#### GENERAL NOTES All construction shall be in accordance with the latest standards and specifications of Howard County plus MSHA standards and specifications, if applicable. The contractor shall notify "Miss Utility" at 1-800-257-7777 at least 48 hours prior to any excavation The contractor is to notify the following utilities or agencies at least five days before starting work on these drawings: 1-800-257-7777 Miss Utility 1-410-954-6281 Verizon Telephone company: 313-2366 Howard County Bureau of Utilities: 393-3553 AT&T Cable Location Division: 850-4620 B.G.&E. Co. Contractor Services: 787-462C B.G.&E. Co. Underground Damage Control: State Highway Administration: 4. Site analysis: Area of Parcel: 2.4987 Ac. Present Zoning: B-2 Use of Structure: Auto Retail Sales & Service Automobile Sales = 7629 sf. Building Area: 21,887 sf. Building Coverage On Site: 0.50 Ac. or 20% of Gross Area Paved Parking Lot/Area On Site: 1.17 Ac. or 46.8% of Gross Area Area of Landscape Island: .09 Ac. or 3.6% of Gross Area Limit of Disturbed Area: 208,350 sf. or 4.78 Ac. 5. Project background: Location: Clarksville, Md.; Tax Map 34, K-2 Zoning : B-2 Subdivision: Holweck Subdivision Section/Area : N/A Site Area: 2.4987 Ac. DPZ references: F-94-38; F-98-144; F-99-205; SP-93-14; WP-93-90; ZB-947M; ZB-1008M; F-01-29; WP-03-41: F-03-202 The contractor shall notify the Department of Public Works/Bureau of Engineering/Construction Inspection Division at (410) 313-1880 at least five (5) working days prior to start of work. 7. Any damage to public right-of-ways, paving, or existing utilities will be corrected at the contractor's expense. Existing utilities located from Road Construction Plans, Field Surveys, Public Water and and Sewer Extension Plans and available record drawings. Approximate location of existing utilities are shown for the contractors information. Contractor shall locate existing utilities well in advance of construction activities and take all necessary precautions to protect the existing utilities and to maintain uninterrupted service. Any damage incurred due to contractor's operation shall be repaired immediately at the contractor's expense. All reinforced concrete for storm drain structures shall have a minimum of 28 days strength of 3,500 p.s.i. 10. Traffic control devices, markings and signing shall be in accordance with the latest edition of the Manual on Uniform Traffic Control Devices (MUTCD). All street and regulatory signs shall be in place prior to the placement of any asphalt. 11. Estimates of earthwork quantities are provided soley for the purpose of calculating fees. 12. Soil compaction specifications, requirements, methods and materials are to be in accordance with the recommendations of the project Geotechnical Engineer. Geotechnical Engineer to confirm acceptability of proposed paving section, based on soil test prior to construction. 13. All storm drain pipe bedding shall be Class 'C' 14. The existing topography is taken from field run survey with two foot contour intervals prepared by Frederick Ward and Associates, dated May 24, 2001. The coordinates shown hereon are based upon the Howard County Geodetic Control which is based upon the Maryland State Plane Coordinate System. 15. A noise study is not required for this project. 16. All paving to be minimum Howard County Standard Detail P—2 unless otherwise noted. (See details, sheet 5) 17. All curb and gutter to be Detail 3.01 unless otherwise noted. (See detail, sheet 5) 18. Contractor responsible to construct all handicap ramps and handicap access in accordance with current ADA requirements. 19. Where drainage flows away from curb, contractor to reverse the gutter pan. 20. All elevations are to flowline/bottom of curb unless otherwise noted. 21. All dimensions are to face of curb unless otherwise noted.

22. Public water is available through Contract No. 39-3942-D; Public sewer is available through Contract No. 39-3942-D. 23. Stormwater management quantity is provided by the proposed underground detention system. Water quality is proposed by a bio-retention system, and an underground sand filter structure. The proposed SWM system and water quality system are to be privately owned and maintained by Win Kelly Mitsubishi. 24. All exterior lighting to conform to section 134 of the Howard County Zoning Regulations. (Detail on Sheet 3)

25. Building to have inside water meter setting. 26. Traffic impact study prepared by Street Traffic Studies, LTD., dated March 20, 2003. APFO Study approval date April 29, 2003.

This plan has been prepared in accordance with the provisions of Section 16.124 of the Howard County Code and Landscape Manual.

28. Any existing street trees damaged or destroyed during construction will be replaced by the contractor.

29. Forest Conservation requirements for Parcel K-1 are provided in conjunction with F-01-029.

30. Financial surety for the required landscaping shall be posted as part of the developer's agreement in the amount of \$12,360.00 for 23 shade trees, 28 evergreen trees, and 42 shrubs. 31. No mezzanine level or second story is permitted in the building unless adequate parking has been approved

by the Department of Planning and Zoning. 32. Reference ZB 947M and ZB 1008M for zoning cases for this site: A) ZB 947M:

1. Date of approval for Decision and Order: March 11, 1994 a. Action Taken: Rezone part of property from B-2 to R-C and R-c to B-2 2. Conditions of approval: A 300' private easement buffer area between the zoned B-2 use on lot 4 and the existing residential house on lot 3. B) ZB 1008M:

1. Date of approval for Decision and Order: December 4, 2000 a. Action Taken: Rezone part of Parcel "L" from B-2 to RC 2. Conditions of approval: No conditions of approval were specified.

33. Debris is to be kept out of all stormwater management facilities during and after construction. 34. Adjoining Parcel E-2 was the subject of WP-03-41, which allowed a temporary sales trailer to be established. A condition of approval required that within 180 days of signature approval of a site plan for Parcel K-1, the temporary use on Parcel E-2 must be vacated and all improvements removed. 35. Geotechnical report prepared by Specialized Engineering, dated January 17, 2003.

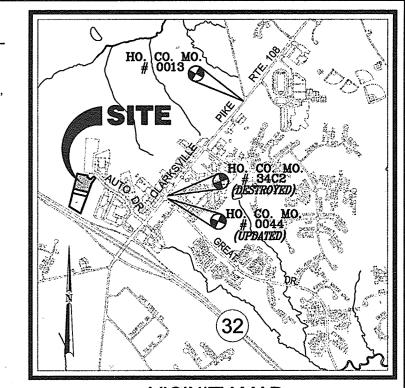
36. THE SWM REQUIREMENTS FOR REDLINE #3 HAS BEEN PROVIDED BY A MICRO-BIORETENTION (M-6) FACILITY, SWM #3 TO BE PRIVATELY OWNED

