

- REVISIONS**
- | NO. | DATE | DESCRIPTION |
|-----|----------|-------------------|
| 1 | 10/25/02 | ADD UNITS 13 & 14 |
- GENERAL:
 - HEIGHT OF BARRIER SHALL BE BASED ON ACOUSTIC REQUIREMENTS.
 - BARRIER WALLS HAVING A HEIGHT OR NOT INDICATED IN THE TABLES SHALL BE CONSTRUCTED AS SHOWN IN THE HIGHER HEIGHT CATEGORY.
 - SEWING:
 - 2 INCH WOOD DECKING MATERIAL SHALL BE UTILIZED TO SPAN HORIZONTALLY BETWEEN POSTS. DESIGN CRITERIA IS BASED ON AN ALLOWABLE BENDING STRESS OF 1000 LBS. PER SQ. IN. AND A 30 USE INCREASE FOR INCREASE IN STRESS FOR WIND LOADS AS CONSIDERED APPROPRIATE.
 - SEWING IN CONTACT WITH THE GROUND AND FOR A DISTANCE OF 6" ABOVE GRADE SHALL BE TREATED WITH WOOD PRESERVATIVE.
 - POST:
 - WOOD POST SHALL BE UTILIZED AT THE SPACING INDICATED ON THE SCHEDULE. DESIGN CRITERIA IS BASED ON AN ALLOWABLE BENDING STRESS OF 1000 LBS. PER SQ. IN. AND A 30 USE INCREASE FOR WIND LOAD.
 - POST EMBEDDED IN CONCRETE SHALL BE TREATED WITH A WOOD PRESERVATIVE IN THE AREA OF EMBEDMENT AND 12" ABOVE GRADE.
 - CONCRETE:
 - CONCRETE IN THE PILES HAVE A 28 DAY COMPRESSIVE STRENGTH OF 2500 LBS. PER SQ. FT.
 - CONCRETE SHALL BE PLACED IN DRILLED PILES UTILIZING THE EARTH AS THE FORM.
 - FOUNDATIONS:
 - THE DRILLED PILES HAVE BEEN DESIGNED UTILIZING AN ALLOWABLE PASSIVE PRESSURE OF 300 LBS. PER SQ. FT. AND THE FOLLOWING FORMULAS:
 - $D = (0.0297/P)^{1/3}$
 - $H = \text{HOLEY AT TOP OF DRILLED PILE STAGES}$
 - $P = \text{ALLOWABLE PASSIVE PRESSURE, 300 LBS. PER SQ. FT.}$
 - $D = \text{DIAMETER OF PILE (FT.)}$
 - $H = \text{DEPTH OF PILE (FT.)}$
 - ALTERNATIVE #1 PRESERVATIVE TREATMENT ALTERNATIVE #1 REPRESENTS THE ADDITIONAL COST FACTOR FOR TREATING THE BASIC WOOD STRUCTURE INDICATED ON THIS REFERENCE PLAN. THE NECESSITY FOR TREATMENT AND THE TYPE OF PRESERVATIVE WILL BE SUBJECT TO LOCAL CONDITIONS. ALL TREATMENTS SHALL CONFORM TO THE AFWA STD C-4.
 - ALTERNATIVE #2 PAINTING ALTERNATIVE #2 REPRESENTS THE ADDITIONAL COST FACTOR REQUIRED TO PAINT ONE SIDE OF THE BASIC WOOD STRUCTURE. SHOW ON THIS REFERENCE PLAN. PAINTING SHALL CONSIST OF 3 APPLICATIONS OF PAINT, 2 COATS OF LATEX BASE PAINT CONFORMING TO FEDERAL SPECIFICATION TT-P-409966 SHALL BE APPLIED OVER FINISH COAT CONFORMING TO FEDERAL SPECIFICATION TT-P-402058.
 - ALTERNATIVE #3 STAINING ALTERNATIVE #3 REPRESENTS THE ADDITIONAL COST FACTOR REQUIRED TO STAIN ONE SIDE OF BASIC WOOD STRUCTURE. STAIN SHALL CONSIST OF 2 COATS OF 50% TRANSPARENT SEALER STAIN APPLIED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS.

FISHER, COLLINS & CARTER, INC.
 CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
 1100 WEST 10TH STREET, SUITE 1000
 ELICOTT CITY, MARYLAND 21043
 (410) 461-2855

DATE	DESCRIPTION	REVISION BLOCK
7/31/03	Revised Parking at Condo Building 'A' (Plan View), Added Sidewalk and Area, Grading Along Meadowridge Road, Added Storm Drain Extension at I-13, Revised Grading at Site Entrance, Added I-15 and Pipe to I-13, Revised 8" Sewer Main MH 110 and MH 125, Revised WAC's to 2" and 4", Revised Elevation Elevation From Condo Buildings 15, F-19.	
3	Rev. grad. lots 1-5 to show As-Built Conditions	
4	Rev FF elev. Bldg A/B to show As Built Cond. 10/22/04	



ENGINEER'S CERTIFICATE
 I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.

CHARLES J. CROVO, SE, P.E., L.S.
 Signature of Engineer (print name below signature) Date: 10/22/02

DEVELOPER'S CERTIFICATE
 I/We certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project will have a certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize persons on the inspection by the Howard Soil Conservation District.

DONALD R. REIJER, JR.
 Signature of Developer (print name below signature) Date: 8/22/02

DEVELOPER/OWNER
 RESK, L.L.C.
 c/o LAND DESIGN AND DEVELOPMENT, INC.
 8000 MAIN STREET
 ELICOTT CITY, MARYLAND 21043

BUILDER
 RYAN HOMES, INC.
 11460 CROWBRIDGE DRIVE
 SUITE 120
 OWINGS MILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Director - Department of Planning and Zoning Date: 10/3/02

Chief, Division of Land Development Date: 10/3/02

Chief, Development Engineering Division Date: 9/20/02

DEED REF.	BLOCK NO.	ZONE	TAX/ZONE	ELEC. DIST.	CENSUS TR.
5609 / 611	3	POR	37	1st	6030

WATER CODE: D 04 SEWER CODE: 2610000

SITE DEVELOPMENT PLAN

THE COURTYARDS AT THE TIMBERS
 TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

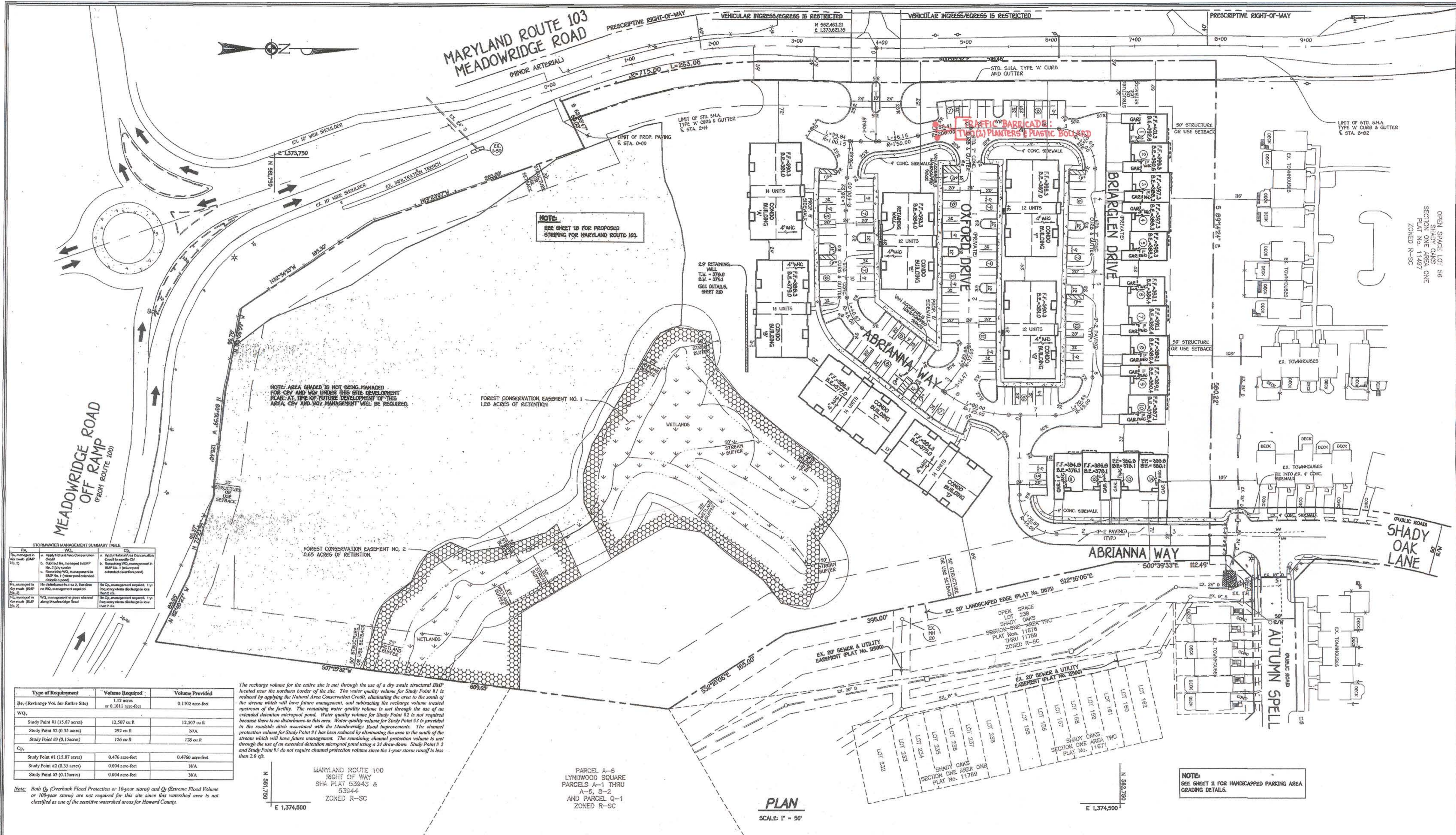
ZONED: POR

TAX MAP No.: 37 PARCEL No.: 617 GRID No.: 3
 FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND

SCALE: AS SHOWN DATE: AUGUST 21, 2002

SHEET 2 OF 24 **SDP 02-55**

03/24/16



STORMWATER MANAGEMENT SUMMARY TABLE

Req. No.	WQ	Cp
1	Recharge Volume (for Entire Site)	
2	WQ	
3	Cp	

Type of Requirement	Volume Required	Volume Provided
Recharge Volume (for Entire Site)	1.12 acres or 0.1011 acre-feet	0.1102 acre-feet
WQ		
Study Point #1 (15.87 acres)	12,507 cu ft	12,507 cu ft
Study Point #2 (0.35 acres)	292 cu ft	N/A
Study Point #3 (0.15 acres)	126 cu ft	126 cu ft
Cp		
Study Point #1 (15.87 acres)	0.476 acre-feet	0.4760 acre-feet
Study Point #2 (0.35 acres)	0.004 acre-feet	N/A
Study Point #3 (0.15 acres)	0.004 acre-feet	N/A

The recharge volume for the entire site is met through the use of a dry swale structural BMP located near the northern border of the site. The water quality volume for Study Point #1 is reduced by applying the Natural Area Conservation Credit, eliminating the area to the south of the stream which will have future management, and substituting the recharge volume treated upstream of the facility. The remaining water quality volume is met through the use of an extended detention micropond. Water quality volume for Study Point #2 is not required because there is no disturbance in this area. Water quality volume for Study Point #3 is provided in the roadside ditch associated with the Meadowridge Road improvements. The channel protection volume for Study Point #1 has been reduced by eliminating the area to the south of the stream which will have future management. The remaining channel protection volume is met through the use of an extended detention micropond using a 24 draw-down. Study Point #2 and Study Point #3 do not require channel protection volume since the 1-year storm runoff is less than 2.0 cfs.

Type of Requirement	Volume Required	Volume Provided
Recharge Volume (for Entire Site)	1.12 acres or 0.1011 acre-feet	0.1102 acre-feet
WQ		
Study Point #1 (15.87 acres)	12,507 cu ft	12,507 cu ft
Study Point #2 (0.35 acres)	292 cu ft	N/A
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Study Point #1 (15.87 acres)	0.476 acre-feet	0.4760 acre-feet
Study Point #2 (0.35 acres)	0.004 acre-feet	N/A
Study Point #3 (0.15 acres)	0.004 acre-feet	N/A

Note: Both Q₁ (Overbank Flood Protection or 10-year storm) and Q₂ (Extreme Flood Volume or 100-year storm) are not required for this site since this watershed area is not classified as one of the sensitive watershed areas for Howard County.

FISHER, COLLINS & CARTER, INC.
 CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
 CENTRAL SQUARE OFFICE PARK - 1822 BALTIMORE NATIONAL PIKE
 ELLICOTT CITY, MARYLAND 21042
 (410) 961-2255

DATE	DESCRIPTION	REVISION BLOCK
10/22/02	ADD UNITS 19 & 14	



ENGINEER'S CERTIFICATE
 I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.

Signature of Engineer (print name below signature) *Charles J. Crowl, Sr.*
 CHARLES J. CROWL, SR., P.E., L.S. 10/10
 DATE

DEVELOPER'S CERTIFICATE
 I/we certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize periodic on-site inspection by the Howard Soil Conservation District.

Signature of Developer (print name below signature) *Donald R. Reuwer, Jr.*
 DONALD R. REUWER, JR. 9/9/02
 DATE

DEVELOPER/OWNER
 RESK, L.L.C.
 c/o LAND DESIGN AND DEVELOPMENT, INC.
 8000 MAIN STREET
 ELLICOTT CITY, MARYLAND 21043

BUILDER
 RYAN HOMES, INC.
 11460 CROWDRIDGE DRIVE
 SUITE 125
 OWINGS HILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Joseph R. Roth 10/13/02
 Director - Department of Planning and Zoning Date

Charles J. Crowl, Sr. 10/15/02
 Chief, Division of Land Development Date

Donald R. Reuwer, Jr. 9/20/02
 Chief, Development Engineering Division Date

SUBDIVISION SECTION/AREA PARCEL NO.
 THE COURTYARDS AT THE TIMBERS 617

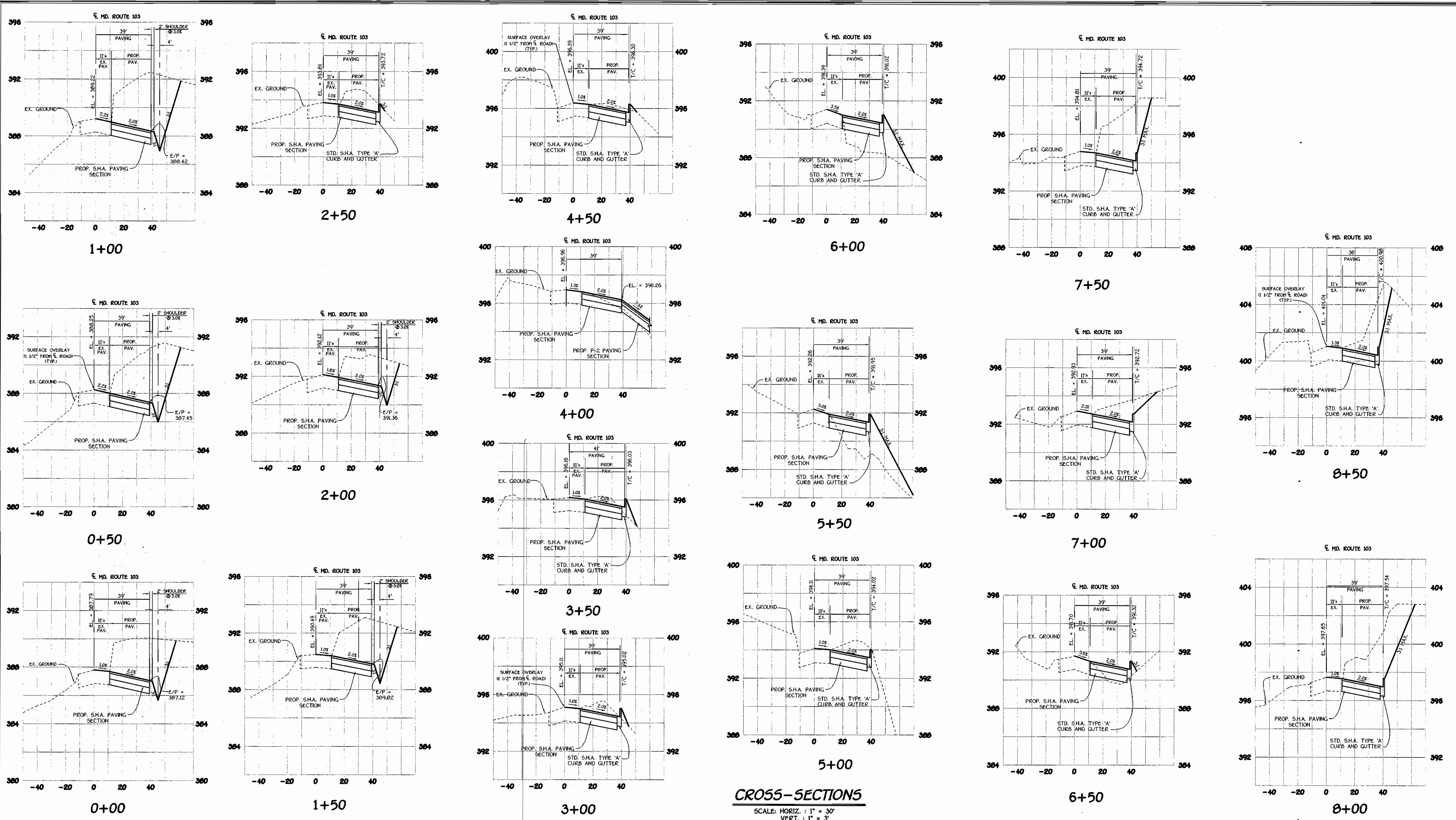
DEED REF. BLOCK NO. ZONE TAX/ZONE ELEC. DIST. CENSUS TR.
 5609 / 611 3 POR 37 1st. 6030

