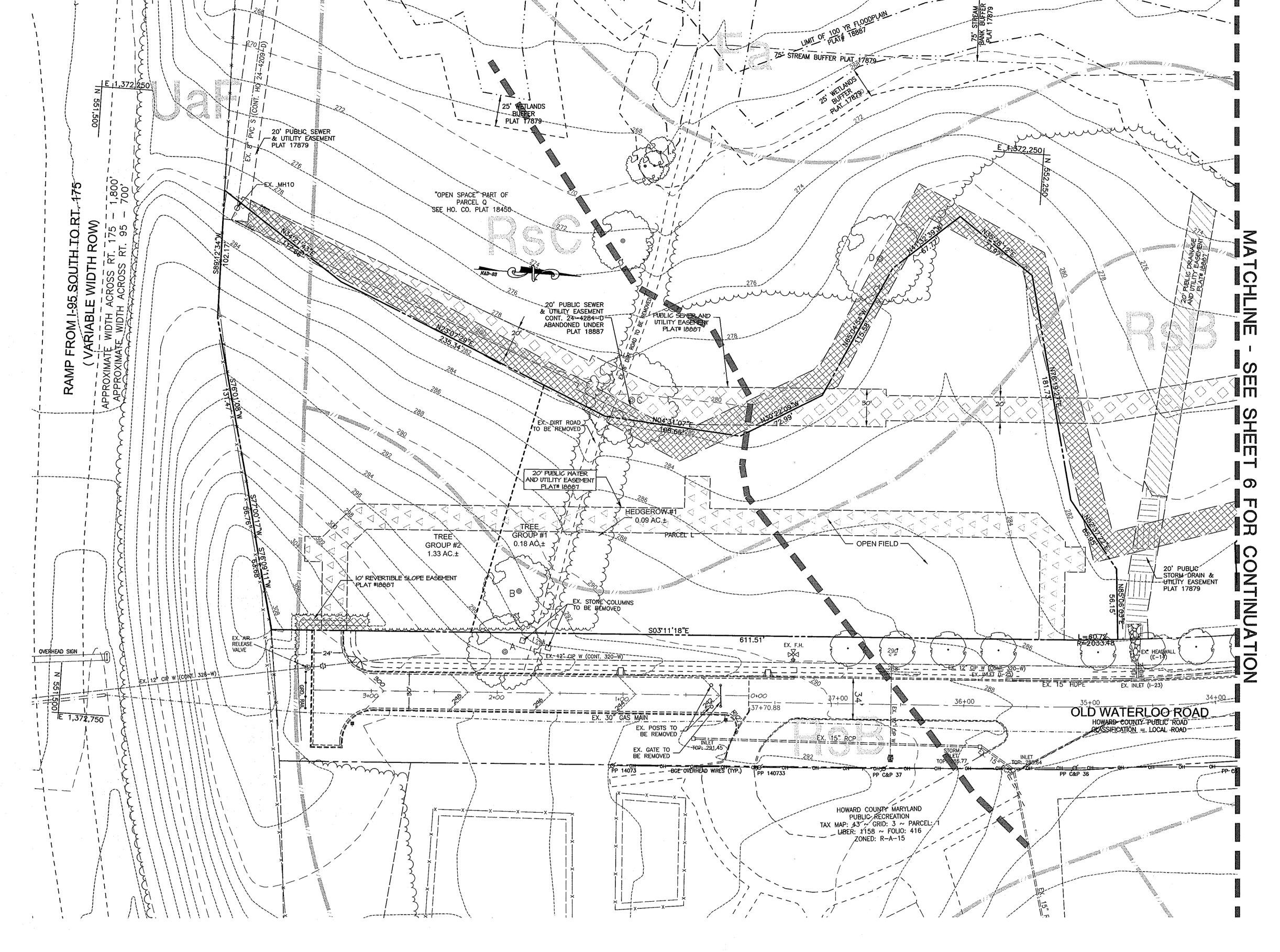
SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

SITE DETAILS

LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382 ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3 1240 LOTS: J. K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

OWNER/DEVELOPER/APPLICANT NAME: MR. BRIAN KNAUFF COMPANY: THE RYLAND GROUP 6011 UNIVERSITY BOULEVARD, SUITE 260 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-480-2467 FAX: 410-480-0543

DATE **REVISIONS** JOB NO.: 14719 SCALE: N.T.S. DATE: 4/24/07 DRAWN BY: MM/MSP DESIGN BY: KSK REVIEW BY: SHEET: 4 OF 33



LEGEND

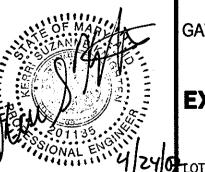
	EX. 2025 65dBA LINE
258 260	EX. 2' CONTOUR
	EX. 10' CONTOUR EX. BUILDING
	EX. CURB
	EX. EASEMENT
xx	EX. FENCE
	EX. FLOODPLAIN EX. GAS
	EX. PAVEMENT
N41'08'00"E_598.31'	EX. PROPERTY LINE
EX. 8" S	ex. Road centerline ex. Sanitary sewer
	EX. STORM DRAIN
	EX. STREAM
	EX. 75' STREAM BUFFER
EX. 10" W	ex. Tree line ex. Waterline
\bowtie	EX. FIRE HYDRANT
—··—·'·—··—	EX. WETLAND
	EX. WETLAND BUFFER
P. CHE TO THE BUY FOR THE PARK THE	SOIL TYPE
	PUBLIC SD & UTILITY ESMIT
	PUBLIC SEWER & UTILITY ESMT
	PUBLIC WATER & UTILITY ESMIT
[<u>////////////////////////////////////</u>	PUBLIC WATER, SEWER, & UTILITY ESMIT
	ABANDONED 20' PUBLIC SEWER & UTILITY ESMIT
	20' PUBLIC SD & UTILITY ESMIT
	ABANDONED 20' PUBLIC SD & UTILITY ESMT



MORRIS & RITCHIE ASSOCIATES, INC.

ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

14280 PARK CENTER DRIVE, SUITE A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690 FAX (410) 792-7395



OWNER/DEVELOPER/APPLICANT

ELLICOTT CITY, MARYLAND 21043

6011 UNIVERSITY BOULEVARD, SUITE 260

NAME: MR. BRIAN KNAUFF COMPANY: THE RYLAND GROUP

PHONE: 410-480-2467 FAX: 410-480-0543 SITE DEVELOPMENT PLAN
GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD
66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

EXISTING CONDITIONS & DEMOLITION PLAN

LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382

ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3

LOTS: J, K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

DATE	REVISIONS	JOB NO.:	14719
		SCALE:	1"=40'
		DATE:	4/24/07
		DRAWN BY:	MM/MSP
		DESIGN BY:	KSK
		REVIEW BY:	KSK
		SHEET:	5 OF 33

	SOILS LEGEND	
SYMBOL	NAME/DESCRIPTION	SOIL TYPE
Fa	FALLSINGTON, SANDY LOAM, 0-2% SLOPES	С
RsB	RUSSETT, FINE SANDY LOAM, 2-5% SLOPES	С
.RsC· ···	RUSSETT, FINE SANDY LOAM, 5-10% SLOPES	С
UaF	UDORTHENTS, HIGHWAY 0-65% SLOPES	С

URBAN LAND, UDORTHENTS COMPLEX 0-15% SLOPES

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

CHIEF, DEVELOPMENT ENGINEERING DIVISION

CHIEF, DIVISION OF LAND DEVELOPMENT

DIRECTOR, DEPARTMENT OF PLANNING AND ZONING

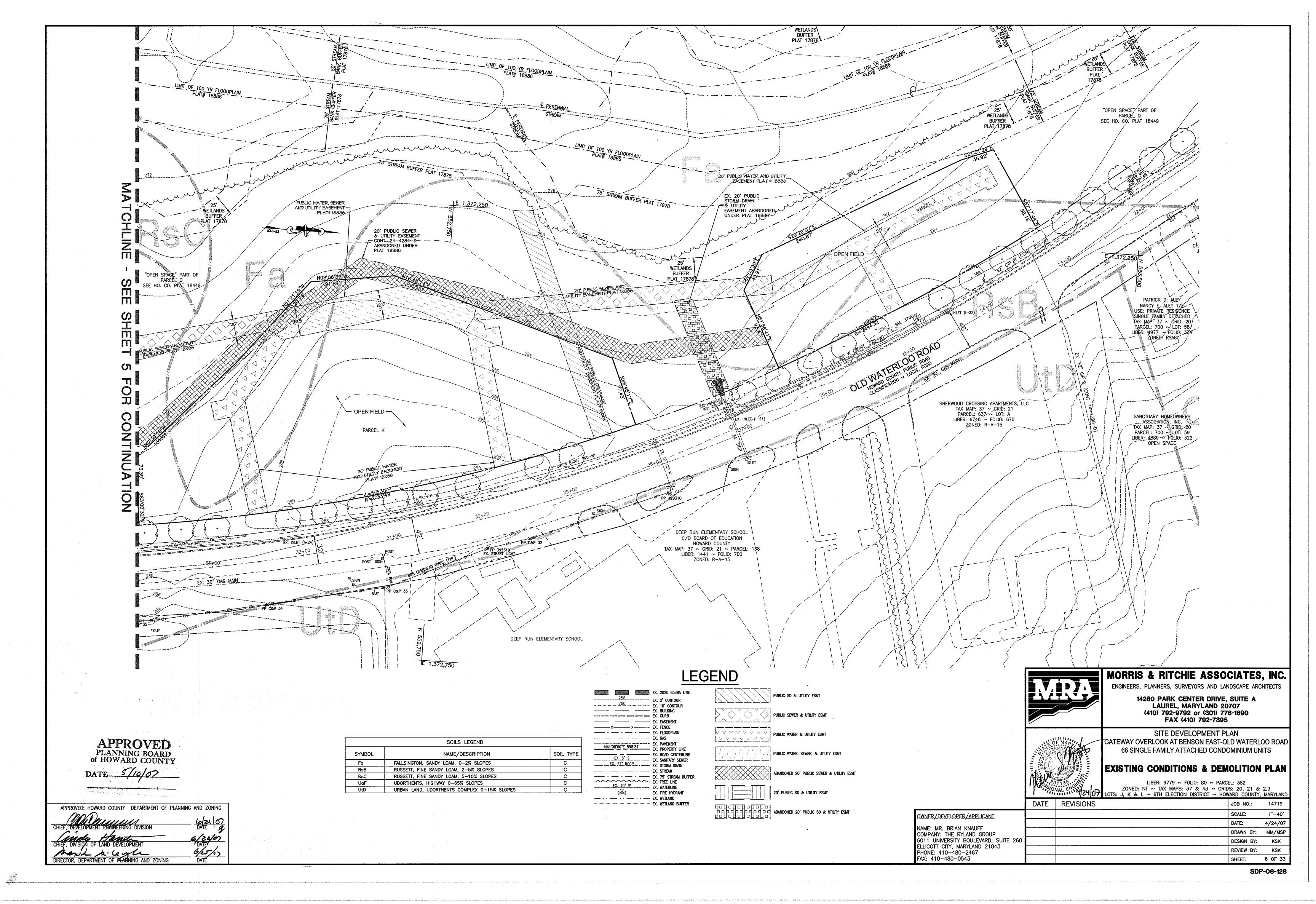
DATE

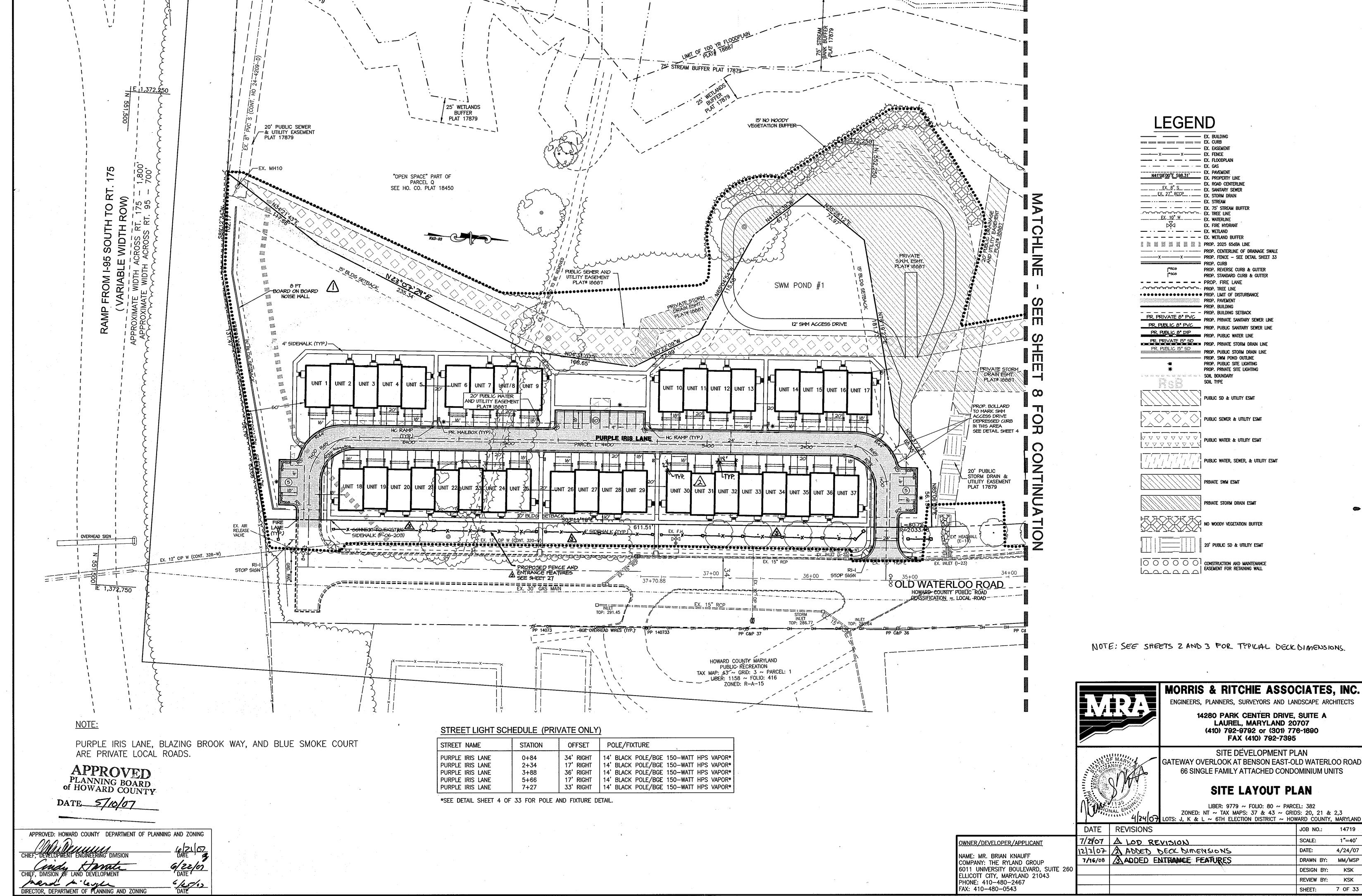
APPROVED
PLANNING BOARD
of HOWARD COUNTY

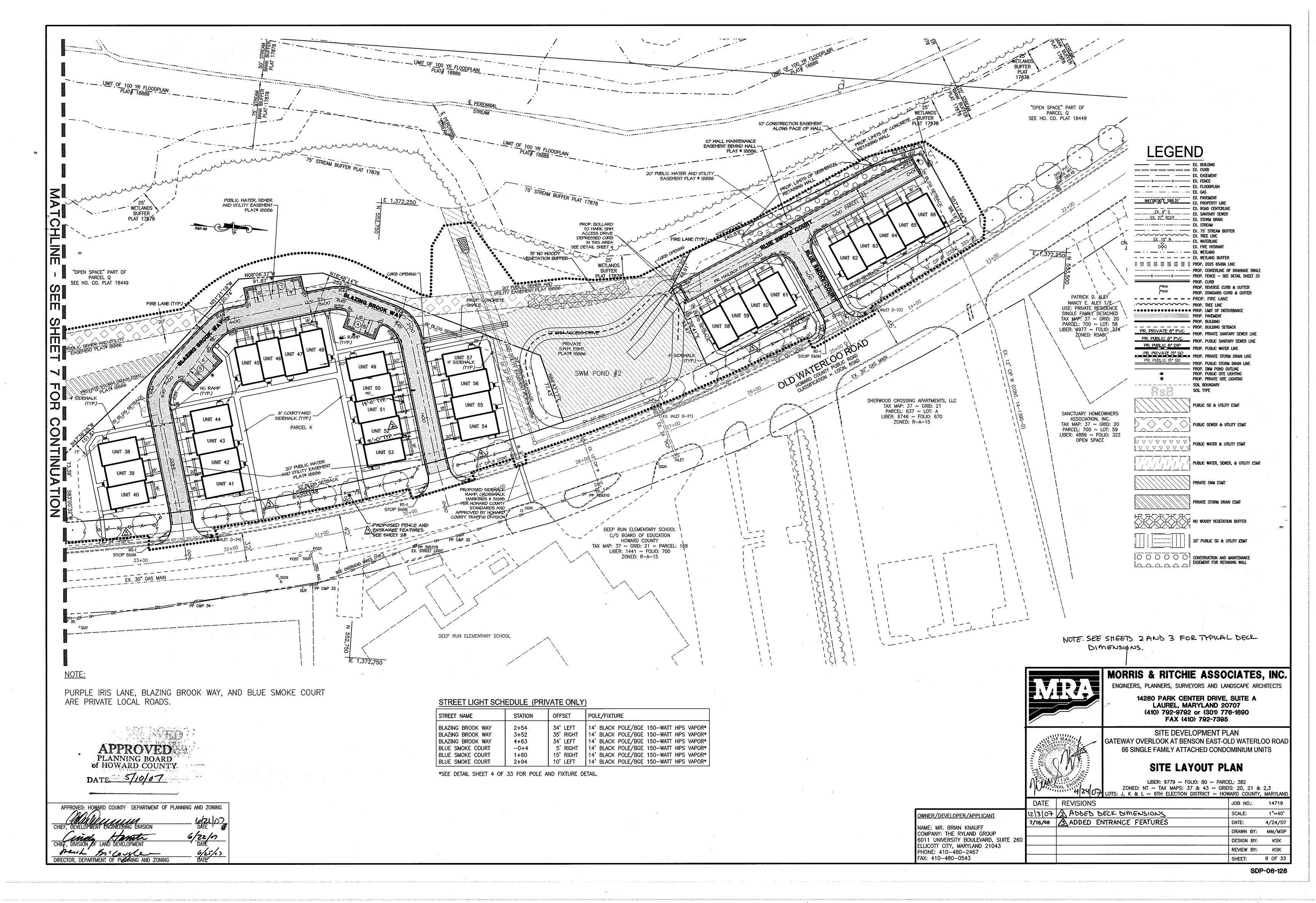
С

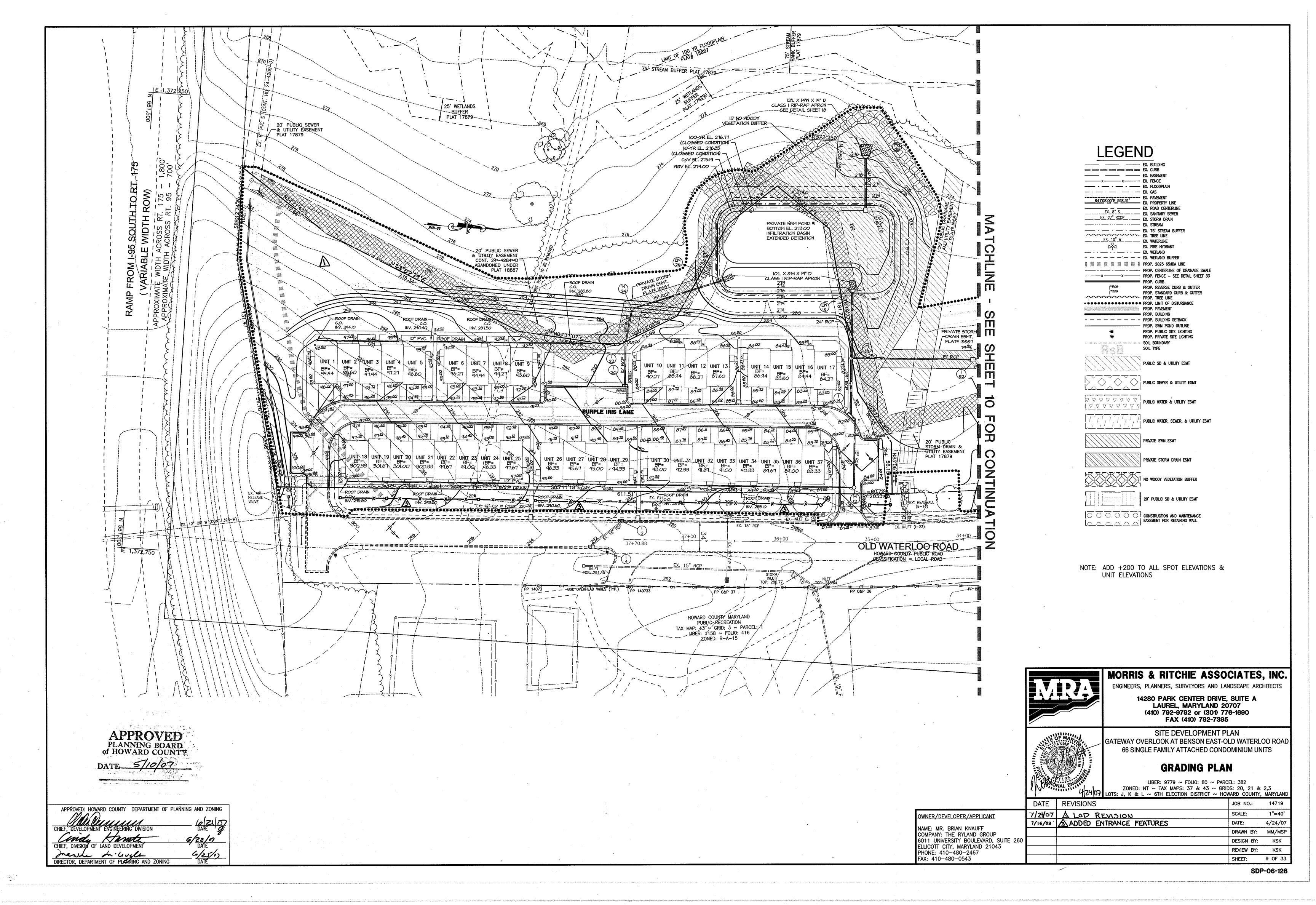
TYPE	AREA	DOMINANT SPECIES	AGE OF STAND (APPROX.)	CONDITION OF STAND	TYPICAL CALIPER
TREE GROUP #I	O.IB AC±	SOUTHERN RED OAKS	80-100 YRS	APPARENTLY HEALTHY	24"
TREE GROUP #2	1.33 AC±	TULIP TREE 40%, RED MAPLE 40%, EASTERN REDCEDAR 20%	20-30 YRS	APPARENTLY HEALTHY	2"-8"
HEDGERON #I	0.09 AC±	EASTERN REDCEDAR	50-70 YRS	APPARENTLY HEALTHY	6"-12"

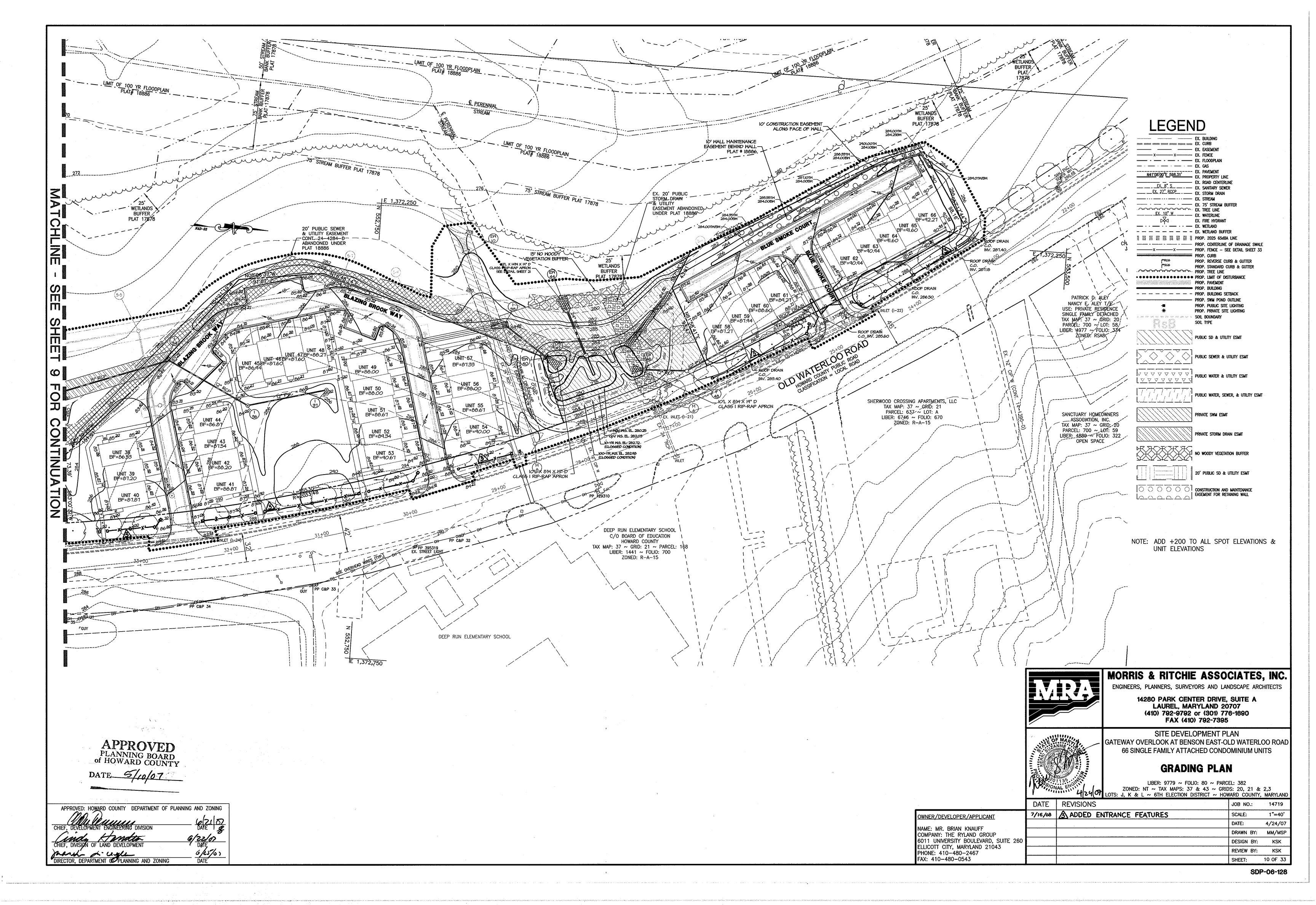
SPEC	CIMEN	SPECIMEN SPECIES	DBH	CONDITION	STATUS
	A	LIRIODENDRON TULIPIFERA - TULIPTREE	40"	POOR	TO BE REMOVED (F-06-203)
	В	QUERCUS FALCATA - SOUTHERN RED OAK	38"	FAIR	TO BE REMOVED
	C	QUERCUS MACROCARPA - BUR OAK	50"	POOR	TO BE REMOVED
	D	QUERCUS ALBA - WHITE OAK	46°	POOR/FAIR	TO BE REMOVED

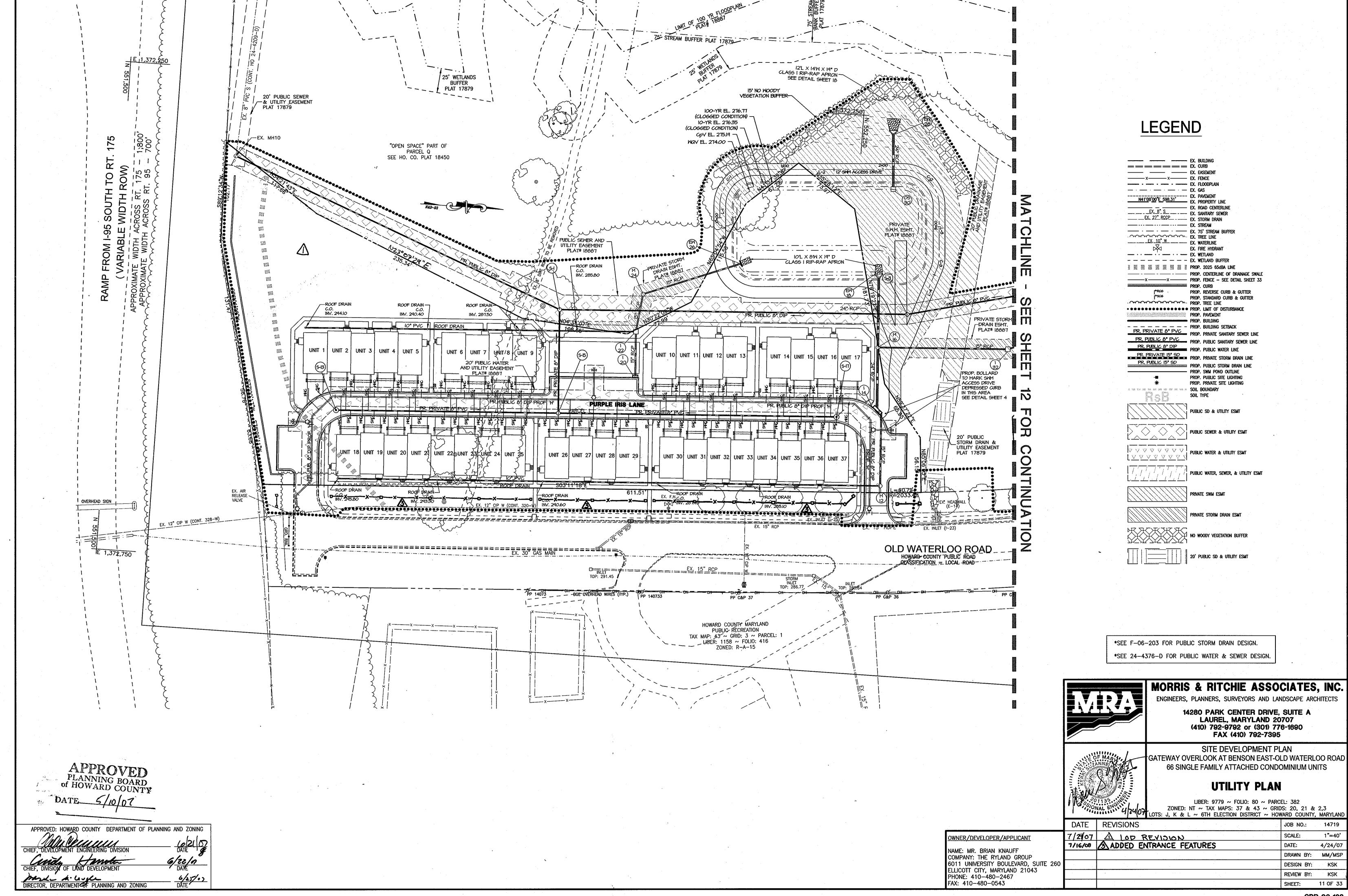


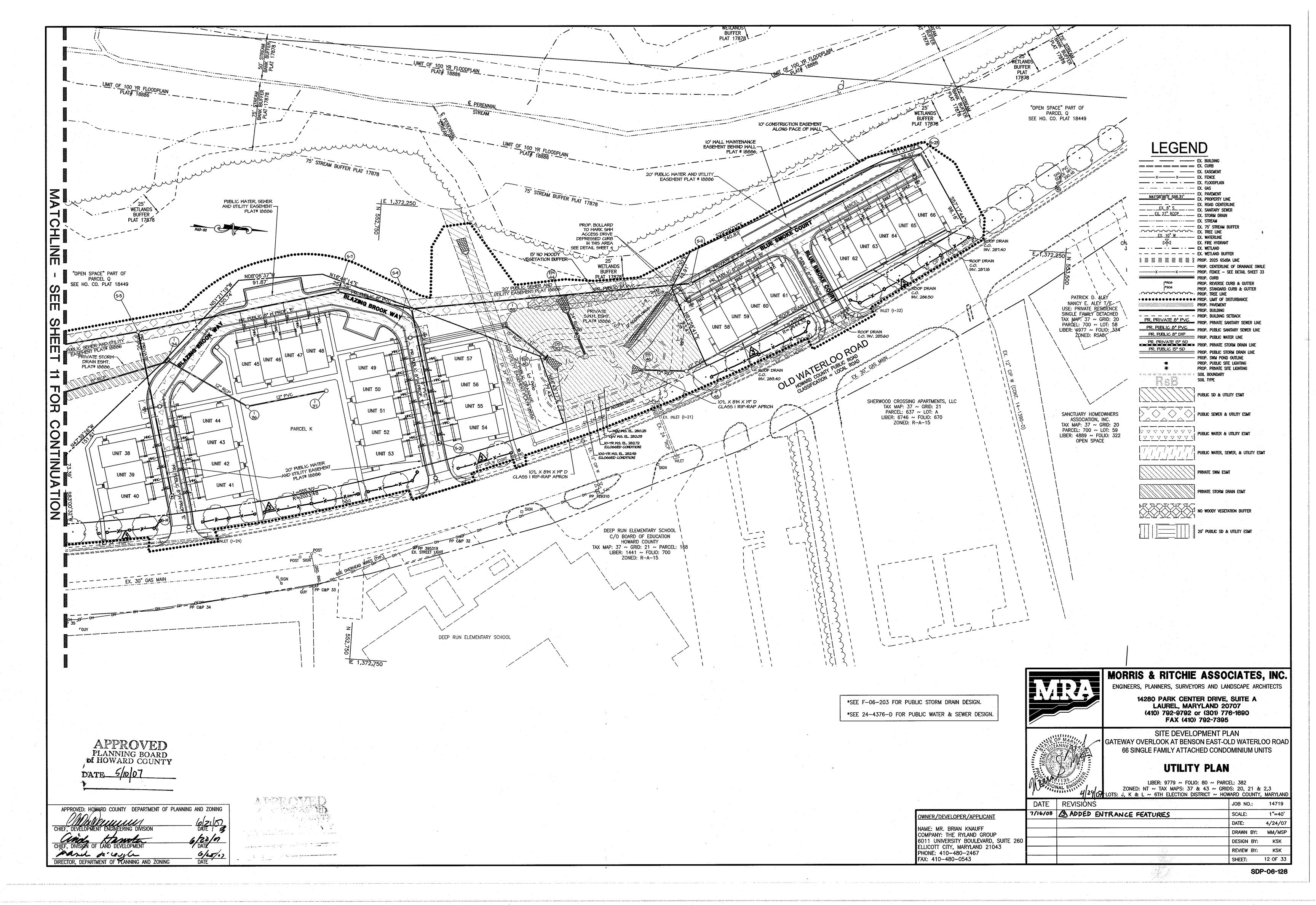












OCORDINATES TO CENTER OF STRUCTURE AT FLOWLINE FOR ALL INLETS & MANHOLES. * TOP ELEVATION FOR GRATE INLETS ARE AT CENTER OF STRUCTURE AT FLOWLINE; MANHOLE ELEVATIONS ARE AT CENTER OF RIM; A-5 & A-10 INLET ELEVATIONS ARE AT CENTER OF STRUCTURE AT TOP OF SLAB

	PRIVATE STORM DRAIN STRUCTURE SCHEDULE						● LOCATION	
STR NO.	* TOP ELEV	INV IN	INV IN	INV OUT	TYPE	REMARKS	NORTHING	EASTING
M-12	285.10		282.00	280.60	SHALLOW PRECAST MANHOLE, HOWARD COUNTY STANDARD DETAIL G5.12		552,293.3923	1,372,630.6226
I14	282.60	278.07		277.32	TYPE 'A-10' INLET, HOWARD COUNTY STANDARD DETAIL SD-4.41		552,285.1328	1,372,553.2589
M-16	282.70	274.17	274.82	274.07	STANDARD PRECAST MANHOLE, HOWARD COUNTY STANDARD DETAIL G5.12		552,281.0921	1,372,489.2552
EW-18	275.75	273.00			TYPE 'A' HEADWALL, HOWARD COUNTY STANDARD DETAIL SD-5.11		552,277.0678	1,372,417.9567
I-20	288.70			282.30	TYPE 'A-5' INLET, HOWARD COUNTY STANDARD DETAIL SD-4.40		552,036.2736	1,372,536.1069
1-22	288.00	280.00	284.00	279.90	SINGLE WR INLET, HOWARD COUNTY STANDARD DETAIL SD-4.37		552,030.7220	1,372,490.3016
M-24	284.50	278.30		278.20	STANDARD PRECAST MANHOLE, HOWARD COUNTY STANDARD DETAIL G5.12		552,036.7216	1,372,465.1894
EW-26	275.00	273.00			TYPE 'C' ENDWALL, HOWARD COUNTY STANDARD DETAIL SD-5.21		552,135.3223	1,372,413.5917
1-32	280.33	276.91		276.81	TYPE 'D' INLET, HOWARD COUNTY STANDARD DETAIL SD-4.39	(1)	552,413.4426	1,372,481.8723
I-34	282.85	279.02		278.77	TYPE 'A-5' INLET, HOWARD COUNTY STANDARD DETAIL SD-4.40		553,529.2461	1,372,437.2989
1-36	284.00	281.14		281.04	YARD INLET, HOWARD COUNTY STANDARD DETAIL SD-4.14		552,619.7583	1,372,481.9126
1-37	285.50		·	281.58	YARD INLET, HOWARD COUNTY STANDARD DETAIL SD-4.14		552,704.33	1,372,458.82
1-38	283.85			281.14	TYPE 'S' INLET, HOWARD COUNTY STANDARD DETAIL SD-4.22	·	552,893.6708	1,372,373.9199
EW-40	282.00	280.00			TYPE 'C' ENDWALL, HOWARD COUNTY STANDARD DETAIL SD-5.21		552,940.7608	1,372,405.6561
ES-46		280.00			CONCRETE END SECTION, STANDARD DETAIL 5.51		552,940.7608	1,372,405.6561
1-48	284.50	281.30		281.10	YARD INLET, HOWARD COUNTY STANDARD DETAIL SD-4.14		552,940.7608	1,372,405.6561

(1) THROAT OPENING AT ELEVATION 279.50 (INSTALL IN SUMP)

CONTROLLED AND COMPACTED FILL PER AASHTO T-180, TO BE CERTIFIED BY AN APPROVED ON-SITE GEOTECHINICAL ENGINEER

COMPACTED SPECIFICATIONS FOR UTILITIES IN FILL

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

6/5/07 DATE

- WHERE UTILITY PIPES ARE TO BE PLACED ON COMPACTED FILL, THE FOLLOWING APPLIES:
- A. PRIOR TO PLACEMENT OF COMPACTED FILL, ANY SOFT OR OTHERWISE UNSUITABLE SOILS ENCOUNTERED AT THE EXISTING REMOVED FROM THE CONSTRUCTION AREA.
- B. ACCEPTABLE COMPACTED FILL SHALL BE PLACED IN SIX INCH THICK LOOSE LIFTS AND COMPACTED TO AT LEAST 98 PERCENT OF THE MAXIMUM DENSITY AS DETERMINED BY A.A.S.H.T.O. METHOD T-180.
- C. THE COMPACTED FILL SHALL BE BENCHED INTO THE EXISTING VIRGIN SLOPES WITH EACH LIFT PLACED TO A SMOOTH TRANSITION FROM VIRGIN TO FILL SOILS.