# SITE DEVELOPMENT PLAN COLEMAN LANDROVER/JAGUAR



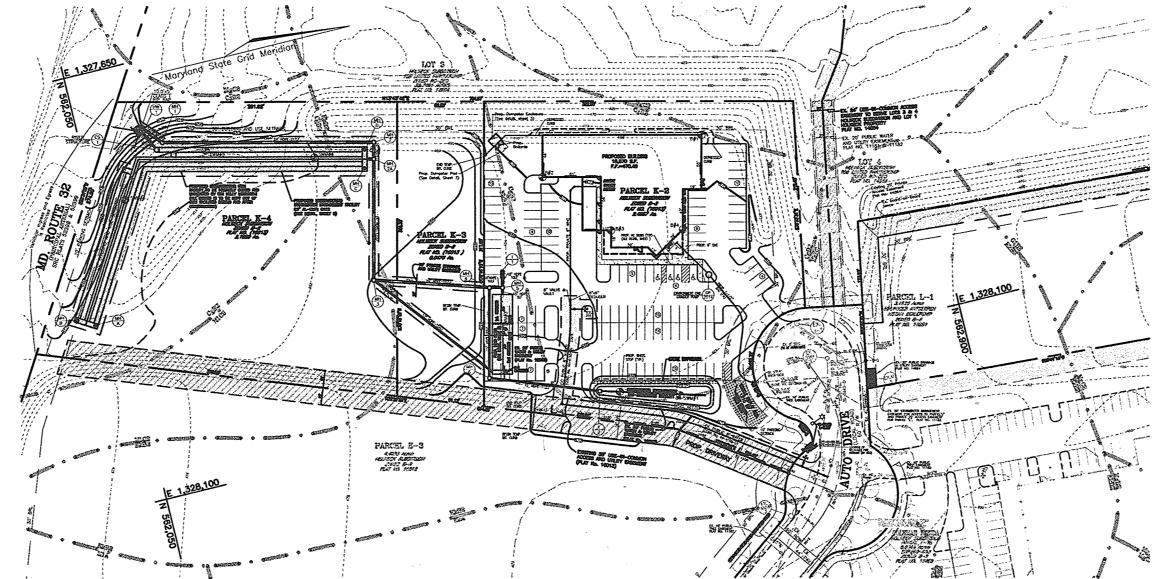
Howard County Station 34C2 (Destroyed) N 562,321.798 E 1,329,750.722 Updated: Station 0044 N 562,176.474 E 1,329,641.868 Elev: 485.252'

Howard County Station 0013 N 564,285.946 E 1,331,309.715 Elev: 484.671'



**VICINITY MAP** 

PROPOSED BUILDING ELEVATION - NORTH (FRONT NOT TO SCALE



LOCATION MAP

SCALE: 1" = 100"

SERVICE (15 BAYS)

DRIVE-THRU 2,038 SI

136,

PROPOSED BUILDING

PARTS

953 SF

20,104 SF

7. MISCELLANEOUS THESE PRACTICES MAY NOT BE CONSTRUCTED UNTIL ALL CONTRIBUTING DRAWAGE AREA HAS BEEN STABILIZED.

| v            |  |       | 58" H"           |
|--------------|--|-------|------------------|
| NOKIN (FROM) | 001 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1  | 24" > | 55-1/2"          |
| , company    | 1500 GALLON SAND/OIL SEPAR Three Compartment NOT TO SCALE  GENERAL NOTES 4000 CONCRETE STRENGTH AT 28 DAYS |       | ND/OIL SEPARATOR |

| SHEET INDEX   |           |
|---|-----------|
| DESCRIPTION :   | SHEET NO. |
| Cover Sheet   | 1 of 7    |
| Site Development, Grading, and Sediment and Erosion Control Plan              | 2 of 7    |
| Sediment and Erosion Control Notes and Details                                | 3 of 7    |
| Storm Drain Drainage Area Map, Storm Drain Profiles and Details               | 4 of 7    |
| Stormwater Management Notes and Details, Water and Sewer Profiles and Details | 5 of 7    |
| Stormwater Management Notes and Details                                       | 6 of 7    |
| Landscape Plan  | 7 of 7    |

| 398                 | EXISTING CONTOUR                              |
|---------------------|---|
| 406                 | PROPOSED CONTOUR                              |
| + 40268             | PROPOSED SPOT ELEVATION                       |
| ×402: <sup>63</sup> | EXISTING SPOT ELEVATION                       |
|                     | EXISTING CURB AND GUTTER                      |
|                     | PROPOSED CURB AND GUTTER                      |
| (                   | EXISTING GUY WIRE                             |
| Ď                   | EXISTING UTILITY POLE                         |
| *                   | EXISTING LIGHT POLE                           |
|                     | EXISTING MAILBOX                              |
|                     | EXISTING SIGN                                 |
| o                   | EXISTING BOLLARD                              |
| <u> </u>            | EXISTING SANITARY MANHOLE                     |
|                     | EXISTING SANITARY LINE                        |
| 000                 | EXISTING CLEANOUT                             |
| <b>}</b> AH         | EXISTING FIRE HYDRANT                         |
|                     | EXISTING WATER LINE                           |
| (EMH)               | EXISTING SD MANHOLE                           |
|                     | EXISTING STORM DRAIN                          |
|                     | PROPOSED STORM DRAIN INLET                    |
| # { x }             | EXISTING TREES (FIELD LOCATED)                |
|                     | EXISTING TREELINE<br>(FIELD LOCATED)          |
|                     | EXISTING VEGETATION (APPROXIMATE LOCATION)    |
| X                   | EXISTING FENCE                                |
|                     | PROPERTY LINE                                 |
|                     | RIGHT-OF-WAY LINE                             |
| BrB2                | SOILS BOUNDARY                                |
| SFSF                | SILT FENCE                                    |
| SSF                 | SUPER SILT FENCE                              |
| LOD LOD             | LIMIT OF DISTURBANCE                          |
| OR OR               | EXISTING PAVING TO BE<br>REMOVED AND REPLACED |
|                     | PROPOSED P-2 PAVING                           |
|                     | PROPOSED BUILDING ADDITION                    |
| C J AGIP            | AT GRADE INLET PROTECTION                     |
|                     | PROPOSED SIDEWALK                             |
| A                   |   |
|                     | STABILIZED CONSTRUCTION ENTRANCE              |
| <b>©</b>            | PROP. ELECTRIC VEHICLE SPACE                  |
| AC DC               | PROP. ELECTRIC CHARGING STATION               |

12500 NEW CAR DRIVE PERMIT INFORMATION CHART PARCEL NUMBER SUBDIVISION NAME HOLWECK SUBDIVISION K-2, K-3, K-24 N/A TAX/ZONE ELECT. DIST. 16013 WATER CODE: 110 SEWER CODE: 665300

| PPROVED: HOWARD COUNTY DEPARTMENT OF    | PLANNING AND ZONING |
|---|---------------------|
| MADerman                                |                     |
| CHIEF, DEVELOPMENT ENGINEERING DIVISION | 4 DATE              |
| and Hamt                                | (2/17/03            |
| CHIEF, DIVISION OF LAND DEVELOPMENT     | HB / DATE           |
| DIRECTOR                                | DATE                |

REVISIONS

3 5-21-20 REVISE THE PLAN TO REFLECT ADDITION OF A 2,526 SF

2 10/24/05 REVISE SANDFILTER; REVISE WHC; ADD.