WATER CODE SEWER CODE
 D 04 2610000

SITE DIMENSION PLAN

THE COURTYARDS AT THE TIMBERS
 TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

TAX MAP No.: 37 PARCEL No.: 617 GRID No.: 3
 FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND
 SCALE: AS SHOWN DATE: AUGUST 21, 2002
 SHEET 3 OF 24 **SOP 02-55**



CROSS-SECTIONS
 SCALE: HORIZ. : 1" = 30'
 VERT. : 1" = 3'

FISHER, COLLINS & CARTER, INC.
 CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
 CENTENNIAL SQUARE OFFICE PARK - 1072 BALTIMORE NATIONAL PIKE
 ELLICOTT CITY, MARYLAND 21117
 (410) 461-2252

DATE	DESCRIPTION	REVISION BLOCK



ENGINEER'S CERTIFICATE
 I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.
 Signature of Engineer (print name below signature) Charles J. Crovo, Sr., P.E., L.S. 8/28/02 Date
DEVELOPER'S CERTIFICATE
 I/we certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize periodic on-site inspection by the Howard Soil Conservation District.
 Signature of Developer (print name below signature) Donald R. Reuver, Jr. 8/22/02 Date

DEVELOPER/OWNER
 RREK, L.L.C.
 c/o LAND DESIGN AND DEVELOPMENT, INC.
 8900 MAIN STREET
 ELLICOTT CITY, MARYLAND 21143

BUILDER
 RYAN HOMES
 11460 CROWNDRIDGE DRIVE
 SUITE 128
 OWINGS MILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Joseph R. Rutter 10/3/02
 Director - Department of Planning and Zoning Date

Charles Hamilton 10/9/02
 Chief, Division of Land Development Date

Chris Paunissen 9/20/02
 Chief, Development Engineering Division Date

SUBDIVISION	SECTION/AREA	PARCEL NO.
THE COURTYARDS AT THE TIMBERS		617
DEED REF. 5609 / 611	BLOCK NO. 3	ZONE POR
WATER CODE D 04	TAX/ZONE 37	ELEC. DIST. 1st
	SEWER CODE	CENSUS TR. 6030
		2610000

CROSS-SECTIONS FOR MEADOWRIDGE ROAD
THE COURTYARDS AT THE TIMBERS
 TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

ZONED: POR

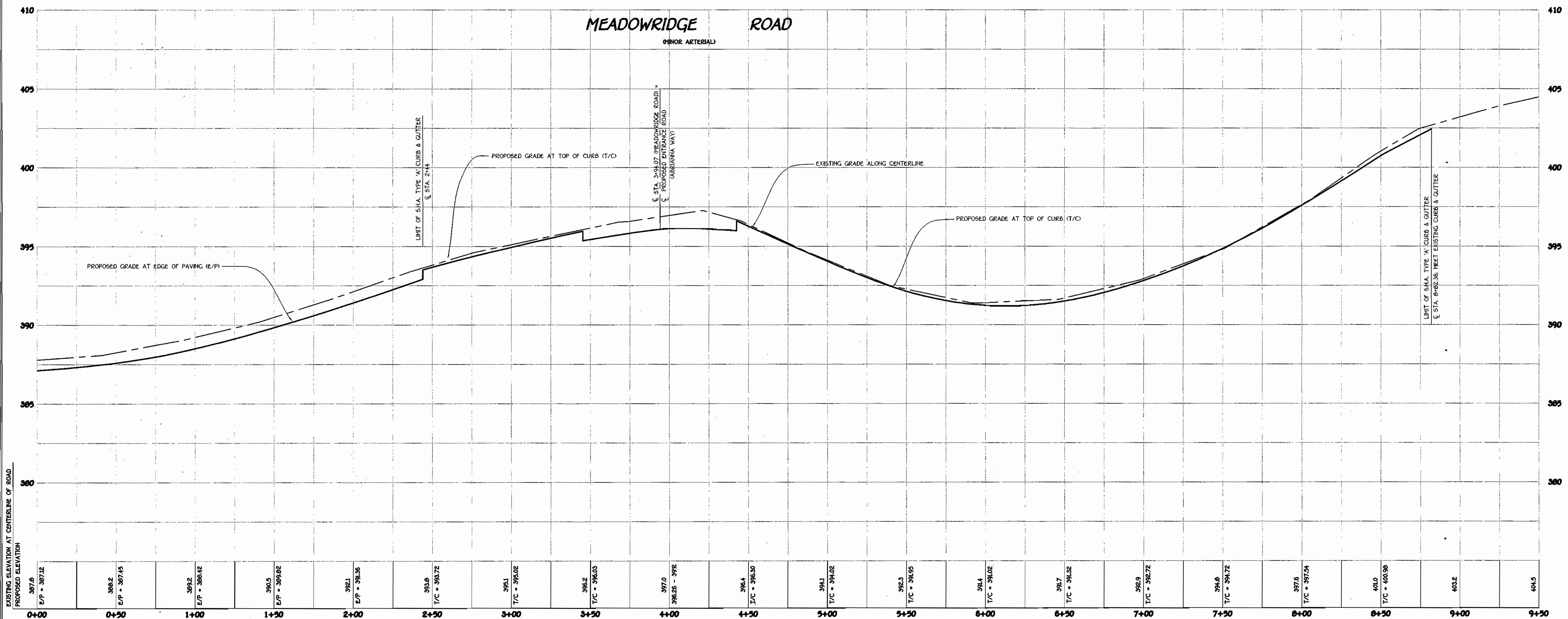
TAX MAP No.: 37 PARCEL No.: 617 GRID No.: 3
 FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND

SCALE: AS SHOWN DATE: AUGUST 21, 2002

SHEET 4 OF 24 **SDP 02-55**

MEADOWRIDGE ROAD

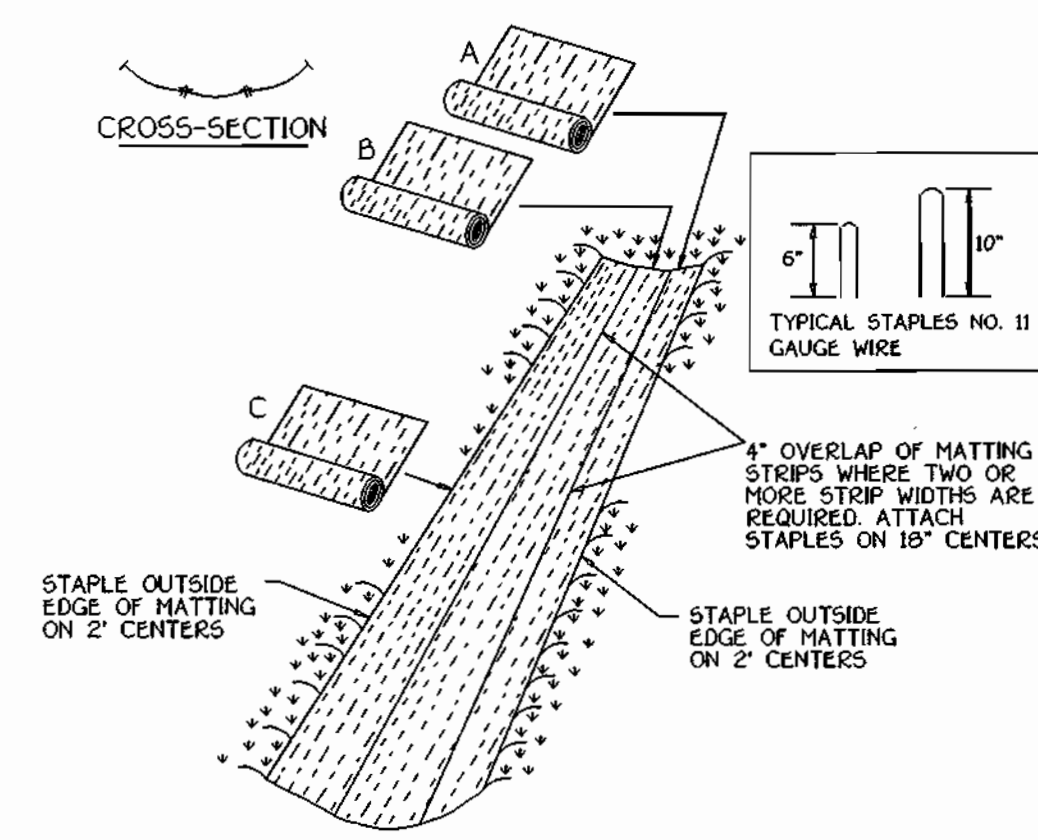
MINOR ARTERIAL



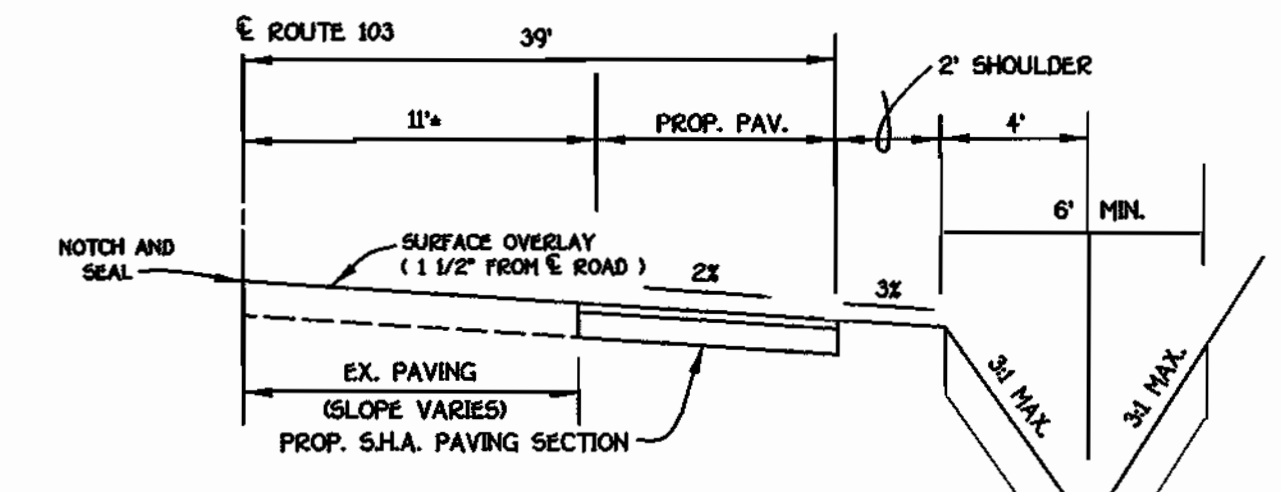
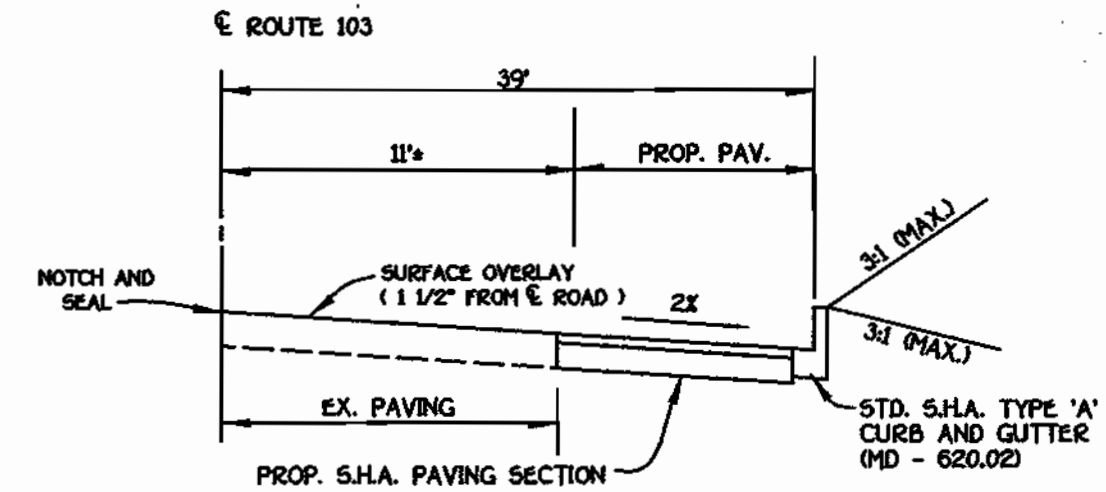
PROFILE

SCALE: HORIZ. : 1" = 30'
VERT. : 1" = 3'

EROSION CONTROL MATTING FOR DITCH LINING



- ### Construction Specifications
- Key-in the matting by placing the top ends of the matting in a narrow trench, 6" in depth. Backfill the trench and tamp firmly to conform to the channel cross-section. Secure with a row of staples about 4" down slope from the trench. Spacing between staples is 6".
 - Staple the 4" overlap in the channel center using an 18" spacing between staples.
 - Before stapling the outer edges of the matting, make sure the matting is smooth and in firm contact with the soil.
 - Staples shall be placed 2' apart with 4 rows for each strip, 2 outer rows, and 2 alternating rows down the center.
 - Where one roll of matting ends and another begins, the end of the top strip shall overlap the upper end of the lower strip by 4", ship-lap fashion. Reinforce the overlap with a double row of staples spaced 6" apart in a staggered pattern on either side.
 - The discharge end of the matting liner should be similarly secured with 2 double rows of staples.
- Note: If flow will enter from the edge of the matting then the area effected by the flow must be keyed-in.



STA. 2+44 TO STA. 8+01
TYPICAL ROADWAY SECTION FOR MD. ROUTE 103 w/ CURB
NO SCALE

S.H.A. SECTION SHALL BE 3" (NOMINAL) BITUMINOUS CONCRETE SURFACE (TWO 1 1/2" COURSES), 4" (NOMINAL) BITUMINOUS CONCRETE BASE AND EITHER ONE OF THE FOLLOWING:
6" BASE COURSE USING GRADED AGGREGATE BASE, GA BASE OR A 7" BASE COURSE USING BANK-RUN GRAVEL BASE, BRG BASE.

STA. 0+00 TO STA. 2+44
TYPICAL ROADWAY SECTION FOR MD. ROUTE 103 w/ DITCH
NO SCALE

S.H.A. SECTION SHALL BE 3" (NOMINAL) BITUMINOUS CONCRETE SURFACE (TWO 1 1/2" COURSES), 4" (NOMINAL) BITUMINOUS CONCRETE BASE AND EITHER ONE OF THE FOLLOWING:
6" BASE COURSE USING GRADED AGGREGATE BASE, GA BASE OR A 7" BASE COURSE USING BANK-RUN GRAVEL BASE, BRG BASE.

FISHER, COLLINS & CARTER, INC.
CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
CENTRAL SQUARE OFFICE PARK - 1872 BALTIMORE NATIONAL PKWY
ELLICOTT CITY, MARYLAND 21114
410-481-2955

DATE	DESCRIPTION



ENGINEER'S CERTIFICATE
"I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District."

Signature of Engineer (print name below signature) Charles J. Crovo, Sr., P.E., L.S. Date 8/22/02

DEVELOPER'S CERTIFICATE
"I/we certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize periodic on-site inspection by the Howard Soil Conservation District."

