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	SHEET INDEX
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24	SEWER PROFILE
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. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST STANDARDS AND SPECIFICATIONS OF HOWARD COUNTY

3. THE CONTRACTOR SHALL NOTIFY "MISS UTILITY" AT 1-800-257-7777 AT LEAST 48 HOURS PRIOR TO ANY EXCAVATION

6. THE EXISTING TOPOGRAPHY IS TAKEN FROM 1) A FIELD RUN SURVEY, WITH 2 FOOT CONTOUR INTERVALS PREPARED BY

DIVISION. AT (410) 313-1880 AT LEAST FIVE (5) WORKING DAYS PRIOR TO THE START OF WORK.

5. ALL PLAN DIMENSIONS ARE TO FACE OF CURB AND FACE OF BUILDING UNLESS OTHERWISE NOTED.

2. THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS/BUREAU OF ENGINEERING/CONSTRUCTION INSPECTION

4. TRAFFIC CONTROL DEVICES, MARKINGS, AND SIGNING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL

ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). ALL STREET AND REGULATORY SIGNS SHALL BE IN PLACE PRIOR TO THE

CLARK, FINEFROCK & SACKETT, INC. DATED 1/07/07, 2) SDP-01-25 BY LDE, INC. FOR AREAS WITHIN THE ROUTE 1 ROW

7. THE COORDINATES SHOWN HEREON ARE BASED UPON THE HOWARD COUNTY GEODETIC CONTROL WHICH IS BASED UPON THE

10. THE STORMWATER MANAGEMENT QUANTITY AND WATER QUALITY PROPOSED FOR THIS SITE WILL BE ACHIEVED BY STORMTECH

11. APPROXIMATE LOCATION OF EXISTING UTILITIES ARE SHOWN. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS

CONTRACTOR'S OPERATION SHALL BE REPAIRED IMMEDIATELY AT THE CONTRACTOR'S EXPENSE. EXISTING UTILITIES ARE

SHOWN BASED ON THE BEST AVAILABLE INFORMATION. THE CONTRACTOR SHALL TEST PIT EXISTING UTILITIES AT LEAST

(5) DAYS BEFORE STARTING WORK SHOWN ON THESE DRAWINGS. CONTRACTOR IS SOLELY RESPONSIBLE FOR CONSTRUCTION

TO PROTECT THE EXISTING UTILITIES AND MAINTAIN UNINTERRUPTED SERVICES. ANY DAMAGE INCURRED DUE TO

A WETLAND EVALUATION REPORT HAS BEEN PREPARED BY MCCARTHY & ASSOCIATES, INC. DATED NOVEMBER 1998.

A GEOTECHNICAL STUDY HAS BEEN PREPARED BY KONDNER ENGINEERING AND TECHNICAL SERVICES, DATED 3/28/05

20. NO PIPE SHALL BE LAID UNTIL LINES OF EXCAVATION HAVE BEEN BROUGHT WITHIN 6" OF FINISHED GRADE.

19. SEE DEPARTMENT OF PLANNING AND ZONING FILE NOs. F-00-31, F-02-20, SDP-01-25,

22. ALL INLETS SHALL BE CONSTRUCTED IN ACCORDANCE WITH HOWARD COUNTY STANDARDS.

QUALIFIED GEOTECHNICAL ENGINEER BASED ON IN-SITU TESTING OF THE FINISHED SUBGRADE.

A FEE-IN-LIEU PAYMENT OF \$16.879.50 WHICH HAS BEEN PAID TO THE FOREST CONSERVATION FUND.

PROVIDED AS SHOWN ON THIS SITE PLAN. SURETY IN THE AMOUNT OF \$14,970 SHALL BE POSTED.

PROPERTIES, AND BE IN ACCORDANCE WITH SECTION 134 OF THE HOWARD COUNTY REGULATIONS.

WITHIN THE WETLANDS AND STREAMS OR REQUIRED BUFFERS, 100 YEAR FLOOD PLAIN EASEMENT

DEVELOPMENT PLAN, WAIVER PETITION APPLICATION OR BUILDING/GRACING PERMIT APPLICATIONS.

34. MDE PERMIT #200066709 IS ISSUED FOR GRADING AND CONSTRUCTION WITHIN THE 100 YEAR FLOODPLAIN

28. ALL LIGHTING IS TO BE DIRECTED/REFLECTED AWAY FROM ADJACENT PUBLIC ROADS AND RESIDENTIALLY ZONED

29. NO GRADING, REMOVAL OF VEGETATIVE COVER OR TREES, PAVING OR NEW STRUCTURE SHALL BE PERMITTED

27. LANDSCAPING IN ACCORDANCE WITH SECTION 16.124 OF THE LANDSCAPE MANUAL SHALL BE

31. THERE ARE NO CEMETERIES OR HISTORIC STRUCTURES LOCATED ON THE SUBJECT PROPERTY

17. THE BOUNDARY SURVEY FOR THIS PROJECT HAS BEEN PREPARED BY CLARK, FINEFROCK & SACKETT, INC. DATED 4/07/02.

18. SUBJECT PROPERTY ZONED CE-CLI PER 2/02/04 COMPREHENSIVE ZONING PLAN AND THE COMP LITE ZONING REGULATION

21. ALL STORM DRAIN PIPE BEDDING SHALL BE CLASS 'C' AS SHOWN IN FIG. 11.4, VOLUME 1 OF HOWARD COUNTRY DESIGN

MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, AND SAFETY PRECAUTIONS AND PROGRAMS.

IMPROVED BY LANDSCAPING & SIDEWALKS, 3) BUILDINGS OF PARCEL 'B' ARE .OCATED BY a) CFS SURVEY DATED 1/1/07 AND

GENERAL NOTES

PLACEMENT OF ANY ASPHALT.

b) CFS SURVEY DATED 6/09/05

8. WATER IS PUBLIC. CONTRACT NO. 24-4433-D.

FILED UNDER SDP-01-25 AND F-00-31.

AMENDMENTS EFFECTIVE ON 7/28/06

MANUAL UNLESS OTHERWISE NOTED.

15. A NOISE STUDY IS NOT REQUIRED FOR THIS PROJECT

F-98-139, WP-98-109, SP-99-07, & F-07-94.

23. ALL PIPE ELEVATIONS SHOWN ARE INVERT Q ELEVATIONS.

AS PART OF THE DEVELOPER'S AGREEMENT.

AND FOREST CONSERVATION EASEMENTS.

REGISTERED PROFESSIONAL ENGINEER.

30. THIS SITE PLAN IS SUBJECT TO COMPLIANCE WITH THE ROUTE 1 MANUAL

9. SEWER IS PRIVATE ONSITE, CONNECTING TO PUBLIC SEWER SYSTEM OFF-SITE.

PLUS MSHA STANDARDS AND SPECIFICATION IF APPLICABLE.

# SITE DEVELOPMENT PLAN

# STORAGE USA FACILITY (PARCEL 'B') AND PARCEL 'A' OF THE A.H. SMITH SUBDIVISION

6th ELECTION DISTRICT HOWARD COUNTY, MARYLAND

### PROPOSED SHARED UNDERGROUND SWM FACILITY EXPANSIONS BUILDING 'A' PARCEL B BUILDING 'B' BUILDING 'C' PROPOSED ONE-STORY FLEX SPACE BUILDING 'A' MARYLAND STATE PLANE COORDINATE SYSTEM. HOWARD COUNTY MONUMENT NOS. 471B AND 471C WERE USED FOR THIS PROJECT. 12. A 100-YEAR FLOODPLAIN STUDY HAS BEEN PREPARED BY CLARK, FINEFROCK & SACKETT, INC. FILED UNDER SDP-01-25 AND F-00-31 14. A TRAFFIC IMPACT ANALYSIS HAS BEEN PREPARED BY STREET TRAFFIC STUDIES, LTD. DATED OCTOBER, 2006 & APPROVED ON 02/06/07. SCALE: 1"=100 36. ROUTE 1 MANUAL REGULATIONS - PERMITTED USE - THIS PROJECT COMPLIES WITH THE NEW PERMITTED USE FOR FLEX-SPACE - STREETSCAPE DESIGN - THIS PROJECT COMPLIES BY PROVIDING NEW STREET TREES AND A 5' WIDE SIDEWALK ALONG - SITE DESIGN - THIS PROJECT COMPLIES BY PROVIDING A MINIMUM 5% AMENITY AREA OF APPROXIMATELY 13.880 SQUARE

VICINITY MAP SCALE: 1" = 2000'

BENCHMARK 471B

**BENCHMARKS** 

HOWARD COUNTY GEODETIC CONTROL # 471 B ELEVATION 180.71

3/4 INCH REBAR WITH ALUMINUM CAP LOCATED 16.7 FEET NORTH OF AN EXISTING FIRE HYDRANT, 2.4 FEET OFF THE EDGE OF THE U.S. ROUTE 1 PAVING ON THE SOUTHWEST QUADRANT OF THE U.S. ROUTE 1/ MAIER ROAD INTERSECTION.

-HOWARD COUNTY BENCHMARK 471C

HOWARD COUNTY GEODETIC CONTROL # 471 C

ELEVATION 189.05 MONUMENT, FLUSH WITH THE GROUND. LOCATED 84.7 FEET SOUTH

# OF PARKING SPACES REQUIRED 71,100 SF @ 2.5 SP/1,000 SF = 178 SPACES

# OF PARKING SPACES PROVIDED = 178 SPACES (INCLUDING 8 HC)

10.74 Ac.

6,20 Ac.

4.54 Ac.

6.20 Ac.

CONCRETE PLANT)

FLEX SPACE

71,100 SF

(26% OF PARCEL AREA)

CE-CLI

VACANT- (ABANDONED

AMENITY AREA REQUIRED = 5%

(OFFICE, RESEARCH AND DEVELOPMENT,

AMENITY AREA PROVIDED = 13,880 SF (OR 5.14%)

LIGHT MANUFACTURING, ASSEMBLY, STORAGE AND SALES)

SDP-01-25, SDP-99-08, WP-98-109, F-00-31, F-98-139, F-07-94, F-02-20 DPZ FILE REFERENCES

PARCEL 'B'

BUILDING FLOOR SPACE PER USE:

MAXIMUM: NUMBER OF EMPLOYEES:

AREA OF PARCEL

PRESENT USE:

PRESENT ZONING

PROPOSED USE:

SITE DATA

PARCEL 'A'

PARCEL 'B'

TOTAL AREA

PARCEL 'A'

PRESENT USE

AREA OF PARCEL

PRESENT ZONING

PROPOSED USE

BUILDING COVERAGE

4.54 Ac.

SELF STORAGE FACILITY CE-CLI

SELF STORAGE EXPANSION

1.) CURRENT STORAGE USAGE: BUILDING EXPANSION:

a.) BUILDING 'A', 1 STORY, 15,150 s.f. a.) BUILDING 'A', (+400 s.f.)

b.) BUILDING 'B', (+18,600 s.f. b.) BUILDING 'B', 2 STORY, 59,250 s.f.

c.) BUILDING 'C', 1 STORY, 13,990 s.f. c.) BUILDING 'C', (+3540 s.f.)

TOTAL EXISTING STORAGE SPACE = 88,390 s.f. TOTAL ADDITION = 22,540 s.f.

TOTAL EXISTING & PROPOSED STORAGE SPACE = 110.930 s.f. PROPOSED OFFICE USE: 0 s.f.

2.) EXISTING OFFICE USE: a.) BUILDING 'D', 1 STORY, 1,500 s.f.

3.) EXISTING APARTMENT:

PROPOSED APARTMENT: 0 s.f. a.) BUILDING 'D', 2nd STORY, 1,500 s.f.

TOTAL EXISTING & PROPOSED OFFICE & APARTMENT = 3,000 s.f.

TOTAL EXISTING FLOOR AREA: 91,390 s.f.

TOTAL EXISTING & PROPOSED FLOOR AREA: 113,930 s.f.

(OFFICE, APARTMENT & WAREHOUSE)

73,505 SF (37% OF PARCEL AREA) BUILDING COVERAGE (EXISTING & PROPOSED)

4 SPACES/1000 s.f. OF OFFICE AREA = 6 SPACES 2 SPACES/ONSITE RESIDENCE = 2 SPACES

TOTAL PARKING REQUIRED = 8 SPACES

1 VAN ACCESSIBLE HANDICAP SPACE + 4 REGULAR SPACES IN PARKING LOT ADJACENT TO OFFICE. ONE (1) SPACE INSIDE BUILDING 'C'

AND ZONING.

CHIEF, DEVELOPMENT ENGINEERING DIVISION

CHIEF, DIVISION OF LAND DEVELOPMENT

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING

AND TWO (2) PARALLEL SPACES BEHIND BLDG D.

TOTAL PARKING PROVIDED = 8 SPACES (EXISTING)

ment tilevell

PAR 'B' - PROP. NEW SF/ EX. SF = % EXPANSION % BLDG EXPANSION 22,540 sf/91,390 SF = 24.7%

PARKING REQUIRED:

PARKING PROVIDED:

% DRIVEWAY EXPANSION 7,560 SF/51,695 SF = 14.6%

% INFRASTRUCTURE EXPANSION = 0%

% LAND GRADED EXPANSION = 0%

TOTAL PERCENTAGE OF EXPANSION = 0.69 ACRES OR 15%

PERCENTAGE OF SITE BROUGHT INTO COMPLIANCE = 15%

ADDRESS CHART

PAR B

FLEX SPACE BUILDING - SIDE ELEVATION (NTS)

A. H. SMITH SUBDIVISION

C04

LOT NUMBER STREET ADDRESS 9205 WASHINGTON BLVD

DATE NO. 9155 WASHINGTON BLVD OWNER (PAR. A) SECT. /AREA: 144, PAR A & B BLOCK #: | ZONE: | TAX MAP NO.

CENSUS TRACT ELECT. DIST. 6 TH 6069.02 SEWER CODE:

LITTLE PATUXENT REAL ESTATE LLC 26949 MILES RIVER ROAD EASTON, MARYLAND 21601 OWNER (PAR. B)

> ROUTE 1 SELF STORAGE LLC PO BOX 15080 CHEVY CHASE, MARYLAND 20825

ELLICOTT CITY, MARYLAND 21043 TELEPHONE: 410-203-2460 PROJECT STORAGE USA FACILITY (PAR 'B') AND 04 - 210

DOUG CHAMBERLAIN

3138 ROGERS AVENUE

CHAMBERLAIN CONSTRUCTION INC

REVISION

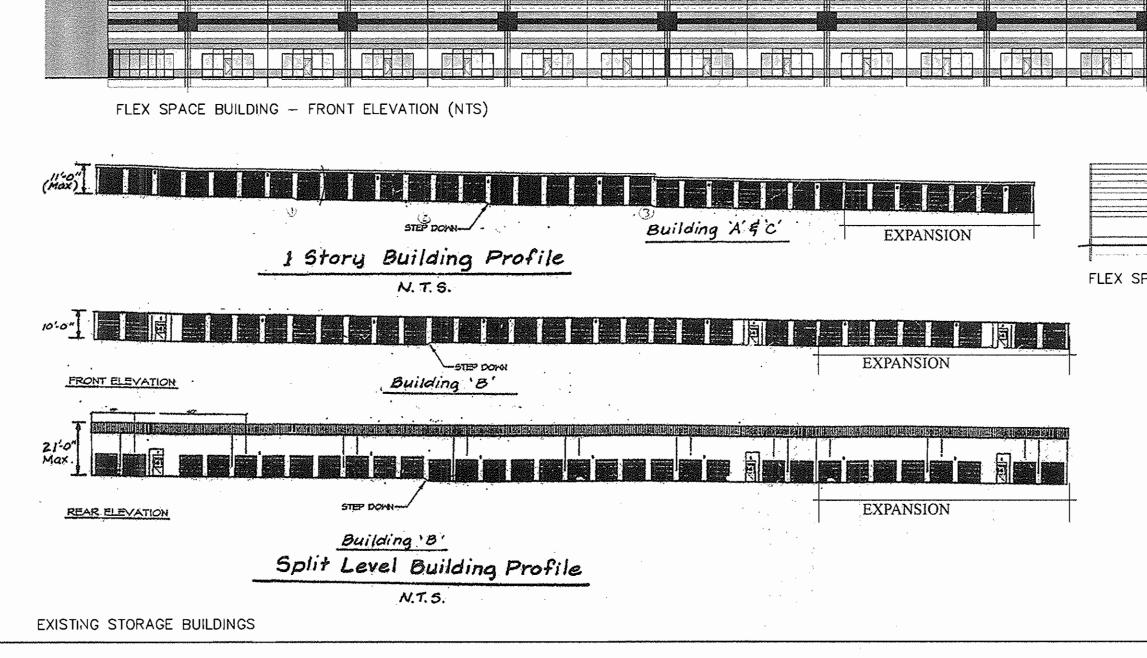
DEVELOPER:

PARCEL 'A' OF THE A.H. SMITH SUBDIVISION SELF-STORAGE FACILITY EXPANSION AND PROPOSED FLEX SPACE BUILDING

6TH ELECTION DISTRICT TAX MAP 47, PARCEL A & B, GRID 18, P/O PARCEL 144

CHECKED BY: \_

SHEET: 1 OF 25



LOCATED AWAY FROM ROUTE 1 AND TO THE SIDE AND REAR OF THE FLEX SPACE BUILDINGS. 24. ALL FILL AREAS WITHIN ROADWAY AND UNDER STRUCTURES TO BE COMPACTED TO A MINIMUM OF 95% COMPACTION OFAASHTO TI80. 25. THE PAVEMENT DETAILS SHOWN FOR THIS SITE REFLECT THE HOWARD COUNTY STANDARD PAVEMENT SECTIONS AND ARE NOT BASED ON SITE SPECIFIC CONDITIONS, PRIOR TO PAVING THE FINAL PAVEMENT SECTIONS SHALL BE DETERMINED BY A 26. THIS PROPERTY ADDRESSED THE REQUIREMENTS OF SECTION 16,1200 OF THE HOWARD COUNTY CODE AND THE FOREST CONSERVATION MANUAL UNDER F-00-31, PLAT NOS. 14746 THRU 48 WITH THE RETENTION OF 2.99 ACRES OF EXISTING ON-SITE FOREST IN EASEMENTS LOCATED ON PARCEL A AND OPEN SPACE LOT 2. UNDER THIS SDP AND F-07-94, AN ADDITIONAL 0.28 ACRES OF TREE CLEARING AND ABANDONMENT IS TO OCCUR WITHIN EXISTING FOREST CONSERVATION EASEMENT AREA WHICH RESULTS IN 2.71 ACRES OF REMAINING RETENTION EASEMENT, A 0.28 ACRE ABANDONMENT OBLIGATION AT \$1.25 PER SQ. FT. AND 0.05 ACRES REFORESTATION OBLIGATION AT \$0.75 PER SQ. FT. TO BE PROVIDED BY 32. THIS PLAN IS SUBJECT TO THE AMENDED FIFTH EDITION OF THE SUBDIVISION AND LAND DEVELOPMENT REGULATIONS PER COUNCIL BILL NO. 45-2003 AND THE ZONING REGULATIONS AS AMENDED BY CB-75-2003. DEVELOPMENT OR CONSTRUCTION ON THIS PROPERTY MUST COMPLY WITH SETBACK AND BUFFER REGULATIONS IN EFFECT AT THE TIME OF SUBMISSION OF THE SITE 33. THE PROPOSED CONSTRUCTION OF THE RETAINING WALL SHALL BE PERFORMED UNDER THE OBSERVATION OF A MARYLAND 35. A KNOX BOX IS TO BE PLACED NO FURTHER THAN 6 FEET AWAY FROM THE MAIN ENTRANCE AND WIRED TO FIRE ALARM PANEL

FEET. THE PROPOSED PARKING AND LOADING ZONES ARE

SUBDIVISION NAME: PLAT #: 19692 14746, 14747, 14748 14987, 14988, F-07-94 WATER CODE:

CONTACT: SANJAY PATEL PHONE: 301-306-3091 x121

18

AB CONSULTANTS, INC. 9450 ANNAPOLIS ROAD LANHAM, MARYLAND 20706 PHONE: (301) 306-3091 FAX: (301) 306-3092

4250000

TITLE SHEET

HOWARD COUNTY, MARYLAND

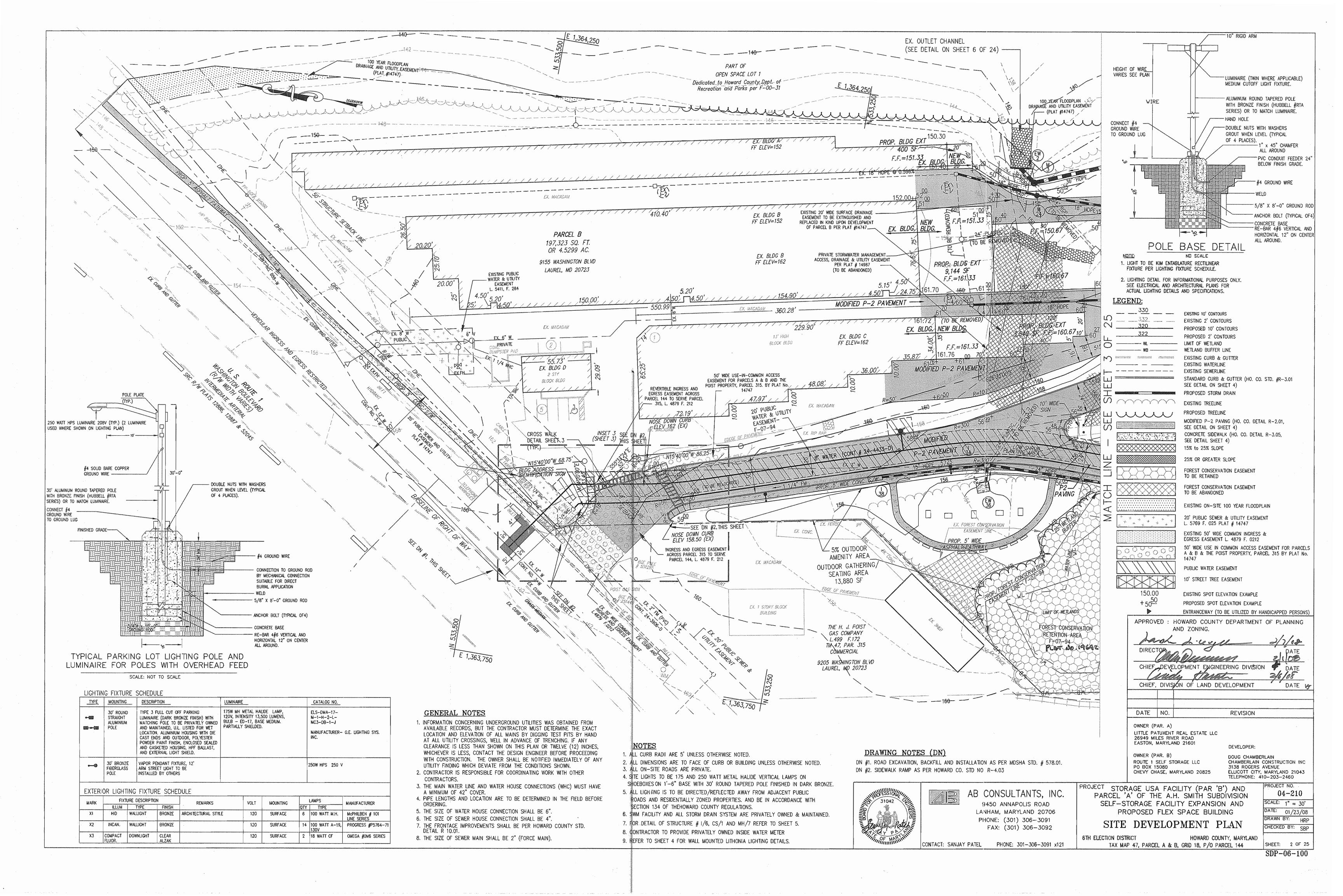
SDP-06-100

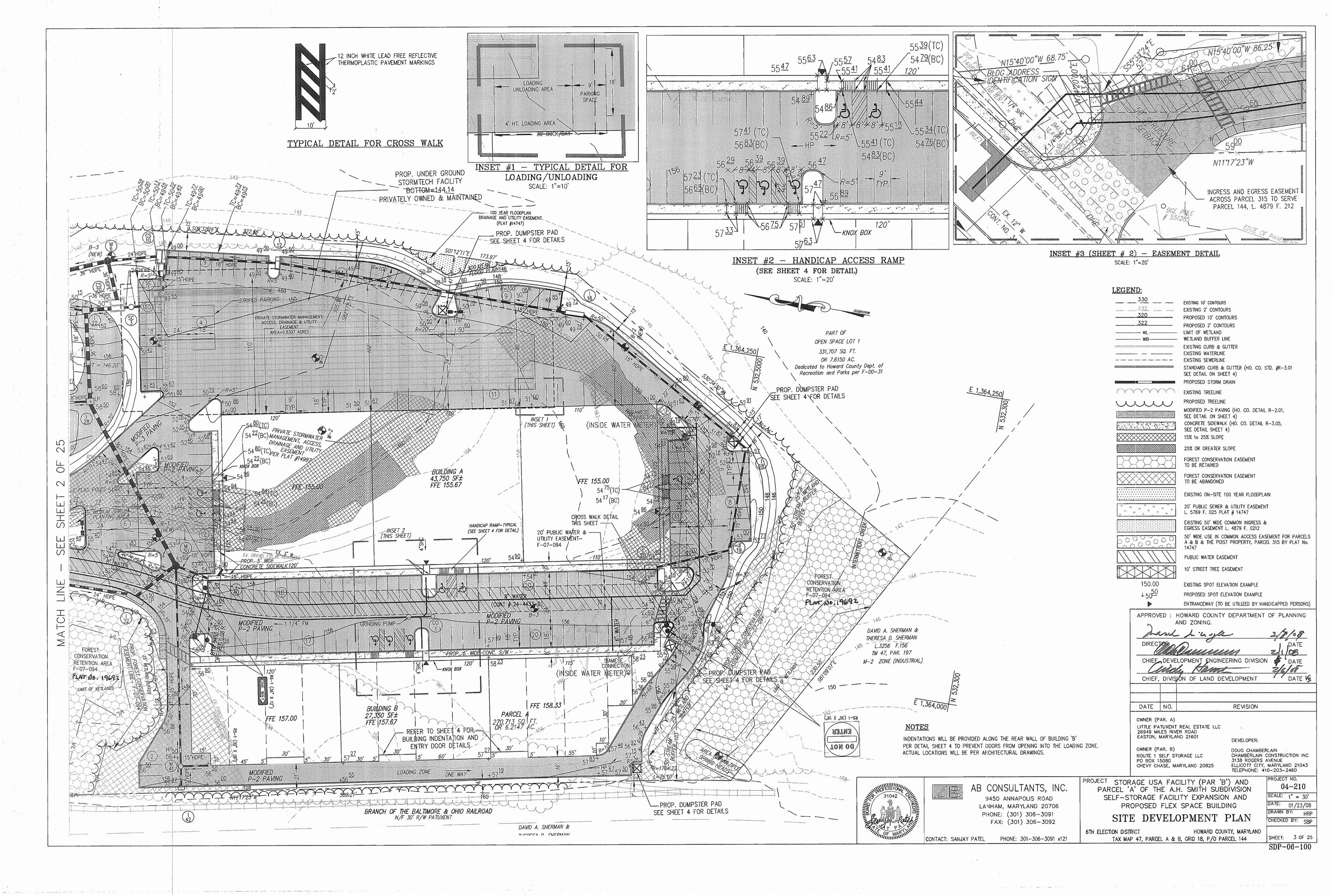
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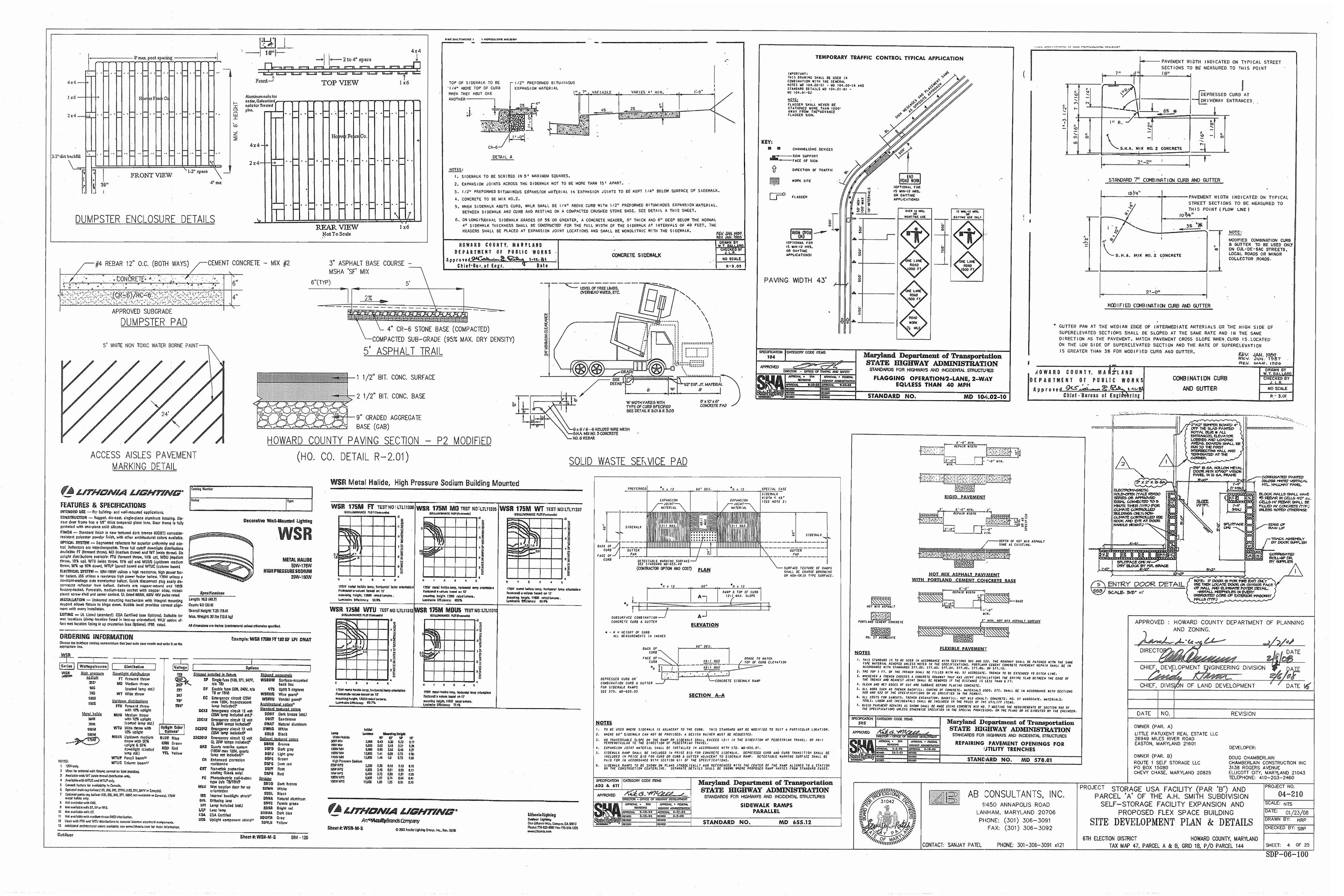
DATE: 01/23/08

