Contractor responsible to construct all handicap parking and handicap access in

Where drainage flows away from curb, contractor to reverse the gutter pan. 20. All elevations are to flowline/bottom of curb unless otherwise noted.

22. Public Water available along Auto Drive (8" Water) Contract #44-3323-D.

Public Sewer available along Auto Drive (1 1/2" Sewer (FM)) Contract #30-3687-D. 23. Stormwater Management shall be provided by an underground detention system for quantity attenuation. Water quality shall be provided by a sand filter structure, and existing Stormceptor will be utilized to provide pre-treatment. The proposed stormwater management system will be privatly owned and maintained by Antwerpen Nissan.

26. Traffic Impact Study prepared by The Traffic Group, dated November 14, 2002. 27. See sheet 4 for lighting detail. Lighting details provided for informational purposes only. See electrical and architectural plans for additional lighting information.

There are no wetlands on-site. 29. Department of Planning and Zoning related DPZ file: SP 93-14, WP 93-90, WP 01-20, F 92-161, F 94-38, ZB 947M, ZB1008M; Plat #11584, #11181, #11183, #11182, #14864

30. There will be no mezzanine levels permitted unless adequate parking has been provided

and approved by the Deptartment of Planning and Zoning. Geotech report prepared by Herbst/Benson \$ Associates on July 11, 2000.

32. Financial Surety for the required landscaping must be posted with the developer agreement in the amount of \$18,900 for 35 shade trees, 53 evergreen trees, and 15 shrubs. The landscape surety will be posted with the developers agreement. 33. Reference ZB 947M and ZB 1008M for zoning cases for this site:

A) ZB 947M: 1. Date of approval for Decision and Order: March 11, 1994 Rezoned 2.05 acres from B-2 to RC, and 1.99 acres from RC to B-2.

B) ZB 1008M

2. Conditions of approval: A 300' private easement buffer area between the zoned B-2 use on lot 4 and the existing residential house on lot 3. 1. Date of approval for Decision and Order: December 4, 2000 Rezoned 0.48 acres of Parcel "L" from B-2 to RC, which became part

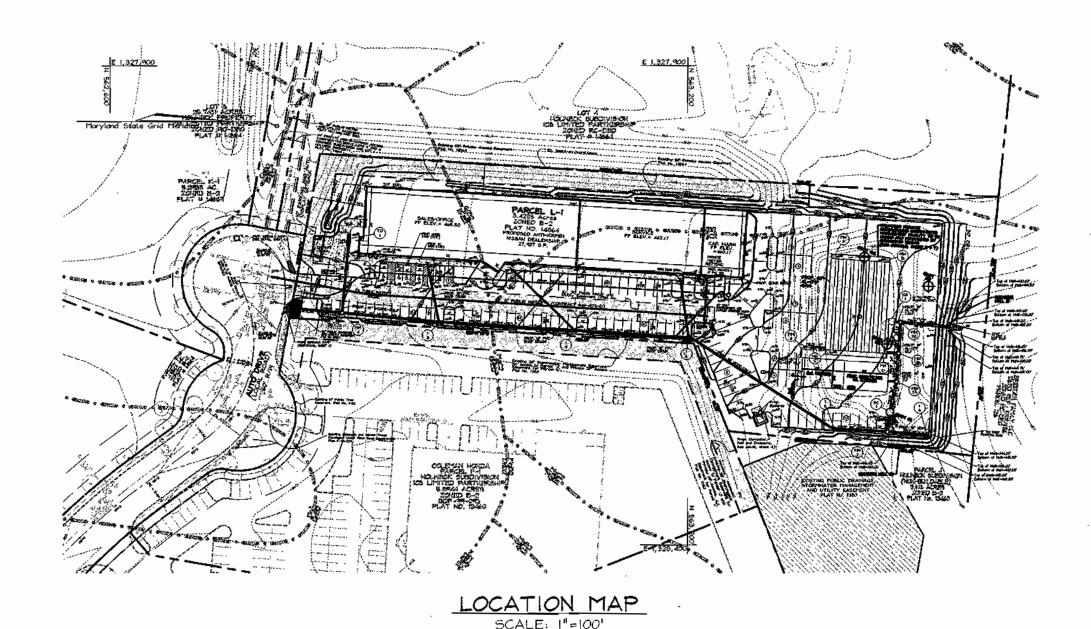
of Lot 4 and was included in the Agricultural Preservation Easement for

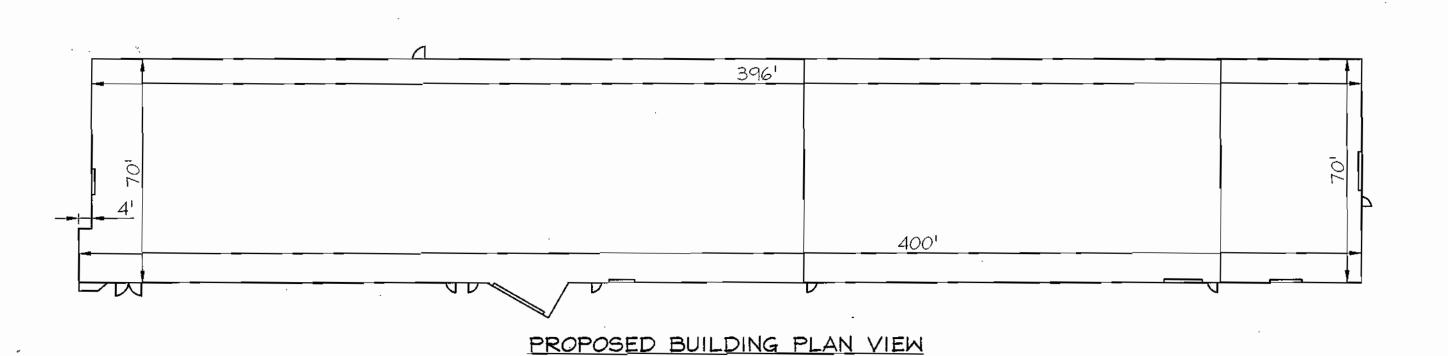
2. Conditions of approval: No conditions of approval were specified. 34. Debris is to be kept out of all stormwater management facilities during and

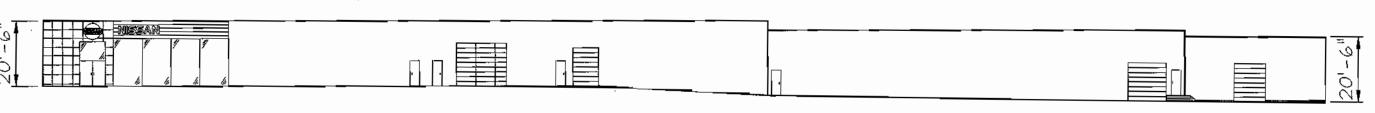
after construction. 35. Forest Conservation requirements for parcel L-1 are provided in conjuction with F-01-29. 36: Existing Stormwater Management Facility was bonded under F-01-029 by 108 Limited Partnership, C/O Win Kelly Chevrolet.

# ANTMERPEN NISSAN

SITE DEVELOPMENT PLAN PARCEL L-1







NOT TO SCALE

PROPOSED BUILDING ELEVATION

5 HANDICAP SPACES

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

PARKING TABULATION AUTOMOBILE DISPLAY: 27,943 SF REQUIRED 28 SPACES @ | SPACE/1000 SF SALES/OFFICE: 15,747 SF @ 2 SPACES/1000 SF 32 SPACES SERVICE BAYS: 18 BAY AUTOMOBILE SERVICE AREA @ 3 SPACES/SERVICE BAY 54 SPACES (SEE SCHEMATIC FOR DOOR LOCATIONS) TOTAL PARKING SPACES REQUIRED: 114 SPACES 114 SPACES INCLUDING

TOTAL PARKING SPACES PROVIDED:

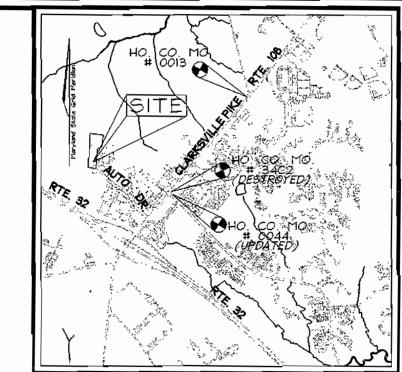
SHEET INDEX DESCRIPTION SHEET NO. Cover Sheet 1 of 9 2 of 9 Existing Conditions and Demolition Plan Site Development, Grading, and Sediment and Erosion Control Plan 3 of 9 Sediment Control Details And Miscellaneous Details 4 of 9 Storm Drain Plan, Drainage Area Map, And SWM Details 5 of 9 Site Landscape Plan 6 of 9 Water & Sewer Profiles and Details, SWM Notes and Details 7 of 9 Cornerstone Retaining Wall Designs 8 of 9 Cornerstone Retaining Wall Designs 9 of 9

LEGEND Existing Contour Proposed Contour Existing Spot Elevation Proposed Spot Elevation Direction of Flow Existing Trees to Remain Light Poles □-\$\time\$ Single Overhead \$\time\$-□-\$\time\$ Double Overhead Concrete

BENCHMARKS

HOWARD COUNTY BENCHMARK 34C2 (DESTROYED) N 562321.798 E 1329750.722 UPDATED: BENCHMARK 0044 N 562176.474 E 1329641.868 ELEV. 485.252

HOWARD COUNTY BENCHMARK 0013 N 564285.946 E 1331309.715 ELEV: 484.671



VICINITY MAP SCALE: 1"=2000'

OWNER/DEVELOPER Antoy LLC 12451 Auto Drive Clarksville, MD 21029-1266

		ADDRE	55	CHART				
LOT/PARCEL:	#	STREET ADDRESS						
L-1	1245!	12451 AUTO DRIVE						
								*
		PERMIT	INF	ORMAT	ION CHA	ART		
SUBDIVISION NAME SECTION/AREA PARCEL I		EL NUMBER						
Ho	Holweck Subdivision					L-I		
PLAT REF.	BLOCK NO.	ZONE	TA	X/ZONE	ELECT.	DIST.	CENSUS	TR.
14864	6	B2		34	5	th	6051	
17007				<b>5</b> 4			2031	
						_		

10.	Ę K	REVISION	DATE

COVER SHEET

SITE DEVELOPMENT PLAN ANTWERPEN NISSAN PARCEL L-1, HOLWECK SUBDIVISION

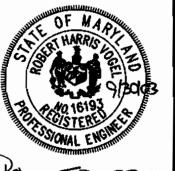
TAX MAP #34 BLOCK #6 5TH ELECTION DISTRICT

PARCEL L-HOWARD COUNTY, MARYLAND



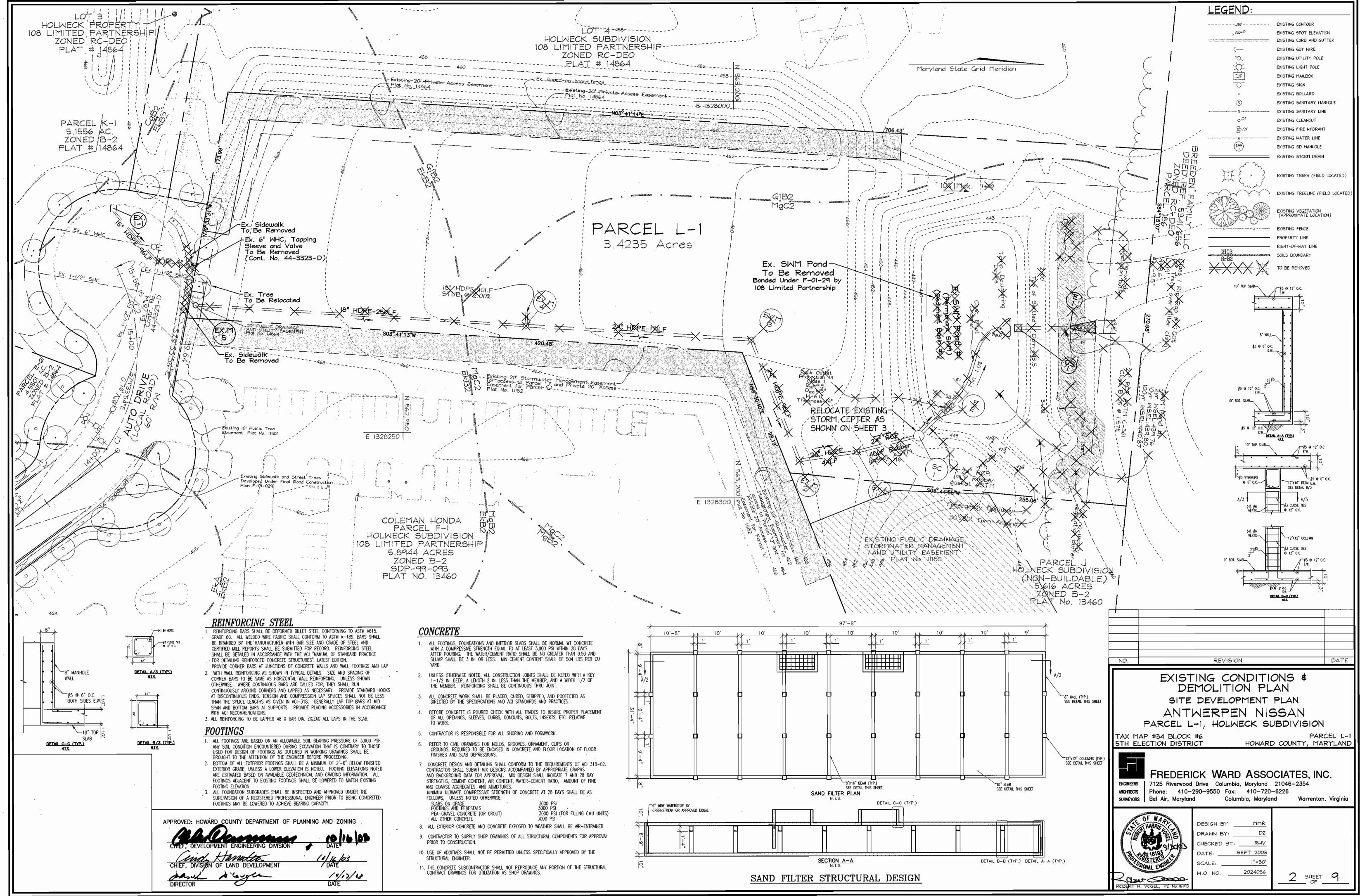
FREDERICK WARD ASSOCIATES, INC.

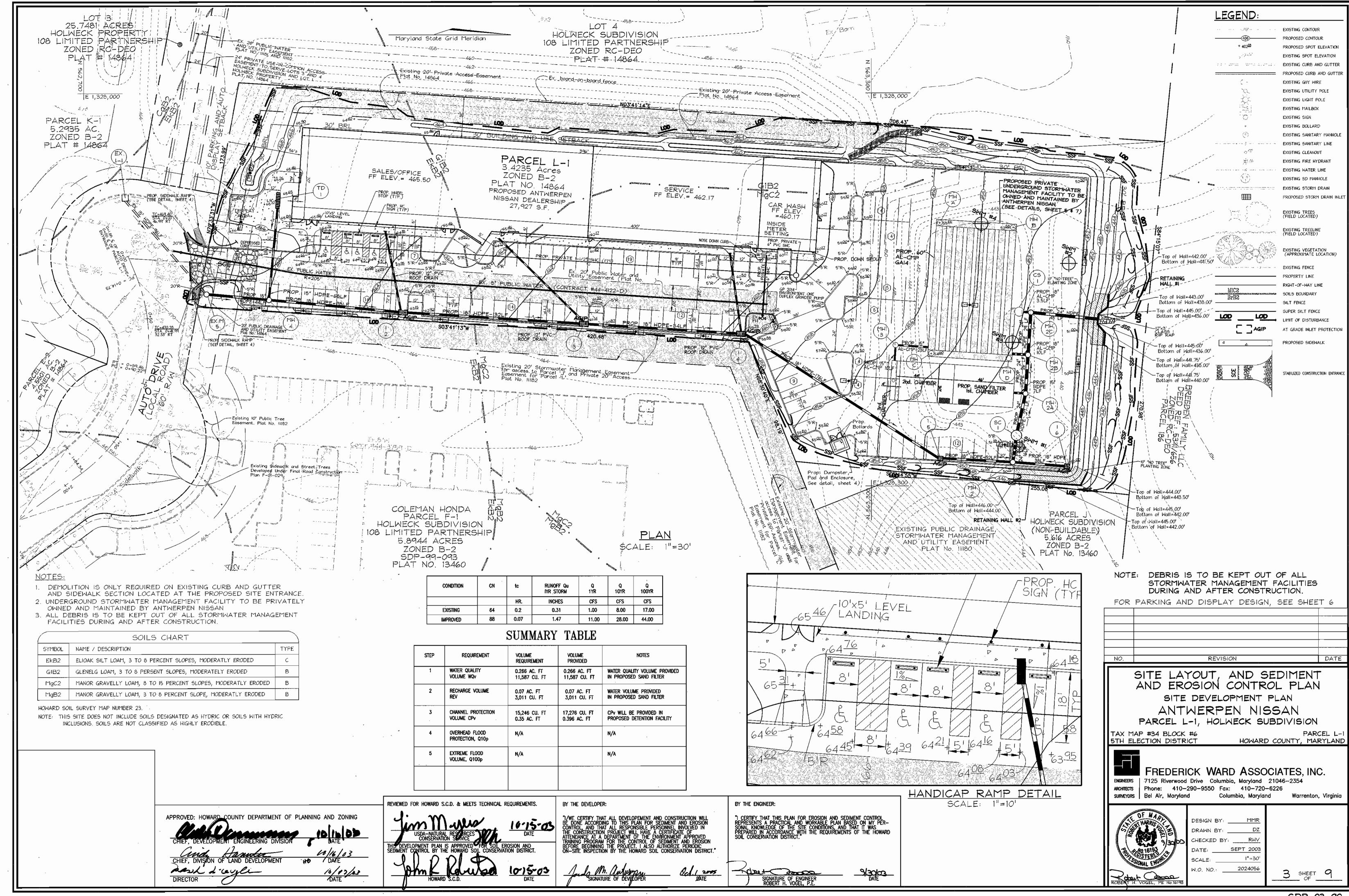
ENGINEERS | 7125 Riverwood Drive Columbia, Maryland 21046-2354 ARCHITECTS Phone: 410-290-9550 Fax: 410-720-6226 surveyors | Bel Air, Maryland Columbia, Maryland Warrenton, Virginia