1 03/28/05 REVISE BUILDING FOOTPEINT, ASSOCIATED GRADING

DISPLAY AREAS

AND ADDITIONAL SITE MODIFICATIONS

REV# DATE

DESCRIPTION

SHOWROOM, ADDITION OF 1,620 SF OF NEW PAVEMENT

SANDIOIL SEPARATOR : REVISE VEHICLE

CHANGES, TITLE AND PROJECT WAME CHANGES

### PARKING TABULATION

AUTOMOBILE DISPLAY: 6,849 SF @ 1 SPACE/1000 SF SALES/OFFICE: 8,114 SF (INCLUDING MEZZANINE) @ 2 SPACES/1000 SF SERVICE BAYS: 15 BAY AUTOMOBILE SERVICE AREA @ 3 SPACES/SERVICE BAY (SEE SCHEMATIC FOR DOOR LOCATIONS) TOTAL SPACES: REQUIRED TOTAL SPACES: PROVIDED

17 SPACES 45 SPACES

REQUIRED 7 SPACES

**75** SPACES (INCLUDING 4 HANDICAP) (INCLUDE 7 ELECTRIC SPACES)

## OWNER/DEVELOPER

1318 COMPANY LLC 10400 AUTO PARK DRIVE BETHESDA, MD 20817

**VOGEL ENGINEERING** 

**TIMMONS GROUP** 

P: 410.461.7666 F: 410.461.8961 www.timmons.com

7125 RIVERWOOD DRI LAND AND WARRENTON, VIRGINIA NEERS PLANNERS SURVEYORS www.frederickward.com

EAST (SIDE)

PROPOSED BUILDING PLAN VIEW

## (REVISED) SITE DEVELOPMENT PLAN

COVER SHEET COLEMAN LANDROVER/JAGUAR



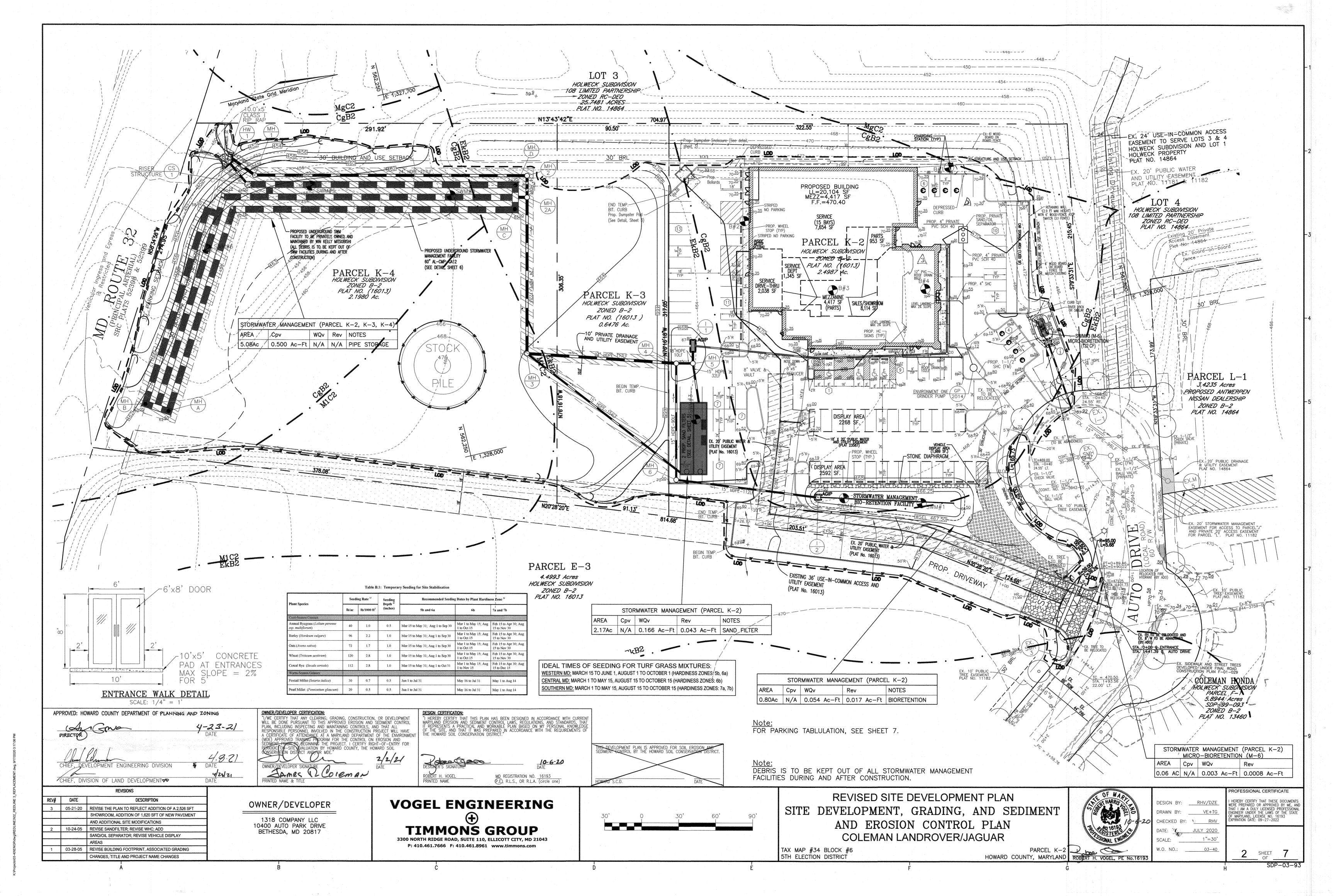
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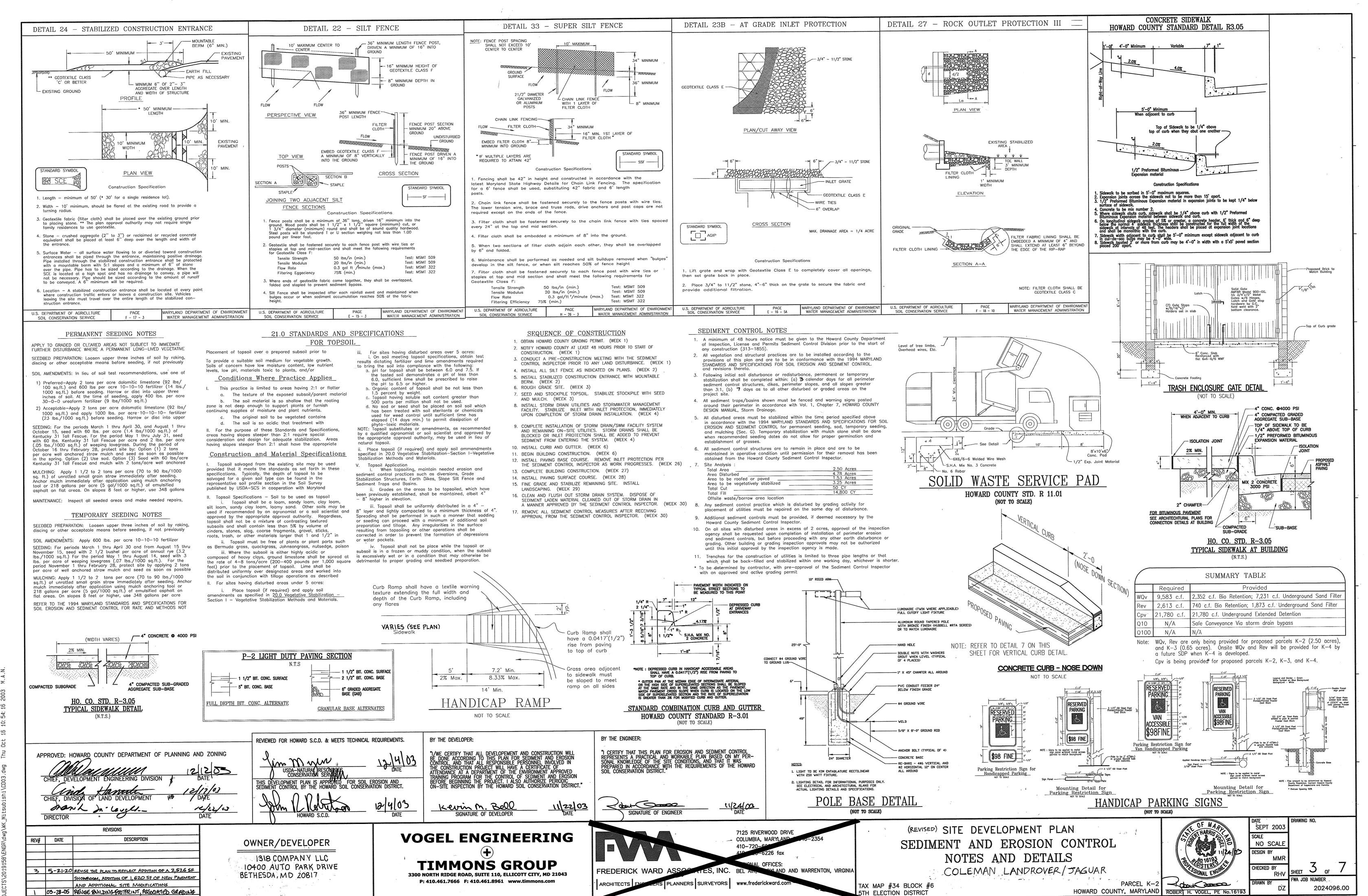
TAX MAP #34 BLOCK #6 TH ELECTION DISTRICT