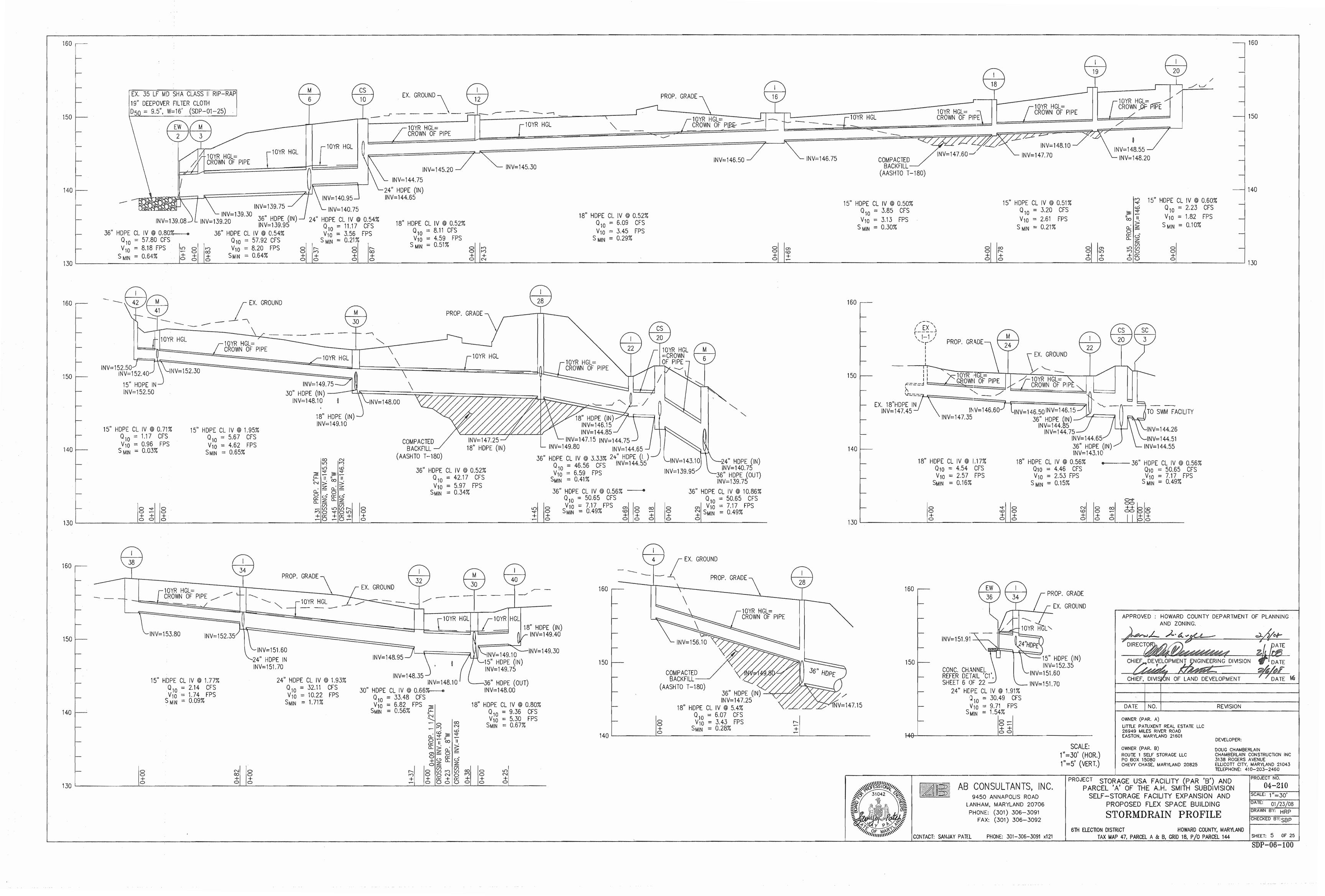
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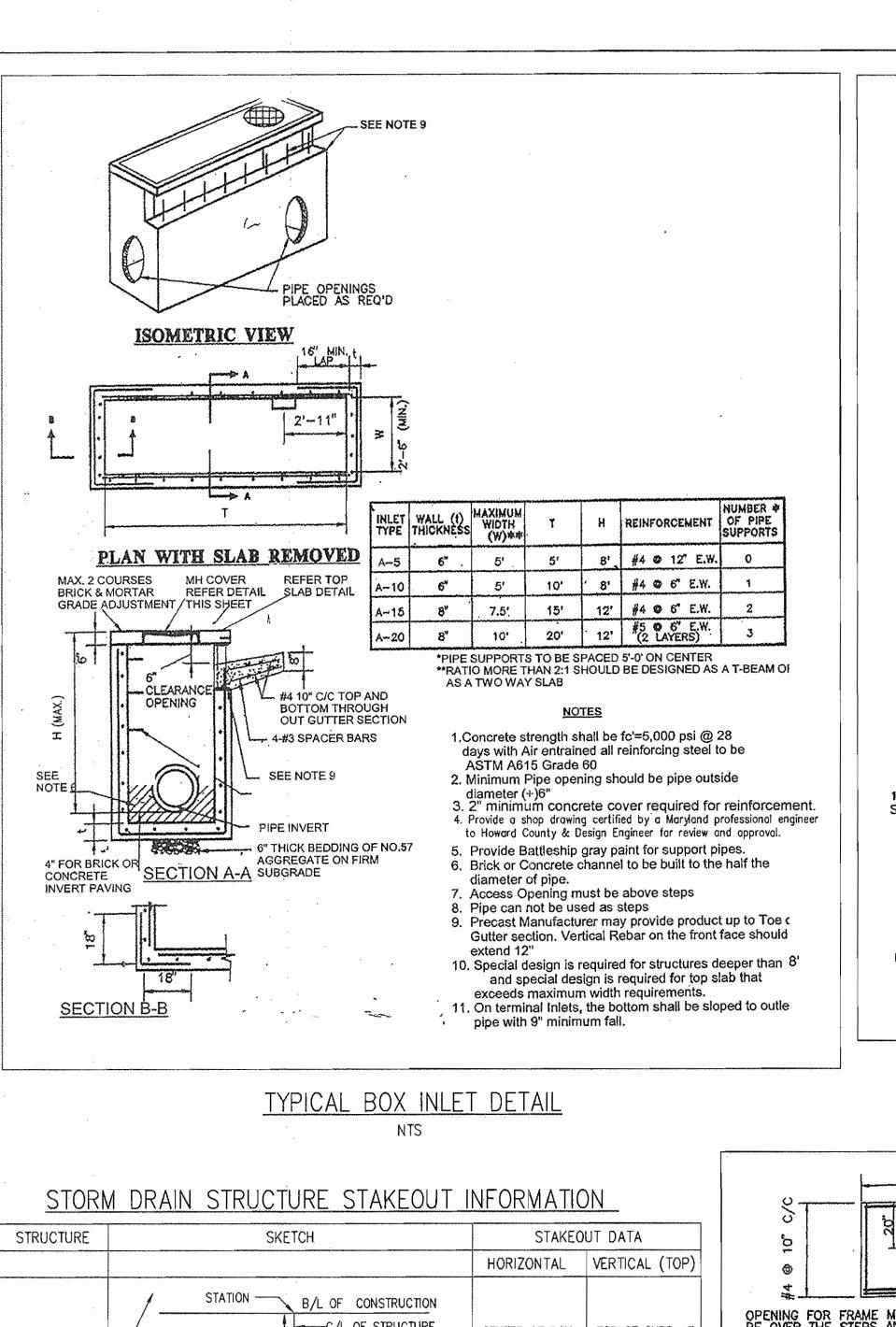
DATE 🌠

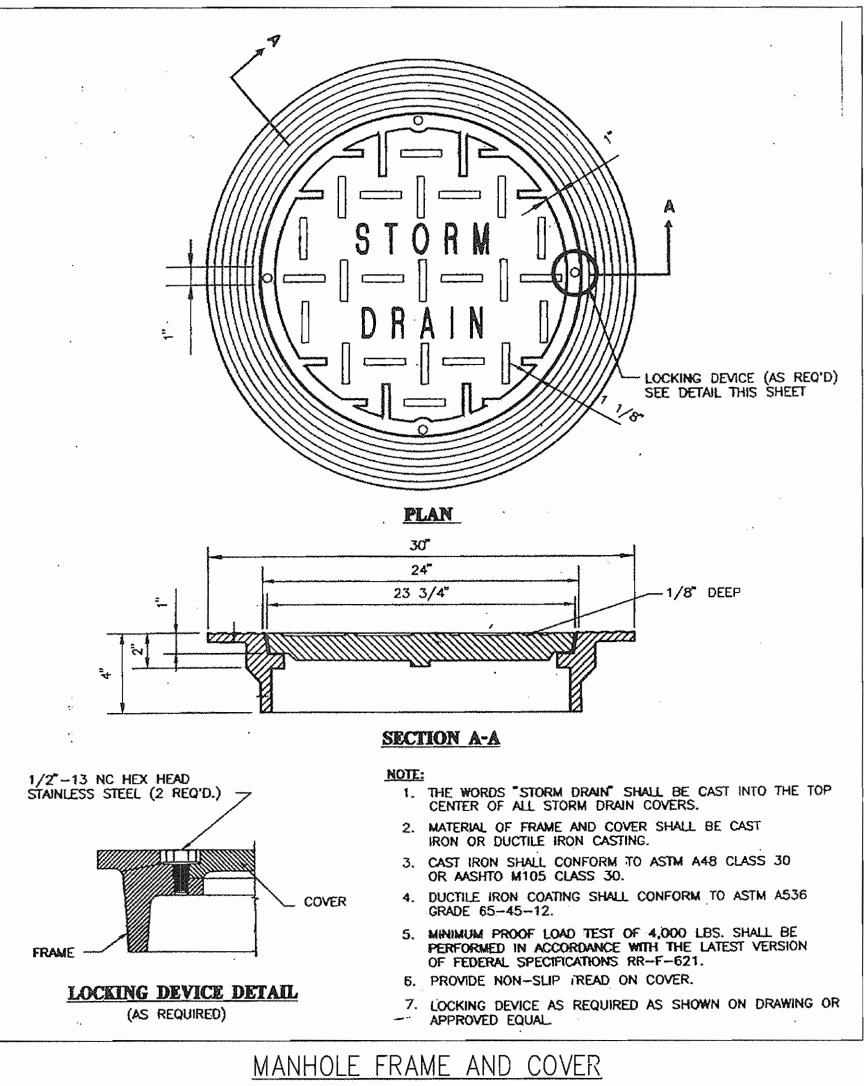








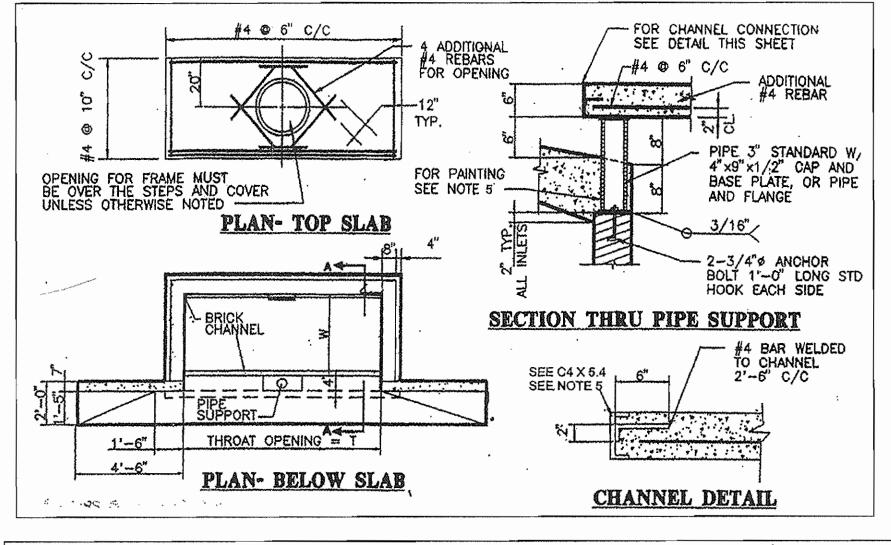




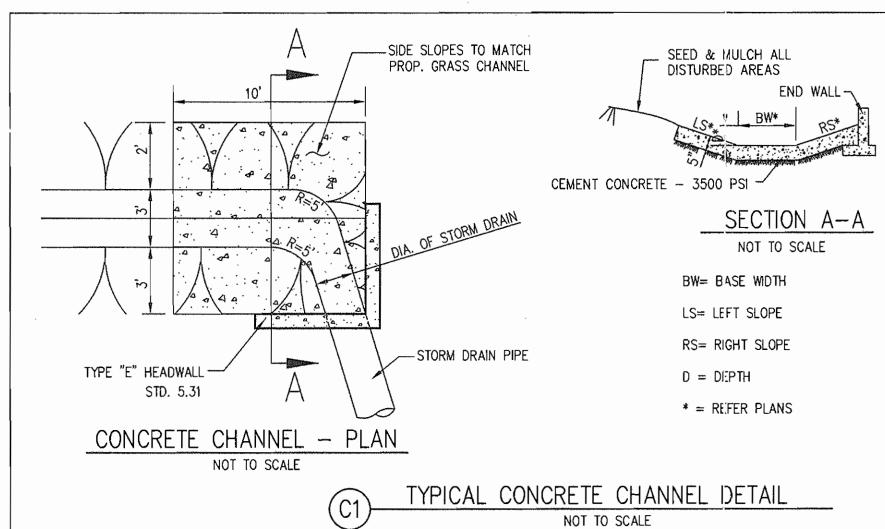
STRUCTURE NO. TOP ELEVATION NORTHING EASTING STD. NO. INV. IN INV. OUT REMARKS/STRUCTURE TYPE 533171.9910 147.35 1364198.6000 147.45 139.03 533120.7450 1364212.3008 TYPE 'C' ENDWALL, FOR 36" CIRCULAR PIPE EW-2 SD-5.21 5'-0" STANDARD AND SHALLOW PRECAST 139.30 533108.1557 1364202,5147 G 5.13 139.20 M-3MANHOLE FOR USE WITH 27" TO 36" PIPES 5'-0" STANDARD AND SHALLOW PRECAST 140.75/139.95 533027.2011 1364236.9560 139.75 MANHOLE FOR USE WITH 27" TO 36" PIPES MODIFIED A-10 INLET (CONTROL STRUCTURE) 144.75/144.65 140.95 532984.7552 1364247.7102 CS-10 149.67 REFER SHEET 8 OF 22 145.30 145.20 532892.1116 1364258.3273 SD-4.02 TYPE 'A-10' INLET TYPICAL BOX INLET 532650.1881 1364258.5584 150.43 146.50 REFER SHEET 6 OF 22 532496.3022 1364174.0797 TYPE 'A-5' INLET I**–**18 147.70 147.60 SD-4.01 TYPE 'A-5' INLET 148.20 532892.1116 1364091.9043 153.92 145.20 TYPE 'A-10' INLET 154.30 148.55 532554.8348 1364067.4277 SD-4.02 CONTROL STRUCTURE 1364203.0664 CS-20 151.35 43.10/144.55 | 533013.2711 144.65 REFER SHEET 8 OF 22 MODIFIED YARD INLET 144.85/146.15 533037.5073 1364194.8132 144.75 SD-4.14 L=6.0', W=5.0' 4'-0" STANDARD & SHALLOW PRECAST 1364192.9577 150.32 146.60 146.50 533104.0979 M - 24MANHOLE FOR USE WITH 24" & SMALLER PIPES 533140.1825 TYPE 'S' INLET |-4 1364096.6207 SD-4.22 MODIFIED YARD INLET 149.80/147.25 533020.7563 1364129.8133 147.15 SD-4.14 L=6.0', W=5.0'5'-0" STANDARD AND SHALLOW PRECAST 532930.7374 | 1364003.5205 153.60 48.10/149010/148.95 148.00 G 5.13 MANHOLE FOR USE WITH 27" TO 36" PIPES TYPICAL BOX INLET 1363971.8209 I-32 154.10 148.95 532958.4777 REFER SHEET 6 OF 22 TYPE 'E' HEADWALL FOR 24" CIRCULAR PIPE EW-36 151.91 (EX.) 533101.9653 1363958.7659 SD - 5.311363971.3109 152.35/151.70 151.60 533104.9867 TYPE 'A-5' INLET 157.06 SD-4.01 4'-0" STANDARD & SHALLOW PRECAST 155.80 152.50/152.40 152.30 532899.1031 1363845.0576 M-41 MANHOLE FOR USE WITH 24" & SMALLER PIPES 533191.7319 1363945.1723 TYPE 'A-10' INLET SD-4.02 154.25 532898.5896 1363998.8031 SD-4.02 TYPE 'A-10' INLET 157.59 532895.6771 | 1363827.8963 TYPE 'A-5' INLET 1-42 SD-4.01REFER SHEET 8 OF 22 SC-3 151.35 533002.0551 1364203.5183 144.51 144.26 REFER SHEET 8 OF 22 149.55 532979.9166 | 1364226.6564

STORM DRAIN STRUCTURE SCHEDULE

STRUCTURE	SKETCH	STAKEO	OUT DATA
		HORIZONTAL	VERTICAL (TOP)
STANDARD COS INLET	STATION B/L OF CONSTRUCTION  C/L OF STRUCTURE  HORZ.  FACE OF CURB  SQUARE OR  RECTANGULAR BOX  TROUGH	CENTER OF BOX AT FACE OF CURB	TOP OF CURB AT CENTER OF BOX
CURB	STATION B/L OF CONSTRUCTION  PAVEMENT ————————————————————————————————————	CENTER OF BOX AT FACE OF CURB	ON GRADE: TOP OF CURB AT ENDS OF BOX
BOX INLET	INLET	COND	ON SUMP: TOP OF CURB AT CENTER OF BOX
YARD INLET	HORZ. & VERT.	CENTER OF STRUCTURE	CENTER OF STRUCTURE
MANHOLES/ SQUARE/ CIRCULAR	HORZ. & VERT.	CENTER OF STRUCTURE	CENTER OF STRUCTURE
END WALLS		MID POINT OF ENDWALLS AT END OF PIPE	N/A



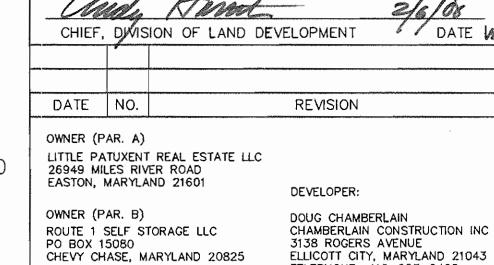
EW/33



S.H.A. CLASS II RIP-RAP 19" DEEP OVER FILTER CLOTH (BASED ON SDP 01-25) \_4" TOP SOIL SEED & MULCH  $Q_{10} = 57.80 \text{ CFS}$ ALL DISTURBED AREAS  $V_{10} = 9.62 \text{ FPS}$ 36" HDPE -<u>PLAN</u> FILTER FABRIC 0.0% \_\_\_ -GEOTEXTILE CLASS SE SECTION 'E-E' EX. OUTLET CHANNEL DETAIL AT EW/2 (BASED ON SDP 01-25) CUT-OFF NOT TO SCALE

NOTE

CONTRACTOR TO SUBMIT SHOP DRAWINGS SIGN AND SEALED BY MARYLAND REGISTERED PROFESSIONAL ENGINEER AND OBTAIN APPROVAL FROM HOWARD COUNTY DEPART OF PUBLIC WORK & DESIGN ENGINEER PRIOR TO ORDERING AND INSTALLING THE STRUCTURE



STORM DRAIN PIPE SCHEDULE

601

588

195

38

AND ZÓNING.

CHIEF, DEVELOPMENT ENGINEERING DIVISION

REMARKS

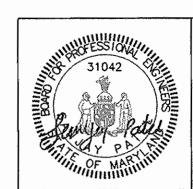
APPROVED : HOWARD COUNTY DEPARTMENT OF PLANNING

PIPE SIZE (IN.) CLASS/TYPE TOTAL LENGTH (FT.)

HDPE CLASS IV

Panal

24



AB CONSULTANTS, INC. 9450 ANNAPOLIS ROAD LANHAM, MARYLAND 20706 PHONE: (301) 306-3091

FAX: (301) 306-3092 CONTACT: SANJAY PATEL PHONE: 301-306-3091 x121

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PROJECT STORAGE USA FACILITY (PAR 'B') AND PARCEL 'A' OF THE A.H. SMITH SUBDIVISION SELF-STORAGE FACILITY EXPANSION AND PROPOSED FLEX SPACE BUILDING STORMDRAIN AND

MISCELLANEOUS DETAILS 6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND TAX MAP 47, PARCEL A & B, GRID 18, P/O PARCEL 144

SCALE: 1"=30" DATE: 01/23/08 DRAWN BY: HRP CHECKED BY: SBP SHEET: 6 OF 25

04 - 210

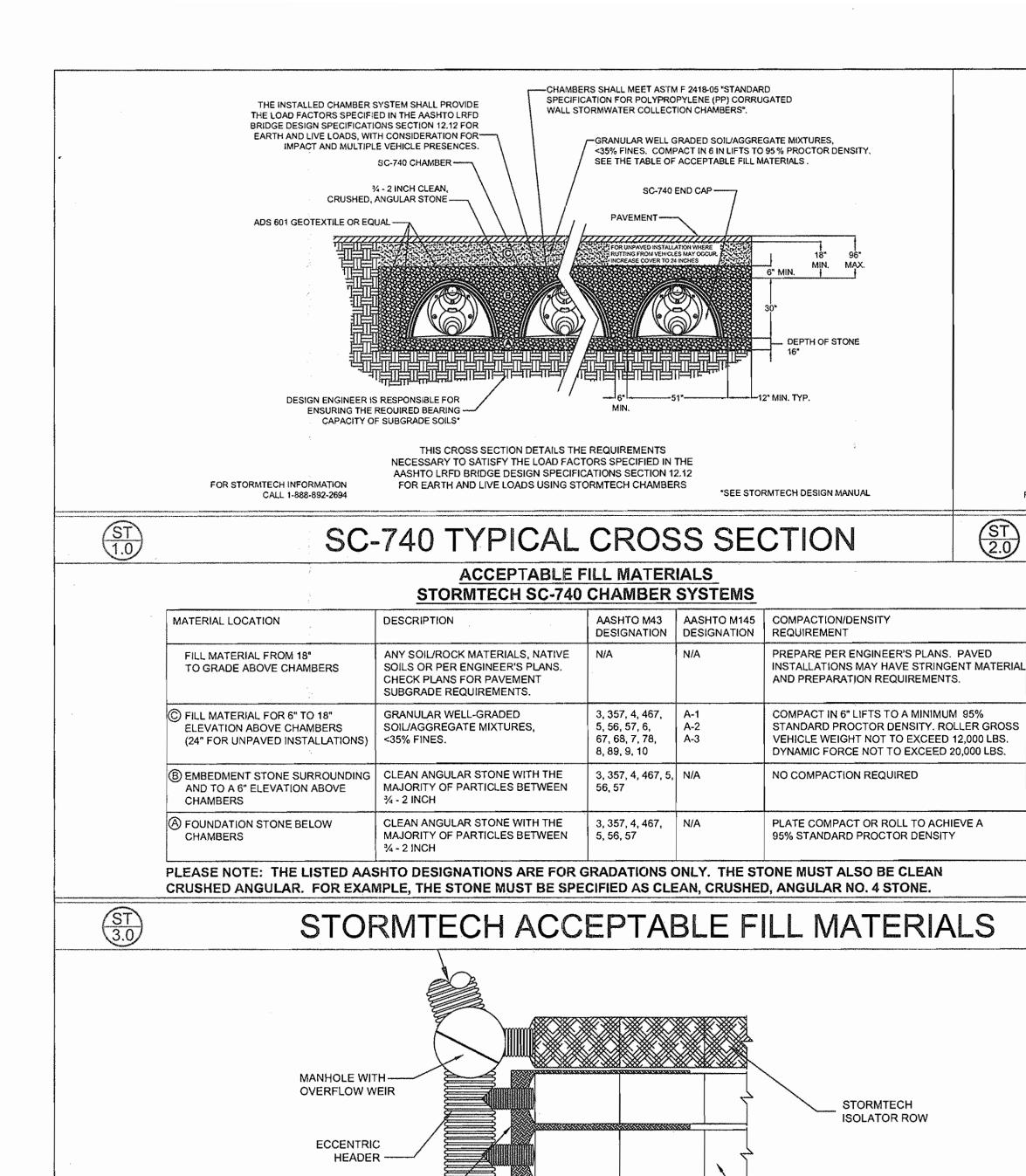
TELEPHONE: 410-203-2460

2/7/02

DATE

DATE W

108



PLACE MINIMUM 12.5' OF AASHTO M288

BEDDING STONE FOR SCOUR PROTECTION

MANHOLE

SUMP DEPTH

BY DESIGN

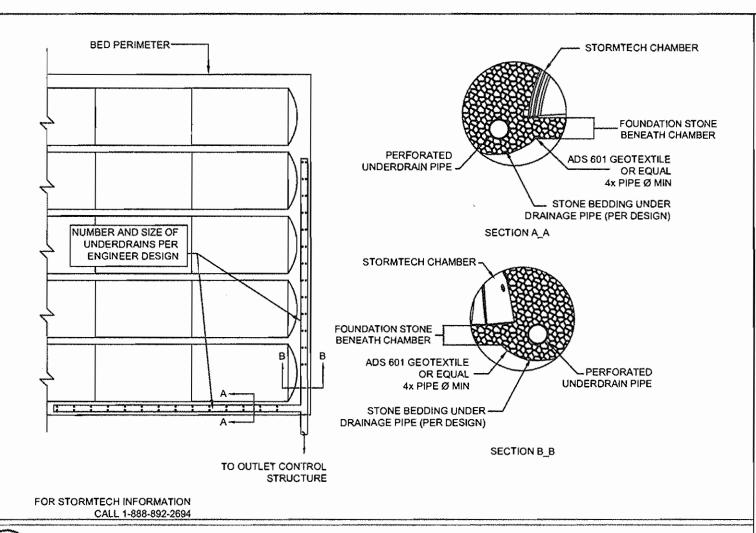
ENGINEER

CLASS 1 WOVEN GEOTEXTILE OVER

AT ALL CHAMBER INLET ROWS

STORMTECH

**PREFABRICATED** 



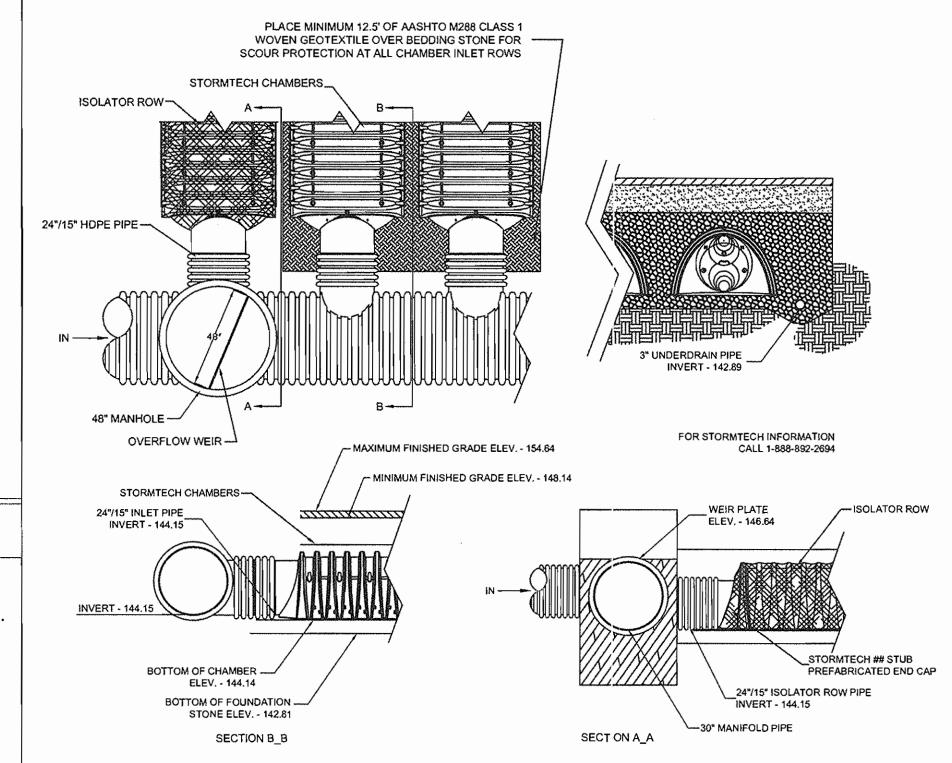
STORMTECH UNDERDRAIN DETAIL

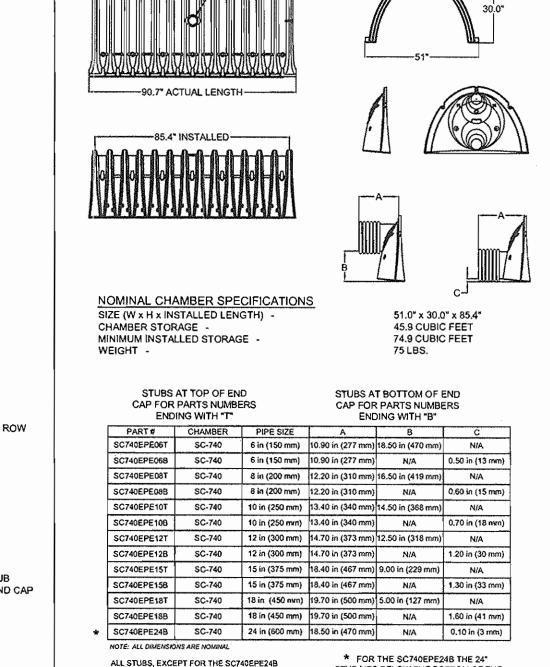
1. ALL DESIGN SPECIFICATIONS FOR STORMTECH CHAMBERS SHALL BE IN ACCORDANCE WITH THE STORMTECH DESIGN MANUAL 2. THE INSTALLATION OF STORMTECH CHAMBERS SHALL BE IN ACCORDANCE WITH THE LATEST STORMTECH INSTALLATION INSTRUCTIONS.