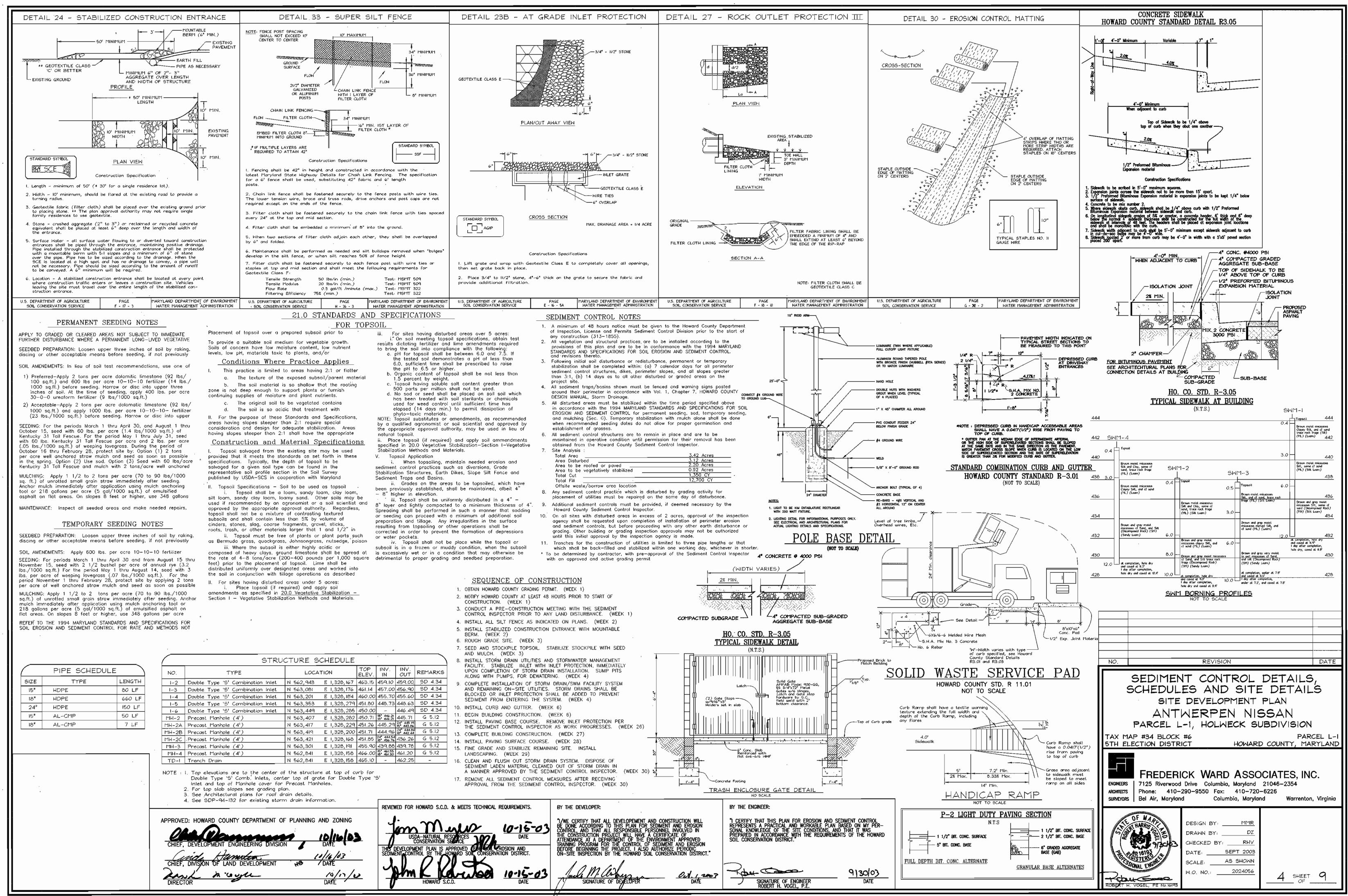


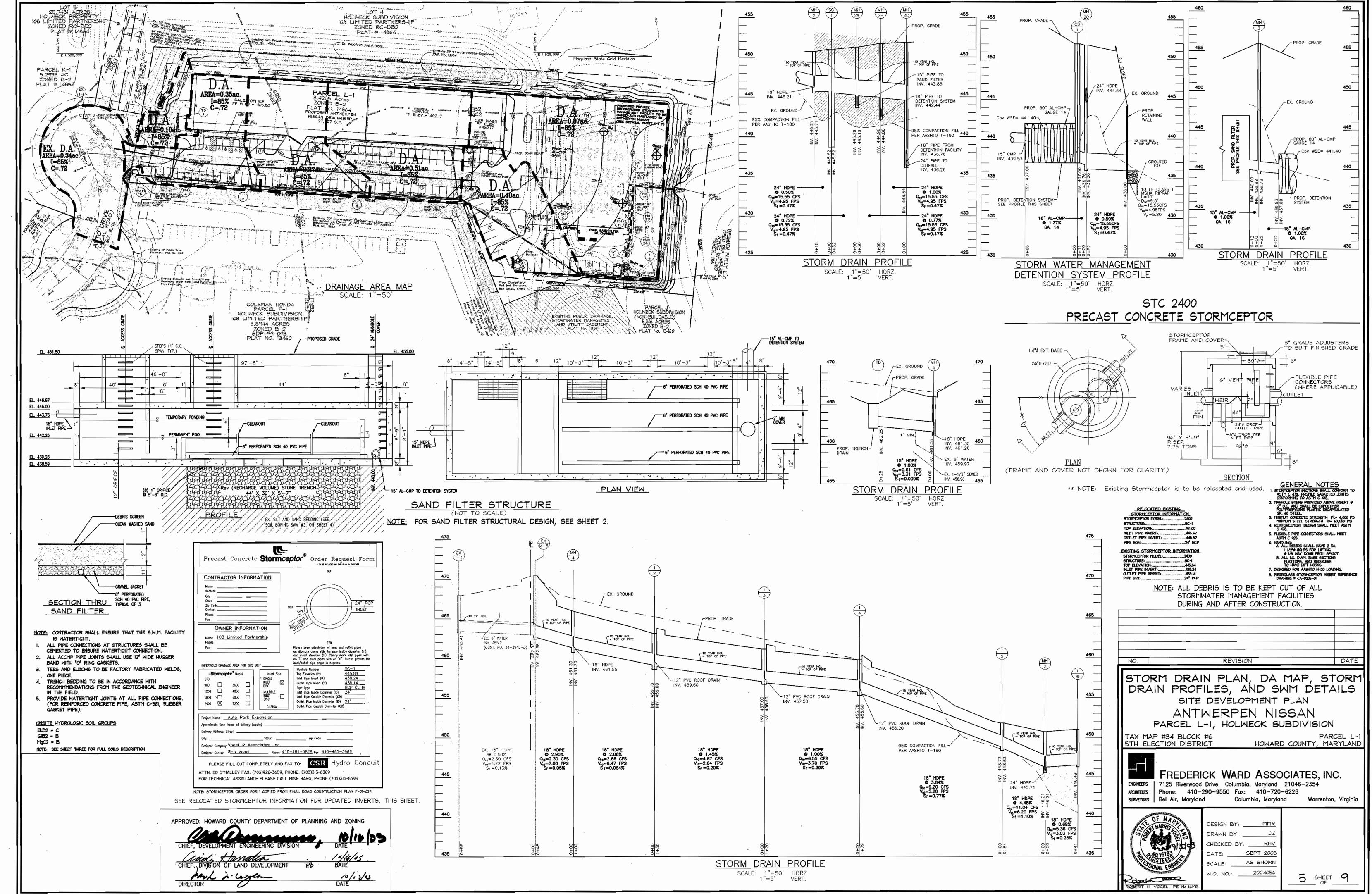
DESIGN BY: DRAWN BY CHECKED BY: SEPT 2003 AS SHOWN SCALE: 2024056 W.O. NO.:

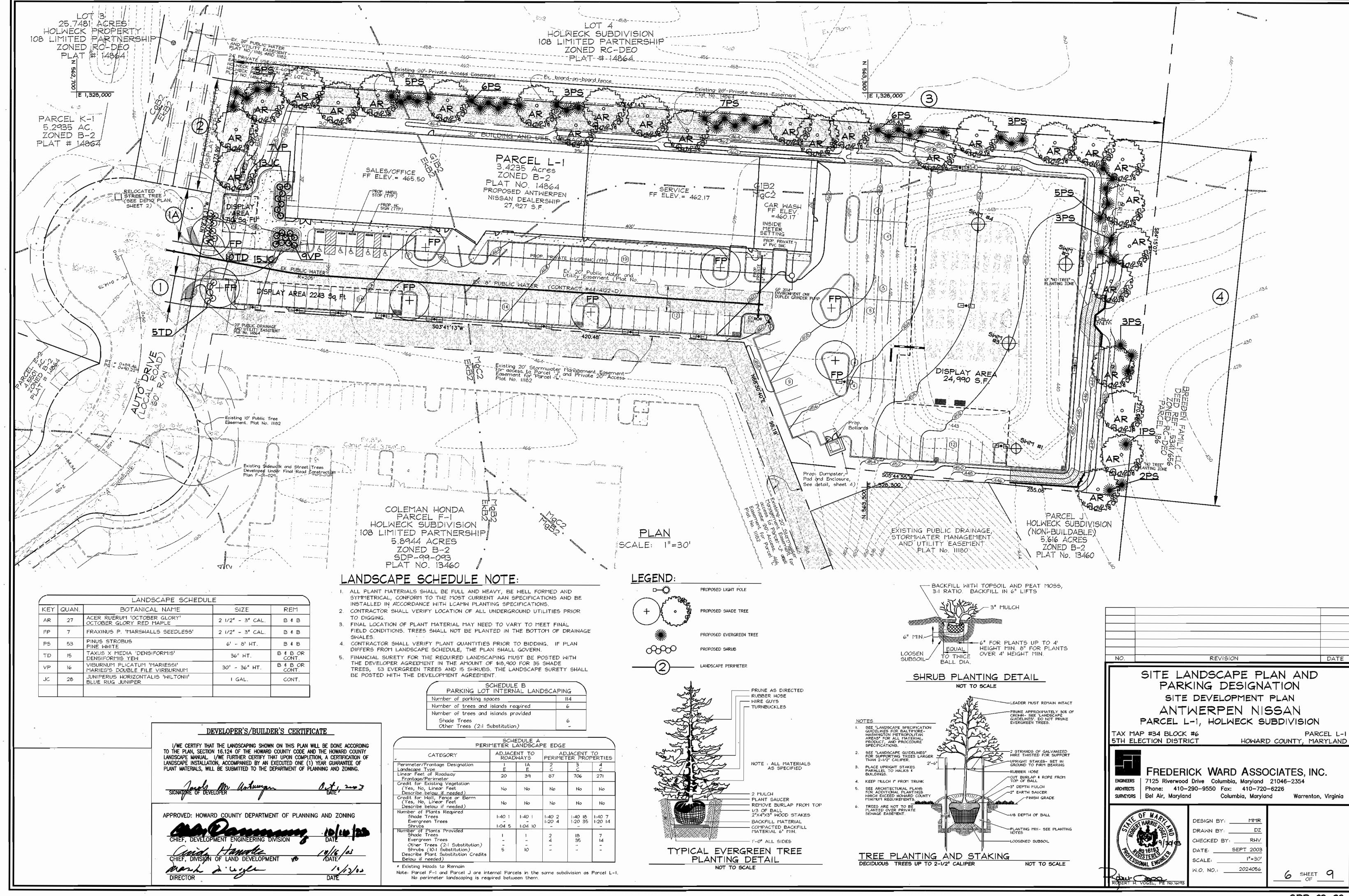
SHEET 9











### CONTROL STRUCTURE NOTES:

OR JOINTED PER MANUFACTURERS DESIGN.

- I. MANHOLES SHALL BE CONSTRUCTED IN ACCORDANCE WITH AASHTO M 199.
- 2. CONCRETE SHALL BE MIX NO. 6 (4500 P.S.I.) 3. WALL REINFORCEMENT FOR BASE UNITS AND RISER UNITS SHALL BE REINFORCEMENT BARS OR WELDED WIRE FABERIC WITH A MINIMUM AREA OF 0.22 In./ft FOR THE 84 DIAMETER
- MANHOLES. WELDED WIRE FABERIC SHALL CONFORM TO ASTM A 185 AND A 82. 4. BASE REINFORCEMENT TO BE REINFORCEMENT BARS OR WELDED WIRE FABERIC WITH A MINIMUM AREA OF 0.27 in./ft. THE BASE SHALL BE CAST MONOLITHIC WITH THE BASE UNIT
- 5. THE MANUFACTURER SHALL FORM MALE AND FEMALE ENDS OF JOINTS USING THEIR OWN DESIGN. THE JOINTS SHALL BE SEALED BY THE CONTRACTOR AND MADE WATERTIGHT USING (WHERE APPLICABLE) MORTAR, RUBBER O-RING GASKETS MEETING ASTM C361 AND C 443 OR FLEXIBLE PLASTIC GASKETS MEETING AASHTO M 198 TYPE B.
- 6. LADDER RUNGS SHALL BE INSTALLED IN VERTICAL ALIGNMENT AT 1'-4" MAXIMUM C/C. RUNG TYPES SHALL BE IN ACCORDANCE WITH STANDARDS MD-383.91 OR MD-383.92. LADDER RUNGS SHALL BE INCIDENTAL TO THE COST OF THE MANHOLE.
- 7. WHEN THE DISTANCE BETWEEN MULTIPLE PIPE OPENINGS IN THE BASE UNIT OR ANY RISER UNIT IS LESS THEN 6" ADDITIONAL NO. 3 BARS ARE REQUIRED AROUND OPENINGS.
- 8. LIFT HOLES OR LIFT EYES SHALL BE PROVIDED IN EACH SECTION FOR HANDLING 9, MIX NO. 2 CONCRETE OR BRICK CHANNEL SHALL BE PROVIDED IN THE FIELD AND SHALL SLOPE 2" PER FOOT TOWARD OUTLET OR AS DIRECTED BY THE ENGINEER.
- 10. THE DRIP STONE LANDING SHALL BE USED ONLY WHEN THERE ARE PIPES CONNECTED TO THE RISER UNITS. SEE STD.MD-384.13 FOR DETAILS.
- II. MINIMUM DEPTH PAYMENT PER EACH SHALL BE 101-11 MEASURED FROM THE BOTTOM OF THE BASE UNIT TO THE TOP OF THE MANHOLE COVER, VERTICAL DEPTH PAYMENT PER LINEAR FOOT SHALL INCLUDE ALL DEPTHS IN EXCESS OF 10'-15 THE COST OF THE DRIP STONE LANDING, NO. 57 AGGREGATE GROUT, SEALENT, AND ALL NESSASARY APPURTENANCES SHALL BE INCIDENTAL TO

### UNDERGROUND SAND FILTER CONSTRUCTION SPECIFICATIONS

- I. PROVIDE MANHOLE AND/OR GRATES TO ALL UNDERGROUND AND BELOW GRADE STRUCTURES, MANHOLES SHALL BE IN COMPLIANCE WITH STANDARD SPECIFICATIONS FOR EACH COUNTY, BUT DIAMETERS SHOULD BE 30" MINIMUM (TO COMPLY WITH OSHA CONFINED SPACE REQUIREMENTS), ALUMINUM AND STEEL LOUVERED DOORS ARE ALSO ACCEPTABLE, TEN INCH WIDE (MINIMUM) MANHOLE STEPS (12"O.C.) SHALL BE CAST IN PLACE OR DRILLED AND MORTARED INTO THE WALL BELOW EACH MANHOLE. A 5' MINIMUM HEIGHT CLEARANCE (FROM THE TOP OF THE SAND LAYER TO THE BOTTOM OF THE UPPER/SURFACE SLAB) IS REQUIRED FOR ALL PERMANENT UNDERGROUND STRUCTURES, LIFT RINGS ARE TO BE SUPPLIED TO REMOVE/REPLACE TOP SLABS ON PRE-FABRICATED STRUCTURES, MANHOLE COVERS SHOULD ALLOW FOR PROPER
- VENTILATION. 2. UNDERGROUND SANDFILTERS SHOULD BE CONSTRUCTED WITH A GATE VALVE LOCATED JUST ABOVE THE TOP OF THE FILTER BED FOR DEWATERING IN THE EVENT THAT CLOGGING OCCURS.
- 3. UNDERGROUND SAND BEDS SHALL BE PROTECTED FROM TRASH ACCUMULATION BY A WIDE MESH GEOTEXTILE SCREEN TO BE PLACED ON THE SURFACE OF THE SAND BED; SCREEN IS TO BE ROLLED UP, REMOVED, CLEANED AND RE-INSTALLED DURING

### PROCEDURE FOR RELOCATING EXISTING STORMCEPTOR

- 1. HAVE THE UNIT THOROUGHLY CLEANED ACCORDING TO MAINTENANCE PROCEDURES 2. EXCAVATE AROUND THE UNIT, BEING CAREFUL NOT TO DAMAGE THE COMPONENTS OF THE UNIT OR DISTURB THE PIPE GOING INTO THE STRUCTURE
- 3. CAREFULLY DISASSEMBLE THE UNIT FROM THE TOP DOWN, WITHOUT DISTURBING THE INTERNAL FIBERGLASS COMPONENTS. 4. INSTALL THE UNIT IN THE NEW LOCATION, FOLLOWING THE PROCEDURES LISTED

### IMPORTANT FACTORS WHEN RELOCATING THE STROMCEPTOR

- I. THE OLD GASKETS SHOULD BE DISCARDED AND THE JOINTS THOROUGHLY CLEANED BEFORE
- ASSEMBLING THE STRUCTURE WITH NEW GASKETS. 2. THE 2" RISER AT THE TOP OF THE CURRENT UNIT WILL NOT BE NEEDED AT THE NEW LOCATION
- THIS DOES NOT EFFECT THE UNIT IN ANY WAY. 3. THE 6" RISER AT THE TOP OF THE UNIT MAY NEED TO BE FIELD CUT TO MEET THE REQUIRED GRADE THE FLATTOP MAY BE REATTACHED BY BOLTING OR GROUTING IT IN PLACE. AGAIN, THIS WILL NOT EFFECT
- THE PREFORMANCE OF THE UNIT. 4. EXTREME CARE SHOULD BE TAKEN NOT TO DAMAGE ANY PART OF THE STRUCTURE DURING THE DISASSEMBLY/REASSEMBLY PROCESS.
- 5. THE INLET AND OUTLET ANGLES WILL NOT MATCH THE NEW CONFIGURATION EXACTLY. THE INLET PIPE SHOULD BE PERPENDICULAR TO THE UNIT, ANY VARIATION CAN BE ACHIEVED BY PUTTING THE OUTLET PIPE AT THE ANGLE NEEDED. THE INTERNAL COMPONENTS ALLOW FOR ADJUSTMENT TO THE OUTLET ANGLE.