PRIVATE	STORM DRAIN PIPE	SCHEDULE
SIZE	TYPE	LENGTH
12"	RCP, CL. IV	146 FT
15"	RCP, CL. IV	580 FT
24"	PCP CL IV	134 FT

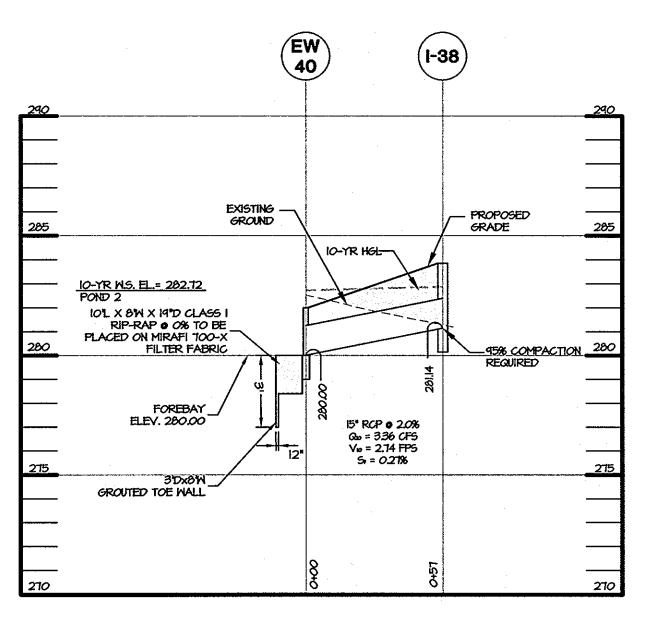
10" PVC, SCHED. 40 1,254 FT (ROOF DRAINS***)

*SEE SHEETS 18-21 FOR SWM POND OUTFALL PROFILES AND STRUCTURE SCHEDULE. **SEE F-06-203 FOR PUBLIC STORM DRAIN DESIGN. *** ROOF DRAINS

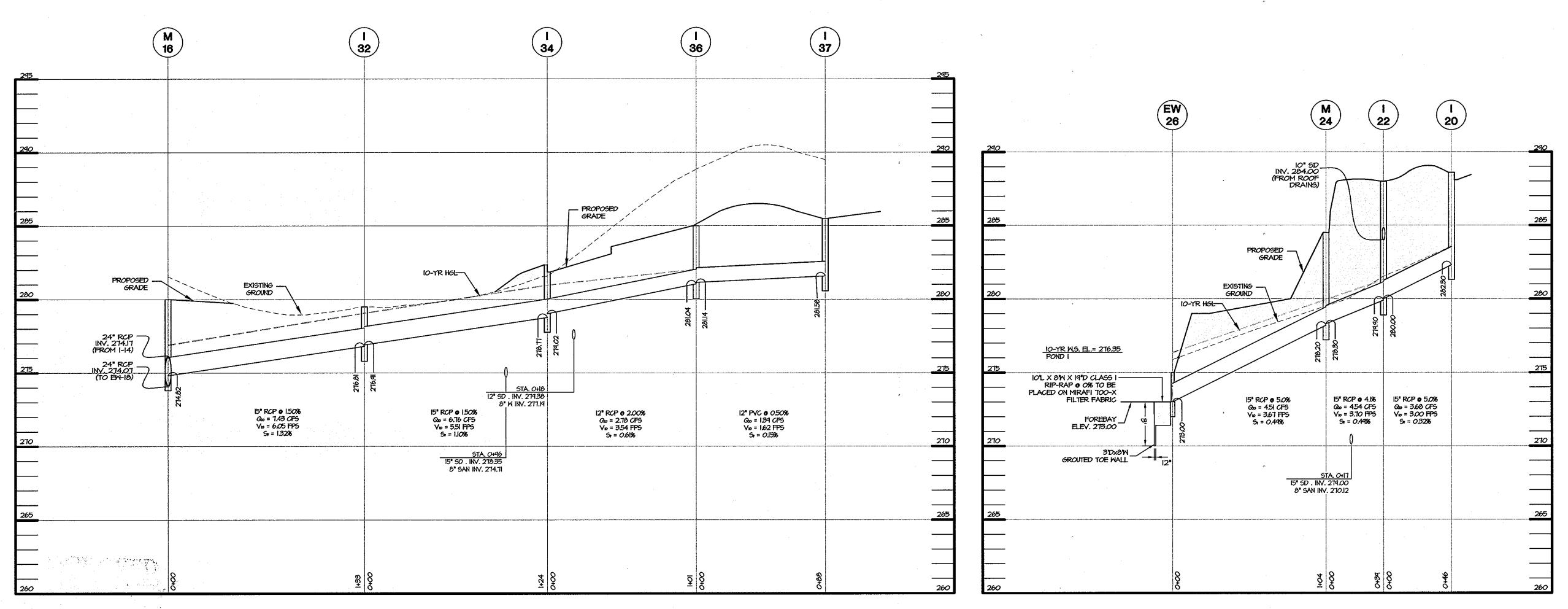
- CONTRACTOR TO INSTALL 10"x4" TEES AND 4" PVC LATERALS TO
- DOWNSPOUTS AT UNITS 1-9, 18-37, & 58-66. - CLEANOUTS PER STANDARD DETAIL S-2.22 MODIFIED FOR 10" PVC
- TRAFFIC BEARING CLEANOUT COVER FOR CLEANOUTS IN PAVED AREAS.

18 18			A 12	•
<u>40</u>				
			PROPOSED GRADE	285
EXISTING —				IO* SD INV. 282.00 (FROM ROOF DRAINS)
GROUND 6ROUND	\	IO-YR HGL		280
	5" RCP NV. 274.92 	and and the second	28060	
O-YR MS. EL.= 276.35 POND T5	(FROM 1-32)	-25#72	7 K	
PLACED ON MIRAFI TOO-X FILTER FABRIC FOREBAY	273.co 274.07	15" SD . II	5TA. 0+22 N. 218.76 NV. 2171.19	
ELEV. 273.00 3'Dx8'W 12" GROUTED TOE WALL	STA_0+37 24" SD . INV. 273 8" SAN INV. 271.3	156 4		270
	24" RCP e 1.5% Co = 12.51 CFS Vo = 3.40 FPS So = 0.31%	24" RCP @ 5.0% 366 = 5.37 CF5 V16 = 1.71 FP5 51 = 0.06%	15" RCP © 3.24% G ₆₀ = 1.44 CFS V ₁₀ = 1.10 FPS S ₁ = 0.05%	·
<u>65</u>	5 ₁ = 0.386			
	\$\$ F+0	0,000	00+0	260

STORM DRAIN PROFILE (PRIVATE) EW-18 TO M-12



STORM DRAIN PROFILE (PRIVATE) EW-40 TO I-38 HOR. 1" = 40' VERT. 1" = 4'

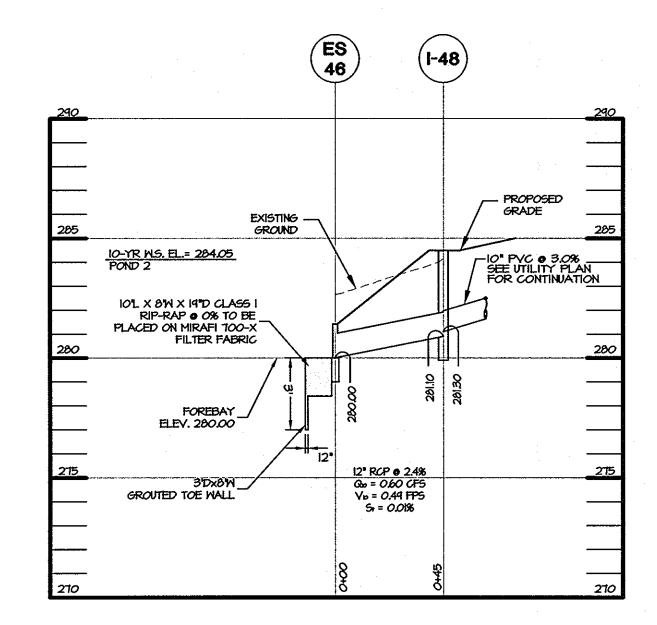


STORM DRAIN PROFILE (PRIVATE) M-16 TO I-37

HOR. 1" = 40' VERT. 1" = 4'

APPROVED
PLANNING BOARD
of HOWARD COUNTY

STORM DRAIN PROFILE (PRIVATE) EW-26 TO I-20



STORM DRAIN PROFILE (PRIVATE)

ES-46 TO I-48

HOR. 1'' = 40'VERT. 1'' = 4'



MORRIS & RITCHIE ASSOCIATES, INC.

ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

14280 PARK CENTER DRIVE, SUITE A LAUREL, MARYLAND 20707

(410) 792-9792 or (301) 776-1690 FAX (410) 792-7395

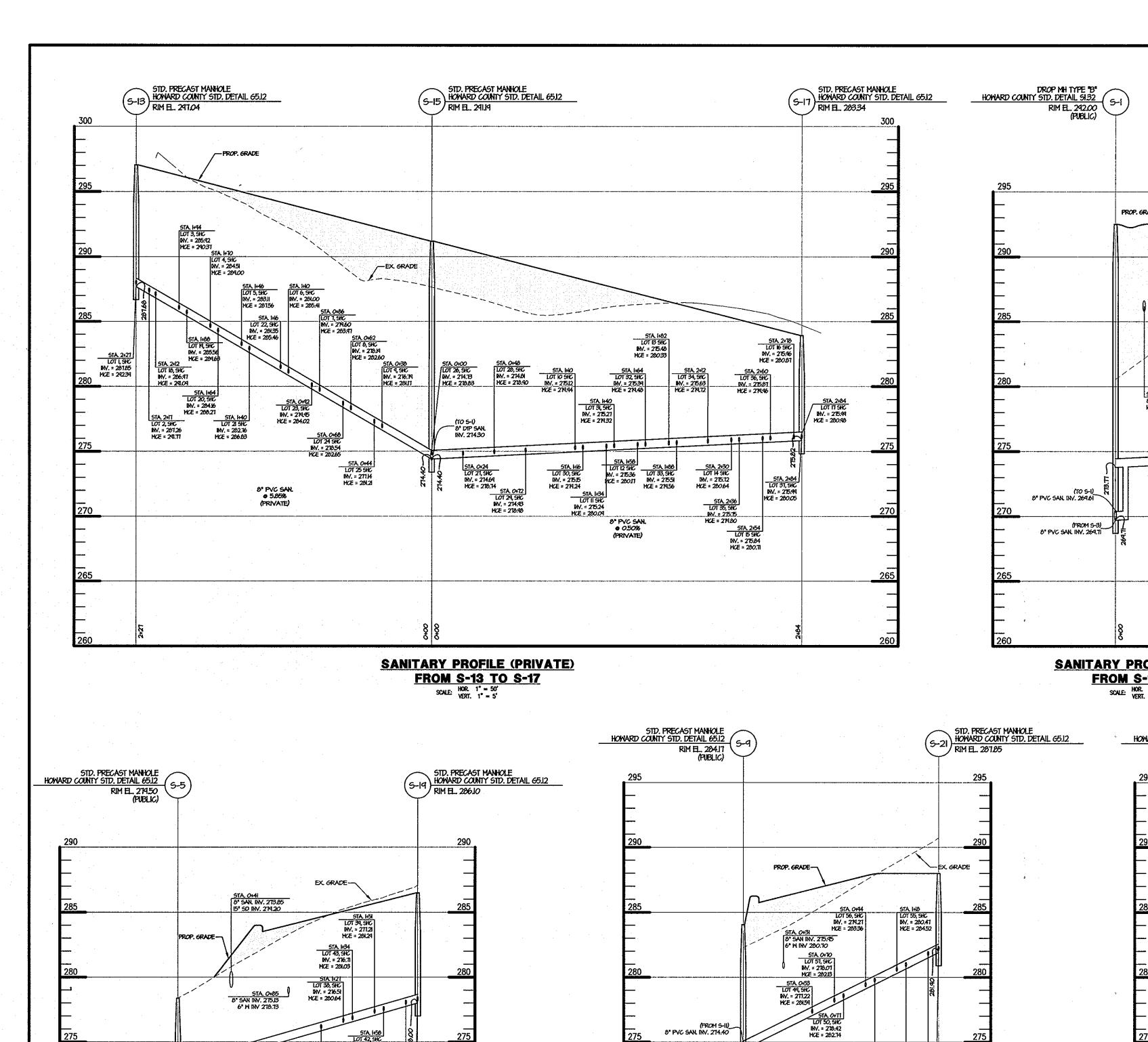


SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

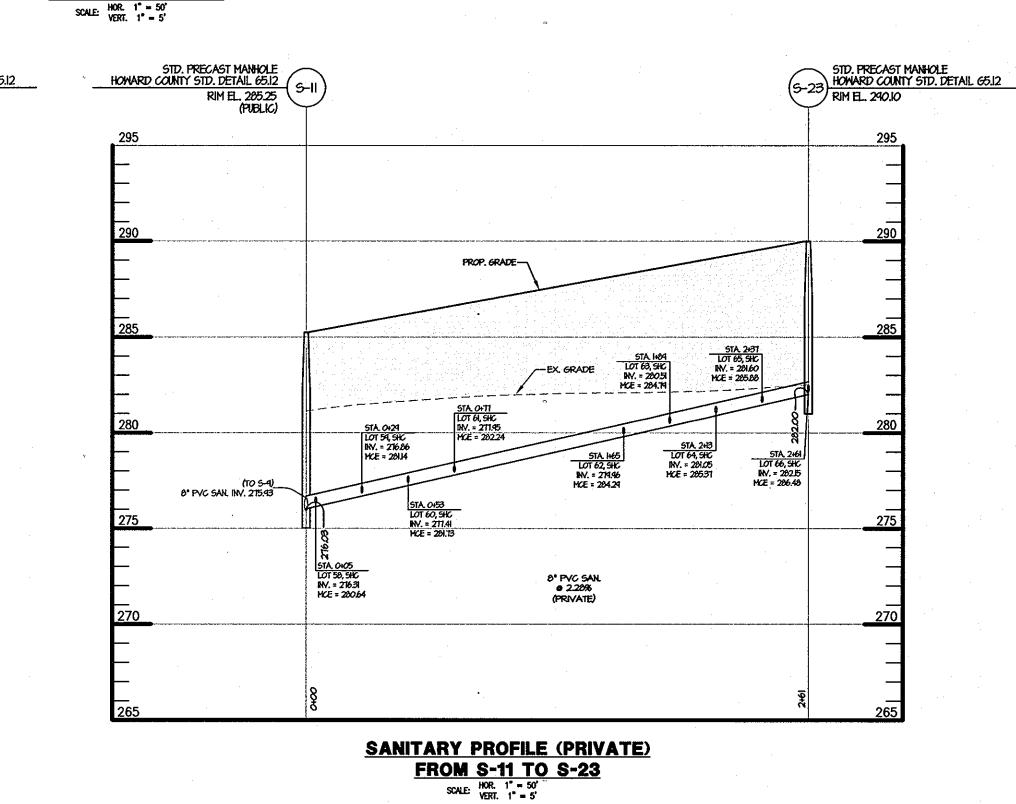
STORM DRAIN PROFILES

LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382 ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3 LOTS: J, K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

DATE REVISIONS JOB NO.: SCALE: AS SHOWN OWNER/DEVELOPER/APPLICANT DATE: 4/24/07 NAME: MR. BRIAN KNAUFF DRAWN BY: COMPANY: THE RYLAND GROUP 6011 UNIVERSITY BOULEVARD, SUITE 260 DESIGN BY: ELLICOTT CITY, MARYLAND 21043 REVIEW BY: PHONE: 410-480-2467 FAX: 410-480-0543 SHEET: 13 OF 33



INV. = 211.40 MCE = 282.02



STID. PRECAST MANHOLE HOWARD COUNTY STID. DETAIL 65.12 RIM BL. 291.9

5TA. 0492 8" SAN INV. 274.23

EX. GRADE

(FROM 5-13) 8" PVC SAN, INV. 274.40 (FROM 5-17) 8" PVC SAN, INV. 274,40

8" DIP SAN. • 0.50% (PRIVATE)

SANITARY PROFILE (PRIVATE)

FROM S-1 TO S-15

5TA. 0+22 8" SAN. INV. 273.88 10" SD 286.20

8" PVC SAN, INV. 264.6

(FROM 5-3)_ 8" PVC SAN, INV. 264,7]

SANITARY PROFILE (PRIVATE) FROM S-9 TO S-21

106 FT

1,071 FT

SANITARY SEWER PIPE SCHEDULE (PRIVATE)* 1,106 FT PVC (SDR-35)

DIP (CLASS 50)

PVC (SDR-35)

8° PVC SAN, INV. 274.30

V DIDID VALLES
APPROVED
PLANNING BOARD
of HOWARD COUNTY
Clala

(FROM 5-7)_ 8" PVC SAN INV. 272.66

8" PVC SAN INV. 272.56

	**
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANN	IING AND ZONING
CHIEF, DEVELOPMENT ENGINEERING DIVISION	10010
CHIEF, DEVELOPMENT ENGINEERING DIVISION	DATE
CHIEF, DIVISION OF LAND DEVELOPMENT	6/23/0
CHIEF, DIVISION OF LAND DEVELOPMENT	DATE
prent in leyle	6/25/03
DIRECTOR, DEPARTMENT OF PLANNING AND ZONING	DATE

SANITARY PROFILE (PRIVATE)

FROM S-5 TO S-19

		SANIT	ARY S	SEWER	STRUCTURE SCHED	ULE (PRIVATE)*	● LOC/	ATION
STR NO.	TOP ELEV	INV IN	INV IN	INV OUT	TYPE	REMARKS	NORTHING	EASTING
SMH-13	297.04			287.68	H.C. STD. DETAIL G5.12	STANDARD PRECAST MH	551,733.793	1,372,591.682
SMH-15	291.19	274.40	274.40	274.30	H.C. STD. DETAIL G5.12	STANDARD PRECAST MH	551,960.381	1,372,579.059
SMH-17	283.34			275.82	H.C. STD. DETAIL G5.12	STANDARD PRECAST MH	552,243.942	1,372,563.264
SMH-19	286.10			278.00	H.C. STD. DETAIL G5.12	STANDARD PRECAST MH	552,543.932	1,372,585.836
SMH-21	287.85	· · 		281.90	H.C. STD. DETAIL G5.12	STANDARD PRECAST MH	552,830.936	1,372,507.547
SMH-23	290.10			282.00	H.C. STD. DETAIL G5.12	STANDARD PRECAST MH	553 313 128	1 372 174 068

* FOR PUBLIC STRUCTURE & PIPE SCHEDULE, SEE WATER AND SEWER PLANS CONTRACT NUMBER: 24-4376-D COORDINATES TO CENTERLINE OF STRUCTURE

GENERAL NOTES

- 1. CONTRACTOR TO FIELD VERIFY ALL EXISTING UTILITIES PRIOR TO CONSTRUCTION.
- CONTRACTOR TO TEST PIT TO DETERMINE EXACT DEPTH, LOCATION AND SIZE FOR ALL PROPOSED UTILITY CROSSINGS WITH EXISTING UTILITIES.
- 3. ALL EXISTING VALVES TO BE RAISED TO PROPOSED GRADE, BY CONTRACTOR, AS NEEDED.
- 4. THIS PLAN ASSUMES DEMOLITION/RELOCATION OF EXISTING UTILITIES. PAVING, STREET SIGNS, CULVERTS, ETC. WITHIN THE AREA OF R/W IMPROVEMENTS HAS ALREADY BEEN
- 5. SEE PUBLIC ROAD & STORM DRAIN PLANS FOR R/W IMPROVEMENTS.
- 6. SEE CONTRACT NO. 24-4376-D FOR CONTINUATION OF PRIVATE WATER & SEWER.
- 7. ALL SEWER HOUSE CONNECTIONS SHALL BE IN ACCORDANCE WITH HOWARD COUNTY STANDARD DETAILS \$2.11 & \$2.22.
- 8. TRAFFIC BEARING CLEANOUT COVERS PER HOWARD COUNTY STANDARD DETAIL \$3.21 ARE TO BE USED FOR CLEANOUTS LOCATED IN PAVED AREAS.

UNIT NO.	STATION OF MAIN	4" INVERT • MAIN	LENGTH TO CLEANOUT (B)	SLOPE OF SERVICE	CLEANOUT INVERT	TOP C/O ELEV.	MIN. CELLAR ELEVATION (MCE)
			S-	13 TO S-15			
1	2+27	287.85	29'	2.00%	288.43	298.00	292.39
2	2+17	287.26	22'	2.00%	287.70	297.26	291.77
3	1+94	285.92	22'	2.00%	286.36	296.56	290.37
4	1+70	284.51	22'	2.00%	284.95	295.84	289.00
. 5	1+46	283.11	22'	2.00%	283.55	295.14	287.56
6	1+10	281.00	22'	2.00%	281.44	294.02	285.41
7	0+86	279.60	22'	2.00%	280.04	294.34	283.97
8	0+62	278.19	22'	2.00%	278.63	292.58	282.60
9	0+38	276.79	22'	2.00%	277.23	291.88	281.17
18	2+12	286.97	8'	2.00%	287.13	297.15	291.09
19	1+88	285.56	8'	2.00%	285.72	296.69	289.63
20	1+64	284.16	8'	2.00%	284.32	295.90	288.27
21	1+40	282.76	8'	2.00%	282.92	295.27	286.83
22	1+16	281.35	8'	2.00%	281.51	294.66	285.46
23	0+92	279.95	8'	2.00%	280.11	293.86	284.02
24	0+68	278.54	8'	2.00%	278.70	293.22	282.65
25	0+44	277.14	8'	2.00%	277.30	292.60	281.21
			· · · · · · · · · · · · · · · · · · ·	15 TO S-17	·		-
10	1+10	275.12	22'	2.00%	275.56	287.95	279.94
11	1+34	275.24	22'	2.00%	275.68	287.24	280.09
12	1+58	275.36	22'	2.00%	275.80	286.60	280.17
13	1+82	275.48	22*	2.00%	275.92	285.95	280.33
14	2+30	275.72	22'	2.00%	276.16	284.80	280.64
15	2+54	275.84	22*	2.00%	276.28	284.00	280.71
16	2+78	275.96	22'	2.00%	276.40	283.48	280.87
17	2+84	275.99	24'	2.00%	276.47	282.80	280.98
26	0+00	274.73	8'	2.00%	274.89	291.34	278.83
27	0+24	274.69	8'	2.00%	274.85	290.68	278.74
28	0+48	274.81	8'	2.00%	274.97	289.99	278.90
29	0+72	274.93	8'	2.00%	275.09	289.51	278.98
30	1+16	275.15	8'	2.00%	275.31	288.15	279.24
31	1+40	275.27	8'	2.00%	275.43	287.58	279.32
32	1+64	275.39	8'	2.00%	275.55	286.94	279.48
33	1+88	275.51	8'	2.00%	275.67	286.15	279.56
34	2+12	275.63	8'	2.00%	275.79	285.49	279.72
35	2+36	275.75	8'	2.00%	275.91	284.85	279.80
36	2+60	275.87	8,	2.00%	276.03	284.18	279.96
37	2+84	275.99	8'	2.00%	276.15	283.54	280.05
			. S-	-5 TO S-19			
44	1+10	276.02	21'	2.00%	276.44	284.75	280.38
38	1+27	276.51	9,	2.00%	276.69	284.82	280.64
43	1+34	276.71	21'	2.00%	277.13	285.34	281.03
39	1+51	277.21	9'	2.00%	277.39	285.46	281.29
42	1+58	277.41	21'	2.00%	277.83	285.90	281.77
40	1+75	277.90	9,	2.00%	278.08	286.00	282.02
41	1+84	278.16	21'	2.00%	278.58	286.73	282.48
			, 	-9 TO S-21			
49	0+53	277.22	22'	2.00%	277.66	285.91	281.59
57	0+70	278.07	8'	2.00%	278.23	286.81	282.13
50	0+77	278.42	22'	2.00%	278.86	286.57	282.74
56	0+94	279.27	8'	2.00%	279.43	286.60	283.36
51	1+01	279.62	22'	2.00%	280.06	287.23	283.98
55	1+18	280.47	8'	2.00%	280.63	287.28	284.52
52	1+25	280.82	22'	2.00%	281.26	287.90	285.14
54	1+42	281.67	8'	2.00%	281.83	287.85	285.76
53	1+50	282.07	22'	2.00%	282.51	288.55	286.45
			S-	11 TO S-23	·		
58	0+05	276.31	20'	2.00%	276.71	285.88	280.64
59	0+29	276.86	20'	2.00%	277.26	286.00	281.14
60	0+53	277.41	20'	2.00%	277.81	286.38	281.73
61	0+77	277.95	20'	2.00%	278.35	286.94	282.24
62	1+65	279.96	20'	2.00%	280.36	288.64	284.29
63	1+89	280.51	20'	2.00%	280.91	289.30	284.79
64	2+13	281.05	20'	2.00%	281.45	289.80	285.37
65	2+37	281.60	20'	2.00%	282.00	290.30	285.88
66	2461	282 15	20'	2 00%	282 55	290.70	286.48

SANITARY SEWER CLEAN-OUT CHART

66 2+61 282.15 20' 2.00% 282.55 290.70 286.48 * SHOWN ON PROFILE VIEW

NOTE: FOR UNITS 45-48, SEE CONTRACT NO. 24-4376-D FOR SANITARY SEWER HOUSE CONNECTION DATA.

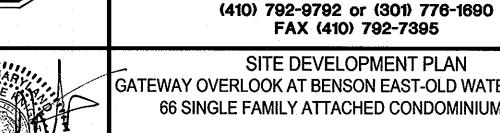
CONTROLLED AND 95% COMPACTED FILL PER AASHTO T-180, TO BE CERTIFIED BY AN APPROVED ON-SITE GEOTECHINICAL ENGINEER

COMPACTED SPECIFICATIONS FOR UTILITIES IN FILL

WHERE UTILITY PIPES ARE TO BE PLACED ON COMPACTED FILL, THE FOLLOWING APPLIES:

- A. PRIOR TO PLACEMENT OF COMPACTED FILL, ANY SOFT OR OTHERWISE UNSUITABLE SOILS ENCOUNTERED AT THE EXISTING RAVINE BOTTOM OR SLOPE, SHALL BE UNDERCUT AND REMOVED FROM THE CONSTRUCTION AREA.
- ACCEPTABLE COMPACTED FILL SHALL BE PLACED IN SIX INCH THICK LOOSE LIFTS AND COMPACTED TO AT LEAST 95 PERCENT OF THE MAXIMUM DENSITY AS DETERMINED BY A.A.S.H.T.O. METHOD T-180.
- C. THE COMPACTED FILL SHALL BE BENCHED INTO THE EXISTING VIRGIN SLOPES WITH EACH LIFT PLACED TO A SMOOTH TRANSITION FROM VIRGIN TO FILL SOILS.





SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

LAUREL, MARYLAND 20707

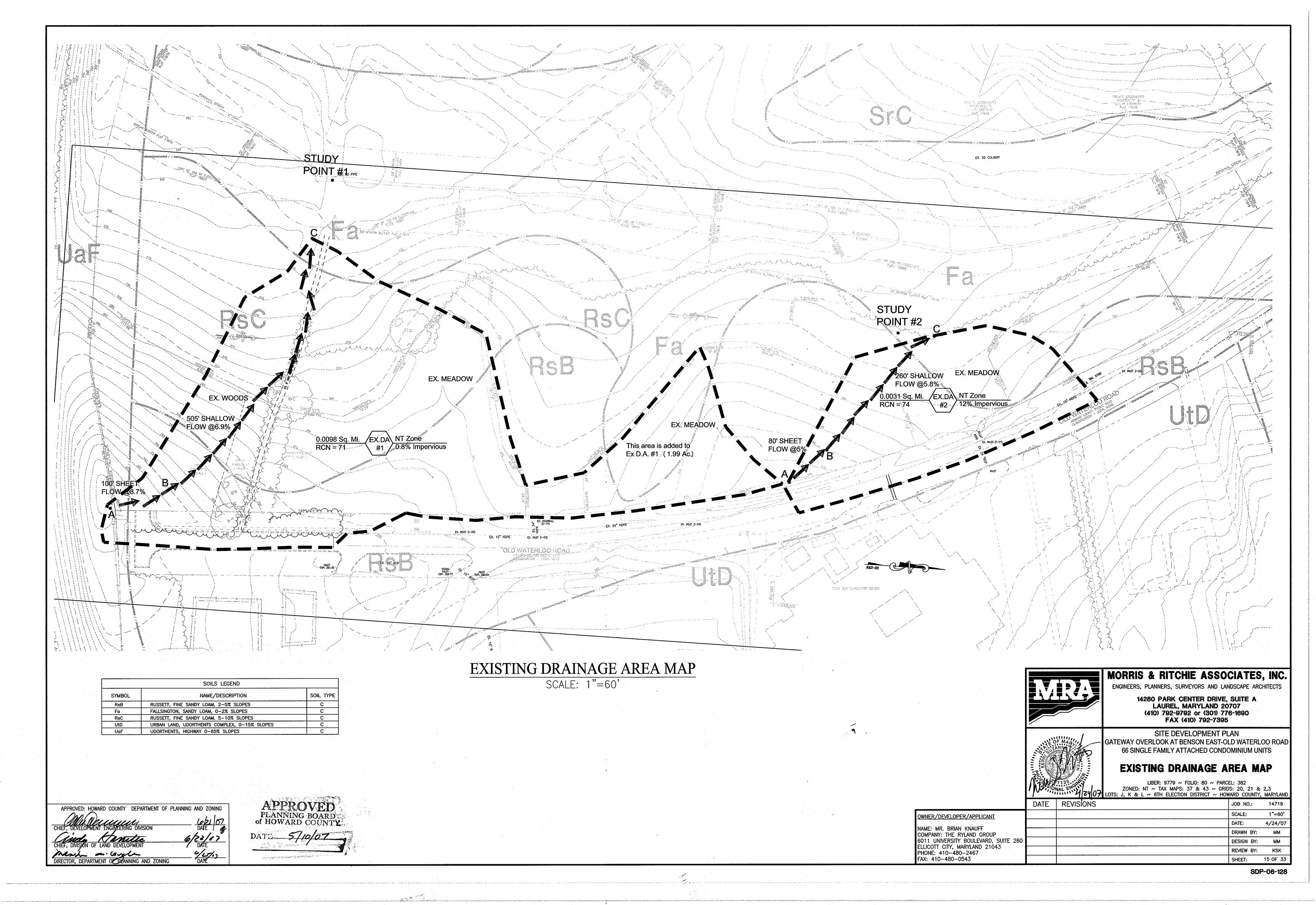
FAX (410) 792-7395

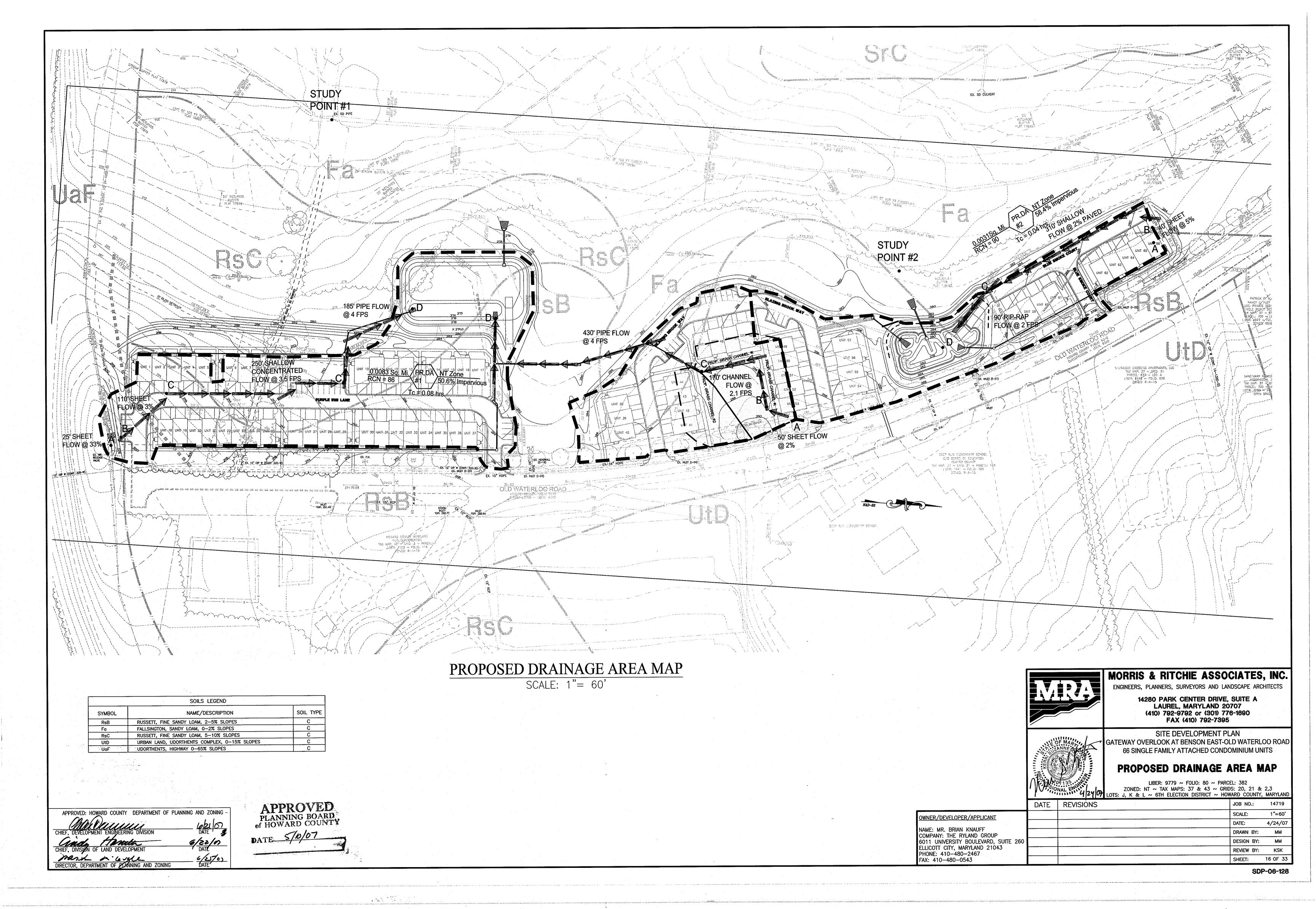
SEWER PROFILES

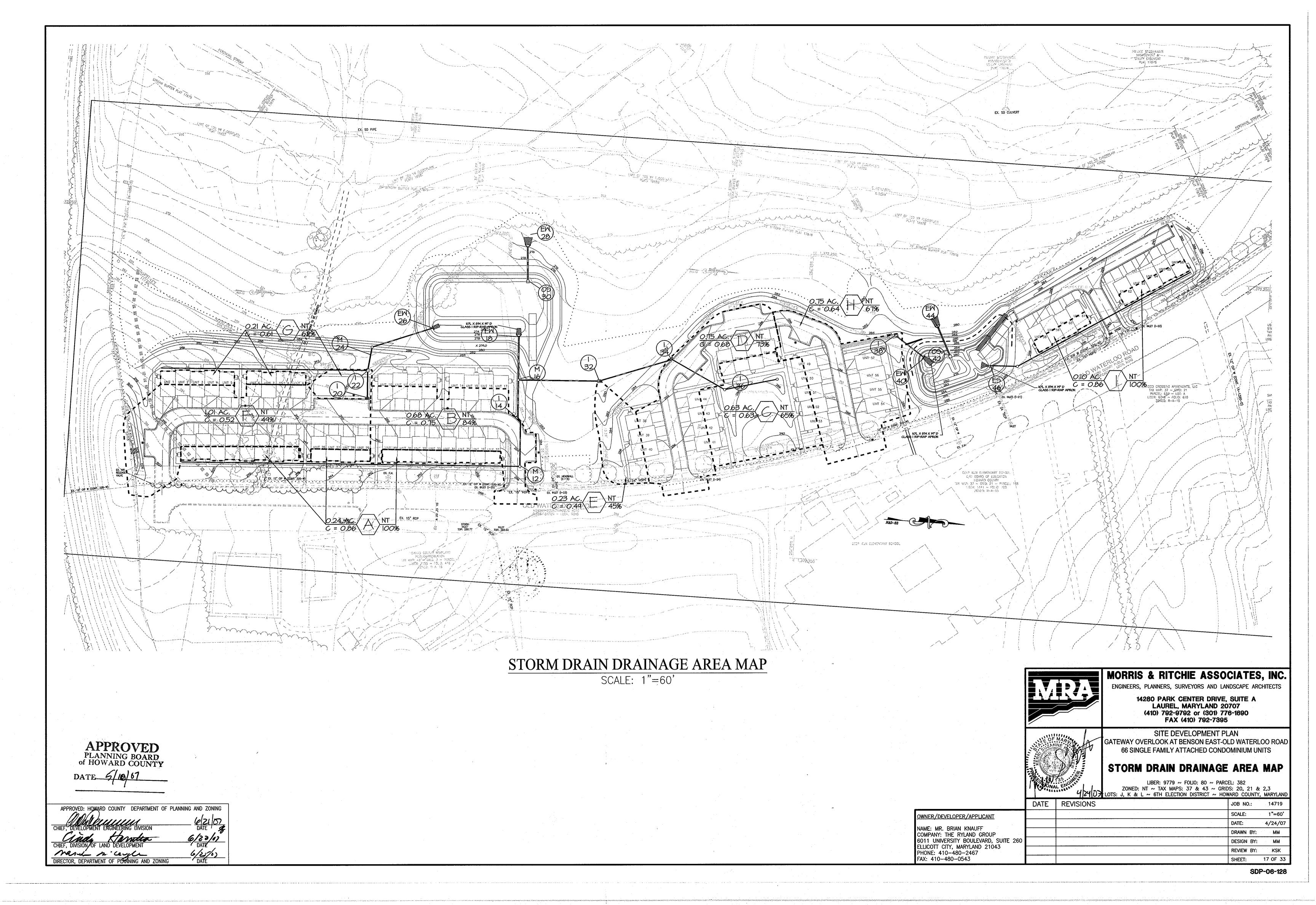
LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382 ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3 LOTS: J, K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