THE CONTRACTOR IS ADVISED TO REVIEW AND UNDERSTAND THE INSTALLATION INSTRUCTIONS PRIOR TO BEGINNING SYSTEM INSTALLATION. CALL 1-888-892-2694 OR VISIT

WWW.STORMTECH.COM TO RECEIVE A COPY OF THE LATEST STORMTECH INSTALLATION INSTRUCTIONS.

4. CHAMBERS SHALL MEET THE DESIGN REQUIREMENTS AND LOAD FACTORS SPECIFIED IN SECTION 12.12 OF THE LATEST EDITION OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS





ARE PLACED AT BOTTOM OF END CAP SUCH THAT THE OUTSIDE DIAMETER OF THE STUB IS

FLUSH WITH THE BOTTOM OF THE ENDICAP

STUB LIES BELOW THE BOTTOM OF THE

REMOVED FROM BELOW THE N-12 STUB

END CAP APPROXIMATELY 1.75\*.

BACKFILL MATERIAL SHOULD BE

SO THAT THE FITTING SETS LEVEL.

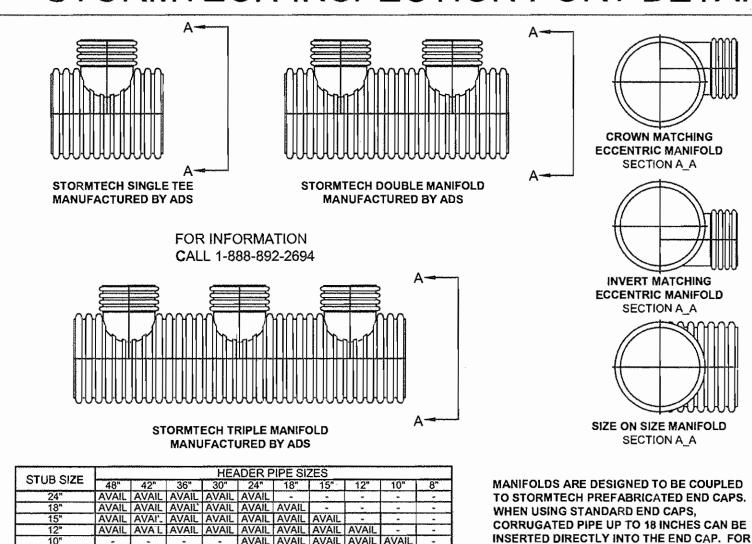
ACCEPTS 4" SCH 40 PIPE



# TECHNICAL DETAILS 627 SC-740 STORMTECH CHAMBERS **TOTAL STORAGE PROVIDED - 70184 CF** (SEE STAGE STORAGE SPREADSHEET FOR VOLUME) DOUBLE OMH WITH WEIR 24" PREFABRICATED ENDICAP

FLOOR BOX FRAME AND LID W/S.S. CAP SCREW LID CLOSURE INSPECTION PORT WITH SCREW-IN CAP PAVEMENT - CLASS "C" CONCRETE AASHTO M288 CLASS 2 NON-WOVEN GEOTEXTILE 4" PVC RISER INSPECTION PORT TO BE SC-740 CHAMBER ATTACHED THROUGH KNOCK-OUT LOCATED AT CENTER OF CHAMBER AASHTO M288 CLASS 2 NON-WOVEN GEOTEXTILE FOR STORMTECH INFORMATION CALL 1-888-892-2694

STORMTECH INSPECTION PORT DETAIL



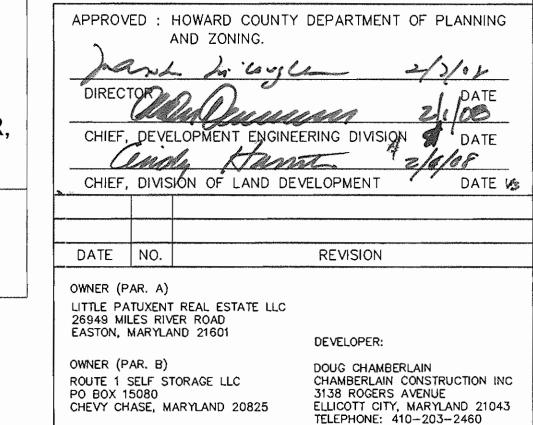
CHAMBERS SHALL MEET THE DESIGN REQUIREMENTS AND LOAD FACTORS SPECIFIED IN SECTION 12.12 OF THE LATEST EDITION OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS LOADS SHALL BE CALCULATED IN ACCORDANCE WITH SECTION 3 AND SHALL INCLUDE H20 DESIGN TRUCK, IMPACT FACTOR, MULTIPLE PRESENCE, AND LANE LOAD.



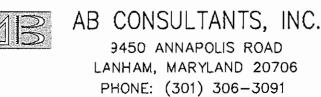
24" INLET PIPES, A CORRUGATED TO SMOOTH

PIPE ADAPTER IS REQUIRED.

STORMTECH SC-740 CHAMBER LAYOUT





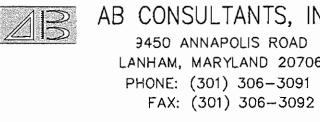


PROJECT STORAGE USA FACILITY (PAR 'B') AND PARCEL 'A' OF THE A.H. SMITH SUBDIVISION SELF-STORAGE FACILITY EXPANSION AND

PROPOSED FLEX SPACE BUILDING SWM FACILITY

**6TH ELECTION DISTRICT** TAX MAP 47, PARCEL A & B, GRID 18, P/O PARCEL 144

04 - 210SCALE: AS SHOWN DATE: 01/23/08 DRAWN BY: \_ CHECKED BY: \_



CONTACT: SANJAY PATEL PHONE: 301-306-3091 x121

HOWARD COUNTY, MARYLAND

SHEET: 7 OF 25 SDP-06-100

STORMTECH ISOLATOR™ ROW DETAIL

STORMTECH ISOLATOR™ ROW MANIFOLD DETAIL

INSPECTION PORT

AASHTO M288 CLASS 1 WOVEN GEOTEXTILE OR EQUAL,

BETWEEN FOUNDATION STONE AND CHAMBERS

SC-740--5'-6' WIDE STRIP

BY DESIGN ENGINEER

-STORMTECH CHAMBERS

COVER ENTIRE ROW WITH

GEOTEXTILE OR EQUAL

SC-740-8' WIDE STRIP

FOR STORMTECH INFORMATION

CALL 1-888-892-2694

- AASHTO M288 CLASS 2 NON-WOVEN

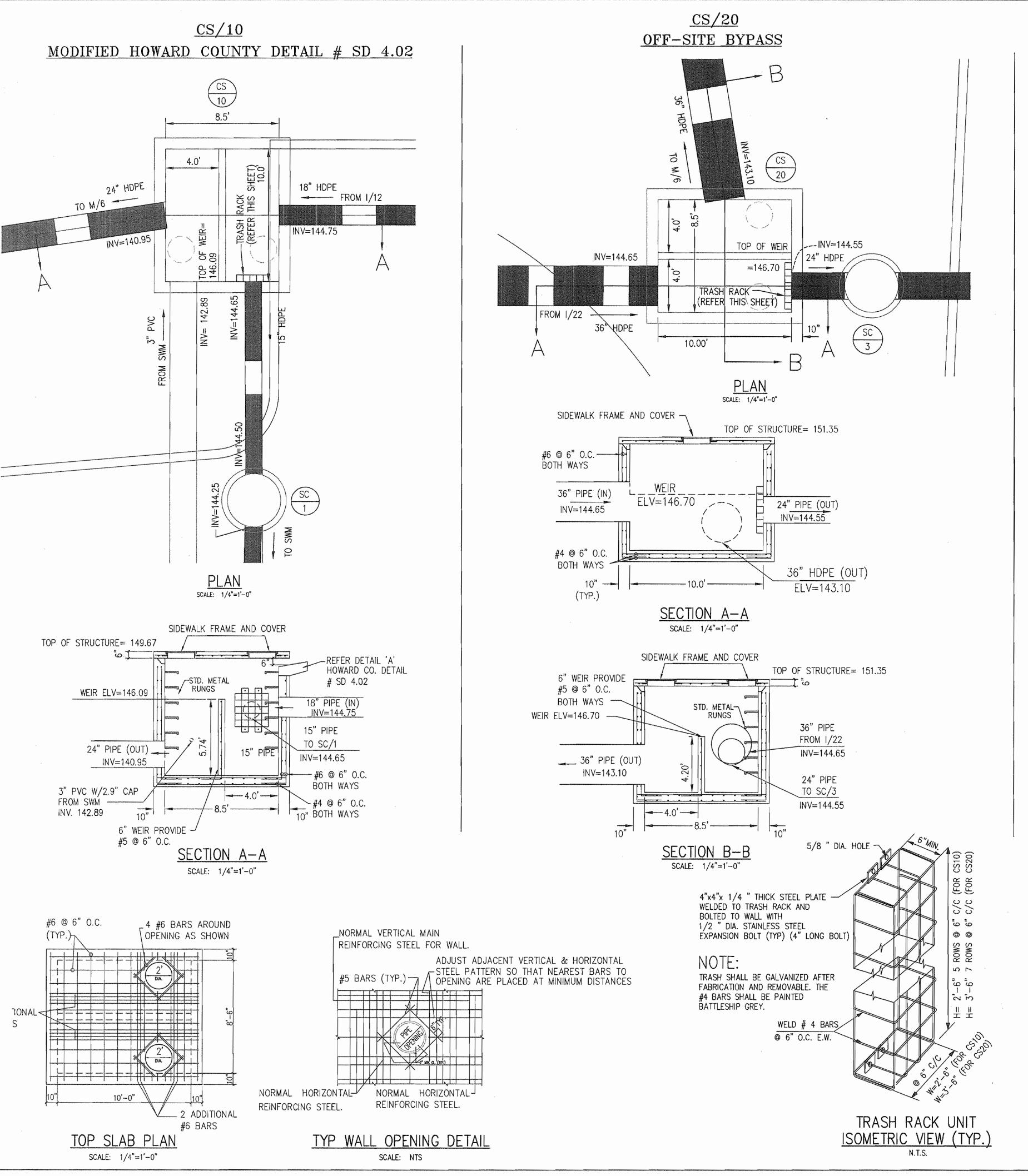
-- STORMTECH ENDCAP

FOR STORMTECH INFORMATION

CALL 1-888-892-2694

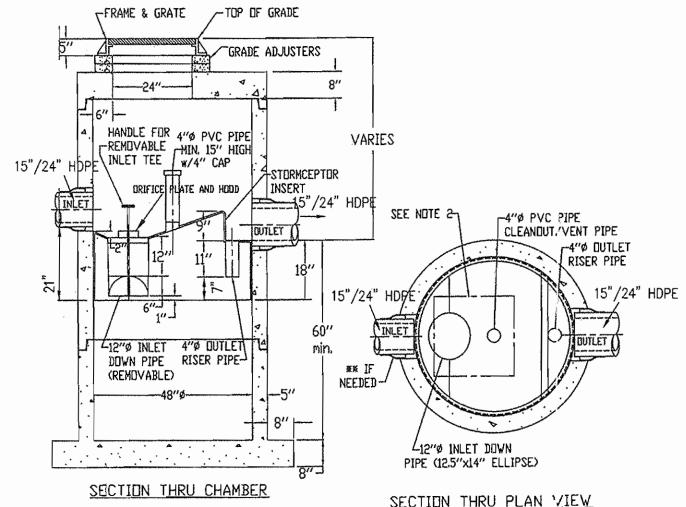
AVAIL - STAN JARD HEADERS AVAILABLE (ST) (7.0)

ADS MANIFOLD DETAILS

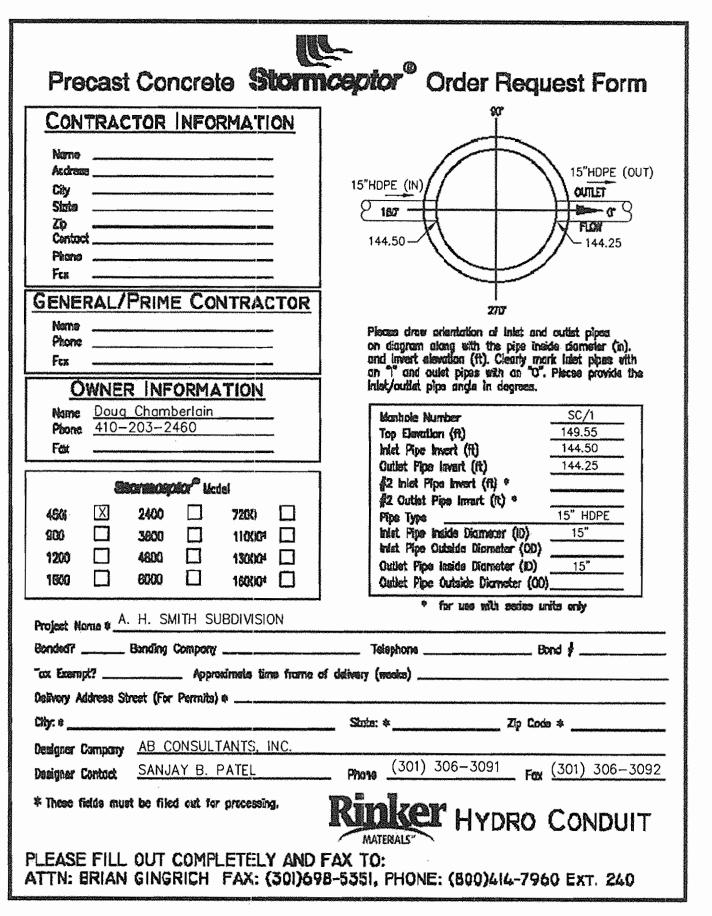


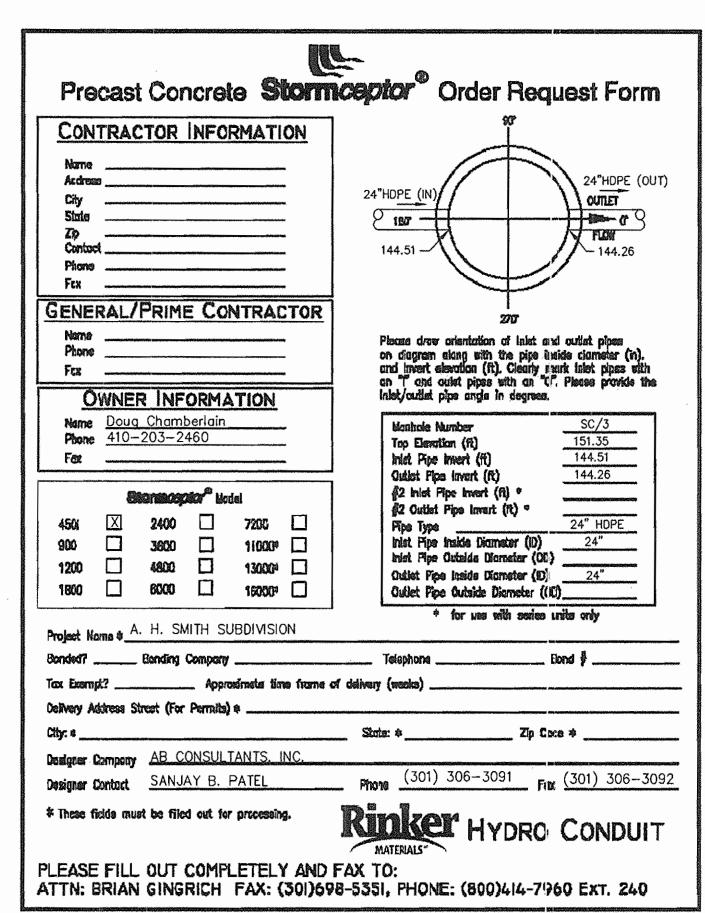
<u>FOR SC-1, SC-3</u>

STC 450i Precast Concrete Stormceptor® (450 US Gallon Capacity)



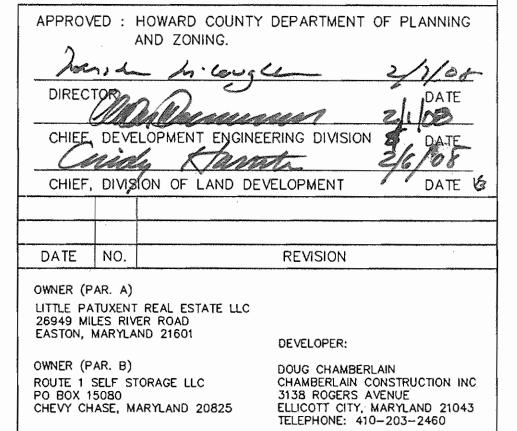
- 1. THE USE OF FLEXIBLE CONNECTIONS IS RECOMMENDED AT THE OUTLET WHERE APPLICABLE.
- 2. THE COVER SHOULD BE POSITIONED OVER THE 4"\$
  CLEANOUT/VENT PIPE.
- 3. THE STORMCEPTOR SYSTEM IS PROTECTED BY ONE OR MORE OF THE FOLLOWING U.S. PATENTS: #4985148, #5498331, #5725760, #5753115, #5849181.
- 4. CONTRACTOR TO PROVIDE CRANE TO SET UNIT (HEAVIEST SECTION WEIGHS 500) LB)

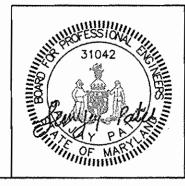




### OPERATION AND MAINTENANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED STORMCEPTOR WATER QUALITY DEVICE

- THE STORMCEPTOR WATER QUALITY STRUCTURE SHALL BE PERIODICALLY INSPECTED AND CLEANED TO MAINTAIN OPERATION AND FUNCTION. THE OWNER SHALL INSPECT THE STORMCEPTOR UNIT YEARLY AT A MINIMUM, UTILIZING THE STORMCEPTOR INSPECTION/MONITORING FORM. INSPECTIONS SHALL BE DONE BY USING A CLEAR PLEXIGLAS TUBE ("SLUDGE JUDGE") TO EXTRACT A WATER COLUMN SAMPLE. WHEN THE SEDIMENT DEPTHS EXCEED THE LEVEL SPECIFIED IN TABLE 6 OF THE STORMCEPTOR TECHNICAL MANUAL, THE UNIT MUST BE CLEANED.
- B. THE STORMCEPTOR WATER QUALITY STRUCTURE SHALL BE CHECKED AND CLEANED IMMEDIATELY AFTER PETROLEUM SPILLS. THE OWNER SHALL CONTACT THE APPROPRIATE REGULATORY AGENCIES.
- THE MAINTENANCE OF THE STORMCEPTOR UNIT SHALL BE DONE USING A VACUUM TRUCK WHICH WILL REMOVE THE WATER, SEDIMENT, DEBRIS, FLOATING HYDROCARBONS AND OTHER MATERIALS IN THE UNIT. PROPER CLEANING AND DISPOSAL OF THE REMOVED MATERIALS AND LIQUID MUST BE FOLLOWED BY THE OWNER.
- D. THE INLET AND OUTLET PIPES SHALL BE CHECKED FOR ANY OBSTRUCTIONS AT LEAST ONCE EVERY SIX MONTHS. IF OBSTRUCTIONS ARE FOUND THE OWNER SHALL HAVE THEM REMOVED. STRUCTURAL PARTS OF THE STORMCEPTOR UNIT SHALL BE REPAIRED AS NEEDED.
- E. THE OWNER SHALL RETAIN AND MAKE THE STORMCEPTOR INSPECTION/MONITORING FORMS AVAILABLE THE HOWARD COUNTY OFFICIALS UPON THEIR REQUEST.







AB CONSULTANTS, INC. 9450 ANNAPOLIS ROAD LANHAM, MARYLAND 20706 PHONE: (301) 306-3091 FAX: (301) 306-3092

CONTACT: SANJAY PATEL. PHONE: 301-306-3091 x121

PROJECT STORAGE USA FACILITY (PAR 'B') AND PARCEL 'A' OF THE A.H. SMITH SUBDIVISION SELF-STORAGE FACILITY EXPANSION AND PROPOSED FLEX SPACE BUILDING

SWM DETAILS

6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND TAX MAP 47, PARCEL A & B, GRID 18, P/O PARCEL 144

SHEET: 8 OF 25

DRAWN BY:

CHECKED BY:

ROJECT NO.

SCALE:

04 - 210

DATE: 01/23/08

### 13.0 Inspection and Maintenance



13.1 TREATMENT TRAIN INSPECTION AND MAINTENANCE The StormTech recommended treatment train inlet system has three tiers of treatment upstream of the StormTech chambers. It is recommended that inspection and maintenance (I&M) be initiated at the furthest upstream treatment tier and continue downstream as necessary. The following I&M procedures follow this approach providing 1&M information in the following order: Tier 1 - Pretreatment (BMP); Tier 2 - StormTech Isolator Row, and; Tier 3 -Eccentric Pipe Header System.

#### 13.2 CATCHBASIN/MANHOLE I&M

Typically a stormwater system will have catchbasins and manholes upstream of the detention/retention system. In some cases these may be the only pre-treatment devices. Regular I&M of catchbasins and manholes should be scheduled and performed as part of a site's routine maintenance plan.

### Catchbasin/Manhole - Step-by-Step

- Maintenance Procedures 1) Inspect catch basins and manholes upstream of
- StormTech chambers for sediment
- Remove grate or cover
- 3) Skim off oils and floatables
- 4) Using a stadia rod, measure the depth of sediment 5) If sediment is at a depth greater than 6" proceed
- to step 6. If not proceed to step 7.
- 6) Vacuum or manually remove sediment
- Replace grate
- 8) Record depth & date and schedule next inspection

### TABLE 10 - Pretreatment Inspection and Maintenance Guidelines

SEDIMENT CONTROL INSPECTION	INSPECTION*	MAINTENANCE**
StermTech Isolator™ Row	Bi-Annually	JetVac - Culvert Cleaning Nozzle Preferr
Sediment Basin	Quarterly or after large storm event	Excavate sediment
Catch Basin Sump	Quarterly	Excavate, pump, or vacuum
Sedimentation Structure	Quarterly	Excavate, pump, or vacuum
Catch Basin Filter Bags	After all storm events	Clean and/or replace filter bags
Porous Pavement	Quarterly	Sweep Pavement
Pipe Header Design	Quarterly	Excavate, pump, or vacuum
Water Quality Inlet	Quarterly:	Excavate, pump, or vacuum
Sand Filters	Quarterly or after storm event	Remove & replace sand filter

\* This schedule does not account for regional or site variables. Local municipal quidelines should be followed for inspection when available. \*\* The methods stated are minimum guidelines for removal and cleaning of system. Other methods may apply.

Figure 17 - Catchbasin/Manhole I&M Steps

4, 5, 6 ....

13.3 PRE-TREATMENT DEVICE I&M

Manufacturer's I&M procedures should be followed for

proprietary pretreatment devices such as baffle boxes,

swirt concentrators, oil-water separators, and filtration

units. **Table 10** provides some general guidelines but is

not a substitute for a manufacturer's specific instructions.

a properly functioning stormwater system. Inspection is easily accomplished through the manhole or optional inspection ports of an Isolator Row. Please follow local and OSHA rules for a confined space entry.

13.4 ISOLATOR™ ROW INSPECTION

Inspection ports can allow inspection to be accomplished completely from the surface without the need for a confined space entry. Inspection ports provide visual access to the system with the use of a flashlight. A stadia rod may be inserted to determine the depth of sediment. If upon visual inspection it is found that sediment has accumulated to an average depth exceeding 3 inches, cleanout is required.

Regular inspection and maintenance are essential to assure

A StormTech Isolator Row should initially be inspected immediately after completion of the site's construction. While every effort should be made to prevent sediment from entering the system during construction, it is during this time that excess amounts of sediments are most likely to enter any stormwater system. Inspection and maintenance, if necessary, should be performed prior to passing responsibility over to the site's owner. Once in normal service, a StormTech Isolator Row should be inspected bi-annually until an understanding of the sites characteristics is developed. The site's maintenance manager can then revise the inspection schedule based on experience or local requirements.

### 13.5 ISOLATOR ROW MAINTENANCE

JetVac maintenance is required if sediment has been collected to an average depth of 3 inches or more inside the Isolator Row. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row while scouring and suspending sediments. As the nozzle is retrieved, a wave of suspended sediments is flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45" are best. Most JetVac reels have a minimum of 400 feet of hose allowing maintenance of an Isolator Row up to 50 chambers long. The JetVac process shall only be performed on StormTech Rows that have AASHTO class 1 woven geotextile over their angular base stone.

### Additional Notes

1. Inspect every 6 months during the first year of operation. Adjust the inspection interval based on previous observations of sediment accumulation and high water 2. Conduct jetting and vactoring only when inspection show that maintenance is

### 13.6 ECCENTRIC PIPE HEADER INSPECTION

Theses guidelines do not supercede a pipe manufacturer's recommended I&M procedures. Consult with the manufacturer of the pipe header system for specific I&M procedures. Inspection of the header system should be carried out quarterly. On sites which generate higher levels of sediment more frequent inspections may be necessary. Headers may be accessed through risers, access ports or manholes. Measurement of sediment may be taken with a stadia rod or similar device. Cleanout of sediment should occur when the sediment volume has reduced the storage area by 25% or the depth of sediment has reached approximately 25% of the diameter of the structure.

### 13.7 ECCENTRIC PIPE HEADER MAINTENANCE

- Cleanout of accumulated material should be accomplished by vacuum pumping the material from the header. Cleanout should be accomplished during dry weath-
- er. Care should be taken to avoid flushing sediments out through the outlet pipes and into the chamber rows.

### Eccentric Header Step-by-Step Maintenance

- 1. Locate manholes, access ports or risers connected
- to the header system 2. Remove grates or covers
- 3. Using a stadia rod, measure the depth of sediment 4. If sediment is at a depth of about 25% pipe volume or 25% pipe diameter proceed to step 5. If not proceed to step 6.
- 5. Vacuum pump the sediment. Do not flush sediment out inlet pipes.
- Replace grates and covers
- 7. Record depth & date and schedule next inspection

### STORMTECH ISOLATOR" ROW - STEP-BY-STEP MAINTENANCE PROCEDURES

Step 1) Inspect Isolator Row for sediment

- A) Inspection ports (if present)
- i. Remove lid from floor box frame
- ii. Remove cap from inspection riser
- iii. Using a flashlight and stadia rod, measure depth of sediment
- iv. If sediment is at, or above, 3 inch depth proceed to Step 2. If not proceed to step 3.