### CONTRACTOR INSTALLATION INSTRUCTIONS: PRECAST CONCRETE STORMCEPTOR

- STAKE-OUT THE LOCATION OF THE STORMCEPTOR AND EXCAVATE HOLE. EXCAVATE ADEQUATE SPACE TO CONNECT INLET AND OUTLET PIPES TO UNIT. INSTALL A 12" DEEP (OR AS REQUIRED) LAYER OF COMPACTED (95% STANDARD PROCTOR DENSITY OR LOCAL AND STATE REQUIREMENTS, AS DIRECTED BY THE INSPECTOR) AGGREGATE SUBBASE AT
- BOTTOM OF EXCAVATION. INSTALL MULE OR SHORING, AS NEEDED. CHECK ELEVATION OF UNIT BY MEASURING ITS SECTIONS FROM BASE OF THE STORAGE CHAMBER (BOTTOM OF UNIT'S SLAB) TO THE INVERT OF STORMCEPTOR BYPASS CHAMBER INLET ELEVATION (FIBERGLASS INSERT). SUBTRACT THIS DISTANCE FROM DESIGN INVERT ELEVATION TO DETERMINE TOP OF SUBBASE ELEVATION. CHECK ELEVATION OF INSTALLED SUBBASE AND ADJUST AS NEEDED.
- SECURE INSPECTOR APPROVAL OF SUBGRADE AND SUBBASE.
- INSTALL STORAGE CHAMBER, INSTALL SCREW INSERTS INTO BASE OF STORAGE CHAMBER. ATTACH CABLES OR CHAINS TO ALL 3 LIFTING LUGS ON THE BASE SLAB. USING LARGE EQUIPMENT OR CRANE LIFT AND PLACE THE BASE SECTION OF THE STORAGE CHAMBER IN THE EXCAVATED HOLE ON THE SUBBASE. MAKE SURE THAT THE BASE IS LEVEL. SPECIFIC ALIGNMENT OF THIS PART IS NOT REQUIRED. INSTALL RUBBER GASKET ON BASE UNIT AND COAT WITH LUBRICATING GREASE (PROVIDED IN SHIPMENT), IF NOT PRELUBRICATED. INSTALL ADDITIONAL STORAGE CHAMBER SECTIONS, AS REQUIRED (PROCEDURE IS SAME AS STEP 8.).
- INSTALL REDUCING SLAB. (STORMCEPTOR MODELS STC-2400, STC-3600, STC-4800, STC-6000 AND STC-7200) CHECK THAT SECTION IS SET FLUSH, LEVEL AND IS AT THE PROPER ELEVATION. INSTALL RUBBER GASKET ON THE TRANSITION SLAB SPIGOT AND COAT WITH LUBRICATING GREASE (PROVIDED IN SHIPMENT).
- INSTALL BYPASS CHAMBER OF STORMCEPTOR WITH FACTORY INSTALLED STORMCEPTOR INSERT. LIFT BYPASS SECTION AND INSTALL, WHILE CHECKING ALIGNMENT AND GRADE OF INLET AND OUTLET DRAINAGE PIPES. CHECK TO MAKE SURE THE BYPASS CHAMBER IS SET FLUSH, LEVEL AND IS AT THE PROPER ELEVATION. THE BYPASS CHAMBER MUST BE ORIENTED SUCH THAT INLET PIPE DISCHARGES INTO THE V-SHAPED FIBERGLASS WEIRS (INSIDE INSERT). INSTALL RUBBER GASKET ON TOP OF BYPASS SECTION AND COAT WITH LUBRICATING GREASE, IF NOT PRELUBRICATED.
- INSTALL STORMCEPTOR DROP PIPES ACCORDING TO STC PIPE INSTALLATION PROCEDURE. INSTALL RISER SECTION. LIFT RISER SECTION AND INSTALL, WHILE CHECKING THAT SECTION IS SET FLUSH AND IS AT PROPER ELEVATION AND THAT UNIT IS LEVEL. SPECIFIC ALIGNMENT OF THIS PART IS REQUIRED IF STEP(S) ARE INCLUDED. ALIGN STEPS ABOVE INLET
- INSPECTION PORT. NOTE: FOR SHALLOW INSTALLATIONS THIS SECTION MAY NOT BE REQUIRED. INSTALL TOP CAP WITH OPENING FOR STORMCEPTOR COVER. IF OPENING IS OFFSET (NOT CENTERED) THE TOP CAP OPENING SHOULD BE ORIENTED ABOVE THE STORMCEPTOR
- INLET INSPECTION PORT (PLUG). 10. BACKFILL STORMCEPTOR WITH APPROVED BACKFILL MATERIAL (NO ORGANIC OR TOPSOIL IS TO BE USED FOR BACKFILL). BACKFILL AND COMPACT IN 8 INCH LIFTS. BACKFILL SHOULD BE COMPACTED TO 95% OF STANDARD PROCTOR DENSITY, OR LOCAL AND STATE
- REQUIREMENTS, AS DIRECTED BY THE INSPECTOR. INSTALL AND SET GRADE ADJUSTING RINGS, AS NEEDED.
- 12. INSTALL AND SET STORMCEPTOR FRAME AND COVER. 13. INSTALL INLET AND OUTLET STORM DRAIN PIPES. CONNECT INLET AND OUTLET STORM DRAIN PIPES WITH FLEXIBLE BOOTS (WHEN PROVIDED) AND WITH NON-SHRINK GROUT
- WHEN NO FLEXIBLE BOOTS ARE PROVIDED. THE INVERT OF THE INLET AND OUTLET PIPE IS TO MATCH WITH THE INVERT OF THE STORMCEPTOR INSERT. FLEXIBLE BOOT INSTALLATION PROCEDURES : CENTER THE PIPE IN THE BOOT OPENING. LUBRICATE THE OUTSIDE OF THE PIPE AND/OR THE INSIDE OF THE BOOT IF THE PIPE OUTSIDE DIAMETER IS THE SAME AS THE INSIDE DIAMETER OF THE BOOT. POSITION THE PIPE CLAMP IN THE GROOVE OF THE BOOT WITH THE SCREW AT THE TOP. TIGHTEN THE PIPE CLAMP SCREW TO 60 INCH POUNDS. IF THE PIPE IS MUCH SMALLER THAN THE BOOT, LIFT THE BOOT SUCH THAT IT CONTACTS THE BOTTOM OF THE PIPE WHILE TIGHTENING THE CLAMP TO ENSURE EVEN CONTRACTION OF THE RUBBER. MOVE THE PIPE HORIZONTALLY AND/OR
- VERTICALLY TO BRING IT TO GRADE. 14. THE STORMCEPTOR SHOULD BE PUMPED OUT WHEN THE SEDIMENT CONTROL MEASURES ARE REMOVED (SITE PERMANENTLY STABILIZED).

### 15. FINAL INSPECTION.

### OPERATION AND MAINTAINANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED UNDERGROUND FACILITIES

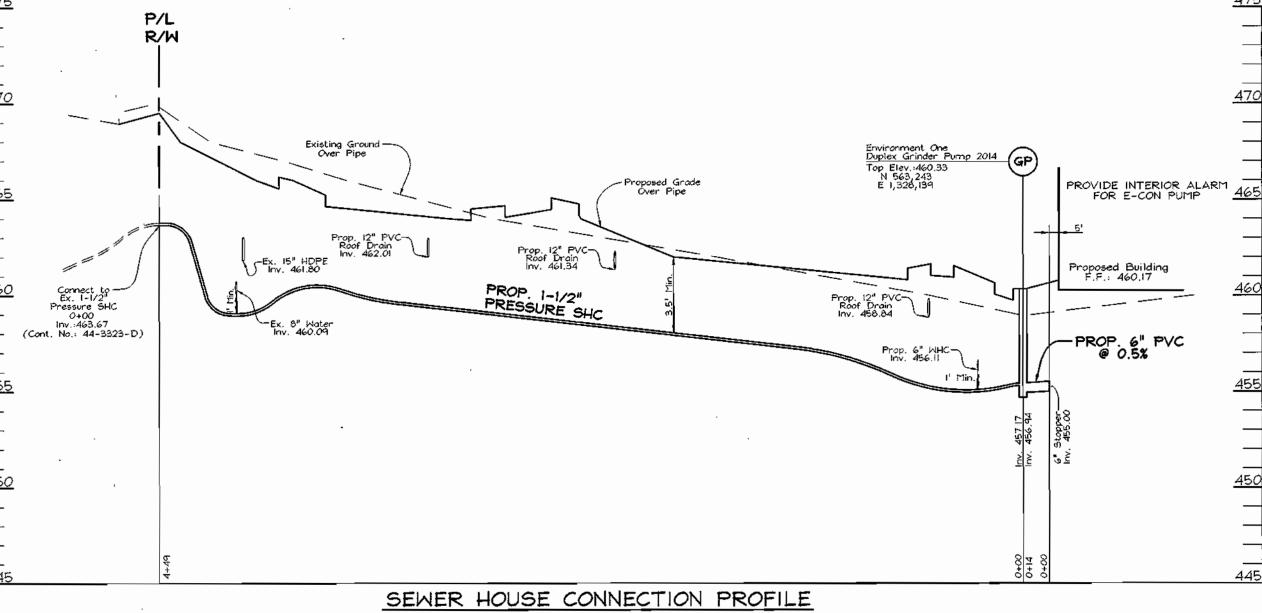
- THE SEDIMENT CHAMBER OUTLET DEVICES SHALL BE CLEANED AND/OR REPAIRED WHEN DRAWDOWN TIMES WITHIN THE CHAMBER EXCEED 36 HOURS. DEBRIS AND LITTER SHALL BE REMOVED AS NECESSARY TO INSURE PROPER OPERATION OF THE SYSTEM.
- SEDIMENT SHALL BE CLEANED OUT OF THE SEDIMENTATION CHAMBER WHEN IT ACCUMULATES TO A DEPTH OF 6 INCHES. VEGETATION WITHIN THE SEDIMENT CHAMBER SHALL BE LIMITED TO THE HEIGHT OF 18 INCHES.
- WHEN WATER PONDS ON THE SURFACE OF THE FILTER BED FOR MORE THAN 72 HOURS, THE TOP FEW INCHES OF DISCOLORED MATERIAL SHALL BE REPLACED WITH FRESH MATERIAL. PROPER CLEANING AND DISPOSAL OF THE REMOVED MATRIALS AND LIQUID MUST BE FOLLOWED BY THE OWNER.
- A LOGBOOK SHALL BE MAINTAINED TO DETERMINE THE RATE AT WHICH THE FACILITY DRAINS.
- THE MAINTENANCE LOGBOOK SHAL BE AVAILABLE TO HOWARD COUNTY FOR INSPECTION TO INSURE COMPLIANCE WITH OPERATION AND MAINTENANCE CRITERIA.
- ONCE THE PERFORMANCE CHARACTERISTICS OF THE INFILTRATION SYSTEM HAVE BEEN VERIFIED, THE MONITORING SCHECUDULE CAN BE REDUCED TO AN ANNUAL BASIS UNLESS THE PROFORMANCE DATA INDICATES THAT A MORE FREQUENT SCHEDULE IS REQUIRED.

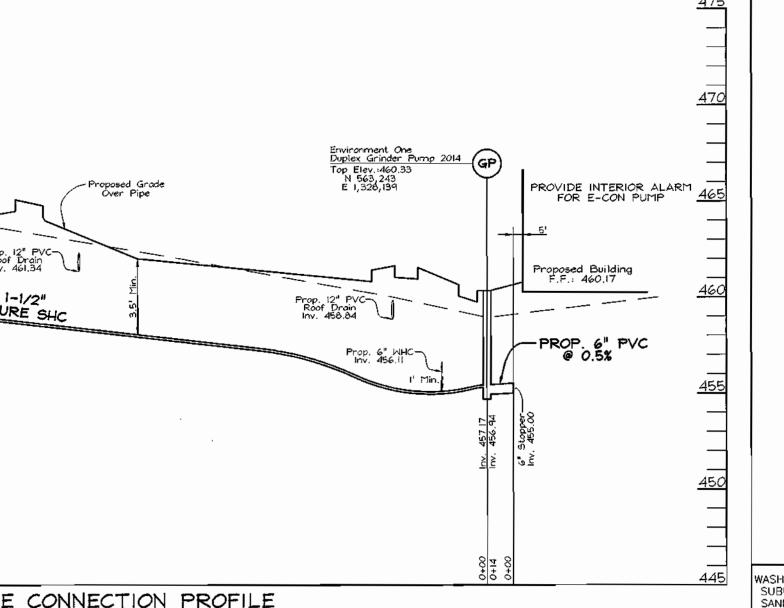
### OPERATION AND MAINTAINANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED <u>UNDERGROUND FACILITIES</u>

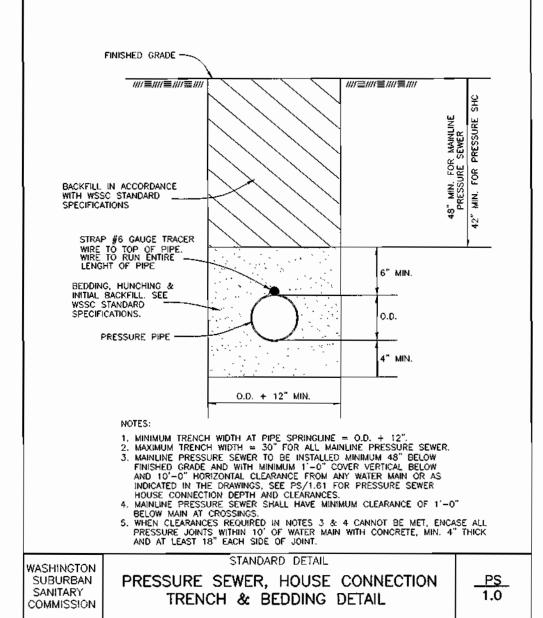
- THE UNDERGROUND STORMWATER MANAGEMENT FACILITY IS PRIVATELY OWNED AND IT SHALL BE THE RESPONSIBILITY OF THE OWNER TO PERIODICALLY INSPECT AND CLEAN THE FACILITY TO MAINTAIN IT'S OPERATION AND FUNCTION.
- THE UNDERGROUND STORMWATER MANAGEMENT FACILITY SHALL BE INSPECTED YEARLY AT A MINIMUM AND AFTER ESPECIALLY SEVERE STORM EVENTS.
- WHEN SEDIMENT ACCUMULATION OF MORE THAN 2" IS OBSERVED OR ANY DEBRIS THAT MIGHT OBSTRUCT THE OUTFALL IS OBSERVED, THE FACILITY SHALL BE CLEANED.
- E. THE SEDIMENT AND DEBRIS SHALL BE REMOVED FROM THE UNDERGROUND STORMWATER MANAGEMENT FACILITY BY VACUUM TRUCK OR OTHER MANUAL MEANS. THE OWNER SHALL FOLLOW PROPER CLEANING AND DISPOSAL OF THE REMOVED MATERIAL AND LIQUID.
- F. THE INLET AND OUTLET PIPES SHALL BE CHECKED FOR ANY OBSTRUCTIONS AT LEAST ONCE EVERY SIX (6) MONTHS. IF OBSTRUCTIONS ARE FOUND, THE OWNER SHALL HAVE THEM REMOVED AND PROPERLY DISPOSED OF.

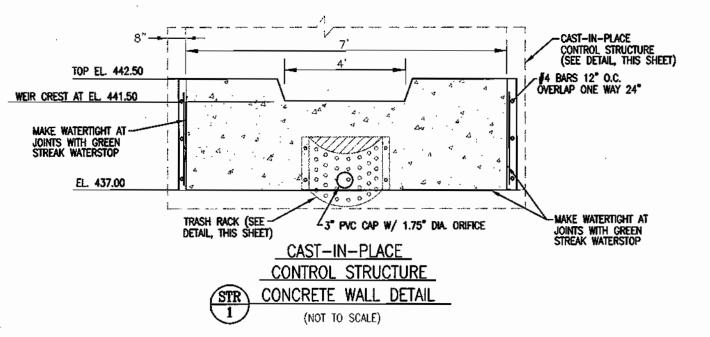
# Environment One Duplex Grinder Pump 2014 GP FOR E-CON PUMP Prop, 12" PVC-Roof Drain Inv. 462.01 PRESSURE SHC (Cont. No.: 44-3323-D 6" PVC SEWER HOUSE CONNECTION PROFILE SCALE : HORIZONTAL - I"=50"

VERTICAL - 1"=5"









## CONTROL STRUCTURE CONCRETE WALL NOTES

- I. CONCRETE WALL SHALL BE MIX NO. 6 (4500 p.s.i.) 2. WALL REINFORCEMENT SHALL BR REINFORCEMENT BARS OR WELDED WIRE FABERIC WITH A MIN. AREA OF 0.21 in./ft WELDED WIRE FABERIC.
- 3. WALL SHALL BE CAST-IN-PLACE, POURED MONOLITHIC USING FORMS. IN LIEU OF MONOLITHIC POUR, WALL SHALL BE CONNECTED BY OVERLAPPING BARS ONE WAY 24", AND SEALED WITH GREEN STREAK WATERSTOP (OR APPROVED EQUIVALENT)

OPERATION AND MAINTENANCE SCHEDULE FOR STORMCEPTOR WATER QUALITY STRUCTURE

1. The stormceptor water quality structure shall be periodically inspected and cleaned to maintain operation and function. The owner shall inspect the

monitoring form. Inspections shall be done using a clear Plexiglass tube

2. The stormceptor water quality structure shall be checked and cleaned

3. The maintenance of the stormceptor unit shall be done using a vacuum

4. The injet and outliet pipes shall be checked for any obstructions at least

materials and liquid must be followed by the owner.

available to the Howard County officials upon their request.

Manual, the unit must be cleaned.

regulatory agencies

stormceptor unit yearly at a minimum, utilizing the stormceptor inspection/

("sludge/judge") to extract a water column sample. When the sediment depths exceed the level specified in Table 6 of the Stormceptor Technical

immediately after petroleum spills. The owner shall contact the appropriate

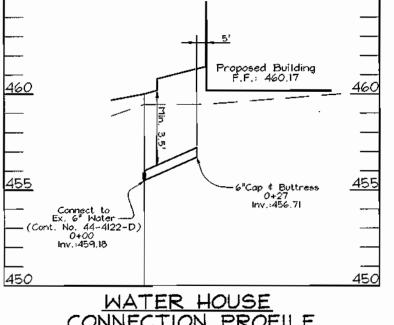
truck which will remove the water, sediment debris, floating hydrocarbons,

and other materials in the unit. Proper cleaning and disposal of the removed

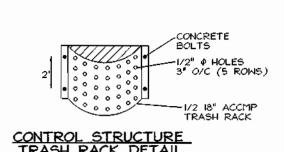
once every six months. If abstructions are found, the owner shall have them

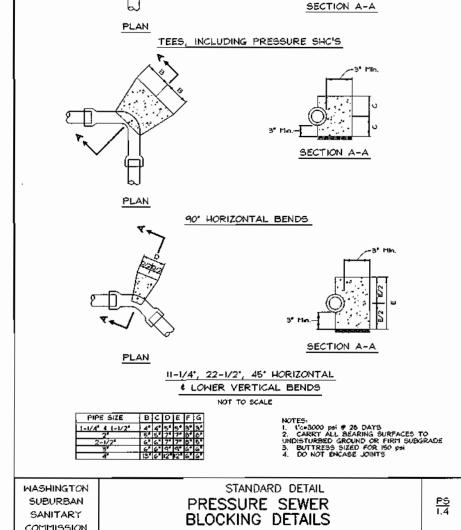
5. The owner shall retain and make the stormceptor inspections/monitoring forms

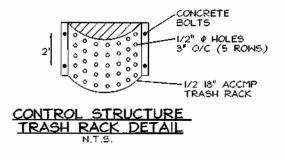
removed. Structural parts of the stormceptor unit shall be repaired as needed











- 36" ACCESS MANHOLE TO BE: 14 Ga. 2 2/3"x 1/2" CORRUGATION ALT2 CSP WITH STEPS WELDED TO CSP ⊕ 12" C.C. -H-12 HUGGER 8AND ~60° STORAGE PIPE

DEBRIS IS TO BE KEPT OUT OF ALL STORMWATER MANAGEMENT FACILITIES DURING AND AFTER CONSTRUCTION.