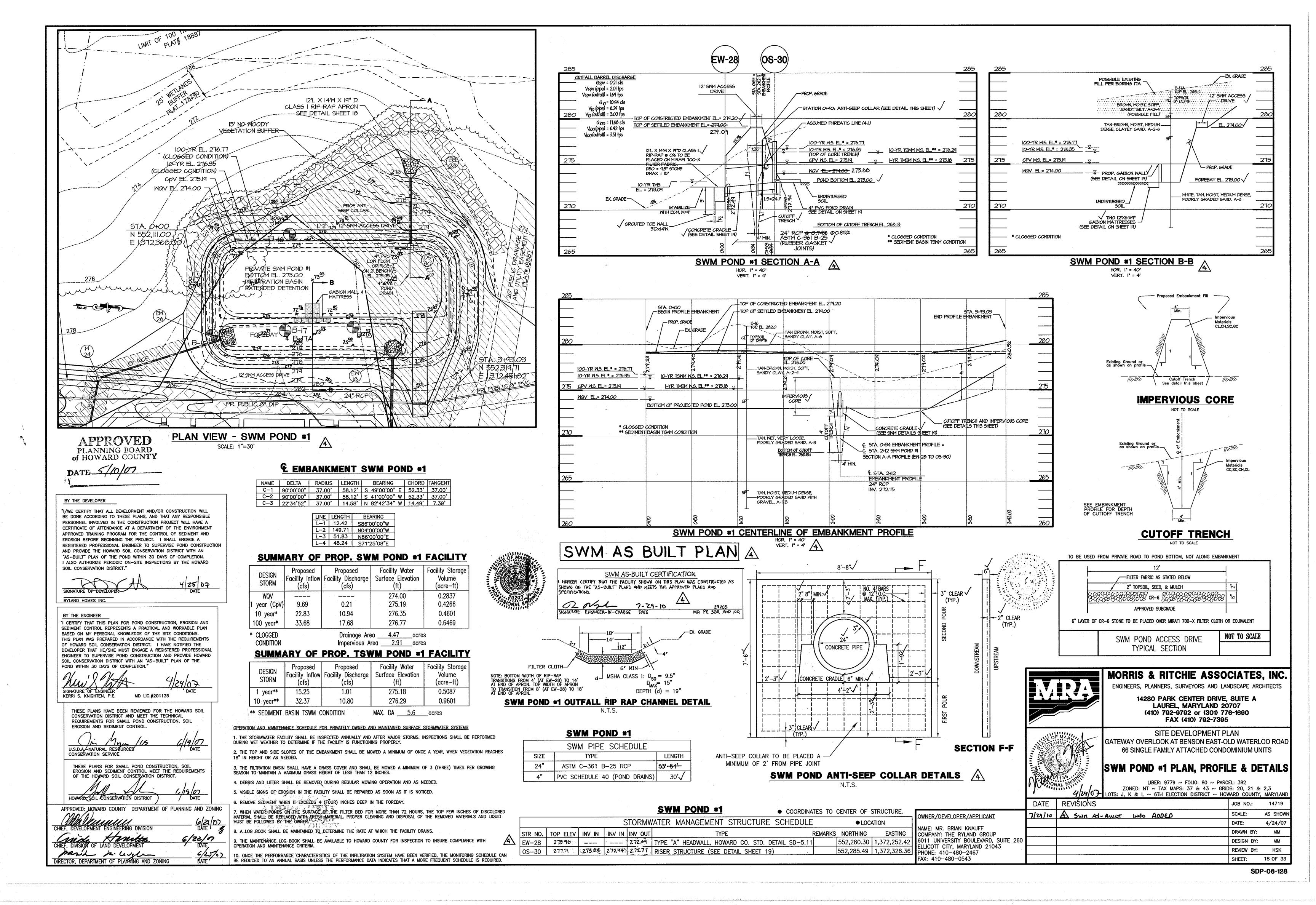
	DATE	REVISIONS	JOB NO.:	14719
APPLICANT			SCALE:	AS SHOWN
			DATE:	4/24/07
NAUFF AND GROUP			DRAWN BY:	MSP
DULEVARD, SUITE 260			DESIGN BY:	KSK
/LAND 21043 2467			REVIEW BY:	KSK
3	'		CUEET.	14 OF 33

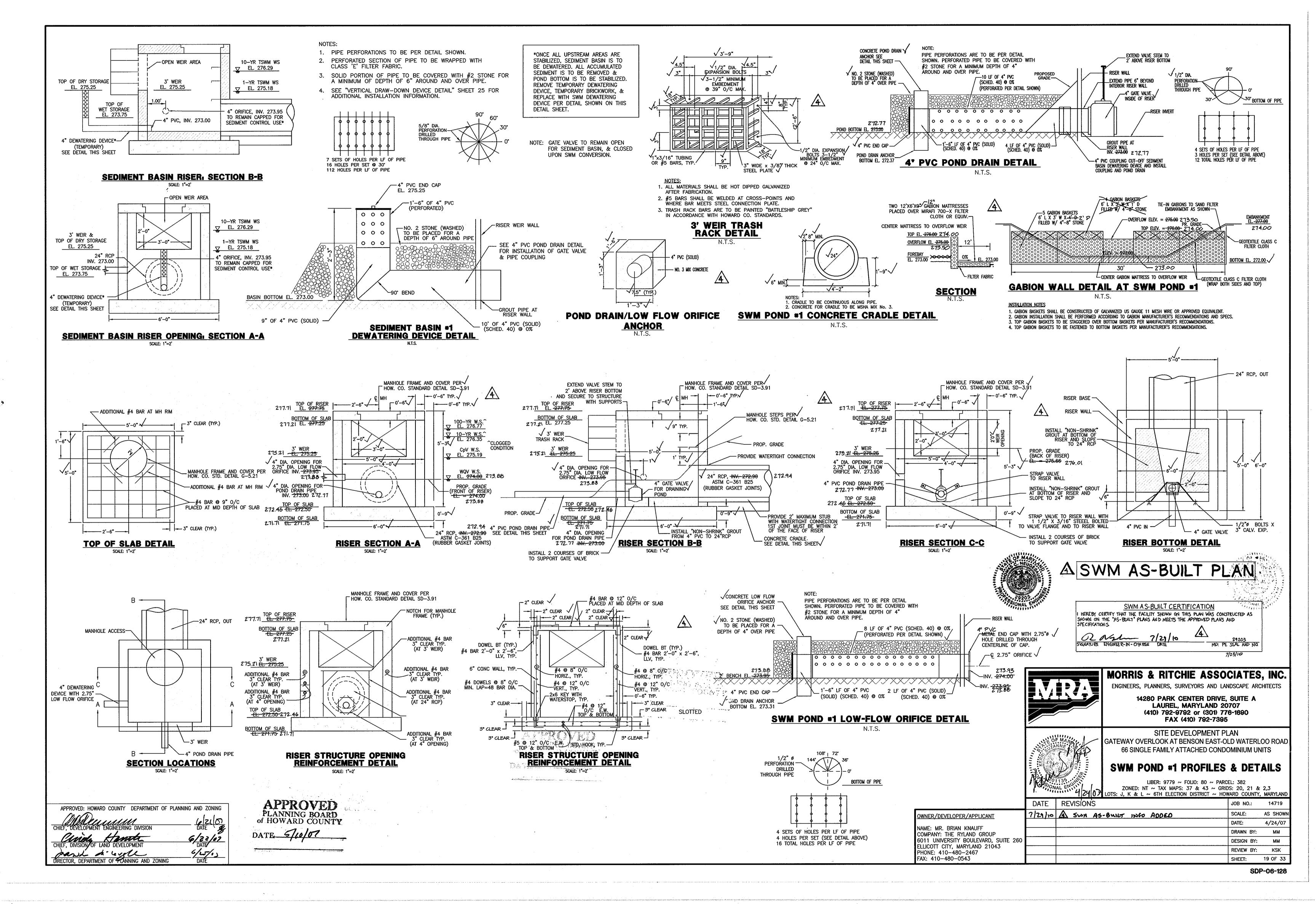
OWNER/DEVELOPER/ NAME: MR. BRIAN KNA COMPANY: THE RYLAN 6011 UNIVERSITY BOU ELLICOTT CITY, MARYL PHONE: 410-480-24 FAX: 410-480-0543

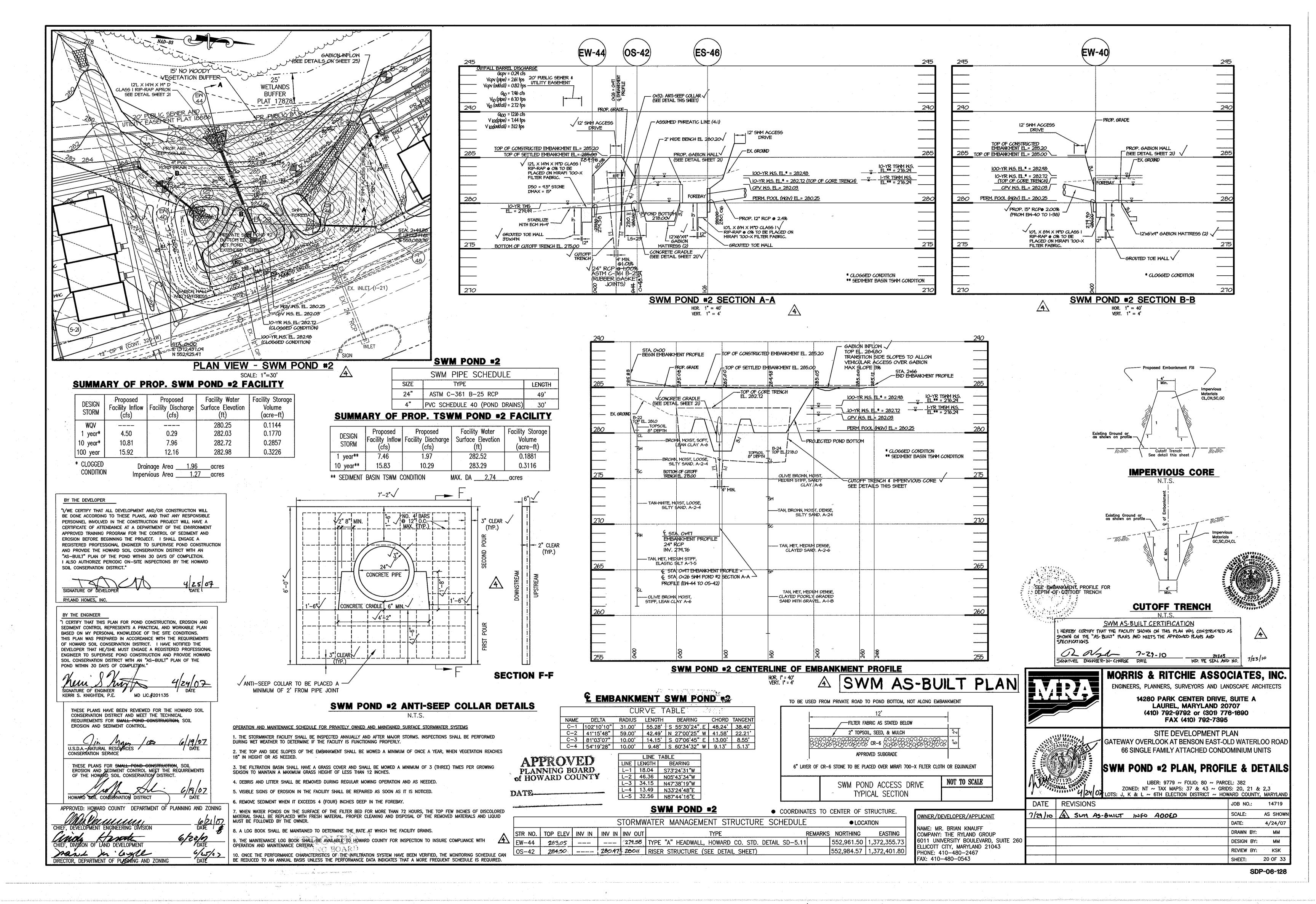


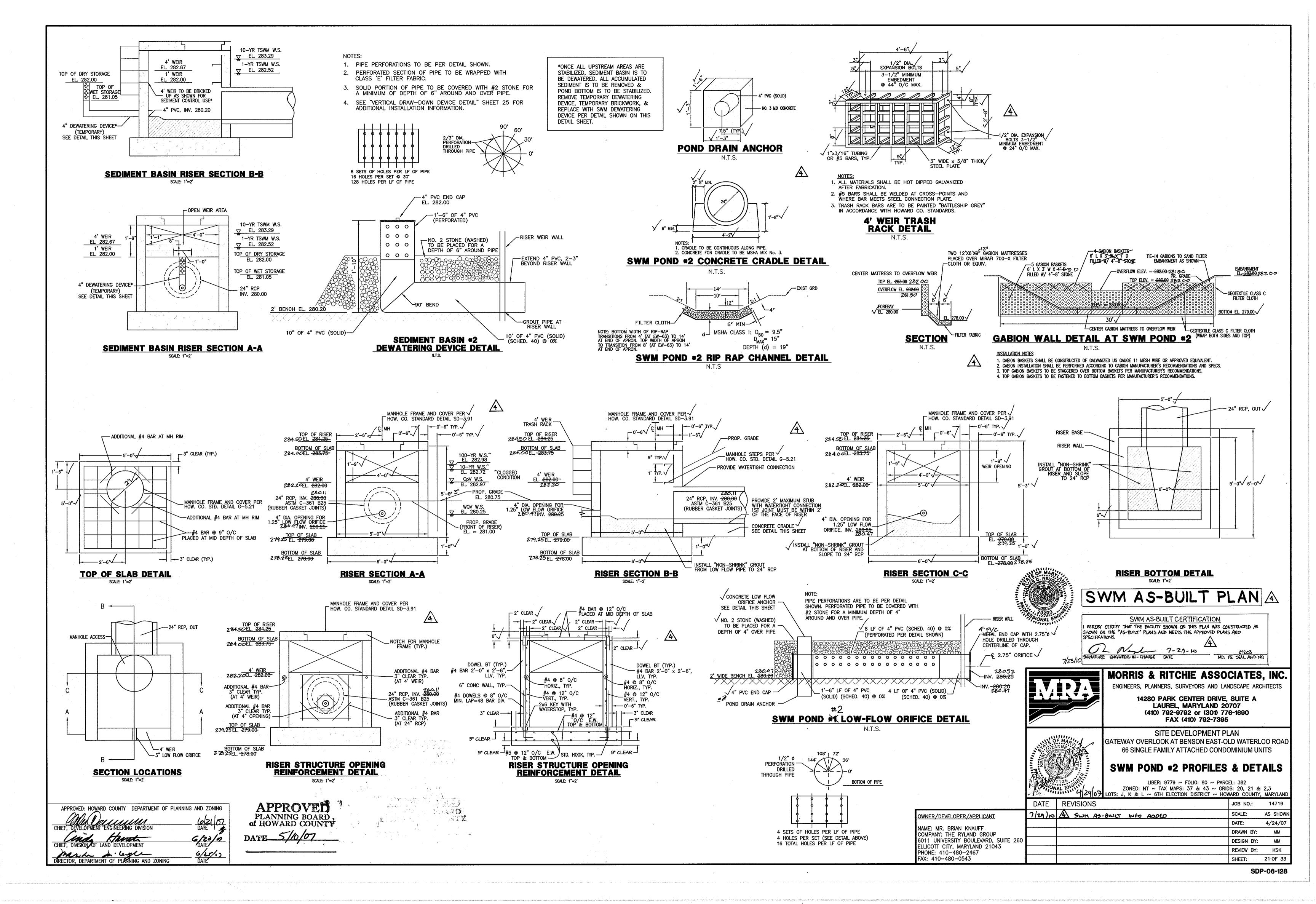


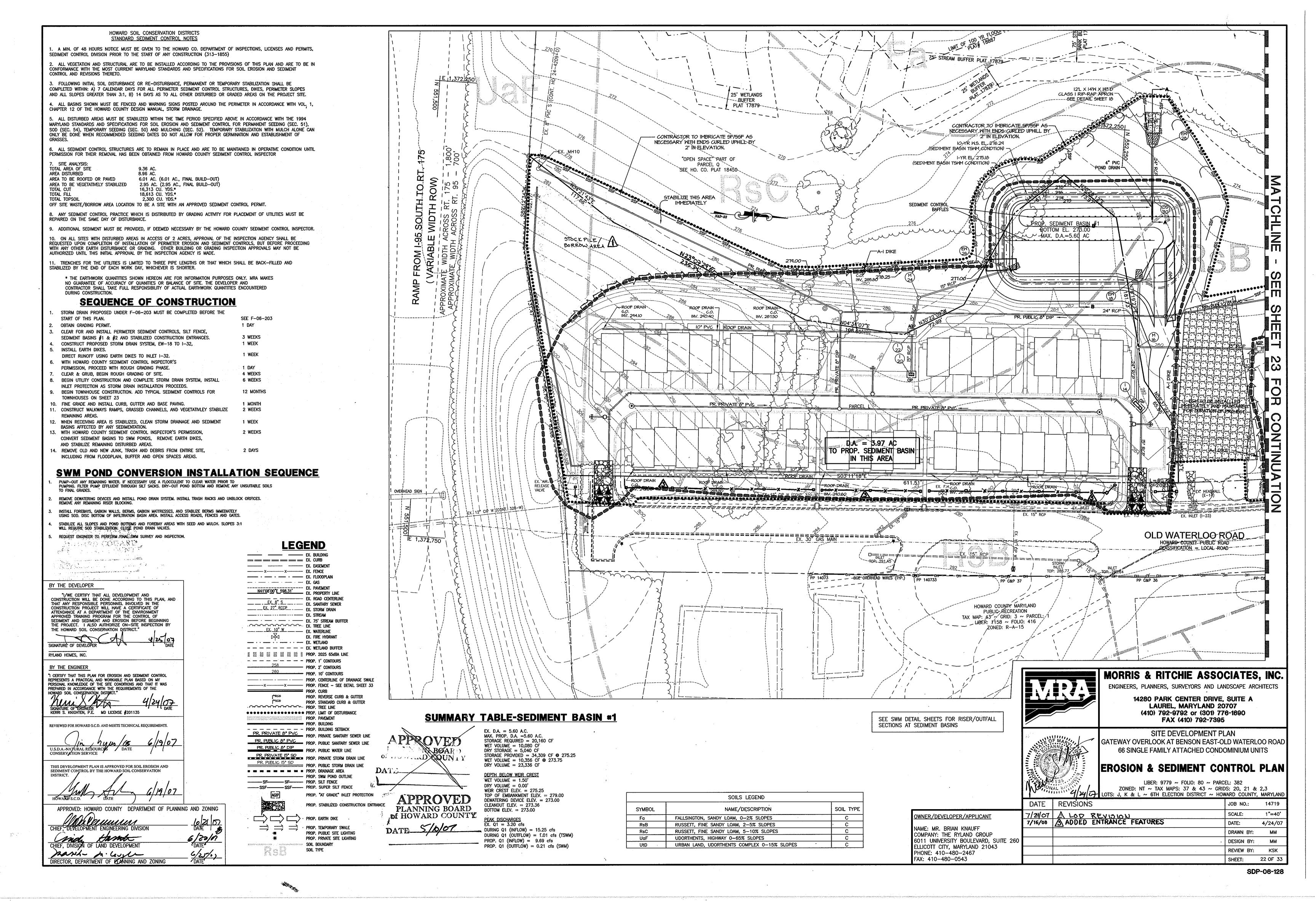


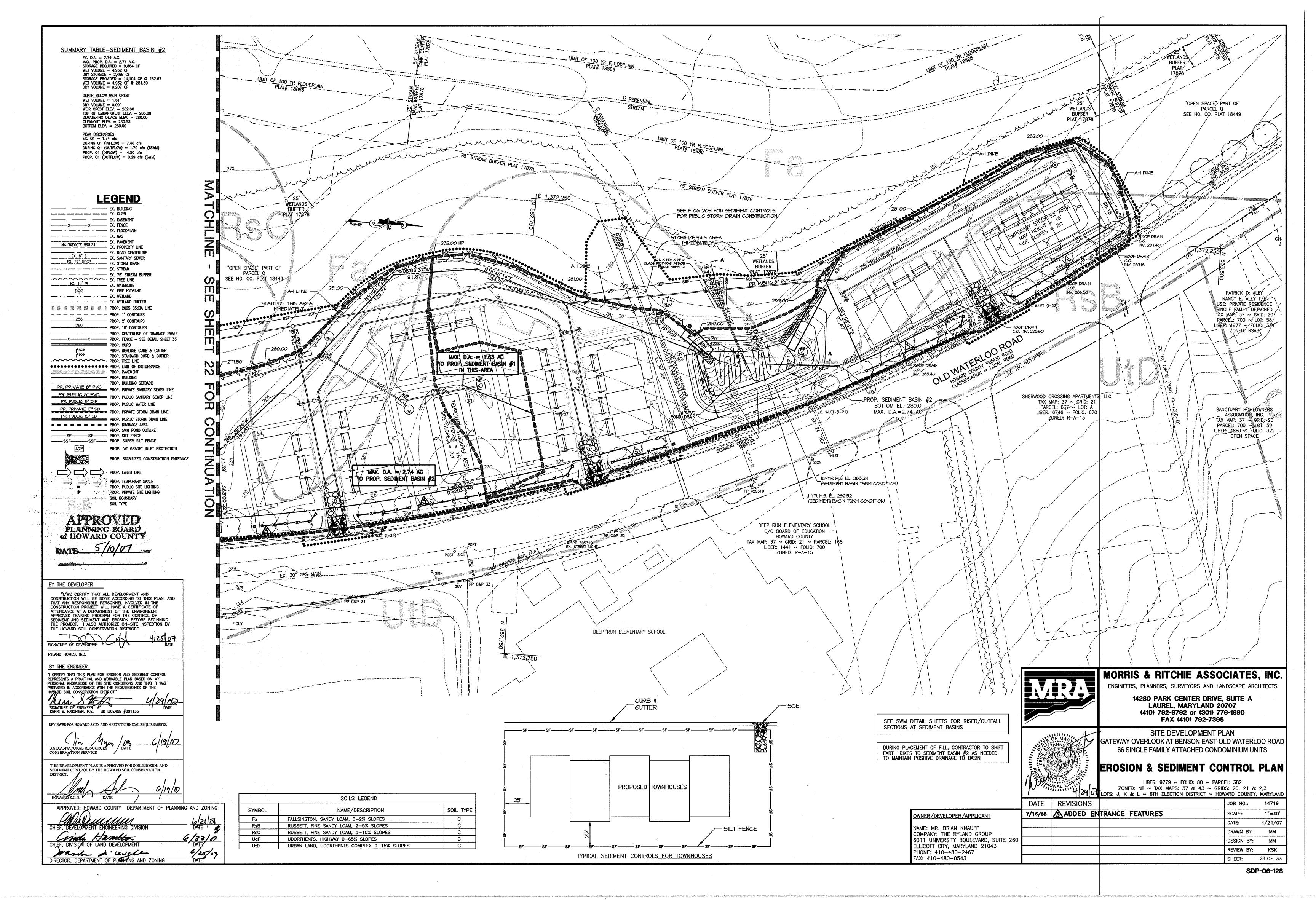


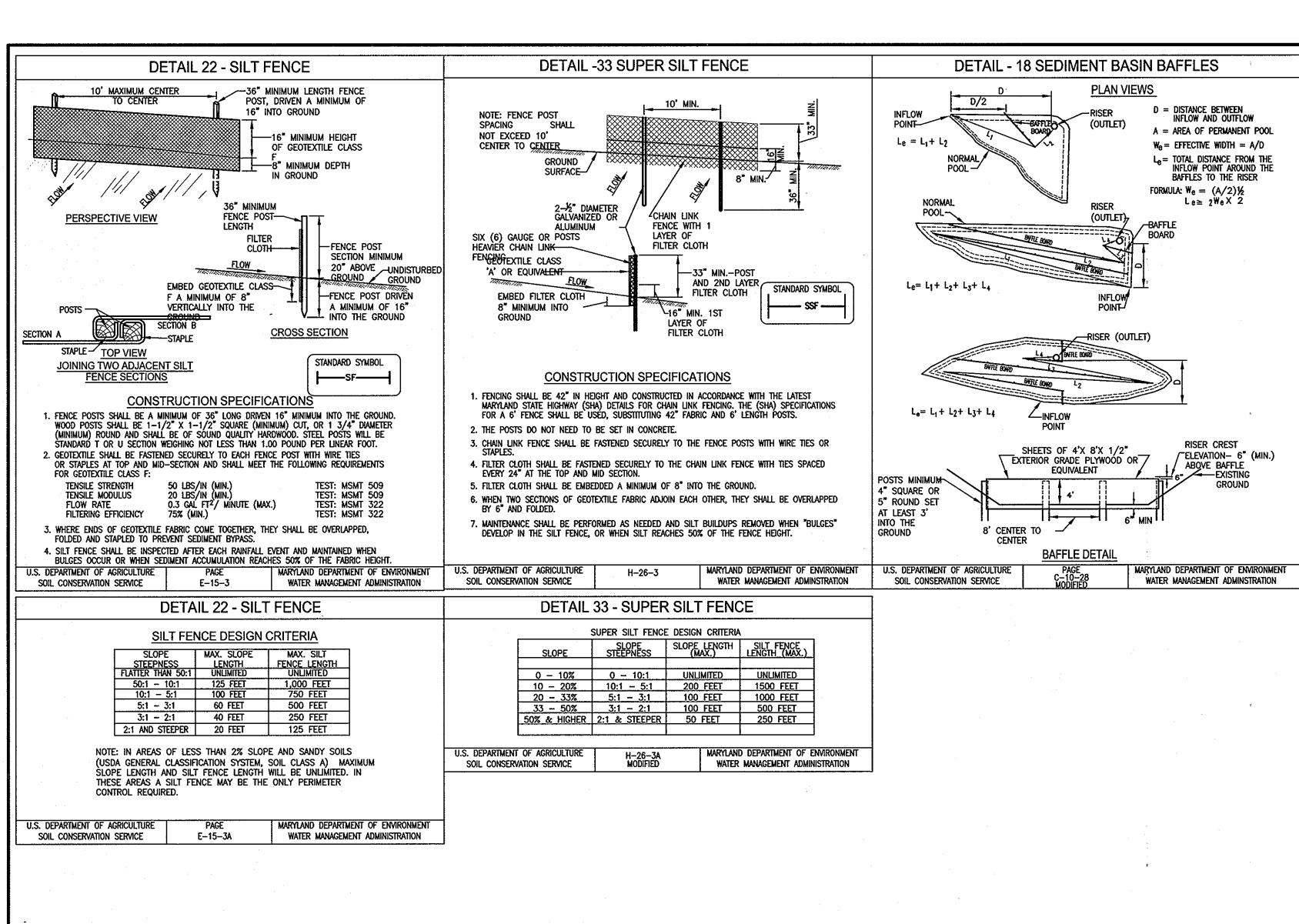


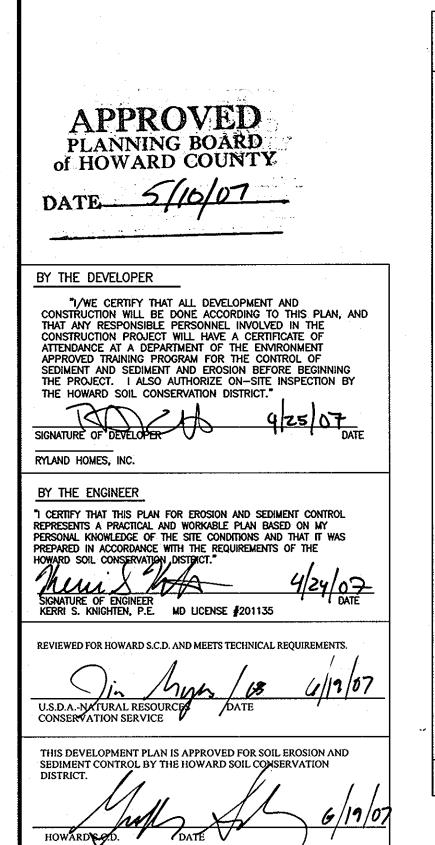






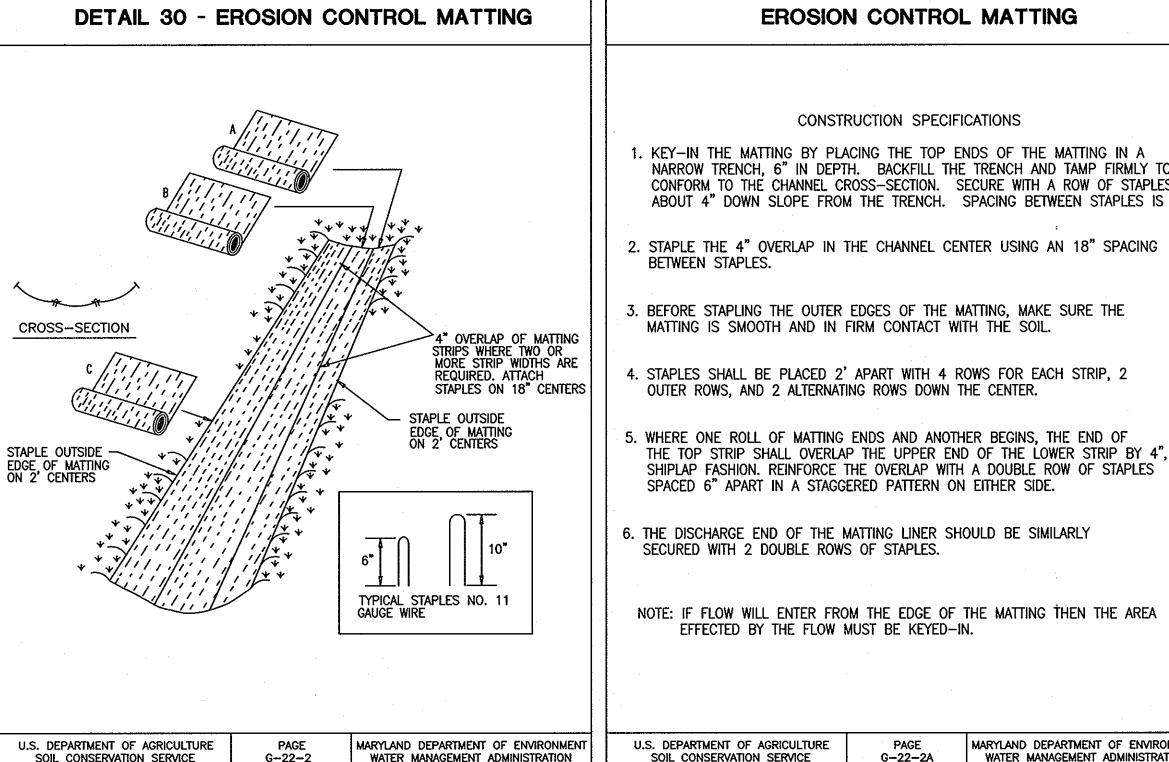






DIRECTOR, DEPARTMENT OF PLANNING AND ZONING

HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING



- KEY-IN THE MATTING BY PLACING THE TOP ENDS OF THE MATTING IN A NARROW TRENCH. 6" IN DEPTH. BACKFILL THE TRENCH AND TAMP FIRMLY TO CONFORM TO THE CHANNEL CROSS-SECTION. SECURE WITH A ROW OF STAPLES ABOUT 4" DOWN SLOPE FROM THE TRENCH. SPACING BETWEEN STAPLES IS 6".
- 2. STAPLE THE 4" OVERLAP IN THE CHANNEL CENTER USING AN 18" SPACING
- 3. BEFORE STAPLING THE OUTER EDGES OF THE MATTING, MAKE SURE THE

- . THE DISCHARGE END OF THE MATTING LINER SHOULD BE SIMILARLY

MARYLAND DEPARTMENT OF ENVIRONMENT WATER MANAGEMENT ADMINISTRATION SOIL CONSERVATION SERVICE G-22-2A

VEGETATIVE STABILIZATION

PERMANENT AND TEMPORARY SEEDING, SODDING AND MULCHING FOLLOWING INITIAL SOIL DISTURBANCE OR RE-DISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN SEVEN CALENDAR DAYS FOR THE SURFACE OF ALL PERIMETER CONTROLS, DIKES, SWALES, DITCHED. PERIMETER SLOPES, AND ALL SLOPES GREATER THAN 3 HORIZONTAL TO 1 VERTICAL (3:1) AND FOURTEEN DAYS FOR ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE. PERMANENT SEEDING NOTES

SOIL TEST: LIME FERTILIZATION WILL BE APPLIED PER SOIL TEST RESULTS FOR THE SITES GREATER THAN 5 ACRES. SOIL TESTS WILL BE DONE AT COMPLETION OF INITIAL ROUGH GRADING OR AS RECOMMENDED BY THE SEDIMENT CONTROL INSPECTOR. RATES AND ANALYSES WILL BE PROVIDED TO THE GRADING INSPECTOR. AS WELL AS THE CONTRACTOR.