### B) All Isolator Rows

- i, Remove cover from manhole at upstream end of Isolator Row
- ii. Using a flashlight, inspect down Isolator Row through outlet pipe
- 1. Mirrors on poles or cameras may be used to avoid a confined space entry
- 2. Follow OSHA regulations for confined space entry if entering manhole
- iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches) proceed to Step 2. If not proceed to Step 3.

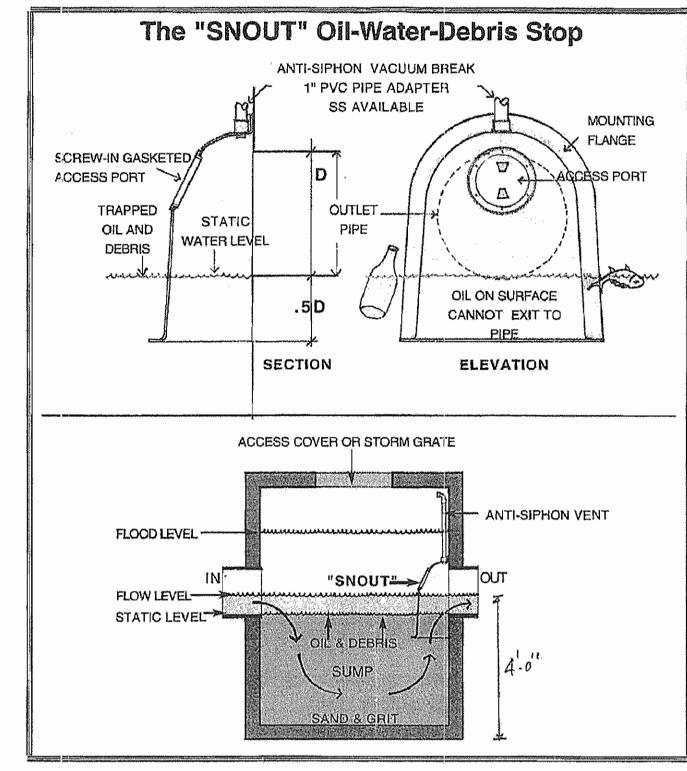
### Step 2) Clean out Isolator Row using the JetVac process

A) A fixed culvert cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable B) Apply multiple passes of JetVac until backflush water is clean C) Vacuum manhole sump as required

### Step 3) Replace all caps, lids and covers

Step 4) Inspect & clean catch basins and manholes upstream of the StormTech system following the procedures for Classic Manifold Inlet System

## Best Management Products, Inc.



For additional information visit our web-site at: <u>www.bestmp.com</u> or contact: T.J. Mullen 888-354-7585, 215-884-6195 fax, tim@bestmp.com

Best Management Products, Inc. • 53 Mt Archer Rd. • Lyme CT• 800-504-8008 • 860-434-3195 fax

STANDARD GRATE HAS H-25 HEAVY DUTY RATING

QUALITY: MATERIALS SHALL CONFORMS TO ASTM

PAINT: CASTINGS ARE FURNISHED WITH A BLACK PAINT

SOLID COVER HAS H-25 HEAVY DUTY RATING

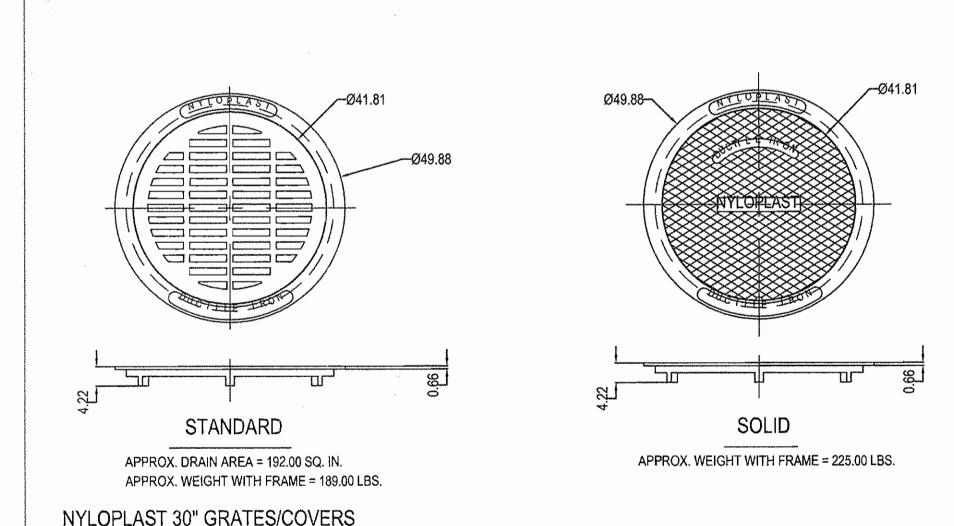
LOCKING DEVICE AVAILABLE UPON REQUEST

PRICE INCLUDES FRAME & GRATE/COVER

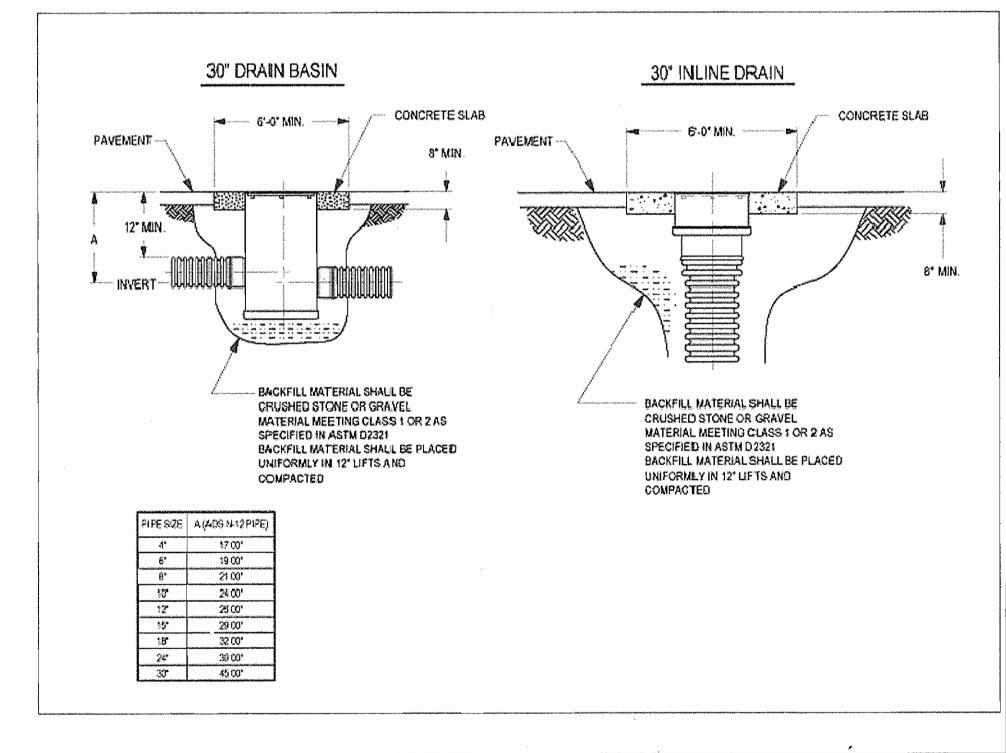
A536 GRADE 70-50-05

MATERIAL: DUCTILE IRON

# INSPECTION & MAINTENANCE



\* SIZE OF OPENING MEETS REQUIREMENTS OF AMERICAN DISABILITY ACT AS STATED IN FEDERAL REGISTER PART III, DEPARTMENT OF JUSTICE, 28 CFR PART 36, NONDISCRIMINATION ON THE BASIS OF DISABILITY BY PUBLIC ACCOMMODATIONS AND IN COMMERCIAL FACILITIES; FINAL RULE.







AB CONSULTANTS, INC. 9450 ANNAPOLIS ROAD LANHAM, MARYLAND 20706 PHONE: (301) 306-3091 FAX: (301) 306-3092

CONTACT: SANJAY PATEL PHONE: 301-306-3091 x121

PROPOSED FLEX SPACE BUILDING STORMTECH SC-740 CHAMBER DETAILS 6TH ELECTION DISTRICT

PROJECT STORAGE USA FACILITY (PAR 'B') AND

DATE NO.

OWNER (PAR. A)

OWNER (PAR, B)

PO BOX 15080

26949 MILES RIVER ROAD

EASTON, MARYLAND 21601

ROUTE 1 SELF STORAGE LLC

CHEVY CHASE, MARYLAND 20825

PARCEL 'A' OF THE A.H. SMITH SUBDIVISION

SELF-STORAGE FACILITY EXPANSION AND

LITTLE PATUXENT REAL ESTATE LLC

TAX MAP 47, PARCEL A & B, GRID 18, P/O PARCEL 144

APPROVED : HOWARD COUNTY DEPARTMENT OF PLANNING

REVISION

DEVELOPER:

DOUG CHAMBERLAIN
CHAMBERLAIN CONSTRUCTION INC

ELLICOTT CITY, MARYLAND 21043

TELEPHONE: 410-203-2460

3138 ROGERS AVENUE

AND ZONING.

CHIEF .. DEVELOPMENT ENGINEERING DIVISION

CHIEF. DIVISION OF LAND DEVELOPMENT

SHEET: 9 OF 25

04 - 210

SCALE: AS SHOWN

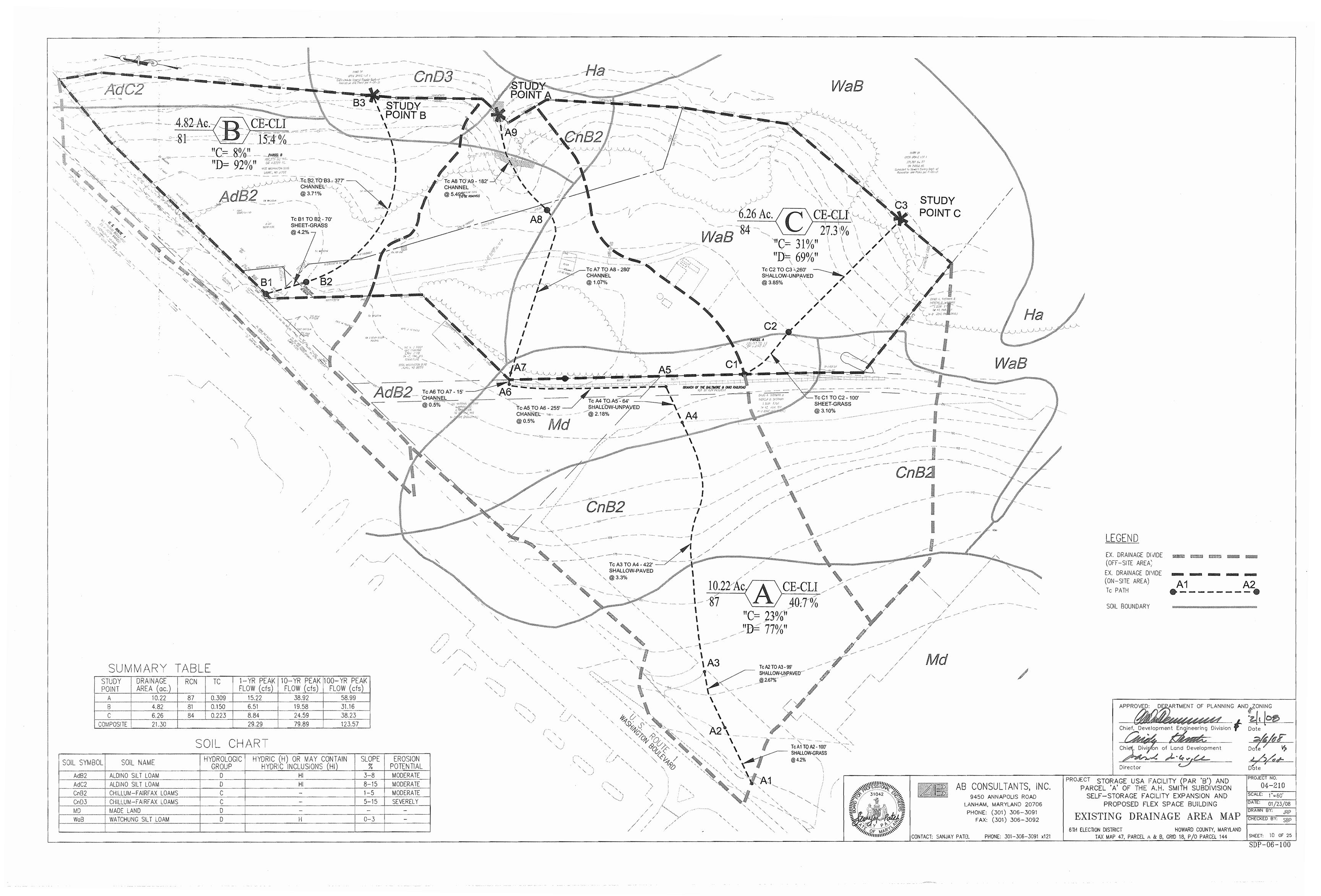
DATE: 01/23/08

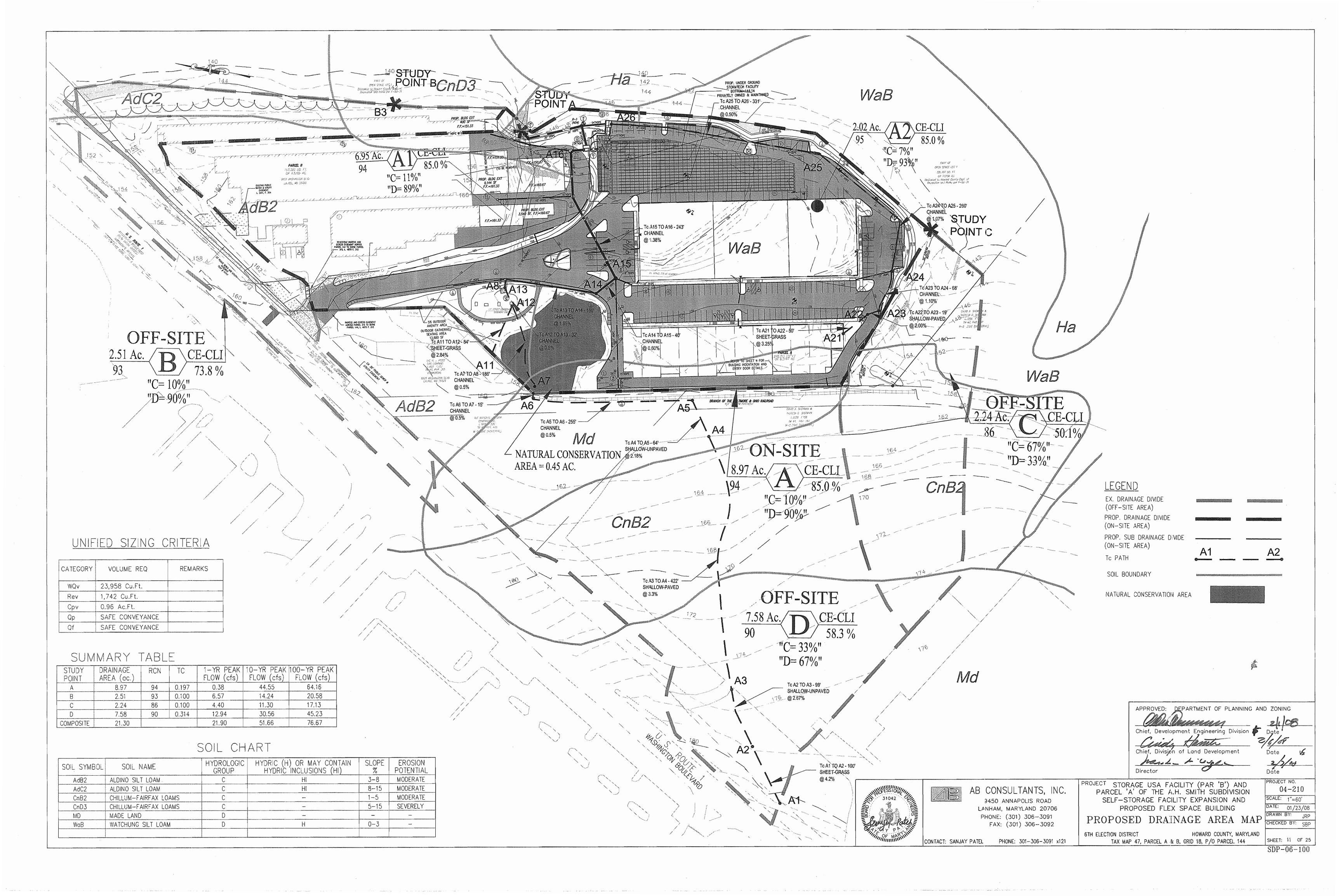
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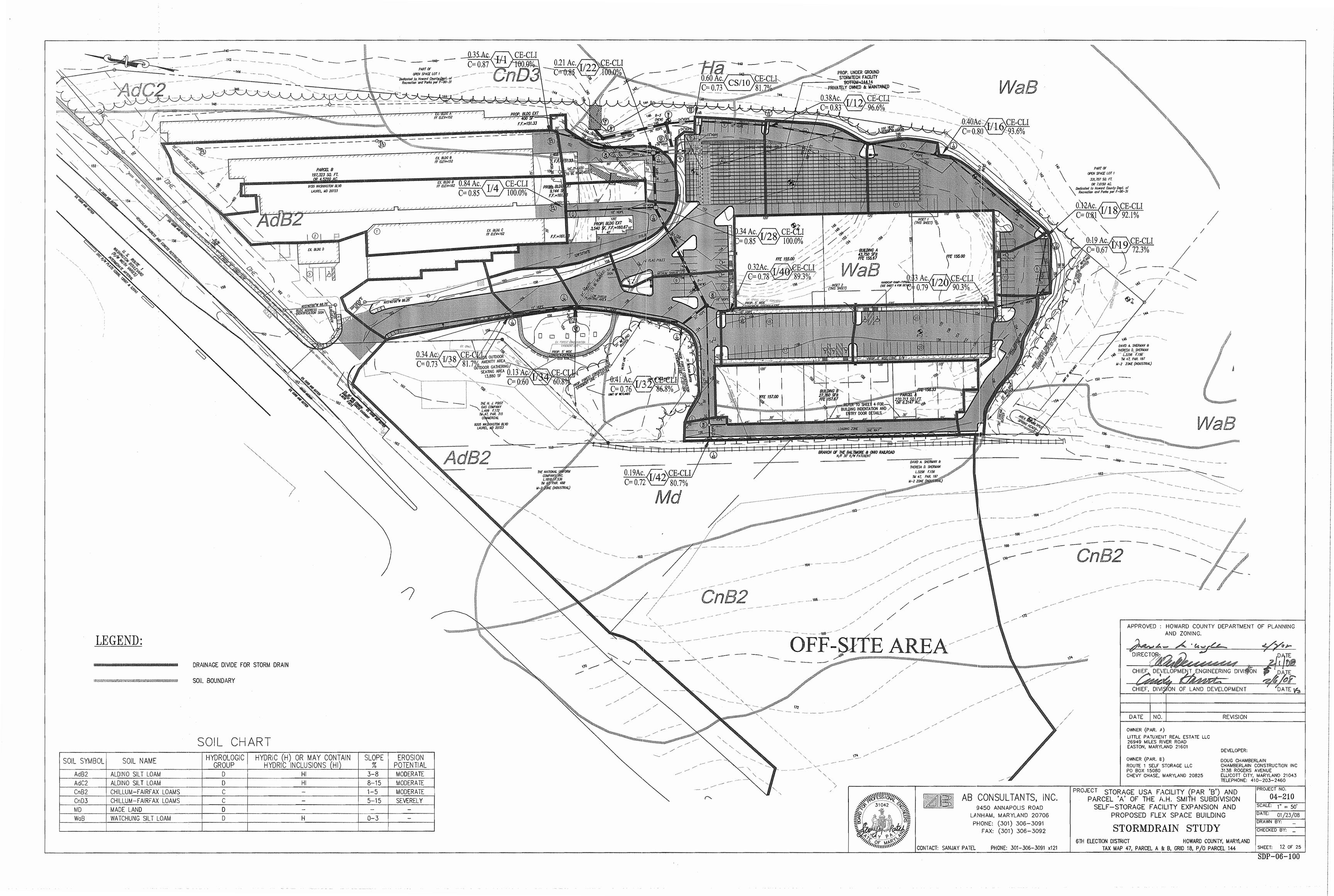
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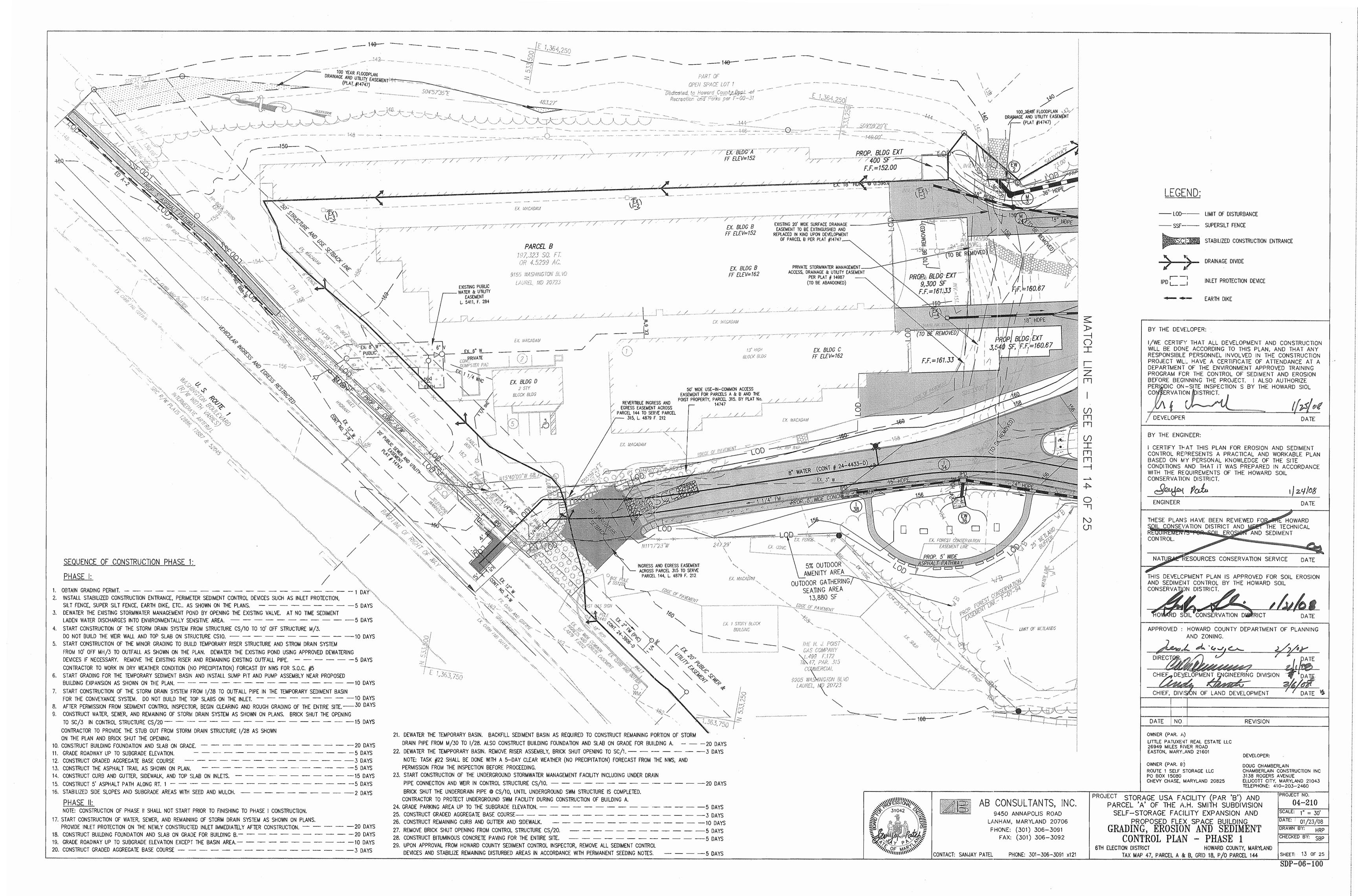
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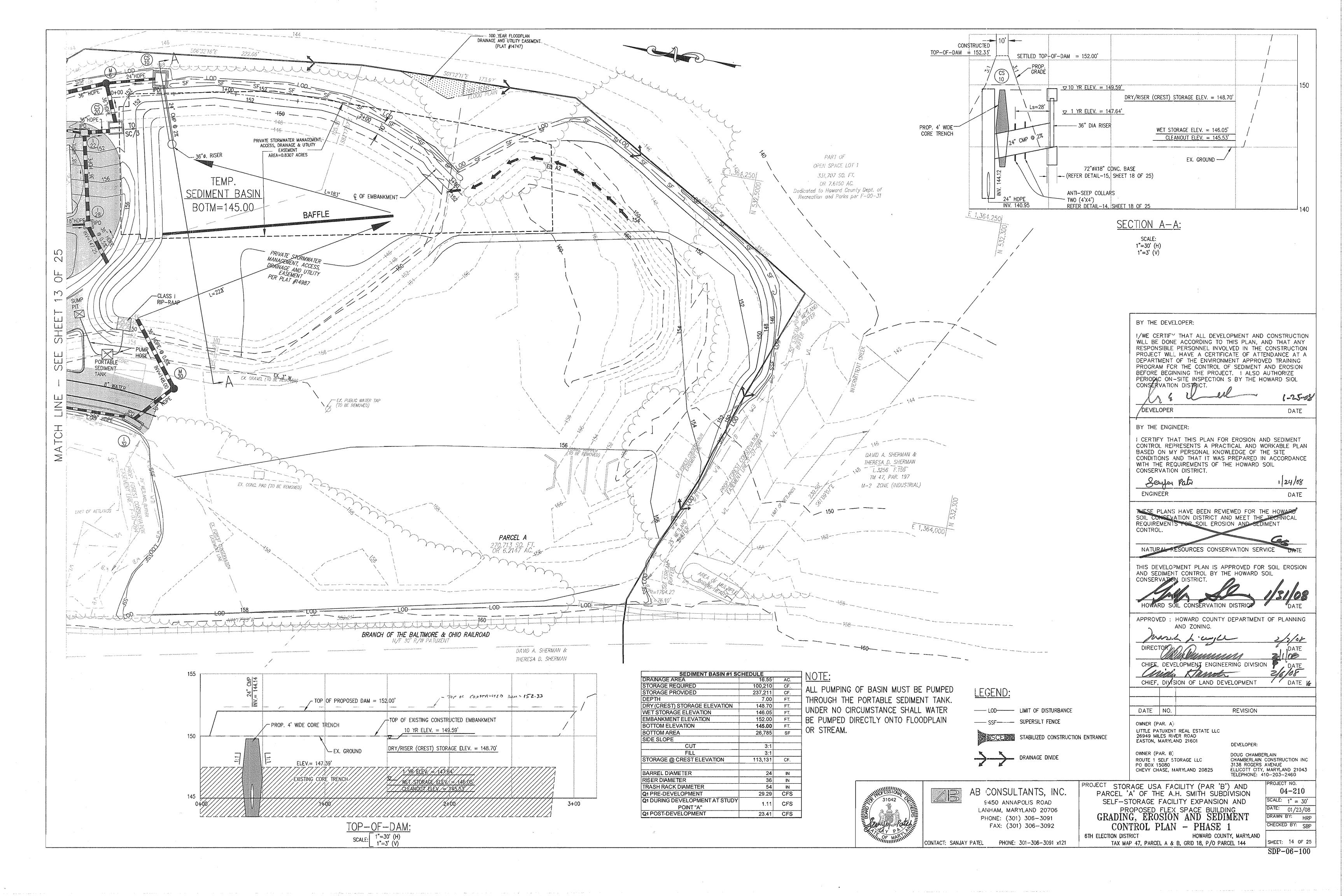
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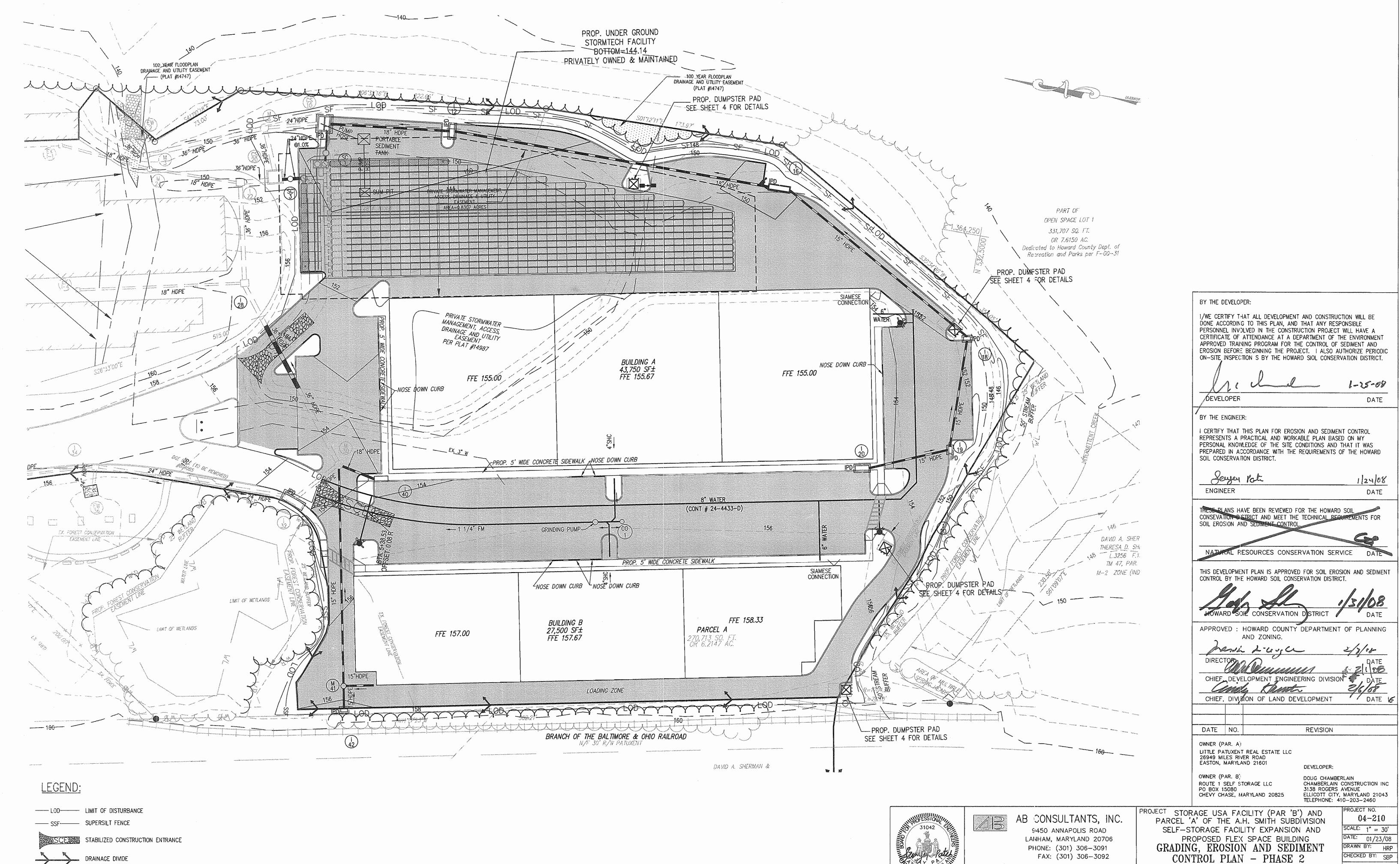