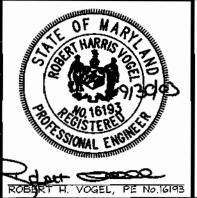
<u>ACCESS MANHOLE DETAIL FOR</u> UNDERGROUND SWM FACILITY (NOT TO SCALE)

REVISION WATER AND SEWER PROFILES AND DETAILS SITE DEVELOPMENT PLAN ANTWERPEN NISSAN

PARCEL L-1, HOLWECK SUBDIVISION TAX MAP #34 BL*O*CK #6 5TH ELECTION DISTRICT

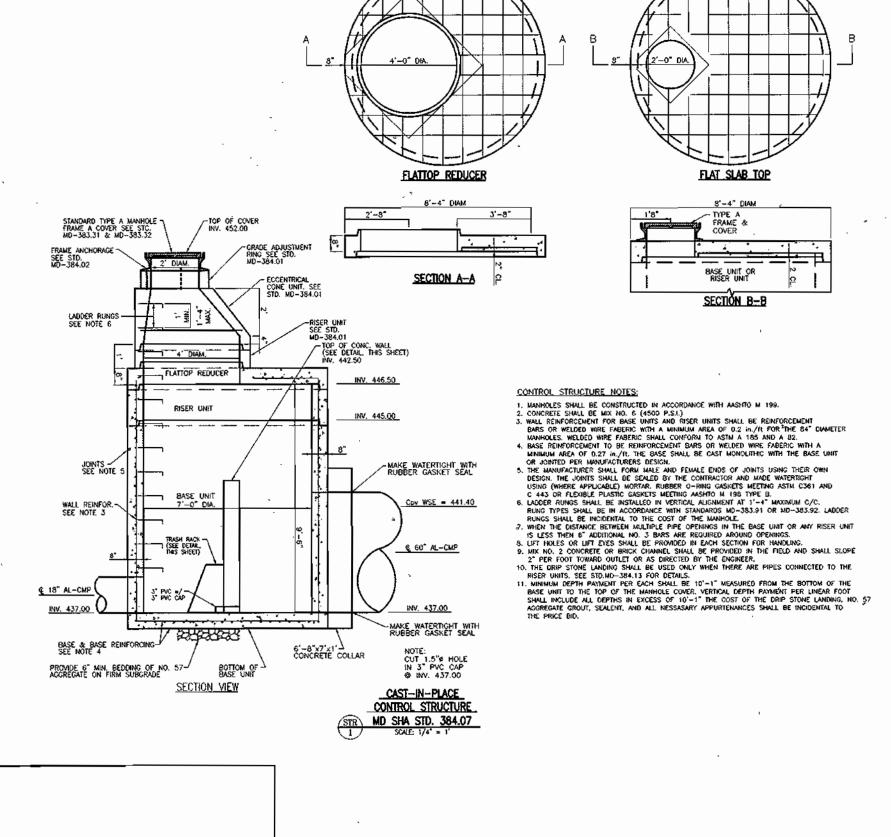
PARCEL L-HOWARD COUNTY, MARYLAND

FREDERICK WARD ASSOCIATES, INC. ENGINEERS 7125 Riverwood Drive Columbia, Maryland 21046-2354 Phone: 410-290-9550 Fax: 410-720-6226 SURVEYORS | Bel Air, Maryland Columbia, Maryland Warrenton, Virginia



DESIGN BY: DRAWN BY CHECKED BY: SEPT 2003 AS SHOWN SCALE: 2024056

SHEET \_\_ OF \_\_



CPY WATER SURFACE −60" AL-CMP (TYP.) GAUGE 14 ELEVATION = 444.90-- 140 N MARAFI GEOTEXTILE FABRIC (TYP.) (typ.) STONE (typ.) UNDERGROUND DETENTION SYSTEM CROSS SECTION

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

DATE

### GENERAL NOTES

1. SOIL PARAMETERS; Based on review of the geotechnical investigation for this site dated 07/11/2000 done by Herbst/ Benson & Associates, an internal angle of friction of 28° was used for the foundation soils (under the wall's gravel leveling pad) and retained soils (behind the wall's reinforced geogrid zone and extending to a distance that is twice the exposed wall height) in this design. This is for a worst case ML (silt) soil type. The borings indicate in-situ soil types of MH (elastic silt), ML and SM (silty sand). The ML and SM soils are acceptable, however the MH is not (the elevation of the MH soil lenses in bonings SWM1 and SWM2 are below the wall's sub-grade so there should not be an impact). CH (fat clay), MH and OH/ OL/ PT (organic) soil types are not acceptable for wall construction. If these unsuitable soils are encountered they must be removed and replaced with soils that meet or exceed the design friction angle of 28. The site geotechnical engineer shall monitor this closely during the construction process. SELECT "CRUSHER RUN" GRAVEL INFILL is required for the entire reinforced geografic zone. This gravel must be CR6, RC6, 21A or 2A modified (clean gravel, such as #57, is not acceptable since its unit weight is too light). The crusher run gravel infill is necessary to enhance the reinforced geogrid zone and minimize the geogrid lengths. A 35° internal angle of friction was used for the infill gravel. Unit weights of 126 PCF (maximum wet density) for the 28° soils (based on the maximum dry density of 109.2 plus an optimum moisture content of 15.8%) and 135 PCF for the crusher run gravel were used. Fluctuations of 5 PCF higher or lower will not affect this design, however if the actual unit weights vary by more than 5 PCF Ryan & Associates (RA) shall be notified so that revised cross sections can be run to verify that all factors of safety are still met. No cohesion was used in any of the calculations.

BEARING CAPACITY: The sub-grade must be tested prior to installation of the gravel leveling pad and have a minimum allowable bearing capacity of 2,500 PSF. The sub-grade must be virgin (natural undisturbed soil) or fill compacted to 95% of a standard proctor maximum dry density. The actual highest bearing pressure exerted by this wall (including any slopes or surcharges above) is 2,165 PSF. Any areas of the sub-grade that do not meet this maximum bearing pressure will require undercutting or an enlarged geogrid reinforced leveling pad.

3. CONSTRUCTION OVERSIGHT: The foundation soils must be examined by the site geotechnical engineer to assure that the actual foundation soil strength meets or exceeds that required in the "Bearing Capacity" section (item #2 above). The construction of this wall must be performed under the observation of a Maryland Registered Professional Engineer to ensure that it is built in accordance with the RA specifications.

<u>4. MANDATORY GRAVEL INFILL;</u> As stated under "Soil Parameters" above, all "crusher run" gravel (CR6, RC6, 21A or 2A modified) must be used for the entire reinforced geogrid zone of this wall. NOTE: clean gravel (such as #57 cannot be used because its unit weight is too light). The site geotechnical engineer must verify that crusher run

5. TANGENT ANGLES: The angle points as drawn on the civil plans at stations 0+10, 0+19, 0+28 and 0+37 should be built as radii (inside and outside curves) to prevent vertical joints.

6. FACTORS OF SAFETY: The following factors of safety have been met in this design: Sliding 1.5, Overturning 2.0, Bearing Capacity 2.0, Geogrid Overstress 1.5 and Geogrid Pullout 1.5 (from the block and from the soil).

7. SLOPES & SURCHARGES: The load of a 2:1 slope was applied to this wall. There are no proposed surcharges above this wall (the live load surcharge exerted by the paving and vehicles is more than twice the exposed wall height away so it is out of the wall's zone of influence).

8. GEOGRIDS: Synteen SF35 and SF55 geogrids, which have LTDS (Long Term Design strengths) of 1371 and 2303 respectively, were used in this design. Mirafi 3XT and 7XT geograds, which have higher LTDS of 1588 and 2961 respectively, are acceptable substitutions. All other geogrid substitutions must have prior approval of RA.

9. SPECIFICATIONS: Construction and materials must conform to the attached "Ryan & Associates segmental retaining wall specifications and installation guidelines for Cornerstone".

10. WALL BATTER; The batter of the Cornerstone units is 4.5" (5/8" setback per block course). NOTE: it is important for the wall installer and the civil engineer/ surveyor to predetermine the wall's batter during stake out. The base of the wall will need to be moved forward if there are critical dimensions that must be met on the high

11. BLOCK SYSTEM: This design is valid only for the Cornerstone system. Each segmental wall system has unique dimensions, connection devices and interacts differently with geogrids; therefore other block types may not be

12. BACK SLOPE: Water management is critical since there is a back slope behind this wall. If water is directed to the wall the top 6" of fill over the reinforced zone must have impermeable soil (clay- such as CL, GC or SC) compacted to a minimum of 90% or an underlying geo-membrane. A swale may be constructed (clay, concrete or asphalt— see detail) behind the wall to divert the water around the end(s) of the wall. This swale should have a minimum depth of 8" and a minimum 1-2% slope laterally from the high point to the end(s) of the wall. Or the water be allowed to pond and saturate the reinforced geogrid zone or be introduced into the 12" drainage layer. NOTE: the soils in the back slope and retained zone (within the wall's zone of influence: behind the reinforced geogrid zone and extending to a distance that is twice the wall's exposed height) must be virgin (natural undisturbed soil) or fill compacted to 95% of a standard proctor maximum dry density. This must be verified by the site geotechnical engineer.

13. EMBEDMENT: 5 blocks decreasing to 2 blocks from station 0+00 to 0+10, 2 blocks increasing to 3 blocks from 0+10 to 0+19, 3 blocks from 0+19 to 0+28, 3 blocks decreasing to 2 blocks from 0+28 to 0+37 and 2 blocks increasing to 7 blocks from 0+37 to 0+48.

14. WALL PROFILE: The elevation drawing was done to represent the grade changes necessary on the civil drawings. It was done in even block course increments of .667' (8"). Minor field changes may be necessary by the wall installer to achieve the desired grade changes. Lineal footage may be added or subtracted if the wall's height is equal to or less than the design height. If the wall needs to be raised in height, RA must be notified so that revised cross sections can be run. NOTE: the cap height of .333' is not shown on the profile drawings, however its height may have been used in some cases to obtain the desired TW elevations.

15. "HDPE" STORMWATER PIPE INTERSECTING THE WALL; A 24" HDPE intersects the wall at approximate station 0+23. The wall may not bear on plastic or steel pipes (such as ADS, CMP, HDPE, PVC, SLCPP, etc.) or utilities (such as electric, gas or water lines) therefore a cast in place lintel must be made in the field. This lintel shall be constructed of 3,000 PSI concrete and have dimensions of 8" high (one block course) X 1' wide (one block width) X a minimum length of 5'. Three #5 grade 60 bars shall be placed 5" down from the top (3" up from the bottom). The two outer bars shall be 3" in from the sides and the middle bar evenly spaced in between. The lintel shall bear a minimum of 9" (half a block width) on either side and the maximum free span shall be 4'. NOTE: riprap stone must be placed in front of the wall between stations 0+19 & 0+28 as shown on the grading

16. SEPARATE 8 1/8" X 11" SUBMITTAL: An 8 1/8" X 11" booklet has also been submitted. This booklet has the complete five page cross sections with earth pressure and all factors of safety.

### 17. SPECIAL HOWARD COUNTY RETAINING WALL SPECIFICATIONS: a. Retaining walls shall only be constructed under the observation of a Registered Professional Engineer and a

(NICET, WACEL or equivalent) certified soils technician. b. The required bearing pressure beneath the footing of the wall shall be verified in the field by a certified soils technician. Testing documentation shall be provided to the Howard County Inspector prior to the start of construction. The required test procedure shall be the Dynamic Cone Penetrometer Test ASTM STP-399.

c. The suitability of the fill material shall be confirmed by the on-site soils technician. Each eight inch lift must

be compacted to 95% Standard Proctor Density and the testing report shall be made available to the Howard

County inspector upon completion of the construction. d. For walls over ten feet in height, one soil boring is required every 100 feet along the length of the wall, copies of the boring reports shall be provided to the Howard County Inspector prior to the start of construction.

# GENERAL NOTES AND SPECIFICATIONS

### SPECIFICATIONS FOR SEGMENTAL RETAINING WALL SYSTEMS

A. Work includes furnishing and installing segmental retaining wall (SRW) units to the lines and grades designated on the construction drawings. Also included is furnishing and installing appurtenant materials required for construction of the retaining wall as shown on the construction

### 1.02 Reference Standards

Sampling and Testing Concrete Masonry Units
Tensile Properties of Geotextiles by the Wide-Width Strip Method. C. ASTM D 5262- Test Method for Evaluating the Unconfined Creep Behavior of Geo-

Single Rib Geogrid Tensile Strength

Geogrid Pullout Moisture Density Relationship for Soils, Standard Method G. ASTM D 422-Gradation of Soils

Atterberg Limits of Soil Specification for Polyvinyl Chloride (PVC) Plastic Pipe I. ASTM 3034--J. ASTM D 1248-Specification for Corrugated Plastic Pipe

### 1.03 Design Standards

A. The following factors of safety must have been met in this design: 1.5 Sliding, 2.0 Overturning, 2.0 Bearing Capacity, 1.5 Grid Overstress and 1.5 Grid Pullout (from the block and

### PART 2: MATERIALS

2.01 Segmental Retaining Wall Units

A. SRW units shall be machine formed, Portland Cement concrete blocks specifically designed for retaining wall applications. The SRW unit currently approved for this project is:

Cornerstone as manufactured by York Building Products NOTE: Where Cornerstone specifications and reference documents conflict with these specifications,

B. SRW units shall be sound and free of cracks or other defects that would interfere with the proper placing of the unit or significantly impair the strength or permanence of the structure. Cracking or excessive chipping may be grounds for rejection. Units showing cracks longer than 1/2" shall not be used within the wall. Units showing chips visible at a distance of 30 from the

C. Concrete used to manufacture SRW units shall have a minimum 28 days compressive strength of 3,000 PSI and a maximum moisture absorption rate, by weight, of 8% as determined in accordance with ASTM C 140. Compressive strength test specimens shall conform to the saw-cut coupon provisions of Section 5.2.4 of ASTM C 140 with the following exception; Coupon shall be taken from the least dimension of the unit of a size and shape representing the geometry of the

D. SRW units molded dimensions shall not differ more than + 1/8 inch from that specified, except height which shall be + 1/16 inch as measured in accordance with ASTM C

### 2.02 Geosynthetic Reinforcement

A. Geosynthetic reinforcement shall consist of geogrids or geotextiles as indicated on the design No geogrid substitutions shall be permitted without the prior approval of Ryan & Associates.

A. Material for the leveling pad shall consist of compacted gravel. Typical gravels used for this leveling pad are #57, CR6, 21A, 2A modified, 2B, etc. NOTE: Lean un-reinforced concrete with strength of 1,500 PSI and 6" deep may also be used as for the leveling pad.

A. Drainage aggregate shall be angular clean gravel consisting of #57 or equivalent stone (i.e. median stone size ½" to 1 ½"). Rounded "pea gravel" type aggregate is not permissible since it does not have the necessary frictional properties. Recycled stone may be used but must

### 2.05 Drainage Pipe

A. The drainage collection pipe shall be a 4" perforated or slotted PVC or corrugated HDPE pipe.

### 2.06 Reinforced (Infill) Soil; the reinforced geogrid zone

A. The soils used must meet or exceed the design friction angle stated in the project's General Notes and noted on the typical wall section and design cross sections. The reinforced material shall be free of debris and organic material (i.e.— no trash, plants, root matter, top soil, etc.). The reinforced zone material shall not consist of CH (fat clay), MH (elastic silt) or OH/ OL/ PT (organic) soils.

B. Rocks may be used as infill material as long as they have a maximum size of 6" and a mean diameter of 3". Recycled concrete is permissible for infill except with certain polyester grids in water applications. In the case of water applications the geogrid manufacturer shall be consulted to see if the alkali in the recycled material will cause corrosive damage to the

2.07 Retained Soil: the area beyond the infill soil and extending to a distance of twice the

A. This soil must meet or exceed the design friction angle stated in the project's General Notes and noted on the typical wall section and design cross sections. This soil must be virgin (natural undisturbed) or fill compacted to 95% of a standard proctor maximum dry density

2.08 Foundation Soil: the area under the wall's footing and under the reinforced geogrid zone

A. The foundation soil must meet or exceed the bearing capacity stated in the project's General Notes and noted on the typical wall section. The sub-grade must be virgin (natural undisturbed) or fill compacted to 95% of a standard practor maximum dry density.

### PART 3: CONSTRUCTION

A. The owner or owner's representative is responsible for verifying that the wall installer meets all of the requirements of the specifications. This includes all submittals for materials and design, qualifications, and proper installation of wall system. All walls over 6' exposed must have the construction certified by a licensed geotechnical/ structural engineer registered in the

B. The wall installer's field construction supervisor shall have demonstrated experience and be aualified to direct all work at the site.

A. The wall installer shall excavate to the lines and grades shown on the project plans. The wall installer shall take precautions to minimize over—excavation. Over—excavation shall be filled with compacted infill material or as directed by the site geotechnical

B. The wall installer shall verify location of existing structures and utilities prior to excavation. The wall installer shall ensure all surrounding structures are protected from the effects of wall excavation. Excavation support (shoring), if required, is the

### 3.03 Foundation Preparation

A. Following excavation, the foundation soil shall be examined by the owner's geotechnical allowable design bearing capacity (this parameter can be found in the design's General Notes). Soils not meeting the required strength shall be removed and replaced with approved select structural fill or gravel and compacted to 95% of a standard proctor maximum dry density for the full depth.

B. In cases of poor bearing capacity or fill soils, an enlarged geogrid reinforced footing may be required. This typically consists of a 1' deep X 4' wide footing with geogrid under (on the sub-grade) and within the gravel (6" above the sub-grade). The sub-grade must be compacted with a "J-Tamp" or "Jumping Jack" type compactor with a minimum of three passes prior to grid placement. These extra measures will increase the soil's bearing capacity by a minimum of 1,000 PSF.