Signature of Developer (print name below signature) Donald R. Reuwer, Jr. Date 8/22/02

DEVELOPER/OWNER
RRSK, L.L.C.
c/o LAND DESIGN AND DEVELOPMENT, INC.
8000 MAIN STREET
ELLICOTT CITY, MARYLAND 21043

BUILDER
RYAN HOMES
11460 CROWBRIDGE DRIVE
SUITE 128
OWINGS MILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Director - Department of Planning and Zoning [Signature] Date 10/3/02
Chief, Division of Land Development [Signature] Date 10/3/02
Chief, Development Engineering Division [Signature] Date 9/20/02

SUBDIVISION	SECTION/AREA	PARCEL NO.
THE COURTYARDS AT THE TIMBERS		617
DEED REF.	BLOCK NO.	ZONE
5609 / 611	3	FOR
WATER CODE	TAX/ZONE	ELEC. DIST.
D 04	37	1st
	SEWER CODE	CENSUS TR.
		6030

PROFILE AND DETAILS FOR MEADOWRIDGE ROAD WIDENING
THE COURTYARDS AT THE TIMBERS
TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

ZONED: POR
TAX MAP No.: 37 PARCEL No.: 617 GRID No.: 3
FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND
SCALE: AS SHOWN DATE: AUGUST 21, 2002

SHEET 5 OF 24 **SDP 02-55**

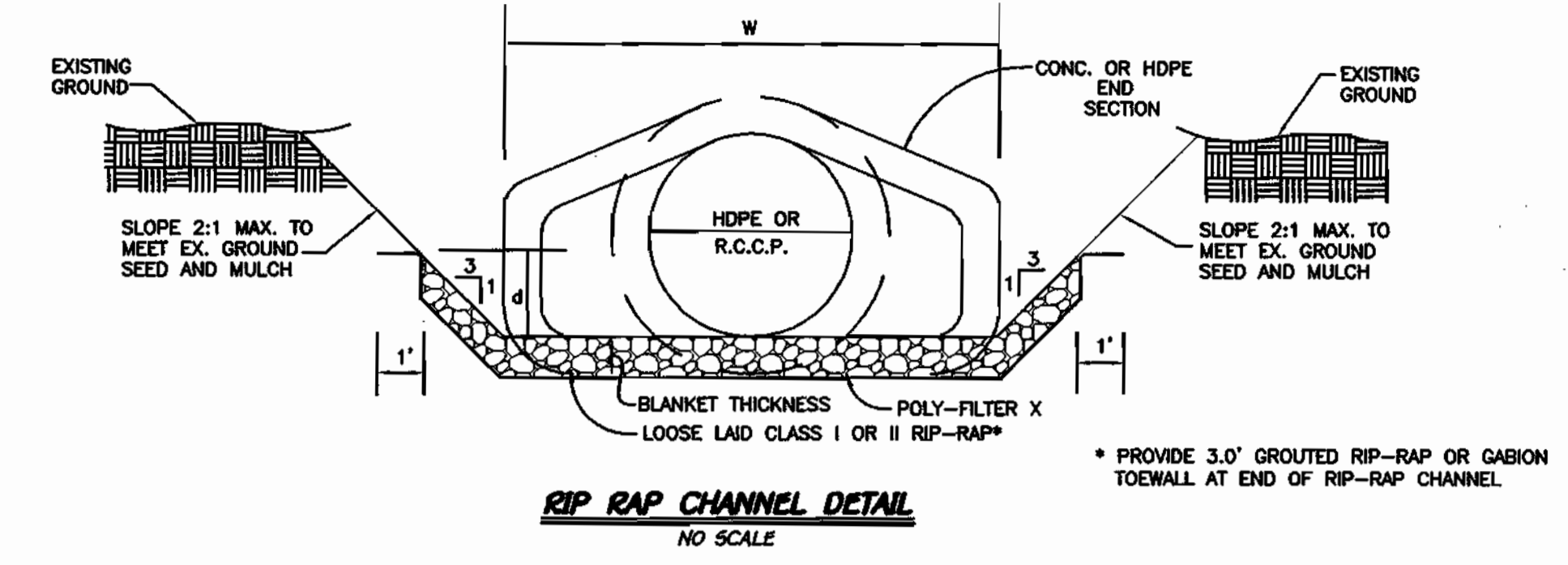
STRUCTURE SCHEDULE

STRUCTURE NO.	TOP ELEVATION	INV. IN	INV. OUT	ROAD NAME	ROAD STA.	OFFSET	TYPE	REMARKS
I-1	* 366.00		362.00		N 562,283.30 E 1,373,387.83		'D' INLET	S.D. 4.39
I-2	366.80	360.25	360.00	ABRIANNA WAY	STA. 9+44.50	12' L	A-10	S.D. 4.41
I-3	379.00	368.70	368.20	ABRIANNA WAY	STA. 1+07.50	12' R	A-10	S.D. 4.41
I-4	* 382.20	377.26, 377.26	376.76		N 561,944.91 E 1,375,678.23		'D' INLET	S.D. 4.39
I-5	383.00	378.06	377.56	BRIARGLEN DRIVE			A-10	S.D. 4.41
I-6	391.73	383.16, 383.16	382.66	BRIARGLEN DRIVE	STA. 4+77	12' L	A-10	S.D. 4.41
I-7	390.50		386.35	MARYLAND ROUTE 103	STA. 6+07	39' R	COG-10 INLET	MD-374.61
I-8	384.50	378.50	376.75	OXFORD DRIVE	N 562,694.74 E 1,374,020.02		A-10	S.D. 4.41
I-9	390.00		384.00	OXFORD DRIVE	STA. 1+74.50	32' L	A-10	S.D. 4.41
I-10	* 365.50		358.61		N 562,519.75 E 1,374,252.77		'D' INLET	S.D. 4.39
I-11	* 385.50	382.00	381.90		N 562,862.97 E 1,374,480.91		'D' INLET	S.D. 4.39
I-12	* 367.17		364.01	BRIARGLEN DRIVE	STA. 4+85	76' R	'D' INLET	S.D. 4.39
I-13	* 390.00		384.06	MARYLAND ROUTE 103	STA. 6+09	22' L	DOUBLE 'K' INLET	MD-378.11
I-14	* 384.15	380.90	380.15		N 562,826.82 E 1,374,074.43		'D' INLET	S.D. 4.39
I-15	391.80		384.93	BRIARGLEN DRIVE	STA. 0+28	12' R	A-10	S.D. 4.41
M-1	365.10	355.72	355.22		N 562,390.95 E 1,374,285.25		4' STD. MANHOLE	G. 5.12
M-2	365.04	357.07	356.57		N 562,459.70 E 1,374,274.45		4' STD. MANHOLE	G. 5.12
** M-2A	368.00	361.99, 368.49, 368.49	357.99		N 562,504.10 E 1,374,230.05		5' STD. MANHOLE	G. 5.13
M-3	367.00	360.44	359.94		N 562,446.10 E 1,374,125.94		4' STD. MANHOLE	G. 5.12
M-4	369.00	361.63, 365.00	361.13		N 562,341.54 E 1,374,033.87		4' STD. MANHOLE	G. 5.12
M-5	381.00	371.91, 371.81	371.31	ABRIANNA WAY	STA. 0+26		4' STD. MANHOLE	G. 5.12
M-6	386.50	385.72	385.22		N 562,756.16 E 1,373,680.44		4' STD. MANHOLE	G. 5.12
M-7	384.00	373.04	372.54		N 562,250.25 E 1,373,717.19		4' STD. MANHOLE	G. 5.12
M-7A	370.00	366.13	362.15		N 562,245.23 E 1,373,809.11		4' STD. MANHOLE	G. 5.12
M-8	393.75	379.98	379.48		N 562,396.12 E 1,373,678.22		4' STD. MANHOLE	G. 5.12
M-9	396.00	383.10	382.60		N 562,658.59 E 1,373,676.81		4' STD. MANHOLE	G. 5.12
R-1	359.50	353.00	352.50		N 562,163.15 E 1,374,280.52		CONC. RISER	
S-1	356.00	354.00	354.00		N 562,506.78 E 1,374,279.89		HDPE END SECTION	
S-2	353.63	351.13	351.13		N 562,193.92 E 1,374,243.22		CONC. END SECTION	S.D. 5.52
S-3	364.00	362.00	362.00		N 562,244.74 E 1,373,884.10		HDPE END SECTION	

* - DENOTES THROAT ELEVATION
** - LINE BOTTOM OF MANHOLE WITH GRANITE BLOCKING

CONSTRUCTION SPECIFICATIONS FOR RIP-RAP OUTFALLS

- The subgrade for the filter, riprap or gabion shall be prepared to the required lines and grades. Any fill required in the subgrade shall be compacted to a density of approximately that of the surrounding undisturbed material.
- The rock or gravel shall conform to the specified grading limits when installed respectively in the riprap or filter.
- Filter cloth shall be protected from punching, cutting or tearing. Any damage other than an occasional hole shall be repaired by placing another piece of cloth over the damaged part or by completely replacing the cloth. All overlaps whether for repairs or for joining two pieces of cloth shall be a minimum of one foot.
- Stone for the riprap or gabion outlets may be placed by equipment. Both shall each be constructed to the full course thickness in one operation and in such a manner as to avoid displacement of underlying materials. The stone for riprap or gabion outlets shall be delivered and placed in a manner that will insure that it is reasonably homogenous with the smaller stones and spalls filling the voids between the larger stones. Riprap shall be placed in a manner to prevent damage to the filter blanket or filter cloth. Hand placement will be required to the extent necessary to prevent damage to the permanent works.



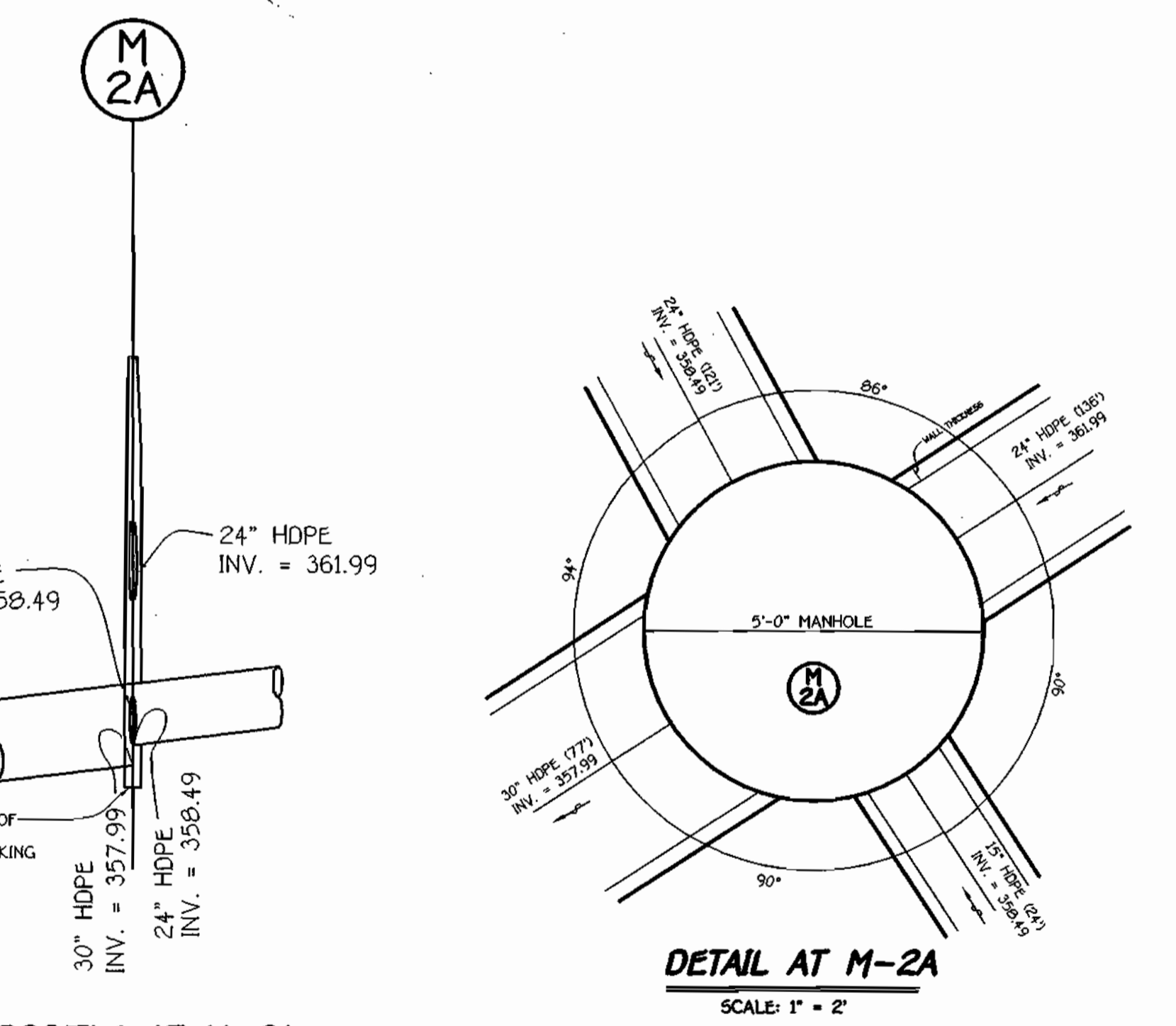
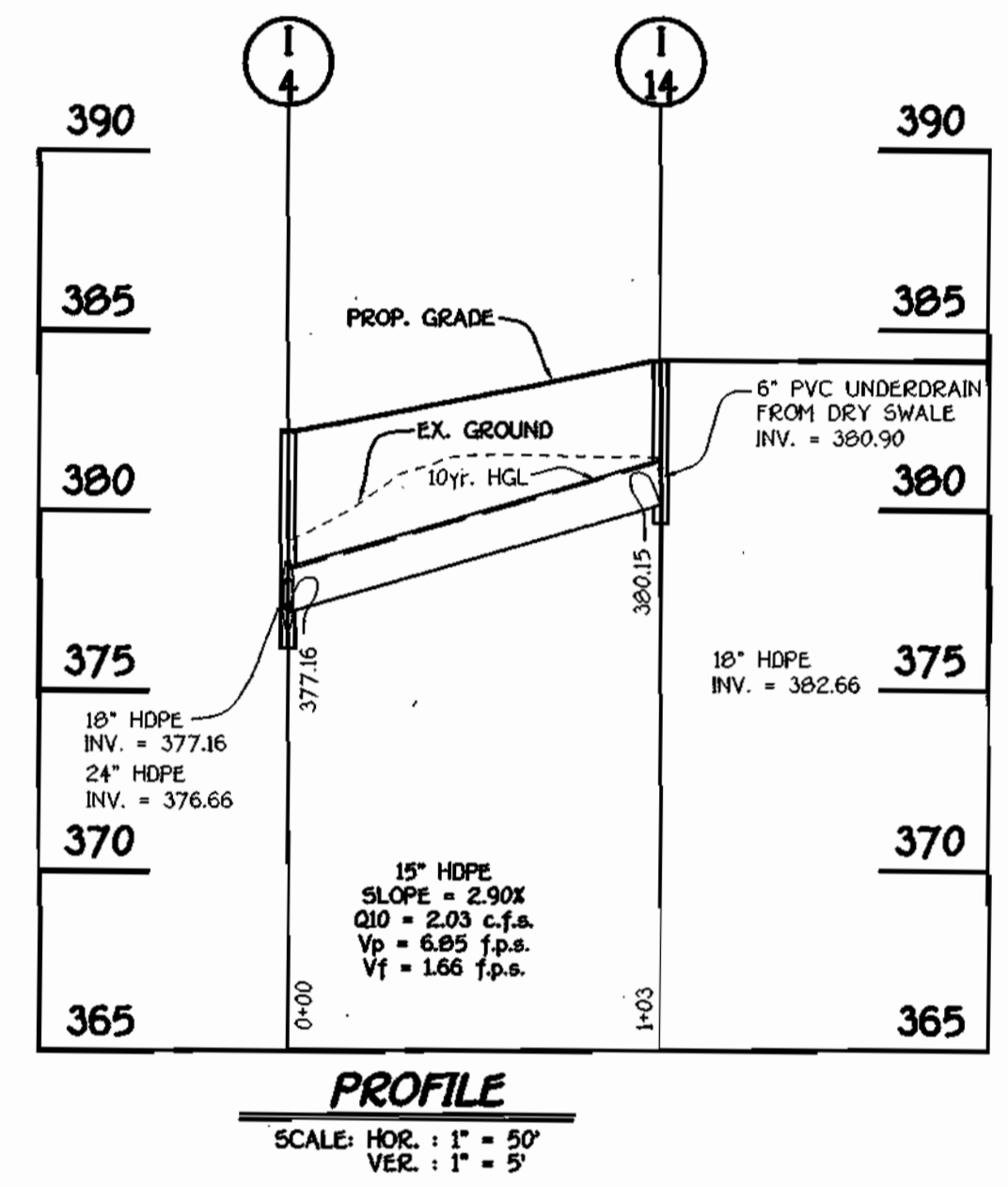
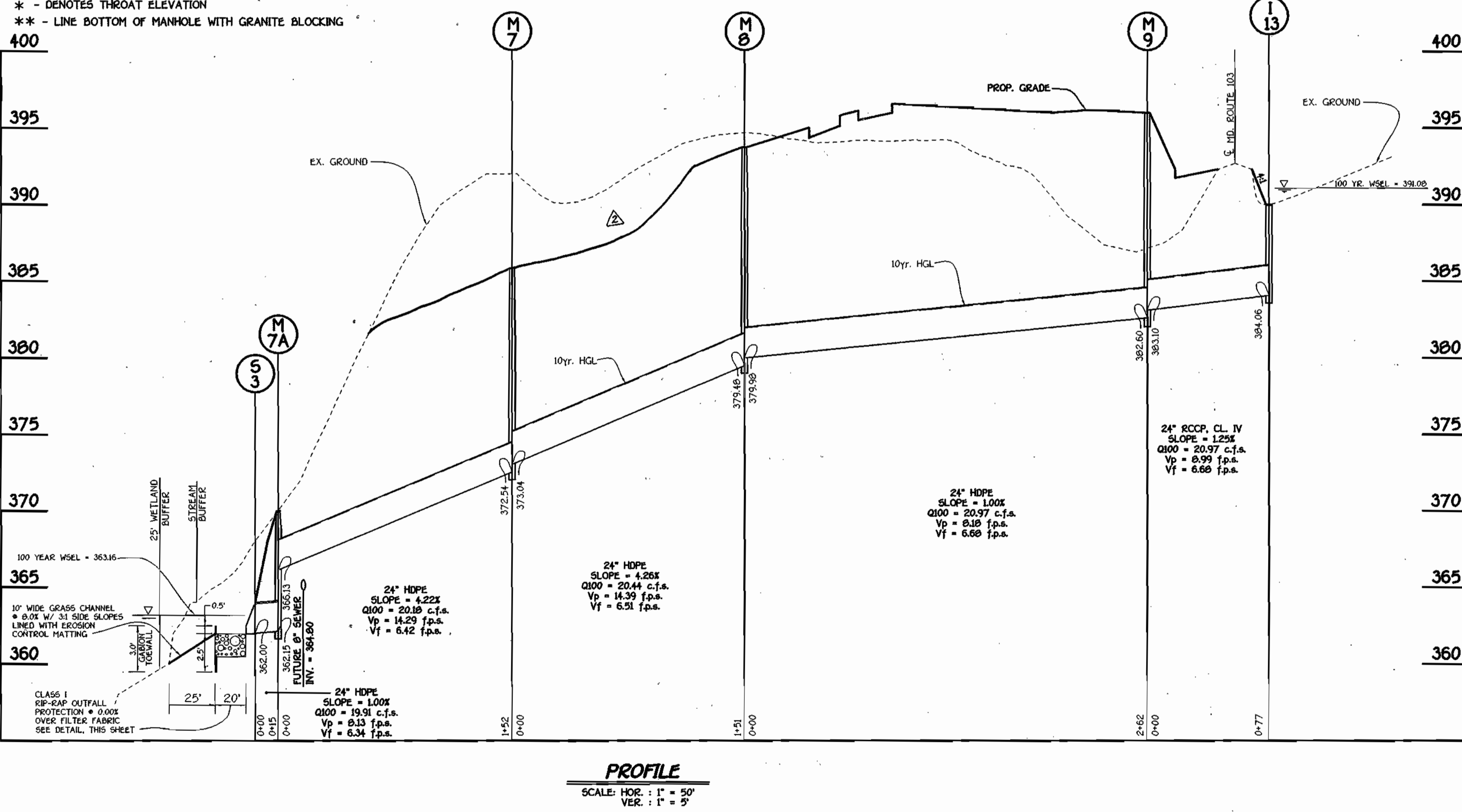
RIP-RAP CHANNEL DESIGN DATA