PARCEL K-2 Del

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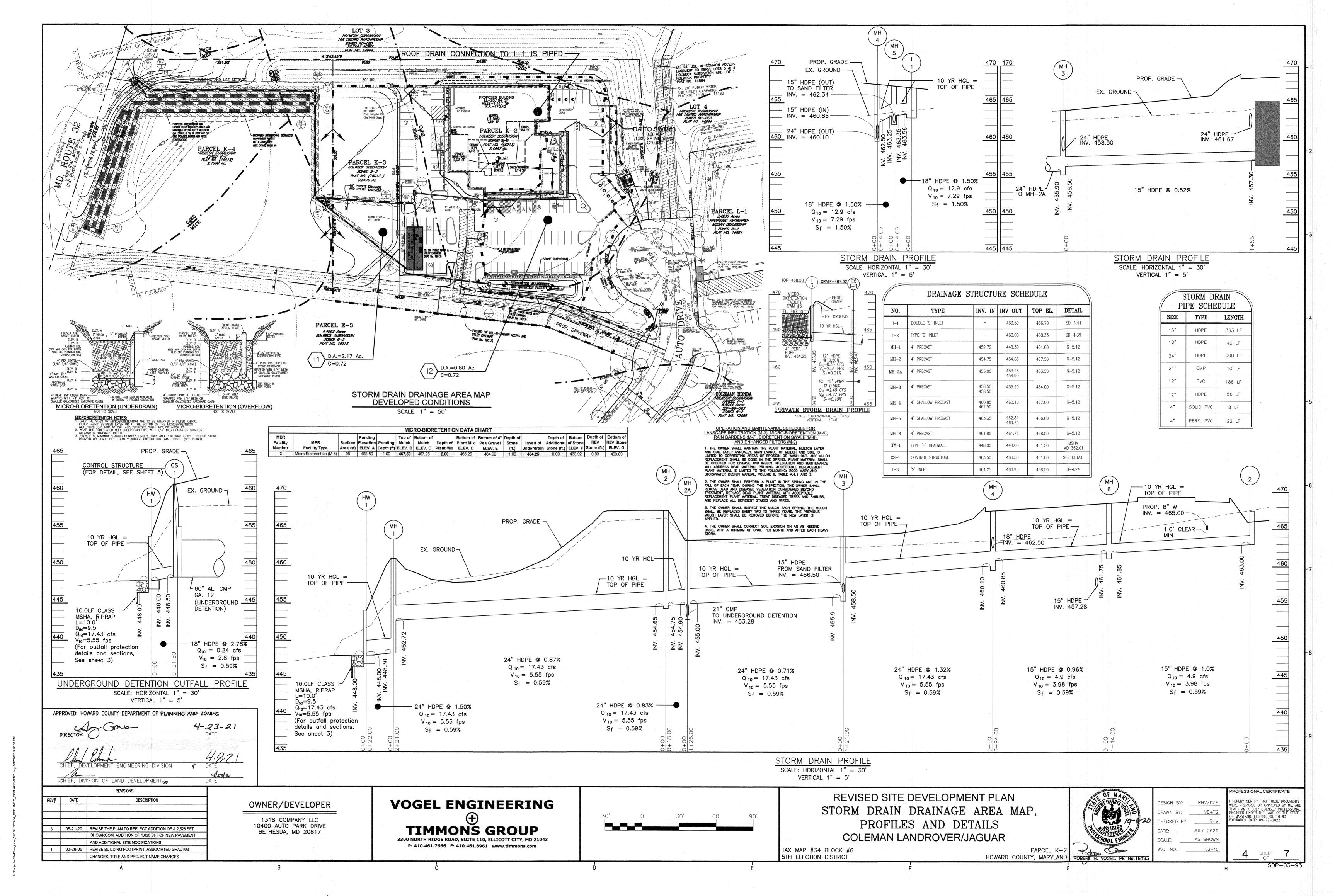
HOWARD COUNTY, MARYLAND ROBERT H. VOGEL, PE No.16193