SHEET: 15 OF 25

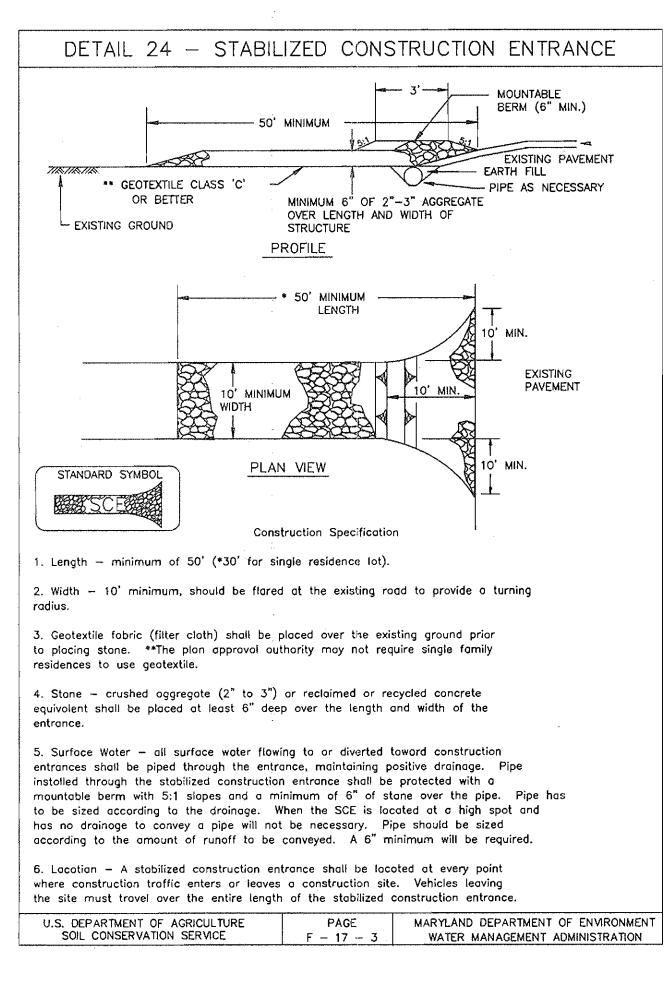
SDP-06-100

HOWARD COUNTY, MARYLAND

TAX MAP 47, PARCEL A &: B, GRID 18, P/O PARCEL 144

6TH ELECTION DISTRICT

CONTACT: SANJAY PATEL PHONE: 301-306-3091 x121



DETAIL 22 - SILT FENCE

36" MINIMUM FENCE

FLOW

TISTISTISTISTISTISTISTIS

POST LENGTH

EMBED GEOTEXTILE CLASS F

A MINIMUM OF 8" VERTICALLY

INTO THE GROUND

SECTION B

Construction Specifications

ground. Wood posts shall be 1 1/2 " x 1 1/2 " square (minimum) cut, or 1 3/4 " diameter

0.3 gal ft. / minute (mox.) Test: MSMT 322

1. Fence posts shall be a minimum of 36" long driven 16" minimum into the

(minimum) round and shall be of sound quality hardwood. Steel posts will be

standard T or U section weighting not less than 1.00 pond per linear foot.

2. Geotextile shall be fostened securely to each fence post with wire ties

3. Where ends of geotextile fabric come together, they shall be overlopped,

4. Silt Fence shall be inspected after each rainfall event and maintained when

bulges occur or when sediment occumulation reached 50% of the fobric height.

or staples at top and mid-section and shall meet the following requirements

50 lbs/in (min.)

20 lbs/in (min.)

10' MAXIMUM CENTER TO

FLOW

- CENTER \_

PERSPECTIVE VIEW

TOP VIEW

STAPLE 4

for Geotextile Closs F:

Tensile Strength

Tensile Modulus

U.S. DEPARTMENT OF AGRICULTURE

SOIL CONSERVATION SERVICE

Flow Rate

JOINING TWO ADJACENT SILT

FENCE SECTIONS

Filtering Efficiency 75% (min.)

folded and stapled to prevent sediment bypass.

FLOW

SECTION A

- 36" MINIMUM LENGTH FENCE POST.

- 16" MINIMUM HEIGHT OF

FENCE POST SECTION

MINIMUM 20" ABOVE

JETETETETETETE

FENCE POST DRIVEN A

MINIMUM OF 16" INTO

STANDARD SYMBOL

MARYLAND DEPARTMENT OF ENVIRONMENT

WATER MANAGEMENT ADMINISTRATION

UNDISTURBED

GROUND

GROUND

\_\_ THE GROUND

GEOTEXTILE CLASS F

8" MINIMUM DEPTH IN

GROUND

CROSS SECTION

Test: MSMT 509

Test: MSMT 509

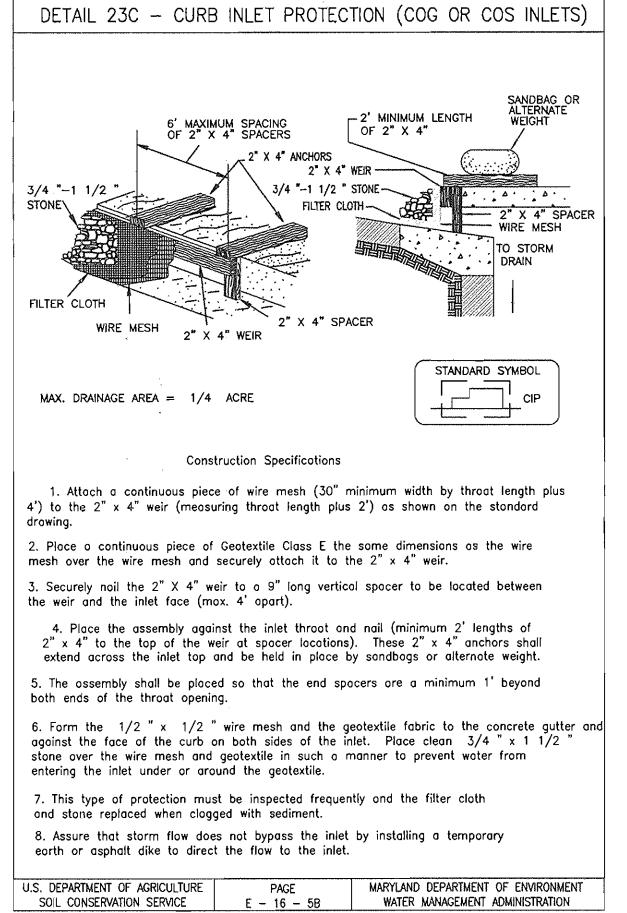
Test: MSMT 322

DRIVEN A MINIMUM OF 16" INTO

GROUND

FILTER

CLOTH ----



DETAIL 33 - SUPER SILT FENCE

10' MAXIMUM

CHAIN LINK FENCE

WITH 1 LAYER OF

MINIMUM

\_\_\_\_\_\_ FILTER CLOTH \*

FILTER CLOTH

TRIBIAN

Construction Specifications

1. Fencing shall be 42" in height and constructed in accordance with the

for a 6' fence shall be used, substituting 42" fabric and 6' length

4. Filter cloth shall be embedded a minimum of 8" into the ground.

develop in the silt fence, or when silt reaches 50% of fence height

latest Maryland State Highway Details for Chain Link Fencing. The specification

2. Chain link fence shall be fastened securely to the fence posts with wire ties.

The lower tension wire, brace and truss rods, drive anchors and post caps are not

3. Filter cloth shall be fastened securely to the chain link fence with ties spaced

5. When two sections of filter cloth odjoin each other, they shall be overlapped

7. Filter cloth shall be fastened securely to each fence post with wire ties or

staples at top and mid section and shall meet the following requirements for

6. Maintenance shall be performed as needed and silt buildups removed when "bulges"

50 lbs/in (min.)

20 lbs/in (min.)

PAGE

NOTE: FENCE POST SPACING

TRITATION

MINIMUM INTO GROUND

\* IF MULTIPLE LAYERS ARE

REQUIRED TO ATTAIN 42

required except on the ends of the fence.

every 24" at the top and mid section.

Tensile Strength

Tensile Modulus

Filtering Efficiency 75% (min.)

Flow Rate

U.S. DEPARTMENT OF AGRICULTURE

by 6" and folded.

Geotextile Class F:

SHALL NOT EXCEED 10'

TISTISTISTISTIS &

GROUND 1

SURFACE

CHAIN LINK FENCING

FILTER CLOTH

EMBED FILTER CLOTH 8"

2 1/2 " DIAMETER

OR ALUMINUM

POSTS

GALVANIZED

CENTER TO CENTER

### DETAIL 23D - MEDIAN INLET PROTECTION CONCENTRATED FLOW 14-6" ... EXISTING DITCH OR GUITER -- INLET NOTCH -- UNDISTURBED /EXISTING CROUND L MIP SECTION A-A Construction Specifications (, Fence posts shall be 36" (mjn.) long, ariven 16" into the ground and spaced 5" (max.) apart. Wood posts shall be 11/2" x 11/2" (min.) square cut or 13/4" (min.) diameter round and shall be of sound quality harawood. Steel posts shall be 2. Georexrile Class F shall be fastened securely to each post with wire riss of stopies of top and mid-section. 3. where ends of georestile fabric come together they shall be overlapped. folded 4. Median (niet Promotion shall be inspected after each rain and maintained 5. Stone used to construct the weir shall be 4'' = 7'' with a 1' thick layer of 34" - 112" stone on the upstream face. MARTLAND DEPARTMENT OF ENVIRONMENT U.S. DEPARTMENT OF AGRICULTURE WATER MANAGEMENT ADMONISTRATION SOIL CONSERVATION STRVICE

### STANDARD EROSION AND SEDIMENT CONTROL NOTES

- A minimum of 48 hours notice must be given to the Howard County Department of Inspections, Licenses and permits, sediment control divisions prior to the stort of any construction (313—1855).
- All vegetative and structural practices are to be installed according to the provisions of this plan and are to be in conformance with the 1994 Maryland standards and specifications for soil erosion and sediment control and revisions thereto.
- 3. Following initial soil disturbance or re-disturbance, permanent or temporary stabilization shall be completed within: A) 7 calendar days for all perimeter sediment control structures, dikes. perimeter slopes, and all slopes steeper than 3:1 B) 14 days as to all other disturbed or graded areas on the project site.
- 4. All sediment traps/basins shown mus be fenced and warning signs posted around the perimeter in accordance with vol.1, chapter 7, of the Howard county design manual, storm drainage.
- 5. All disturbed areas must be stabilized within the time period specified above in accordance with the 1994 Maryland standards and specifications for soil erosion and sediment control for permanenet seeding, SOD, temporary seeding, and mulching. ( see 6). Temporary stabilization with mulch alone shall only be done when recommended seeding dates do not allow for proper germination and established of grosses.
- 6. All sediment control structures are to be remain in place and are to be maintained in operative condition until permission for their removal has been obtained from the Howard county sediment control inspector.
- 7. Site Analysis:

4" MINIMUM

TINTINTIN

36" MINIMUM

STANDARD SYMBOL

— S\$F —

FLOW

- 16" MIN. 1ST LAYER OF

Test: MSMT 509

Test: MSMT 322

0.3 gal/ft /miñute (mox.) Test: MSMT 322

Test: MSMT 509

MARYLAND DEPARTMENT OF ENVIRONMENT

WATER MANAGEMENT ADMINISTRATION

Total area of site	10.74 Acres
Area Disturbed	6.51 Acres
Area to be roofed ar paved	7.56 Acres
Area to be vegetatively stabilized	1.57 Acres
Total cut	4821 CY
Total fill	3364D CY
offsite waste/barrow location to h	nave an active grading pe

- Any sediment control practice which is disturbed by grading activity for placement of utilities must be repaired on the same day of disturbance.
- Additional sediment controls must be provided, if deemed necessary by the Howard county sediment control inspector.
- 10. On all sites with disturbed greas in excess of 2 Acres, approval of the inspection agency shall be requested upon completian of installation of perimeter erosion and sediment controls, but before proceeding with any other earth disturbance or granding. Other building or grading inspection approvals may not be authorized until this initial approval by the inspection agency
- Trenches for the construction of utilities is limited to three pipe lengths or that which shall be back-filled and stabilized within one working day, whichever is shorter.
- 12. Site grading will begin only after all perimeter sediment control measures have been installed and are in a functioning condition.
- 13. Sediment will be removed from traps when its depth reaches clean out elevation shown on the
- 14. Cut and fill quantities provided under site analysis do not represent bid quantities. These quantities do not distinguish between topsoil, structural fill or embankment material. nor do they reflect consideration of undercutting or removal of unsuitable material. The contractor shall familiarize himself with site conditions which may affect the wark.

## Vegetation — annual grass ar grain used to provide cover on disturbed areas for up to 12 months. For langer duration of vegetative cover, Permanent Seeding is required. A. Seed Mixtures - Temporary Seeding Select one or more of the species or mixtures listed in Table 26 for the appropriate Plant hardiness Zone (from Figure 5) and enter them in the Temporary Seeding Summary below, along with application rates, seeding dotes and seeding depths. If this summary is not put on the plans and completed, then Table 26 must be put an the plans.

Temporary Seeding

**BARLEY** 

Permanent Seeding

NO.

WEEPING LOVEGRASS

generally receiving low maintenance.

SPECIES

(85%)

(10%)

(5%)

PERENNIAL RYEGRASS

KENTUCKY BLUEGRASS

TALL FESCUE

For sites having soil test performed, the rates shown on this table shall be deleted and the rates recommended by the testing agency shall be written in. Soil tests are not required for Temporary

SEEDING SEEDING

8/15-11/15 1"-2"

Seeding gross and legumes to establish ground cover for a minimum period of one year an disturbed areas

the rates recommended by the sail testing agency shall be written in.

Select one or more of the species or mixtures listed in Table 25 for the appropriate Plant

Hardiness Zone (from Figure 5) and enter them in the Permanent Seeding Summary below, along with application rates and seeding dates. Seeding depths can be estimated using Table 26. If this Summary is not put on the construction plans and completed, then Table 25 must be put on the plans. Additional planting specifications for exceptional sites such as shorelines,

streambanks, or dunes or for special purposes such as wildlife or aesthetic treatment may be found in USDA-SCS Technical Field Office guide, Section 342 — Critical Area Planting. For special lown maintenance areas, see Sections IV Sod and V Turfgrass.

iii. Far oreas receiving low maintenance, apply ureaform fertilizer (46-0-0) at 3 1/2 lbs/1000 sq.ft. (150 lbs/ac), in addition to the above soil amendments shown in the table below, to be

For sites having disturbed area over 5 acres, the rates shown on this table shall be deleted and

SEEDING SEEDING

8/15-10/15 | 1/4"-1/2"

8/15-10/15 1/4"-1/2"

FOR TOPSOIL

To provide a suitable soil medium for vegetative growth. Sails of concern have low moisture content, low

c. The original soil to be vegetated cantains material toxic to plant growth.

II. For the purpose of these Standards and Specifications, areas having slopes steeper than 2:1 require

special consideration and design for adequote stabilization. Areas having slopes steeper than 2:1

Topsoil salvaged from the existing site may be used provided that it meets the standards as set

be found in the representative soil profile section in the Soil Survey published by USDA-SCS in

ii. Topsoil must be free of plants or plant parts such as bermudo grass, quackgrass, Johnsongrass, nutsedge, poison ivy, thistle, or others os specified.

forth in these specifications. Typically, the depth of topsoil to be salvaged for a given soil type can

d. The sail is so acidic that treatment with limestone is not feasible.

Construction and Material Specifications

a. The texture of the exposed subsoil/parent material is not adequate to produce vegetative

The soil material is so shallow that the rooting zone is not deep enough to support plants or furnish continuing supplies of moisture and plant nutrients.

Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay loam, loamy sand. Other soils may be used if recommended by an agronomist or soil scientist and approved by the appropriate approval authority. Regardless, topsoil shall not be a mixture of contrasting textured subsoils and shall contain less than 5% by volume of cinders, stanes, slag, coarse fragments, gravel, sticks, roots, trash, or other materials larger than 1 1/2" in diameter.

Where the subsoil is either highly acidic or composed of heavy clays, ground limestone shall be spread at the rate of 4-8 tons/acre (200-400 pounds per 1,000 square feet) prior to

the placement of topsoil. Lime shall be distributed uniformly over designated oreas and warked into the soil in conjunction with tillage operations as described in the following

i. Place topsoil (if required) and apply soll amendments as specified in 20.0 Vegetative Stabilization

21.0 STANDARD AND SPECIFICATIONS

Placement of topsoil over a prepared subsail prior to establishment of permanent vegetation.

nutrient levels, low pH, materials toxic to plants, and/cr unacceptable soil gradation

I. This proctice is limited to areas having 2:1 or flatter slopes where:

shall have the appropriate stabilization shown on the plans.

coaperation with Maryland Agricultural Experimental Station.

III. For sites having disturbed areas under 5 acres:

IV. For sites having disturbed areas over 5 acres:

II. Topsoil Specifications - Soil to be used as topsoil must meet the following:

Section I — Vegetative Stabilization Methods and Materials.

To permit dissipation of phyto-toxic materials.

i. On soil meeting Topsoil specifications, obtain test results dictating fertilizer and lime amendments required to bring the soil into compliance with the following:

a. PH for topsoil shall be between 6.0 and 7.5. If the tested soil demonstrates a pH

of less than 6.0, sufficient lime shall be prescribed to raise the pH to 6.5 or higher.

b. Organic content of topsoil shall be not less than 1.5 percent by weight.

c. Topsail having soluble salt content greater than 500 parts per million shall not be

d. No sod or seed shall be placed on soil which has been treated with soil sterilants or

chemicals used far weed control until sufficient time has elapsed (14 days min.)

Conditions Where Practice Applies

Definition

DATE

PERMANENT SEEDING SUMMARY

DEPTH

1"-2"

FERTILIZER RATE LIME RATE

(15 LB/1000SF) (100LB/1000SF)

FERTILIZER RATE

P<sub>2</sub>0<sub>5</sub>

(2 LB/| (4 LB/| (4 LB/|<sub>(100LB</sub>/

1000 SF)|1000 SF)|1000 SF)| 1000SF)

(10-20-20)

LIME RATE

2 TONS/AC

2 TONS/AC

(10-10-10)

600 LBS/AC

TEMPORARY SEEDING SUMMARY

RATE (LB/AC) DATE DEPTH

2/1-4/30

5/1~8/14

SEED MIXTURE(HARDINESS ZONE 6B)

APPLICATION |

122

140

Seed Mixtures - Permanent Seeding

performed of the time of seeding.

SEED MIXTURE(HARDINESS ZONE 68)

FROM TABLE 25

APPLICATION

RATE (LB/AC)

FROM TABLE 26

AB CONSULTANTS, INC. 9450 ANNAPOLIS ROAD LANHAM, MARYLAND 20706

PHONE: (301) 306-3091 FAX: (301) 306-3092

CONTACT: SANJAY PATEL PHONE: 301-306-3091 x121 Note: Topsoil substitutes or amendments, as recommended by a qualified agronomist or soil scientistand approved by the appropriate opproval authority, may be used in lieu of

ii. Place topsoil (if required) and apply soil amendments as specified in 20.0 Stabilization - Section I - Vegetative Stabilization Methods and Materials.

When topsoiling, maintain needed erosion and sediment control practices such as diversions, Grade Stabilization Structures, Earth Dikes, Slope Silt Fence and Sediment

Grade an the areas to be topsoiled, which have been previously established, shall be maintained, albeit 4" — 8" higher in elevation.

iii. Tapsoil shall be uniformly distributed in a 4" - 8" layer and lightly campacted to a minimum thickness of 4". Spreoding shall be performed in such a manner that sodding a seeding can praceed with a minimum of additional soil preparation and tillage. Any irregularities in the surface resulting from topsoiling or other operations shall be corrected in order to prevent the formation of depressions or water pockets.

iv. Tapsoil shall not be placed while the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet ar in a condition that may otherwise be detrimental to proper grading and seedbed preparation.

Vi. Alternative for Permanent Seeding — Instead of applying the full amounts of lime and commercial fertilizer, composted studge and amendments may be applied as specified below:

Composted Sludge Moterial use as a soil conditioner for sites having disturbed areas over 5 ocres shall be tested to prescribe amendments and for sites having disturbed areas under 5 acres shall conform to the fallowing requirements:

o. Composted sludge shall be supplied by, or ariginate from, a person or persons that permitted (at the time of ocquisition of the compost) by the Moryland Department of the Environment under COMAR 26.04.06.

Camposted sludge shall contain at least 1 percent nitragen, 1.5 percent phosphorus, and 0.2 percent potassium and have a Ph of 7.0 to 8.0. If compost does not meet these requirements, the appropriate constituents must be added to

c. Composted sludge shall be applied at a rate of 1 tan/1,000 square feet.

iv. Composted sludge shall be amended with a potassium fertilizer applied at the rate of 4 lb/1,000 square feet, and 7/3 the normal lime application rate.

References: Guideline Specifications, Soil Preparation and Sadding. Md—VA, Pub. #1, Cooperative Extension Service, University of Maryland and Virginia Polytechnic Institutes.

BY THE DEVELOPER:

I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTION S BY THE HOWARD SIOL CONSERVATION DISTRICT.

1/16	L.A.	1-18.08
EVELOPER		DATE

BY THE ENGINEER:

CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT

Leuje	Pot	1/24/08
ENGINEER		DATE

THESE PLANS HAVE BEEN REVIEWED FOR THE HOWARD SOU CONSEVATION DISTRICT AND MEET THE TECHNICAL REQUIREMENTS FOR SOIL EROSION AND SEDIMENT CONTROL.

FESOURCES CONSERVATION SERVICE DATE

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.

APPROVED : HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING.

Ja Cosch DIRECTOR \_DEVELOPMENT ENGINEERING DIVISION

CHIEF, DIVISION OF LAND DEVELOPMENT

DATE NO. REVISION

OWNER (PAR, A) LITTLE PATUXENT REAL ESTATE LLC

26949 MILES RIVER ROAD EASTON, MARYLAND 21601

OWNER (PAR. B) ROUTE 1 SELF STORAGE LLC PO BOX 15080 CHEVY CHASE, MARYLAND 20825

DOUG CHAMBERLAIN CHAMBERLAIN CONSTRUCTION INC 3138 ROGERS AVENUE ELLICOTT CITY, MARYLAND 21043 TELEPHONE: 410-203-2460

DEVELOPER:

PROJECT STORAGE USA FACILITY (PAR 'B') AND PARCEL 'A' OF THE A.H. SMITH SUBDIVISION SELF-STORAGE FACILITY EXPANSION AND PROPOSED FLEX SPACE BUILDING

EROSION & SEDIMENT CONTROL NOTES AND DETAILS

6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND TAX MAP 47, PARCEL A 8; B, GRID 18, P/O PARCEL 144

04 - 210SCALE: NTS DATE: 01/23/08 DRAWN BY: CHECKED BY: \_

DATE

DATE 🔏

SHEET: 16 OF 25 SDP-06-100

### 10.0 STANDARD AND SPECIFICATIONS

SEDIMENT BASINS

### Definition

A temporary barrier or dam constructed across a drainage way or at other suitable locations to intercept sediment laden runoff. This barrier may be combined with excavation to achieve the required storage.

### Purpose

Sediment basins protect downstream properties and drainageways by trapping sediment and controlling the release of stormwater runoff.

#### Wet and Dry Storage

The minimum storage volume requirement for sediment basins is 3600 cubic feet per acre of contributory drainage area. The basin storage volume of 3600 cubic feet per acre shall be divided equally into "dry" or dewatered storage and "wet" or retention storage. Basins shall be dewatered to the wet pool elevation corresponding to 1800 cubic feet of storage per acre of drainage area.

#### Conditions Where Practice Applies

A sediment basin is required to control runoff and sediment from large areas where sediment traps are not appropriate. Stormwater management ponds may be used as sediment basins provided that they meet the requirements of this section and that the construction sequence addresses converting the sediment basin to the permanent stormwater management pond.

#### Conditions of Use

This standard applies to the installation of temporary sediment basins on sites where: (a) failure of the structure would not result in less of life, damage to homes or buildings, or interruption of use or service of public roads or utilities; (b) the drainage area does not exceed 100 acres; (c) the maximum embankment height does not exceed 15 feet measured from the natural ground to the embankment top along the centerline of embankment; and (d) the basin is to be removed within 36 months after the beginning of construction of the basin. Where these criteria cannot be met, the structure shall be designed to conform with the Natural Resources Article, Title 8, Subtitle 8, Annotated Code of Maryland or Maryland SCS Standards and Specifications No. 378 for Ponds. The total volume of permanent sediment basins shall equal or exceed the capacity requirements for temporary basins contained herein.

### C-10-1

Construction Specifications

shall be cleared of all brush, trees, and other objectionable materials.

the backfilling-compaction operations. For dewatering see Section D.

the design height to allow for settlement.

erosion free during the life of the basin.

hazards of soft sediment and floodwater.

+ 0.2 feet.

1. Site Preparation: Perimeter sediment control devices must be installed prior to clearing and

grubbing. Areas where the embankment is to be placed shall be cleared, grubbed, and stripped of

topsoil to remove trees, vegetation, roots or other objectionable material. The pool area shall not be

cleared until completion of the dam embankment unless the pool area is to be used for borrow. In

order to facilitate clean-out and restoration, the pool area (measured at the top of the pipe spillway)

2. Cut-off Trench: A cut-off trench shall be excavated along the centerline of earth fill embankments.

The minimum depth shall be four feet. The cut-off trench shall extend up both abutments to the riser

crest elevation. The minimum bottom width shall be two feet, but wide enough to permit operation

of excavation and compaction equipment. The side slopes shall be no steeper than 1:1. Compaction

requirements shall be the same as those for the embankment. The trench shall be dewatered during

3. Embankment: The fill material shall be taken from approved areas shown on the plans. It shall

be clean mineral soil free of roots, woody vegetation, oversized stones, rocks, or other objectionable

material. Relatively pervious materials such as sand or gravel (Unified Soil Classes GW, GP, SW

& SP) or organic materials (Unified Soil Classes OL and OH) shall not be placed in the embankment.