STRUCTURE	AREA	WETTED PERIMETER	R	R ^{2/3}	S	S ^{1/2}	W	d	N	V (F.P.S.)	Q10 (C.F.S.)	10 YR WSEL	RIIP-RAP SIZE CLASS	BLANKET THICKNESS	PIPE DIA.	
S-1	13.11 SF	13.96'	0.9391	0.9590	0.0050	0.0707	5.0'	1.42'	0.04	2.52	33.03	357.76	I 9.5"	15"	19"	30"
S-2	13.10 SF	13.96'	0.9384	0.9585	0.0050	0.0707	5.0'	1.42'	0.04	2.52	33.0	351.53	II 16"	24"	32"	30"
S-3	9.09 SF	11.93'	0.7619	0.8342	0.0050	0.0707	5.0'	1.10'	0.04	2.19	19.81	363.10	I 9.5"	15"	19"	24"

100 YR FOR S-3

PIPE SCHEDULE

SIZE	CLASS	LENGTH
15"	HDPE	638'
18"	HDPE	970'
24"	HDPE	837'
30"	HDPE	209'
24"	RCCP, CL. IV	77'



FISHER, COLLINS & CARTER, INC.
CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
CENTRAL SQUARE OFFICE PARK - 10722 BALTIMORE NATIONAL PIKE
ELLICOTT CITY, MARYLAND 21117
4100 W. - 2005

7/31/02 ADDED INLET I-15 AND STORM DRAIN PIPE TO I-2 AND REVISED

DATE DESCRIPTION REVISION BLOCK

ENGINEER'S CERTIFICATE
I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.

Signature of Engineer (print name below signature) **CHARLES J. CROVO, SR., P.E., L.S.** Date **8/22/02**

DEVELOPER'S CERTIFICATE
I/We certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize periodic on-site inspection by the Howard Soil Conservation District.

Signature of Developer (print name below signature) **DONALD R. REUWER, JR.** Date **8/22/02**

DEVELOPER/OWNER
RESK, L.L.C.
c/o LAND DESIGN AND DEVELOPMENT, INC.
8000 MAIN STREET
ELLICOTT CITY, MARYLAND 21103

BUILDER
RYAN HOMES, INC.
11660 CROWNDRIVE DRIVE
SUITE 128
OWINGS MILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Director - Department of Planning and Zoning **10/3/02** Date

Chief, Division of Land Development **10/3/02** Date

Chief, Development Engineering Division **9/22/02** Date

SUBDIVISION THE COURTYARDS AT THE TIMBERS SECTION/AREA 617 PARCEL NO. 617

DEED REF. BLOCK NO. ZONE TAX/ZONE ELEC. DIST. CENSUS TR.
5609 / 611 3 POR 37 1st. 6030

WATER CODE D 04 SEWER CODE 2810000

STORM DRAIN PROFILES

THE COURTYARDS AT THE TIMBERS
TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

ZONED: POR

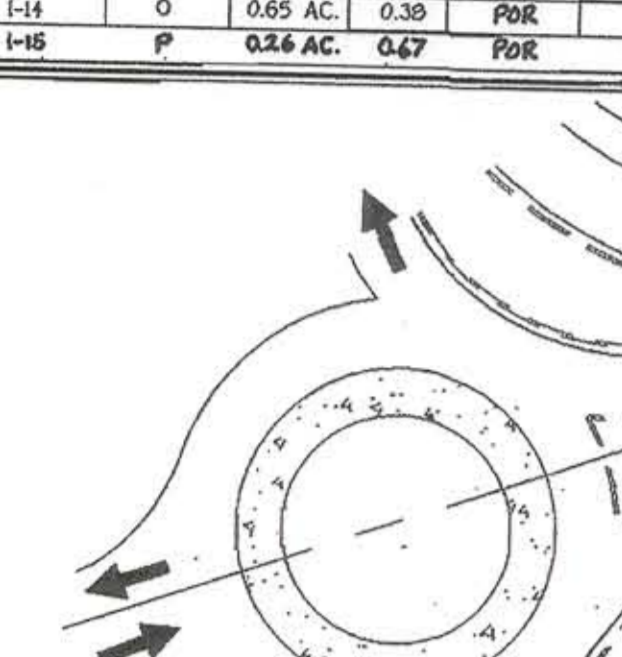
TAX MAP No.: 37 PARCEL No.: 617 GRID No.: 3
FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND
SCALE: AS SHOWN DATE: AUGUST 21, 2002

SHEET 8 OF 24 **SDP 02-55**

DRAINAGE AREA DATA					
STRUCTURE NO.	DRAINAGE AREA	AREA	C	ZONED	I IMP.
I-1	A	0.64 AC.	0.30	POR	131
I-2	B	0.32 AC.	0.57	POR	674
I-3	C	0.74 AC.	0.67	POR	691
I-4	D	0.97 AC.	0.75	POR	821
I-5	E	0.20 AC.	0.45	POR	351
I-6	F	0.61 AC.	0.66	POR	631
I-7	G	0.47 AC.	0.61	POR	351
I-8	H	0.55 AC.	0.44	POR	351
I-9	I	0.42 AC.	0.76	POR	631
I-10	J	0.51 AC.	0.77	POR	631
I-11	K	0.70 AC.	0.24	POR	61
I-12	L	0.16 AC.	0.22	POR	61
I-13	M	0.04 AC.	0.21	POR	61
I-14	N	5.86 AC.	0.45	R-5C	651
I-15	O	0.65 AC.	0.39	POR	261
I-16	P	0.44 AC.	0.47	POR	691



MARYLAND ROUTE 103
MEADOWRIDGE ROAD
(MINOR ARTERIAL)



MEADOWRIDGE ROAD
OFF RAMP
FROM ROUTE 100

NOTE: AREA SHOWN IS NOT BEING MANAGED FOR CPD AND WQ UNDER THIS SITE DEVELOPMENT PLAN. AT THE TIME OF FUTURE DEVELOPMENT OF THIS AREA, CPD AND WQ MANAGEMENT WILL BE REQUIRED.

MARYLAND ROUTE 100
RIGHT OF WAY
SHA PLAT 53943 &
53944
ZONED R-5C

LOC 1196 N
E 1,374,500

PARCEL A-6
LYNDWOOD SQUARE
PARCELS A-1 THRU
A-5, B-1
AND PARCEL Q-1
ZONED R-5C

PROPOSED B.M.P. NO. 1
S.W.M. POND
TYPE: MICRO-POOL
HAZARD CLASS 'A'

PLAN
SCALE: 1" = 50'
E 1,374,500

ENGINEER'S CERTIFICATE
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Signature of Engineer (print name below signature) CHARLES J. CROVO, SR., P.E., I.S. 1/23/02 Date

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Signature of Developer (print name below signature) DONALD R. REUWER, JR. 8/22/02 Date

DEVELOPER/OWNER
R2K, L.L.C.
c/o LAND DESIGN AND DEVELOPMENT, INC.
8000 MAIN STREET
ELLICOTT CITY, MARYLAND 21043

BUILDER
RYAN HOMES, INC.
11460 CROWNDRIVE
SUITE 129
OWINGS MILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING			
Director	10/3/02	Date	
Chief, Division of Land Development	10/3/02	Date	
Chief, Department Engineering Division	9/26/02	Date	
SUBDIVISION	SECTION/AREA	TAX/ZONE	PARCEL NO.
THE COURTYARDS AT THE TIMBERS	617	3	617
DEED REF.	BLOCK NO.	ELEC. DIST.	CENSUS TR.
5609 / 611	3	37	6030
WATER CODE	SEWER CODE		
D 04	2810000		

DRAINAGE AREA MAP
THE COURTYARDS AT THE TIMBERS
TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS
ZONED: POR
TAX MAP No: 37 PARCEL No: 617 GRID No: 3
FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND
SCALE: AS SHOWN DATE: AUGUST 21, 2002
SHEET 9 OF 24 S0P 02-55

FISHER, COLLINS & CARTER, INC.
CIVIL, ENGINEERING CONSULTANTS & LAND SURVEYORS
CENTRAL GARAGE, OFFICE PARK - 10772 BALTIMORE NATIONAL PARK
ELLICOTT CITY, MARYLAND 21114
(410) 461-2895



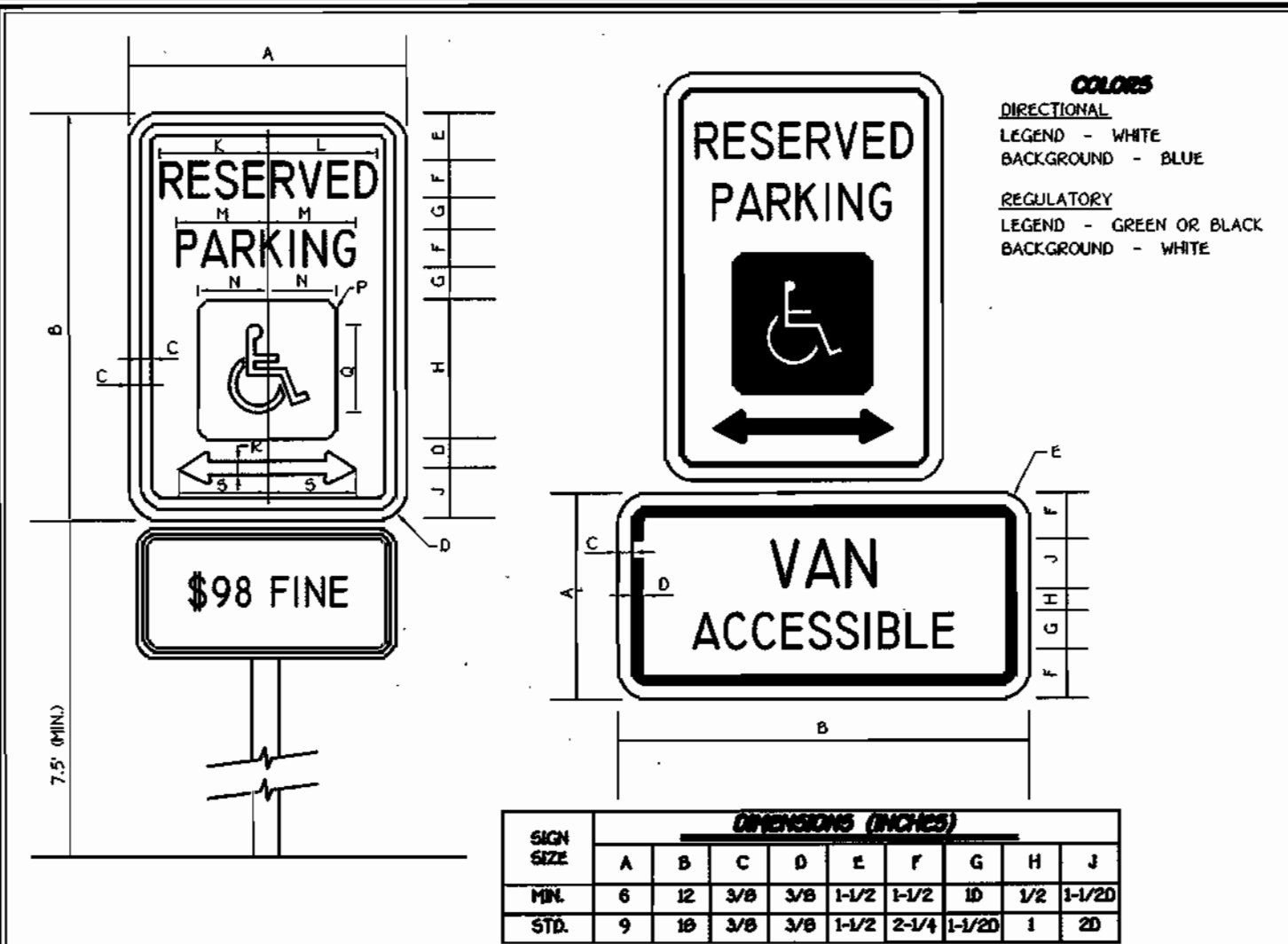
DATE	DESCRIPTION	REVISION BLOCK
7/31/03	REVISED PARKING & CONDO BUILDING 'A' (PLAN VIEW), ADDED SIDEWALK AND ASSOCIATED GRADING ALONG MEADOWRIDGE ROAD, ADDED STORM DRAIN EASEMENT AT I-13, REVISED GRADING AT SITE ENTRANCE, ADDED INLET I-15 AND STORM DRAIN PIPE TO I-2, REVISED 6" SEWER MAIN M4-120 AND M4-125, REVISED MHC TO 2' AND 6", AND REMOVED BASEMENT ELEVATION FROM CONDO BUILDINGS 'E', 'F' & 'G'	
10/22/02	ADD UNITS 13 & 14	