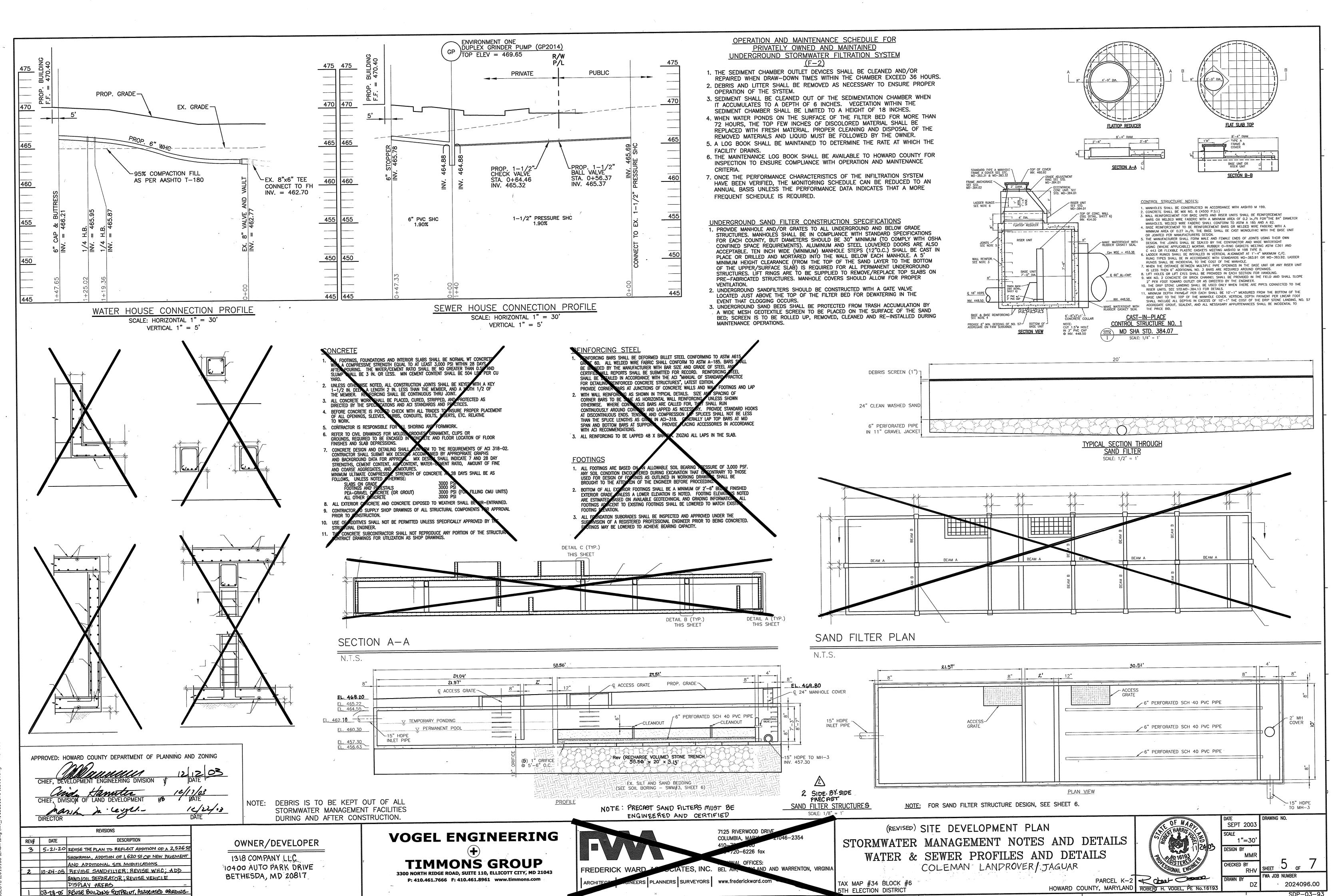




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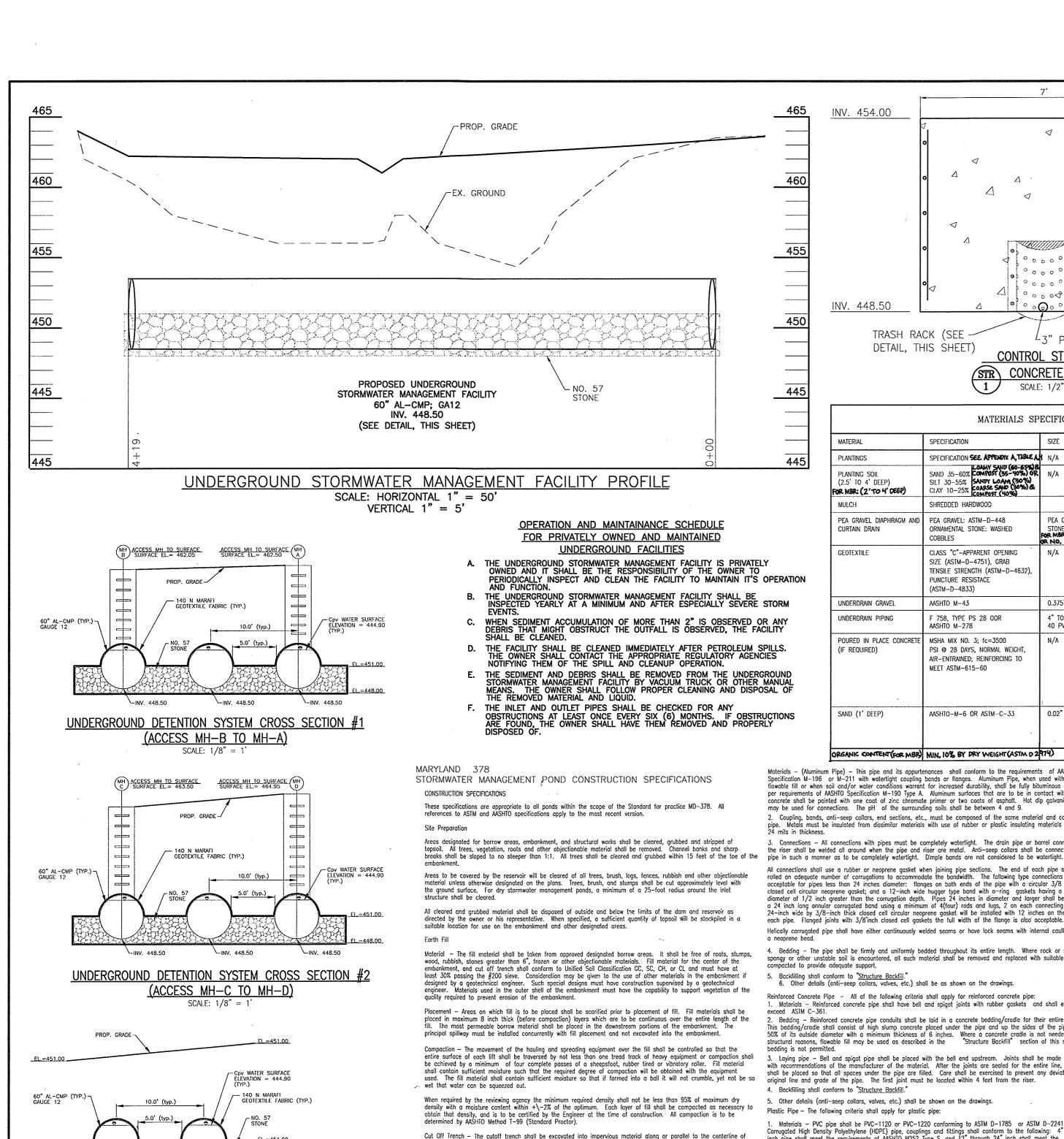
HANGES, TITLE AND PROJECT NAME CHANGE





obn JerskyphadsRyfenGD\dwn\wK Mitsuhishi\CD05 dwa Thu Oct 16 11:03:28 2

CHANGES. TITLE AND PROJECT NAME CHANG



MATERIALS SPECIFICATIONS FOR BIO-RETENTION AND MICRO-BIORETENTION MATERIAL SPECIFICATION SPECIFICATION SEE APPENDIX A, TABLE A.H N/A PLANTINGS ARE SITE-SPECIFIC SAND 35-60% COMPOST (35-40%) OR PLANTING SOIL USDA SOIL TYPES LOAMY SAND, SANDY LOAM OR LOAM SILT 30-55% SANDY LOAM (30%) COAYSE SAND (30%) & COAYSE SAND (30%) & COMPOST (40%) (2.5' TO 4' DEEP) FOR MBR: (2'TO 4' DEEP) PEA GRAVEL DIAPHRAGM AND | PEA GRAVEL: ASTM-D-448 PEA GRAVEL NO. 6 STONE: 2" TO 5"
FOR MOR: PEA GRAVEL NO. 8
OR NO. 9 (1/8"TO 3/8") CURTAIN DRAIN ORNAMENTAL STONE: WASHED COBBLES GEOTEXTILE CLASS "C"-APPARENT OPENING FOR USE AS NECESSARY BENEATH UNDERDRAINS ONLY FOR MBR: PE TYPE I NONWOVEN SIZE (ASTM-D-4751), GRAB TENSILE STRENGTH (ASTM-D-4632), FOR MBR: PERFORATED PIPE SHALL BE WRAPPED WITH 1/4-INCH GALVANIZED HARDWARE CLOTH PUNCTURE RESISTACE (ASTM-D-4833) FOR MBR: USE NO. 57 OR NO. 6 AGGREGATE (3/8"TO 3/4) 0.375" TO 0.75" AASHTO M-43 F 758, TYPE PS 28 OOR 3/8" PERF. @ 6" O.C., 4 HOLES PER ROW: MIN. OF 3" OF AASHTO M-278 40 PVC OR SDR35 GRAVEL OVER PIPES; NOT NECESSARY UNDERNEATH PIPES POURED IN PLACE CONCRE ON-SITE TESTING OF POURED-IN-PLACE CONCRETE REQUIRED 28 DAY STRENGTH AND SLUMP TEST; ALL CONCRETE DESIGN (CAST-IN-PLACE (IF REQUIRED) PSI @ 28 DAYS, NORMAL WEIGHT, OR PRE-CAST) NOT USING PREVIOUSLY APPROVED STATE OR LOCAL AIR-ENTRAINED; REINFORCING TO STANDARDS REQUIRES DESIGN DRAWINGS SEALED AND APPROVED BY A MEET ASTM-615-60 PROFESSIONAL STRUCTURAL ENGINEER LICENSED IN THE STATE OF MARYLAND DESIGN TO INCLUDE MEETING ACI CODE 350.R/89; VERTICAL LOADING (H-10 OR H-20); ALLOWABLE HORIZONTAL LOADING (BASED ON SOIL PRESSURES); AND ANALYSIS OF POTENTIAL CRACKING SAND SUBSTITUTIONS SUCH AS DIABASE AND GRAYSTONE #10 ARE NOT SAND (1' DEEP) AASHTO-M-6 OR ASTM-C-33 0.02" TO 0.04" ACCEPTABLE, NO CALCIUM CARBONATED OR DOLOMITIC SAND SUBSTITUTIONS ARE ACCEPTABLE. NO "ROCK DUST" CAN BE USED FOR Organic content (for MBR) Min. 10% by Dry Weight (Astm 0 2974)