A. The leveling pad shall be placed as shown on the construction drawings with a and a minimum width of 24\*. The leveling pad should at a minimum extend laterally at

B. The leveling pad material shall be compacted with a vibratory plate compactor to provide a firm level—bearing surface on which to place the first course of units. Compaction will be with mechanical plate compactors to achieve 95% of a standard proctor maximum dry density (ASTM D 698). A thin layer (not to exceed ¼") of well-graded sand or stone dust may be used to smooth the top of the leveling pad.

A. All SRW units shall be installed at the proper elevation and orientation as shown on the wall profiles and details on the construction plans. The SRW units shall be installed in general accordance with the manufacturer's recommendations. The design engineer of record (Ryan & Associates) specifications and drawings shall govern in any conflict

B. The first course of SRW units shall be placed on the leveling pad. The units shall be leveled side—to—side, front—to—rear and with adjacent units, and aligned to ensure intimate contact with the leveling pad. The first course is the most important to ensure accurate and acceptable results. Alignment may be done by means of a string line or offset from the base line to the back of the units. SRW units shall have a minimum 4" overlap of units on each successive course so that the wall is interlocked and continuous. No gaps greater than 1/4" between the faces of adjacent units are permitted.

C. Lay out of curves and corners shall be installed in accordance with the plan details or in general accordance with SRW manufacturer's installation guidelines. In general, all tangent angles shown on the civil drawings should be changed into curves to enhance the wall's strength and appearance. Continuous vertical joints are not permitted unless glued. Inside and outside 90? corners may be constructed without compromising the wall's integrity. Inside corners should be constructed so that the SRW units interlock (according to manufacturer's recommendations) and outside corners should incorporate

D. Clean all excess debris from top of units and install the next course

### E. Repeat procedures to extent of wall height.

F. A +2° tolerance is permitted horizontally for wall batter (block setback). In no case shall a wall go beyond vertical (have a negative batter). A +1.5" tolerance over a 10' distance is permitted vertically (as checked from left to right along the wall).

G. Embedment shall follow the general rule of 1" buried for every 1' of wall exposed with one block minimum when the front slope is 4:1 or greater (more level). Walls constructed on 3:1 front slopes or less (more steep) require additional buried blocks. See the project General Notes and elevation for exact embedment.

### 3.06 Geosynthetic Reinforcement Placement

A. All geogrid reinforcement shall be installed at the proper elevation and orientation as shown on the wall profiles and details on the final construction plans. Partial geogrid coverage is not acceptable— no gaps shall be present between geogrid sections. NOTE: 100% coverage is required, but it is not necessary to overlap the geogrid pieces.

B. At the lengths and elevations shown on the plans, the geogrid shall be laid horizontally on the compacted infill and on top of the concrete SRW units. The geogrid must be embedded into the SRW units to the face. The wall installer shall verify that the orientation of the geogrid is in accordance with the geogrid manufacturer's recommendations. The highest strength direction of the geogrid must be perpendicular to

C. Geogrid reinforcement layers shall be one continuous piece for their entire embedment length. Overlap of the geogrid in the design strength direction (perpendicular to the wall

D. Tracked construction equipment shall not be operated directly on the geogrid. A minimum of 6" of backfill is required prior to operation of tracked vehicles over the geogrid. Turning should be kept to a minimum. Rubber-tired equipment may pass over the geogrid reinforcement at slow speeds (less than 5 MPH).

E. The geogrid shall be in tension and free of wrinkles prior to placement of the infill soil. Nominal tension shall be applied to the geogrid and secured in place with staples, stakes or by hand until it is covered by 6" of infill soil.

F. On inside and outside corners the geogrid shall be placed according to the manufacturer's instructions. There must be total grid coverage. On outside corners the geogrid should be shifted up or down one course and alternated.

A. Drainage aggregate shall be installed to the line, grades and sections shown on the final plans. The drainage gravel shall be placed to a minimum thickness of 12" behind the SRW units. Drainage gravel shall also fill all voids between and within (if hollow) the units. SRW units must be filled with drainage aggregate in one course lifts (units may not be stacked in two or three course lifts then filled).

B. Drain pipes are mandatory and shall be vented to daylight at the end(s) of the wall, at a central low point of the wall, or through the wall face at maximum intervals of 30' O. C. (no more than 6" above finished grade when vented through wall face). The pipe(s) must maintain gravity flow of water outside the reinforced geogrid zone. Water must drain to an outlet and have positive flow. If a continuous pipe is run, it shall daylight into a storm sewer manhole or along a slope at an elevation lower than the lowest point of the pipe within the drainage aggregate. Rear drain pipes are only required when groundwater is encountered or when directed by the site geotechnical engineer or

BY THE ENGINEER:

. C. All drainage zone aggregate, including the gravel placed within the block cells shall be compacted with a vibratory plate compactor with a minimum of two passes.

A. The reinforced backfill shall be placed as shown in the construction plans in the maximum compacted lift thickness of 10" and shall be compacted to a minimum of 95% of a standard proctor maximum dry density (ASTM D 698) at a moisture content within 2% of optimum. The backfill shall be placed and spread in such a manner as to eliminate wrinkles or movement of the geogrid and the SRW units. Compaction testing shall be done at 25%, 50%, 75% and 100% of the wall height or as specified by the site geotechnical engineer.

B. Only a vibratory plate or small-scale vibratory smooth drum compactor equipment shall be allowed within 3' of the front of the wall face. Compaction within the 3' behind the wall face shall be achieved by at least three (3) passes of the lightweight mechanical plate compactor or roller. Heavy equipment (such as track hoes, ride on rollers, pans, etc.) must be kept back a minimum of 3' from the rear of the wall.

C. At the end of each day's operation, the wall installer shall slope the last level o backfill away from the wall facing to direct water runoff away from the wall face.

D. At completion of wall construction if final grading, paving, landscaping and/or storm drainage installation adjacent to the wall is not placed immediately after wall completion temporary grading shall be provided to ensure water runoff is not allowed to collect or

E. Filter fabric is neither required nor recommended between the 12" drainage layer and the compacted infill soil. Installation of filter fabric has proven to result in poor wall construction and its benefit has not been proven when used with clays, silts and mixed soils. The exception is when the infill soil is classified as poorly graded or well graded sand (SP or SW) since these soils are non-cohesive and could potentially stough. clogging the drainage layer.

A. SRW caps shall be properly aligned and glued to the underlying SRW units with a flexible high-strength concrete adhesive designed for "concrete to concrete" applications (not for plastic or wood). Rigid adhesive or mortar is not acceptable.

A. When walls are installed in water applications (such as storm water ponds, streams, bulkheads, areas adjacent to flood plains, etc.) all clean gravel must be used as infill up to 1' above the 100 year flood elevation or the high water level. This gravel must be free draining and have less than 10% fines (#57 stone or equivalent). The leveling pad and the reinforced zone (up to the extent of the gravel infill) must be wrapped in filter fabric (Mirafi 140N or equivalent) to prevent the migration of fines. Rip rap stone is required in front of the bottom three courses on walls installed in tidal waters. Rip rap may also be required to prevent scouring and erosion where pipes that frequently carry

### 3.11 Rails, Fences & Other Structures

A. Open rails and fences not subject to wind loads may be placed directly behind the wall as long as they are not subject to vehicular impact. Solid or semi-solid fences that are subject to wind loads must be kept back a minimum of 3' from the rear of the wall to prevent loading of the wall.

B. Guardralls subject to vehicular impact must be kept back a minimum of 3' to prevent loading of the wall. Guardrails may be placed closer than this 3' minimum only if a barrier (such as wheel stops, curbing, etc.) prevents impact.

C. Concrete storm structures may be located behind a wall and within the reinforced zone as dictated by the project's civil drawings. If the structure(s) cannot be moved out of the reinforced zone and the grid installed to the full design length the following shall apply. On small structures (such as manholes, collection boxes, concrete pipes less than 20" O.D., etc.) it is acceptable to shorten the grid from the design length and meet the structure. The area between the wall and structure must be filled with #57 gravel or equivalent-- not the site soil. On large structures and in cases where pipes parallel the

D. The wall's integrity may be compromised if storm water pipes or structures develop leaks and cause saturation of the reinforced geo—grid zone. Neither Ryan & Associates or the wall installer is responsible for wall failure that results from storm structures or utilities that allow water to leak into the reinforced soil mass causing saturation and disruption of

### 3.12 Construction Adjacent to Completed Wall

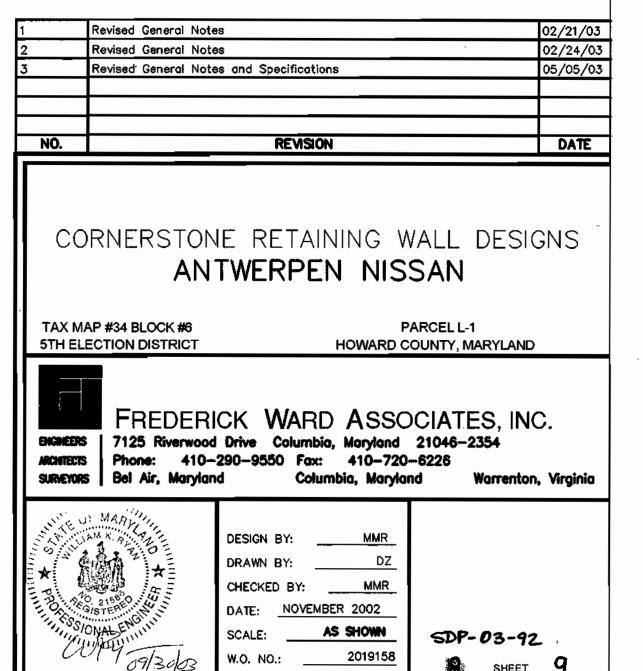
A. The owner or owner's representative is responsible for ensuring that construction adjacent to the wall by others does not disturb the wall or place temporary construction loads on the wall that exceed design loads, including loads such as water pressure, temporary grades, or equipment loading. Heavy paving or grading equipment shall be kept a minimum of three feet behind the back of the wall face. Equipment with wheel loads in excess of 150 PSF live load shall not be operated with 10' of the face of the retaining wall during construction adjacent to the wall. Care should be taken by the general contractor or owner to ensure water runoff is directed away from the wall structure until

B. Care must be taken when installing appurtenances (such as transformers, generators, etc.) within the reinforced zone of the wall. The compaction integrity of the reinforced zone must be maintained, both below and beside (around) the appurtenance. Neglecting to do so may cause hydrostatic pressure and wall failure.

### 3.13 Storm Water Management

A. The segmental retaining wall is not a storm water management structure. Therefore, it is absolutely essential that surface water be prevented from entering the reinforced zone. This is usually accomplished by the site engineer (owner's civil engineer) grading the surface behind the wall to direct surface water to swales that divert the water around the wall ends, to inlets or over the top of the wall through scuppers. If water is directed to the wall, the top six inches of compacted fill over the reinforced zone must have impermeable soil such as CL, GC, SC or an underlying geo-membrane (geo-synthetic

END OF SECTION Revised 07-18-02



SIGNATURE OF DEVELOPER APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING 10/13/02

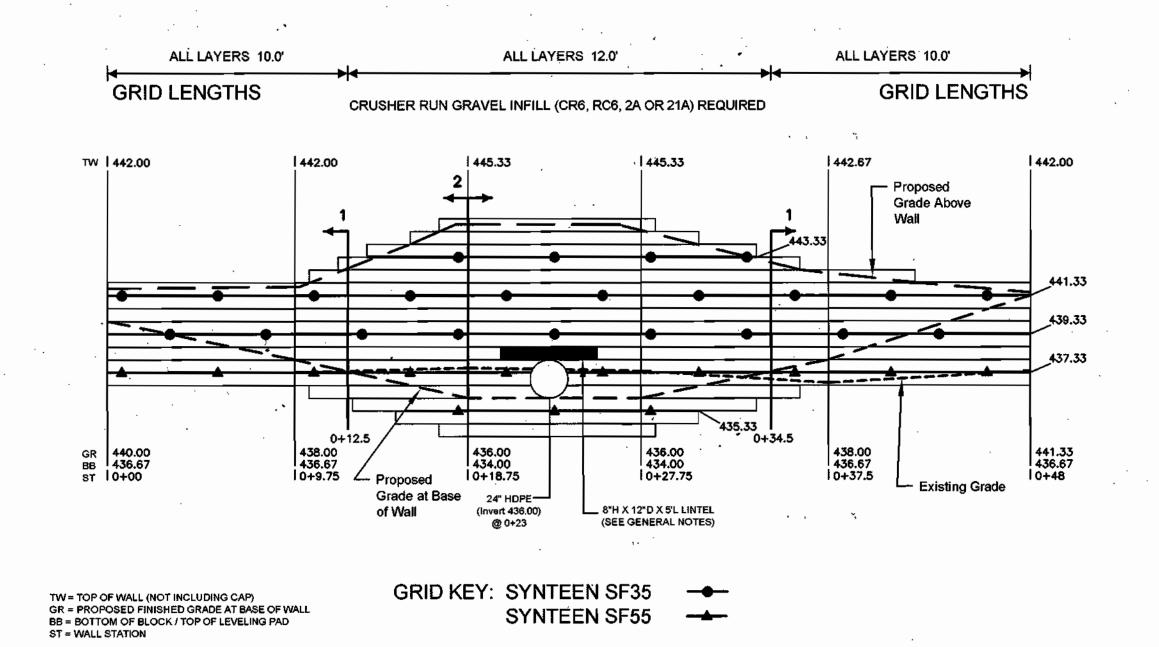
DEVELOPER'S/BUILDER'S CERTIFICATE

I/WE CERTIFY THAT THE LANDSCAPING SHOWN ON THIS PLAN WILL BE DONE ACCORDING TO THE PLAN. SECTION 16.124 OF THE HOWARD COUNTY CODE AND THE HOWARD COUNTY LANDSCAPE MANUAL. I/WE FURTHER CERTIFY THAT UPON COMPLETION, A CERTIFICATION OF LANDSCAPE INSTALLATION, ACCOMPANIED BY AN EXECUTED ONE (1) YEAR GUARANTEE OF PLANT MATERIALS. WILL BE SUBMITTED TO THE DEPARTMENT OF PLANNING AND ZONING.

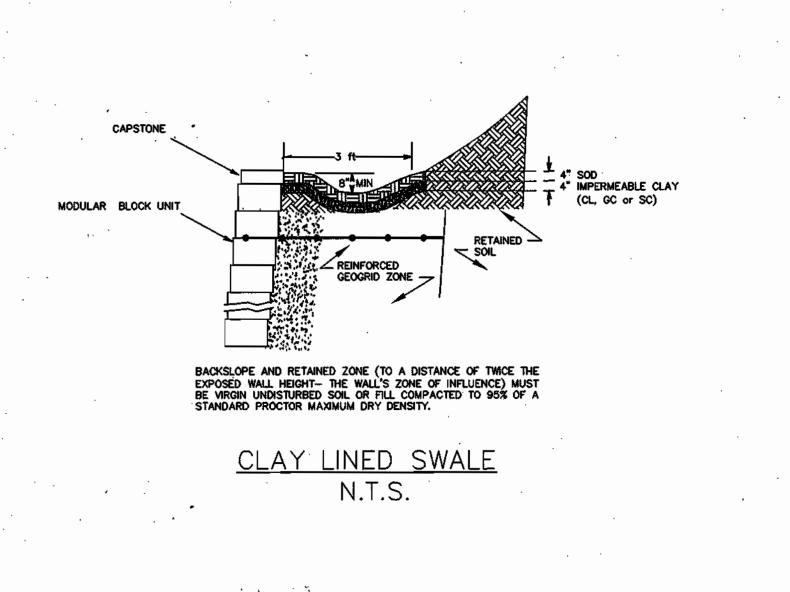
> REVIEWED FOR HOWARD S.C.D. & MEETS TECHNICAL REQUIREMENTS. THIS DEVELOPMENT PLAN SEDIMENT CONTROL BY TH OR SOIL EROSION AND CONSERVATION DISTRICT. HOWARD S.C.D.

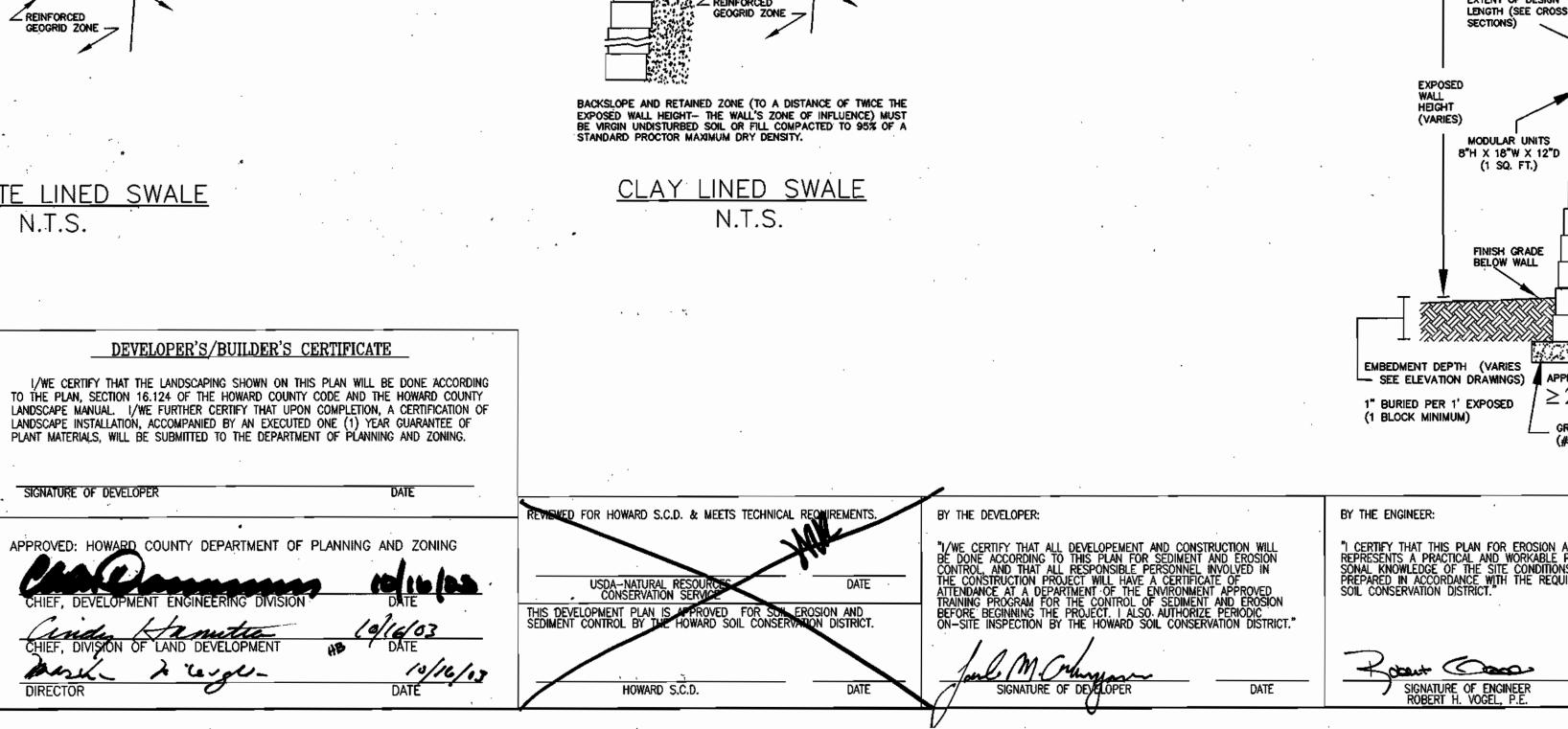
"I CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS, AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT." SIGNATURE OF ENGINEER ROBERT H. VOCEL. P.E.