Areas on which fill is to be placed shall be scarified prior to placement of fill. The fill material shall

contain sufficient moisture so that it can be formed by hand into a ball without crumbling. If water can be squeezed out of the ball, it is too wet for proper compaction. Fill material shall be placed in

six-inch to eight-inch thick continuous lifts over the entire length of the fill. Compaction shall be

obtained by routing and hauling the construction equipment over the fill so that the entire surface of

each layer of the fill is traversed by at least one wheel or tread track of the equipment or by the use

of a compactor. The embankment shall be constructed to an elevation 10 percent higher than

4. Principal Spillway: Steel risers shall be securely attached to the barrel or barrel stub by welding

the full circumference making a watertight structural connection. Concrete risers shall be poured with

the principal spillway in place or precast with voids around the principal spillway filled with concrete

or shrink proof grout for watertight connection. The barrel stub must be attached to the riser at the

same percent (angle) of grade as the outlet conduit. The connection between the riser and the riser

base shall be watertight. All connections between barrel sections must be achieved by approved

watertight band assemblies. The barrel and riser shall be placed on a firm, smooth foundation of

impervious soil as the embankment is constructed. Breaching the embankment to install the barrel

is unacceptable. Pervious materials such as sand, gravel, or crushed stone shall not be used as

backfill around the pipe or anti-seep collars. The fill material around the pipe spillway shall be

placed in four inch lifts and hand compacted under and around the pipe to at least the same density as the adjacent embankment. A depth of 1.5 times the pipe diameter (min.) shall be backfilled over

5. Emergency Spillway: The emergency spillway shall be installed in undisturbed ground. The

achievement of planned elevations, grades, design width, entrance and exit channel slopes are critical

to the successful operation of the emergency spillway and must be constructed within a tolerance of

6. Vegetative Treatment: Stabilize the embankment in accordance with the appropriate vegetative

Standard and Specifications immediately following construction. In no case shall the embankment

remain unstabilized for more than seven (7) days. Once constructed, the top and outside face of the

embankment shall be stabilized with seed and mulch. The remainder of the interior slopes should be

stabilized (one time) with seed and mulch upon basin completion and monitored and maintained

7. Safety: Local requirements concerning fencing and signs shall be met, warning the public of

8. Maintenance: Repair all damage caused by soil erosion and construction equipment at or before

the end of each working day. Sediment shall be removed from the basin when it reaches the specified

distance below the top of the riser as shown on the riser. This sediment shall be placed in such a

manner that it will not erode from the site. The sediment shall not be deposited downstream from

9. Final Disposal: When temporary structures have served their intended purpose and the

contributing drainage area has been properly stabilized, the embankment and resulting sediment

deposits are to be leveled or otherwise disposed of in accordance with the approved sediment control

plan. The proposed use of a sediment basin site will often dictate final disposition of the basin and

any sediment contained therein. If the site is scheduled for future construction, then the basin

material and trapped sediments must be removed and safely disposed of and the basin shall be

backfilled with a structural fill. When the basin area is to remain open space, the pond may be

10. Conversion to Stormwater Management Structure: After permanent stabilization of all disturbed

contributory, drainage areas, temporary sediment basins, if initially built and certified to meet

permanent standards, may be converted to permanent stormwater management structures. To convert

the basin from temporary to permanent use, the outlet structure must be modified in accordance with

approved stormwater management design plans. Additional grading may also be necessary to provide

the required storage volume in the basin. Conversion can only take place after all disturbed areas

have been permanently stabilized to the satisfaction of the inspection authority and storm drains

the embankment, adjacent to a stream or floodplain. Disposal areas must be stabilized.

pumped dry (using methods in Section D - Dewatering), graded, and back filled.

the principal spillway and hand compacted before crossing it with construction equipment.

### 1994

#### Design Criteria

Design and construction shall comply with state and local laws, ordinances, rules and regulations.

1. Location The sediment basin should be located to obtain the maximum storage benefit from the terrain and for ease of cleanout of the trapped sediment. It should be located to minimize interference with construction activities and construction of utilities. Whenever possible, sediment basins should be located so that storm drains may outfall or be diverted into the basin.

2. Volume of the Basin The volume of the sediment basin, as measured from the bottom of the basin to the elevation of the principal spillway crest shall be at least 3600 cubic feet per acre of total drainage area (134 cubic yards). This 3600 cubic feet of storage is approximately equal to 1 inch of runoff per acre of drainage area. The sediment basin storage volume of 3600 cubic feet minimum per acre shall be divided equally into "dry" or dewatered storage and "wet" or retention storage. See Basin Draw-Down for dewatering criteria.

Sediment basins shall be cleaned out when the basia is filled with sediment to 900 cf/acre of total drainage area. Cleanout shall be performed to restore the original design volume to the hasin. The elevation corresponding to the maximum allowable sediment level shall be determined and shall be stated in the design data as a distance below the top of the riser. The distance between the top of the riser and the cleanout elevation shall be clearly shown on the riser, above the pool elevation.

3. Surface Area Basins shall be designed so that the ratio of acres of surface area to cubic feet/second of discharge (from a 10 year storm) is greater than or equal to 0,0035. The surface area shall be measured at the design high water elevation.

#### 4. Shape of the Basin It is recommended that the designer of a sediment basin strive to incorporate the following features:

- a. Length to width ratio greater than 2:1, where length is the distance between the inlet and outlet.
- b. A wedge shape with the inlet located at the narrow end. c. In situations where the above conditions cannot be met, baffles designed to maximize detention time may be required.
- d. The dimensions necessary to obtain the required basin volume and surface area shall be clearly shown on the plans to facilitate plan review, construction and inspection.

5. Inflow Protection Whenever the inflow to the basin is not stabilized refer to the inflow protection specifications. Inflow protection provides safe conveyance of concentrated runoff into temporary sediment basins to prevent erosion. Inflow protection shall meet or exceed the practices found in Section B of these Standards and Specifications. Points of runoff entry should be located as far away from the riser as possible, to maximize travel time in conjunction with dikes, swales or other water control devices as warranted by site conditions.

C-10-2

Basin Volume design

237,211 ft<sup>3</sup>

6 Elevation corresponding to min. required volume of basin (riser crest elevation 148.70 ft

Spillway Design

Principal Spillway (Ops) (See Detail 11)

Riser Height

54 in ; Trash Rack Height =

NOTE: A table showing design data shall be included on the plan for each basin.

146.05 ft

145.53 ft

2 To convert ft3 to yd3, divide ft3 by 27. To convert ft2 to yd2, divide ft2 by 9

0 yd3) to obtain required capacity

78.82 cfs (peak discharge from 10-yr, 24-hr storm event, attach computations)

24 in. Note: QP, must equal or exceed Design QP

x(length correction factor 1.0)

1-YR., 24 HR. EXTENDED DETENTION DESIGN

3.7 ft;

 $1.800 \text{ ft}^3/\text{ac} \times 16.55 \text{ ac.} = 29.790 \text{ ft}^3$ 

 $900 \text{ ft}^3/\text{ac} \times 16.55 \text{ ac.} = 14.895$ 

drainage = 101,510 ft<sup>3</sup>

(58,392+41,731)

7.88 cfs

8.97 Acres

0.20 Hrs.

2.60 Inches

0.63 Inches

1.97 Inches

0.13 Inches

850.00 csm/in

0.52 CFS

0.96 Ac.Ft.

0.65

23.52 CFS

0.05

0.022

1 Also see Surface Area Design #30, this form

8 Distance from riser crest elevation to permanent pool elevation

12 Design Principal Spillway (Barrel) discharge, Design Qp, =

4.58 ft; Barrel length =

36

(min. 10% of 10 year peak or 8" Diameter Pipe)

10 Distance from riser crest elevation to cleanout elevation 3.17

1 Min, required vol. =  $3,600 \text{ ft}^3/\text{ac} \text{ x}$ 

2 Actual Volume of basin =

4 Vol. at dewatening elev. =

5 Vol. of basin at cleanout=

9 Basin cleanout elevation

3 Excavate

14 Barrel Diam.

BASIN#1

RCN =

la / P =

Tc≍

15 Riser Diameter

Riser Head (h) =

16 Trash Rack Diam.

Drainage Area, A =

1-yr. rainfall depth, P =

S=1000/RCN - 10 =

Fig D 11.2-MDE

From TR\_55 Manual, Eqn. 2.3,

Peak Discharge, qi = qu.A.qa =

Runoff depth,  $qa = [(P-0.2S)^2]/(P+0.8S) =$ 

From Tc & P, find unit peak discharge, qu =

T=24 hours

 $Vs/Vr = 0.683-1.43(qo/qi)+1.64(qo/qi)^2-0.804(qo/qi)^3 =$ 

Extended Detention Storage Volume, Vs = (Vs/Vr)xgaxA/12 =

qu & extended detention time T, qo/qi =

Peak Outflow discharge, qo=(qo/qi)xqi =

Initial abstraction, la = 200/RCN - 2 =

QP, = Q (from Table 13 or 14)

POST DEVELOPMENT CONDITION

### 6. Embankment The embankment plans shall include elevations at the top of earth fill at constructed

7. Side Slopes The combined upstream and downstream side slopes of the settled embankment shall not be less than five horizontal to one vertical (5:1) with neither slope steeper than 2:1. Slopes must be designed to be stable in all cases.

8. Top Width For dam embankments up to ten (10) feet, the top will be level and a minimum of eight (8) feet in width. For embankments between ten (10) feet and fifteen (15) feet, the top width will be ten (10) feet.

9. Spillway Design Runoff shall be computed by the method outlined in Chapter 2. Estimating Runoff, "Engineering Field Manual for Conservation Practices" available in the Soil Conservation Service offices, or by TR-55, Urban Hydrology. Runoff computations shall be based upon the worst soil-cover conditions expected to prevail in the contributing drainage area during the anticipated effective life of the structure. The combined capacities of the principal and emergency spillways shall be sufficient to pass the "routed" peak rate of runoff from a 10-year frequency storm.

10. Principal Spillway A spillway shall be provided which consists of a vertical pipe or box type riser joined (watertight connection) to a pipe (barrel) which shall extend through the embankment and outlet beyond the downstream toe of the fill. The storage volume required shall be measured from the riser crest elevation to the bottom of the basin. The minimum size of the barrel shall be what is required to pass 10% of the 10 year storm or 8 inches in diameter whichever is larger.

- a. Crest elevation The crest elevation of the riser shall be a minimum of one foot below the elevation of the control section of the emergency spillway.
- h. Watertight Riser and Barrel Assembly The riser and all pipe connections shall be completely watertight except for the inlet opening at the top or dewatering openings and shall not have any other holes, leaks, rips or perforations.
- c. Basin Draw-down The water in the basin from the riser crest to the permanent pool shall be drawn down over a 10 hour period through an internal orifice in a draw-down device. A draw-down device shall be included in the sediment basin plans submitted for approval and shall be installed during construction of the basin. Design of a draw-down device shall be required if an orifice size other than those provided in Table 11 is to be used. Design of perforations in the horizontal or vertical dewatering device is required. Draw-down shall be done in such a manner as to remove the clean water without removing sediment that has settled out or floating debris. This shall be done by constructing a perforated horizontal or vertical draw-down device with an internal orifice to control discharge. If perforating the riser is desired as a draw-down device, the minimum detention time shall be 10 hours, however, the riser must be wrapped with 1/2" hardware cloth and Geotextile Class E or C17. Other methods may be used as long as detailed drawings are provided on the approved sediment control plans.

**Emergency Spillway (Qes)** 

Anti-Seep Collar Design (If Required)

Design Elevations

Surface Area Design

Draw-down Device

6.5 ft

 $A_t = (# \text{ of perforation/foot}) \text{ (perforation area ft2)(perforated section length ft.)}$ 

6 in. (From Table 11)Permanent pool

16.55 Acres

0.30 Hrs.

5.10 Inches

0.82 Inches

4.23 Inches

0.16 Inches

720.00 csm/in

78.82 CFS

0.03

92

10-YR., 24 HR. EXTENDED DETENTION DESIGN

152,00 ft

ft; z = 3:1; pipe slope = 2%;

( No emergency spillway )

17 Refer to Table 27.

19 Entrance channel slope

20 Exit channel slope

projection =

Ls=Y(Z+4)

(1-4So)

23 Riser Crest =

24 Design High Water =

26 Min. settled top of dam =

25 Emergency Spillway Crest =

27 Permanent pool = 146.05 ft

28 Bottom of basin = 145.00 ft

31 Draw-down device orifice diameter =

Ao = Internal orifice area =

Draw down pipe length =

Drainage Area, A =

10-yr, rainfall depth, P =

S=1000/RCN - 10 =

From TR 55 Manual, Eqn. 2.3,

Peak Discharge, qi = qu.A.qa =

Runoff depth,  $qa = [(P-0.2S)^2]/(P+0.8S) =$ 

From Tc & P, find unit peak discharge, qu =

Initial abstraction, la = 200/RCN - 2 =

BASIN#1

RCN =

la / P =

32 A<sub>t</sub> = Total area of perforations > 4A0

POST DEVELOPMENT CONDITION

29 Draw - down orifice invert = 146.05 ft

30 Min. basin surface area: SA > 0.0035 x Q10 =

18 Width

17 Emergency spillway cap., Q,, = Q10 - Q,5 =

2 collars,

P1=0.075Ls

148.7 ft

0,8 ft<sup>2</sup>

NOTE: If the basin is to be converted to a stormwater management pond the riser should not be perforated. If PVC pipe is used for the principle spillway then the concrete pipe chart will be used for hydraulic design. Use manufacturer's specification for loading.

- d. Anti-vortex Device and Trash Rack An anti-vortex device and trash rack shall be securely installed on top of the riser and shall be the concentric type meeting these specifications for corrugated metal pipe risers and shall meet MD 378 for all others.
- e. Base. The riser shall have a base attached with a waterlight connection and shall have sufficient weight to prevent flotation of the riser. Steel base plates of at least 1/4" thickness shall be twice the diameter of the riser and shall have at least 2' of compacted earth, stone or gravel placed over it to prevent flotation. Concrete riser bases shall be twice the diameter of the riser, a minimum of 18" thick, contain steel reinforcement as shown in Detail 15, and shall have the riser embedded 9" minimum. Risers over 10 feet in height require that anti-floatation calculations be performed and shall be based on the following:
- 1. The riser shall be analyzed for floatation assuming all orifices and pipes are plugged.
- 2. The factor of safety against floatation shall be 1.2 or greater.
- f. Anti-seep Collars Anti-seep collars shall be installed around all conduits through earth fills of impoundment structures according to the following criteria:
- 1. Collars shall be placed to increase the seepage length along the conduit by a minimum of 15 percent of the pipe length located within the saturation zone.
- 2. Collar spacing shall be between 5 and 14 times the vertical projection of each collar.
- 3. Collars should be placed within the saturation zone. In cases where spacing limit will not allow this, at least one collar shall be placed in the saturation zone.
- 4. All anti-seep collars and their connections shall be watertight.
- 5. Anti-seep collars shall be placed a minimum of two feet from pipe joints.
- 6. Anti-seep collars must have 2' minimum projection.

SEDIMENT BASIN

"d o INTERNAL

DIAMETER(IN.)

see table 1.

ORIFICE

g. Outlet - An outlet shall be provided, including a means of conveying the discharge in an erosion free manner to an existing stable channel. Where discharge occurs at the property line, drainage easements will be obtained in accordance with local ordinances. Adequate notes and references concerning the easements will be shown on the erosion and sediment control plan. Protection against scour at the discharge end of the pipe spillway shall be provided. See Section 18.0 Standard and Specifications for Rock Outlet Protection.

C-10-4

HORIZONTAL DRAW- DOWN DEVICE

DRY STORAGE

WET STORAGE

TABLE 1

Basin 1

d₀ <u>6</u> IN

B 3.5 LF

C 1 IN D 4 FT

ROWS 2 NO

PERFORATED CMP

WATER TIGHT

BASIN BOTTOM

SOLID SOLID

CONSTRUCTION SPECIFICATIONS

perforated portion of draw-down pipe. Include o watertight end cap.

wet storage elevation. If necessary provide multiple draw-down pipes.

ground and securely attached to the pipe with 12 gauge minimum wire

1.The perforations shall be 1" in diameter spaced 3" on center. Allow no perforations

within 6" of weir connection. Provide internal orifice plate if do is smaller than C,

2.Securely attach 1/2" hardware cloth then Geotextile Class E fabric around entire

4.Provide support of draw-down pipe > Use a minimum of 3 support rods (#6

5.An alternative method is to stake both sides of draw-down device with 1" steel

angle, or 1" by 4" square or 2" round wooden posts set 3' minimum into the

rebar) welded to riser and pipe. both sides and bottom of draw—down pipe.

3.Draw-down pipe shall not exceed 7 1/2' in total length and shall be placed at the

11. Emergency Spillways The entire flow area of the emergency spillway shall be constructed in undisturbed ground (not fill). The emergency spillway cross-section shall be trapezoidal with a minimum bottom width of eight feet. This spill way channel shall have a straight, level control section of at least 25 feet in length. The exit changel section shall have sufficient slope such that the discharge capacity of the spillway is not hindered in any way and allows the discharge to be released at a non-erosive velocity.

- a. Capacity The minimum capacity of the emergency spillway shall be that required to pass the peak rate of runoff from the 10-year 24-hour duration storm, less any reduction due to flow in the principal spillway. Emergency spillway diracnsions may be determined by using the method in Detail 12 and Table 14
- b. Velocities The velocity of flow in the exit channel shall not exceed 5 feet per second for vegetated channels. For channels with erosion protection other than vegetation, velocities shall be within the non-crosive range for the type of protection used.
- c. Freeboard Freeboard is the difference between the design high water elevation in the emergency spillway and the top of the settled embankment. Freeboard shall be at least one foot.

12. Sediment Disposal The sediment basin plans shall indicate the method(s) of disposing of the sediment removed from the basin. The sediment shall be placed in such a manner that it will not erode from the site. The sediment shall not be deposited downstream from the basin or adjacent to a stream or floodplain. Disposal sites must be considered in an approved sediment control plan. The sediment basin plans shall show the method of disposal of the sediment basin after the drainage area is stabilized, and shall include the stabilization of the sediment basin site. Sediment shall not be allowed to flush into a stream or drainage way. For dewatering methods see Section D.

### BASIN #1 **ELEVATION - STORAGE TABLE** FLEVATION SUR AREA DIFFERENCE | STORAGE | CLIM STORAGE | CLIM STORAGE

IN FT.	IN SQ.FT.	IN ELEV.	IN CU.FT.	IN CU.FT.	IN AC.FT.
mangroom a springs of a major providence with advantable contractions.	grade hade in the first transfer and the property of the description of the second of	An indicate which was not according to complete the control of	promptomerous species and a second species of the s	a sugarthanustra seugrusme Samuelam Samuelam Augustra melala seugra seguitar se	
145.00	26,785	0.00	0	0	0.000
145.53	27,925	0.53	14,608	14,608	0.335
146.00	28,900	0.47	13,240	27,848	0.639
146.05	28,963	0.05	1,447	29,294	0.673
148.00	32,900	1.95	60,316	89,611	2.057
148.70	34,301	0.70	23,520	113,131	2.597
149.00	34,900	0.30	10,380	123,511	2.835
150.00	36,900	1.00	35,900	159,411	3.660
152.00	40,900	2.00	77,800	237,211	5.446

BY THE DEVELOPER:

I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIS ON-SITE INSPECTION S BY THE HOWARD SIOL

DEVELOPER DATE	CONSERVATION	L.L	1-25-08
	SEVELOPER	- Hardward Control	DATE

BY THE ENGINEER:

I CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.

Jeryen	Pater	1/24/08
ENGINEER		DATE
······································	<del></del>	

THESE PLANS HAVE BEEN REVIEWED FOR THE HOWARD SOIL CONSEVATION DISTRICT AND MEET THE TECHNICAL COULTEMENTS FOR SOIL EROSION AND SEDIMENT

DURCES CONSERVATION SERVICE THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL

APPROVED : HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING.

2/3/08 CHIEF, DEVELOPMENT, ENGINEERING DIVISION

CHIEF, DIVISION OF LAND DEVELOPMENT DATE 🎉

DATE NO. REVISION OWNER (PAR. A) LITTLE PATUXENT REAL ESTATE LLC

OWNER (PAR. B) ROUTE 1 SELF STCRAGE LLC PO BOX 15080 CHEVY CHASE, MARYLAND 20825

26949 MILES RIVER ROAD

EASTON, MARYLAND 21601

DOUG CHAMBERLAIN CHAMBERLAIN CONSTRUCTION INC 3138 ROGERS AVENUE ELLICOTT CITY, MARYLAND 21043 TELEPHONE: 410-203-2460

DEVELOPER:

PARCEL 'A' OF THE A.H. SMITH SUBDIVISION PROPOSED FLEX SPACE BUILDING

PROJECT STORAGE USA FACILITY (PAR 'B') AND 04 - 210SCALE: NTS DATE: 01/23/08 DRAWN BY: \_

ROJECT NO.

TAX MAP 47, PARCEL A & E, GRID 18, P/O PARCEL 144

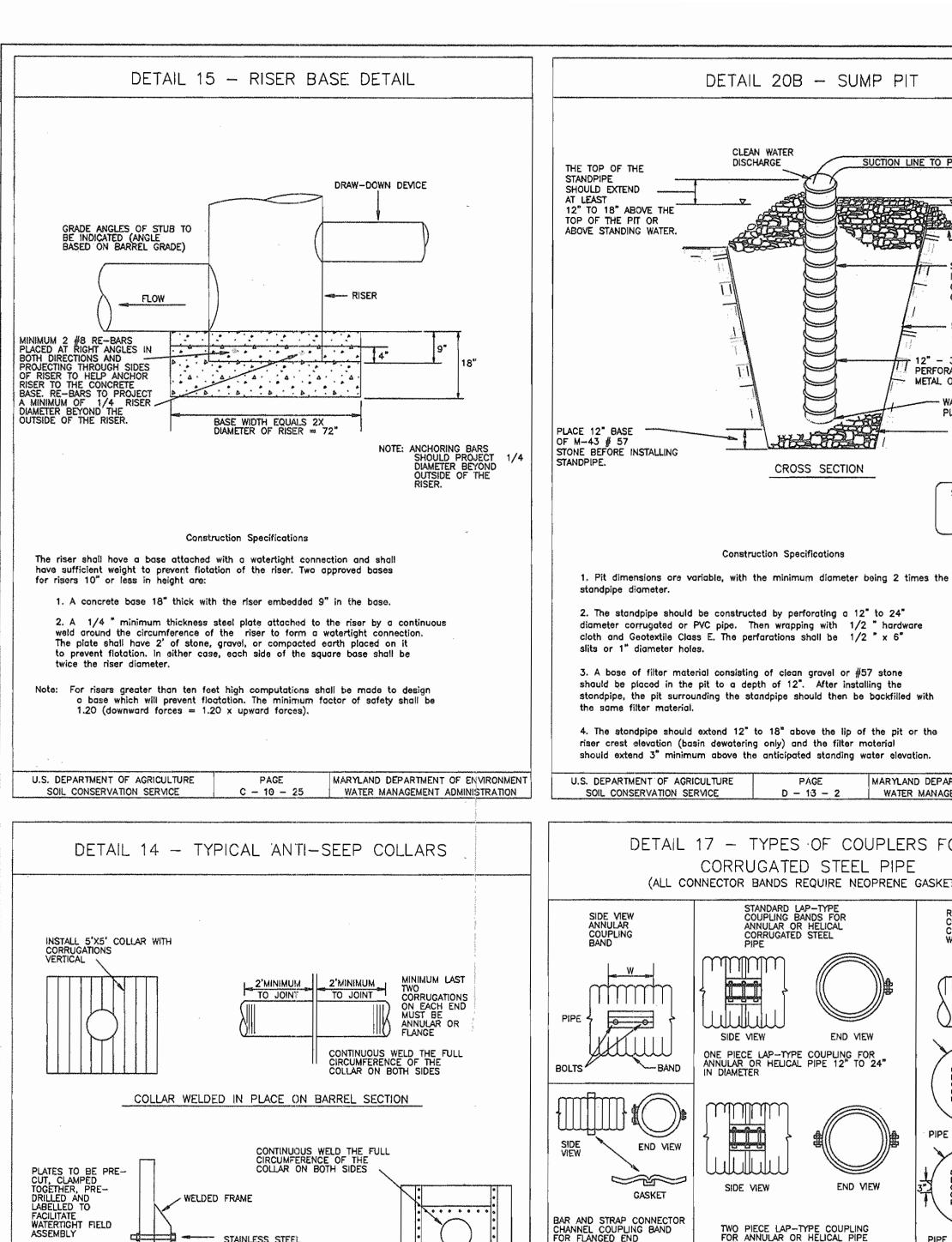
9450 ANNAPOLIS ROAD PHONE: (301) 306-3091 FAX: (301) 306-3092

CONTACT: SANJAY PATEL PHONE: 301-306-3091 x121

AB CONSULTANTS, INC. LANHAM, MARYLAND 20706

SELF-STORAGE FACILITY EXPANSION AND EROSION & SEDIMENT CONTROL NOTES AND DETAILS 6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND

CHECKED BY: \_ SHEET: 17 OF 25



. . . . . . . .