L3" PVC CAP W/ 1.75" DIAM. ORIFICE

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CONTROL STRUCTURE NO. 1

STR CONCRETE WALL DETAIL

Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of ASHIO Specification M-196 or M-211 with waterlight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be between 4 and 9. . Coupling, bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at lease 24 mils in thickness. . Connections - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the

Il connections shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be rerolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches diameter: flanges on both ends of the pipe with a circular 3/8 inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by

a 24 inch long annular corrugated band using a minimum of 4(four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8'inch closed cell gaskets the full width of the flange is also acceptable. Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or

4. Bedding – The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

Backfilling shall conform to "Structure Backfill."

6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings. Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete pipe:

Materials - Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASIM C-361. Bedding – Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length.
his bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in the "Structure Backfill" section of this standard. Gravel

3. Laying pipe — Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser. 4. Backfilling shall conform to "Structure Backfill." 5. Other details (anti-seep collars, valves, etc.) shall be shown on the drawings.

Plastic Pipe — The following criteria shall apply for plastic pipe:

INV. 454.00

TRASH RACK (SEE

DETAIL, THIS SHEET)

Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241 Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" -10" inch pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" inch shall meet the the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at least four feet below existing grade or requirements of AASHTO M294 Type S. as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with 2. Joints and connections to anti-seep collars shall be completely watertight.

3. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth imbankment Core — The core shall be parallel to the centerline of the embankment as shown on the plans. The top compacted to provide adequate support. width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction Backfilling shall conform to "Structure Backfill." equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed concurrently with the outer shell of the embankment.

5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings. Drainage Diaphragms — When a drainage diaphragm is used, a registered professional engineer will supervise the Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3. fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operated closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall

Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction Materials, Section 311. Geotexile shall be placed under all riprap and shall meet requirements of Maryland Department of Transportation State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the excavations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water rom the required excavation and the foundation shall be accomplished in a manner and to the extent that will naintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which nay require draining the water sumps from which the water shall be pumped.

All borrow areas shall be graded to provide proper drainage and left I a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans

OPERATION AND MAINTENANCE SCHEDULE FOR BIO-RETENTION AREAS ANNUAL MAINTENANCE OF PLANT MATERIAL, MULCH LAYER AND SOIL LAYER IS REQUIRED. MAINTENANCE OF MULCH AND SOIL IS LIMITED TO CORRECTING

- #4 BARS 12" O.C.

WELDED WIRE FABRIC.

OVERLAP ONE WAY 24'

CONTROL STRUCTURE CONCRETE WALL NOTES

OR WELDED WIRE FABRIC WITH A MIN. AREA OF 0.21 in./ft

USING FORMS. IN LIEU OF MONOLITHIC POUR, WALL SHALL

SEALED WITH GREEN STREAK WATERSTOP (OR APPROVED

BE CONNECTED BY OVERLAPPING BARS ONE WAY 24", AND

CONCRETE WALL SHALL BE MIX NO. 6 (4500 p.s.i.) 2. WALL REINFORCEMENT SHALL BE REINFORCEMENT BARS

WALL SHALL BE CAST-IN-PLACE, POURED MONOLITHIC

AREAS OF EROSION OR WASH OUT. ANY MULCH REPLACEMENT SHALL BE DONE IN THE SPRING. PLANT MATERIAL SHALL BE CHECKED FOR DISEASE AND INSECT INFESTATION AND MAINTENANCE WILL ADDRESS DEAD MATERIAL AND PRUNING. 2. SCHEDULE OF PLANT INSPECTION WILL BE TWICE A YEAR IN SPRING AND

THIS INSPECTION WILL INCLUDE REMOVAL OF DEAD AND DISEASED VEGETATION CONSIDERED BEYOND TREATMENT, TREATMENT OF ALL DEFICIENT STAKES AND WIRES. 3. MULCH SHALL BE INSPECTED EACH SPRING. REMOVE PREVIOUS MULCH LAYER BEFORE APPLYING NEW LAYER ONCE EVERY 2 TO 3 YEARS. . SOIL EROSION TO BE ADDRESSED ON AN AS NEEDED WITH A MINIMUM OF ONCE PER MONTH AND AFTER HEAVY STORM EVENTS.

NOTE: CONTRACTOR SHALL ENSURE THAT THE S.W.M. FACILITY

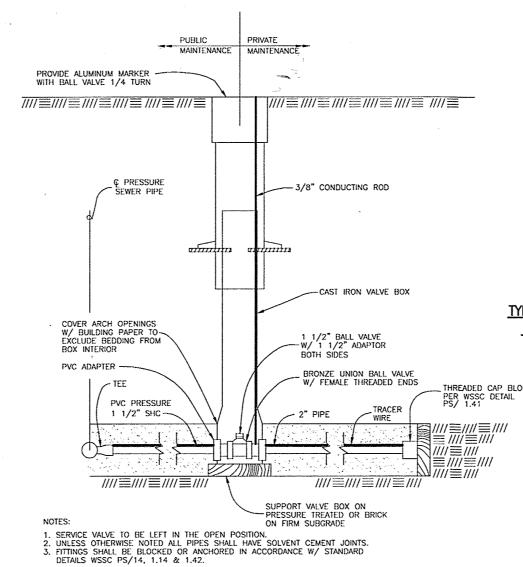
IS WATERTIGHT.

ALL PIPE CONNECTIONS AT STRUCTURES SHALL BE CEMENTED TO ENSURE WATERTIGHT CONNECTION.