GENERAL SPACE LOT 56
SHADY OAKS
SECTION ONE AREA ONE
PLAT No. 11497
ZONED R-5C

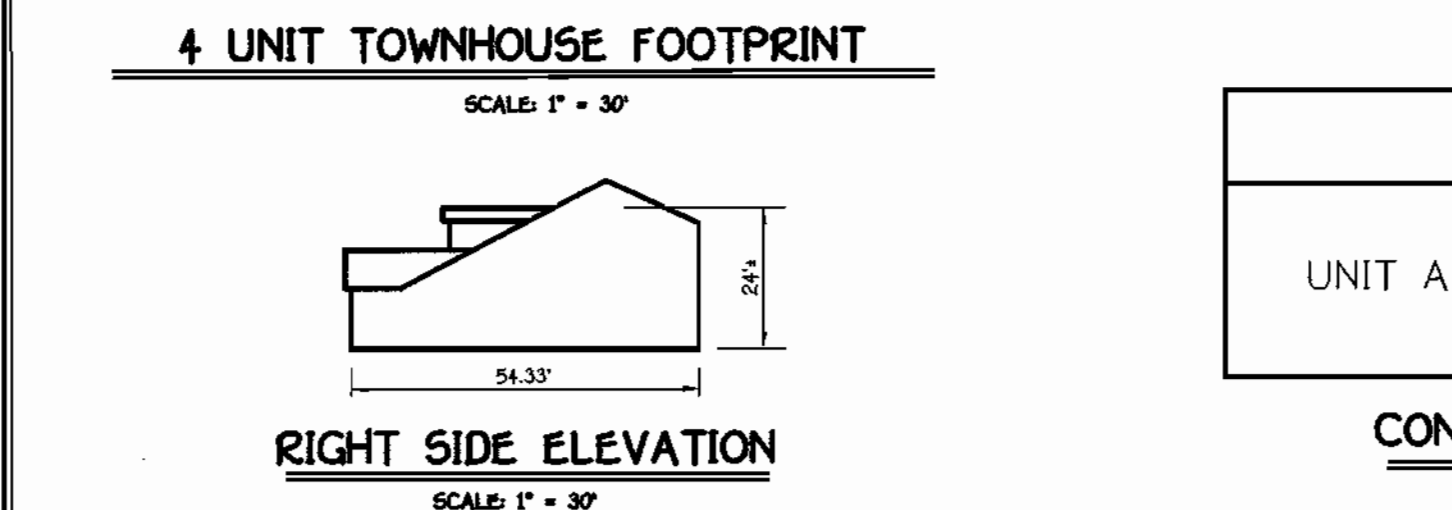
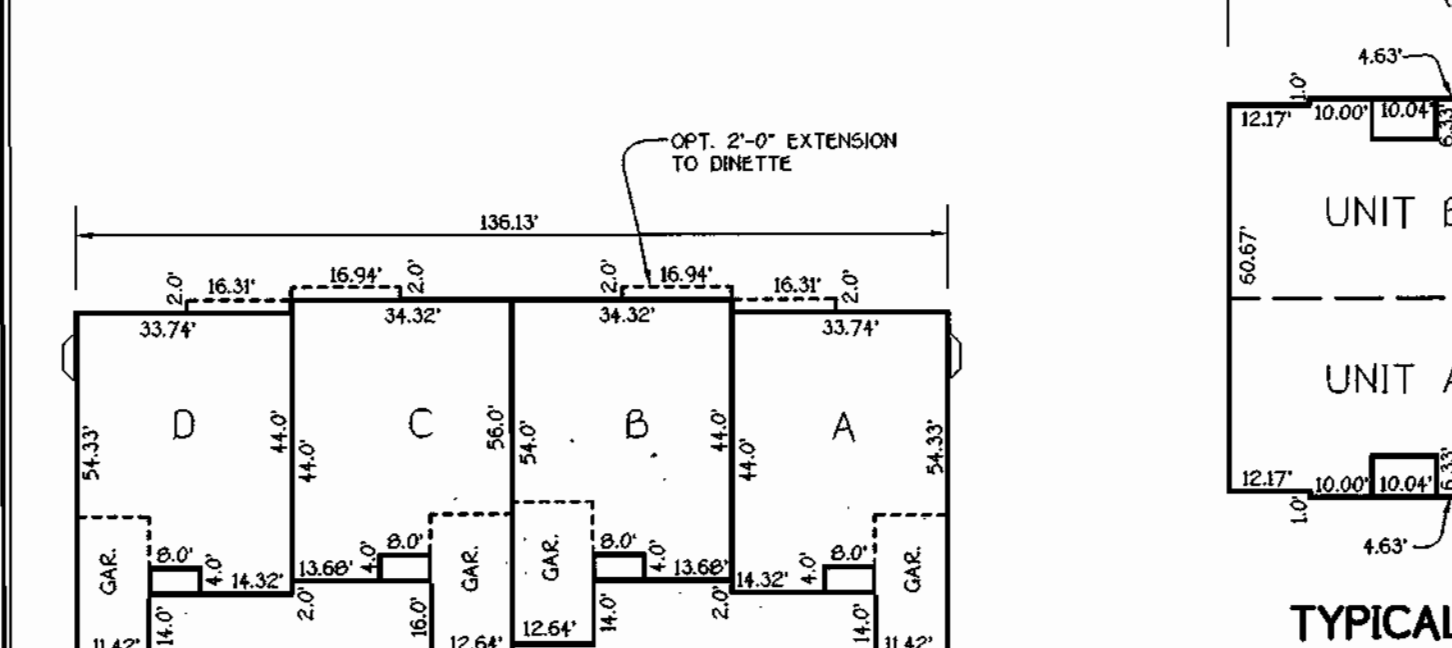
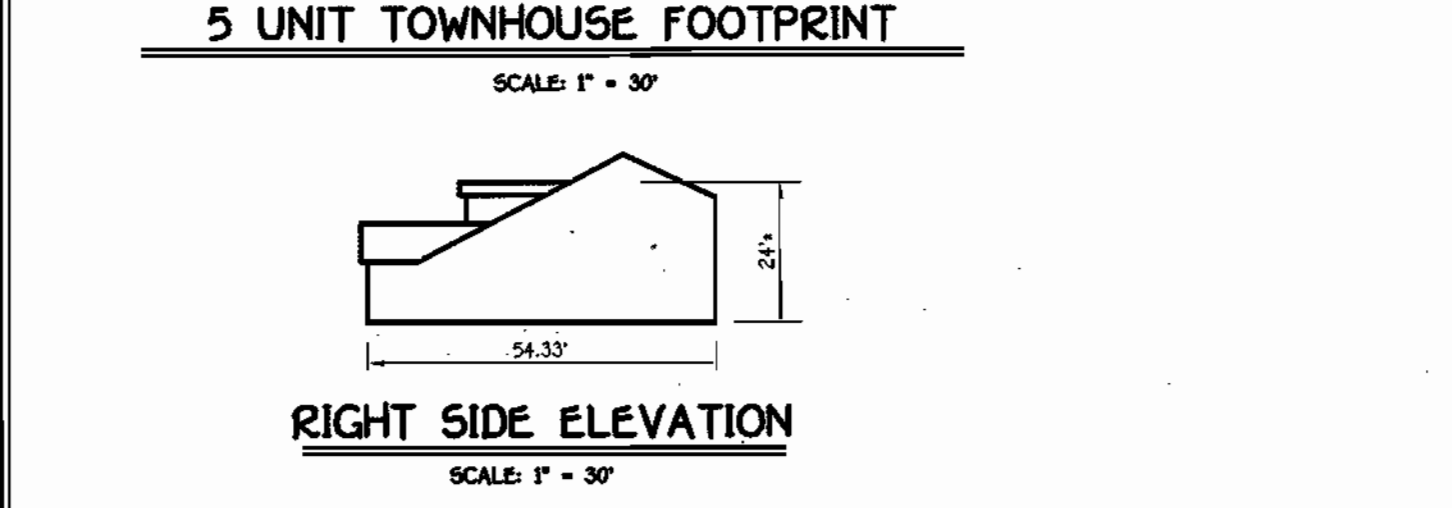
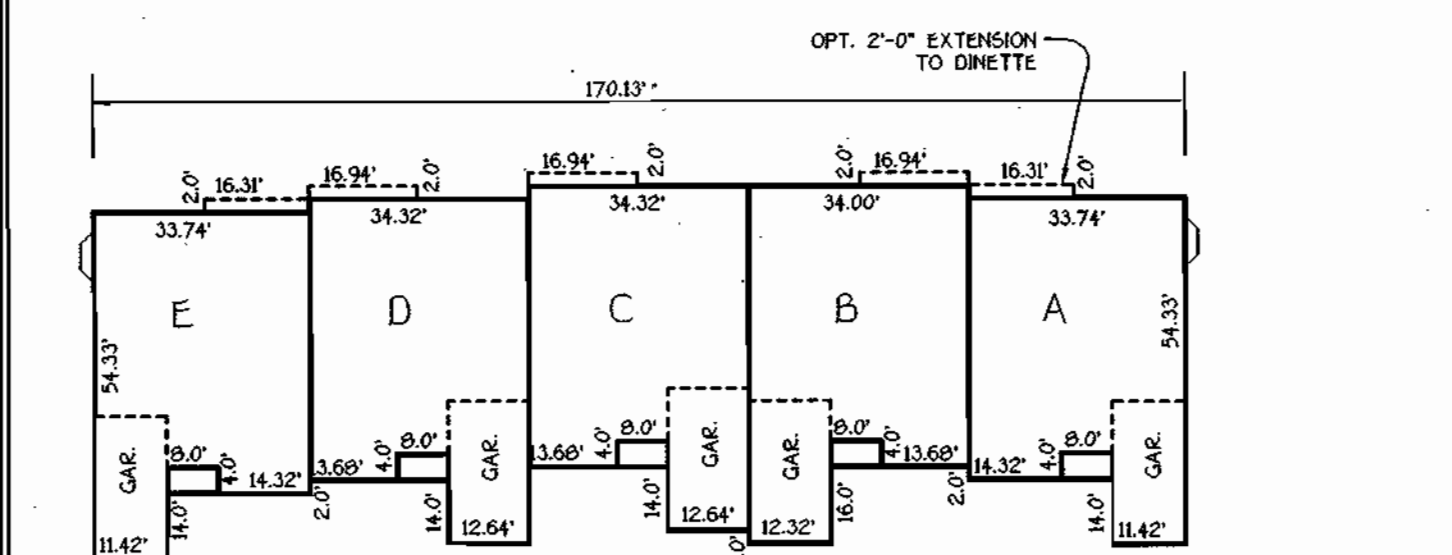
PROPOSED B.M.P. NO. 2
DRY SWALE

PUBLIC ROAD
SHADY OAK LANE

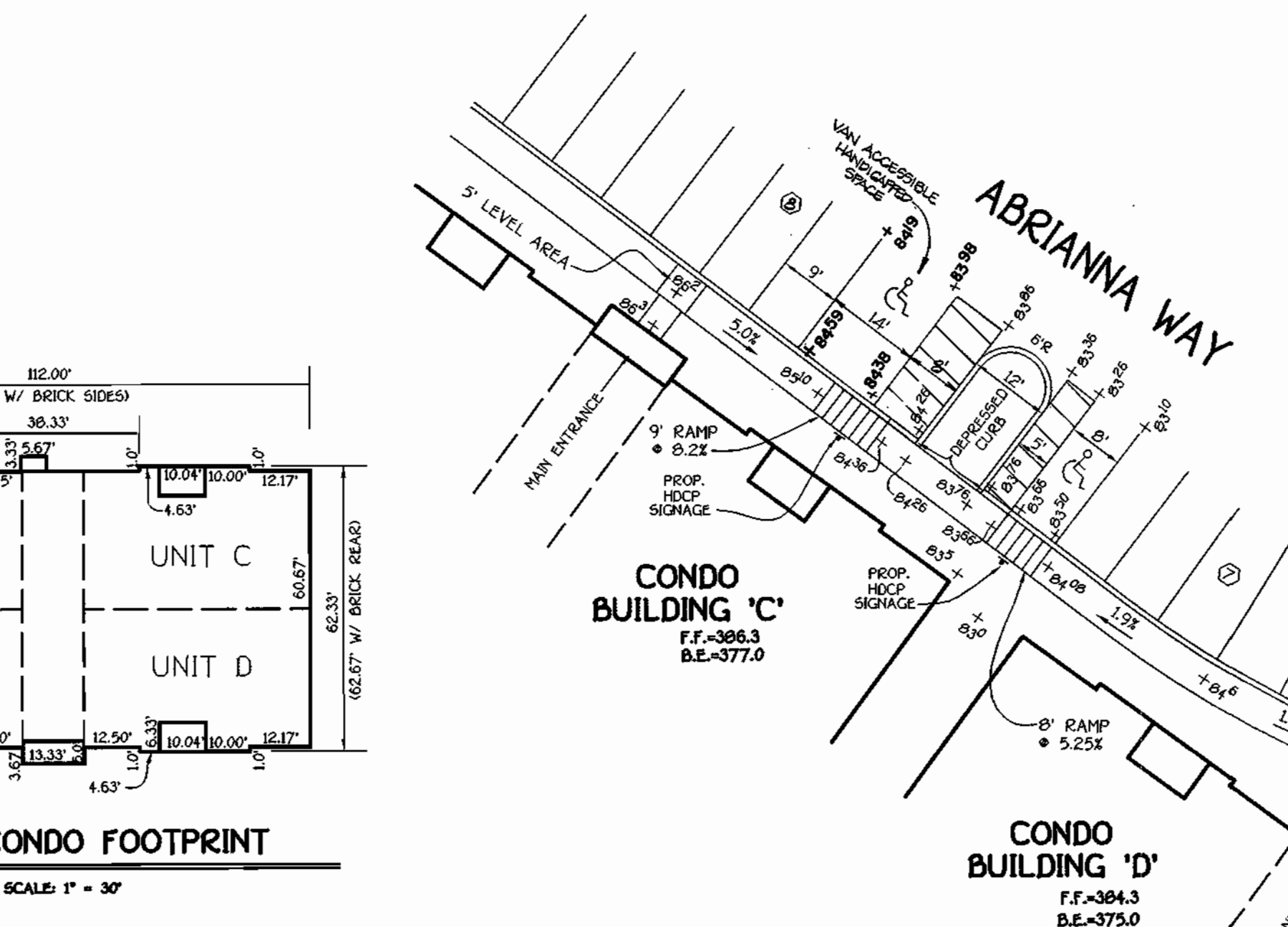
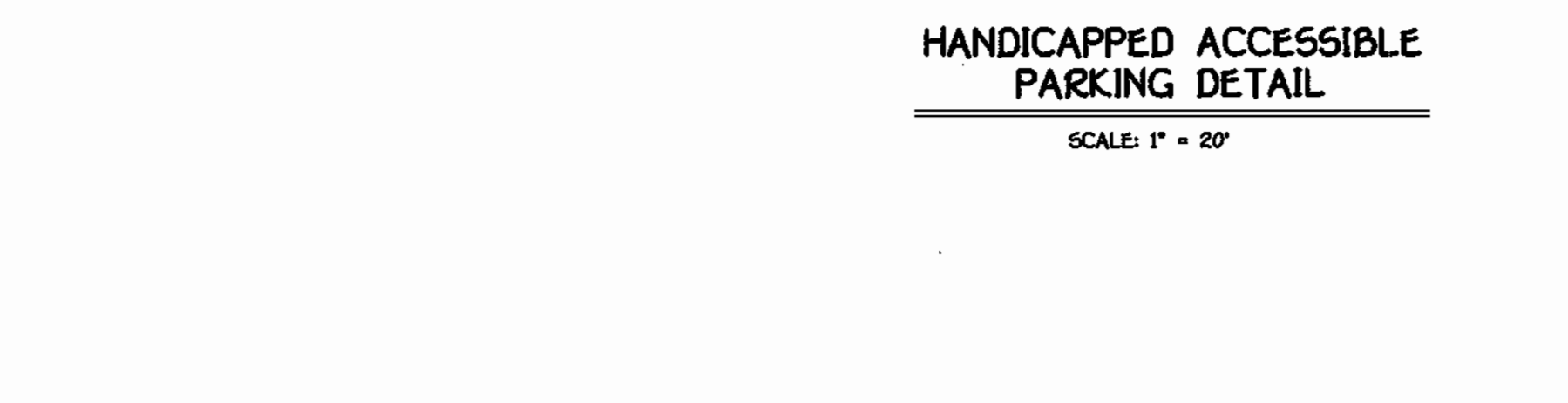
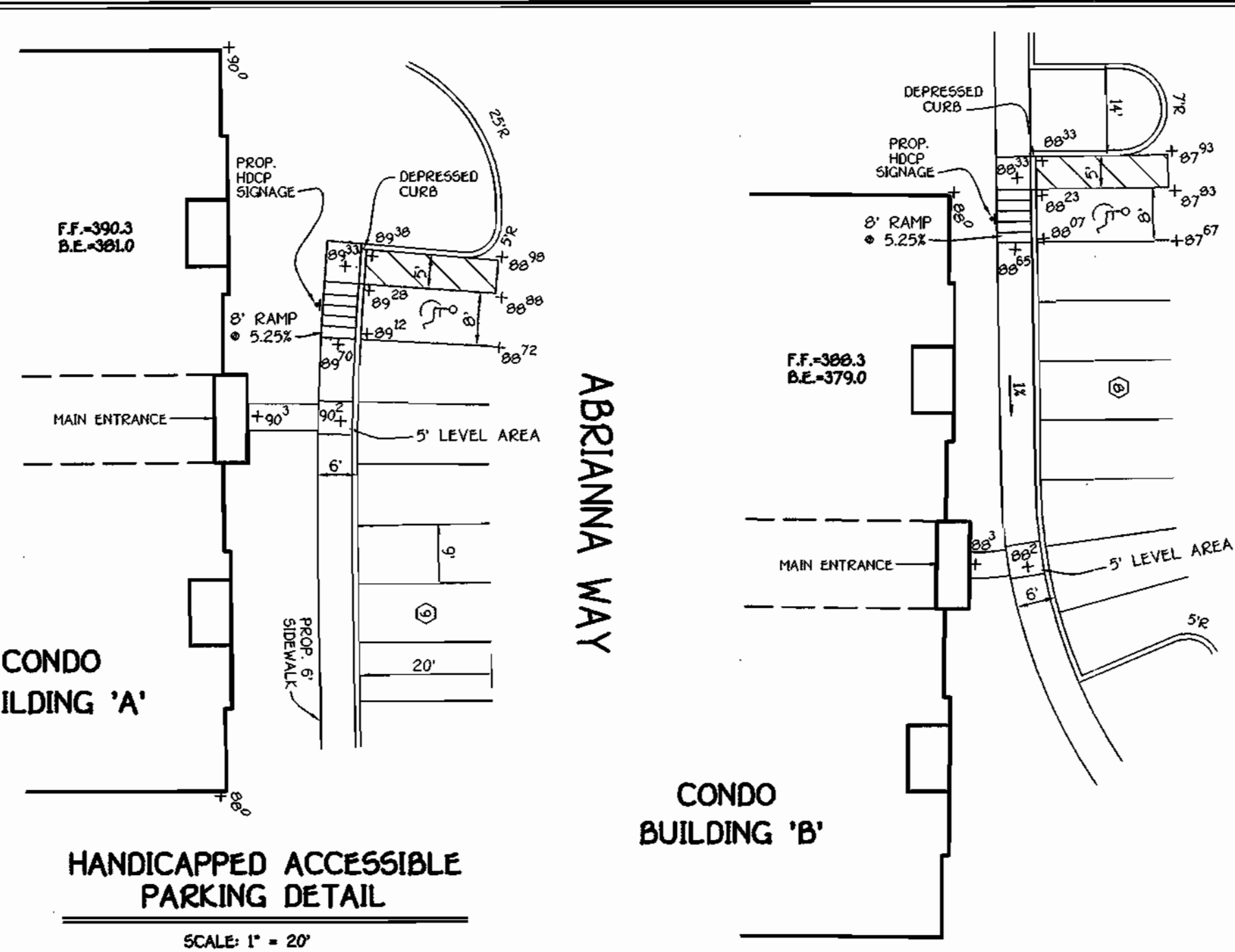
AUTUMN SPELL
PUBLIC ROAD