OCCURRENCE OF ACID SULFATE SOLID (GRAYISH BLACK COLOR) WILL REQUIRE COVERING WITH A MUM OF 12 INCHES OF CLEAN SOIL WITH 6 INCHES MINIMUM CAPPING OF TOP SOIL NO STOCKPILING OF MATERIAL IS ALLOWED. IF NEEDED, SOIL TESTS SHOULD BE DONE BEFORE AND

AFTER A 6-WEEK INCUBATION PERIOD TO ALLOW OXIDATION OF SULFATES. APPLY GRADE OR CLEARED AREAS NOT SUBJECT TO IMMEDIATE FURTHER DISTURBANCE WHERE PERMANENT LONG-LIVED VEGETATIVE COVER IS NEEDED.

EEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISKING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED.

SOIL AMENDMENTS: IN LIEU OF SOIL TEST RECOMMENDATIONS, USE ONE OF THE FOLLOWING SCHEDULES: PREFERRED: APPLY 2 TONS/ ACRE DOLOMITIC LIMESTONE (92 LBS./ 1,000 SQ. FT.) AND ACRE 10-10-10 FERTILIZER (14. LBS./ 1.000 SQ. FT.) BEFORE SEEDING. UPPER THREE INCHES OF SOIL. AT TIME OF SEEDING, APPLY HARROW OR DISK INTO 400 LBS./ ACRE 30-0-0 UREAFORM FERTILIZER (9 LBS./ 1.000 SQ. FT.). ACCEPTABLE: APPLY 2 TONS/ ACRE DOLOMITIC LIMESTONE (9 LBS./ 1,000 SQ. FT.) AND ACRE 10-10-10 FERTILIZER (23 LBS./ 1,000 SQ. FT.) BEFORE SEEDING. HARROW OR DISK INTO

SEEDING: FOR THE PERIODS OF MARCH 1- APRIL 30, AND AUGUST 1- OCTOBER15, SEED WITH 60 LBS./ ACRE (1.4 LBS./ 1.000 SQ. FT.) OF KENTUCKY 31 TALL FESCUE. FOR THE PERIOD OF MAY 1- JULY 31. SEED WITH 60 LBS. OF KENTUCKY 31 TALL FESCUE PER ACRE AND 2 LBS./ ACRE (.05/ 1,000 SQ. FT.) OF WEEPING LOVEGRASS, DURING THE PERIOD OF OCTOBER 16- FEBRUARY 28, PROTECT SITE BY: OPTION 1: TWO TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING.

OPTION 3: SEED WITH 60 LBS./ ACRE KENTUCKY 30 TALL FESCUE AND MULCH WITH 2 TONS/ ACRES WELL ANCHORED STRAW. OPTION 2: USE SOD.

MULCHING: APPLY 1- 1/2 TO 2 TONS PER ACRE (70 TO 90 LBS./ 1,000 SQ. FT.) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ACHORING TOOL OR 218 GALLONS PER ACRE (5 GAL / 1,000 SQ. FT.) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPE 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL./ 1,000 SQ. FT.) FOR

SECURING STRAW MULCH: STRAW MULCH SHALL BE SECURED IMMEDIATELY FOLLOWING MULCH APPLICATIONS TO MINIMIZE MOVEMENT BY WIND OR WATER. THE FOLLOWING METHODS ARE PERMITTED:

- USE A MULCH-ANCHORING TOOL WHICH IS DESIGNED TO PUNCH AND ANCHOR MULCH INTO THE SOIL SURFACE TO A MINIMUM DEPTH OF 2 INCHES. THIS IS THE MOST EFFECTIVE METHOD FOR SECURING MULCH, HOWEVER, IT IS LIMITED TO RELATIVELY FLAT AREAS WHERE EQUIPMENT CAN
- WOOD CELLULOSE FIBER MAY BE USED FOR ANCHORING STRAW. APPLY THE FIBER BINDER AT A NET DRY WEIGHT OF 750 POUNDS PER ACRE. IF MIXED WITH WATER, USE 50 POUNDS OF WOOD CELLULOSE FIBER PER 100 GALLONS OF WATER.
- (iii) LIQUID BINDERS MAY BE USED. APPLY AT HIGHER RATES AT THE EDGES, WHERE WIND CATCHES MULCH, SUCH AS IN VALLEYS AND ON CRESTS OF SLOPES. THE REMAINDER OF THE AREA SHOULD APPEAR UNIFORM AFTER BINDER APPLICATION. BINDERS LISTED IN THE 1994 STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL OR APPROVED EQUAL SHALL BE APPLIED AT RATES RECOMMENDED BY THE MANUFACTURERS
- (iv) LIGHTWEIGHT PLASTIC NETTING MAY BE USED TO SECURE MULCH. THE NETTING WILL BE STAPLED TO THE GROUND ACCORDING TO MANUFACTURER'S RECOMMENDATIONS MAINTENANCE: INSPECT ALL SEEDING AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND RESEEDINGS.

TEMPORARY SEEDING NOTES APPLY TO GRADED OR CLEARED AREAS LIKELY TO BE RE-DITURBED WHERE A SHORT-TERM VEGETATIVE

DBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISKING OR OTHER APPLICABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED.

SOIL AMENDMENTS: APPLY 600 LBS./ ACRE 10-10-10 FERTILIZER (14 LBS./ 1,000 SQ. FT.).

SEEDING: FOR PERIODS MARCH 1- APRIL 30 AND FROM AUGUST 15- OCTOBER 15, SEED WITH 2-1/2 BUSHEL PER ACRE OF ANNUAL RYE (3.2 LBS./ 1,000 SQ. FT.). FOR THE PERIOD MAY 1- AUGUST 14, SEED WITH 3 LBS./ ACRE OF WEEPING LOVEGRASS (.07 LBS./ 1,000 SQ. FT.). FOR THE PERIOD NOVEMBER 16- FEBRUARY 28, PROTECT SITE BY APPLYING 2 TONS/ ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING, OR USE SOD.

MULCHING: APPLY 1-1/2 TO 2 TONS/ ACRE (70 TO 90 LBS./ SQ. FT.). OF UNROTTED WEED-FREE. SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GAL / ACRE (5 GAL / 1,000 SQ. FT.). OF EMULSIFIED ASPHALT ON FLAT AREAS. NO FILLS MAY BE PLACED ON FROZEN GROUND, ALL FILL TO BE PLACED IN APPROXIMATELY HORIZONTAL LAYER EACH LAYER HAVING A LOOSE THICKNESS OF NOT MORE THAN 8 INCHES ALL FILL IN ROADWAYS AND PARKING AREA IS TO BE CLASSIFIED TYPE 2 AS PER HOWARD COUNTY COUNDY CODE- ARTICLE 21, SECTION 2-308, AND COMPACTED TO 90% DENSITY: COMPACTION TO BE DETERMINED BY ASTM D-1557-66T (MODIFIED PROCTOR). ANY FILL WITHIN THE BUILDING AREA IS TO BE COMPACTED TO A MINIMUM OF 95 % DENSITY AS DETERMINED BY METHODS PREVIOUSLY MENTIONED. FILLS FOR POND EMBANKMENTS SHALL BE COMPACTED AS PER MD-378 CONSTRUCTION SPECIFICATIONS. ALL OTHER FILLS SHALL BE COMPACTED SUFFICIENTLY SO AS TO BE STABLE AND PREVEN EROSION AND SLIPPAGE

PERMANENT SOD INSTALLATION OF SOD SHOULD FOLLOW PERMANENT SEEDING DATES. SEEDBED PREPARATION FOR SOD SHALL BE AS NOTED IN SECTION (B) ABOVE. PERMANENT SOD IS TO BE TALL FESCUE, STATE APPROVED SOD; LIME AND FERTILIZER PER PERMANENT SEEDING SPECIFICATIONS AND LIGHTLY IRRIGATE SOIL PRIOR TO LAYING SOD. SOD IS TO BE LAID ON THE CONTOUR WITH ALL ENDS TIGHTLY ABUTTING. JOINTS ARE TO BE STAGGERED BETWEEN ROWS. WATER AND ROLL OR SOD TO INSURE POSITIVE ROOT CONTACT WITH THE SOIL. ALL SLOPES STEEPER THAN 3:1 s shown, are to be permanently sodded or protected with an approved erosion CONTROL NETTING. ADDITIONAL WATER FOR ESTABLISHMENT MAY BE REQUIRED. SOD IS NOT TO BE INSTALLED ON FROZEN GROUND. SOD SHALL NOT BE TRANSPLANTED WHEN MOISTURE CONTENT (DRY OR WET) AND/OR EXTREME TEMPERTURE MAY BE ADVERSELY AFFECT ITS SURVIVAL. IN THE ABSENCE OF ADEQUATE RAINFALL, IRRIGATION SHOULD PERFORMED TO ENSURE ESTABLISHMENT OF SOD

MINING OPERATIONS DOES NOT APPLY TO THIS PROJECT

VII. TOPSOIL SHALL BE APPLIED AS PER THE STANDARD AND SPECIFICATIONS FOR TOPSOIL FROM THE CURRENT MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL

USE OF THIS INFORMATION PRECLUDE MEETING ALL OF THE REQUIREMENT OF THE CURRENT MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT. NOTE: PROJECTS WITHIN 4 MILES OF THE BWI AIRPORT WILL NEED TO ADHERE TO MARYLAND AVIATION ADMINISTRATION'S SEEDING SPECIFICATION RESTRICTIONS.

STANDARDS AND SPECIFICATIONS FOR TOPSON

PLACEMENT OF TOPSOIL OVER A PREPARED SUBSOIL PRIOR TO ESTABLISHMENT OF PERMANENT VEGETATION.

TO PROVIDE A SUITABLE SOIL MEDIUM FOR VEGETATIVE GROWTH. SOILS OF CONCERN HAVE LOW MOISTURE CONTENT, LOW NUTRIENT LEVELS, LOW pH, MATERIAL TOXIC TO PLANTS, AND/OR UNACCEPTABLE SOIL GRADATION.

CONDITIONS WHERE PRACTICE APPLIES 1. THIS PRACTICE IS LIMITED TO AREAS HAVING 2:1 OR FLATTER SLOPES WHERE:

- A. THE TEXTURE OF THE EXPOSED SUBSOIL/PARENT MATERIAL IS NOT ADEQUATE TO PRODUCE VEGETATIVE GROWTH.
- B. THE SOIL MATERIAL IS SO SHALLOW THAT THE ROOTING ZONE IS NOT DEEP ENOUGH TO SUPPORT PLANTS OR FURNISH CONTINUING SUPPLIES OF MOISTURE
- C. THE ORIGINAL SOIL TO BE VEGETATED CONTAINS MATERIAL TOXIC TO PLANT

ADEQUATE STABILIZATION. AREAS HAVING SLOPES STEEPER THAN 2:1 SHALL HAVE THE

D. THE SOIL IS SO ACIDIC THAT TREATMENT WITH LIMESTONE IS NOT FEASIBLE. II. FOR THE PURPOSE OF THESE STANDARDS AND SPECIFICATIONS, AREAS HAVING SLOPES STEEPER THAN 2:1 REQUIRE SPECIAL CONSIDERATION AND DESIGN FOR

APPROPRIATE STABILIZATION SHOWN ON THE PLANS. CONSTRUCTION AND MATERIAL SPECIFICATIONS

I. TOPSOIL SALVAGED FROM THE EXISTING SITE MAY BE USED PROVIDED THAT IT MEETS STANDARDS AS SET FORTH IN THESE SPECIFICATIONS. TYPICALLY, THE DEPTH OF REPRESENTATIVES SOIL PROFILE SECTION IN THE SOIL SURVEY PUBLISHED BY USDA-SCS IN COOPERATION WITH MARYLAND AGRICULTURAL EXPERIMENTAL STATION.

II. TOPSOIL SPECIFICATIONS-SOIL TO BE USED AS TOPSOIL MUST MEET THE FOLLOWING:

- I. TOPSOIL SHALL BE A LOAM, SANDY LOAM, CLAY LOAM, SILT LOAM, SANDY CLAY LOAM, LOAMY SAND. OTHER SOILS MAY USED IF RECOMMENDED BY AN AGRONOMIST OR SOIL SCIENTIST AND APPROVED BY THE APPROPRIATE APPROVAL AUTHORITY. REGARDLESS TOPSOIL SHALL NOT BE A MIXTURE OF CONTRASTING TEXTURED SUBSOILS AND SHALL CONTAIN LESS THAN 5% BY VOLUME OF CINDERS, STONES, SLAG, COARSE FRAGMENTS, GRAVELS, STICKS, ROOTS, TRASH, OR OTHER MATERIALS LARGER THAN 1 1/2" IN
- II. TOPSOIL MUST BE FREE OF PLANTS OR PLANT PARTS SUCH AS BERMUDA GRASS, QUICKGRASS, JOHNSONGRASS, NUTSEDGE, POISON IVY, THISTLE, OR

III. WHERE THE SUBSOIL IS EITHER HIGHLY ACIDIC OR COMPOSED OF HEAVY CLAYS, GROUND LIMESTONE SHALL BE SPREAD AT THE RATE OF 4-8 TONS/ACRE (200-400 POUNDS PER 1,000 SF) PRIOR TO THE PLACEMENT OF TOPSOIL. LIME SHALL BE DISTRIBUTED UNIFORMLY OVER DESIGNATED AREAS AND WORKED INTO THE SOIL IN CONJUNCTION WITH TILLAGE OPERATIONS AS DESCRIBED IN THE FOLLOWING PROCEDURES.

II. FOR SITE HAVING DISTURBED AREAS UNDER 5 ACRE

1. PLACE TOPSOIL (IF REQUIRED) AND APPLY SOIL AMENDMENTS AS SPECIFIED IN 20.0 VEGETATIVE STABILIZATION—SECTION I—VEGETATIVE STABILIZATION METHODS AND MATERIALS.

III. TOPSOIL APPLICATION

- . WHEN TOPSOILING, MAINTAIN NEEDED EROSION AND SEDIMENT CONTROL PRACTICES SUCH AS DIVERSIONS, GRADE STABILIZATION STRUCTURES, EARTH DIKES, SLOPE SILT FENCE AND SEDIMENT TRAPS AND BASINS
- II. GRADES ON THE AREAS TO BE TOPSOILED, WHICH HAVE BEEN PREVIOUSLY ESTABLISHED, SHALL BE MAINTAINED, ALBEIT 4"-8"HIGHER IN ELEVATION.
- III. TOPSOIL SHALL BE UNIFORMLY DISTRIBUTED IN A 4"-8" LAYER AND LIGHTLY COMPACTED TO A MINIMUM THICKNESS OF 4". SPREADING SHALL BE PERFORMED IN SUCH A MANNER THAT SODDING OR SEEDING CAN PROCEED WITH A MINIMUM OF ADDITIONAL SOIL PREPARATION AND TILLAGE. ANY IRREGULARITIES IN THE SURFACE RESULTING FROM TOPSOILING (other operations shall be corrected in order to prevent the Formation of Depressions or Water Pockets.
- M. TOPSOIL SHALL NOT PLACED WHILE THE TOPSOIL OR SUBSOIL IS IN A FROZEN OR MUDDY CONDITION, WHEN THE SUBSOIL IS EXCESSIVELY WET OR IN A CONDITION THAT MAY OTHERWISE BE DETRIMENTAL TO PROPER CRADING AND SECRETARIES.
- VI. ALTERNATIVE FOR PERMANENT SEEDING-INSTEAD OF APPLYING THE FULL AMOUNTS OF LIME AND COMMERCIAL FERTILIZER, COMPOSTED SLUDGE AND AMENDMENTS MAY BE APPLIED AS SPECIFIED BELOW:
- I. COMPOSTED SLUDGE MATERIAL FOR USE A SOIL CONDITIONER FOR SITES HAVING DISTURBED AREAS OVER 5 ACRE SHALL BE TESTED TO PRESCRIBE AMENDMENTS AND FOR SITES HAVING DISTURBED AREAS UNDER 5 ACRES SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
- A. COMPOSTED SLUDGE SHALL BE SUPPLIED BY, OR ORIGINATE FROM, A PERSON OR PERSONS THAT ARE PERMITTED (AT THE TIME OF ACQUISITION OF THE COMPOST) BY THE MARYLAND DEPARTMENT OF THE ENVIRONMENT UNDER COMAR 26.04.06.
- 3. COMPOSTED SLUDGE SHALL CONTAIN AT LEAST 1 PERCENT NITROGEN 1.5 PERCENT PHOSPHOROUS, AND 0.2 PERCENT POTASSIUM AND HAVE A pH OF 7.0 TO 8.0. IF COMPOST DOES NOT MEET THESE REQUIREMENTS, THE APPROPRIATE CONSTITUENTS MUST BE ADDED TO MEET THE REQUIREMENTS PRIOR TO USE.

C. COMPOSTED SLUDGE SHALL BE APPLIED AT A RATE OF 1 TON/1,000

- II. COMPOSTED SLUDGE SHALL BE AMENDED WITH A POTASSIUM FERTILIZER
- APPLIED AT THE RATE OF 4LB/1,000 SF, AND 1/3 THE NORMAL LIME

GUIDELINE SPECIFICATIONS, SOIL PREPARATION AND SODDING. MD-VA. PUB. #1, COOPERATIVE EXTENSION SERVICE, UNIVERSITY OF MARYLAND AND VIRGINIA POLYTECHNIC

REVISED 1973.

STANDARDS AND SPECIFICATIONS FOR LAND GRADING

RESHAPING OF THE EXISTING LAND SURFACE IN ACCORDANCE WITH A PLAN AS DETERMINED BY ENGINEERING SURVEY AND LAYOUT.

THE PURPOSE OF A LAND GRADING SPECIFICATION IS TO PROVIDE FOR EROSION CONTROL AND VEGETATIVE ESTABLISHMENT ON THOSE AREAS WHERE THE EXISTING LAND SURFACE IS TO BE RESHAPED BY GRADING

Design Criteria

THE GRADING PLAN SHOULD BE BASED UPON THE INCORPORATION OF BUILDING DESIGNS AND STREET LAYOUTS THAT FIT AND UTILIZE EXISTING TOPOGRAPHY AND DESIRABLE NATURAL SURROUNDINGS TO AVOID EXTREME GRADE MODIFICATIONS. INFORMATION SUBMITTED MUST PROVIDE SUFFICIEN OPOGRAPHIC SURVEYS AND SOIL INVESTIGATIONS TO DETERMINE LIMITATIONS THAT MUST BE IMPOSED ON THE GRADING OPERATION RELATED TO SLOPE STABILITY, EFFECT ON ADJACENT PROPERTIES AND DRAINAGE PATTERNS, MEASURES FOR DRAINAGE AND WATER REMOVAL AND VEGETATIVE TREATMENT,

MANY COUNTIES HAVE REGULATIONS AND DESIGN PROCEDURES ALREADY ESTABLISHED FOR LAND GRADING AND CUT AND FILL SLOPES. WHERE THESE REQUIREMENTS EXIST, THEY SHALL BE FOLLOWED. THE PLAN MUST SHOW EXISTING AND PROPOSED CONTOURS OF THE AREA(S) TO BE GRADED. THE PLAN SHALL ALSO INCLUDE PRACTICES FOR EROSION CONTROL. SLOPE STABILIZATION, SAFE DISPOSAL OF RUNOFF WATER AND DRAINAGE, SUCH AS WATERWAYS, LINED DITCHES. REVERSE SLOPE BENCHES (INCLUDE GRADE AND CROSS SECTION), GRADE STABILIZATION STRUCTURES, RETAINING WALLS, AND SURFACE AND SUBSURFACE DRAINS. THE PLAN SHALL ALSO INCLUDE PHASING OF THESE PRACTICES. THE FOLLOWING SHALL BE INCORPORATED

- . PROVISIONS SHALL BE MADE TO SAFELY CONVEY SURFACE RUNOFF TO STORM DRAINS, PROTECTED OUTLETS OR TO STABLE WATER COURSES TO INSURE THAT SURFACE RUNOFF WILL NOT DAMAGE SLOPES OR OTHER GRADED AREAS.
- II. CUT AND FILL SLOPES THAT ARE TO BE STABILIZED WITH GRASSES SHALL NOT BE STEEPER THAN 2:1. (WHERE THE SLOPE IS TO BE MOWED THE SLOPE SHOULD BE NO STEEPER THAN 3:1, 4:1 IS PREFERRED BECAUSE OF SAFETY FACTORS RELATED TO MOWING STEEP SLOPES.) SLOPES EXCEEDING 2:1 SHALL REQUIRE SPECIAL DESIGN AND STABILIZATION CONSIDERATIONS THAT SHALL BE ADEQUATELY SHOWN ON THE PLANS.
- III. REVERSE BENCHES SHALL BE PROVIDED WHENEVER THE VERTICAL INTERVAL (HEIGHT) OF ANY 2:1 SLOPE EXCEEDS 20 FEET; FOR 3:1 T SHALL BE INCREASED TO 30 FEET AND FOR 4:1 TO 40 FEET. BENCHES SHALL BE LOCATED TO DIMDE THE SLOPE FACE AS EQUALLY AS POSSIBLE AND SHALL CONVEY THE WATER TO A STABLE OUTLET. SOILS, SEEPS, ROCK OUTCROPS, ETC., SHALL ALSO BE TAKEN INTO CONSIDERATION WHEN DESIGNING BENCHES.
 - A. BENCHES SHALL BE A MINIMUM OF SIX FEET WIDE TO PROVIDE FOR EASE OF MAINTENANCE
 - B. BENCHES SHALL BE DESIGNED WITH A REVERSE SLOPE OF 6:1 OR FLATTER TO THE TOE OF THE UPPER SLOPE IND WITH A MINIMUM OF ONE FOOT IN DEPTH. BENCH GRADIENT TO THE OUTLET SHALL BE BETWEEN 2 PERCENT AND 3 PERCENT, UNLESS ACCOMPANIED BY APPROPRIATE DESIGN AND COMPUTATIONS.
- C. THE FLOW LENGTH WITHIN A BENCH SHALL NOT EXCEED 800' UNLESS ACCOMPANIED BY APPROPRIATE DESIGN AND COMPUTATIONS. FOR FLOW CHANNEL STABILIZATION SEE
- IV. SURFACE WATER SHALL BE DIVERTED FROM THE FACE OF ALL CUT AND/OR FILL SLOPES BY THE USE OF EARTH DIKES, DITCHES AND SWALES OR CONVEYED DOWNSLOPE BY THE USE OF A DESIGNED
 - A. THE FACE OF THE SLOPE IS OR SHALL BE STABILIZED AND THE FACE OF ALL GRADED SLOPES SHALL BE PROTECTED FORM SURFACE RUNOFF UNTIL THEY ARE
 - B. THE FACE OF SLOPE SHALL NOT BE SUBJECT TO ANY CONCENTRATED FLOWS OF SURFACE WATER SUCH AS FROM NATURAL DRAINAGEWAYS, GRADED SWALES, DOWNSPOUTS,
 - . C. THE FACE OF THE SLOPE WILL BE PROTECTED BY SPECIAL EROSION CONTROL MATERIALS, TO INCLUDE, BUT NOT LIMITED TO: APPROVED VEGETATIVE STABILIZATION PRACTICES (SEE SECTION G), RIP-RAP OR OTHER APPROVED STABILIZATION METHODS.
- V. CUT SLOPES OCCURRING IN RIPABLE ROCK SHALL BE SERRATED A SHOWN ON THE FOLLOWING DIAGRAM. THESE SERRATION SHALL BE MADE WITH CONVENTIONAL EQUIPMENT AS THE EXCAVATION IS MADE. EACH STEP OR SERRATION SHALL BE CONSTRUCTED ON THE CONTOUR AND WILL HAVE STEPS CUT AT NOMINAL TWO-FOOT INTERVALS WITH NOMINAL THREE-FOOT HORIZONTAL SHELVES. THESE STEPS WILL VARY DEPENDING ON THE SLOPE RATIO OR THE CUT SLOPE. THE NOMINAL SLOPE LINE IS 1:1. THESE STEPS WILL WEATHER AND ACT TO HOLD MOISTURE, LIME, FERTILIZER AND SEED THUS PRODUCING A MUCH QUICKER AND LONGER LIVED VEGETATIVE COVER AND BETTER SLOPE STABILIZATION, OVERLAND FLOW SHALL BE DIVERTED FROM THE TOP OF ALL SERRATED CUT SLOPES AND
- VI. SUBSURFACE DRAINAGE SHALL BE PROVIDED WHERE NECESSARY TO INTERCEPT SEEPAGE THAT WOULD OTHERWISE ADVERSELY AFFECT SLOPE STABILITY OR CREATE EXCESSIVELY WET SITE CONDITIONS.

CARRIED TO A SUITABLE OUTLET.

- VII. SLOPES SHALL NOT BE CREATED SO CLOSE TO PROPERTY LINES AS TO ENDANGER ADJOINING PROPERTIES WITHOUT ADEQUATELY PROTECTING SUCH PROPERTIES AGAINST SEDIMENTATION, EROSION, SLIPPAGE, SETTLEMENT, SUBSIDENCE OR OTHER RELATED DAMAGES.
- VIII. FILL MATERIAL SHALL BE FREE OF BRUSH, RUBBISH, ROCKS, LOGS, STUMPS, BUILDING DEBRIS, AND OTHER OBJECTIONABLE MATERIAL IT SHOULD BE FREE OF STONES OVER TWO (2) INCHES IN DIAMETER WHERE COMPACTED BY HAND OR MECHANICAL TAMPERS OR OVER EIGHT (8) INCHES IN DIAMETER WHERE COMPACTED BY ROLLERS OR OTHER EQUIPMENT. FROZEN MATERIAL SHALL NOT BE PLACED IN THE FILL NOR SHALL THE FILL MATERIAL BE PLACED ON A FROZEN FOUNDATION.
- IX. STOCKPILES, BORROW AREAS AND SPOIL SHALL BE SHOWN ON THE PLANS AND SHALL BE SUBJECT TO THE PROVISIONS OF THIS STANDARD AND SPECIFICATIONS.
- X. ALL DISTURBED AREAS SHALL BE STABILIZED STRUCTURALLY OR VEGETATIVELY IN COMPLIANCE WITH 20.0 STANDARDS AND SPECIFICATIONS FOR VEGETATIVE STABILIZATION.

FOR UTILITY WORK ONLY OR FOR OFF-SITE UTILITY WORK

- 1. PLACE ALL EXCAVATED MATERIAL ON HIGH SIDE OF TRENCH.
- 2. ONLY DO AS MUCH WORK AS CAN BE DONE IN ONE DAY SO BACKFILLING FINAL GRADING, AND SEEDING AND MULCHING CAN OCCUR.
- 3. ANY SEDIMENT CONTROL MEASURES DISTURBED BY CONSTRUCTION WILL BE REPAIRED THE SAME DAY.

STOCKPILE NOTES

 NO STOCKPILING ALLOWED ON ASPHALT. 2. ALL STOCKPILES LEFT AT THE END OF THE DAY NEED TO BE STABILIZED UNTIL THE NEXT REDISTURBANCE.



MORRIS & RITCHIE ASSOCIATES, INC

ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

14280 PARK CENTER DRIVE, SUITE A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690 FAX (410) 792-7395

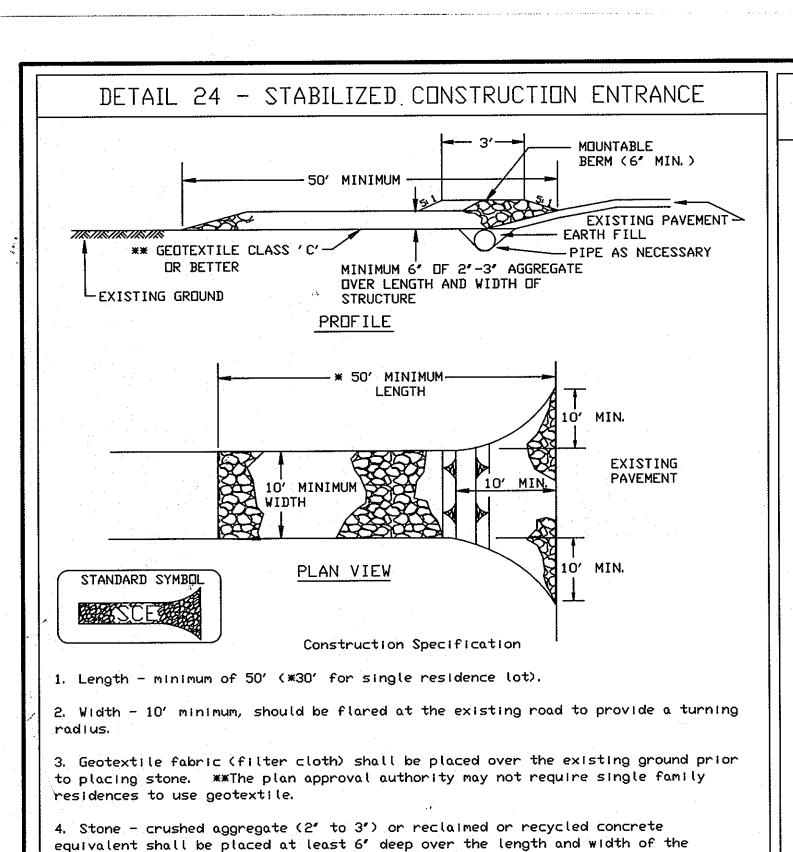


SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

ESC NOTES & DETAILS

LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382 ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3 OTS: J. K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

·	DATE	REVISIONS	JOB NO.:	1 4 719
NER/DEVELOPER/APPLICANT			SCALE:	AS SHOWN
· · ·			DATE:	4/24/07
ME: MR. BRIAN KNAUFF MPANY: THE RYLAND GROUP 11 UNIVERSITY BOULEVARD, SUITE 260 LICOTT CITY, MARYLAND 21043 ONE: 410-480-2467 X: 410-480-0543			DRAWN BY:	MM
			DESIGN BY:	MM
			REVIEW BY:	KSK
			SHEET:	24 OF 33



5. Surface Water - all surface water flowing to or diverted toward construction

has no drainage to convey a pipe will not be necessary. Pipe should be sized

entrances shall be piped through the entrance, maintaining positive drainage. Pipe

mountable berm with 5:1 slopes and a minimum of 6" of stone over the pipe. Pipe has

to be sized according to the drainage. When the SCE is located at a high spot and

installed through the stabilized construction entrance shall be protected with a

according to the amount of runoff to be conveyed. A 6' minimum will be required.

6. Location - A stabilized construction entrance shall be located at every point

where construction traffic enters or leaves a construction site. Vehicles leaving the site must travel over the entire length of the stabilized construction entrance.

F - 17 - 3

entrance.

U.S. DEPARTMENT OF AGRICULTURE

SOIL CONSERVATION SERVICE

APPROVED

PLANNING BOARD

of HOWARD COUNTY

THE PROJECT. I ALSO AUTHORIZE ON-SITE INSPECTION BY

"I CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL

PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE

REVIEWED FOR HOWARD S.C.D. AND MEETS TECHNICAL REQUIREMENTS.

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND

SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION

REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY

THE HOWARD SOIL CONSERVATION DISTRICT."

KERRI S. KNIGHTEN, P.E. MD LICENSE #201135

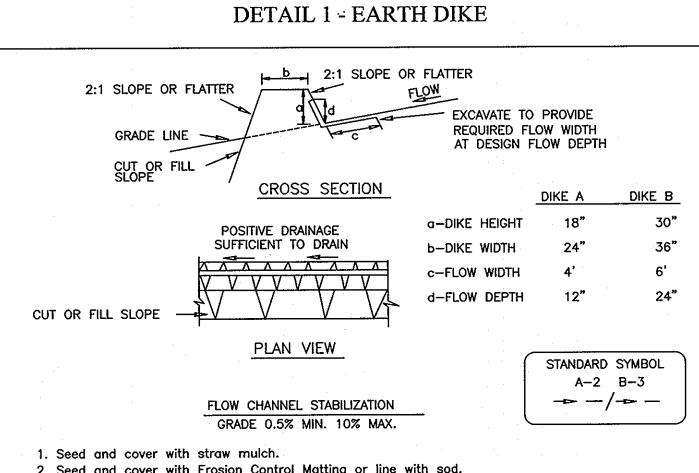
U.S.D.A.-NATURAL RESOURCE

CONSERVATION SERVICE

BY THE DEVELOPER

RYLAND HOMES, INC.

BY THE ENGINEER



2. Seed and cover with Erosion Control Matting or line with sod 3. 4" - 7" stone or recycled concrete equivalent pressed into the soil 7" minimum

Construction Specifications

- 1. All temporary earth dikes shall have uninterrupted positive grade to an outlet. Spot elevations may be necessary for grades less than 1%.
- 2. Runoff diverted from a disturbed area shall be conveyed to a sediment
- 3. Runoff diverted from an undisturbed area shall outlet directly into an undisturbed, stabilized area at a non-erosive velocity.
- 4. All trees, brush, stumps, obstructions, and other objectional material shall be removed and disposed of so as not to interfere with the proper functioning of the dike.
- 5. The dike shall be excavated or shaped to line, grade and cross section as required to meet the criteria specified herein and be free of bank projections or other irregularities which will impede normal flow.
- 6. Fill shall be compacted by earth moving equipment.
- 7. All earth removed and not needed for construction shall be placed so that it will not interfere with the functioning of the dike.
- 8. Inspection and maintenance must be provided periodically and after

U.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT SOIL CONSERVATION SERVICE WATER MANAGEMENT ADMINISTRATION A-1-6

DETAIL 23A - STANDARD INLET PROTECTION

2" X 4" FRAMING

-TOP ELEVATION

POST DRIVEN

INTO GROUND

Construction Specifications

1. Excavate completely around the inlet to a depth of 18" below the

2. Drive the 2" x 4" construction grade lumber posts 1' into the ground at each corner of the inlet. Place nail strips between the

posts on the ends of the inlet. Assemble the top portion of the

2" x 4" frame using the overlap joint shown on Detail 23A. The top of the frame (weir) must be 6' below adjacent roadways where

3. Stretch the $1/2^{\circ} \times 1/2^{\circ}$ wire mesh tightly around the frame

4. Stretch the Geotextile Class E tightly over the wire mesh with

inlet notch elevation. Fasten the geotextile firmly to the frame.