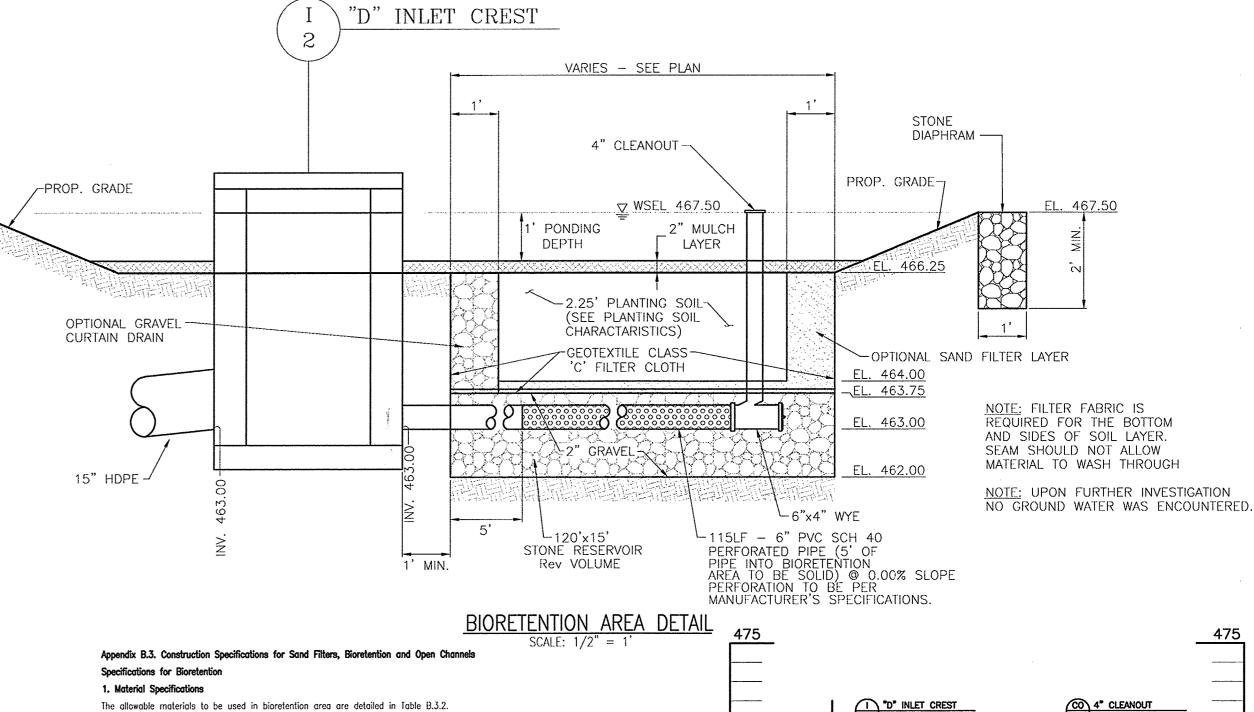
2. ALL ACCMP PIPE JOINTS SHALL USE 12" WIDE HUGGER BAND WITH

"O" RING GASKETS. 3. TEES AND ELBOWS TO BE FACTORY FABRICATED WELDS, ONE PIECE. 4. TRENCH BEDDING TO BE IN ACCORDANCE WITH RECOMMMENDATIONS

FROM THE GEOTECHNICAL ENGINEER IN THE FIELD. PROVIDE WATERTIGHT JOINTS AT ALL PIPE CONNECTIONS. (FOR REINFORCED CONCRETE PIPE, ASTM C-361, RUBBER GASKET PIPE).



BALL VALVE CONNECTION DETAIL SCALE: N.T.S.



CO 4" CLEANOUT 1 TOP ELEV = 467.50 1 "D" INLET CREST 2 TOP ELEV = 467.50 The soil shall be a uniform mix, free of stones, stumps, roots or other similar objects larger than two inches. No other materials or substances shall be mixed or dumped within the bioretention area that may be harmful to plant growth, or prove a hindrance to the planting or maintenance operations. The planting soil shall be free of Bermuda grass, Quackgrass, Johnson grass, or other noxious weeds as specified under COMAR 15.08.01.05. 465 All bioretention areas shall have a minimum of one test. Each test shall consist of both the standard soil test for pH, phosphorus, and potassium and additional test of organic matter, and soluble salts. A textural analysis shall be performed for each location where the top soil was excavated. L6" PVC SCH 40 PERFORATED PIPE - 15" 460 It is very important to minimize compaction of both the base of the bioretention area and the required backfill. When possible, use hoes to remove original soil. If, bigretention areas are excavated using loader, the contractor should use wide track or marsh track equipment, or light equipment with turt type tire. Use of equipment with narrow tracks or narrow tires, rubber tires with large lugs, or high pressure tires will cause excessive compaction resulting in reduced infiltration rates and is not acceptable. Compaction will significantly contribute to design failure. ----Compaction can be alleviated at the base of the bioretention facility by using a primary tilling operation such as chisel plow, ripper, or subsoiler. These tilling operations are to refracture the soil profile through the 12 inch compaction zone. Substitute methods must be approved by the engineer. Rototillers typically do not till deep enough to reduce the effects of compaction from heavy 455 Rototill 2 to 3 inches of sand into the base of the bioretenion facility before backfilling the required sand layer. Pump any ponded When backfilling the topsoil over the sand layer, first place 3 to 4 inches of topsoil over the sand, then rototill the sand/topsoil to create a gradation zone. Backfill the remainder of the topsoil to final grade. When backfilling the bioretention facility, place soil in lifts 12" to 18". Do not use heavy equipment within the bioretention basin. Heavy equipment can be used around the perimeter of the basin to supply soils and sand. Grade bioretention materials with light equipment such as a compact loader or a dozer/loader with marsh tracks. Recommended plant material for bioretention areas can be found in Appendix A, Section A.2.3. of the 2000 Maryland Stormwater Design Manual.

**BIORETENTION PROFILE** 

SCALE: HORIZONTAL 1" = 50'

VERTICAL 1" = 5"

-CONCRETE BOLTS 0000 -1/2" ø HOLES 0000 3" O/C (5 ROWS) -1/2 18" ACCMP TRASH RACK CONTROL STRUCTURE TRASH RACK DETAIL

The bioretention facility may not be constructed until all contributing drainage area has been stabilized - 36" ACCESS MANHOLE TO BE: 14 GA. 2 2/3"x 1/2" CORRUGATION ALT2 CSP WITH STEPS WELDED TO CSP @ 12" C.C. SCALE: 1/2'' = 1'-H-12 HUGGER BAND Fill: Tan, silty, sand, moist, trace gravel, trace clay, loose to medium dense Fill: Tan, sandy, silt, trace rock fragments, moist, very loose to medium dense SILT: Brown, sandy, very fine to fine, rock fragments, dry, loose to medium dense (ML) TYPICAL ACCESS MANHOLE DETAIL FOR FOR UNDERGROUND SWM FACILITY (NOT TO SCALE) FILL: Ton, silty, dry to wet, trace FILL: Brown, clayey SILT, sandy, moist, trace rock fragments, very loose to loose

7125 RIVERWOOD DRIVE COLUMBIA, MARY AND WARRENTON, VIRGINIA (REVISED) SITE DEVELOPMENT PLAN STORMWATER MANAGEMENT NOTES AND DETAILS COLEMAN LANDROVER/ JAGUAR

SEPT 2003 AS SHOWN CHECKED BY O OF / 2024096.00

REVISIONS DATE DESCRIPTION REV# 3 5-21-20 REVISE THE PLAN TO REFLECT ADDITION OF A 2,526 SF HOWROOM, ADDITION OF 1,620 SF OF NEW PAVING AND ADDITIONAL SITE MODIFICATIONS 03-28-05 REVISE BUILDING FOOTPRINT, ASSOCIATED GRADING

CHANGES, TITLE AND PROJECT NAME CHANGES

UNDERGROUND DETENTION SYSTEM CROSS SECTION #3

NOTE: DEBRIS IS TO BE KEPT OUT OF ALL

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

STORMWATER MANAGEMENT FACILITIES

DURING AND AFTER CONSTRUCTION.