WATER MANAGEMENT ADMINISTRATION

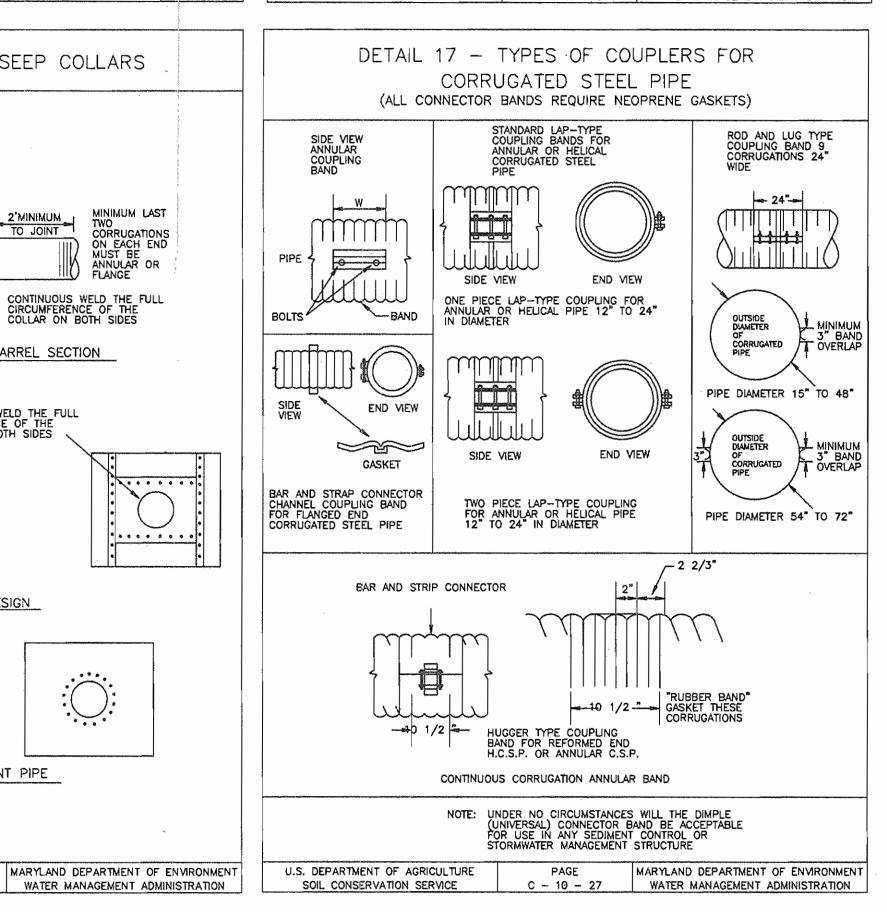
ANTI-SEEP COLLAR DESIGN

COLLAR FOR FLANGE JOINT PIPE

USE "MASTIK" OR EQUIVALENT BETWEEN PLATE AND FRAME

U.S. DEPARTMENT OF AGRICULTURE

SOIL CONSERVATION SERVICE



SUCTION LINE TO PUMP

\_EXISTING GROUND LINE

STANDPIPE WRAPPED

CLASS E

12" - 36" DIAMETER

METAL OR PVC PIPE

PERFORATED CORREGATED

WATERTIGHT CAP OR

AGGREGATE FILL

CLEAN GRAVEL OR

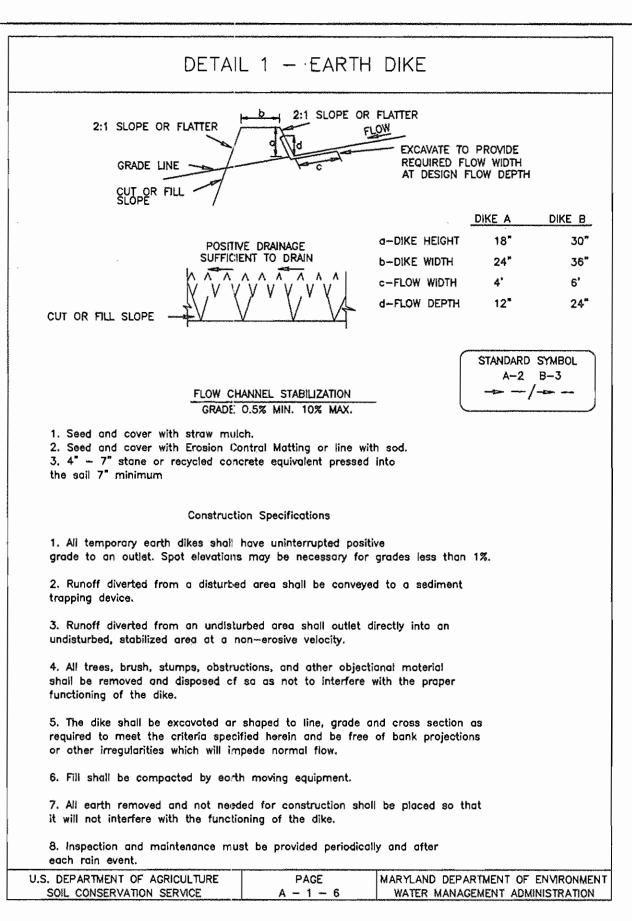
STANDARD SYMBOL

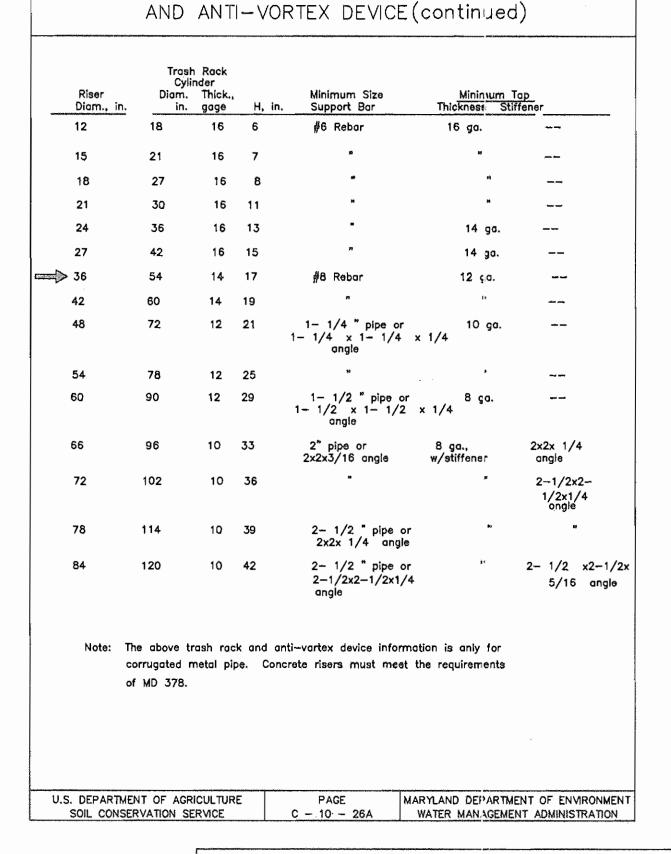
⊠ SP

MARYLAND DEPARTMENT OF ENVIRONMENT

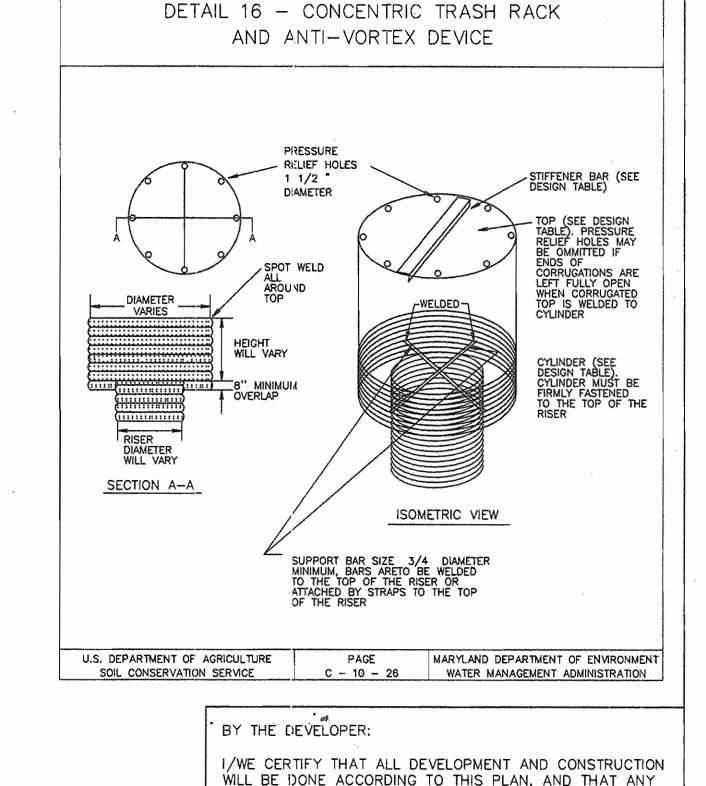
AASHTO M-43 # 57

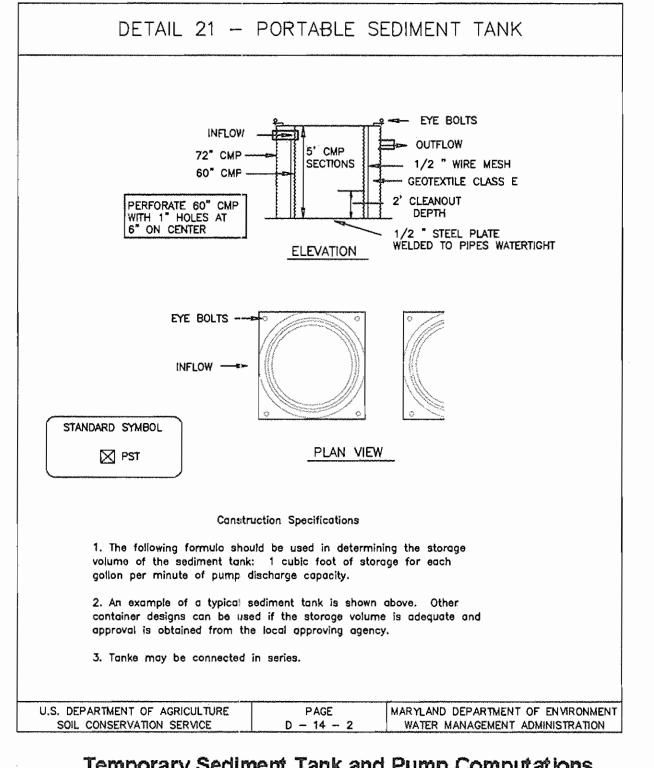
IN 1/2 " HARDWARE CLOTH AND GEOTEXTILE

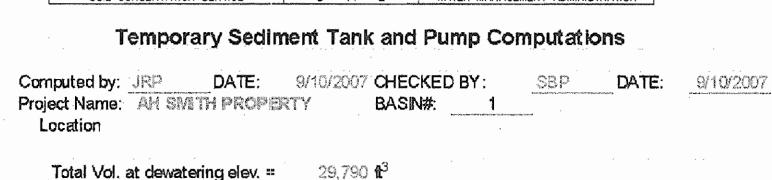




DETAIL 16 CONCENTRIC TRASH RACK







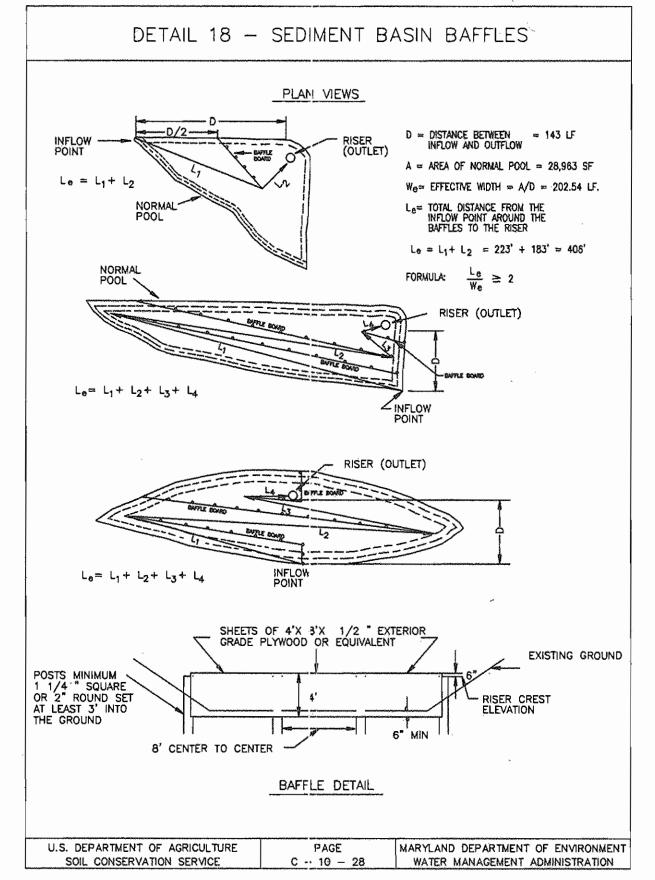
222,844 Gallons

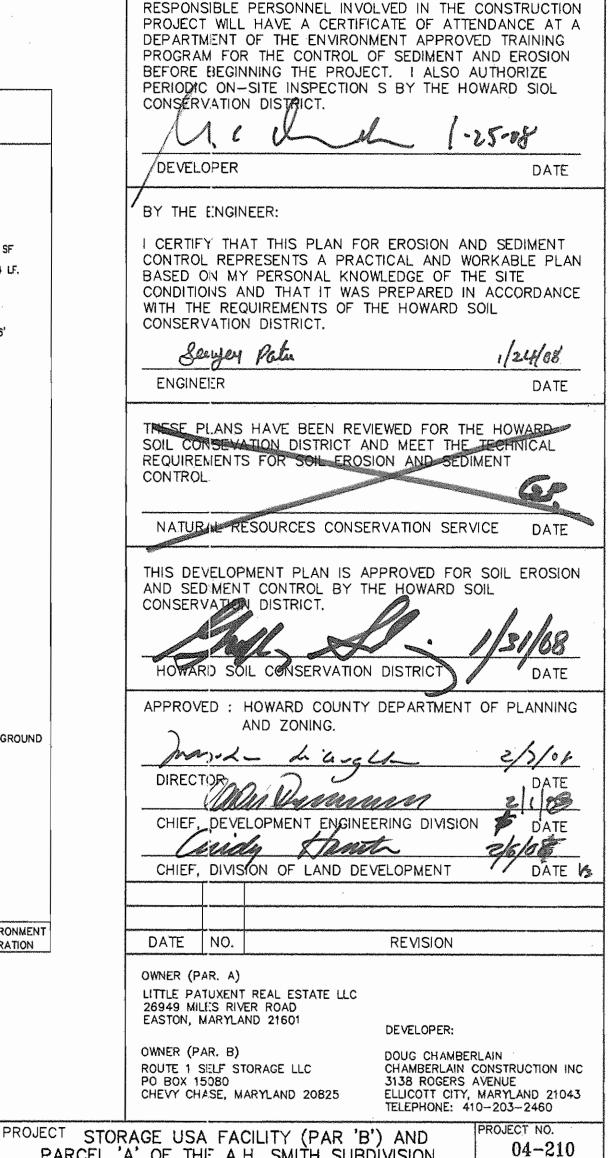
Tank Size 1 cubic foot of storage for each gallon per minute of pump discharge capacity. Pump size need to dewater in 4 HR. Pump Size = (222,844/4 hr) = 928.52 gallon/min = 1,000 GPM

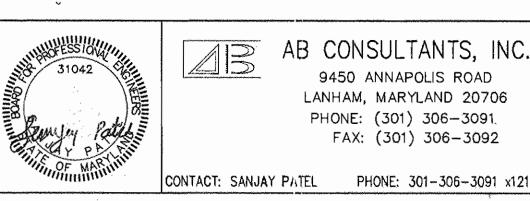
Provide Two Pump of 500 GPM and Four Sediment Tank.

1 Cu. Ft. = 7.4805 Gallons =

NOTE: CONTRACTOR HAS OPTION TO CHANGE PUMP SIZE AND SEDIMENT TANK BASED ON HIS NEEDS







AB CONSULTANTS, INC. 9450 ANNAPOLIS ROAD LANHAM, MARYLAND 20706 PHONE: (301) 306-3091

FAX: (301) 306-3092

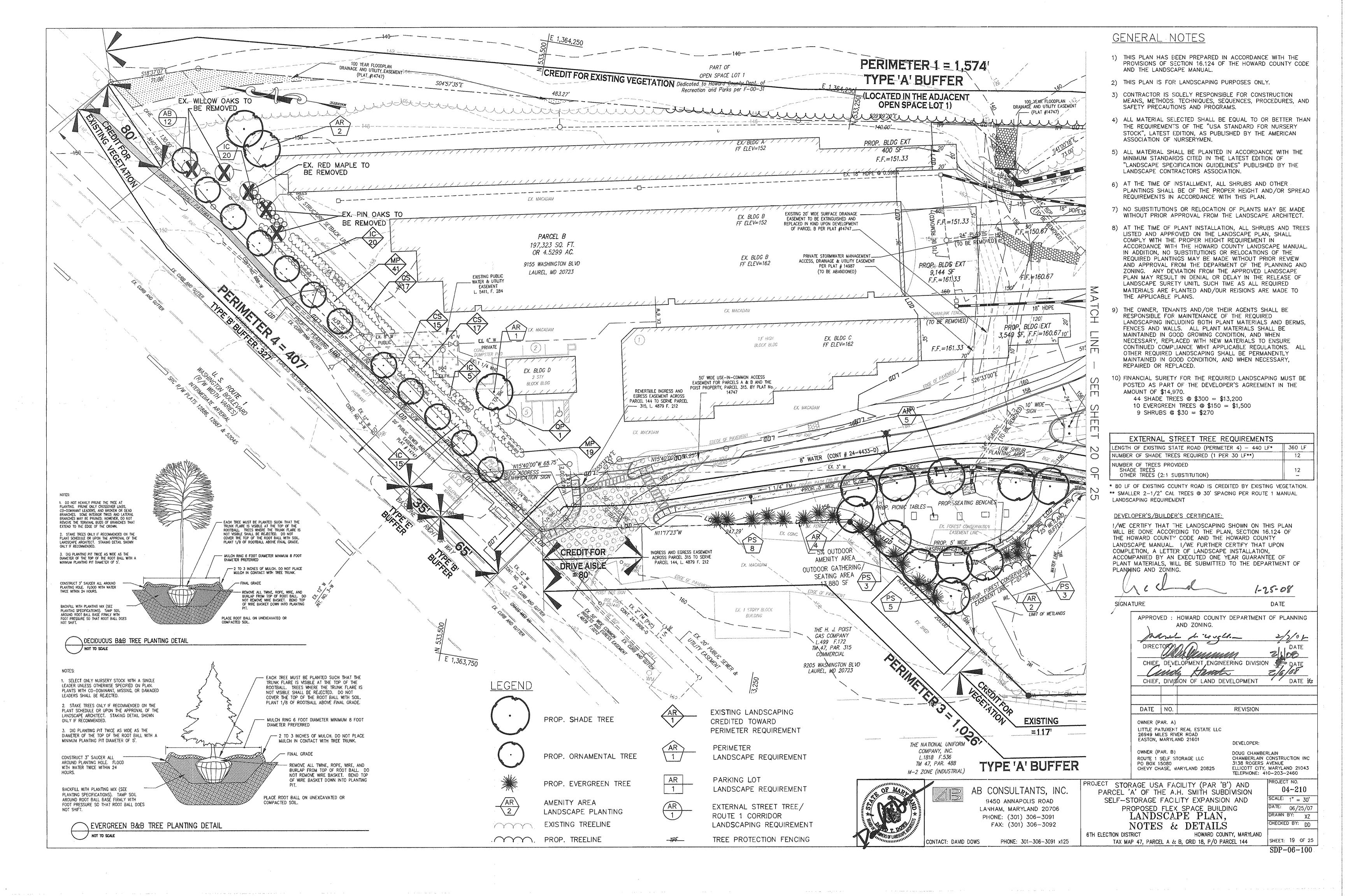
PARCEL 'A' OF THE A.H. SMITH SUBDIVISION

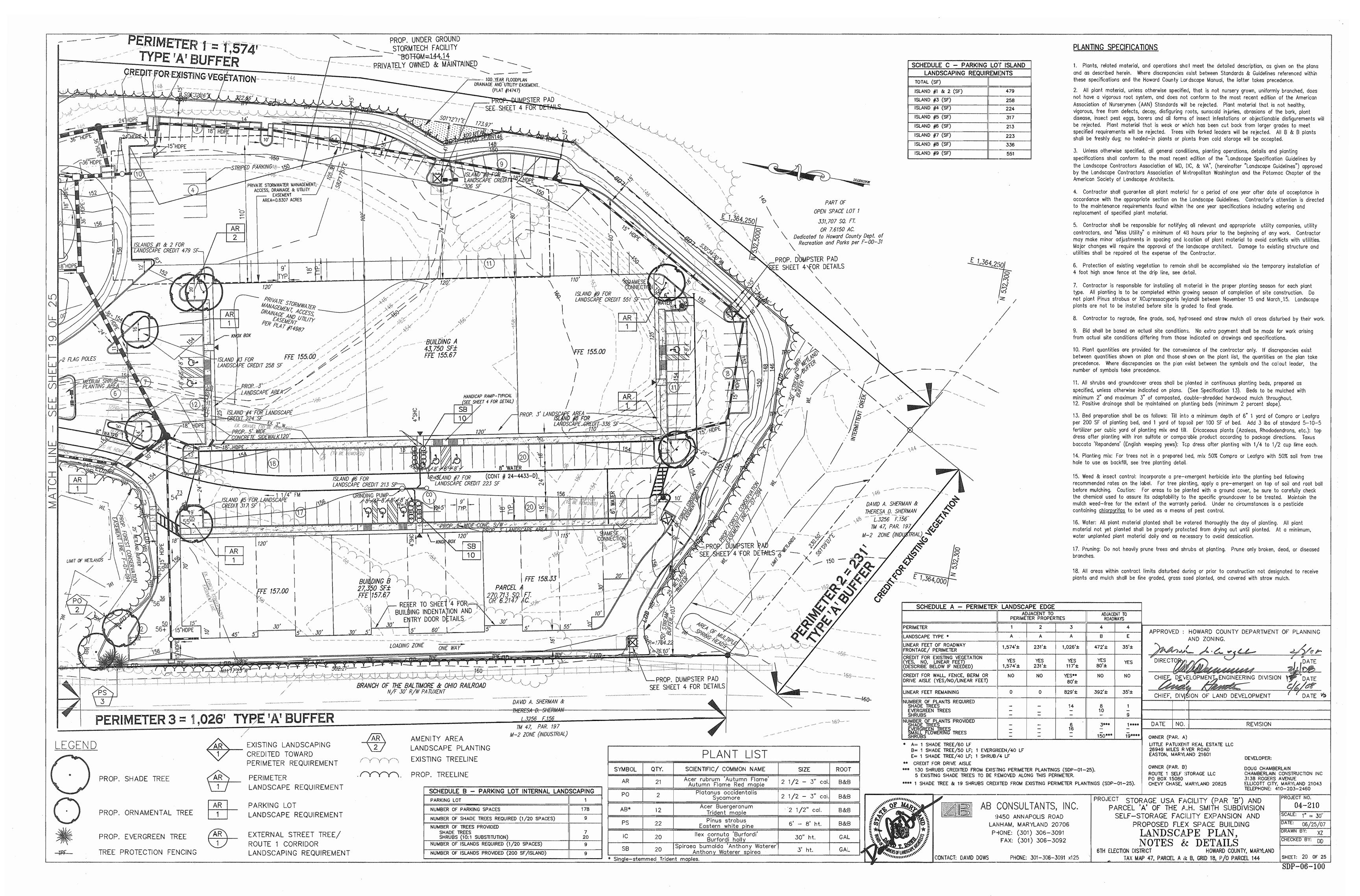
6TH ELECTION DISTRICT

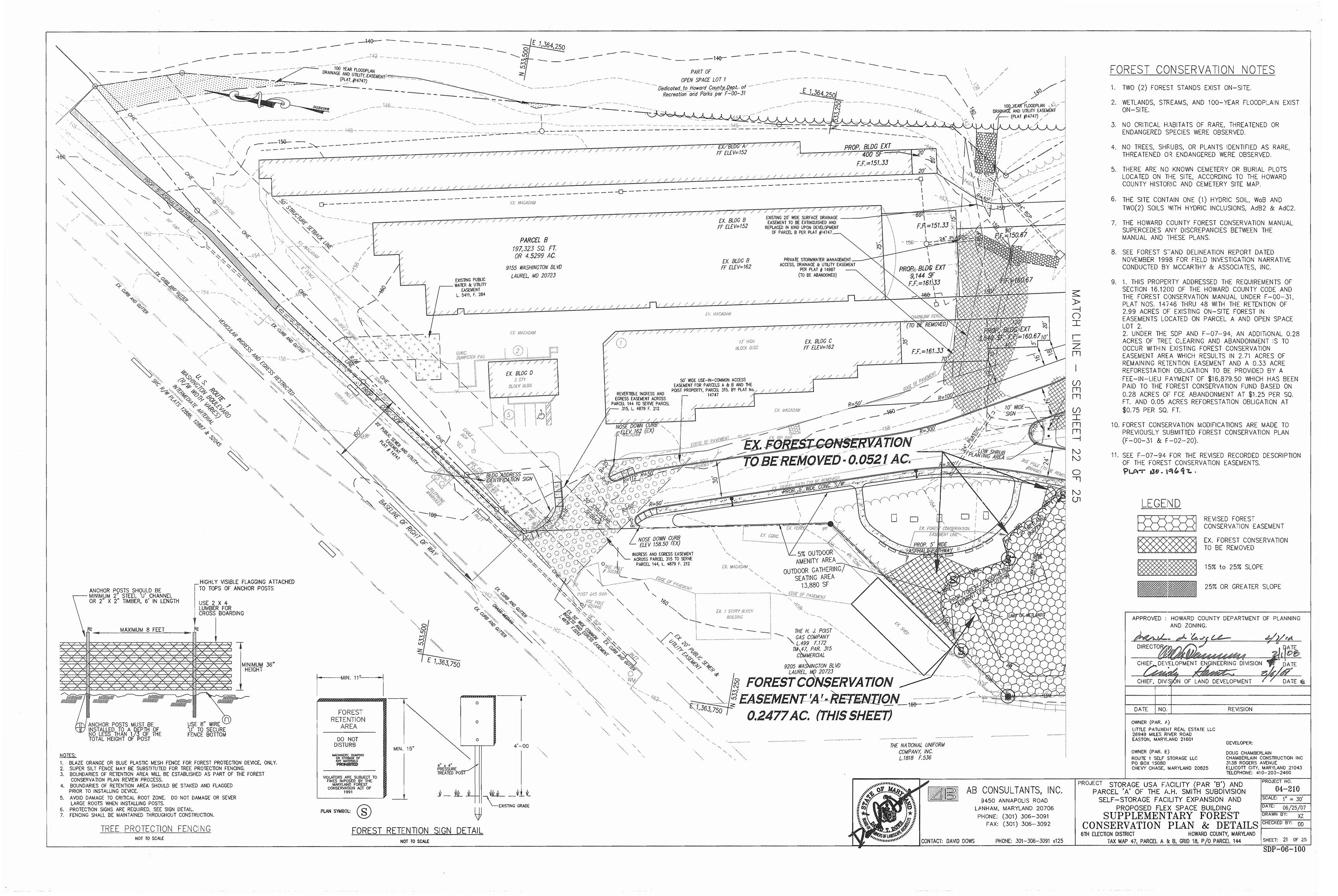
SELF-STORAGE FACILITY EXPANSION AND PROPOSED FLEX SPACE BUILDING EROSION & SEDIMENT CONTROL DETAILS CHECKED BY: SP

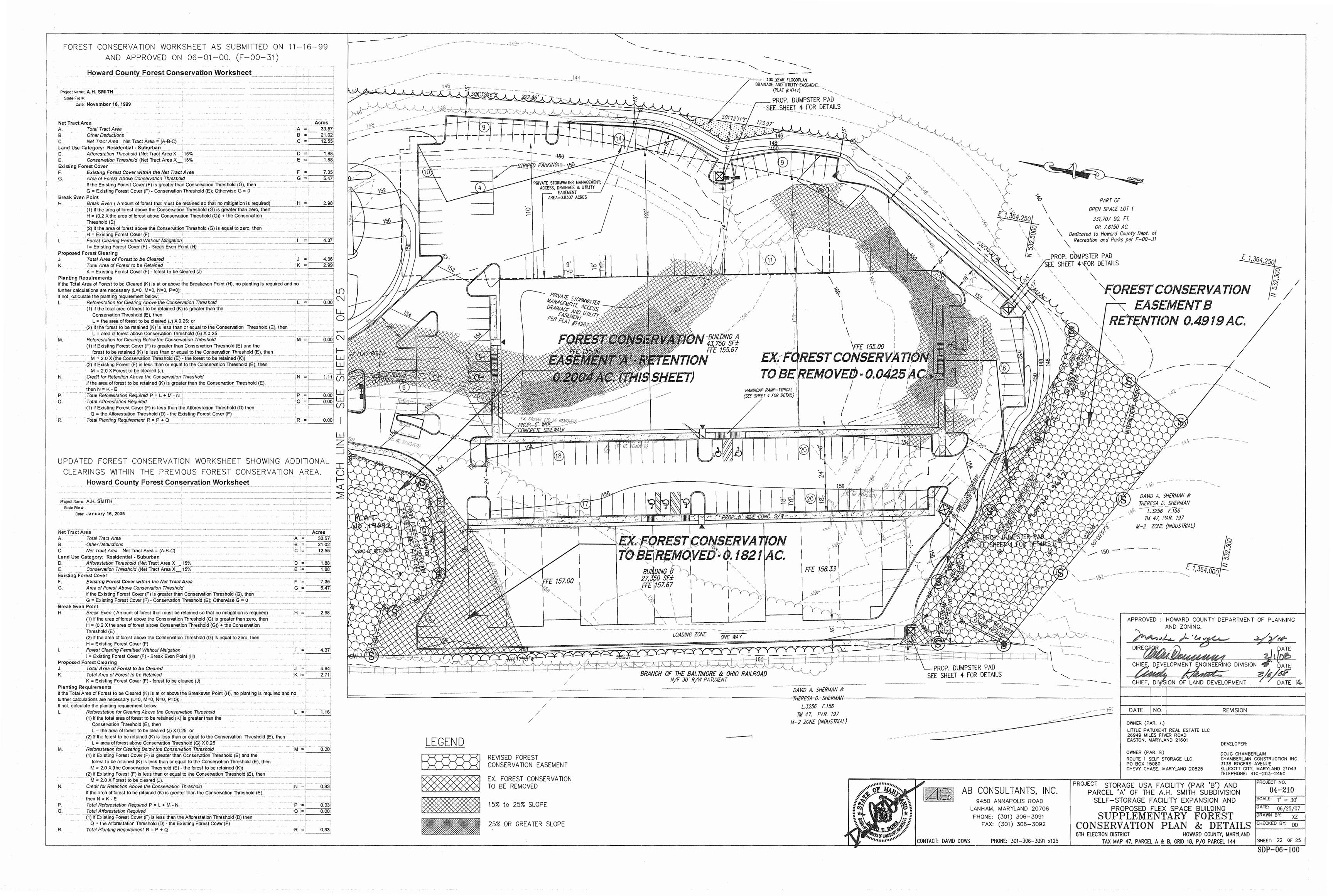
DATE: 01/23/08 DRAWN BY: KJ HOWARD COUNTY, MARYLAND TAX MAP 47, PARCEL A & B, GRID 18, P/O PARCEL 144 SHEET: 18 OF 25 SDP-06-100