SIGN SIZE	A	B	C	D	E	F	G	H	I	J	K	L	M	N	P	Q	R	S
STD.	12	18	1/2	1-1/2	2	2	2	1	6	2-1/2	1-7/8	5	4-1/4	3	1/2	4	3/4	3-7/8



DATE	DESCRIPTION	REVISION BLOCK

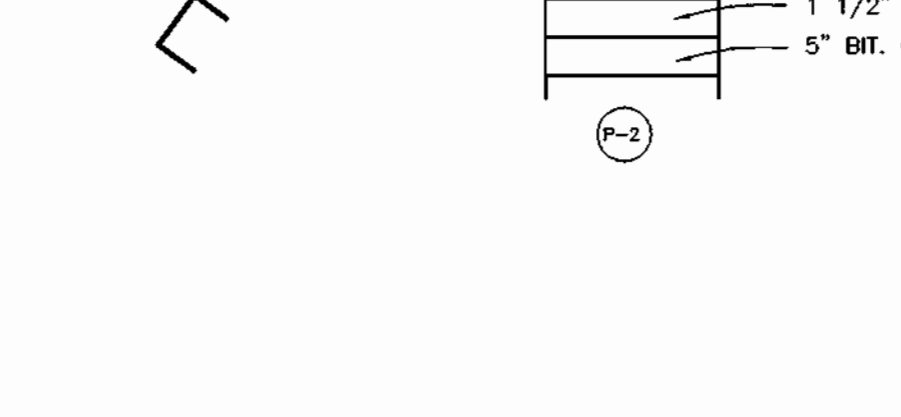
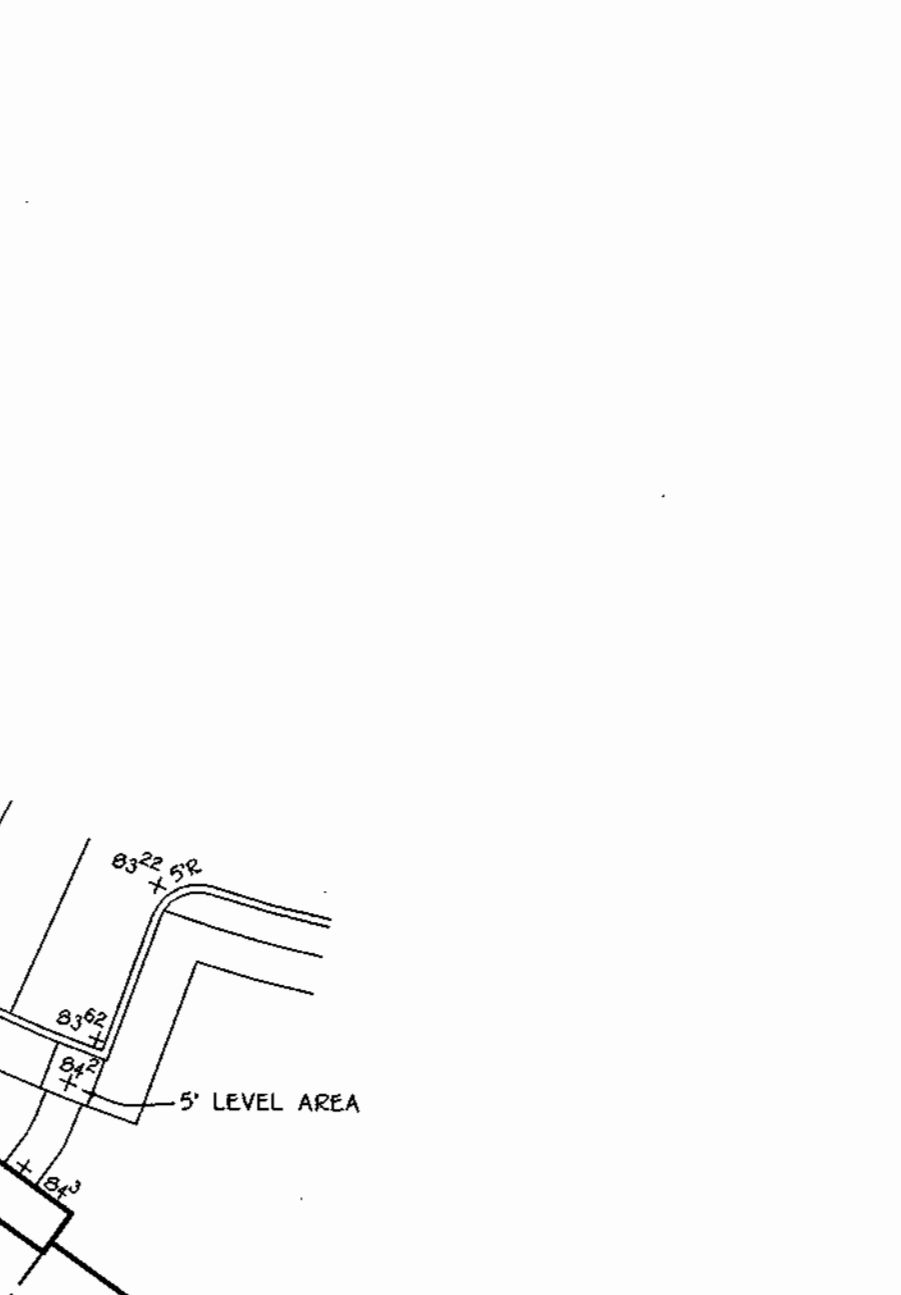
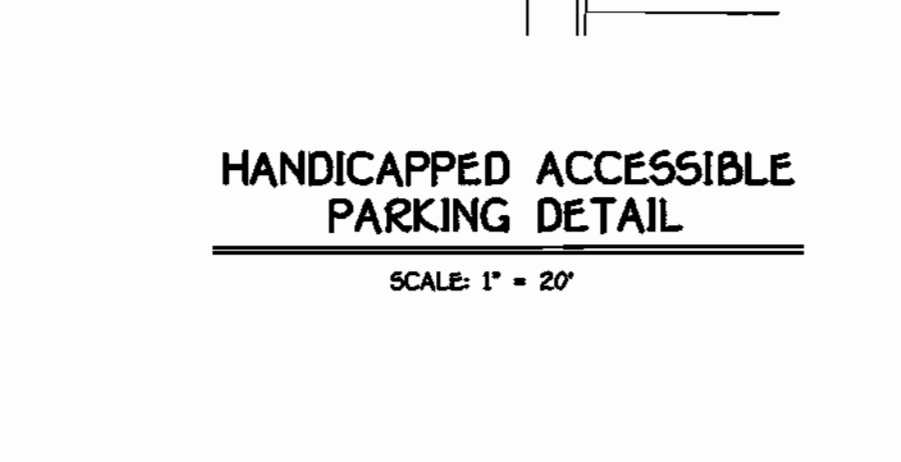
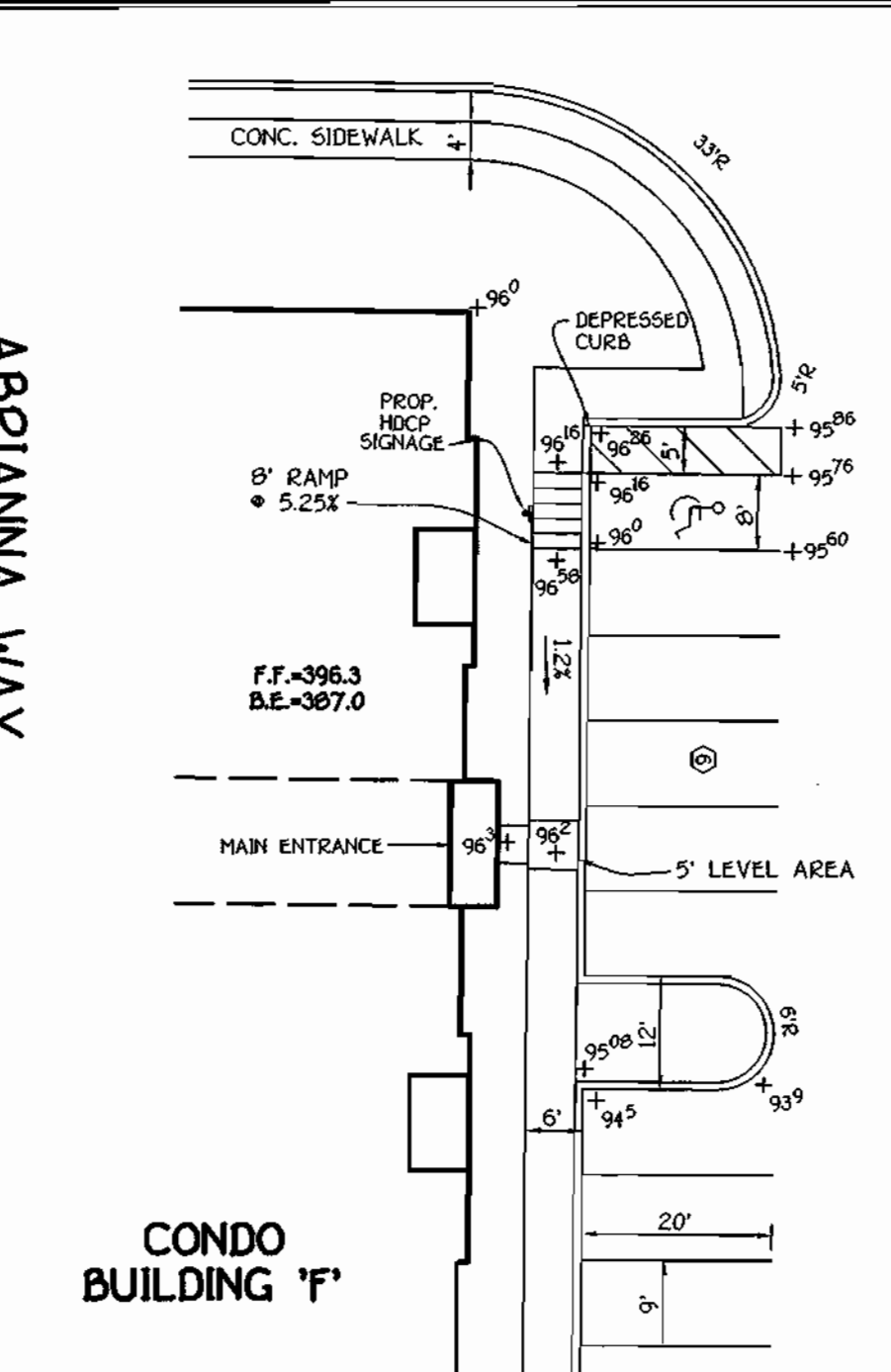


ENGINEER'S CERTIFICATE
 I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.