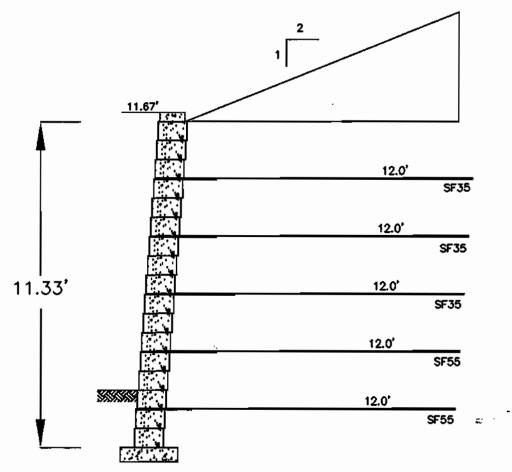
10/17/03



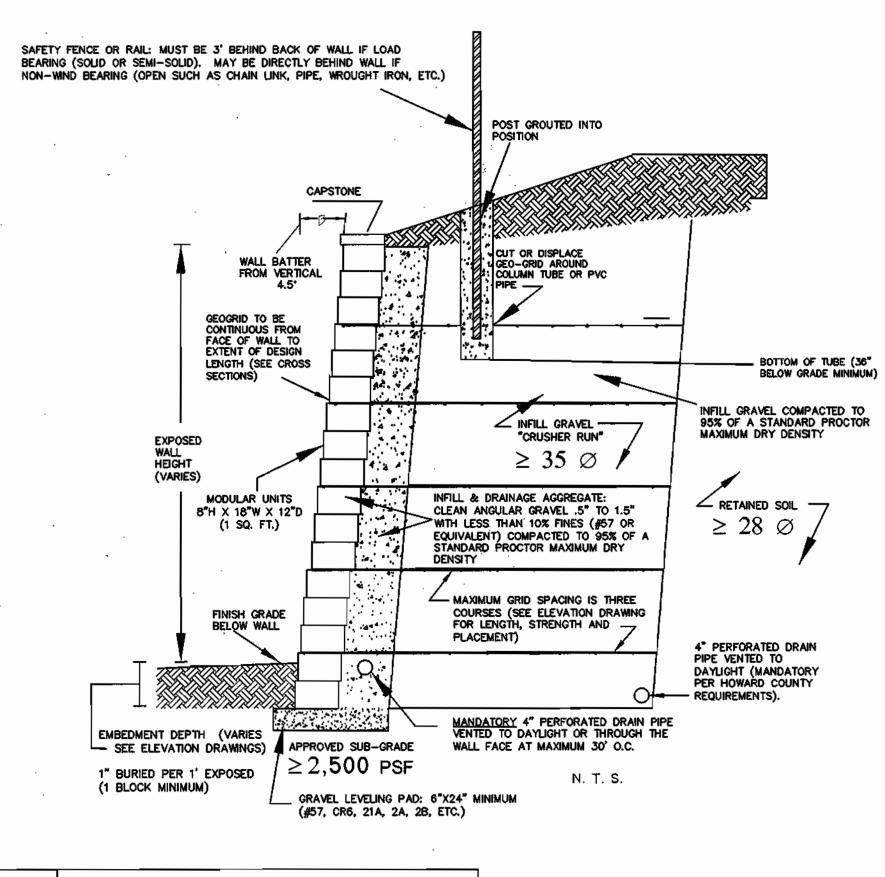
WALL #1 (N of underground SWM facility) Scale 1" = 5'





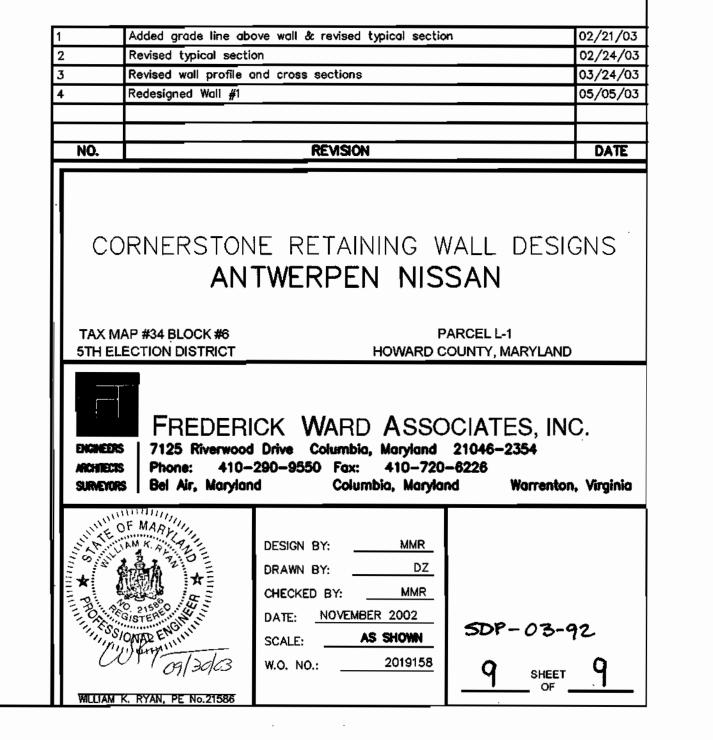


WALL #I, SECTION 2



WALL #I, SECTION I

N.T.S.



- All construction shall be in accordance with the latest standards and specifications of Howard County plus MSHA standards and specifications, if applicable.
- The contractor shall notify "Miss Utility" at 1-800-257-7777 at least 48 hours prior to any excavation work.
- 3. The contractor is to notify the following utilities or agencies at least five days before starting work on these drawings:

Miss Utility 1-800-257-7777 410-754-6281 Verizon 313-2366 Howard County Bureau of Utilities: 393-3553 AT\$T Cable Location Division: 850-4620 B.G.&E. Co. Contractor Services: B.G.&E. Co. Underground Damage Control: 787-4620 State Highway Administration:

4. Site analysis: Area of parcel: 3.4235 Ac. Present zoning: B2

Use of structure: Automobile Sales: 15,747 sf Automobile Service (Including Car Wash): 12,180 sf

Building area: 27,927 sf Disturbed area: 138,050sf

Building coverage on site: 0.641 Ac. or 18.72 % of gross area Paved parking lot/area: 1.73 Ac. or 50.2 % of gross area Area of landscape island: .108 Ac. = 4,701 sf

Cut: 1,350 CY Fill: 12,700 CY

Project background: Location: Columbia, Md.; Tax Map 34, Block 6, Parcel L-1 Zoning : B-2

Subdivision : Holweck Subdision Section/Area : 1/1 Site Area: 3.4235 Acres

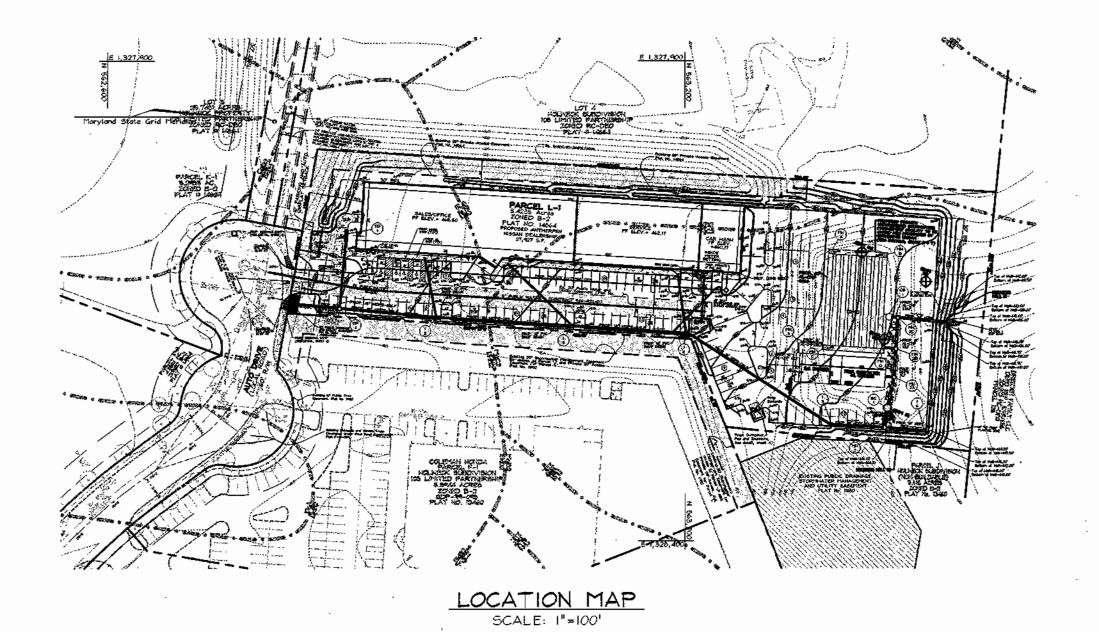
DPZ references : Plat# 11182,; and DPZ file#: F-01-29

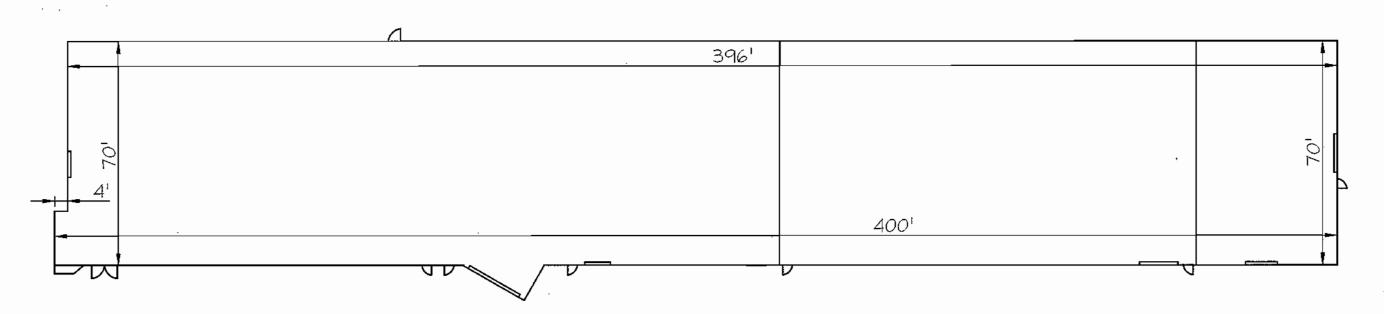
- The contractor shall notify the Department of Public Works/Bureau of Engineering/ Construction Inspection Division at (410) 313-1880 at least five (5) working days prior
- Any damage to public right-of-ways, paving, or existing utilities will be corrected at the contractor's expense.
- Existing utilities located from Road Construction Plans, Field Surveys, Public Water and and Sewer Extension Plans and available record drawings. Approximate location of existing utilities are shown for the contractors information. Contractor shall locate existing utilities well in advance of construction activities and take all necessary precautions to protect the existing utilities and to maintain uninterrupted service. Any damage incurred due to contractor's operation shall be repaired immediately at the contractor's expense. All reinforced concrete for storm drain structures shall have a minimum of 28 days
- strenath of 3,500 p.s.i.
- 10. Traffic control devices, markings and signing shall be in accordance with the latest edition of the Manual on Uniform Traffic Control Devices (MUTCD). All street and regulatory signs shall be in place prior to the placement of any asphalt. Estimates of earthwork quantities are provided soley for the purpose of calculating fees.
- 12. Soil compaction specifications, requirements, methods and materials are to be in accordance with the recommendations of the project Geotechnical Engineer. Geotechnical Engineer to confirm acceptability of proposed paving section, based on soil test prior to construction.
- 13. All storm drain pipe bedding shall be Class 'C'
- 14. The existing topography is taken from field run survey with two foot contour intervals prepared by Frederick Ward \$ Associates, Inc. dated 5/22/01. The coordinates shown hereon are based upon the Howard County Geodetic Control which is based upon the Maryland State Plane Coordinate System.
- 15. A noise study is not required for this project
- 16. All paving to be P-2 paving per Howard County standard details. (See note #12)
- 17. All curb and gutter to be Howard County Standard concrete Detail 3.01 unless
- 18. Contractor responsible to construct all handicap parking and handicap access in accordance with current ADA requirements.
- Where drainage flows away from curb, contractor to reverse the autter pan.
- 20. All elevations are to flowline/bottom of curb unless otherwise noted. 21 All dimensions are to face of curb unless otherwise noted.
- 22. Public Water available along Auto Drive (8" Water) Contract #44-3323-D.
- Public Sewer available along Auto Drive (1 1/2" Sewer (FM)) Contract #30-3687-D. 23. Stormwater Management shall be provided by an underground detention system for quantity attenuation. Water quality shall be provided by a sand filter structure, and existing Stormceptor will be utilized to provide pre-treatment. The proposed stormwater management system will be privatly owned and maintained by Antwerpen Nissan.
- 24. All exterior lighting shall conform to Zoning Regulations Section 134.
- Building to have Inside Water Meter setting.
- 26. Traffic Impact Study prepared by The Traffic Group, dated November 14, 2002.
- See sheet 4 for lighting detail. Lighting details provided for informational purposes only. See electrical and architectural plans for additional lighting information.
- 28. There are no wetlands on-site.
- 29. Department of Planning and Zoning related DPZ file: SP 93-14, WP 93-90, WP 01-20, F 92-161, F 94-38, ZB 947M, ZBIQ08M; Plat #11584, #11181, #11183, #11182, #14864
- 30. There will be no mezzanine levels permitted unless adequate parking has been provided and approved by the Deptartment of Planning and Zonina.
- 31. Geotech report prepared by Herbst/Benson \$ Associates on July 11, 2000.
- 32. Financial Surety for the required landscaping must be posted with the developer agreement in the amount of \$18,900 for 35 shade trees, 53 evergreen trees, and 15 shrubs.
- The landscape surety will be posted with the developers agreement. 33. Referance ZB 947M and ZB 1008M for zoning cases for this site:
  - A) ZB 947M: 1. Date of approval for Decision and Order: March 11, 1994 Rezoned 2.05 acres from B-2 to RC, and 1.99 acres from RC to B-2. 2. Conditions of approval: A 300' private easement buffer area between the zoned B-2 use on lot 4 and the existing residential house on lot 3.
  - B) ZB 1008M
    - 1. Date of approval for Decision and Order: December 4, 2000 Rezoned 0.48 acres of Parcel "L" from B-2 to RC, which became part of Lot 4 and was included in the Agricultural Preservation Easement for
- 2. Conditions of approval: No conditions of approval were specified.
- 34. Debris is to be kept out of all stormwater management facilities during and
- after construction. 35. Forest Conservation requirements for parcel L-I are provided in conjuction with F-01-29.
- 36. Existing Stormwater Management Facility was bonded under F-01-029 by 108 Limited Partnership, C/O Win Kelly Chevrolet.

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

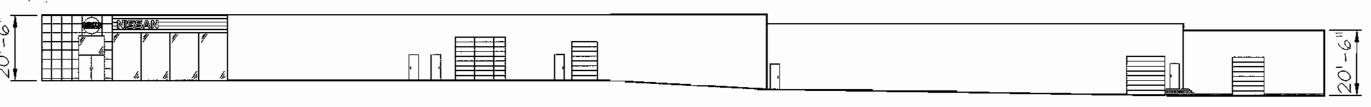
# ANTWERPEN NISSAN

# SITE DEVELOPMENT PLAN PARCEL L-1





PROPOSED BUILDING PLAN VIEW



PROPOSED BUILDING ELEVATION NOT TO SCALE

114 SPACES INCLUDING

5 HANDICAP SPACES

### PARKING TABULATION REQUIRED 28 SPACES AUTOMOBILE DISPLAY: 27,943 SF @ 1 SPACE/1000 SF SALES/OFFICE: 15,747 SF 32 SPACES @ 2 SPACES/1000 SF SERVICE BAYS: 18 BAY AUTOMOBILE SERVICE AREA @ 3 SPACES/SERVICE BAY 54 SPACES (SEE SCHEMATIC FOR DOOR LOCATIONS) TOTAL PARKING SPACES REQUIRED: 114 SPACES

TOTAL PARKING SPACES PROVIDED:

DESCRIPTION	SHEET NO.
	1 of 9
Cover Sheet	
Existing Conditions and Demolition Plan	2 of 9
Site Development, Grading, and Sediment and Erosion Control Plan	3 of 9
Sediment Control Details And Miscellaneous Details	4 of 9
Storm Drain Plan, Drainage Area Map, And SWM Details	5 of 9
Site Landscape Plan	6 of 9
Water \$ Sewer Profiles and Details, SWM Notes and Details	7 of 9
Cornerstone Retaining Wall Designs	8 of 9
Cornerstone Retaining Wall Designs	9 of 9

SHEET INDEX

### LEGEND Existing Contour Proposed Contour

Existing Spot Elevation Proposed Spot Elevation Direction of Flow

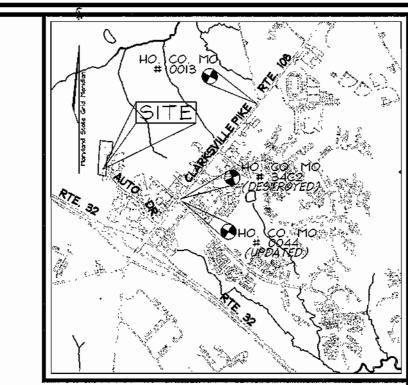
Existing Trees to Remain Light Poles □-♡ Single Overhead ♡-□-♡ Double Overhead

### BENCHMARKS

Concrete

HOWARD COUNTY BENCHMARK 34C2 (DESTROYED) N 562321.798 E 1329750.722 UPDATED: BENCHMARK 0044 N 562176,474 E 1329641,868 ELEV. 485,252

HOWARD COUNTY BENCHMARK 0013 N 564285.946 E 1331309,715 ELEV: 484.671'