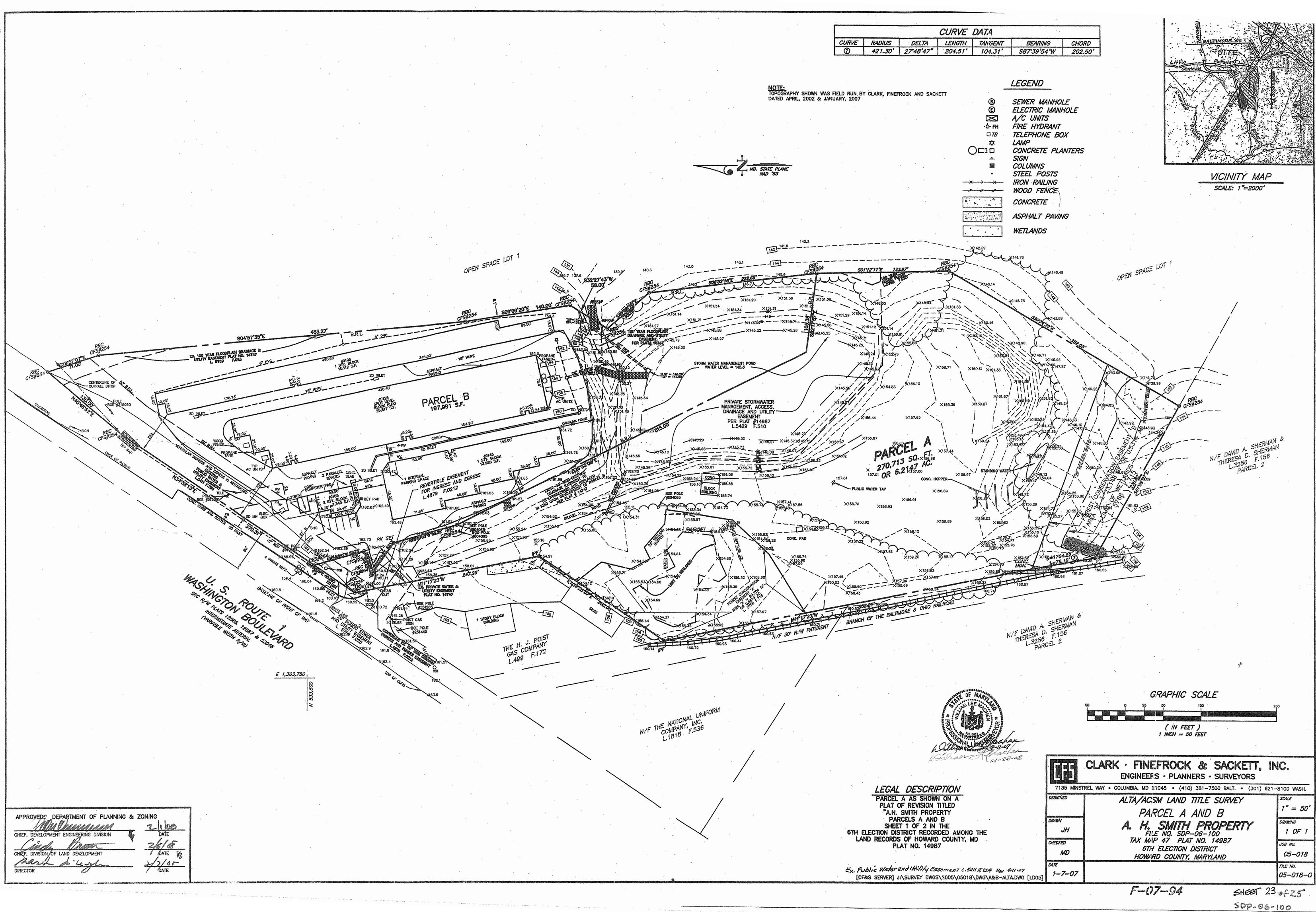
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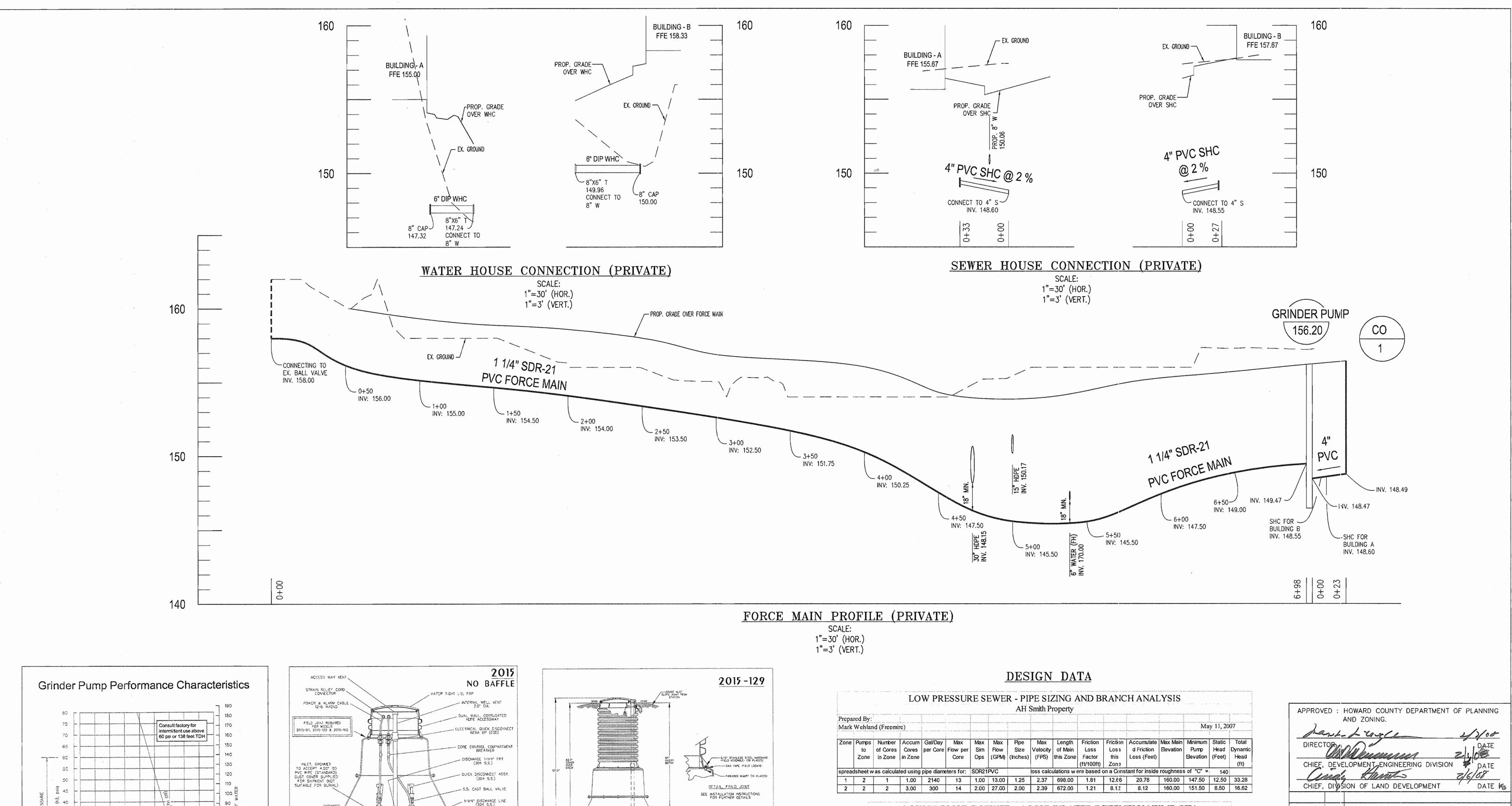


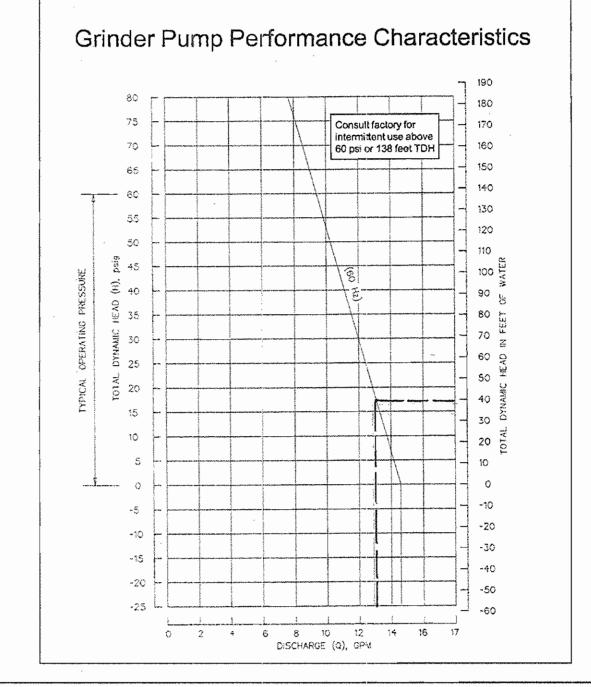


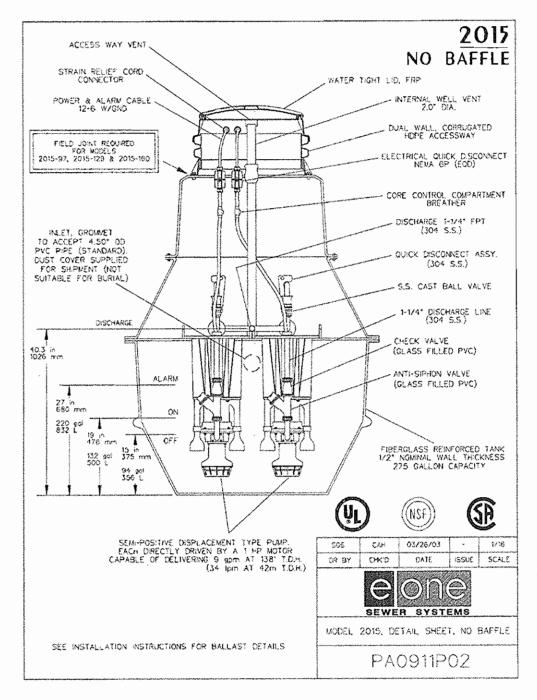


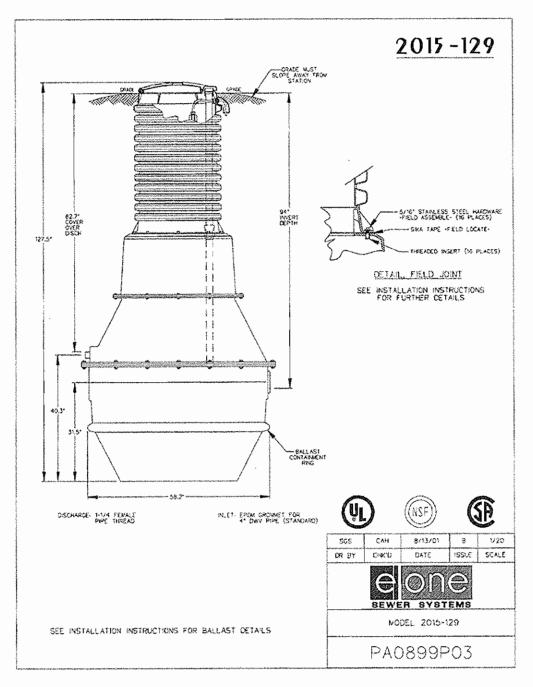






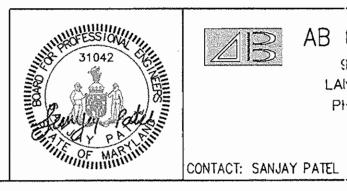


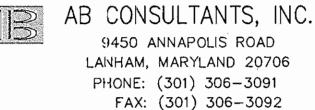




Prepared By	<b>/</b> :	Again talify		i	er de company					
Mark Wehl	and (Freemi	re)	ane et a fallamente et agentistis (sept de misse et et en fallament et en fallament et en fallament et en fall Benedikter et en fallament et	egites, esta accominada principales per de mante e recedidade en mente de másiles.  Ante esta contrata e e escuela e e escuela en mente e actual e en esta contrata en en entrata en en entrata	ig menering digitalism, angungan sugah, remeningan grassumlan gang digitalism digitalis	eligini i menance ana yani di Quala dan dan dan dan kenana masabi dan dan selemberah dan selemberah dan selemb Selemberah dan selemberah dan	artina jarah digili kaja gerakan kalendari yang bana, dapish angan dapan gerakan jarah jarah kalendari yang da An an	iganing e e e	May	1, 2007
Zone	Pumps to	Accumulate	Existing	Gallons per	Length of	Capacity of	Average	Average	Average	Accumulate
Number	Zone	d Total of Cores this Zone	Pipe Size	100 Lineal   Feet	Zone	Zone	Daily Flow	Fluid Changes per Day	Retention Time (Hr)	d Retention Time (Hr)
This spre	adsheet was	calculated us	ing pipe dian	neters for:	SDR21PVC	and the state of t	AMP			
1	2	1.00	1,25	9.14	698.00	63.82	2,140.00	33.53	0.72	1.82
2	2	3.00	2.00	18.84	672.00	126.62	2,740.00	21.64	1,11	1,11

APPROV		HOWARD COUNTY	DEPARTMENT	OF PLANNING
Ja	not.	Luze		2/2/00
DIRECT	TOP.	Menger	eA	DATE
CHIEF,	DEVE	LOPMENT ENGINEE	RING DIVISION	DATE PLANE
CHIEF,	DIVIS	ION OF LAND DEV	ELOPMENT	DATE KS
·		**************************************		
DATE	NO.		REVISION	
OWNER (P	,	DEAL FOTATE LLO		
26949 MIL EASTON, I	ES RIV		DEVELOPER:	
PO BOX 1	SELF \$1 5080	FORAGE LLC ARYLAND 20825	3138 ROGERS	CONSTRUCTION INC AVENUE MARYLAND 21043
		CILITY (PAR 'B' H. SMITH SUBD		PROJECT NO. 04-210





PHONE: 301-306-3091 x121

PROJECT S

PARCEL 'A' OF THE A.H. SMITH SUBDIVISION SELF-STORAGE FACILITY EXPANSION AND PROPOSED FLEX SPACE BUILDING SEWER PROFILE

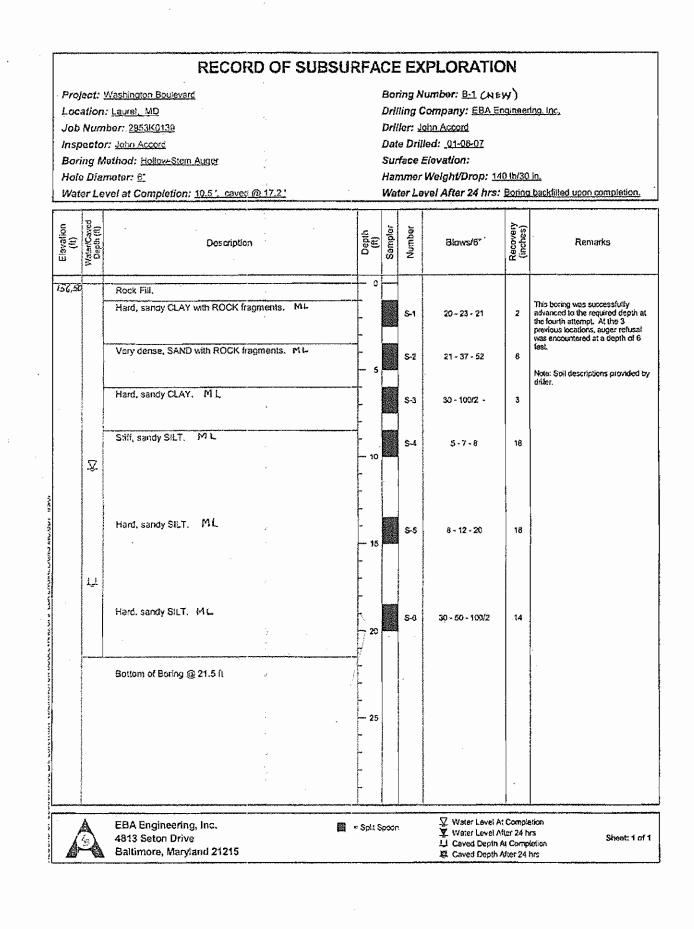
6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND TAX MAP 47, PARCEL A 8: B, GRID 18, P/O PARCEL 144

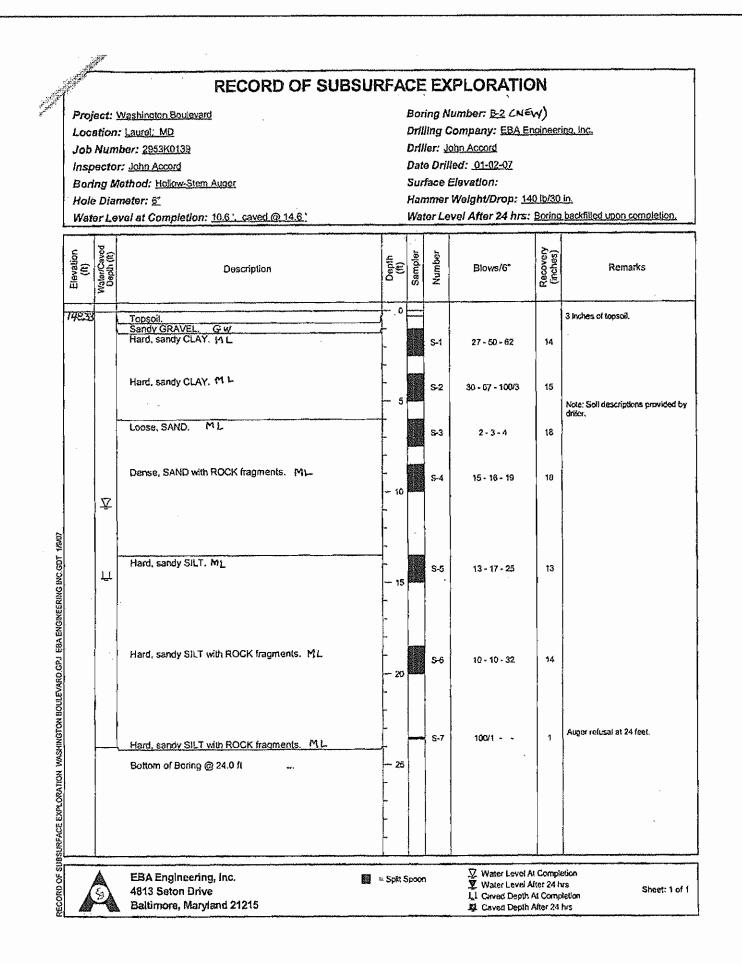
SHEET: 24 OF 25 SDP-06-100

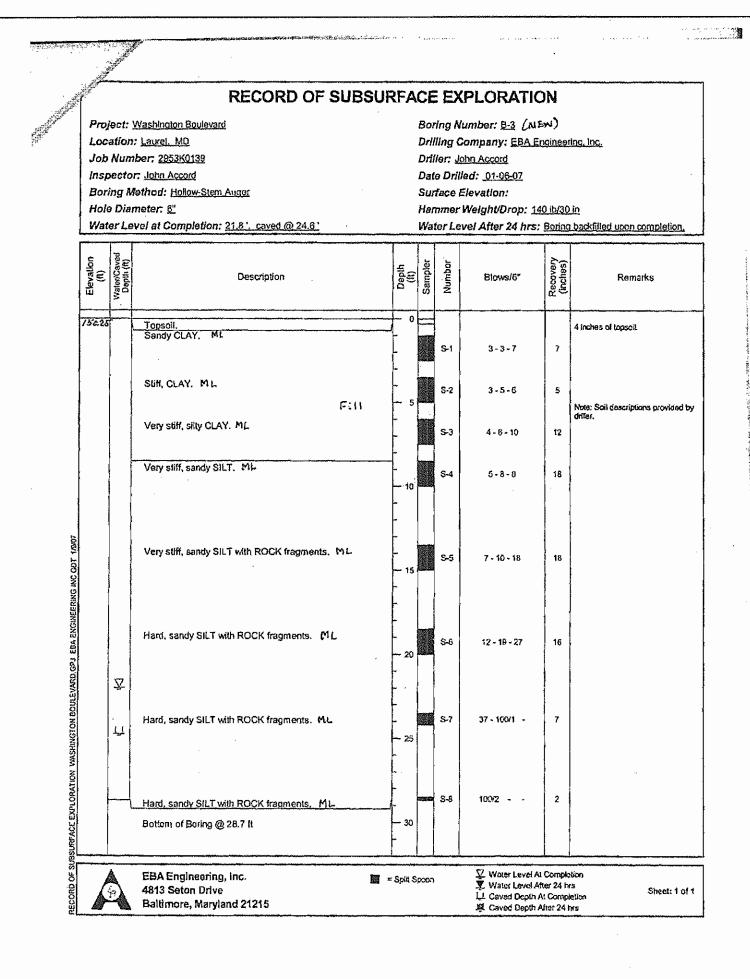
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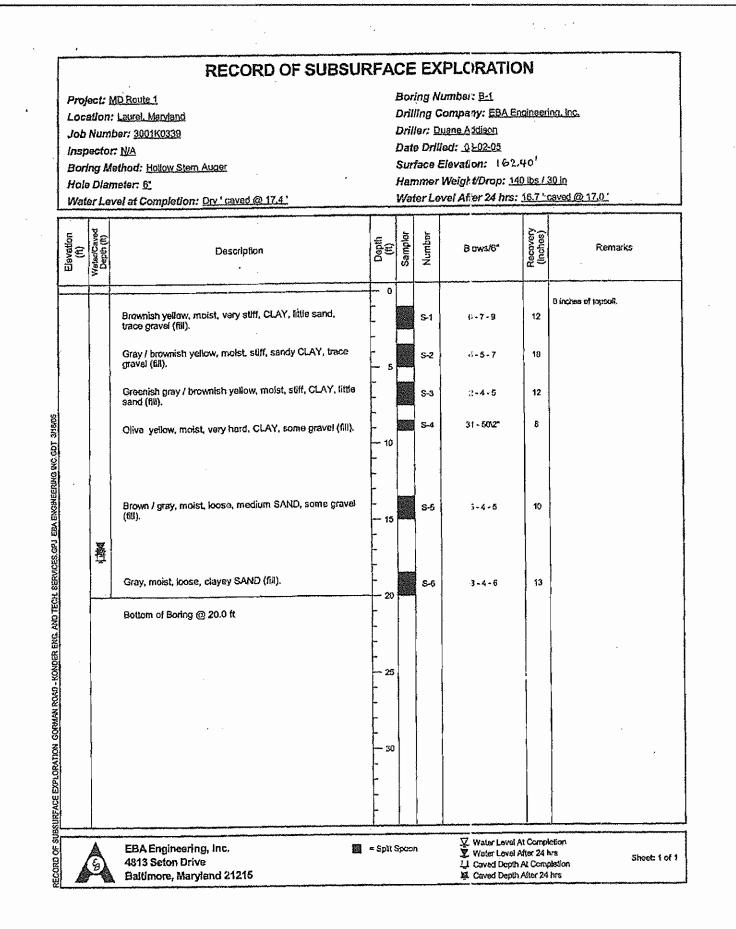
DATE: 01/23/08 DRAWN BY: HRP

CHECKED BY: SBP

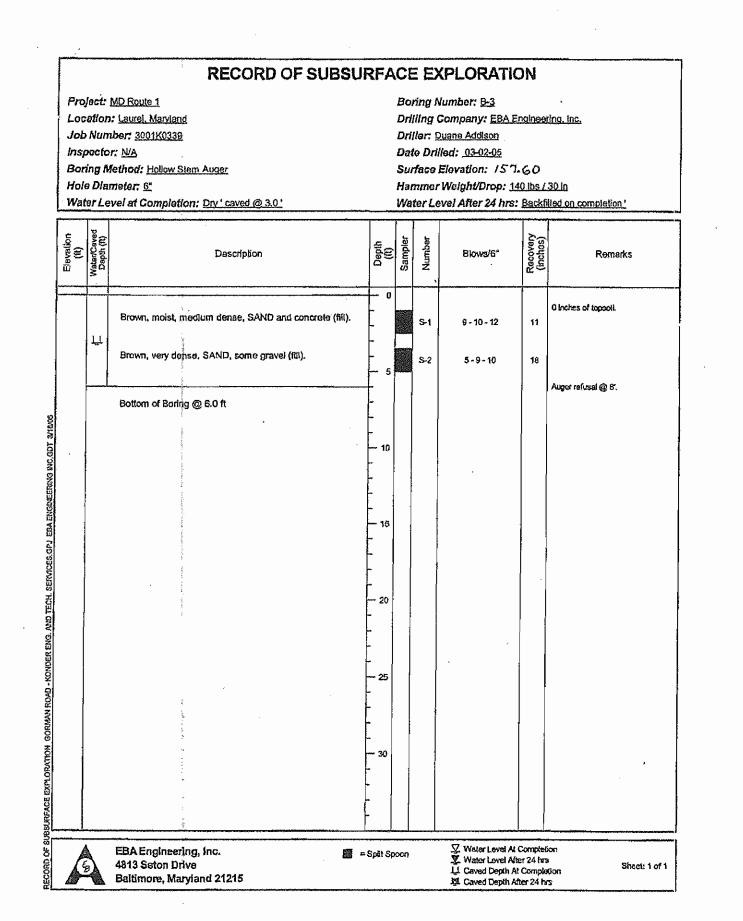


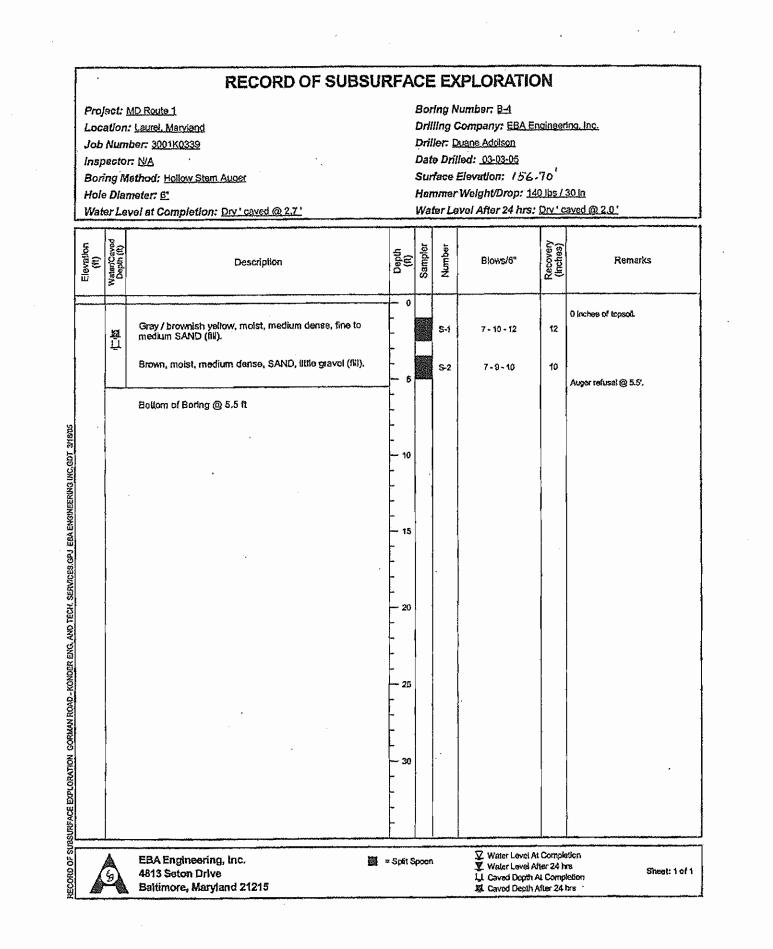


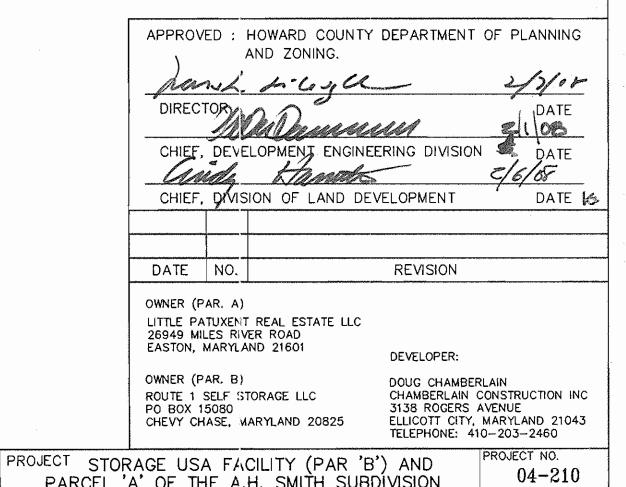


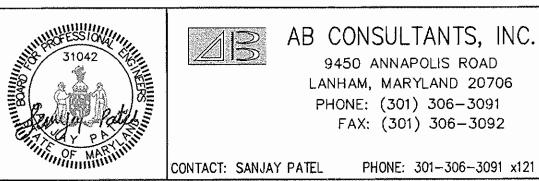


		RECORD OF SU	BSU	RFA	C	E EX	PLORATI	ON	
Loc Jab Insj Bor Hold	ation Num secto ing M e Dia	MD Route 1 : Laurel, Maryland : ber: 3001K0339 r: N/A lethod: Hollow Stem Auger neter: 6"			Dri Dri Da Su Hai	lling C lior: <u>D</u> te Drill rface E mmer	umber: <u>B-2</u> company: <u>EBA (</u> company: <u>EBA (</u> company: <u>EBA (</u> company: <u>EBA (</u> company: <u>15</u> company: 15 Welght/Drop: 1	7,25 40 lbs/	1 30 in
Wat	erLe	vel at Completion: 17.0 ' caved @ 17.5'			Wa	ter Le	vel After 24 hrs	: <u>7.5 ' c</u>	aved @ 11.0'
Elevation (ft)	Water/Caved Depth (ft)	Description .		Depth	Sampler	Number	Blows/6*	Recovery (inches)	Remarks
**************************************	The state of the s	Brown, moist, medium dense, silty SANO with grav	vel	a		5-1	9-8-11	8	O inches of topsoli.
		Brown / dark gray, moist, very dense, SAND, some gravel (fill).	0	5		S-2	10 - 30 - 48	12	i v
	Ā	Brownish yellow / gray, moist, stiff, CLAY, trace sli	L	1		S-3	10-8-8	12	
	Į.	Gray, moist, medium dense, fine SANO.		- - 10		S-4	7 - 10 - 11	12	
		Pale green, moist, very stiff, sandy SILT.		15		S-5	8 - 10 - 13	18	
**************************************	<b>圣</b>	Grayish green, moist, hard, sandy SILT.		- 20		S-6	12 - 16 - 23	18	
		Gravish green, moist, hard, sandy SILT.  Bottom of Boring @ 24.0 ft		25	and the proof of	S-7	50\6*	6	
				-					
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				-					
1		EBA Engineering, Inc. 4813 Seton Drive Baltimore, Maryland 21215	刨	- Sp‼l S	nooc		Water Level A     Water Level A     U Caved Depth A	iter 24 hrs	Sheet 1 of











AB CONSULTANTS, INC. 9450 ANNAPOLIS ROAD LANHAM, MARYLAND 20706 PHONE: (301) 306-3091 FAX: (301) 306-3092

PARCEL 'A' OF THE A.H. SMITH SUBDIVISION SELF-STORAGE FACILITY EXPANSION AND PROPOSED FLEX SPACE BUILDING

SCALE: NTS DATE: 01/23/08 DRAWN BY: SOIL BORING LOG CHECKED BY: \_

HOWARD COUNTY, MARYLAND 6TH ELECTION DISTRICT TAX MAP 47, PARCEL A & B, GRID 18, P/O PARCEL 144

SHEET: 25 OF 25 SDP-06-100