CHARLES J. CROVO, SR., P.E., L.S. 2/22/02
 Signature of Engineer (print name below signature) Date

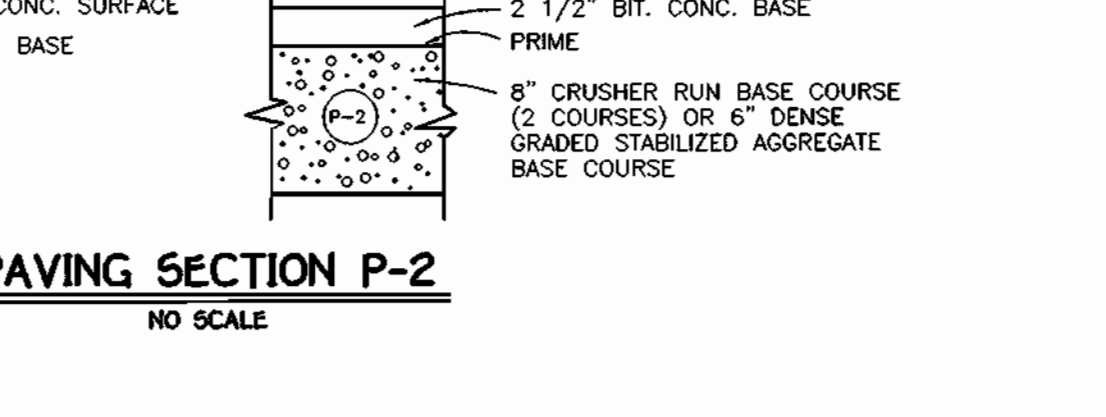
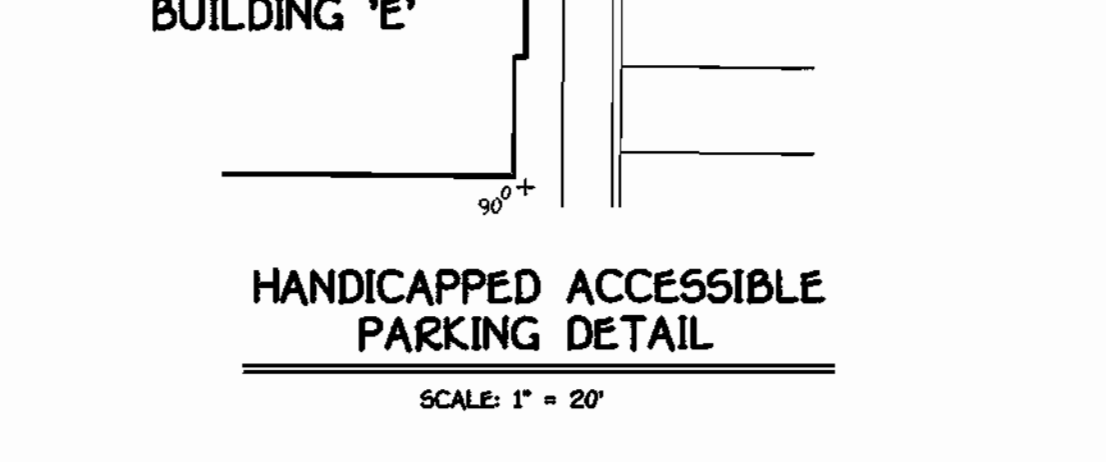
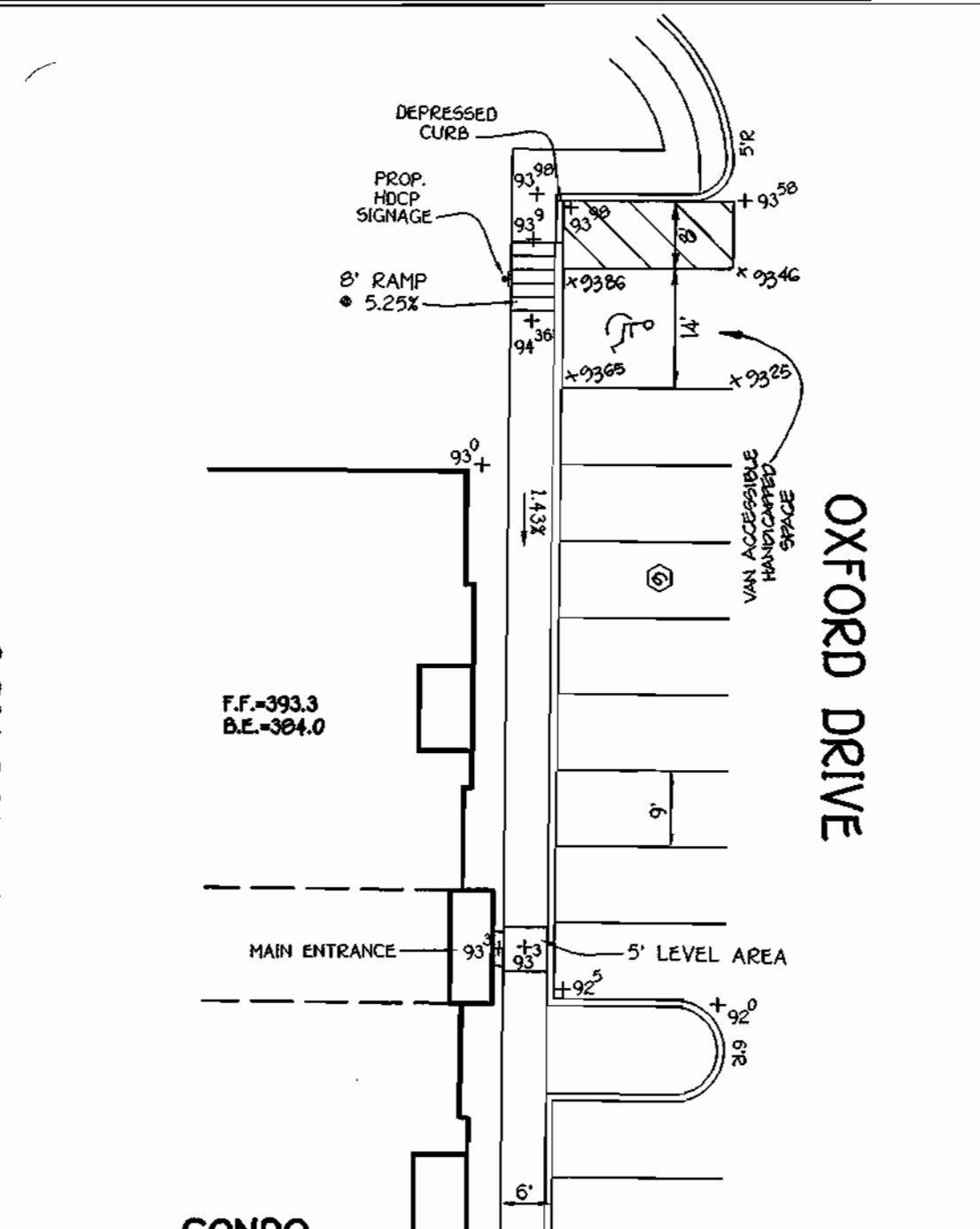
DEVELOPER'S CERTIFICATE
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DONALD R. REUWER, JR. 8/22/02
 Signature of Developer (print name below signature) Date



DEVELOPER/OWNER
 REEK, L.L.C.
 c/o LAND DESIGN AND DEVELOPMENT, INC.
 8000 MAIN STREET
 ELLICOTT CITY, MARYLAND 21043

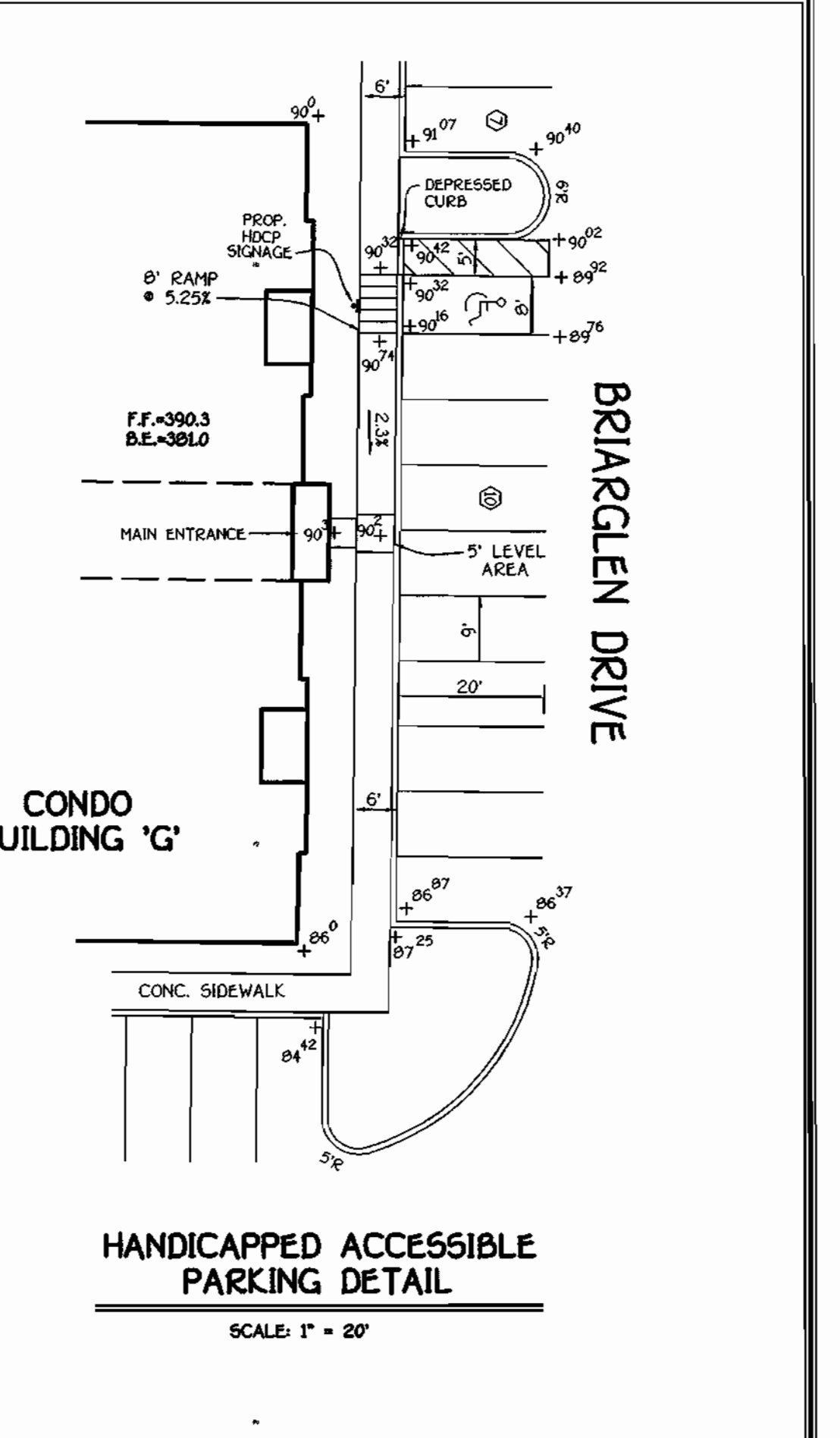
BUILDER
 RYAN HOMES
 11460 CROWNDRIVE DRIVE
 SUITE 120
 OWINGS MILLS, MARYLAND 21117



APPROVED: DEPARTMENT OF PLANNING AND ZONING

Director, Department of Planning and Zoning: 10/3/02
 Chief, Division of Land Development: 11/3/02
 Chief, Development Engineering Division: 9/2/02

SUBDIVISION	SECTION/AREA	PARCEL NO.
THE COURTYARDS AT THE TIMBERS		617
DEED REF.	BLOCK NO.	ZONE
5609 / G11	3	POR
WATER CODE	TAX/ZONE	ELEC. DIST.
D 04	37	1st.
	SEWER CODE	CENSUS TR.
		6030



HANDICAPPED PARKING DETAILS, BUILDING FOOTPRINTS & SEQUENCE OF CONSTRUCTION

THE COURTYARDS AT THE TIMBERS
 TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

ZONED: POR

TAX MAP No.: 37 PARCEL No.: 617 GRID No.: 3
 FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND
 SCALE: AS SHOWN DATE: AUGUST 21, 2002

SHEET 11 OF 24 **SDP 02-55**

K:\Drawings\330744\KHAM Property\SDP - NO OFFICE BLDG\330744-SHT11-DETAILSHEET.dwg, 8/22/2002, 2:01:38 PM

20.0 STANDARDS AND SPECIFICATIONS FOR VEGETATIVE STABILIZATION

DEFINITION

Using vegetation as cover for barren soil to protect it from forces that cause erosion.

Vegetative stabilization specifications are used to promote the establishment of vegetation on exposed soil. When soil is stabilized with vegetation, the soil is less likely to erode and more likely to allow infiltration of rainfall, thereby reducing sediment loads and runoff to downstream areas, and improving wildlife habitat and visual resources.

This practice shall be used on denuded areas as specified on the plans and may be used on highly erodible or critically eroding areas. This specification is divided into Temporary Seeding, to quickly establish vegetative cover for short duration (up to one year), and Permanent Seeding, for long term vegetative cover. Examples of appropriate areas for Temporary Seeding are Temporary Soil Stockpiles, cleared areas, and left-of-way construction phases, earth ditches, etc. For Permanent Seeding are lawns, dams, cut and fill slopes and other areas, former stockpile and staging areas, etc.

Planting vegetation in disturbed areas will have an effect on the water budget, especially on volumes and rates of runoff, infiltration, evaporation, transpiration, percolation, and groundwater recharge. Over time, with increased organic matter content and improved water holding capacity of the soil and subsequent plant growth, vegetation will help reduce the movement of sediment, nutrients, and other chemicals carried by runoff to receiving waters. Plants will also help protect groundwater supplies by assimilating those substances present within the root zone. Sediment control devices must remain in place during grading, seeded preparation, seeding, mulching and vegetative establishment to prevent large quantities of sediment and associated chemicals and nutrients from washing into surface waters.

SECTION 1 - VEGETATIVE STABILIZATION METHODS AND MATERIALS

- Site Preparation**
 - Install erosion and sediment control structures (either temporary or permanent) such as diversions, grade stabilization structures, berms, waterways, or sediment control basins.
 - Perform all grading operations at right angles to the slope. Final grading and shaping is not usually necessary for temporary seeding.
 - Schedule required soil tests to determine soil amendment composition and application rates for its use.
- Soil Amendments (Fertilizer and Lime Specifications)**
 - Soil tests must be performed to determine the exact ratios and application rates for both lime and fertilizer on sites having disturbed areas over 5 acres. Soil tests may be performed by the University of Maryland or a recognized commercial laboratory. Soil samples taken for engineering purposes may also be used for chemical analysis.
 - Fertilizers shall be uniform in composition, free flowing and suitable for accurate application by approved equipment. Manure may be substituted for fertilizer with prior approval from the appropriate authority. Fertilizers shall be labeled according to the applicable state fertilizer laws and shall bear the name, trade name, or trademark and warranty of the producer.
 - Lime materials shall be ground limestone (hydrated or burnt lime) may be substituted which contains at least 50% total oxides (calcium oxide plus magnesium oxide). Limestone shall conform to such fineness that at least 50% will pass through a #100 mesh sieve and 90-100% will pass through a #20 mesh sieve.
- Incorporate lime and fertilizer into the top 3-5" of soil by diking or other suitable means.**

- Temporary Seeding**
 - Seeded preparation shall consist of loosening soil to a depth of 3" to 5" by means of suitable agricultural or construction equipment, such as disk harrows or chisel plows or rippers mounted on construction equipment. After the soil is loosened it should not be compacted, but left in the rough condition. Seeding shall be done in a regular pattern with ridges running parallel to the contour of the slope.
 - Apply fertilizer and lime as prescribed on the plans.
 - Incorporate lime and fertilizer into the top 3-5" of soil by diking or other suitable means.
- Permanent Seeding**
 - Minimum soil conditions required for permanent vegetative establishment:
 - Soluble salts shall be less than 500 parts per million (ppm).
 - The soil shall contain less than 40% clay, but enough clay to hold a moderate amount of moisture. An exception is if loess or other deposits is to be placed, then a sand/silt/clay soil plus clay will be acceptable.
 - Soil shall contain 1.5% minimum organic matter by weight.
 - Soil must contain sufficient pore space to permit adequate root penetration.
 - If these conditions cannot be met by soils on site, adding topsoil is required in accordance with Section 21 and specifications for topsoil.
 - Areas previously graded in conformance with the drawings shall be maintained in a true and even grade, then scarified or otherwise loosened to a depth of 3-5" to permit bonding of the topsoil to the surface and to create horizontal erosion check spots to prevent topsoil from sliding down a slope.
 - Apply soil amendments as per soil test or as included on the plans.
 - Soil amendments into the top 3-5" of topsoil by diking or other suitable means. Lawn areas should be rolled to compact, remove lumps and objects like tree branches, and ready the area for seed and application, where site conditions will not permit normal seeded preparation, loosen surface soil by dragging with a heavy chain or other equipment to roughen the surface. Steep slopes (greater than 2:1) should be treated by a doker leaving the soil in an irregular condition with ridges running parallel to the contour of the slope. The top 1-2" of soil shall be loose and friable. Seeding loosening may not be necessary on newly disturbed areas.

- Seed Specifications**
 - All seed must meet the requirements of the Maryland State Seed Law. All seed shall be subject to re-testing by a recognized seed laboratory. All seed used shall have been tested within 6 months immediately preceding the use of the seed material on this job.
 - Note: Seed tags shall be made available to the inspector to verify type and rate of seed used.
 - Incubant - The incubant for testing purposes shall be a pure culture of the microorganism designated for the species. Incubants shall not be used later than the date indicated on the container. Add fresh incubant as directed on package. Use four times the recommended amount. Note: Incubant may be used for up to 12 months. Temperatures above 75-80° F. can weaken bacteria and make the incubant less effective.
- Methods of Seeding**
 - Hydroseeding** - Apply seed uniformly with hydroseeder (slurry includes seed and fertilizer), broadcast or drop seeded of cultivated soil.
 - Seed germination shall be incorporated into the subsoil at the rates prescribed on the Temporary or Permanent Seeding summaries or Tables 265 or 26. The seeded area shall then be rolled with a weighted roller to provide firm contact between seed and soil.
 - Where practical, seed should be applied in two directions perpendicular to each other. Apply half the seeding rate in each direction.
 - Drill or Cultipacker Seeding** - Mechanized seeders that apply and cover seed with soil.
 - Cultipacker seeders are required to bury the seed in such a fashion as to provide at least 1/4 inch of soil coverage. Seeded must be firm after planting.
 - Where practical, seed should be applied in two directions perpendicular to each other. Apply half the seeding rate in each direction.