The ends of the geotextile must meet at a post, be overlapped and

5. Backfill around the inlet in compacted 6' layers until the

should be at least 6' higher than the top of the frame.

laver of earth is level with the notch elevation on the ends and

6. If the inlet is not in a sump, construct a compacted earth dike

7. The structure must be inspected periodically and after each

across the ditch line directly below it. The top of the earth dike

E - 16 - 5

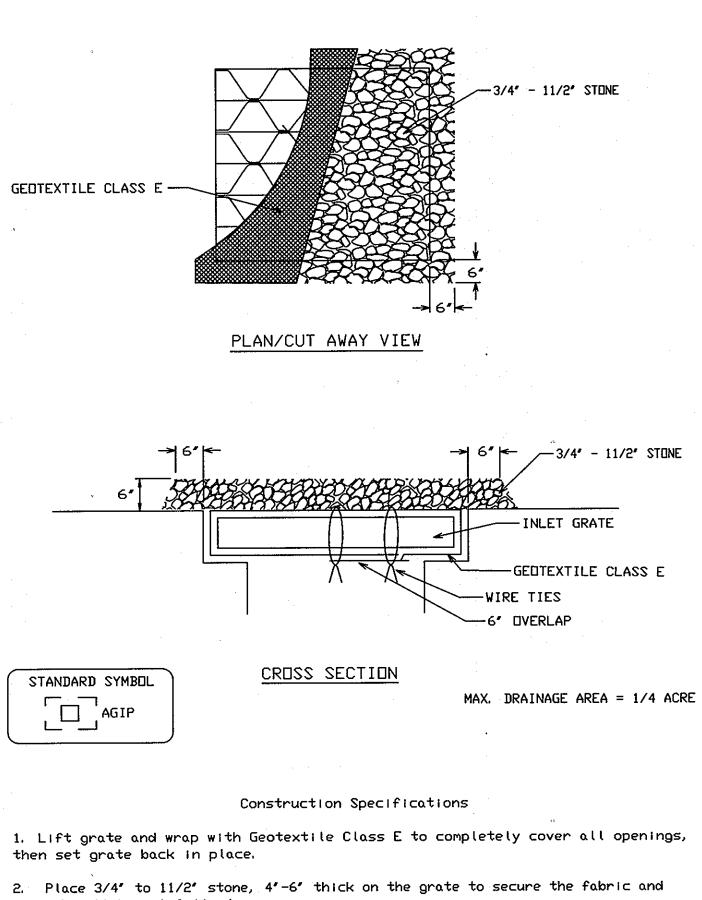
the geotixtile extending from the top of the frame to 18' below the

and fasten securely. The ends must meet and overlap at a

flooding and safety issues may arise.

folded, then fastened down.

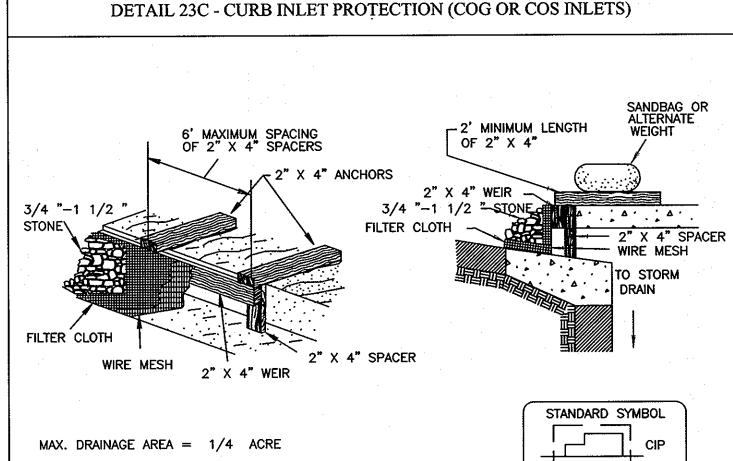
top elevation on the sides.



DETAIL 23B - AT GRADE INLET PROTECTION

provide additional filtration,

MARYLAND DEPARTMENT OF ENVIRONMENT U.S. DEPARTMENT OF AGRICULTURE WATER MANAGEMENT ADMINISTRATION SDIL CONSERVATION SERVICE E - 16 - 5A



Construction Specifications

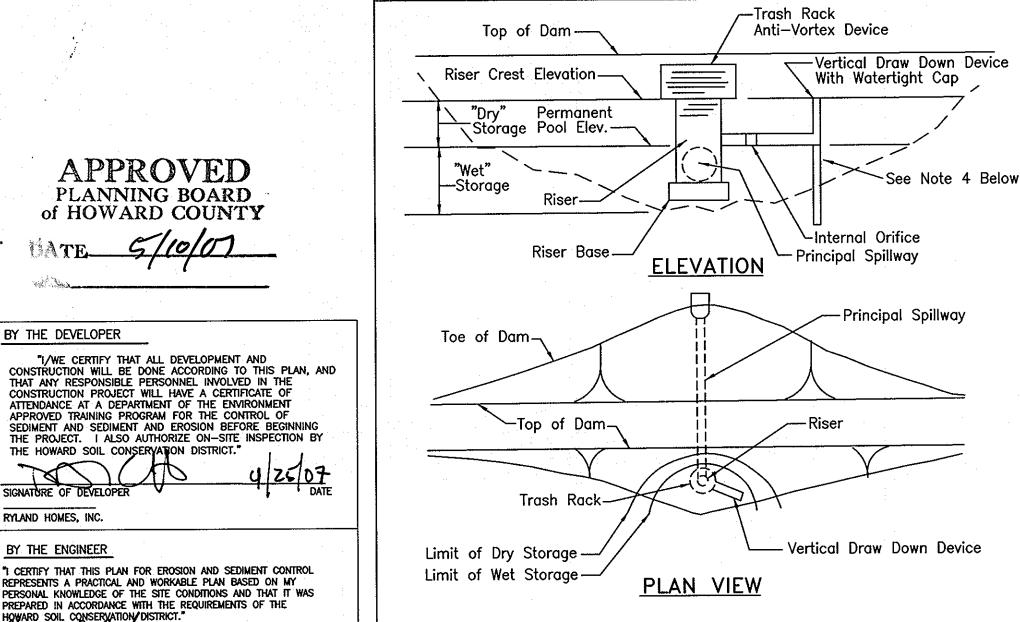
- 1. Attach a continuous piece of wire mesh (30" minimum width by throat length plus 4') to the 2" x 4" weir (measuring throat length plus 2') as shown on the standard
- 2. Place a continuous piece of Geotextile Class E the same dimensions as the wire mesh over the wire mesh and securely attach it to the 2" x 4" weir.
- 3. Securely nail the 2" X 4" weir to a 9" long vertical spacer to be located between the weir and the inlet face (max. 4' apart).
- 4. Place the assembly against the inlet throat and nail (minimum 2' lengths of 2" x 4" to the top of the weir at spacer locations). These 2" x 4" anchors shall extend across the inlet top and be held in place by sandbags or alternate weight.
- 5. The assembly shall be placed so that the end spacers are a minimum 1' beyond both ends of the throat opening.
- 6. Form the 1/2 " x 1/2 " wire mesh and the geotextile fabric to the concrete gutter and against the face of the curb on both sides of the inlet. Place clean 3/4 " x 1 1/2 " stone over the wire mesh and geotextile in such a manner to prevent water from entering the inlet under or around the geotextile.
- 7. This type of protection must be inspected frequently and the filter cloth and stone replaced when cloqued with sediment.
- 8. Assure that storm flow does not bypass the inlet by installing a temporary
- earth or asphalt dike to direct the flow to the inlet.

MARYLAND DEPARTMENT OF ENVIRONMENT U.S. DEPARTMENT OF AGRICULTURE PAGE WATER MANAGEMENT ADMINISTRATION SOIL CONSERVATION SERVICE E-16-5B

BASIN DRAWDOWN SCHEMATIC VERTICAL DRAW-DOWN DEVICE —Trash Rack Anti-Vortex Device Top of Dam-

MARYLAND DEPARTMENT OF ENVIRONMENT

WATER MANAGEMENT ADMINISTRATION



CONSTRUCTION SPECIFICATIONS

- the internal orifice.
- hardware cloth and geotextile fabric. The geotextile fabric shall meet the specifications for for Geotextile Class E.
- 4. Provide support of draw-down device to prevent sagging and floatation. An acceptable preventative measure is to stake both sides of draw-down device with 1" steel angle, or by 1' by 4" square or 2" round wooden posts set 3' minimum into the ground then joining them to the device by wrapping with

12 gauge minimum wire. U.S. DEPARTMENT OF AGRICULTURE

SOIL CONSERVATION SERVICE APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING The first of the first of the second of the rang personal and represent the color of the

GEDTEXTILE CLASS E MAX. DRAINAGE AREA = 1/4 ACRE

- 1. Perforations in the draw-down device may not extend into the wet storage.
- 2. The total area of the perforations must be greater than 2 times the area of
- 3. The perforated portion of the draw-down device shall be wrapped with 1/2"

MARYLAND DEPARTMENT OF ENVIRONMENT WATER MANAGEMENT ADMINISTRATION

rain and the geotextile replaced when it becomes clogged.

U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE

notch elevation.

MARYLAND DEPARTMENT OF ENVIRONMENT WATER MANAGEMENT ADMINISTRATION

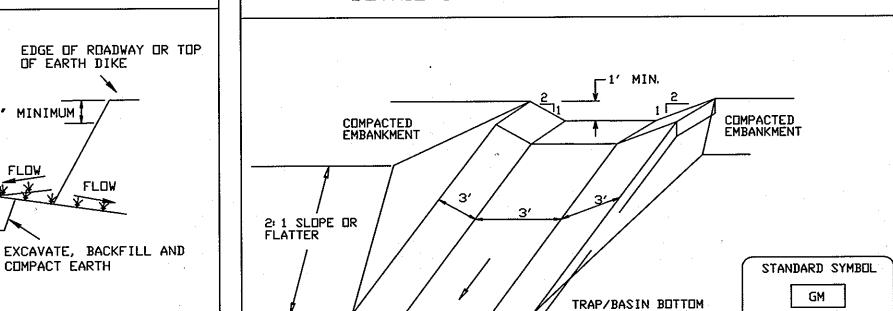
OF EARTH DIKE

6' MINIMUM!

COMPACT EARTH

STANDARD SYMBOL

SIP



DETAIL 6 - GABION INFLOW PROTECTION

PERSPECTIVE VIEW TRAP/BASIN BOTTOM

PROFILE ALONG CENTERLINE

Construction Specifications

- 1. Gabion inflow protection shall be constructed of 9' imes 3' imes 9' gabion baskets forming a trapezoidal cross section 1' deep, with 2:1 side slopes, and a 3' bottom width.
- 2. Geotextile Class C shall be installed under all gabion baskets.
- 3. The stone used to fill the gabion baskets shall be 4" 7".
- 4. Gabions shall be installed in accordance with manufacturers recommendations.
- 5. Gabion Inflow Protection shall be used where concentrated flow is present on slopes steeper than 4:1.

U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE B - 7 - 2

MARYLAND DEPARTMENT OF ENVIRONMENT WATER MANAGEMENT ADMINISTRATION

OWNER/DEVELOPER/APPLICANT

NAME: MR. BRIAN KNAUFF COMPANY: THE RYLAND GROUP 6011 UNIVERSITY BOULEVARD, SUITE 260 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-480-2467 FAX: 410-480-0543



MORRIS & RITCHIE ASSOCIATES, INC. ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

14280 PARK CENTER DRIVE. SUITE A LAUREL. MARYLAND 20707

(410) 792-9792 or (301) 776-1690 FAX (410) 792-7395



SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAL 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

ESC DETAILS

LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382 ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3 LOTS: J. K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

JOB NO.: DATE | REVISIONS SCALE: AS SHOWN 4/24/07 DATE: DRAWN BY: DESIGN BY: REVIEW BY: SHEET: 25 OF 33

MATERIAL

AREAS DESIGNATED FOR BORROW AREAS, EMBANKMENT, AND STRUCTURAL WORKS SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL. ALL TREES, VEGETATION, ROOTS AND OTHER OBJECTIONABLE MATERIAL SHALL BE REMOVED. CHANNEL BANKS AND SHARP BREAKS SHALL BE SLOPED TO NO STEEPER THAN 1:1. ALL TREES SHALL BE CLEARED AND GRUBBED WITHIN 15 FEET OF THE TOE OF THE EMBANKMENT. AREAS TO BE COVERED BY THE RESERVOIR WILL BE CLEARED OF ALL TREES, BRUSH, LOGS, FENCES, RUBBISH AND OTHER OBJECTIONABLE MATERIAL UNLESS OTHERWISE DESIGNATED ON THE PLANS. TREES, BRUSH, AND STUMPS SHALL BE CUT APPROXIMATELY LEVEL WITH THE GROUND SURFACE. FOR DRY STORMWATER MANAGEMENT PONDS, A MINIMUM OF A 25-FOOT RADIUS AROUND THE INLET STRUCTURE SHALL BE CLEARED. ALL CLEARED AND GRUBBED MATERIAL SHALL BE DISPOSED OF OUTSIDE AND BELOW THE LIMITS OF THE DAM AND RESERVOIR AS DIRECTED BY THE OWNER OR HIS REPRESENTATIVE. WHEN SPECIFIED, A SUFFICIENT QUANTITY OF TOPSOIL WILL BE STOCKPILED IN A SUITABLE LOCATION FOR USE ON THE EMBANKMENT AND OTHER DESIGNATED AREAS. EARTH FILL

THE FILL MATERIAL SHALL BE TAKEN FROM APPROVED DESIGNATED BORROW AREAS. IT SHALL BE FREE OF ROOTS, STUMPS, WOOD, RUBBISH, STONES GREATER THAN 6°, FROZEN OR OTHER OBJECTIONABLE MATERIALS. FILL MATERIAL FOR THE CENTER OF THE EMBANKMENT, AND CUT OFF TRENCH SHALL CONFORM TO UNIFIED SOIL CLASSIFICATION GC, SC, CH, OR CL AND MUST HAVE AT LEAST 30% PASSING THE #200 SIEVE. CONSIDERATION MAY BE GIVEN TO THE USE OF OTHER MATERIALS IN THE EMBANKMENT IF DESIGNED BY A GEOTECHNICAL ENGINEER. SUCH SPECIAL DESIGNS MUST HAVE CONSTRUCTION SUPERVISED BY A GEOTECHNICAL ENGINEER. MATERIALS USED IN THE OUTER SHELL OF THE EMBANKMENT MUST HAVE THE CAPABILITY TO SUPPORT VEGETATION OF THE QUALITY REQUIRED TO PREVENT EROSION OF THE EMBANKMENT.

areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials SHALL BE PLACED IN MAXIMUM 8 INCH THICK (BEFORE COMPACTION) LAYERS WHICH ARE TO BE CONTINUOUS OVER THE ENTIRE LENGTH OF THE FILL. THE MOST PERMEABLE BORROW MATERIAL SHALL BE PLACED IN THE DOWNSTREAM PORTIONS OF THE EMBANKMENT. THE PRINCIPAL SPILLWAY MUST BE INSTALLED CONCURRENTLY WITH FILL PLACEMENT AND NOT EXCAVATED INTO THE EMBANKMENT.

the movement of the hauling and spreading equipment over the fill shall be controlled so that the entire THE MOVEMENT OF THE HAULING AND SPREADING EQUIPMENT OVER THE FILL SHALL BE CONTROLLED SO THAT THE ENTIRE SURFACE OF EACH LIFT SHALL BE TRAVERSED BY NOT LESS THAN ONE TREAD TRACK OF HEAVY EQUIPMENT OR COMPACTION SHALL BE ACHIEVED BY A MINIMUM OF FOUR COMPLETE PASSES OF A SHEEPSFOOT, RUBBER TIRED OR VIBRATORY ROLLER. FILL MATERIAL SHALL CONTAIN SUFFICIENT MOISTURE SUCH THAT THE REQUIRED DEGREE OF COMPACTION WILL BE OBTAINED WITH THE EQUIPMENT USED. THE FILL MATERIAL SHALL CONTAIN SUFFICIENT MOISTURE SO THAT IF FORMED INTO A BALL IT WILL NOT CRUMBLE, YET NOT BE SO WET THAT WATER CAN BE SQUEEZED OUT. WHEN REQUIRED BY THE REVIEWING AGENCY THE MINIMUM REQUIRED DENSITY SHALL NOT BE LESS THAN 95% OF MAXIMUM DRY DENSITY WITH A MOISTURE CONTENT WITHIN 1/-2% OF THE OPTIMUM. EACH LAYER OF FILL SHALL BE COMPACTED AS NECESSARY TO OBTAIN THAT DENSITY, AND IS TO BE CERTIFIED BY THE ENGINEER AT THE TIME OF CONSTRUCTION. ALL COMPACTION IS TO BE DETERMINED BY AASHTO METHOD T-99 (STANDARD PROCTOR) DETERMINED BY AASHTO METHOD T-99 (STANDARD PROCTOR).

CUT OFF TRENCH THE CUTOFF TRENCH SHALL BE EXCAVATED INTO IMPERVIOUS MATERIAL ALONG OR PARALLEL TO THE CENTERLINE OF THE EMBANKMENT AS SHOWN ON THE PLANS. THE BOTTOM WIDTH OF THE TRENCH SHALL BE GOVERNED BY THE EQUIPMENT USED FOR EXCAVATION, WITH THE MINIMUM WIDTH BEING FOUR FEET. THE DEPTH SHALL BE AT LEAST FOUR FEET BELOW EXISTING GRADE OR AS SHOWN ON THE PLANS. THE SIDE SLOPES OF THE TRENCH SHALL BE 1 TO 1 OR FLATTER. THE BACKFILL SHALL BE COMPACTED WITH CONSTRUCTION EQUIPMENT, ROLLERS, OR HAND TAMPERS TO ASSURE MAXIMUM DENSITY AND MINIMUM PERMEABILITY

THE CORE SHALL BE PARALLEL TO THE CENTERLINE OF THE EMBANKMENT AS SHOWN ON THE PLANS. THE TOP WIDTH OF THE CORE SHALL BE A MINIMUM OF FOUR FEET. THE HEIGHT SHALL EXTEND UP TO AT LEAST THE 10 YEAR WATER ELEVATION OR AS SHOWN ON THE PLANS. THE SIDE SLOPES SHALL BE 1 TO 1 OR FLATTER. THE CORE SHALL BE COMPACTED WITH CONSTRUCTION EQUIPMENT, ROLLERS, OR HAND TAMPERS TO ASSURE MAXIMUM DENSITY AND MINIMUM PERMEABILITY. IN ADDITION, THE CORE SHALL BE PLACED CONCURRENTLY WITH THE OUTER SHELL OF THE EMBANKMENT.

STRUCTURE BACKFILL BACKFILL ADJACENT TO PIPES OR STRUCTURES SHALL BE OF THE TYPE AND QUALITY CONFORMING TO THAT SPECIFIED FOR THE ADJOINING FILL MATERIAL. THE FILL SHALL BE PLACED IN HORIZONTAL LAYERS NOT TO EXCEED FOUR INCHES IN THICKNESS AND COMPACTED BY HAND TAMPERS OR OTHER MANUALLY DIRECTED COMPACTION EQUIPMENT. THE MATERIAL NEEDS TO FILL COMPLETELY ALL SPACES UNDER AND ADJACENT TO THE PIPE. AT NO TIME DURING THE BACKFILLING OPERATION SHALL DRIVEN EQUIPMENT BE ALLOWED TO OPERATE CLOSER THAN FOUR FEET, MEASURED HORIZONTALLY, TO ANY PART OF A STRUCTURE. UNDER NO CIRCUMSTANCES SHALL EQUIPMENT BE DRIVEN OVER ANY PART OF A CONCRETE STRUCTURE OR PIPE, UNLESS THERE IS A COMPACTED FILL OF 24" OR GREATER OVER THE STRUCTURE OR PIPE. STRUCTURE BACKFILL MAY BE FLOWABLE FILL MEETING THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION, STATE HIGHWAY ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION 313 AS MODIFIED. THE MIXTURE SHALL HAVE A 100—200 PSI; 28 DAY UNCONFINED COMPRESSIVE STRENGTH. THE FLOWABLE FILL SHALL HAVE A MINIMUM PH OF 4.0 AND A MINIMUM RESISTIVITY OF 2,000 OHM—CM. MATERIAL SHALL BE PLACED SUCH THAT A MINIMUM OF 6" (MEASURED PERPENDICULAR TO THE OUTSIDE OF THE PIPE) OF FLOWABLE FILL SHALL BE UNDER (BEDDING), OVER AND, ON THE SIDES OF THE PIPE. IT ONLY NEEDS TO EXTEND UP TO THE SPRING LINE FOR RIGID CONDUITS. AVERAGE SLUMP OF THE FILL SHALL BE 7" TO ASSURE FLOWABILITY OF THE MATERIAL. ADEQUATE MEASURES SHALL BE BITUMINOUS COATED. ANY ADJOINING SOIL FILL SHALL BE PLACED IN HORIZONTAL LAYERS NOT TO EXCEED FOUR INCHES IN THICKNESS AND COMPACTED BY HAND TAMPERS OR OTHER MANUALLY DIRECTED COMPACTION EQUIPMENT. THE MATERIAL SHALL COMPLETELY FILL ALL VOIDS BACKFILL ADJACENT TO PIPES OR STRUCTURES SHALL BE OF THE TYPE AND QUALITY CONFORMING TO THAT SPECIFIED FOR TAMPERS OR OTHER MANUALLY DIRECTED COMPACTION EQUIPMENT. THE MATERIAL SHALL COMPLETELY FILL ALL VOIDS ADJACENT TO THE FLOWABLE FILL ZONE. AT NO TIME DURING THE BACKFILLING OPERATION SHALL DRIVEN EQUIPMENT BE ALLOWED TO OPERATE CLOSER THAN FOUR FEET, MEASURED HORIZONTALLY, TO ANY PART OF A STRUCTURE. UNDER NO CIRCUMSTANCES SHALL EQUIPMENT BE DRIVEN OVER ANY PART OF A STRUCTURE OR PIPE UNLESS THERE IS A COMPACTED FILL OF 24" OR GREATER OVER THE STRUCTURE OR PIPE. BACKFILL MATERIAL OUTSIDE THE STRUCTURAL BACKFILL (FLOWABLE FILL) ZONE SHALL BE OF THE TYPE AND QUALITY CONFORMING TO THAT SPECIFIED FOR THE CORE OF THE EMBANKMENT OR OTHER EMBANKMENT MATERIALS.

PIPE CONDUITS ALL PIPES SHALL BE CIRCULAR IN CROSS SECTION.

CORRUGATED METAL PIPE

MATERIALS — (POLYMER COATED STEEL PIPE) —STEEL PIPES WITH POLYMERIC COATINGS SHALL HAVE A MINIMUM COATING THICKNESS OF 0.01 INCH (10 MIL) ON BOTH SIDES OF THE PIPE. THIS PIPE AND ITS APPURTENANCES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO SPECIFICATIONS M—245 & M—246 WITH WATERTIGHT COUPLING BANDS OR FLANGES. MATERIALS — (ALUMINUM COATED STEEL PIPE) — THIS PIPE AND ITS APPURTENANCES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO SPECIFICATION M—274 WITH WATERTIGHT COUPLING BANDS OR FLANGES. ALUMINUM COATED STEEL PIPE, WHEN USED WITH FLOWABLE FILL OR WHEN SOIL AND/OR WATER CONDITIONS WARRANT THE NEED FOR INCREASED DURABILITY, SHALL BE FULLY BITUMINOUS COATED PER REQUIREMENTS OF AASHTO SPECIFICATION M—190 TYPE A. ANY ALUMINUM COATING DAMAGED OR OTHERWISE REMOVED SHALL BE REPLACED WITH COLD APPLIED BITUMINOUS COATING COM—POUND. ALUMINUM SURFACES THAT ARE TO BE IN CONTACT WITH CONCRETE SHALL BE PAINTED WITH ONE COAT OF ZINC CHROMATE PRIMER OR TWO COATS OF AASHTO SPECIFICATION M—196 OR M—211 WITH WATERTIGHT COUPLING BANDS OR FLANGES. ALUMINUM PIPE, WHEN USED WITH FLOWABLE FILL OR WHEN SOIL AND/OR WATER CONDITIONS WARRANT FOR INCREASED DURABILITY, SHALL BE FULLY BITUMINOUS COATED PER REQUIREMENTS OF AASHTO SPECIFICATION M—190 TYPE A. ALUMINUM SURFACES THAT ARE TO BE IN CONTACT WITH CONCRETE SHALL BE PAINTED WITH ONE COAT OF ZINC CHROMATE PRIMER OR TWO COATS OF ASSHALL HOT SPECIFICATIONS. THE PH OF THE SURROUNDING SOILS SHALL BE BETWEEN 4 AND 9.

COUPLING BANDS, ANTI—SEEP COLLARS, END SECTIONS, ETC., MUST BE COMPOSED OF THE SAME MATERIAL AND COATINGS CORRUGATED METAL PIPE COUPLING BANDS, ANTI-SEEP COLLARS, END SECTIONS, ETC., MUST BE COMPOSED OF THE SAME MATERIAL AND COATINGS AS THE PIPE. METALS MUST BE INSULATED FROM DISSIMILAR MATERIALS WITH USE OF RUBBER OR PLASTIC INSULATING

CONNECTIONS — ALL CONNECTIONS WITH PIPES MUST BE COMPLETELY WATERTIGHT. THE DRAIN PIPE OR BARREL CONNECTION TO THE RISER SHALL BE WELDED ALL AROUND WHEN THE PIPE AND RISER ARE METAL. ANTI—SEEP COLLARS SHALL BE CONNECTED TO THE PIPE IN SUCH A MANNER AS TO BE COMPLETELY WATERTIGHT. DIMPLE BANDS ARE NOT CONSIDERED TO BE WATERTIGHT. ALL CONNECTIONS SHALL USE A RUBBER OR NEOPRENE GASKET WHEN JOINING PIPE SECTIONS. THE TO BE WATERTIGHT: ALL CONNECTIONS SHALL USE A RUBBER OR NEOPRENE GASKET WHEN JOINING PIPE SECTIONS. THE END OF EACH PIPE SHALL BE RE-ROLLED AN ADEQUATE NUMBER OF CORRUGATIONS TO ACCOMMODATE THE BANDWIDTH. THE FOLLOWING TYPE CONNECTIONS ARE ACCEPTABLE FOR PIPES LESS THAN 24 INCHES IN DIAMETER: FLANGES ON BOTH ENDS OF THE PIPE WITH A CIRCULAR 3/8 INCH CLOSED CELL NEOPRENE GASKET, PRE-PUNCHED TO THE FLANGE BOLT CIRCLE, SANDWICHED BETWEEN ADJACENT FLANGES; A 12-INCH WIDE STANDARD LAP TYPE BAND WITH 12-INCH WIDE BY 3/8-INCH THICK CLOSED CELL CIRCULAR NEOPRENE GASKET; AND A 12-INCH WIDE HUGGER TYPE BAND WITH 0-RING GASKETS HAVING A MINIMUM DIAMETER OF 1/2 INCH GREATER THAN THE CORRUGATION DEPTH. PIPES 24 INCHES IN DIAMETER AND LARGER SHALL BE CONNECTED BY A 24 INCH LONG ANNULAR CORRUGATED BAND USING A MINIMUM OF 4 (FOUR) RODS AND LUGS, 2 ON EACH CONNECTING PIPE END. A 24-INCH WIDE BY 3/8-INCH THICK CLOSED CELL CIRCULAR NEOPRENE GASKET WILL BE INSTALLED WITH 12 INCHES ON THE END OF EACH PIPE. FLANGED JOINTS WITH 3/8 INCH CLOSED CELL GASKETS THE FULL WIDTH OF THE FLANGE IS ALSO ACCEPTABLE. HELICALLY CORRUGATED PIPE SHALL HAVE EITHER CONTINUOUSLY WELDED SEAMS OR HAVE LOCK SEAMS WITH INTERNAL CAULKING OR A NEOPRENE BEAD. WELDED SEAMS OR HAVE LOCK SEAMS WITH INTERNAL CAULKING OR A NEOPRENE BEAD.

BEDDING - THE PIPE SHALL BE FIRMLY AND UNIFORMLY BEDDED THROUGHOUT ITS ENTIRE LENGTH. WHERE ROCK OR SOFT, SPONGY OR OTHER UNSTABLE SOIL IS ENCOUNTERED, ALL SUCH MATERIAL SHALL BE REMOVED AND REPLACED WITH SUITABLE EARTH COMPACTED TO PROVIDE ADEQUATE SUPPORT.

BACKFILLING SHALL CONFORM TO "STRUCTURE BACKFILL" 6. OTHER DETAILS (ANTI-SEEP COLLARS, VALVES, ETC.) SHALL BE AS SHOWN ON THE DRAWINGS.

REINFORCED CONCRETE PIPE

MATERIALS - REINFORCED CONCRETE PIPE SHALL HAVE BELL AND SPIGOT JOINTS WITH RUBBER GASKETS AND SHALL EQUAL OR EXCEED ASTM C-361. BEDDING - REINFORCED CONCRETE PIPE CONDUITS SHALL BE LAID IN A CONCRETE BEDDING/CRADLE FOR THEIR ENTIRE LENGTH. THIS BEDDING/CRADLE SHALL CONSIST OF HIGH SLUMP CONCRETE PLACED UNDER THE PIPE AND UP THE SIDES OF THE PIPE AT LEAST 50% OF ITS OUTSIDE DIAMETER WITH A MINIMUM THICKNESS OF 6 INCHES. WHERE A CONCRETE CRADLE IS NOT NEEDED FOR STRUCTURAL REASONS, FLOWABLE FILL MAY BE USED AS DESCRIBED IN THE "STRUCTURE BACKFILL" SECTION OF THIS STANDARD. GRAVEL BEDDING IS NOT PERMITTED.

LAYING PIPE - BELL AND SPIGOT PIPE SHALL BE PLACED WITH THE BELL END UPSTREAM. JOINTS SHALL BE MADE IN ACCORDANCE WITH RECOMMENDATIONS OF THE MANUFACTURER OF THE MATERIAL. AFTER THE JOINTS ARE SEALED FOR THE ENTIRE LINE, THE BEDDING SHALL BE PLACED SO THAT ALL SPACES UNDER THE PIPE ARE FILLED. CARE SHALL BE EXERCISED TO PREVENT ANY DEVIATION FROM THE ORIGINAL LINE AND GRADE OF THE PIPE. THE FIRST JOINT MUST BE LOCATED WITHIN 4 FEET FROM THE RISER.

4. BACKFILLING SHALL CONFORM TO "STRUCTURE BACKFILL".

5. OTHER DETAILS (ANTI-SEEP COLLARS, VALVES, ETC.) SHALL BE AS SHOWN ON THE DRAWINGS.

C. PLASTIC PIPE MATERIALS - PVC PIPE SHALL BE PVC-1120 OR PVC-1220 CONFORMING TO ASTM D-1785 OR ASTM D-2241. CORRUGATED HIGH DENSITY POLYETHYLENE (HDPE) PIPE, COUPLINGS AND FITTINGS SHALL CONFORM TO THE FOLLOWING: 4" 10" INCH PIPE SHALL MEET THE REQUIREMENTS OF AASHTO M252 TYPE S, AND 12" THROUGH 24" INCH SHALL MEET THE REQUIREMENTS OF AASHTO M294 TYPE S.

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING p una DEPARTMENT OF PLANNING AND ZONING



C. PLASTIC PIPE (CONTINUED)

JOINTS AND CONNECTIONS TO ANTI-SEEP COLLARS SHALL BE COMPLETELY WATERTIGHT. BEDDING —THE PIPE SHALL BE FIRMLY AND UNIFORMLY BEDDED THROUGHOUT ITS ENTIRE LENGTH. WHERE ROCK OR SOFT, SPONGY OR OTHER UNSTABLE SOIL IS ENCOUNTERED, ALL SUCH MATERIAL SHALL BE REMOVED AND REPLACED WITH SUITABLE EARTH COMPACTED TO PROVIDE ADEQUATE SUPPORT.

4. BACKFILLING SHALL CONFORM TO "STRUCTURE BACKFILL".

SECTION 909.02 (STEEL FOR MISCELLANEOUS USE).

OTHER DETAILS (ANTI-SEEP COLLARS, VALVES, ETC.) SHALL BE AS SHOWN ON THE DRAWINGS. VII. DRAINAGE DIAPHRAGMS WHEN A DRAINAGE DIAPHRAGM IS USED, A REGISTERED PROFESSIONAL ENGINEER WILL SUPERVISE THE DESIGN AND

CONSTRUCTION INSPECTION. VIII. CONCRETE STRUCTURES CONCRETE SHALL MEET THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION, STATE HIGHWAY

ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION 414, MIX NO. 3. REINFORCEMENT SHALL MEET THE MINIMUM REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION 416 (REINFORCEMENT FOR CONCRETE STRUCTURES): SECTION 908 (REINFORCING STEEL - GRADE 60, WIRE ROPE AND WIRE FABRIC AND

ROCK RIPRAP SHALL MEET THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION, STATE HIGHWAY ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION 311. GEOTEXTILE SHALL BE PLACED UNDER ALL RIPRAP AND SHALL MEET THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION state highway administration standard specifications for construction and materials, section 921.09, class c.

X. CARE OF WATER DURING CONSTRUCTION all work on permanent structures shall be carried out in areas - free from water, the contractor shal CONSTRUCT AND MAINTAIN ALL TEMPORARY DIKES, LEVEES, COFFERDAMS, DRAINAGE CHANNELS, AND STREAM DIVERSIONS NECESSARY TO PROTECT THE AREAS TO BE OCCUPIED BY THE PERMANENT WORKS. THE CONTRACTOR SHALL ALSO FURNISH, INSTALL, OPERATE, AND MAINTAIN ALL NECESSARY PUMPING AND OTHER EQUIPMENT REQUIRED FOR REMOVAL OF WATER FROM VARIOUS PARTS OF THE WORK AND FOR MAINTAINING THE EXCAVATIONS, FOUNDATION, AND OTHER PARTS OF TI-WORK FREE FROM WATER AS REQUIRED OR DIRECTED BY THE ENGINEER FOR CONSTRUCTING EACH PART OF THE WORK. AFTER HAVING SERVED THEIR PURPOSE, ALL TEMPORARY PROTECTIVE WORKS SHALL BE REMOVED OR LEVELED AND GRADED TO THE EXTENT REQUIRED TO PREVENT OBSTRUCTION IN ANY DEGREE WHATSOEVER OF THE FLOW OF WATER TO THE SPILLWAY OR outlet works and so as not to interfere in any way with the operation or maintenance of the structure. STREAM DIVERSIONS SHALL BE MAINTAINED UNTIL THE FULL FLOW CAN BE PASSED THROUGH THE PERMANENT WORKS. THE REMOVAL OF WATER FROM THE REQUIRED EXCAVATION AND THE FOUNDATION SHALL BE ACCOMPLISHED IN A MANNER AND TO THE EXTENT THAT WILL MAINTAIN STABILITY OF THE EXCAVATED SLOPES AND BOTTOM REQUIRED EXCAVATIONS AND WILL ALLOW SATISFACTORY PERFORMANCE OF ALL CONSTRUCTION OPERATIONS. DURING THE PLACING AND COMPACTING OF MATERIAL IN REQUIRED EXCAVATIONS, THE WATER LEVEL AT THE LOCATIONS BEING REFILLED SHALL BE MAINTAINED BELOW THE BOTTOM OF THE EXCAVATION AT SUCH LOCATIONS WHICH MAY REQUIRE DRAINING THE WATER SUMPS FROM WHICH THE WATER SHALL BE PUMPED.

XI. STABILIZATION ALL BORROW AREAS SHALL BE GRADED TO PROVIDE PROPER DRAINAGE AND LEFT IN A SIGHTLY CONDITION. ALL EXPOSED SURFACES OF THE EMBANKMENT, SPILLWAY, SPOIL AND BORROW AREAS, AND BERMS SHALL BE STABILIZED BY SEEDING, LIMING, FERTILIZING AND MULCHING IN ACCORDANCE WITH THE NATURAL RESOURCES CONSERVATION SERVICE STANDARDS AND SPECIFICATIONS FOR CRITICAL AREA PLANTING (MD-342) OR AS SHOWN ON THE ACCOMPANYING DRAWINGS.

SPECIFICATIONS -- SOD SHALL BE "K-31" TALL FESCUE OR KENTUCKY BLUEGRASS/RED FESCUE MIXTURE OR APPROVED EQUAL. CLASS OF TURFGRASS SOD SHALL BE MARYLAND OR VIRGINIA STATE CERTIFIED OR APPROVED SOD.

SITE PREPARATION — WHERE SOIL IS ACIDIC OR COMPOSED OF HEAVY CLAYS, GROUND LIMESTONE SHALL BE SPREAD AT THE RATE OF 100 LBS./1000 SQ.FT. IN ALL SOILS 5-10-5 FERTILIZER OR APPROVED EQUAL SHALL BE APPLIED AT THE RATE OF 30 LBS/1000 SQ.FT. FERTILIZER SHALL BE UNIFORMLY APPLIED AND MIXED INTO THE TOP 3" OF SOIL WITH THE REQUIRED LIME. SLOW RELEASE NITROGEN, AT THE RATE OF 3.5 LBS/1000 SQ. FT., SHALL BE APPLIED TO THE PREPARED SOIL IMMEDIATELY PRIOR TO SOD INSTALLATION. THIS MATERIAL SHALL BE APPROXIMATELY ONE—THIRD IMMEDIATELY AVAILABLE AND TWO-THIRDS WATER INSOLUBLE NITROGEN. UREA FORMALDEHYDE (UF) AND ISOBUTYLIDENE (IBDU) MEET THESE STANDARDS.