VICINITY MAP SCALE: 1"=20001

## OWNER/DEVELOPER

12451 Auto Drive Clarksville, MD 21029-1266

		ADDRE	SS CHART				
LOT/PARCEL	#	STREET ADDRESS					
L-1	12451	12451 AUTO DRIVE					
							-
		PERMIT	INFORMAT	ION CHA	ART		
SUBDIVISION NAME			SECTION	SECTION/AREA PAR		CEL NUMBER	
Høl	lweck Subdiv	rision				L-I	
PLAT REF.	BLOCK NO.	ZONE	TAX/ZONE	ELECT.	DIST.	CENSUS	TR.
14864	6	B2	34	34 5		6051	
WATER CO	DE: .107		SEWER	CODE:	6653	00	

1	REVISED GRADING	12-29.
	<del>-  -</del>	
NO.	REVISION	DAT
NO.	REVISION	

## COVER SHEET

SITE DEVELOPMENT PLAN ANTWERPEN NISSAN PARCEL L-1, HOLWECK SUBDIVISION

TAX MAP #34 BLOCK #6 5TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND



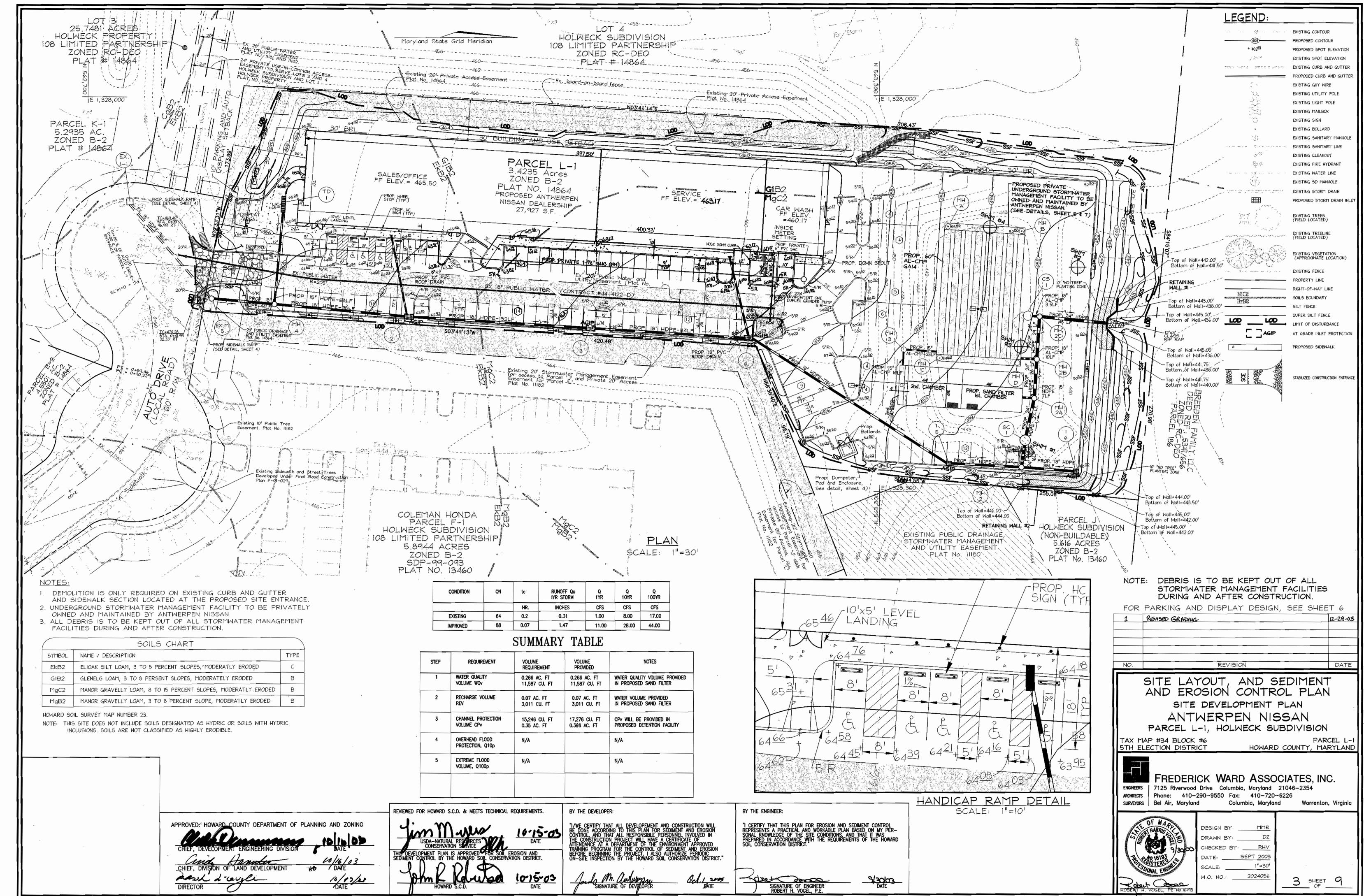
FREDERICK WARD ASSOCIATES, INC. ENGINEERS | 7125 Riverwood Drive Columbia, Maryland 21046-2354

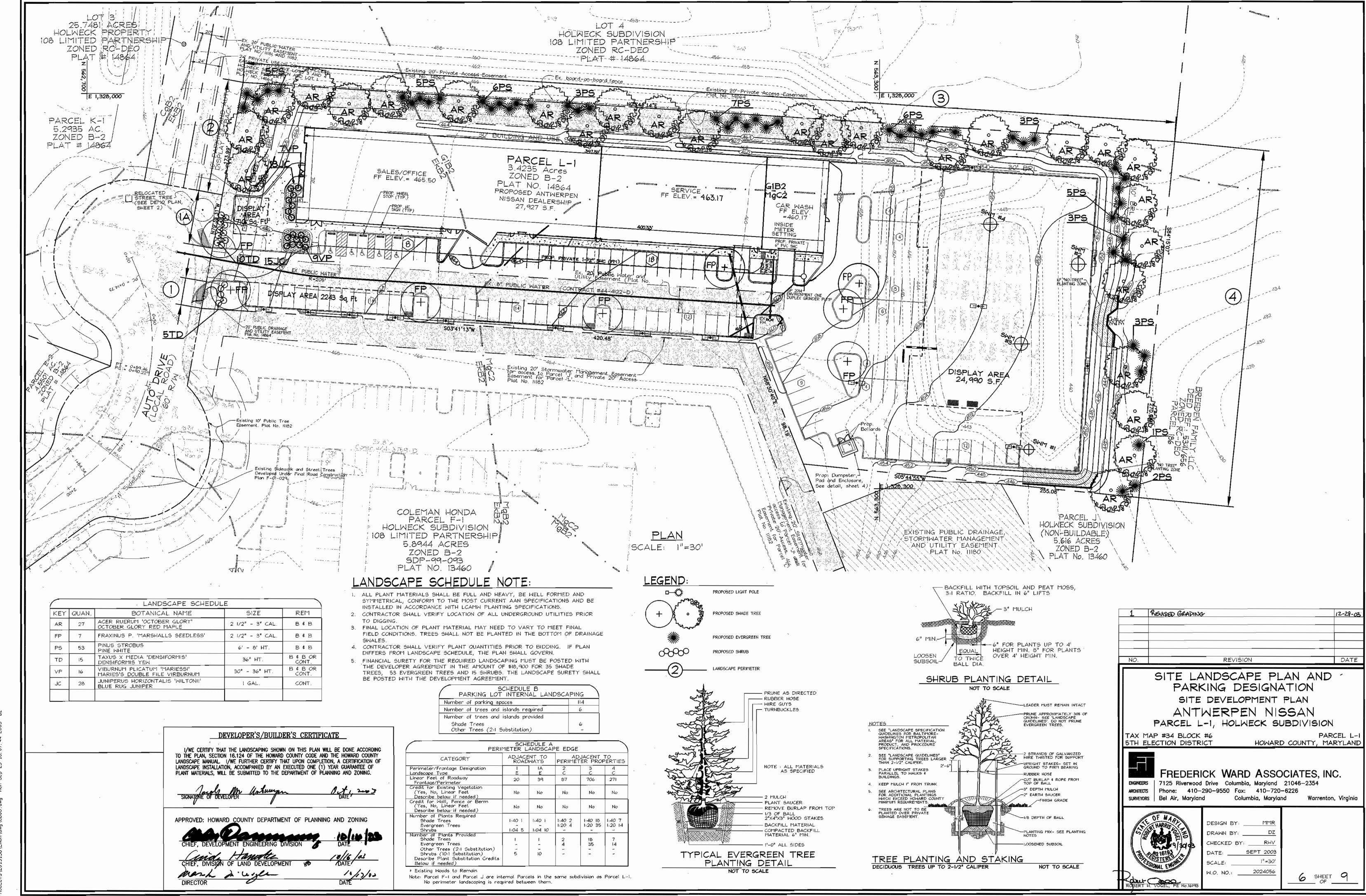
ARCHITECIS | Phone: 410-290-9550 Fax: 410-720-6226 SURVEYORS | Bel Air, Maryland Columbia, Maryland Warrenton, Virginia



DESIGN BY: \_\_\_ AS SHOWN 2024056

SHEET 9





and the many control of the control

### CONTROL STRUCTURE NOTES:

MANHOLES SHALL BE CONSTRUCTED IN ACCORDANCE WITH AASHTO M 199.

- 2. CONCRETE SHALL BE MIX NO. 6 (4500 P.S.I.) 3. WALL REINFORCEMENT FOR BASE UNITS AND RISER UNITS SHALL BE REINFORCEMENT
- BARS OR WELDED WIRE FABERIC WITH A MINIMUM AREA OF 0.22 in./ft FOR THE 84" DIAMETER MANHOLES, WELDED WIRE FABERIC SHALL CONFORM TO ASTM A 185 AND A 82. 4. BASE REINFORCEMENT TO BE REINFORCEMENT BARS OR WELDED WIRE FABERIC WITH A MINIMUM AREA OF 0.27 in./ft. THE BASE SHALL BE CAST MONOLITHIC WITH THE BASE UNIT
- OR JOINTED PER MANUFACTURERS DESIGN. 5. THE MANUFACTURER SHALL FORM MALE AND FEMALE ENDS OF JOINTS USING THEIR OWN DESIGN. THE JOINTS SHALL BE SEALED BY THE CONTRACTOR AND MADE WATERTIGHT USING (WHERE APPLICABLE) MORTAR, RUBBER O-RING GASKETS MEETING ASTM C361 AND C 443 OR FLEXIBLE PLASTIC GASKETS MEETING AASHTO M 198 TYPE B.
- 6. LADDER RUNGS SHALL BE INSTALLED IN VERTICAL ALIGNMENT AT 1'-4" MAXIMUM C/C. RUNG TYPES SHALL BE IN ACCORDANCE WITH STANDARDS MD-383.91 OR MD-383.92, LADDER RUNGS SHALL BE INCIDENTAL TO THE COST OF THE MANHOLE.
- 7. WHEN THE DISTANCE BETWEEN MULTIPLE PIPE OPENINGS IN THE BASE UNIT OR ANY RISER UNIT IS LESS THEN 6" ADDITIONAL NO. 3 BARS ARE REQUIRED AROUND OPENINGS.
- 8. LIFT HOLES OR LIFT EYES SHALL BE PROVIDED IN EACH SECTION FOR HANDLING. 9. MIX NO. 2 CONCRETE OR BRICK CHANNEL SHALL BE PROVIDED IN THE FIELD AND SHALL SLOPE
- 2" PER FOOT TOWARD OUTLET OR AS DIRECTED BY THE ENGINEER.
- 10, THE DRIP STONE LANDING SHALL BE USED ONLY WHEN THERE ARE PIPES CONNECTED TO THE RISER UNITS. SEE STD.MD-384.13 FOR DETAILS. II. MINIMUM DEPTH PAYMENT PER EACH SHALL BE 10'-1" MEASURED FROM THE BOTTOM OF THE
- BASE UNIT TO THE TOP OF THE MANHOLE COVER, VERTICAL DEPTH PAYMENT PER LINEAR FOOT SHALL INCLUDE ALL DEPTHS IN EXCESS OF 10'-1" THE COST OF THE DRIP STONE LANDING, NO. 57 AGGREGATE GROUT, SEALENT, AND ALL NESSASARY APPURTENANCES SHALL BE INCIDENTAL TO

### UNDERGROUND SAND FILTER CONSTRUCTION SPECIFICATIONS

- 1. PROVIDE MANHOLE AND/OR GRATES TO ALL UNDERGROUND AND BELOW GRADE STRUCTURES. MANHOLES SHALL BE IN COMPLIANCE WITH STANDARD SPECIFICATIONS FOR EACH COUNTY, BUT DIAMETERS SHOULD BE 30" MINIMUM (TO COMPLY WITH OSHA CONFINED SPACE REQUIREMENTS). ALUMINUM AND STEEL LOUVERED DOORS ARE ALSO ACCEPTABLE. TEN INCH WIDE (MINIMUM) MANHOLE STEPS (12'O.C.) SHALL BE CAST IN PLACE OR DRILLED AND MORTARED INTO THE WALL BELOW EACH MANHOLE. A 5' MINIMUM HEIGHT CLEARANCE (FROM THE TOP OF THE SAND LAYER TO THE BOTTOM OF THE UPPER/SURFACE SLAB) IS REQUIRED FOR ALL PERMANENT UNDERGROUND STRUCTURES. LIFT RINGS ARE TO BE SUPPLIED TO REMOVE/REPLACE TOP SLABS ON PRE-FABRICATED STRUCTURES. MANHOLE COVERS SHOULD ALLOW FOR PROPER
- . UNDERGROUND SANDFILTERS SHOULD BE CONSTRUCTED WITH A GATE VALVE LOCATED JUST ABOVE THE TOP OF THE FILTER BED FOR DEWATERING IN THE EVENT THAT CLOGGING OCCURS.
- . UNDERGROUND SAND BEDS SHALL BE PROTECTED FROM TRASH ACCUMULATION BY A WIDE MESH GEOTEXTILE SCREEN TO BE PLACED ON THE SURFACE OF THE SAND BED; SCREEN IS TO BE ROLLED UP, REMOVED, CLEANED AND RE-INSTALLED DURING

### PROCEDURE FOR RELOCATING EXISTING STORMCEPTOR

FLATTOP REDUCER

RISER UNIT

BASE & BASE REINFORCING—V SEE NOTE 4

PROVIDE 6" MIN. BEDDING OF NO. 57-7 AGGREGATE ON FIRM SUBGRADE

<del>| ••••••••••</del>••••

SECTION VIEW

- I, HAVE THE UNIT THOROUGHLY CLEANED ACCORDING TO MAINTENANCE PROCEDURES. 2. EXCAVATE AROUND THE UNIT, BEING CAREFUL NOT TO DAMAGE THE COMPONENTS OF THE UNIT OR DISTURB THE PIPE GOING INTO THE STRUCTURE.
- 3. CAREFULLY DISASSEMBLE THE UNIT FROM THE TOP DOWN, WITHOUT DISTURBING THE INTERNAL FIBERGLASS COMPONENTS. 4. INSTALL THE UNIT IN THE NEW LOCATION, FOLLOWING THE PROCEDURES LISTED

### IMPORTANT FACTORS WHEN RELOCATING THE STROMCEPTOR

- 1. THE OLD GASKETS SHOULD BE DISCARDED AND THE JOINTS THOROUGHLY CLEANED BEFORE
- ASSEMBLING THE STRUCTURE WITH NEW GASKETS. 2. THE 2" RISER AT THE TOP OF THE CURRENT UNIT WILL NOT BE NEEDED AT THE NEW LOCATION THIS DOES NOT EFFECT THE UNIT IN ANY WAY.
- 3. THE 6" RISER AT THE TOP OF THE UNIT MAY NEED TO BE FIELD CUT TO MEET THE REQUIRED GRADE THE FLATTOP MAY BE REATTACHED BY BOLTING OR GROUTING IT IN PLACE. AGAIN, THIS WILL NOT EFFECT THE PREFORMANCE OF THE UNIT.

FLATTOP REDUCER

SECTION A-A

MAKE WATERTIGHT WITH RUBBER GASKET SEAL

€ 60" AL-CMP

NOTE: CUT 1.5"Ø HOLE IN 3" PVC CAP Ø INV, 437.00

\_\_CAST\_IN\_PLACE\_ CONTROL\_STRUCTURE

STR MD SHA STD, 384.07

INV, 446.50

INV. 445.00

- 4. EXTREME CARE SHOULD BE TAKEN NOT TO DAMAGE ANY PART OF THE STRUCTURE DURING THE DISASSEMBLY/REASSEMBLY PROCESS.
- 5. THE INLET AND OUTLET ANGLES WILL NOT MATCH THE NEW CONFIGURATION EXACTLY. THE INLET PIPE SHOULD BE PERPENDICULAR TO THE UNIT. ANY VARIATION CAN BE ACHIEVED BY PUTTING THE OUTLET PIPE AT THE ANGLE NEEDED. THE INTERNAL COMPONENTS ALLOW FOR ADJUSTMENT TO THE OUTLET ANGLE.