(SECTION AT BEND)

OWNER/DEVELOPER

All pipes shall be circular in cross section.

construction equipment, rollers, or hand tampers—to assure maximum density and minimum permeability.

equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2,000 ohm-cm. Material shall be placed such that minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sides of the pipe.

It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When

using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction

equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the

any part of the structure. Under no circumstances shall equipment be driven over any part of a structure or pip nless there is a compacted fill of 24" or greater over the structure or pipe. Backfill (flowable fill)zone shall be of

the type and quality conforming to that specified for the core of the embankment or other embankment materials.

1. Materials — (Polymer Coated steel pipe)— Steel pipes with polymeric coating shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the

Materials - (Aluminum Coated Steel Pipe) - This pipe and its appurtenances shall conform to the requirements of ASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used

with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or

otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are

to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt

Corrugated Metal Pipe - All of the following criteria shall apply for corrugated metal pipe:

equirements of AASHTO Specifications M-245 & M-246 with watertight coupling bands or flanges.

ackfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to

1318 COMPANY LLC 10400 AUTO PARK DRIVE BETHESDA, MD 20817

**VOGEL ENGINEERING** 

shall detail erosion and sediment control measures.

**TIMMONS GROUP** P: 410.461.7666 F: 410.461.8961 www.timmons.com

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TAX MAP #34 BLOCK #6 TH ELECTION DISTRICT

6. Underdrains

PARCEL K-2

SOIL BORING DETAIL

The planting soil shall be tested and shall meet the following criteria

Since different lab calibrate their testing equipment differently, all testing results shall come from the same testing facility. Should the pH fall out of the acceptable range, it may be modified (higher) with lime or (lower) with iron sulfate plus sulfur.

Mulch should be placed to a uniform thickness of 2" to 3". Shredded hardwood much is the only accepted mulch. Pine mulch and wood chips will float and move to the perimeter of the bioretention area during a storm event and are not acceptable. Shredded mulch must be well aged (6 to 12 months) for acceptance.

Root stock of the plant material shall be kept moist during transport and on—site storage. The plant root ball should be planted so 1/8th of the ball is above final grade surface. The diameter of the planting pit shall be at least six inches larger than the diameter of the planting ball. Set and maintain the plant straight during the entire planting process.

Trees shall be braced using 2" by 2" stakes only as necessary and for the first growing season only. Stakes are to be equally spaced on the outside of the tree ball.

Grasses and legume seed should be drilled into the soil to a depth of at least one inch. Grass and legume plugs shall be planted following the non-grass ground cover plating specifications.

Underdrains are to be placed on a 3'-0'' wide section filter cloth. Pipe is placed next, followed by the gravel bedding. The ends of underdrain pipes not terminating in an observation well shall be capped.

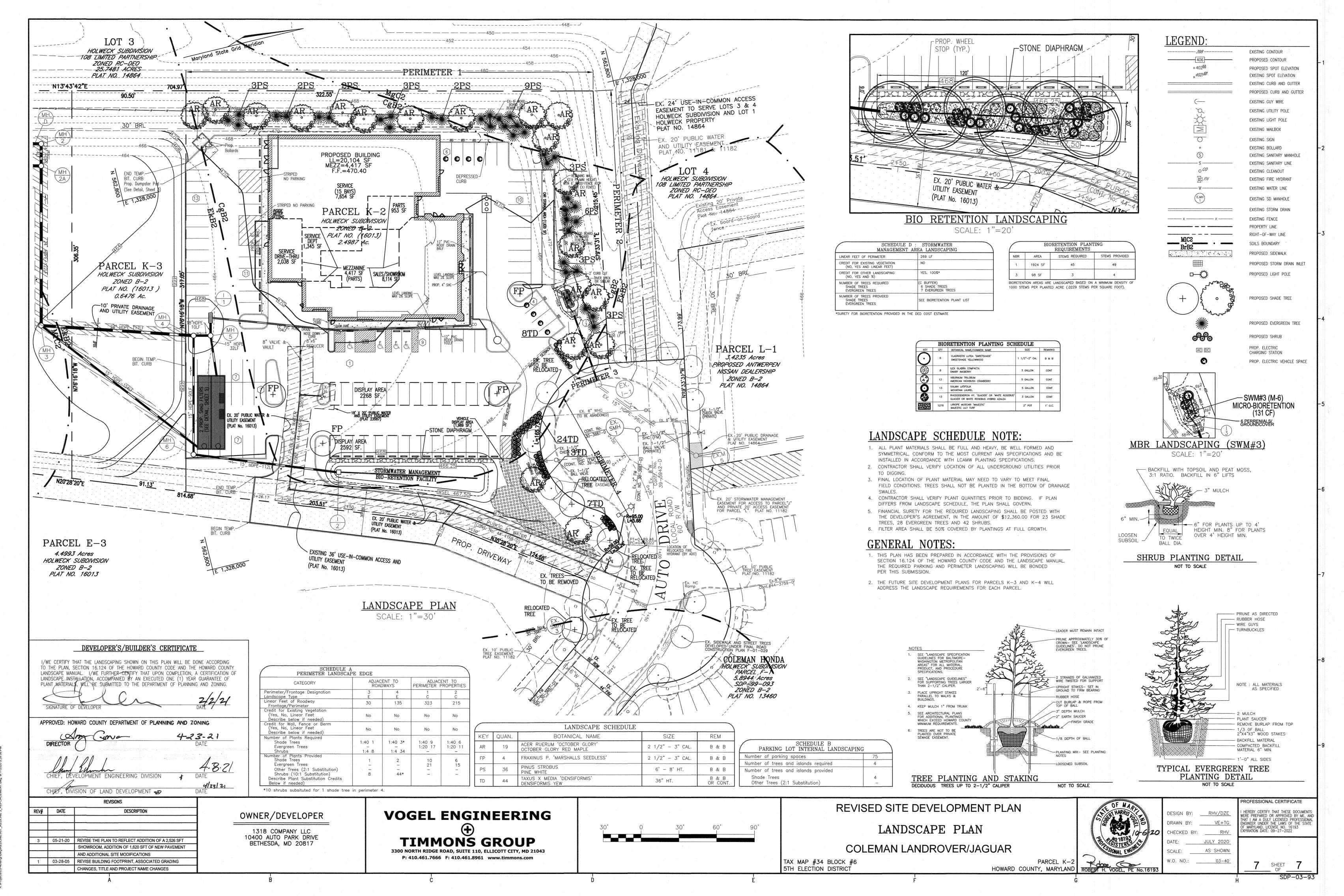
The topsoil specifications provide enough organic material to adequately supply nutrients from natural cycling. The primary function of the bioretention structure is to improve water quality. Adding fertilizers defeats, or at a minimum, impedes this goal. Only add fertilizer if wood chips or mulch are used to amend the soil. Rotatill urea fertilizer at a rate of 2 pounds of nitrogen per 1000 square feet.

The main collector pipe for underdrain systems shall be constructed at a minimum slope of 0.5%. Observation well and/or clean—out pipes must be provided (one minimum per every 1000 square feet of surface area).

pH range 5.2 - 7.0 organic matter 1.5 - % (by weight) magnesium 35 lb/ac phosphorus (phosphate - P 20.3 75 lb./ac potassium (potash - K 20) 85 lb./ac soluble salts nit to exceed 500 ppm

SILT: Brown, fine sands, mois loose to medium danse (ML)

HOWARD COUNTY, MARYLAND ROBERT H. VOGEL, PE No.16193



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