3. SOD INSTALLATION — THE FIRST ROW OF SOD SHALL BE LAID IN A STRAIGHT LINE WITH SUBSEQUENT ROWS PLACE PARALLEL TO AND TIGHTLY WEDGED AGAINST EACH OTHER. LATERAL JOINTS SHALL BE STAGGERED TO PROMOTE MORE UNIFORM GROWTH AND STRENGTH. INSURE THAT SOD IS NOT STRETCHED OR OVERLAPPED AND THAT ALL JOINTS ARE BUTTED TIGHT IN ORDER TO PREVENT VOIDS WHICH WOULD CAUSE AIR DRYING OF THE ROOTS. ON SLOPING AREAS WHERE EROSION MAY BE A PROBLEM, SOD SHALL BE LAID WITH LONG EDGES PARALLEL TO THE CONTOUR AND WITH STAGGERED JOINTS. SECURE THE SOD BY TAMPING AND PEGGING OR OTHER APPROVED METHODS. AS SODDING IS COMPLETED IN ANY ONE SECTION, THE ENTIRE AREA SHALL BE ROLLED OR TAMPED TO INSURE SOLID CONTACT OF ROOTS WITH THE SOIL SURFACE. SOD SHALL BE WATERED IMMEDIATELY AFTER ROLLING OR TAMPING UNTIL THE UNDERSIDE OF THE NEW SOD PAD AND SOLID SURFACE BELOW THE SOD ARE THOROUGHLY WET. THE OPERATION OF LAYING, TAMPING AND IRRIGATING FOR ANY PIECE OF SOD SHALL BE COMPLETED within eight hours. B. PERMANENT SEEDING

ALL DISTURBED AREAS SHALL BE STABILIZED AS FOLLOWS:

 SEEDBED PREPARATION - LOOSEN UPPER 3 INCHES OF SOIL BY RAKING, DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING. SOIL AMENDMENTS - APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS./1000 SQ. FT.), 600 LBS. PER ACRE 10-10-10 FERTILIZER (14 LBS./1000 SQ. FT.) AND 400 LBS. PER ACRE OF 30-0-0 UREAFORM FERTILIZER (9.2 LBS./100 SQ. FT.). HARROW OR DISC LIME AND FERTILIZER INTO UPPER THREE INCHES OF SOIL. AT TIME OF SEEDING, APPLY 400 LBS PER BS./1000 SQ. FT.) OF 30-0-0 UREAFORM FERTILIZER AND 500 LBS. PER ACRE (11.5 LBS./1000 SQ.FT.) OF

10-10-10 FERTILIZER. SEEDING - FOR THE PERIOD MARCH 1 THROUGH APRIL 30 SEED WITH 40 LBS. PER ACRE KENTUCKY 31 TALL FESCUE AND 15 LBS. SEEDING — FOR THE PERIOD MARCH 1 THROUGH APRIL 30 SEED WITH 40 LBS. PER ACRE KENTUCKY 31 TALL FESCUE AND 15 I PER ACRE INOCULATED CROWNVETCH. FOR THE PERIOD MAY 1 THROUGH JULY 31 SEED WITH 60 LBS. PER ACRE KENTUCKY 31 TALL FESCUE AND 2 LBS. PER ACRE INOCULATED WEEPING LOVEGRASS. FOR THE PERIOD OF AUGUST 1 THROUGH OCTOBER 15 SEED WITH 40 LBS. PER ACRE KENTUCKY 31 TALL FESCUE AND 20 LBS. PER ACRE INOCULATED INTERSTATE SERICA IESPEDEZA. DURING THE PERIOD OF OCTOBER 16 THROUGH FEBRUARY 28, PROTECT SITE BY: OPTION (1) — 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING. OPTION (2) — USE SOD.. OPTION (3) — SEED WITH 60 LBS. PER ACRE KENTUCKY 31 TALL FESCUE AND MULCH WITH 2 TONS PER ACRE WELL ANCHORED STRAW. FOR THE PERIOD OF MAY 1 THROUGH FEBRUARY 28, INOCULATED CROWNVETCH SHALL BE APPLIED DURING THE SUBSEQUENT PERIOD OF MARCH 1 THROUGH APRIL 30 AT THE RATE OF 15 LBS. PER ACRE.

4. MULCHING — APPLY 1.5 TO 2 TONS PER ACRE OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING 218 GALLONS PER ACRE OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPE 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE FOR ANCHORING.

5. MAINTENANCE - INSPECT ALL SEEDED AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND RESEEDING. C. TEMPORARY SEEDING

1. SEEDBED PREPARATION - LOOSEN UPPER 3 INCHES OF SOIL BY DISCING, RAKING OR OTHER ACCEPTABLE MEANS BEFORE

SOIL AMENDMENTS - APPLY 600 LBS. PER ACRE OF 10-10-10 FERTILIZER. WHERE SOIL IS ACIDIC OR COMPOSED OF HEAVY CLAYS, GROUND LIMESTONE SHALL BE APPLIED AT THE RATE OF 2 TONS PER ACRE (92 LBS./1000 SQ.FT.)

SEEDING - FOR PERIODS MARCH 1 THROUGH APRIL 30, AND FROM AUGUST 15 THROUGH NOVEMBER 15, SEED WITH 2.5 BUSHELS PER ACRE ANNUAL RYE. FOR THE PERIOD MAY 1 THROUGH AUGUST 14, SEED WITH 3 LBS. PER ACRES OF WEEPING LOVEGRASS. FOR THE PERIOD NOVEMBER 16 THROUGH FEBRUARY 28, PROTECT SITE BY APPLYING 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING OR USE SOD. MULCHING - SAME AS PERMANENT SEEDING.

EROSION AND SEDIMENT CONTROL CONSTRUCTION OPERATIONS WILL BE CARRIED OUT IN SUCH A MANNER THAT EROSION WILL BE CONTROLLED AND WATER AND AIR POLLUTION MINIMIZED. STATE AND LOCAL LAWS CONCERNING POLLUTION ABATEMENT WILL BE FOLLOWED. CONSTRUCTION PLANS SHALL DETAIL EROSION AND SEDIMENT CONTROL MEASURES.

XI. FENCING FENCING SHALL BE 42" HIGH CHAIN LINK FENCE CONSTRUCTED IN ACCORDANCE WITH THE LATEST MARYLAND STATE HIGHWAY ADMINISTRATION STANDARD DETAILS 615.02 AND 615.03. THE SPECIFICATIONS FOR A 6'-0" FENCE SHALL BE USED, SUBSTITUTING 42" FABRIC AND 6'-8" LINE POSTS. GATE SHALL BE CONSTRUCTED IN ACCORDANCE WITH STATE HIGHWAY ADMINISTRATION STANDARD DETAIL 692.01 WITH 42' FABRIC. FABRIC FOR FENCE AND GATE SHALL CONFORM TO ASSHTO DESIGNATION ML8L74. DARK VINYL COATING IS REQUIRED FOR THE FENCE POSTS AND WIRE FABRIC IN ACCORDANCE WITH THE LANDSCAPE MANUAL ADOPTED BY RESOLUTION 56-90, OCTOBER 1, 1990.

FILTER CLOTH FILTER CLOTH TO BE MIRAFI 140N OR APPROVED EQUAL.

GABIONS GABIONS TO BE PVC COATED. SEE HOWARD COUNTY STANDARD SPECIFICATIONS AND DETAILS.

INSPECTION THE CONTRACTOR SHALL NOTIFY THE ENGINEER AT LEAST 5 WORKING DAYS PRIOR TO STARTING ANY WORK SHOWN ON THESE PLANS SO THAT STORMWATER MANAGEMENT POND MAY BE INSPECTED DURING CONSTRUCTION.

REFERENCES UNLESS OTHERWISE NOTED, ALL MATERIALS AND CONSTRUCTION PRACTICES SHALL CONFORM TO THE FOLLOWING: "STANDARD SPECIFICATIONS AND DETAILS FOR CONSTRUCTION" OF THE HOWARD COUNTY, MARYLAND, DEPARTMENT OF

PUBLIC WORKS, AS AMENDED. "STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS", 1993, OF THE MARYLAND STATE HIGHWAY ADMINISTRATION,

"STANDARD AND SPECIFICATIONS FOR PONDS" OF THE SOIL CONSERVATION SERVICE OF MARYLAND (MD-378), JANUARY 2000

AND AS AMENDED. 4. "REVISED STORMWATER MANAGEMENT POLICY," REVISED SEPTEMBER 11, 1984, HOWARD COUNTY MARYLAND.

OPERATION, MAINTENANCE AND INSPECTION

INSPECTION OF THE POND(S) SHOWN HEREON SHALL BE PERFORMED AT LEAST ANNUALLY, IN ACCORDANCE WITH THE CHECKLIST AND REQUIREMENTS CONTAINED WITHIN USDA, SCS "STANDARDS AND SPECIFICATIONS FOR PONDS" (MD-378). THE POND OWNER(S) AND THE HEIRS, SUCCESSORS. OR ASSIGNS SHALL BE RESPONSIBLE FOR THE SAFETY OF THE POND AND THE CONTINUED OPERATION, SURVEILLANCE, INSPECTION, AND MAINTENANCE THEREOF. THE POND OWNER(S) SHALL PROMPTLY NOTIFY THE SOIL CONSERVATION DISTRICT OF ANY UNUSUAL OBSERVATIONS THAT MAY BE INDICATIONS OF DISTRESS SUCH AS EXCESSIVE SEEPAGE, TURBID SEEPAGE, SLIDING OR SLUMPING.

STRUCTURAL NOTES

A. ALL CONSTRUCTION SHALL CONFORM WITH THE 2000 INTERNATIONAL BUILDING CODE AND ALL SUBSEQUENT SUPPLEMENTS. B. IN ADDITION, ALL CONSTRUCTION SHALL CONFORM WITH THE GOVERNING LOCAL BUILDING CODE.

A. THE MINIMUM DESIGN DEAD LOADING FOR ALL FRAMING IS BASED ON THE CONSTRUCTION MATERIALS SHOWN ON THE DRAWINGS AND INDICATED IN THE SPECIFICATIONS.

3. MISCELLANEOUS A. THE CONTRACTOR SHALL REVIEW CIVIL DRAWINGS PREPARED BY MORRIS & RITCHIE, INC., DATED 7/20/04 FOR LOCATION AND DIMENSION OF CHASES, INSERTS, OPENINGS, SLEEVES, DEPRESSIONS AND OTHER PROJECT REQUIREMENTS WHICH IMPACT THE STRUCTURAL COMPONENTS. B. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS SHOWN ON THE CONTRACT DRAWINGS BEFORE PROCEEDING WITH CONSTRUCTION. ALL

DISCREPANCIES AND OMISSIONS SHALL BE BROLIGHT TO THE ATTENTION OF THE ARCHITECT. THE CONTRACTOR SHALL NOT SUBMIT REPRODUCTIONS OF THE STRUCTURAL CONTRACT DOCUMENTS AS SHOP DRAWINGS. SCALES SHOWN ON THE STRUCTURAL CONTRACT DRAWINGS ARE FOR GENERAL INFORMATION ONLY. DIMENSIONAL INFORMATION SHALL NOT BE OBTAINED BY

4. FOUNDATIONS A. ALL FOUNDATIONS HAVE BEEN DESIGNED FOR AN ASSUMED NET ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF. THE ALLOWABLE SOIL BEARING PRESSURE SHALL BE FIELD VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER AND APPROVED PRIOR TO PLACING FOUNDATIONS. SHOULD THE ACTUAL SOIL BEARING PRESSURE BE LESS THAN 2500 PSF, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER.

B. ALL FILL PLACED UNDER FOUNDATIONS SHALL BE COMPACTED TO A DRY DENSITY OF AT LEAST 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM ALL EXCAVATION AND BACKFILLING OPERATIONS WITHIN THE BUILDING BUILDING FOOTPRINT, INCLUDING ALL COMPACTION TESTS AND INSPECTIONS, SHALL BE DONE UNDER THE DIRECTION AND SUPERVISION OF A REGISTERED GEOTECHNICAL ENGINEER. CONTRACTOR SHALL BACKFILL AROUND PERIMETER OF RISER AT AN EVEN RATE SO AS NOT TO CAUSE AN OVERTURNING MOMENT. D. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ALL FOUNDATION AND SOIL CONDITIONS WHICH DIFFER FROM THOSE ANTICIPATED OR INDICATED IN THE CONTRACT DOCUMENTS. ALL EXISTING SOIL CONTAINING GRAVEL, CONSTRUCTION OR DEMOLITION DEBRIS, ORGANIC SUBSTANCES, OR OTHER FOREIGN OBJECTS SHALL BE REMOVED FROM

THE REGION WITHIN THE FOOTPRINT OF THE STRUCTURE. 5. CAST IN PLACE CONCRETE A. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO THE "SPECIFICATIONS FOR ENVIRONMENTAL ENGINEERING CONCRETE STRUCTURES (ACI 350)"; AND TO THE

"BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318)".

B. IN ADDITION TO THE ABOVE, ALL CONCRETE WORK SHALL CONFORM TO THE FOLLOWING: RECOMMENDED PRACTICE FOR HOT WEATHER CONCRETING (ACI 305).

RECOMMENDED PRACTICE FOR COLD WEATHER CONCRETING (ACI 306).

RECOMMENDED PRACTICE FOR CONCRETE FORMWORK (ACI 347). C. ALL CONCRETE, UNLESS NOTED OTHERWISE, SHALL BE STONE AGGRÉGATE CONCRETE HAVING A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI. ALI CONCRETE EXPOSED TO WEATHER SHALL 6% ± 1%. NO ADMIXTURES HAVE AN AIR ENTRAINMENT OF 6 CONTAINING CALCIUM CHLORIDE SHALL BE PERMITTED. MAXIMUM AGGREGATE SIZE SHALL BE 3/4", WATER/CEMENT RATIO SHALL BE 0.45, MAXIMUM SLUMP SHALL BE 4", 3" FOR SLABS ON GRADE. ALL CONCRETE, EXCEPT FOOTINGS, SHALL CONTAIN A WATER REDUCING ADMÍXTURE. PORTLAND CEMENT SHALL CONFORM TO ASTM C 150 AND NORMAL WEIGHT AGGREGATES SHALL CONFORM TO ASTM C 33 D. ALL REINFORCING BARS SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A 615 GRADE 60. ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A 185. LAP ALL REINFORCING BARS A MINIMUM OF 48 BAR DIAMETERS AND ALL W.W.F. A MINIMUM OF TWO FULL GRIDS, UNLESS OTHERWISE INDICATED. E. ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH THE CRSI "MANUAL OF STANDARD PRACTICE", ACI 315" DETAILS AND DETAILING OF CONCRETE REINFORCEMENT", ACI SP 66 "DETAILING MANUAL". ALL CONCRETE MIX DESIGNS, INCLUDING CEMENT CONTENT, WATER CEMENT RATIO, FINE AND COARSE AGGREGATE CONTENT AND ALL ADMIXTURES, SHALL BE REVIEWED BY ENGINEER PRIOR TO PLACING FIRST CONCRETE. G. ALL CONCRETE SHALL BE SAMPLED AND TESTED BY THE TESTING AGENCY. THE CONTRACTOR SHALL NOTIFY THE TESTING AGENCY 48 HOURS PRIOR TO THE PLACING OF ANY CONCRETE. GROUND BLAST FURNACE SLAG MAY BE USED TO REPLACE UP TO 50 PERCENT OF THE PORTLAND CEMENT IN A CONCRETE MIX, AND FLY ASH OR POZZOLAN MAY BE USED TO REPLACE UP TO 25 PERCENT OF PORTLAND CEMENT, SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER AND SHALL CONFORM TO ASTM C 989. MINIMUM COVER FOR ALL REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE INDICATED: FOUNDATIONS 3 INCHES

2 INCHES THE GENERAL CONTRACTOR SHALL SUBMIT PLANS SHOWING ALL PENETRATIONS THROUGH THE FRAMED CONCRETE SLABS. THE OPENINGS SHALL BE ACCURATELY LOCATED AND DIMENSIONED.

A. RISER WALLS HAVE BEEN DESIGNED WITH BACKFILL MATERIAL HAVING THE FOLLOWING CHARACTERISTICS:

= 130.0 PCF= 0.40

= 2.25IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO INSURE THE BACK FILL MATERIAL MEETS THESE CHARACTERISTICS.

B. RISER HAS BEEN DESIGNED FOR THE FOLLOWING MINIMUM FACTORS OF SAFETY: OVERTURNING SHIDING

ROLLYANCY C. ALL RISER WALLS SHALL BE BRACED AND SHORED AS REQUIRED DURING BACKFILLING. BOTH SUPPORTING ELEMENTS SHALL BE IN PLACE AND DEVELOPING FULL REQUIRED STRENGTH PRIOR TO BACK FILLING OF WALLS SUPPORTED AT TOP AND BOTTOM.

CONSTRUCTION INSPECTION SCHEDULE

DETENTION AND RETENTION STRUCTURES

I. INSPECTIONS SHALL BE CONDUCTED BY THE AS-BUILT CERTIFYING ENGINEER:

. UPON THE COMPLETION OF EXCAVATION TO SUB-FOUNDATION AND WHEN REQUIRED, AND UPON THE INSTALLATION OF STRUCTURAL SUPPORTS OR REINFORCEMENT FOR STRUCTURES. INCLUDING BUT NOT LIMITED TO:

CORE TRENCHES FOR STRUCTURAL EMBANKMENT.

B. INLET OR OUTLET STRUCTURES AND ANTI-SEEP STRUCTURES. WATER-TIGHT CONNECTORS ON PIPES.

D. TRENCHES FOR ENCLOSED STORM DRAIN FACILITIES.

E. UTILITY CROSSINGS OF EMBANKMENT. 2. DURING THE PLACEMENT OF STRUCTURAL FILL AND CONCRETE, AND INSTALLATION OF PIPING AND CATCH BASINS.

3. DURING BACKFILL OF FOUNDATIONS AND TRENCHES.

4. DURING EMBANKMENT CONSTRUCTION.

5. UPON COMPLETION OF FINAL GRADING, INSTALLATION OF FOREBAY, GABION, ETC.,

AND ESTABLISHMENT OF PERMANENT STABILIZATION.

6. UPON COMPLETION OF INSTALLATION FOR TRASH RACKS, POND DRAINS AND VALVE.

MAINTENANCE & REPAIR SCHEDULE

MAINTENANCE AND REPAIR SHALL BE CONDUCTED IN ACCORDANCE WITH ALL FEDERAL, STATE, AND LOCAL REQUIREMENTS. IN GENERAL, THE PRACTICES CONTAINED IN THE "MAINTENANCE AND REPAIR" CHAPTER OF THE "MARYLAND DAM SAFETY MANUAL", DNR-WRA 1988, SHOULD BE FOLLOWED AND CONDUCTED UNDER THE SUPERVISION OF A QUALIFIED PROFESSIONAL ENGINEER FAMILIAR WITH DAM CONSTRUCTION, OPERATION, MAINTENANCE, AND REPAIR.

IN ADDITION, THE MD-378 DAM INSPECTION CHECKLIST MAY BE USED AS A GUIDE FOR MAINTENANCE AND INSPECTION.

AT A MINIMUM, THE STORMWATER MANAGEMENT FACILITY SHALL BE INSPECTED BI-ANNUALLY FOR THE FOLLOWING ITEMS. 1. CONDITION OF EMBANKMENT

2. CONDITION OF VEGETATIVE COVER

3. CONDITION OF FENCES AND MAINTENANCE ACCESS ROAD.

4. CONDITION OF SPILLWAYS AND PIPE OUTLETS. 5. CONDITION OF POND RESERVOIR STORAGE AREA

SPECIFICATIONS FOR CUTOFF CONTRUCTION.

NOTE: OWNER AND CONTRACTOR

THE ENGINEER SHALL BE NOTIFIED 2 WEEKS PRIOR TO THE

AS-BUILT PLAN. THE CONTRACTOR SHALL CONSTRUCT THE

SEDIMENT BASIN IN ACCORDANCE WITH THE APPROVED SWM

COMMENCEMENT OF CONSTRUCTION. THE SWM AND WATER

QUAILITY FACILITIES MUST BE FIELD INSPECTED FOR THE

PLANS.

GEOTECHNICAL NOTES & RECOMMENDATIONS

PER MD-378 SPECIFICATIONS, SOILS USED FOR CUTOFF TRENCH CONSTRUCTION SHALL MEET USCS CLASSIFICATION CL (LOW PLASTICITY CLAY), CH (HIGH PLASTICITY CLAY), SC (CLAYEY SAND), OR GC (CLAYEY GRAVEL).

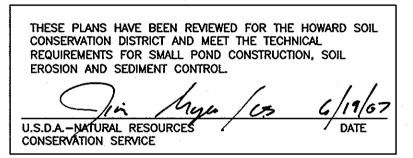
2. MATERIALS USED FOR BACKFILL OF PIPE OUTFALL SHOULD BE SIMILAR TO THOSE USED FOR THE CUTOFF TRENCH. 3. ALTHOUGH SUBSURFACE EXPLORATIONS INDICATE THAT NO SUITABLE CUTOFF TRENCH MATERIAL EXISTS ON SITE TEST PIT EXPLORATION MAY BE PERFORMED IN AN ATTEMPT TO IDENTIFY APPROPRIATE MATERIALS MEETING SCS

4. IF MATERIALS SUITABLE FOR CUTOFF TRENCH CONSTRUCTION ARE NOT IDENTIFIED ON SITE, OFF—SITE BORROW MAY BE UTILIZED PROVIDED THAT IT MEETS ALL CLASSIFICATIONS REQUIRED BY MD-378, AND BE APPROVED BY A QUALIFIED GEOTECHNICAL ENGINEER IN THE FIELD.

MD-378 SPECIFIES THAT REFERENCE SOIL CLASSIFICATIONS SUITABLE FOR CUTOFF TRENCH CONSTRUCTION ARE ALSO SUITABLE FOR EMBANKMENT CONSTRUCTION. THE MOST PLASTIC MATERIAL (PI=11 OR MORE) AVAILABLE ON SITE SHALL BE USED FOR EMBANKMENT CONSTRUCTION.

6. FILL FOR CUTOFF TRECH AND EMBANKMENT CONSTRUCTION SHOULD BE PLACED IN 8" LOOSE LIFTS, COMPACTED TO AT LEAST 95% OF THE MAXIMUM DRY DENSITY IN ACCORDANCE WITH THE STANDARD PROCTOR, ASTM D-698.
FILL AROUND THE PIPE OUTFALL SHOULD BE PLACED IN 4" LIFTS AND COMPACTED TO THE SAME STANDARD WITH HAND EQUIPMENT. COMPACTION OF THESE AREAS SHOULD BE MONITORED BY A QUALIFIED GEOTECHNICAL ENGINEER.

WHERE INDICATED BY SUBSURFACE CONDITIONS, A 6" GRANULAR BEDDING SHOULD BE PLACED UNDER THE PIPE OUTFALL (BEYOND THE CONCRETE CRADLE) TO PROVIDE UNIFORM PIPE SUPPORT.



THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. HOWARD SOIL CONSERVATION DISTRICT



MORRIS & RITCHIE ASSOCIATES, INC. ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

14280 PARK CENTER DRIVE. SUITE A LAUREL, MARYLAND 20707

(410) 792-9792 or (301) 776-1690 FAX (410) 792-7395

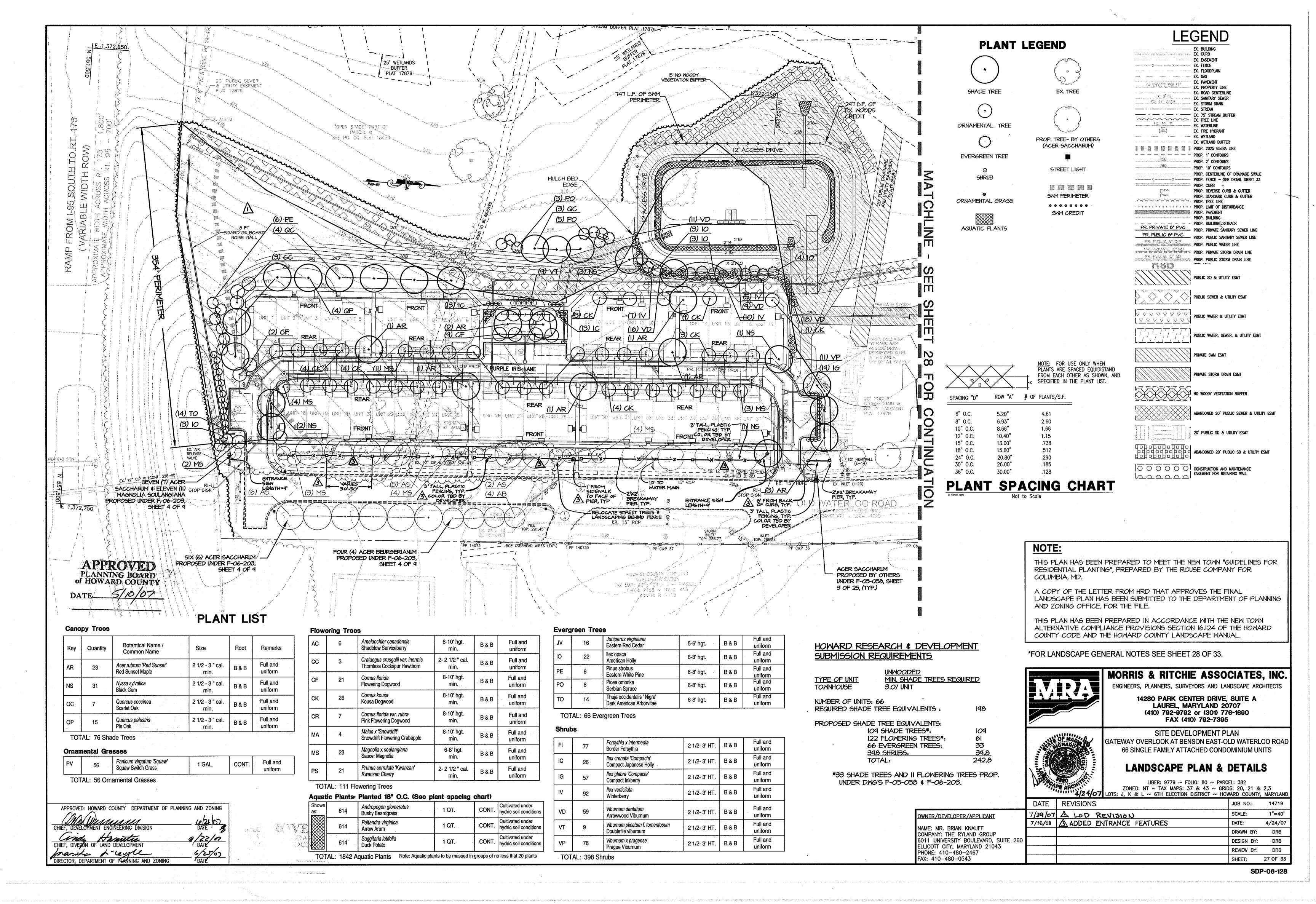


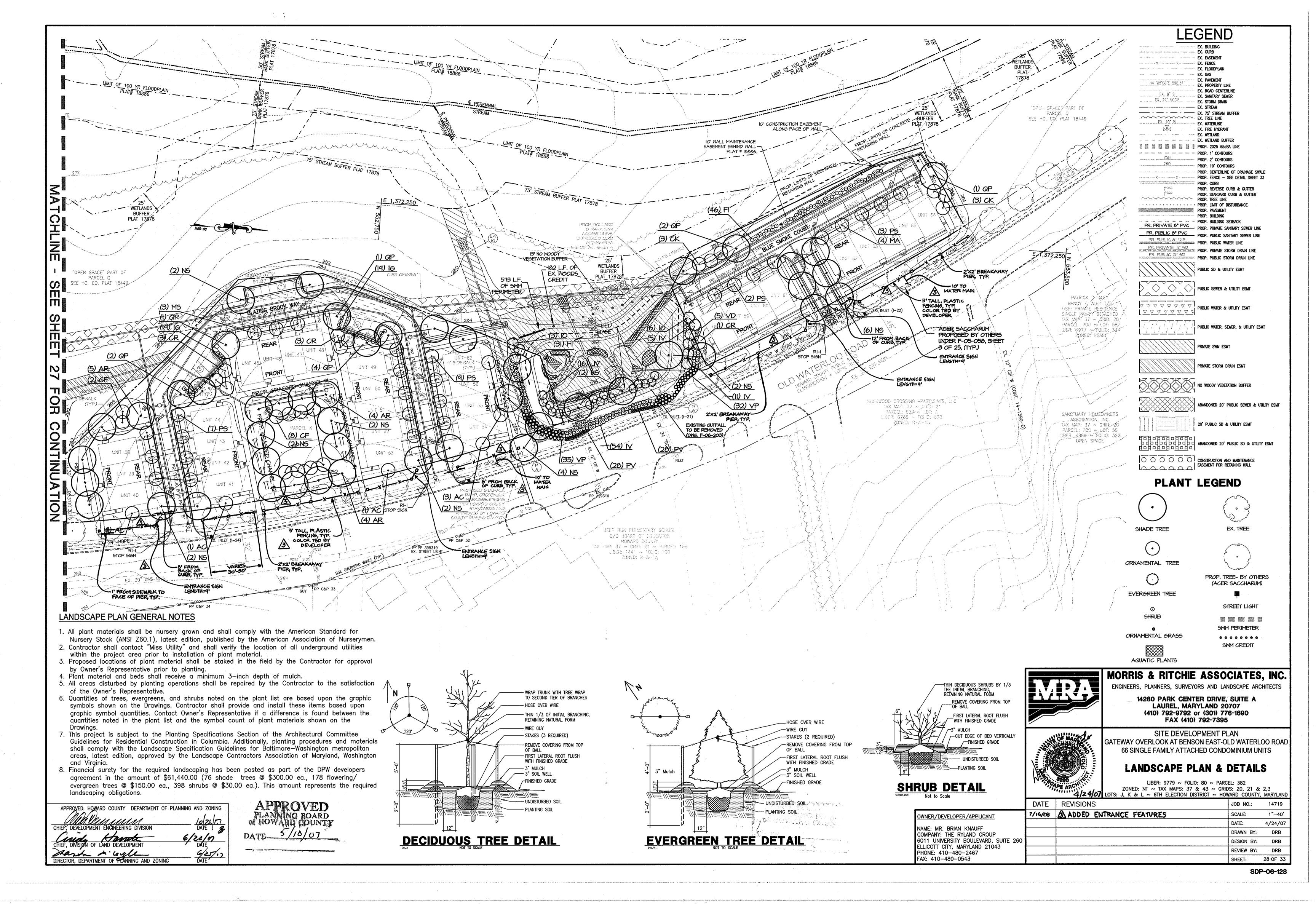
SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

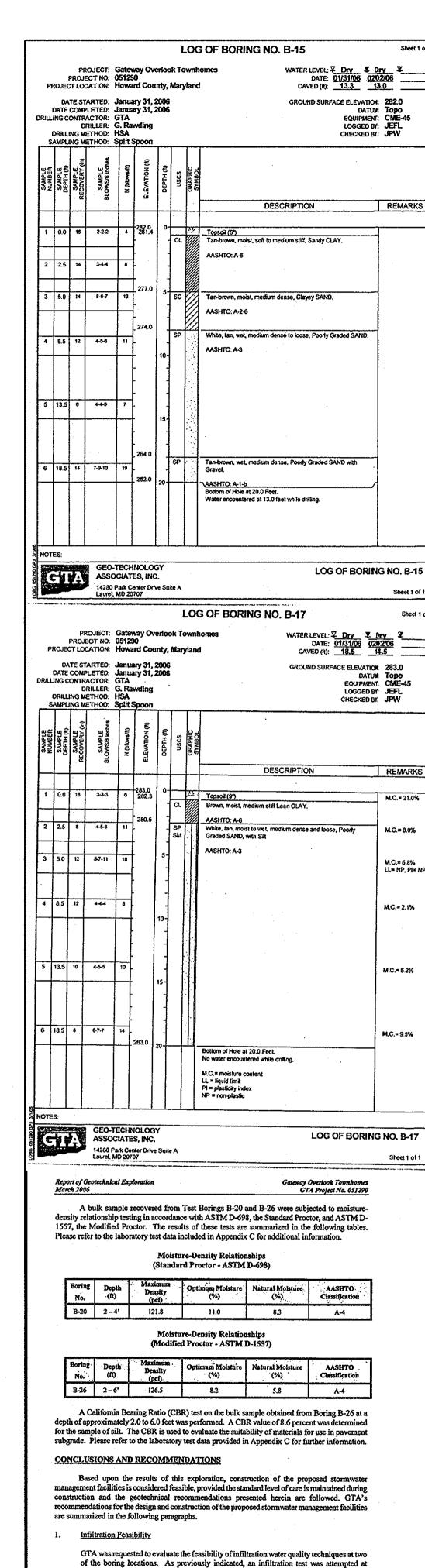
MD-378 POND NOTES

LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382 ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2.3 LOTS: J, K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

		DATE	REVISIONS	JOB NO.:	14719
OWNER/DEVELOPER/APPLICANT NAME: MR. BRIAN KNAUFF COMPANY: THE RYLAND GROUP 6011 UNIVERSITY BOULEVARD, SUITE 260 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-480-2467			SCALE:	AS SHOW	
			DATE:	4/24/07	
			DRAWN BY:	MM	
			DESIGN BY:	ММ	
			REVIEW BY:	KSK	
	FAX: 410-480-0543			SHEET:	26 OF 33







aleway Overlank Townhomes (MDE) recommend a vertical buffer of 2 to 4 feet between the infiltration invert and groundwater or rock. In addition, the minimum acceptable average infiltration rate for Howard County is 0.5 inch per hour. Based on the high groundwater levels observed at Borings I-1 and I-2, infiltration stormwater management techniques are not considered feasible at the depths and locations explored, due to the presence of shallow groundwater and low-permeability clayey soils.

Stormwater Management Facilities A California Bearing Ratio (CBR) test on the bulk sample obtained from Boring B-26 at a The proposed SWM Pond No. 1 is to be located on the southwestern portion of the depth of approximately 2.0 to 6.0 feet was performed. A CBR value of 8.6 percent was determined for the sample of silt. The CBR is used to evaluate the suitability of materials for use in pavement site. The bottom of pond elevation is planned at 272 feet above MSL. Based on existing grades, excavation (cut) of up to 10 feet will be required to achieve pond bottom elevation. The main outfall will include a concrete riser and a reinforced concrete pipe located on the subgrade. Please refer to the laboratory test data provided in Appendix C for further information. south side of the pond, which will outfall into an existing low-lying drainage feature along the southwestern portion of the site.