### CONTRACTOR INSTALLATION INSTRUCTIONS: PRECAST CONCRETE STORMCEPTOR

- 1. STAKE-OUT THE LOCATION OF THE STORMCEPTOR AND EXCAVATE HOLE. EXCAVATE ADEQUATE SPACE TO CONNECT INLET AND OUTLET PIPES TO UNIT. INSTALL A 12" DEEP (OR AS REQUIRED) LAYER OF COMPACTED (95% STANDARD PROCTOR DENSITY OR LOCAL AND STATE REQUIREMENTS, AS DIRECTED BY THE INSPECTOR) AGGREGATE SUBBASE AT BOTTOM OF EXCAVATION. INSTALL MULE OR SHORING, AS NEEDED.
- CHECK ELEVATION OF UNIT BY MEASURING ITS SECTIONS FROM BASE OF THE STORAGE CHAMBER (BOTTOM OF UNIT'S SLAB) TO THE INVERT OF STORMCEPTOR BYPASS CHAMBER INLET ELEVATION (FIBERGLASS INSERT). SUBTRACT THIS DISTANCE FROM DESIGN INVERT ELEVATION TO DETERMINE TOP OF SUBBASE ELEVATION. CHECK ELEVATION OF INSTALLED Subbase and adjust as needed.
- SECURE INSPECTOR APPROVAL OF SUBGRADE AND SUBBASE. INSTALL STORAGE CHAMBER, INSTALL SCREW INSERTS INTO BASE OF STORAGE CHAMBER. ATTACH CABLES OR CHAINS TO ALL 3 LIFTING LUGS ON THE BASE SLAB. USING LARGE EQUIPMENT OR CRANE LIFT AND PLACE THE BASE SECTION OF THE STORAGE CHAMBER IN THE EXCAVATED HOLE ON THE SUBBASE. MAKE SURE THAT THE BASE IS LEVEL SPECIFIC ALIGNMENT OF THIS PART IS NOT REQUIRED. INSTALL RUBBER GASKET ON BASE UNIT AND COAT WITH LUBRICATING GREASE (PROVIDED IN SHIPMENT), IF NOT PRELUBRICATED. INSTALL ADDITIONAL STORAGE CHAMBER SECTIONS, AS REQUIRED (PROCEDURE IS SAME AS STEP 8.).
- INSTALL REDUCING SLAB. (STORMCEPTOR MODELS STC-2400, STC-3600, STC-4800, STC-6000 AND STC-7200) CHECK THAT SECTION IS SET FLUSH, LEVEL AND IS AT THE PROPER ELEVATION. INSTALL RUBBER GASKET ON THE TRANSITION SLAB SPIGOT and coat with lubricating grease (provided in shipment).
- INSTALL BYPASS CHAMBER OF STORMCEPTOR WITH FACTORY INSTALLED STORMCEPTOR insert. Lift bypass section and install, while checking alignment and grade OF INLET AND OUTLET DRAINAGE PIPES. CHECK TO MAKE SURE THE BYPASS CHAMBER IS SET FLUSH, LEVEL AND IS AT THE PROPER ELEVATION. THE BYPASS CHAMBER MUST BE ORIENTED SUCH THAT INLET PIPE DISCHARGES INTO THE V-SHAPED FIBERGLASS WEIRS (INSIDE INSERT). INSTALL RUBBER GASKET ON TOP OF BYPASS SECTION AND COAT WITH LUBRICATING GREASE, IF NOT PRELUBRICATED.
- INSTALL STORMCEPTOR DROP PIPES ACCORDING TO STC PIPE INSTALLATION PROCEDURE. INSTALL RISER SECTION. LIFT RISER SECTION AND INSTALL, WHILE CHECKING THAT SECTION IS SET FLUSH AND IS AT PROPER ELEVATION AND THAT UNIT IS LEVEL. SPECIFIC ALIGNMENT OF THIS PART IS REQUIRED IF STEP(S) ARE INCLUDED. ALIGN STEPS ABOVE INLET inspection port. Note: for shallów installations this section may not be required.
- INSTALL TOP CAP WITH OPENING FOR STORMCEPTOR COVER. IF OPENING IS OFFSET (NOT CENTERED) THE TOP CAP OPENING SHOULD BE ORIENTED ABOVE THE STORMCEPTOR INLET INSPECTION PORT (PLUG).
- BACKFILL STORMCEPTOR WITH APPROVED BACKFILL MATERIAL (NO ORGANIC OR TOPSOIL IS TO BE USED FOR BACKFILL). BACKFILL AND COMPACT IN 8 INCH LIFTS. BACKFILL SHOULD BE COMPACTED TO 95% OF STANDARD PROCTOR DENSITY, OR LOCAL AND STATE REQUIREMENTS, AS DIRECTED BY THE INSPECTOR.
- INSTALL AND SET GRADE ADJUSTING RINGS, AS NEEDED.
- 12. INSTALL AND SET STORMCEPTOR FRAME AND COVER. 13. INSTALL INLET AND OUTLET STORM DRAIN PIPES. CONNECT INLET AND OUTLET STORM Drain Pipes with Flexible Boots (when provided) and with Non-Shrink Grout WHEN NO FLEXIBLE BOOTS ARE PROVIDED. THE INVERT OF THE INLET AND OUTLET PIPE IS TO MATCH WITH THE INVERT OF THE STORMCEPTOR INSERT. FLEXIBLE BOOT INSTALLATION PROCEDURES: CENTER THE PIPE IN THE BOOT OPENING. LUBRICATE THE OUTSIDE OF THE PIPE AND/OR THE INSIDE OF THE BOOT IF THE PIPE OUTSIDE DIAMETER is the same as the inside diameter of the Boot. Position the PIPE clamp in the GROOVE OF THE BOOT WITH THE SCREW AT THE TOP. TIGHTEN THE PIPE CLAMP SCREW TO 60 INCH POUNDS. IF THE PIPE IS MUCH SMALLER THAN THE BOOT, LIFT THE BOOT SUCH THAT IT CONTACTS THE BOTTOM OF THE PIPE WHILE TIGHTENING THE CLAMP TO
- VERTICALLY TO BRING IT TO GRADE. 14. THE STORMCEPTOR SHOULD BE PUMPED OUT WHEN THE SEDIMENT CONTROL MEASURES ARE REMOVED (SITE PERMANENTLY STABILIZED).

ENSURE EVEN CONTRACTION OF THE RUBBER. MOVE THE PIPE HORIZONTALLY AND/OR

### 15. FINAL INSPECTION.

FLAT SLAB TOP

SECTION B-B

# SEWER HOUSE CONNECTION PROFILE SCALE : HORIZONTAL - 1"=50" VERTICAL - I"=5" TOP EL. 442.50 WEIR CREST AT EL. 441.50 MAKE WATERTIGHT AT — JOINTS WITH GREEN STREAK WATERSTOP 43" PVC CAP W/ 1.75" DIA. ORIFICE

R/h

0+00 [ny.:463.67

## CONTROL STRUCTURE CONCRETE WALL NOTES

PRESSURE SHC

I. CONCRETE WALL SHALL BE MIX NO. 6 (4500 p.s.i.) 2. WALL REINFORCEMENT SHALL BR REINFORCEMENT BARS OR WELDED WIRE FABERIC WITH A MIN. AREA OF 0.21 in./fi

CAST-IN-PLACE

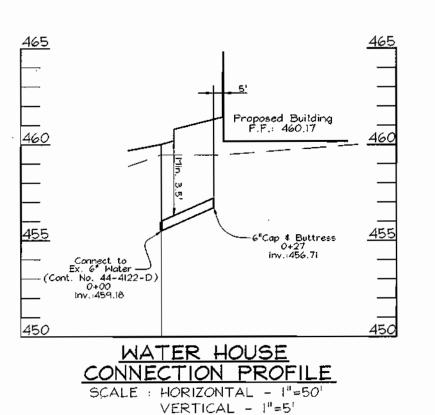
(NOT TO SCALE)

CONTROL STRUCTURE

WELDED WIRE FABERIC. 3. WALL SHALL BE CAST-IN-PLACE, POURED MONOLITHIC USING FORMS. IN LIEU OF MONOLITHIC POUR, WALL SHALL BE CONNECTED BY OVERLAPPING BARS ONE WAY 24", AND SEALED WITH GREEN STREAK WATERSTOP (OR APPROVED

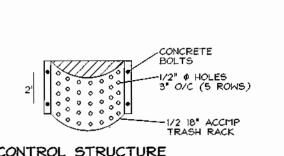
OPERATION AND MAINTENANCE SCHEDULE FOR STORMCEPTOR WATER QUALITY STRUCTURE

- I. The stormceptor water quality structure shall be periodically inspected and cleaned to maintain operation and function. The owner shall inspect the stormceptor unit yearly at a minimum, utilizing the stormceptor inspection/ monitoring form. Inspections shall be done using a clear Plexiglass tube ("sludge/judge") to extract a water column sample. When the sediment depths exceed the level specified in Table 6 of the Stormceptor Technical Manual, the unit must be cleaned.
- 2. The stormceptor water quality structure shall be checked and cleaned immediately after petroleum spills. The owner shall contact the appropriate
- regulatory agencies. 3. The maintenance of the stormceptor unit shall be done using a vacuum truck which will remove the water, sediment debris, floating hydrocarbons, and other materials in the unit. Proper cleaning and disposal of the removed
- materials and liquid must be followed by the owner. 4. The inlet and outlet pipes shall be checked for any obstructions at least once every six months. If obstructions are found, the owner shall have them
- removed. Structural parts of the stormceptor unit shall be repaired as needed 5. The owner shall retain and make the stormceptor inspections/monitoring forms available to the Howard County officials upon their request.



FOR E-CON PUMP

Duplex Grinder Pump 2014 Top Elev.:460.33 N 563, 243 E 1,326,139



CONTROL STRUCTURE TRASH RACK DETAIL - 36" ACCESS MANHOLE TO BE: 14 GA. 2 2/3"x 1/2" CORRUGATION ALT2 CSP WITH STEPS WELDED TO CSP 12" C.C. -H-12 HUGGER BAND

-- 60" STORAGE PIPE

ACCESS MANHOLE DETAIL FOR

UNDERGROUND SWM FACILITY

(NOT TO SCALE)

DEBRIS IS TO BE KEPT OUT OF ALL STORMWATER MANAGEMENT FACILITIES REUISED GRADING

WASHINGTON

SUBURBAN

SANITARY

COMMISSION

///董州/超州/至州

BACKFILL IN ACCORDANCE WITH WSSC STANDARD

INITIAL BACKFILL SEI

PRESSURE PIPE

O.D. + 12" MIN.

1. MINIMUM TRENCH WIDTH AT PIPE SPRINGLINE = 0.D. + 12".
2. MAXIMUM TRENCH WIDTH = 30" FOR ALL MAINLINE PRESSURE SEWER.
3. MAINLINE PRESSURE SEWER TO BE INSTALLED MINIMUM 48" BELOW FINISHED GRADE AND WITH MINIMUM 1'-0" COVER VERTICAL BELOW AND 10'-0" HORIZONTAL CLEARANCE FROM ANY WATER MAIN OR AS INDICATED IN THE DRAWINGS, SEE PS/1.61 FOR PRESSURE SEWER HOUSE CONNECTION DEPTH AND CLEARANCES.
4. MAINLINE PRESSURE SEWER SHALL HAVE MINIMUM CLEARANCE OF 1'-0" BELOW MAIN AT CROSSINGS.

STANDARD DETAIL

TRENCH & BEDDING DETAIL

PRESSURE SEWER, HOUSE CONNECTION

TEES, INCLUDING PRESSURE SHC'S

90" HORIZONTAL BENDS

11-1/4", 22-1/2", 45" HORIZONTA

4 LOWER VERTICAL BENDS

STANDARD DETAIL

PRESSURE SEWER

BLOCKING DETAILS

4. MAINLINE PRESSURE SEWER SHALL HAVE MINIMUM CLEARANCE OF 1 -0
BELOW MAIN AT CROSSINGS.
5. WHEN CLEARANCES REQUIRED IN NOTES 3 & 4 CANNOT BE MET, ENCASE ALL
PRESSURE JOINTS WITHIN 10' OF WATER MAIN WITH CONCRETE, MIN. 4" THICK
AND AT LEAST 18" EACH SIDE OF JOINT.

SECTION A-A

SECTION A-A

SECTION A-A

NOTES:

1. 10-2000 pol @ 28 DAYS

2. CARRY ALL BEARING SURFACES TO UNDISTURBED GROUND OR FIRM SUBGRADE

3. BUTTRESS SIZED FOR 180 psi

4. DO NOT ENCASE JOINTS

<u>PS</u>

PARCEL L

HOWARD COUNTY, MARYLAND

WSSC STANDARD SPECIFICATIONS.

WASHINGTON

SANITARY

SUBURBAN

COMMISSION

DURING AND AFTER CONSTRUCTION. 12-29-03 NO. REVISION DATE

WATER AND SEWER PROFILES AND DETAILS SITE DEVELOPMENT PLAN ANTWERPEN NISSAN PARCEL L-1, HOLWECK SUBDIVISION

TAX MAP #34 BLOCK #6 5TH ELECTION DISTRICT

ARCHITECTS

FREDERICK WARD ASSOCIATES, INC. ENGINEERS | 7125 Riverwood Drive Columbia, Maryland 21046-2354

Phone: 410-290-9550 Fax: 410-720-6226 SURVEYORS | Bel Air, Maryland Columbia, Maryland Warrenton, Virginia



DESIGN BY: DRAWN BY CHECKED BY SCALE:

SEPT 2003 AS SHOWN 2024056 SHEET 9

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

CONTROL STRUCTURE NOTES:

### OPERATION AND MAINTAINANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED UNDERGROUND FACILITIES

- THE SEDIMENT CHAMBER OUTLET DEVICES SHALL BE CLEANED AND/OR REPAIRED WHEN DRAWDOWN TIMES WITHIN THE CHAMBER EXCEED 36 HOURS. DEBRIS AND LITTER SHALL BE REMOVED AS NECESSARY TO INSURE PROPER OPERATION OF THE SYSTEM.
- SEDIMENT SHALL BE CLEANED OUT OF THE SEDIMENTATION CHAMBER WHEN IT ACCUMULATES TO A DEPTH OF 6 INCHES. VEGETATION WITHIN THE SEDIMENT CHAMBER SHALL BE LIMITED TO THE HEIGHT OF 18 INCHES.
- WHEN WATER PONDS ON THE SURFACE OF THE FILTER BED FOR MORE THAN 72 HOURS, THE TOP FEW INCHES OF DISCOLORED MATERIAL SHALL BE REPLACED WITH FRESH MATERIAL. PROPER CLEANING AND DISPOSAL OF THE REMOVED MATRIALS AND LIQUID MUST BE FOLLOWED BY THE OWNER. 5. A LOGBOOK SHALL BE MAINTAINED TO DETERMINE THE RATE AT WHICH THE FACILITY DRAINS.
- THE MAINTENANCE LOGBOOK SHAL BE AVAILABLE TO HOWARD COUNTY FOR INSPECTION TO INSURE COMPLIANCE WITH OPERATION AND MAINTENANCE CRITERIA.
- ONCE THE PERFORMANCE CHARACTERISTICS OF THE INFILTRATION SYSTEM HAVE BEEN VERIFIED, THE MONITORING SCHECUDULE CAN BE REDUCED TO AN ANNUAL BASIS UNLESS THE PROFORMANCE DATA INDICATES THAT A MORE FREQUENT SCHEDULE IS REQUIRED.

### OPERATION AND MAINTAINANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED UNDERGROUND FACILITIES

- A. THE UNDERGROUND STORMWATER MANAGEMENT FACILITY IS PRIVATELY OWNED AND IT SHALL BE THE RESPONSIBILITY OF THE OWNER TO PERIODICALLY INSPECT AND CLEAN THE FACILITY TO MAINTAIN IT'S OPERATION AND FUNCTION.
- B. THE UNDERGROUND STORMWATER MANAGEMENT FACILITY SHALL BE INSPECTED YEARLY AT A MINIMUM AND AFTER ESPECIALLY SEVERE STORM EVENTS. C. WHEN SEDIMENT ACCUMULATION OF MORE THAN 2" IS OBSERVED OR ANY DEBRIS THAT MIGHT OBSTRUCT THE OUTFALL IS OBSERVED, THE FACILITY SHALL BE CLEANED.
- CONTROL STRUCTURE NOTES:

  1. MANHOLES SHALL BE CONSTRUCTED IN ACCORDANCE WITH AASHTO M 199.

  2. CONCRETE SHALL BE MIX NO. 6 (4500 P.S.I.)

  3. WALL REINFORCEMENT FOR BASE UNITS AND RISER UNITS SHALL BE REINFORCEMENT BARS OR WELDED WIRE FABERIC WITH A MINIMUM AREA OF 0.2 in./f. FOR "THE 84" DIAMETER MANHOLES, WELDED WIRE FABERIC SHALL CONFORM TO ASTM A 185 AND A 82.

  4. BASE REINFORCEMENT TO BE REINFORCEMENT BARS OR WELDED WIRE FABERIC WITH A MINIMUM AREA OF 0.27 in./ft. THE BASE SHALL BE CAST MONOLITHIC WITH THE BASE UNIT OR JOINTED PER MANUFACTURERS DESIGN.

  5. THE MANUFACTURER SHALL FORM MALE AND FEMALE ENDS OF JOINTS USING THEIR OWN DESIGN. THE JOINTS SHALL BE SEALED BY THE CONTRACTOR AND MADE WATERTIGHT USING (WHERE APPLICABLE) MORTIAR RUBBER O-RING GASKETS MEETING ASTM C.361 AND C. 445 OR FLEUBLE PLASTIC GASKETS MEETING ALSHITO M. 198 TYPE B.

  6. LADDER RUNGS SHALL BE INSTALLED IN VERTICAL AUGINMENT AT 1"—4" MAXIMUM C/C. RUNG TYPES SHALL BE IN ACCORDANCE WITH STANDARDS MO-383.91 OR MD-383.92. LADDER RUNGS SHALL BE INCIDENTAL TO THE COST OF THE MANHOLE.

  7. WHEN THE DISTANCE BETWEEN MULTIPLE PIPE OPENINGS IN THE BASE UNIT OR ANY RISER UNIT IS LESS THEM 6" ADDITIONAL NO. 3 BARS ARE REQUIRED AROUND OPENINGS.

  8. LIFT HOLES OR LIFT EYES SHALL BE PROVIDED IN THE BASE UNIT OR ANY RISER UNIT IS LESS THEM 6" ADDITIONAL NO. 3 BARS ARE REQUIRED AROUND OPENINGS.

  9. MIX NO. 2 CONCRETE OR BRICK CHANNEL SHALL BE PROVIDED IN THE FIELD AND SHALL SLOPE 2" PER FOOT TOWARD OUTLET OR AS DIRECTED BY THE ENGINEER.

  10. THE DRIP STONE LANDING SHALL SE USED ONLY WHEN THERE ARE PIPES CONNECTED TO THE RISER UNITS. SEE STD.MD-384.13 FOR OCTALS.

  11. MINIMUM DEPTH PAYMENT PER EACH SHALL BE 10"—1" MEASURED FROM THE BOTTOM OF THE BASE UNIT TO THE TOP OF THE MANHOLE COVER. VERTICAL DEPTH PAYMENT PER LINEAR FOOT SHALL INCLUDE ALL DEPTHS IN EXCESS OF 10"—1" THE COST OF THE DRIP STONE LANDING, NO. 57 AGGREGATE GROUT, SEALENT, AND ALL NESSASARY APPURTENANCES SHALL BE INCIDENTAL TO THE PRICE BIO. E. THE SEDIMENT AND DEBRIS SHALL BE REMOVED FROM THE UNDERGROUND STORMMATER MANAGEMENT FACILITY BY VACUUM TRUCK OR OTHER MANUAL MEANS. THE OWNER SHALL FOLLOW PROPER CLEANING AND DISPOSAL OF THE REMOVED MATERIAL AND LIQUID.
  - F. THE INLET AND OUTLET PIPES SHALL BE CHECKED FOR ANY OBSTRUCTIONS AT LEAST ONCE EVERY SIX (6) MONTHS. IF OBSTRUCTIONS ARE FOUND, THE OWNER SHALL HAVE THEM REMOVED AND PROPERLY DISPOSED OF.

— 60" AL-CMP (TYP.) Cpv WATER SURFACE GAUGE 14 ELEVATION = 444.90-— 140 N MARAFI GEOTEXTILE FABRIC (TYP.) STONE (typ.) UNDERGROUND DETENTION SYSTEM CROSS SECTION