DATUME TOPO
EQUIPMENT: CME-4

LOGGED BY: JEFL CHECKED BY: JPW

Sheet 1 of

REMARKS

M.C.= 8.0%

M.C.= 6.8% LL= NP, PI= N

M.C.= 2.1%

M.C.= 5.2%

M.C.= 9.5%

Based upon the results of this exploration, construction of the proposed stormwater The proposed SWM Pond No. 2 is to be located on the northern central portion of the management facilities is considered feasible, provided the standard level of care is maintained during site. The bottom of pond elevation is planned at 280 feet above MSL. Excavations (cut) of construction and the geotechnical recommendations presented herein are followed. GTA's up to 2 feet will be required to achieve pond bottom elevation and up to 5 feet of fill will be recommendations for the design and construction of the proposed stormwater management facilities required to achieve proposed top of embankment grades. The main outfall will include a concrete riser and a reinforced concrete pipe located on the west side of the pond, which will outfall into an existing low-lying drainage feature along the central western portion of the

Borings I-1 and I-2. However, groundwater was encountered at depths ranging from 4.3 to A. Material Requirements 6.4 feet below existing grades which corresponds to elevations ranging from 278 to 284 feet above mean sea level. Guidelines adopted by the Maryland Department of Environment GTA understands that Maryland Specification 378 (MD 378) governs design and construction of the pond facilities. MD 378 specifies that soils for use in cutoff trench construction meet USCS Classification CL (low plasticity clay), CH (high plasticity clay), SC (clayey sand), or GC (clayey gravel). Furthermore, GTA recommends that similar materials be used for backfill adjacent to the outfall

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

PROJECT: Gateway Overlook WATER LEVEL: \$ Dity PROJECT NO: 051290
PROJECT LOCATION: Howard County, Marviano GROUND SURFACE ELEVATION: 282.0 DATE STARTED: January 31, 2006 DATUM: Topo
EQUIPMENT: CME-45
LOGGED 8Y: JEFL
CHECKED BY: JPW DATE COMPLETED: January 31, 200 RILLING CONTRACTOR DRILLING METHOD: HSA SAMPLING METHOD: S. AASHTO: A-3 AASHTO: A-1-b Bottom of Hole at 20.0 Feet LOG OF BORING NO. B-16 ASSOCIATES, INC. 14280 Park Center Drive Suite A Lauret, MD 20707 LOG OF BORING NO. B-18 WATER LEVEL: ₹ Dry ₹ Dry ₹ Dry 020206 020206 13.5 PROJECT: Gateway Overlook Townhome PROJECT NO: 051290

LOG OF BORING NO. B-16

PROUND SURFACE ELEVATION: 282.0 DATE COMPLETED: January 31, 2006 LOGGED BY: JEFL CHECKED BY: JPW DRILLING METHOD: AASHTO: A-2-4 AASHTO: A-2-4 Tan, wet, medium dense to loose, Poorly Graded SAND. 6 18.5 12 GEO-TECHNOLOGY ASSOCIATES, INC. LOG OF BORING NO. B-18

14280 Park Center Drive Suite A

structure. The use of the fine-grained plastic material adjacent to the pipe should

decrease the potential for embankment failure induced by "piping" erosion processes.

GTA's exploration identified soils generally classified as USCS CL, MH, CH, and

CL. Based on the boring data, sufficient materials suitable for cutoff trench

construction are likely available from on-site sources. In general, the clayey

materials are typically over their optimum moisture content and may require moisture

conditioning prior to placement as cutoff trench backfill. Materials from other on-

site cut areas should be evaluated during construction to assess if they meet the

PROJECT: Gateway Overlook Townhome
PROJECT NO: 051290 DATE STARTED: February 1, 200 DATE COMPLETED: February 1, 2006 DATUM: Topo
EQUIPMENT: CME-4
LOGGED BY: JEFL
CHECKED BY: JPW DRILLER: G. Rawding DRILLING METHOD: SAMPLING METHOD: Split Spor DESCRIPTION AASHTO: A-2-4 AASHTO: A-2-6 Tan, wet, medium stiff, Elastic SILT. VASHTO: A-7-5 AASHTO: A-6 Bottom of Hole at 20.0 Fee GEO-TECHNOLOGY ASSOCIATES, INC. LOG OF BORING NO. B-22 Sheet 1 of 1

Gateway Overlook Townhomes

requirements for cut-off trench construction. If required, off-site borrow sources

meeting the required classifications should be identified prior to mass grading. All

core trench material should meet the classifications required by MD 378, and be

MD 378 specifies that all of the referenced soil classifications suitable for cutoff

trench construction are also suitable for embankment construction. The USCS ML

and SM soils present are also deemed suitable, and should be available on site. GTA

recommends that the most plastic native materials available be used for embankment

construction. GTA should evaluate embankment soils during the pond construction

Based on the referenced plan, excavations up to 10 feet will be required to establish

the basin elevation, and fills up to approximately 5 feet will be required on the embankments. Excavation of the basins can generally be accomplished by standard

Groundwater was observed in Borings I-1, I-2, and B-21 through B-25 across the site.

of the pond areas during construction. Slopes subject to groundwater seepage may be prone to crosion and sloughing, and should be suitably stabilized.

If groundwater is encountered above proposed basin bottom levels, dewatering of

basins and outfall structure subgrades may be required during the construction phase. Proposed slopes should be designed in accordance with MD 378. Prior to the

placement of compacted fill or the construction of the outfall and structures, areas

supporting the proposed embankments and structures should be stripped and grubbed

to remove all existing trees, surface vegetation, topsoil/rootmat and other organic

matter. GTA anticipates an average stripping thickness on the order of 6 to 12 inches in areas of the SWM facilities. However, stripping thickness in upward of 12 inches

should be anticipated in some areas of the site, particularly in wooded areas, or in

swales. The actual stripping thickness will be dependant on topsoil development, soil

moisture, construction traffic disturbance, and contractor care. After removal,

existing fill meeting the project requirements for structural fill, and free of deleterious

The contractor should be prepared to dewater excavations and maintain trafficability

approved by GTA prior to placement as fill.

B. Basin Excavation and Embankment Construction

scraping techniques.

materials may be reused.

GTA Project No. 051290

LOG OF BORING NO. B-22

LOG OF BORING NO. B-23 PROJECT: Gateway Overlook To PROJECT NO: 051290 DATUM: Topo
EQUIPMENT: CME-45
LOGGED BY: JEFL
CHECKED BY: JPW DATE COMPLETED: February 1, 200 DRILLING METHOD: HSA SAMPLING METHOD: Split Spo REMARKS M.C.= 15.9% M.C.× 13.4% ¥. M.C.= 11.5% AASHTO: A-2-6 M.C.≂ 17.5% ☑ M.C.= 57.7% Bottom of Hole at 20.0 Feet. LOG OF BORING NO. B-23 14280 Park Center Drive Suite A

Gateway Overlook Townhomes GTA Project No. 051290

20' 24" RCP

278

PLAN VIEW - SWM POND #1

SCALE: 1"=30'

LOG OF BORING NO. B-24 DATE STARTED: February 1, 2000 DATE COMPLETED: February 1, 2000 PROUND SURFACE ELEVATION: 278.0 DATUM: Topo
EQUIPMENT: CME-45
LOGGED BY: JEFL
CHECKED BY: JPW DRILLER: G. Rawding DRILLING METHOD: HSA SAMPLING METHOD: Split Spoo DESCRIPTION AASHTO: A-6 AASHTO: A-6 Tan, moist, medium dense, Clayey SAND. AASHTO: A-1-b 6 18.5 7 6-7-8 15 Bottom of Hole at 20.0 Fee LOG OF BORING NO. B-24 ASSOCIATES, INC. 14280 Park Center Drive Suite A LaureL MD 20707

280 24" RCP-PLAN VIEW - SWM POND #2

> Gateway Overlook Townhome GTA Project No. 051290

> > Maryland Specification 378 (MD 378) will likely govern the design and construction of the proposed ponds. MD 378 specifies that soils for use in cutoff trench construction meet USCS Classification CL (low plasticity clay), CH (high plasticity clay), SC (clayey sand), or GC (clayey gravel). Furthermore, GTA recommends that similar materials be used for backfill adjacent to the outfall structure. The use of the fine-grained plastic material adjacent to the pipe should decrease the potential for embankment failure caused by piping.

> > For SWM pond construction, fills for cutoff trench and embankments should be placed in eight-inch loose lifts, and compacted to at least 95 percent of the maximum dry density in accordance with the Standard Proctor, ASTM D-698. Fills around the outfall works should be placed in 4-inch monitored with in-place density testing as performed by a qualified representative under the direction of a professional engineer.

> > GTA's exploration identified layers of CL and SC soils in several areas. Clayey materials suitable for cut-off trench construction are likely present on-site. However, based on the limited excavations planned for this development, it is not likely that sufficient quantities of these materials will be generated. If required, on-site or off-site borrow meeting the required classifications may be used. Borrow materials should meet the classifications required by MD 378, and be approved by GTA prior to placement as fill.

> > MD 378 specifies that all of the referenced soil classifications suitable for cutoff trench construction are also suitable for embankment construction. GTA recommends that the most plastic material available be used for embankment construction. GTA anticipates that the Sandy Silt (ML) soils and Silty and Clayey Sands (SM and SC) materials encountered at the site are likely to be suitable for embankment construction. Sands which are relatively free of Silt and Clay, such as USCS SP soils, are not considered suitable for embankment construction. GTA should be retained to evaluate embankment soils prior to construction.

FOR ADDITIONAL FINDINGS & RECOMMENDATIONS SEE "REPORT OF GEOTECHNICAL EXPLORATION" BY GEOTECHNOLOGY ASSOCIATES, INC. DATED: NOVEMBER 2005 & MARCH 2006.



MORRIS & RITCHIE ASSOCIATES, INC. ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS

14280 PARK CENTER DRIVE, SUITE A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690

FAX (410) 792-7395



SITE DEVELOPMENT PLAN GATEWAY OVERLOOK AT BENSON EAST-OLD WATERLOO ROAD 66 SINGLE FAMILY ATTACHED CONDOMINIUM UNITS

GEO-TECHNICAL REPORT INFORMATION

LIBER: 9779 ~ FOLIO: 80 ~ PARCEL: 382 ZONED: NT ~ TAX MAPS: 37 & 43 ~ GRIDS: 20, 21 & 2,3 OTS: J, K & L ~ 6TH ELECTION DISTRICT ~ HOWARD COUNTY, MARYLAND

JOB NO.: 14719 SCALE: AS SHOWN DATE: 4/24/07 DRAWN BY: GTA DESIGN BY: GTA REVIEW BY: GTA FAX: 410-480-0543

engineer or their qualified representative. Unstable soils identified by proofrolling should be removed from subgrade. Fill material should not be placed or foundations constructed until the geotechnical engineer approves the subgrade. Fills for cutoff trench and embankment construction should be placed in eight-inch loose lifts, and compacted to at least 95 percent of the maximum dry density in accordance with the Standard Proctor, ASTM D-698. Fills around the outfall works

Compactive effort should be monitored with in-place density testing as performed by a qualified representative under the direction of a professional engineer.

GTA understands that Old Waterloo Road will be extended approximately 300 feet in the south direction to access the south side of the proposed site. The proposed roadway will likely be designed and constructed to Howard County standards. Based on the results of the laboratory testing performed at Boring B-26 from 2.0 to 6.0 feet below existing grades, a CBR value of 8.6 percent was determined for the silty soil. GTA recommends that subgrade material with a minimum CBR of 5 be utilized in the upper 12 inches of pavement subgrade. Please refer to our "Report of Geotechnical Exploration, Gateway Overlook Townhomes" dated November 23, 2005 for more detailed information regarding pavement design recommendations.

After stripping, the subgrade should be proof-rolled as directed by a geotechnical

should be placed in 4-inch lifts and compacted to the same standard with hand

equipment. Based on laboratory analysis, some of the on-site soils may be wet of the

optimum moisture for compaction, and moisture conditioning may be required.

Construction during a dry season is recommended to facilitate the drying of soils.

Report of Geotechnical Exploration

This report has been prepared for the exclusive use of The Ryland Group in accordance with generally accepted geotechnical engineering practice. No warranty, express or implied, is made.

The analysis and recommendations contained in this report are based on the data obtained from limited observation and testing of the surface materials. The test borings indicate soil conditions only at specific locations and times, and only to the depths penetrated. They do not necessarily reflect strata variations that may exist between the test boring locations. Consequently, the analysis and recommendations must be considered preliminary until the subsurface conditions can be verified by direct observation at the time of construction. If variations in subsurface conditions from those described are noted during construction, recommendations in this report may

APPROVED PLATINING BOAR OF HOWARD COUNT

OWNER/DEVELOPER/APPLICANT NAME: MR. BRIAN KNAUFF COMPANY: THE RYLAND GROUP 6011 UNIVERSITY BOULEVARD, SUITE 260 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-480-2467

SDP-06-128

29 OF 33

SHEET:

STRUCTURAL NOTES

- 1. BUILDING CODES
 - A. ALL CONSTRUCTION SHALL CONFORM WITH THE 2003 IBC BUILDING CODE AND ALL APPLICABLE SUPPLEMENTS.
 - B. IN ADDITION, ALL CONSTRUCTION SHALL CONFORM WITH THE GOVERNING LOCAL BUILDING CODE.

3. MISCELLANEOUS

- A. SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS MUST BE SUBMITTED BY THE CONTRACTOR OR OWNER FOR REVIEW BY THE ENGINEER. IF THE CONTRACTOR OR OWNER FAILS TO SUBMIT THE SHOP DRAWINGS, THE ENGINEER WILL NOT BE RESPONSIBLE FOR STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT. THE SHOP DRAWINGS SHALL INDICATE ANY DEVIATIONS OR OMISSIONS FROM THE CONTRACT DOCUMENTS. THE GENERAL CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMISSION AND MAKE ALL CORRECTIONS DEEMED NECESSARY.
- THE CONTRACTOR SHALL REVIEW THE CIVIL DRAWINGS PREPARED BY LOIEDERMAN SOLTESZ ASSOCIATES FOR LOCATION AND DIMENSION OF OPENINGS, SLEEVES, ELEVATIONS AND OTHER PROJECT REQUIREMENTS WHICH IMPACT THE STRUCTURAL COMPONENTS.
- THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS SHOWN ON THE CONTRACT DRAWINGS BEFORE PROCEEDING WITH CONSTRUCTION. ALL DISCREPANCIES AND OMISSIONS SHALL BE BROUGHT TO THE ATTENTION
- THE CONTRACTOR SHALL NOT SUBMIT REPRODUCTIONS OF THE STRUCTURAL CONTRACT DOCUMENTS AS SHOP DRAWINGS.
- SCALES SHOWN ON THE STRUCTURAL CONTRACT DRAWINGS ARE FOR GENERAL INFORMATION ONLY. DIMENSIONAL INFORMATION SHALL NOT BE OBTAINED BY SCALING THE DRAWINGS.
- THE CONTRACTOR SHALL LOCATE ALL UTILITIES IN THE AREA OF CONSTRUCTION AND PREVENT DAMAGE TO THEM. SHOULD DAMAGE OCCUR TO ANY UTILITIES, THE CONTRACTOR IS REQUIRED TO REPAIR THE DAMAGE TO THE SATISFACTION OF THE OWNER AT HIS OWN EXPENSE.

8. RETAINING WALLS

- FOOTINGS FOR ALL RETAINING WALLS HAVE BEEN DESIGNED FOR AN ASSUMED NET ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. THE ALLOWABLE SOIL BEARING PRESSURE SHALL BE FIELD YERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER USING THE DYNAMIC CONE PENETROMETER TEST (ASTM STP-399) AND APPROVED PRIOR TO PLACING FOOTINGS. TESTING DOCUMENTATION SHALL BE PROVIDED TO THE HOWARD COUNTY INSPECTOR PRIOR TO PLACING FOOTINGS. SHOULD THE ACTUAL SOIL BEARING PRESSURE BE LESS THAN 2000 PSF, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER.
- RETAINING WALLS HAVE BEEN DESIGNED WITH BACK FILL MATERIAL HAVING THE FOLLOWING CHARACTERISTICS:

SATURATED DENSITY = 125 PCF = 30 DEGREES

= 0.37

IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO INSURE THE BACK FILL MATERIAL MEETS THESE CHARACTERISTICS.

RETAINING WALLS HAVE BEEN DESIGNED FOR THE FOLLOWING MINIMUM FACTORS OF SAFETY:

SLIDING OVERTURNING 2.0

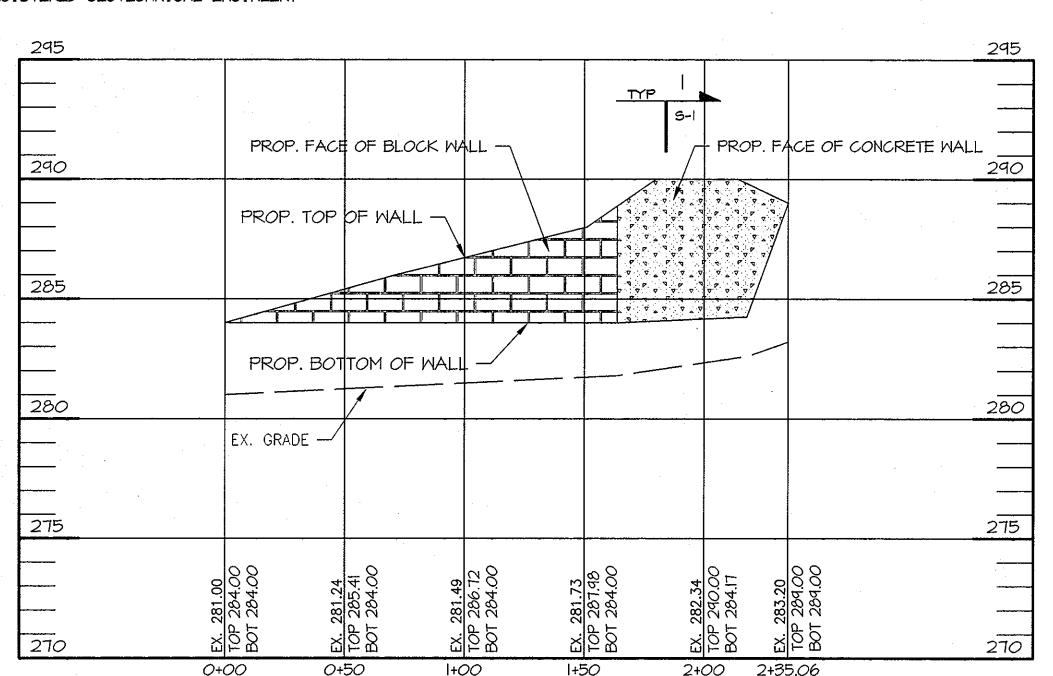
CONSTRUCTION OF ALL RETAINING WALLS SHALL BE PERFORMED UNDER THE SUPERVISION OF A REGISTERED GEOTECHNICAL ENGINEER

9. STRUCTURAL FILL

- NEW FILL MATERIAL AND EXISTING BASE MATERIAL SHALL BE FREE OF ALL REFUSE, DEBRIS, AND ORGANIC MATTER AND SHALL BE APPROVED FOR USE BY A REGISTERED GEOTECHNICAL ENGINEER.
- FILL MATERIAL SHALL BE DEPOSITED IN 8 INCH MAXIMUM LOOSE LIFTS AND COMPACTED TO A DRY DENSITY OF AT LEAST 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D 698. FILL SHALL BE PLACED AND COMPACTED IN 8 INCH LOOSE LIFTS TO DESIRED FINISHED GRADE UNDER THE GUIDANCE AND OBSERVATION OF A PROFESSIONAL GEOTECHNICAL ENGINEER REGISTERED IN THE STATE OF MARYLAND. TESTING REPORT SHALL BE MADE AVAILABLE TO THE HOWARD COUNTY INSPECTOR UPON COMPLETION OF CONSTRUCTION.
- WHEN WORK IS INTERRUPTED BY RAIN, FILL OPERATIONS SHALL NOT RESUME UNTIL FIELD TESTS INDICATE THAT THE MOISTURE CONTENT AND SOIL DENSITY OF THE TOP & INCHES OF FILL IS WITHIN THE LIMITS SPECIFIED.
- ALL FILL MATERIAL SHALL BE PLACED IN SUCH A MANNER THAT THE SURFACE IS SLOPED TO PREVENT THE PONDING OF WATER.

10. CAST IN PLACE CONCRETE

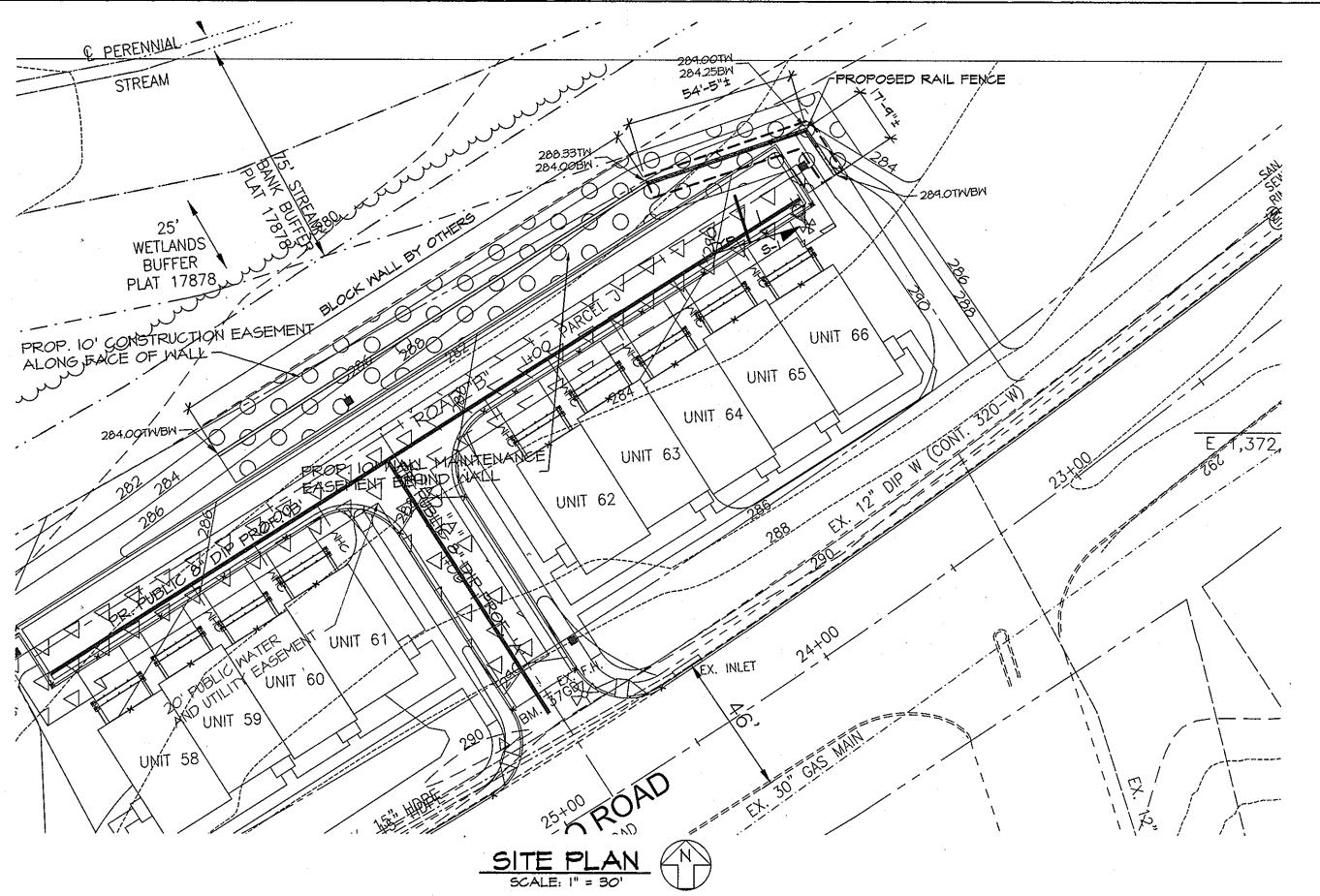
- ALL CONCRETE CONSTRUCTION SHALL CONFORM TO THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301)"; AND TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318)".
- B. ALL CONCRETE, UNLESS NOTED OTHERWISE, SHALL BE STONE AGGREGATE CONCRETE HAVING A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3500 PSI. ALL CONCRETE EXPOSED TO WEATHER SHALL HAVE AN AIR ENTRAINMENT OF 6% +/- 1%. NO ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL BE PERMITTED. MAXIMUM AGGREGATE SIZE SHALL BE 1", AND MAXIMUM SLUMP SHALL BE 5" 4" FOR SLABS ON GRADE. ALL CONCRETE, EXCEPT FOOTINGS, SHALL CONTAIN A WATER REDUCING ADMIXTURE. PORTLAND CEMENT SHALL CONFORM TO ASTM C 150 AND NORMAL WEIGHT AGGREGATES SHALL CONFORM TO ASTM C 33.
- ALL REINFORCING BARS SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A 615 GRADE 60. ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A 185. LAP ALL REINFORCING BARS A MINIMUM OF 48 BAR DIAMETERS AND ALL W.W.F. A MINIMUM OF TWO FULL GRIDS, UNLESS OTHERWISE INDICATED.
- ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH THE CRSI "MANUAL OF STANDARD PRACTICE", ACI 315" DETAILS AND DETAILING OF CONCRETE REINFORCEMENT" ACI SP 66 "DETAILING MANUAL".
- ALL CONCRETE MIX DESIGNS, INCLUDING CEMENT CONTENT, WATER CEMENT RATIO, FINE AND COARSE AGGREGATE CONTENT AND ALL ADMIXTURES, SHALL BE REVIEWED BY ENGINEER PRIOR TO PLACING FIRST CONCRETE.
- F. ALL CONCRETE SHALL BE SAMPLED AND TESTED BY THE TESTING AGENCY. THE CONTRACTOR SHALL NOTIFY THE TESTING AGENCY 48 HOURS PRIOR TO THE PLACING OF ANY CONCRETE.
- GROUND BLAST FURNACE SLAG MAY BE USED TO REPLACE UP TO 50 PERCENT OF THE PORTLAND CEMENT IN A CONCRETE MIX, AND FLY ASH OR POZZOLAN MAY BE USED TO REPLACE UP TO 25 PERCENT OF PORTLAND CEMENT, SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER AND SHALL CONFORM TO ASTM C 989.



WALL PROFILE

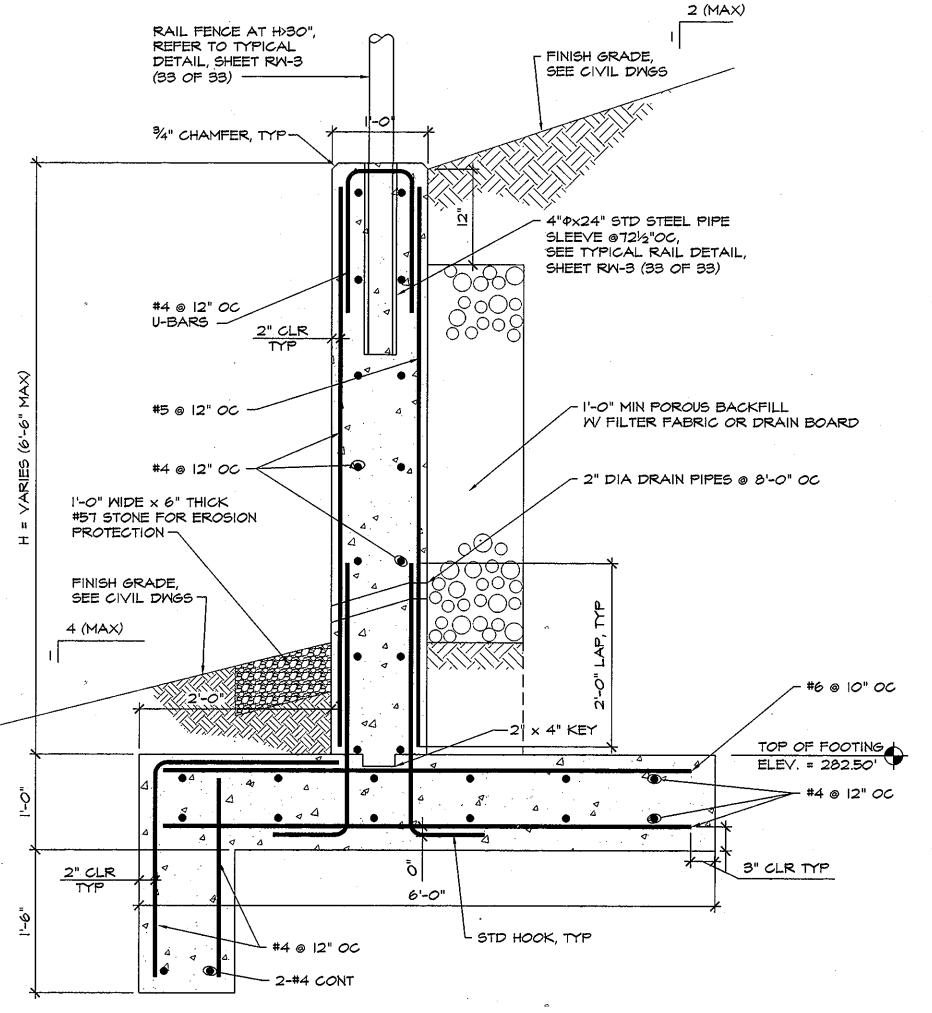
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING 6/23/07 DATE DATE

PLANNING BOARD



SITE PLAN NOTES:

- ALL EASEMENTS AND BUFFERS ARE INDICATED ON CIVIL DRAWINGS PREPARED BY MORRIS AND RITCHIE ASSOCIATES, INC LOCATIONS INDICATED ON THESE PLANS ARE FOR REFERENCE ONLY. REFER TO CIVIL DRAWINGS FOR EXACT LOCATIONS.
- 2. RETAINING WALLS AND SUPPORTS SHALL BE LOCATED OUTSIDE ALL HOWARD COUNTY RIGHT-OF-WAYS AND EASMENTS. WALLS AND SUPPORTS SHALL NOT BE MODIFIED WITHOUT APPROVAL FROM ENGINEER OF RECORD IF OVERLAPS OCCUR.



SCALE: |" = |-0"

S-1

MORRIS & RITCHIE ASSOCIATES, INC. ENGINEERS, PLANNERS, SURVEYORS AND LANDSCAPE ARCHITECTS 1220-C East Joppa Road Suite 505

TOWSON, MARYLAND 21286 (410) 821-1690 FAX (410) 821-1748



Concrete Retaining Wall Strucural Details

GATEWAY OVERLOOK

REVISIONS DATE JOB NO.: 14719 SCALE: AS NOTED DATE: 03/16/07 DRAWN BY: NDB

SECTION 2/5-3 DESIGN BY: NDB REVIEW BY: GSH SHEET: 30 OF 33

SEGMENTAL RETAINING WALL SPECIFICATIONS

PART 1 - GENERAL --

1.1 WORK INCLUDES

FURNISHING AND INSTALLING SEGMENTAL RETAINING WALL UNITS, GEOGRID REINFORCEMENT, WALL FILL, AND BACKFILL TO THE LINES AND GRADES SHOWN ON THE CONSTRUCTION DRAWINGS AND AS SPECIFIED HEREIN. THE CONTRACT ALSO INCLUDES THE FURNISHING AND INSTALLING ALL APPURTENANT MATERIALS, EQUIPMENT, AND LABOR REQUIRED FOR CONSTRUCTION OF THE GEOGRID REINFORCED, SEGMENTAL RETAINING WALL. ALL EXISTING AND PROPOSED CONSTRUCTION AND SITE GRADING INFORMATION WAS REFERENCED FROM THE SITE DEVELOPMENT PLAN, SHEET 7, DATED 9-1-06, REVISED 11-30-06, PREPARED BY MORRIS & RITCHIE, ASSOCIATES, INC.

1.2 REFERENCE STANDARDS

ASTM C90 - HOLLOW LOAD BEARING MASONRY UNITS.

K. GRI-GG5 - DETERMINATION OF GEOGRID (SOIL) PULLOUT

ASTM C140 - SAMPLING AND TESTING CONCRETE MASONRY UNITS. ASTM C145 - SOLID LOAD BEARING CONCRETE MASONRY UNITS.

GEOSYNTHETIC RESEARCH INSTITUTE (GRI), GRI-GG4 - DETERMINATION OF LONG TERM DESIGN STRENGTH OF

ASTM D 638 - TEST METHOD FOR TENSILE PROPERTIES OF PLASTIC.

ASTM D 1248 - SPECIFICATION OF POLYETHYLENE PLASTICS MOLDING AND EXTRUSION MATERIALS.

ASTM D 4218 - TEST METHOD FOR CARBON BLACK CONTENT IN POLYETHYLENE COMPOUNDS BY THE MUFFLE FURNACE TECHNIQUE.

ASTM D 3034 - SPECIFICATION FOR POLYVINYL CHLORIDE (PVC) PIPE.

ASTM C 1372 - SPECIFICATIONS FOR SEGMENTAL RETAINING WALL UNITS GRI-GG4 - DETERMINATION OF LONG-TERM DESIGN STRENGTH OF GEOGRIDS

1.3 DELIVERY, STORAGE AND HANDLING

A. CONTRACTOR SHOULD CHECK THE MATERIALS UPON DELIVERY TO ASSURE THAT PROPER MATERIAL HAS BEEN

CONTRACTOR SHOULD PREVENT EXCESSIVE MUD, WET CEMENT, EPOXY, AND LIKE MATERIALS WHICH MAY AFFIX THEMSELVES, FROM COMING IN CONTACT WITH THE MATERIALS.

GEOGRIDS SHOULD BE STORED ABOVE -20° F. CONTRACTOR SHOULD PROTECT THE MATERIALS FROM DAMAGE. DAMAGED MATERIAL SHOULD NOT BE INCORPORATED INTO THE REINFORCED RETAINING WALL.

1.4 SUBMITTALS/CERTIFICATION

THE CONTRACTOR SHALL SUBMIT A MANUFACTURER'S CERTIFICATION, PRIOR TO THE START OF THE WORK, THAT THE RETAINING WALL SYSTEM COMPONENTS MEET THE REQUIREMENTS OF ASTM C 1372 AND OTHER REQUIREMENTS SPECIFIED HEREIN. THIS CERTIFICATION SHOULD BE PROVIDED TO THE GEOTECHNICAL ENGINEER FOR REVIEW AND APPROVAL PRIOR TO WALL CONSTRUCTION.

PART 2 - PRODUCTS

2.1 DEFINITIONS

GEOGRID IS A HIGH DENSITY POLYETHYLENE, POLYESTER, OR POLYPROPYLENE GRID, SPECIFICALLY FABRICATED FOR USE AS

CONCRETE RETAINING WALL, UNITS ARE AS DETAILED ON THE DRAWINGS AND AS SPECIFIED HEREIN. GEOSYNTHETIC DRAINAGE COMPOSITES ARE POLYETHYLENE NET STRUCTURE WITH NON-WOVEN GEOTEXTILES BONDED TO

EROSION CONTROL BLANKETS CONSIST OF A WEB OF POLYOLEFIN FIBERS SECURELY BOUNDED BY POLYOLEFIN THREADS BETWEEN TWO HIGH STRENGTH POLYOLEFIN NETS.

BACKFILL IS THE SOIL WHICH IS USED AS FILL FOR THE REINFORCED SOIL MASS. FOUNDATION SOIL IS THE IN-SITU SOIL OR CONTROLLED COMPACTED FILL PLACED BELOW THE BOTTOM OF THE RETAINING

DRAINAGE MATERIALS INCLUDE THE STONE PLACED WITHIN AND 1-FOOT BEHIND THE CONCRETE UNITS, THE PERFORATED LONGITUDINAL PVC PIPES, THE LATERAL SOLID PVC PIPES AND THE DRAINAGE GEOCOMPOSITE, IF REQUIRED.

2.2 MATERIALS

THE CONTRACTOR SHOULD SUBMIT MANUFACTURER'S CATALOG AND SAMPLES OF THE PROPOSED MATERIALS FOR APPROVAL BY THE PROJECT GEOTECHNICAL ENGINEER A MINIMUM OF SEVEN DAYS BEFORE THE START OF CONSTRUCTION. MATERIALS SHOULD BE TRANSPORTED TO THE SITE ONLY AFTER APPROVAL OF THE PROPOSED MATERIALS BY THE PROJECT GEOTECHNICAL ENGINEER.

MASONRY UNITS SHOULD BE KEYSTONE STANDARD II RETAINING WALL UNITS. SUBSTITUTION OF OTHER CONCRETE UNITS

MAY BE ALLOWED WITH THE PRIOR APPROVAL OF THE GEOTECHNICAL ENGINEER. CONCRETE WALL UNITS SHOULD HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI, IN ACCORDANCE WITH ASTM C-90. THE CONCRETE SHOULD HAVE ADEQUATE FREEZE/THAW PROTECTION WITH A MAXIMUM MOISTURE ABSORPTION OF 6 PERCENT.

MODULAR CONCRETE MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 1372 - STANDARD SPECIFICATIONS FOR SEGMENTAL RETAINING WALL UNITS.

THE UNITS SHALL PASS 100 FREEZE/THAW CYCLES IN WATER WITH LESS THAN 1% WEIGHT LOSS IN ACCORDANCE WITH

EXTERIOR DIMENSIONS MAY VARY, UNITS ARE REQUIRED TO HAVE A MINIMUM OF 1.0 SQUARE FOOT OF FACE AREA

6. UNITS SHOULD HAVE ANGLED SIDES AND BE CAPABLE OF ATTAINING CONCAVE AND CONVEX ALIGNMENT CURVES IN

ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. UNITS SHOULD BE INTERLOCKED WITH NON-CORROSIVE REINFORCED FIBERGLASS PINS. 8. UNITS SHOULD BE INTERLOCKED AS TO PROVIDE A MAXIMUM OF 24-INCH OF SETBACK PER BLOCK, WHERE REQUIRED.

B. LEVELING PAD MATERIAL FOR LEVELING PAD/FOOTING SHOULD CONSIST OF LEAN CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH OF

2,000 PSI AT 28 DAYS.

C. FIBERGLASS CONNECTING PINS

THERMOSET ISOPTHALIC POLYESTER RESIN PULTRUDED FIBERGLASS REINFORCEMENT RODS, A MINIMUM ONE-HALF INCH IN DIAMFTER.

PINS SHOULD HAVE A MINIMUM FLEXURAL STRENGTH OF 128,000 PSI AND SHORT BEAM SHEAR OF 6,400 PSI. FOR SUBSTITUTE CONCRETE UNITS, USE OF OTHER COMPATIBLE CONNECTOR SYSTEMS MAY BE ALLOWED WITH THE PRIOR

APPROVAL OF THE GEOTECHNICAL ENGINEER. D. GEOGRID GEOGRID SHALL BE MIRAGRID 3XT, OR EQUIVALENT AS APPROVED BY THE GEOTECHNICAL ENGINEER. THE GEOGRID SHOULD

HAVE A LONG-TERM DESIGN STRENGTH OF 3,000 POUNDS PER FOOT. THE LONG-TERM DESIGN STRENGTH IS DEFINED AS THE ULTIMATE STRENGTH DIVIDED BY REDUCTION FACTORS FOR CREEP, DURABILITY, INSTALLATION DAMAGE AND AN OVERALL FACTOR

E. REINFORCED BACKFILL

REINFORCED BACKFILL SOILS SHOULD BE #57 STONE.

ON-SITE OR BORROW SOILS MEETING THE REQUIREMENTS OF AASHTO A-4 OR MORE GRANULAR. ALL FILL MATERIALS

F. CONTROLLED FILL

PROPOSED TO BE PLACED BEHIND THE REINFORCED BACKFILL SHOULD BE PLACED AS CONTROLLED FILL COMPACTED TO 95 PERCENT OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH THE STANDARD PROCTOR, ASTM D-698.

CONTROLLED FILL SOILS TO BE PLACED OUTSIDE THE REINFORCED BACKFILL AREA AND WHERE SPECIFIED SHOULD CONSIST OF

G. LOW-PERMEABILITY SOIL

LOW-PERMEABILITY SOILS TO BE PLACED AT THE TOP OF THE WALL WHERE SPECIFIED SHOULD CONSIST OF SANDY, SILTY OR CLAYEY SOILS MEETING THE REQUIREMENTS OF ML, CL, SM, OR SC WITH A MINIMUM OF 25% PASSING THE #200 SIEVE.

OPEN-GRADED SOILS TO BE PLACED WITHIN THE VOIDS OF THE RETAINING WALL UNITS AND A MINIMUM OF 12 INCHES BEHIND THE UNITS, WHERE SPECIFIED, SHOULD CONSIST OF ASTM #57 STONE AND SHOULD BE WRAPPED IN FILTER FABRIC.

THE DRAINAGE PIPES SHOULD BE PERFORATED OR SLOTTED PVC PIPE MANUFACTURED IN ACCORDANCE WITH ASTM D-3034.

FILTER FABRIC SHOULD BE NON-WOVEN, POLYPROPYLENE GEOTEXTILE, 140N MANUFACTURED BY NICOLON MIRAFI GROUP OR APPROVED EQUIVALENT.

K. EROSION CONTROL BLANKET

EROSION CONTROL BLANKET SHOULD BE TENSAR TB 1000 MANUFACTURED BY THE TENSAR CORPORATION OR APPROVED

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

PART 3 - EXECUTION

3.1. EXCAVATION

THE CONTRACTOR SHOULD EXCAVATE TO THE LINES AND GRADES SHOWN ON THE CONSTRUCTION DRAWINGS. UNDER NO CIRCUMSTANCES SHOULD THE EXCAVATION LINES AND GRADES BE EXCEEDED, EXCEPT WITH OWNER'S APPROVAL. THE CONTRACTOR SHOULD PROTECT THE EXCAVATION FROM SLOUGHING BY PLACING A MEMBRANE OVER THE FACE OF THE

PRIOR TO RETAINING WALL CONSTRUCTION AND THE PLACEMENT OF FILL, ALL TOPSOIL SHOULD BE STRIPPED AND REMOVED

EXCAVATIONS SHOULD BE SLOPED OR OTHERWISE SUPPORTED IN ACCORDANCE WITH OCCUPATION SAFETY AND HEALTH ADMINISTRATION (OSHA) AND OTHER LOCAL AND STATE REGULATIONS.

3.2. FOUNDATION SUBGRADE PREPARATION

MATERIAL DOWN TO THE ORIGINAL SUBGRADE ELEVATION.

FOUNDATION SOIL SHOULD BE EXCAVATED AS REQUIRED FOR INSTALLATION OF LEVELING PAD, GEOGRID AND OTHER ELEMENTS AND AS SHOWN ON THE CONSTRUCTION DRAWINGS. ALLOWABLE BEARING PRESSURE FOR NATURAL AND CONTROLLED, COMPACTED FILL SOILS SHOULD BE AS SPECIFIED IN PART

FOUNDATION SOIL SHOULD BE TESTED BY THE ENGINEER TO ASSURE THAT THE ACTUAL FOUNDATION SOIL STRENGTH MEETS OR EXCEEDS THE ASSUMED DESIGN STRENGTH. SOILS NOT MEETING REQUIRED STRENGTH SHOULD BE REMOVED AND

REPLACED WITH CONTROLLED, COMPACTED MATERIAL THE EXPOSED FOUNDATION SUBGRADE SHOULD BE PROOFROLLED WITH A LOADED DUMP TRUCK, IF POSSIBLE. ANY SOFT OR UNSTABLE AREAS IDENTIFIED DURING PROOFROLLING SHOULD BE OVEREXCAVATED AND BACKFILLED WITH CONTROLLED FILL. OVER-EXCAVATED AREAS SHOULD BE FILLED WITH SELECT AND APPROVED MATERIAL AND COMPACTED TO 95 PERCENT OF

THE MAXIMUM DRY DENSITY IN ACCORDANCE WITH THE STANDARD PROCTOR, ASTM D-698. ANY FILLS REQUIRED TO ESTABLISH SLOPING SURFACES IN FRONT OF THE WALLS SHOULD CONSIST OF CONTROLLED FILL AND SHOULD BE PLACED, COMPACTED AND FIELD TESTED IN ACCORDANCE WITH THE REQUIREMENTS SPECIFIED HEREIN. THE CONTRACTOR MAY ELECT TO EXTEND THE FOUNDATIONS OF THE WALL THROUGH THE NEWLY PLACED STRUCTURAL FILL

3.3. LEVELING PAD

THE LEVELING PAD SHOULD BE PLACED AS SHOWN ON THE CONSTRUCTION DRAWINGS WITH A MINIMUM THICKNESS OF 12

LEVELING PAD MATERIALS SHOULD BE INSTALLED UPON UNDISTURBED IN-SITU SOILS OR CONTROLLED, COMPACTED BACKFILL. LEVELING PAD SHOULD BE PREPARED TO INSURE COMPLETE CONTACT OF RETAINING WALL UNIT WITH BASE. GAPS SHOULD NOT BE ALLOWED.

3.4. UNIT INSTALLATION

FIRST COURSE OF CONCRETE WALL UNITS SHOULD BE PLACED ON THE LEVELING PAD. USE LEAN CONCRETE OR CEMENT/SAND MORTAR TO SET AND LEVEL THE FIRST COURSE. THE UNITS SHOULD BE CHECKED FOR LEVEL AND ALIGNMENT. THE FIRST COURSE IS THE MOST IMPORTANT TO PROVIDE ACCURATE AND ACCEPTABLE RESULTS.

INSURE THAT UNITS ARE IN FULL CONTACT WITH BASE. UNITS ARE PLACED SIDE BY SIDE FOR FULL LENGTH OF WALL ALIGNMENT. ALIGNMENT MAY BE DONE BY MEANS OF A

INSTALL FIBERGLASS CONNECTING PIN. THE RECEIVING SLOT IN THE COURSE BENEATH. REPEAT PROCEDURE TO THE EXTENT OF WALL HEIGHT.

AT THE END OF EACH COURSE WHERE THE WALL CHANGES ELEVATION, UNITS SHOULD BE TURNED INTO THE BACKFILL. UNITS SHOULD BE LAID AS TO CREATE THE MINIMUM RADIUS POSSIBLE. UNLESS OTHERWISE SHOWN ON THE DRAWINGS, A MINIMUM . OF ONE UNIT SHOULD BE INSTALLED INTO THE GRADE. ONLY THE FRONT FACE OF THE UNITS SHOULD BE VISIBLE FROM THE SIDE OF THE WALL

CONVEX AND CONCAVE CURVES SHOULD BE MADE USING COMPAC UNITS OR BY TRIMMING THE STANDARD II UNITS AS REQUIRED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

CAP UNITS SHOULD BE INSTALLED AND BONDED WITH CONSTRUCTION ADHESIVE OR EPOXY CEMENT AS REQUIRED BY CONTRACTOR SHOULD PROVIDE POSITIVE DRAINAGE FOR THE BACK OF THE RETAINING WALL DURING CONSTRUCTION.

3.5. GEOGRID INSTALLATION

ALL UTILITIES IN THE VICINITY OF ANY RETAINING WALL OR GEOGRID REINFORCEMENT MUST BE INSTALLED AND PROPERLY

BACKFILLED PRIOR TO PLACING THE GEOGRID SOIL REINFORCEMENT OR CONSTRUCTING THE WALL. THE GEOGRID SOIL REINFORCEMENT SHOULD BE LAID HORIZONTALLY ON COMPACTED BACKFILL AND CONNECTED TO THE CONCRETE WALL UNITS. HOOK GRID OVER THE FIBERGLASS CONNECTING PIN, PULL TAUT, AND ANCHOR BEFORE BACKFILL IS

SLACK IN THE GEOGRID AT THE WALL UNIT CONNECTIONS SHOULD BE REMOVED IN A MANNER, AND TO SUCH A DEGREE, AS

GEOGRID SHOULD BE LAID AT THE PROPER ELEVATION AND ORIENTATION AS SHOWN ON THE CONSTRUCTION DRAWINGS OR CORRECT ORIENTATION (ROLL DIRECTION) OF THE GEOGRID SHOULD BE VERIFIED BY THE CONTRACTOR.

GEOGRID SHOULD BE SECURED IN-PLACE WITH STAPLES, PINS, SAND BAGS, OR BACKFILL AS REQUIRED BY FILL PROPERTIES, FILL PLACEMENT PROCEDURES, OR WEATHER CONDITIONS, OR AS DIRECTED BY THE ENGINEER.

1. UNIAXIAL GEOGRID DOES NOT NEED TO BE OVERLAPPED IN THE ACROSS THE ROLL DIRECTION, EXCEPT TO CONTAIN THE FILL AT THE SLOPE FACE WHEN WRAP-AROUND FACING IS USED. UNIAXIAL GRID SHOULD BE OVERLAPPED 48" IN THE ROLLED DIRECTION.

2. A LAYER OF SOIL A MINIMUM OF 4 INCHES IN THICKNESS SHOULD BE SPREAD BETWEEN UNIAXIAL GEOGRID LAYERS IN THE AREA TO BE OVERLAPPED, OR AS DIRECTED.

3.6. FILL PLACEMENT

A. WALL BACKFILL MATERIAL SHOULD BE PLACED IN NO MORE THAN 8-INCH LIFTS AND COMPACTED TO 95 PERCENT OF THE STANDARD PROCTOR (ASTM D-698).

BACKFILL SHOULD BE PLACED, SPREAD, AND COMPACTED IN SUCH A MANNER THAT MINIMIZES THE DEVELOPMENT OF WRINKLES IN AND/OR MOVEMENT OF THE GEOGRID.

ONLY HAND-OPERATED COMPACTION EQUIPMENT SHALL BE ALLOWED WITHIN 4 FEET OF THE WALL FACE. BACKFILL SHOULD BE PLACED FROM THE WALL OUTWARD TO INSURE THAT THE GEOGRID REMAINS TAUT. TRACKED CONSTRUCTION EQUIPMENT SHOULD NOT BE OPERATED BEHIND OR ABOVE THE WALL. RUBBER-TIRED EQUIPMENT MAY PASS OVER THE GEOGRID REINFORCEMENT AT SLOW SPEEDS, LESS THAN 10 MPH. SUDDEN

BRAKING AND SHARP TURNING SHOULD BE AVOIDED. PLACE FILTER FABRIC BETWEEN THE UNIT CORE/DRAINAGE FILL AND THE REINFORCED BACKFILL AS SHOWN ON PLANS. THE FILTER FABRIC SHOULD BE EMBEDDED A MINIMUM OF TWO FEET INTO THE REINFORCED FILL.

THE FINISHED SLOPING SURFACE ON THE TOE SIDE OF RETAINING WALLS SHOULD BE PROTECTED BY INSTALLING THE PERMANENT EROSION CONTROL BLANKET AND LOAMING AND SEEDING IN ACCORDANCE WITH PROJECT REQUIREMENTS.

3.7. DRAINAGE

A. DRAINAGE FILL SHOULD BE PLACED BEHIND THE WALL TO THE LIMITS SHOWN. THE DRAINAGE FILL SHOULD BE A MINIMUM OF 12-INCHES THICK AND SHOULD MEET THE REQUIREMENTS OF ASTM #57 STONE. THE DRAINAGE FILL SHOULD BE WRAPPED IN FILTER FABRIC (MIRAFI 140N OR EQUAL) AS SHOWN ON THE DRAWINGS.

POSITIVE DRAINAGE SHOULD BE MAINTAINED DURING AND AFTER CONSTRUCTION. SOILS WITHIN THE REINFORCED ZONE THAT BECOME WET DURING CONSTRUCTION SHOULD BE DRIED TO OPTIMUM MOISTURE OR REMOVED. INSTALL THE PERFORATED DRAINAGE PIPES AND LATERAL DRAINAGE PIPES INCREMENTALLY ALONG WITH THE INSTALLATION OF

CONCRETE UNITS AND PLACEMENT OF FILL.

PART 4 - CONSTRUCTION OBSERVATION AND TESTING

A. RETAINING WALLS SHOULD BE CONSTRUCTED UNDER THE OBSERVATION OF GTA. GTA'S PARTICIPATION WILL FACILITATE COMPLIANCE WITH GTA'S RECOMMENDATIONS AND ALLOW CHANGES TO BE MADE IN THE EVENT THAT SUBSURFACE CONDITIONS ARE FOUND TO VARY FROM THOSE ANTICIPATED DURING THIS DESIGN. GTA WILL NOT BE RESPONSIBLE FOR PROBLEMS RELATED TO THE CONSTRUCTION OF THIS WALL IF WE ARE NOT RETAINED FOR CONSTRUCTION OBSERVATION.

B. THE REQUIRED BEARING PRESSURE BENEATH THE FOOTING OF THE WALL SHOULD BE VERIFIED IN THE FIELD BY A CERTIFIED SOILS TECHNICIAN. TESTING DOCUMENTATION MUST BE PROVIDED TO THE GEOTECHNICAL ENGINEER PRIOR TO THE START OF WALL CONSTRUCTION. THE REQUIRED TEST PROCEDURE SHALL BE THE DYNAMIC CONE PENETROMETER (DCP) TEST.

PART 5 - DESIGN CRITERIA

REQUIRED MINIMUM ALLOWABLE FOUNDATION BEARING PRESSURE IS 2,000 PSF.

C. THE SUITABILITY OF FILL MATERIAL SHOULD BE CONFIRMED BY THE ON-SITE SOILS TECHNICIAN.

B. DESIGN INTERNAL FRICTION ANGLE FOR REINFORCED SOIL = 30 DEGREES. C. DESIGN MOIST UNIT WEIGHT FOR REINFORCED SOIL = 120 PCF.

FOUNDATION AND RETAINED SOIL INTERNAL FRICTION ANGLE = 28 DEGREES AND COHESION = 0 PSF. FOUNDATION AND RETAINED SOIL DESIGN MOIST UNIT WEIGHT = 120 PCF.

RETAINING WALLS ARE NOT DESIGNED TO RESIST HYDROSTATIC PRESSURE. G. THIS RETAINING WALL DESIGN WAS BASED ON THE FOLLOWING MINIMUM FACTORS-OF-SAFETY (F.S.):

F.S. FOR OVERTURNING: 2.0 F.S. FOR PULLOUT:

F.S. FOR BEARING CAPACITY:

1.5 3.0 F.S. FOR GLOBAL STABILITY:

10' CONSTRUCTION EASEMENT 284.25BW 284.10BW 288.33TW 284.00BM -289.0TW/BW 284.00BM BF=91.60 284.00TW/BW BF=91.60 BF=90.94 BF=90.94 BF=88.60 BF=87.94

RETAINING WALL LOCATION PLAN

THE LOCATION PLAN WAS ADOPTED FROM THE SITE DEVELOPMENT PLAN, SHEET 7, DATED 09/01/06, AND LAST REVISED 04/24/07, PREPARED BY MORRIS & RITCHIE ASSOCIATES, INC.



REVISIONS

GEO-TECHNOLOGY ASSOCIATES, INC. GEOTECHNICAL AND ENIVRONMENTAL CONSULTANTS

14280 PARK CENTER DRIVE, SUITE: A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690 FAX: (410) 792-7395 WWW.MRAGTA.COM

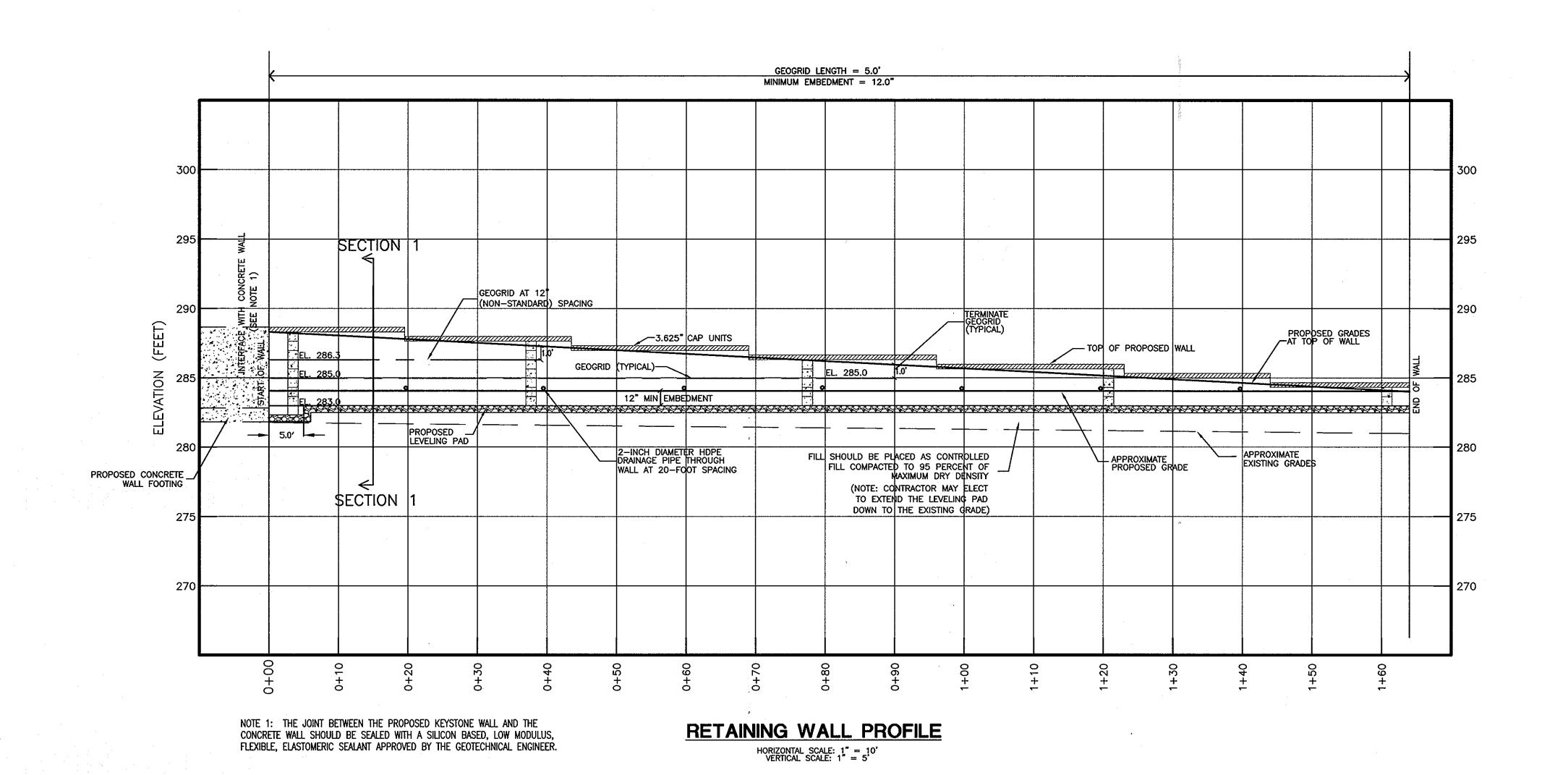
GATEWAY OVERLOOK TOWNHOMES **RETAINING WALL**

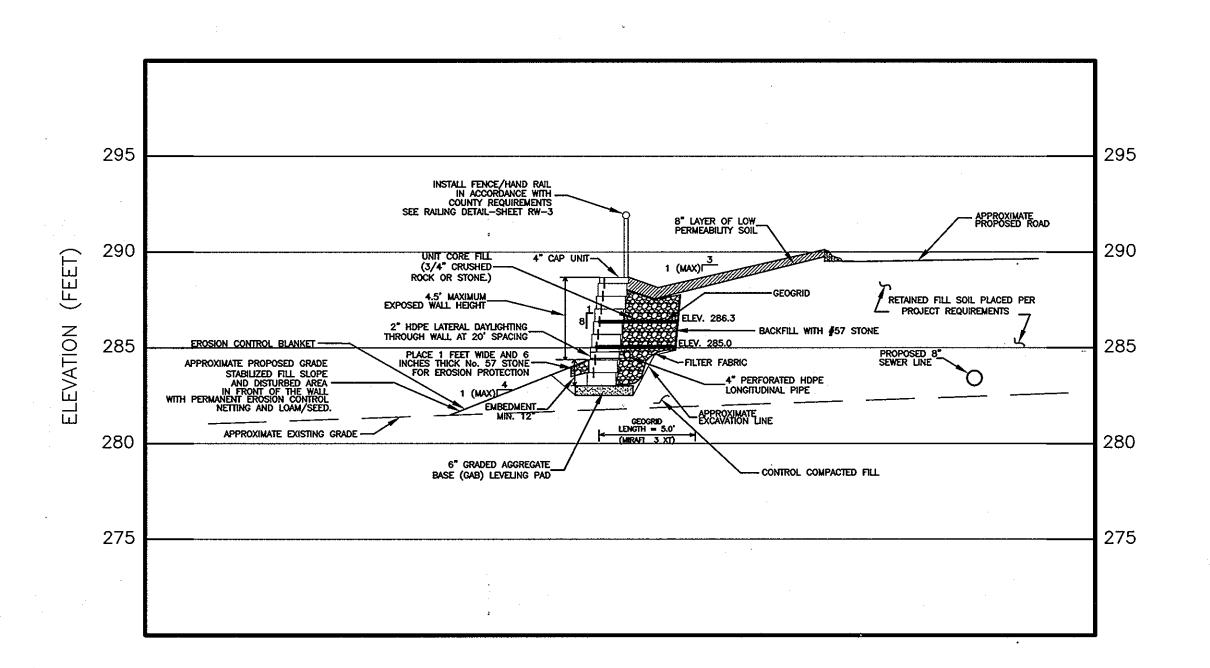
HOWARD COUNTY, MARYLAND

PLAN AND GENERAL NOTES

JOB NO.: 051290

11/30/06 ADDED CALLOUTS/ DETAILS OF PROPOSED DEVELOPMENT SCALE: AS SHOWN DATE: 09/06/06 AND EASEMENTS 04/24/07 ADDED HOWARD COUNTY SIGNATURE BLOCKS & SDP #. DESIGNED BY: GTA DRAWN BY: JPW UPDATED BASE DRAWING BY MRA. REVIEW BY: RPM SHEET: 31 OF 33





RETAINING WALL - SECTION 1 (APPROXIMATE STATION 0+15)

HORIZONTAL SCALE: 1" = 5' VERTICAL SCALE: 1" = 5'

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

CHIEF, DEVELOPMENT ENGINEERING DIVISION

CHIEF, DIVISION OF LAND DEVELOPMENT

DIRECTOR DEPARTMENT OF PLANNING AND ZONING

DATE

APPROVED
PLANNING BOARD
OF HOWARD COUNTY
DATE \$/10/07

RW-2



GEO-TECHNOLOGY ASSOCIATES, INC. GEOTECHNICAL AND ENIVRONMENTAL CONSULTANTS

14280 PARK CENTER DRIVE, SUITE: A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690 FAX: (410) 792-7395

WWW.MRAGTA.COM



GATEWAY OVERLOOK TOWNHOMES
RETAINING WALL

PROFILE AND SECTION

HOWARD COUNTY, MARYLAND

ATE	REVISIONS	JOB NO.: 051290
0/06	UPDATED CROSS-SECTION	SCALE: AS SHOWN
4/07	ADDED HOWARD COUNTY SIGNATURE BLOCKS & SDP #.	DATE: 09/06/06
		DESIGNED BY: GTA
		DRAWN BY: JPW
		REVIEW BY: RPM
		SHEET: 32 OF 33



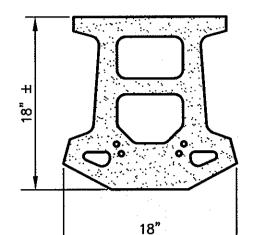
STANDARD II ELEVATION



CAP UNIT ELEVATION

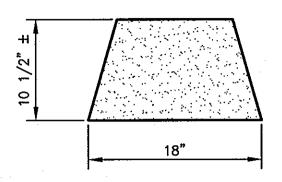


CAP UNIT ELEVATION

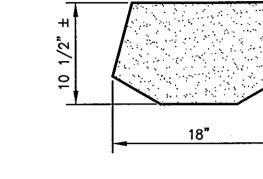


STANDARD II PLAN

STANDARD II UNIT

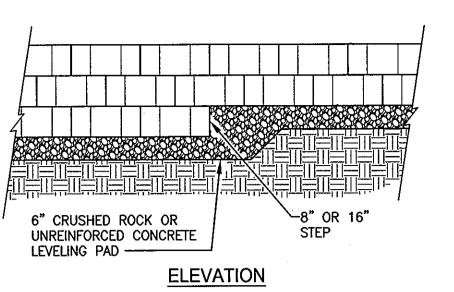


CAP UNIT PLAN



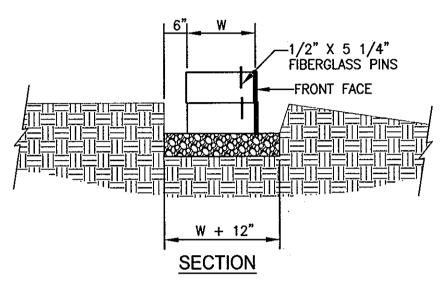
CAP UNIT PLAN

UNIVERSAL 3-PLANE SPLIT
CAP UNIT OPTION
CAP UNIT OPTION



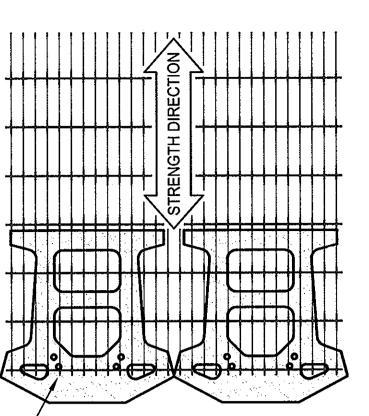
NOTE:

1. THE LEVELING PAD IS TO BE CONSTRUCTED OF CRUSHED STONE OR 2000 PSI ± UNREINFORCED CONCRETE.



LEVELING PAD DETAIL

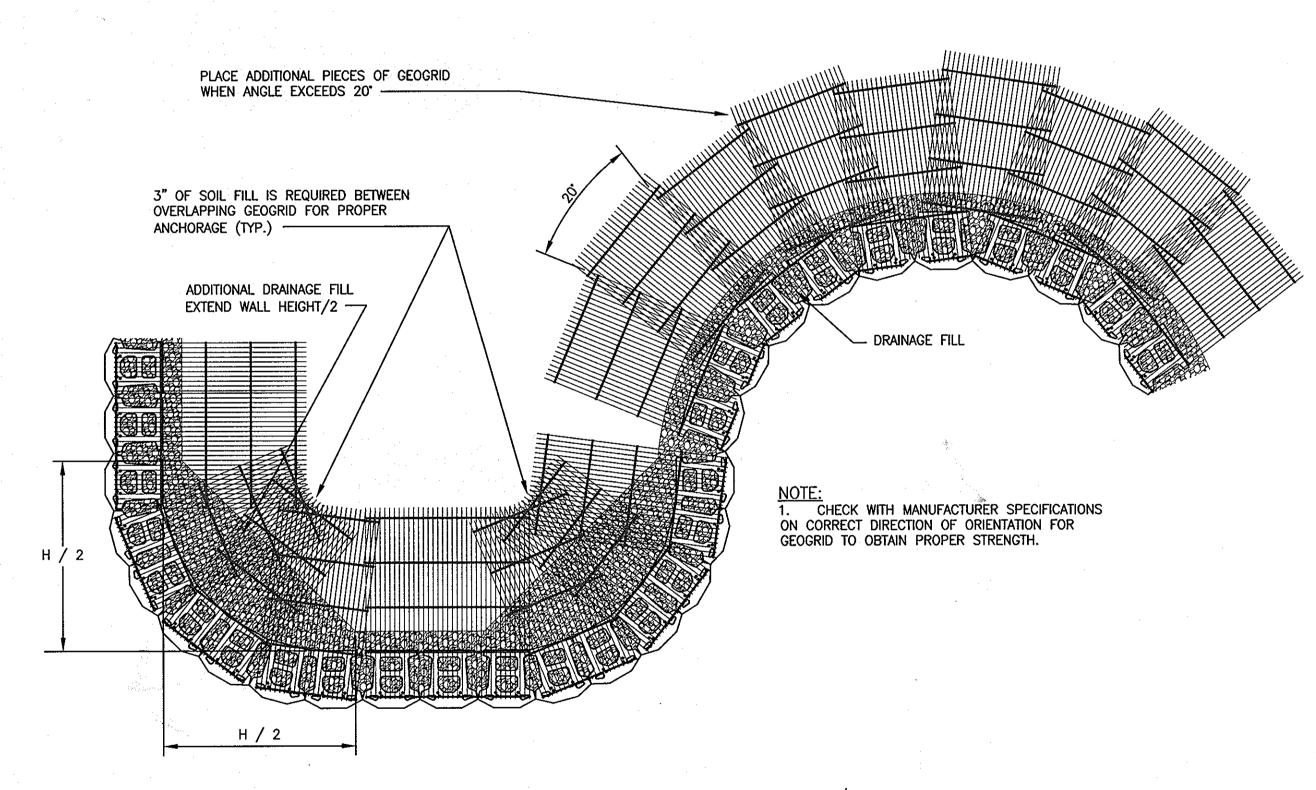
NOT TO SCALE



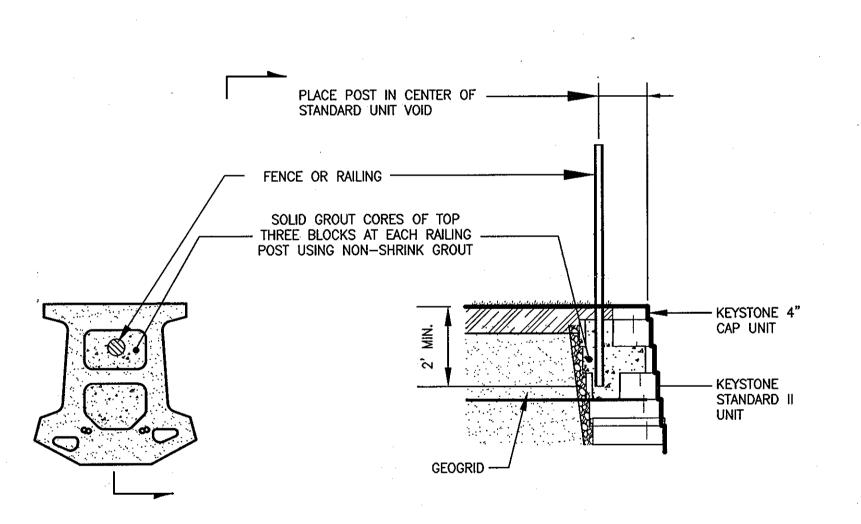
GEOGRID IS TO BE PLACED ON LEVEL BACKFILL AND EXTENDED OVER THE FIBERGLASS PINS. PLACE NEXT UNIT. PULL GRID TAUGHT AND BACKFILL. STAKE AS REQUIRED.

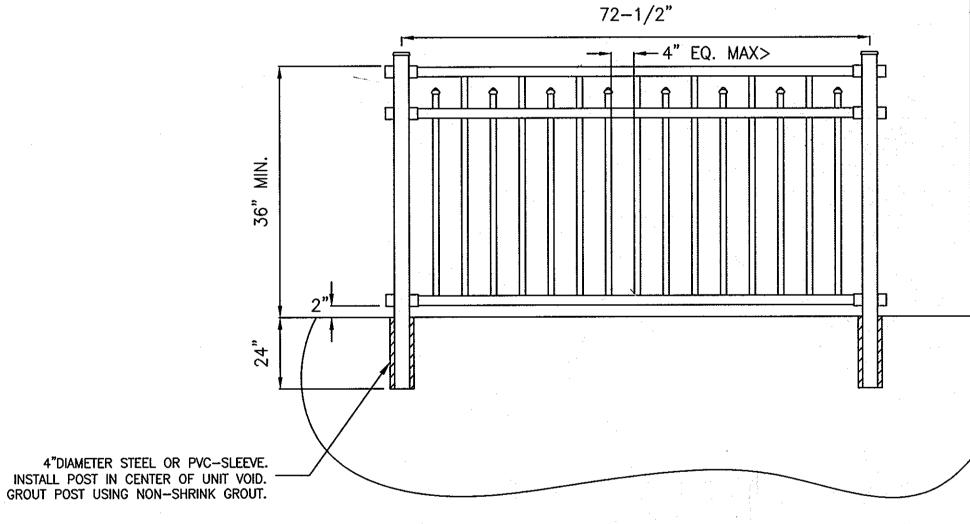
GRID & PIN CONNECTION

NOT TO SCALE



GEOGRID INSTALLATION ON CURVES





BLACK ALUMINUM DECORATIVE FENCE
RESIDENTIAL STRENGTH - #200 (OR EQUAL)
MANUFACUTRED BY JERITH MANUFACTURING COMPANY, INC.
SIMILAR FENCE SUPPLY BY OTHER MANUFACTURER MAY BE
USED WITH PRIOR APPROVAL OF THE DESIGN ENGINEER.

ORNAMENTAL FENCE TYPICAL DETAIL

NOT TO SCALE



GEO-TECHNOLOGY ASSOCIATES, INC. GEOTECHNICAL AND ENIVRONMENTAL CONSULTANTS

14280 PARK CENTER DRIVE, SUITE: A LAUREL, MARYLAND 20707 (410) 792-9792 or (301) 776-1690 FAX: (410) 792-7395

WWW.MRAGTA.COM



GATEWAY OVERLOOK TOWNHOMES
RETAINING WALL

TYPICAL DETAILS

HOWARD COUNTY, MA	RYLAND
	JOB NO.: 051290
	SCALE: AS SHOWN
RE BLOCKS & SDP #.	DATE: 09/06/06
	DESIGN BY: GTA
	DRAWN BY: JPW

AND ZONING

APPROVED

PLANNING BOARD
of HOWARD COUNTY

DATE

S/10/07

DATE

APPROVED

PLANNING BOARD
of HOWARD COUNTY SIGNATURE

DATE

S/10/07

DATE

APPROVED

PLANNING BOARD
OF HOWARD COUNTY SIGNATURE

DATE

S/10/07

REVIEW BY: RPM
SHEET: 33 OF 33