- Mulch Specifications (in order of preference)**
 - Show shall consist of thoroughly threshed wheat, rye or oat straw, reasonable bright in color, and shall not be musty, moldy, clumped, or excessively chaffy and shall be free of noxious weed seeds as specified in the Maryland Seed Law.
 - Wood Cellulose Fiber Mulch (WCFM)
 - WCFM shall consist of specially prepared wood cellulose processed into a uniform fibrous physical mesh.
 - WCFM shall be dried green or contain a green dye in the package that will provide 10 days protection to facilitate inspection of the uniformity of the WCFM.
 - WCFM, including dye, shall contain no germination or growth inhibiting factors.
 - WCFM material shall be manufactured and processed in such a manner that the wood cellulose fiber mesh will remain in uniform suspension in water under agitation and will blend with seed, fertilizer and other additives to form a homogeneous slurry. The mulch material shall form a dense ground cover on application having moisture absorption and percolation properties and shall cover and hold grass seed in contact with the soil without inhibiting the growth of the grass seedlings.
 - WCFM material shall contain no chemical or other contaminants that will be phytotoxic.
 - WCFM must conform to the following physical requirements: fiber length to approximately 10 mm., diameter approximately 1 mm., pH range of 4.0 to 8.5, ash content of 1.5% maximum and water holding capacity of 25% minimum.

- Notes:**
 - Only sterile mulch should be used in areas where one species of grass is desired.
 - Mulching Seeded Areas - Mulch shall be applied to all seeded areas immediately after seeding.
 - If grading is completed outside of the seeding season returns and seeding can be performed in accordance with these specifications.
 - When straw mulch is used, it shall be spread over all seeded areas at the rate of 2 tons/acre. Mulch shall be applied to a uniform loose depth of between 1" and 2". Mulch applied shall achieve a uniform distribution and depth so that the soil surface is not exposed. If a mulch anchoring tool is to be used, the rate should be increased to 2.5 tons/acre.
 - Wood cellulose fiber used as a mulch shall be applied at a net dry weight of 1,500 lbs. per acre. The wood cellulose fiber shall be mixed with water and the mixture shall contain a maximum of 50 lbs. of wood cellulose fiber per 100 gallons of water.

- Securing Straw Mulch Which Anchoring** - Mulch anchoring shall be performed immediately following mulch application to minimize loss by wind or water. This may be done by one of the following methods listed by preference, depending upon size of area and erosion hazard:
 - A mulch anchoring system is a tractor drawn implement designed to punch and anchor mulch into the soil surface to a minimum of two (2) inches. This practice is most effective on large areas, but is limited to flatter slopes where equipment can operate safely. If used on sloping sides, this practice should be used on the contour if possible. The fibers in the mulch binder should be applied at a net dry weight of 100 lbs. per 100 gallons of water.
 - Wood cellulose fiber may be used for anchoring straw. The fibers in the binder shall be applied at a net dry weight of 100 lbs. per 100 gallons of water. The mixture shall contain a maximum of 50 pounds of wood cellulose fiber per 100 gallons of water.
 - Application of liquid binders should be heavier at the edges where wind catches much such as in valleys and the crest of the area. The binder should be applied after the mulch has been applied. Synthetic binders - such as Acrylic DLR (Ago-Tack), DCA-70 (Petrolon), Terra Tex 10, Terra Tack AK, or other approved quality binders as recommended by the manufacturer to anchor mulch.
 - Lightweight plastic netting may be staked over the mulch according to manufacturer's recommendations. Netting is usually available in rolls 4' to 12' wide and 300 to 3,000 feet long.

No.	Species	Application Rate (lb/ac)	Seeding Dates	Seeding Depths	Fertilizer Rate (lb/100-10)	Lime Rate
1	RYE	150	3/15 - 5/31 6/1 - 10/31	1" - 2"	600 lb/ac 05 lb/1000sq	2 tons/ac 1000 lb/1000sq
2	BARLEY OR RYE PLUS FOXTAIL MILLOT	150	6/1 - 7/31	1"		

No.	Species	Application Rate (lb/ac)	Seeding Dates	Seeding Depths	Fertilizer Rate (lb/100-20)	Lime Rate
1	TALL FESCUE (95%) KENTUCKY BLUEGRASS (5%)	125 15	3/15 - 6/1 6/1 - 10/1	1" - 2"	90 lb/ac 1000lb/1000sq	2 tons/ac 1000lb/1000sq
2	PERENNIAL RYEGRASS (50%) TALL FESCUE (50%) HARD FESCUE (20%)	150 15 15	3/15 - 6/1 6/1 - 10/1	1" - 2"	90 lb/ac 1000lb/1000sq	2 tons/ac 1000lb/1000sq

No.	Species	Application Rate (lb/ac)	Seeding Dates	Seeding Depths	Fertilizer Rate (lb/100-20)	Lime Rate
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No.	Species	Application Rate (lb/ac)	Seeding Dates	Seeding Depths	Fertilizer Rate (lb/100-20)	Lime Rate
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2	PERENNIAL RYEGRASS (50%) TALL FESCUE (50%) HARD FESCUE (20%)	150 15 15	3/15 - 6/1 6/1 - 10/1	1" - 2"	90 lb/ac 1000lb/1000sq	2 tons/ac 1000lb/1000sq

No.	Species	Application Rate (lb/ac)	Seeding Dates	Seeding Depths	Fertilizer Rate (lb/100-20)	Lime Rate
1	TALL FESCUE (95%) KENTUCKY BLUEGRASS (5%)	125 15	3/15 - 6/1 6/1 - 10/1	1" - 2"	90 lb/ac 1000lb/1000sq	2 tons/ac 1000lb/1000sq
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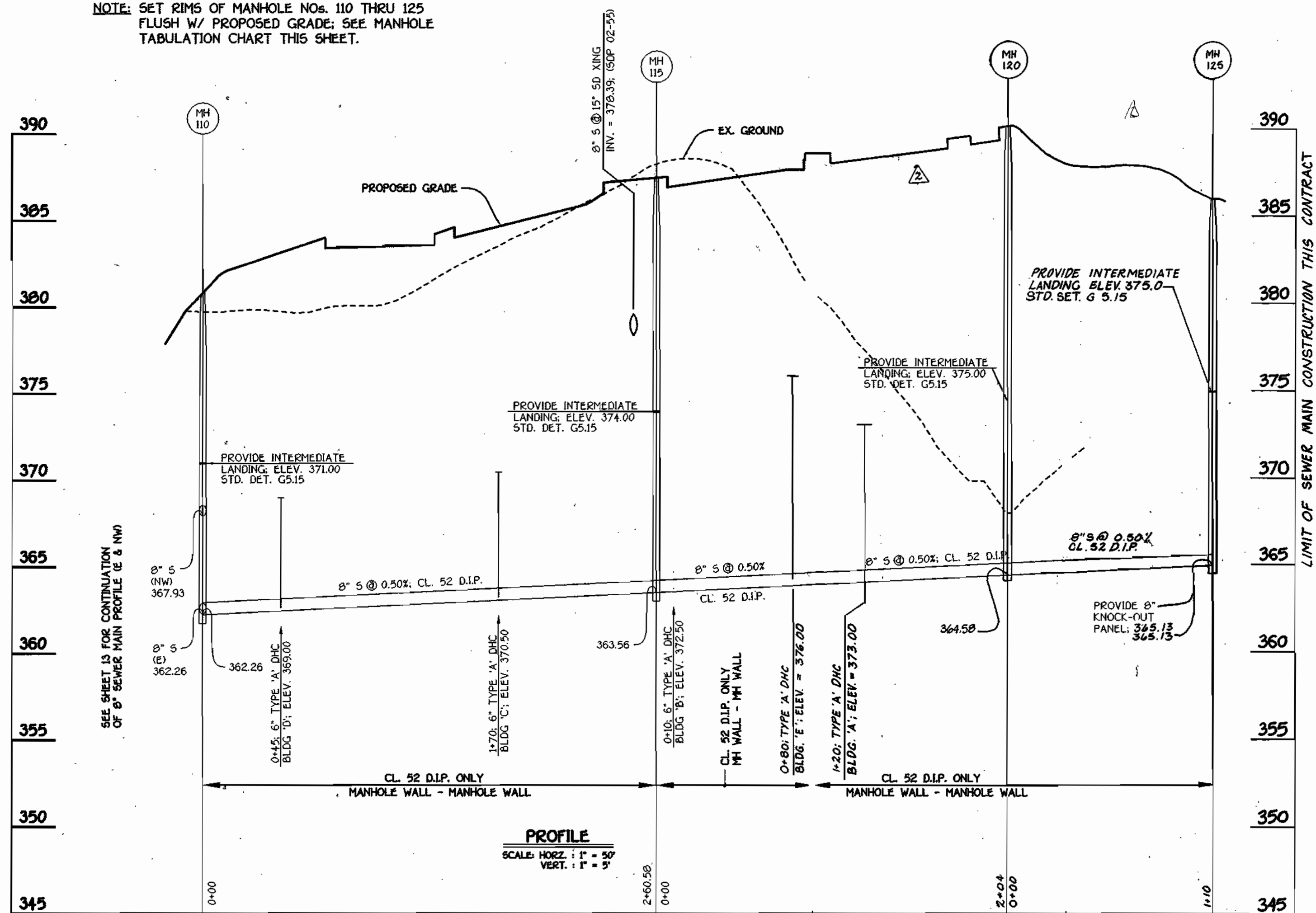
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NOTE: SET RIMS OF MANHOLE NOS. 110 THRU 125 FLUSH W/ PROPOSED GRADE; SEE MANHOLE TABULATION CHART THIS SHEET.



8" SEWER MAIN: BUILDINGS 'A' THRU 'E'

SHC INVERT @ PROPERTY LINE CHART		
STATION	UNIT	ELEVATION
MH 105 TO MH 106		
0+80 LT.	14	371.07
1+03 LT.	13	371.59
1+57 LT.	12	373.31
⊙ MH 106 LT.	11	373.41
MH 110 TO MH 111		
⊙ MH 111 RT.	10	371.51
MH 111 TO MH 112		
0+25 RT.	9 (DHC)	375.86
0+70 LT.	BUILDING 'G'	375.05
0+80 RT.	8	374.59
1+15 RT.	7	375.95
1+30 RT.	6 (DHC)	375.78
2+00 RT.	5	375.35
2+15 RT.	4 (DHC)	383.86
2+25 LT.	BUILDING 'F'	381.25
2+70 RT.	3	382.19
3+00 RT.	2 (DHC)	386.32
⊙ MH 112 RT.	1 (DHC) CONN. @ MH	387.78
MH 110 TO MH 115		
0+45 LT.	BUILDING 'D' (DHC)	369.22
1+70 LT.	BUILDING 'C' (DHC)	370.84
MH 115 TO MH 120		
0+10 LT.	BUILDING 'B' (DHC)	372.90
0+80 RT.	BUILDING 'E' (DHC)	377.16
1+20 LT.	BUILDING 'A' (DHC)	373.44

MANHOLE TABULATION CHART			
NO.	NORTHING	EASTING	RIM ELEVATION
100*	562498.90	1374247.95	387.17
105*	562608.36	1374193.89	376.05
106*	562780.34	1374196.17	383.59
110*	562809.48	1374108.99	380.60
111*	562739.86	1374042.23	384.50
112*	562744.05	1373726.26	387.00
115*	562406.77	1373945.25	387.50
120*	562408.95	1373741.27	589.50
125*	562294.74	1373728.08	386.50

* SET MH RIMS FLUSH W/ PROPOSED FINISHED GRADE

M.C.E. CHART	
UNIT	M.C.E.
1	391.88
2	390.42
3	386.29
4	387.96
5	383.45
6	383.88
7	380.05
8	378.89
9	379.96
10	375.61
11	377.51
12	377.41
13	375.69
14	375.17
BUILDING 'A'	379.30
BUILDING 'B'	378.10
BUILDING 'C'	378.04
BUILDING 'D'	374.42
BUILDING 'E'	383.38
BUILDING 'F'	382.70
BUILDING 'G'	380.45



DATE	DESCRIPTION
7/31/03	REVISED 8" SEWER MAIN PROFILE, MH 120 & MH 125, REVISED 8" MANHOLE TABULATION CHART.



ENGINEER'S CERTIFICATE
 I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.
 Signature of Engineer (print name below signature) *Charles J. Crovo, Sr.* Date *8/22/02*
 CHARLES J. CROVO, SR., P.E., L.S.

DEVELOPER'S CERTIFICATE
 I/We certify that all development and construction will be done according to this plan for sediment and erosion control and that all responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize periodic onsite inspection by the Howard Soil Conservation District.
 Signature of Developer (print name below signature) *Donald P. Reuser, Jr.* Date *8/22/02*
 DONALD P. REUSER, JR.

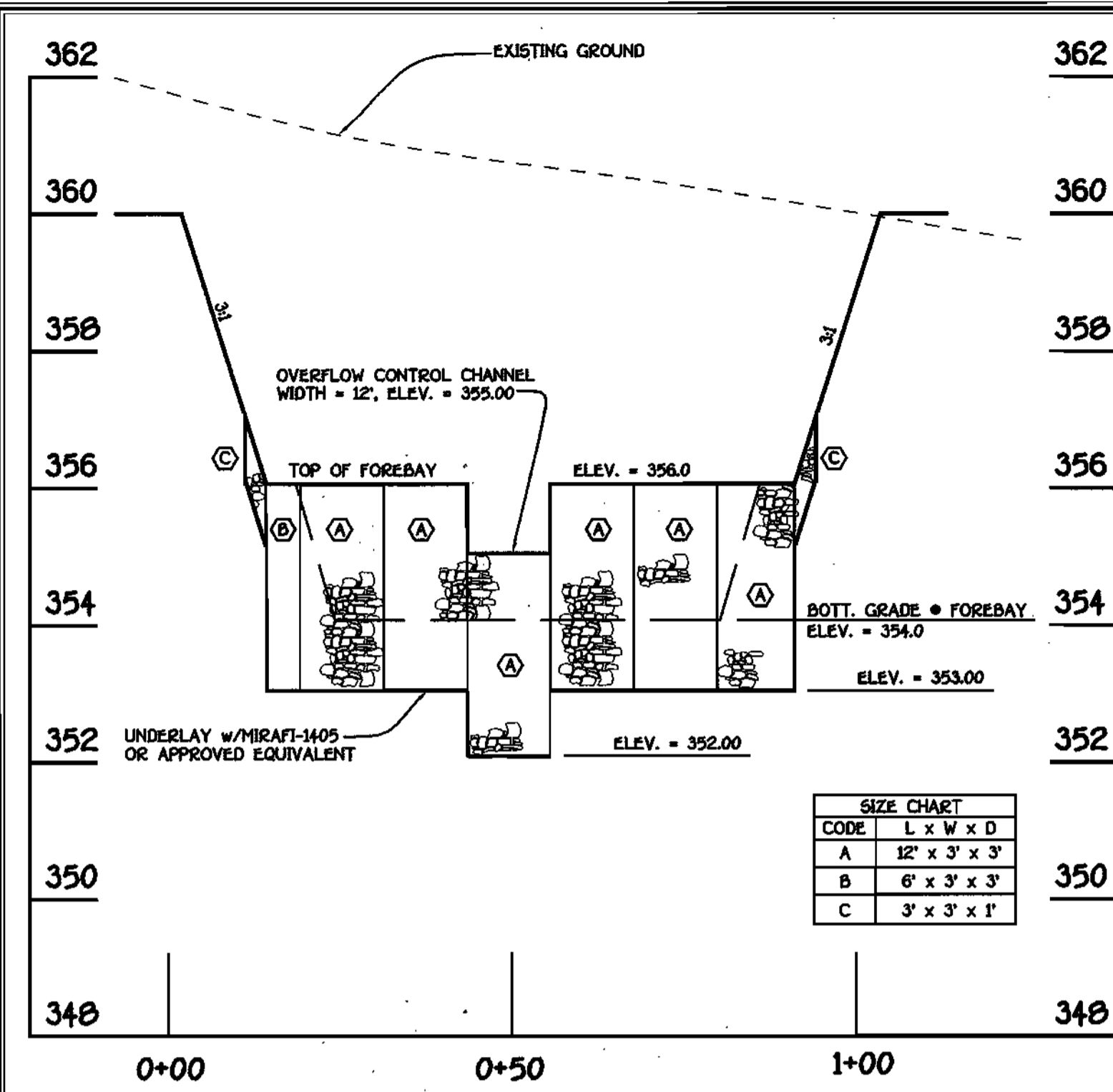
DEVELOPER/OWNER
 RSK, LLC
 c/o LAND DESIGN AND DEVELOPMENT, INC.
 8000 MAIN STREET
 ELICOTT CITY, MARYLAND 21043

BUILDER
 RYAN HOMES, INC.
 11460 CROWBRIDGE DRIVE
 SUITE 129
 OWINGS MILLS, MARYLAND 21117

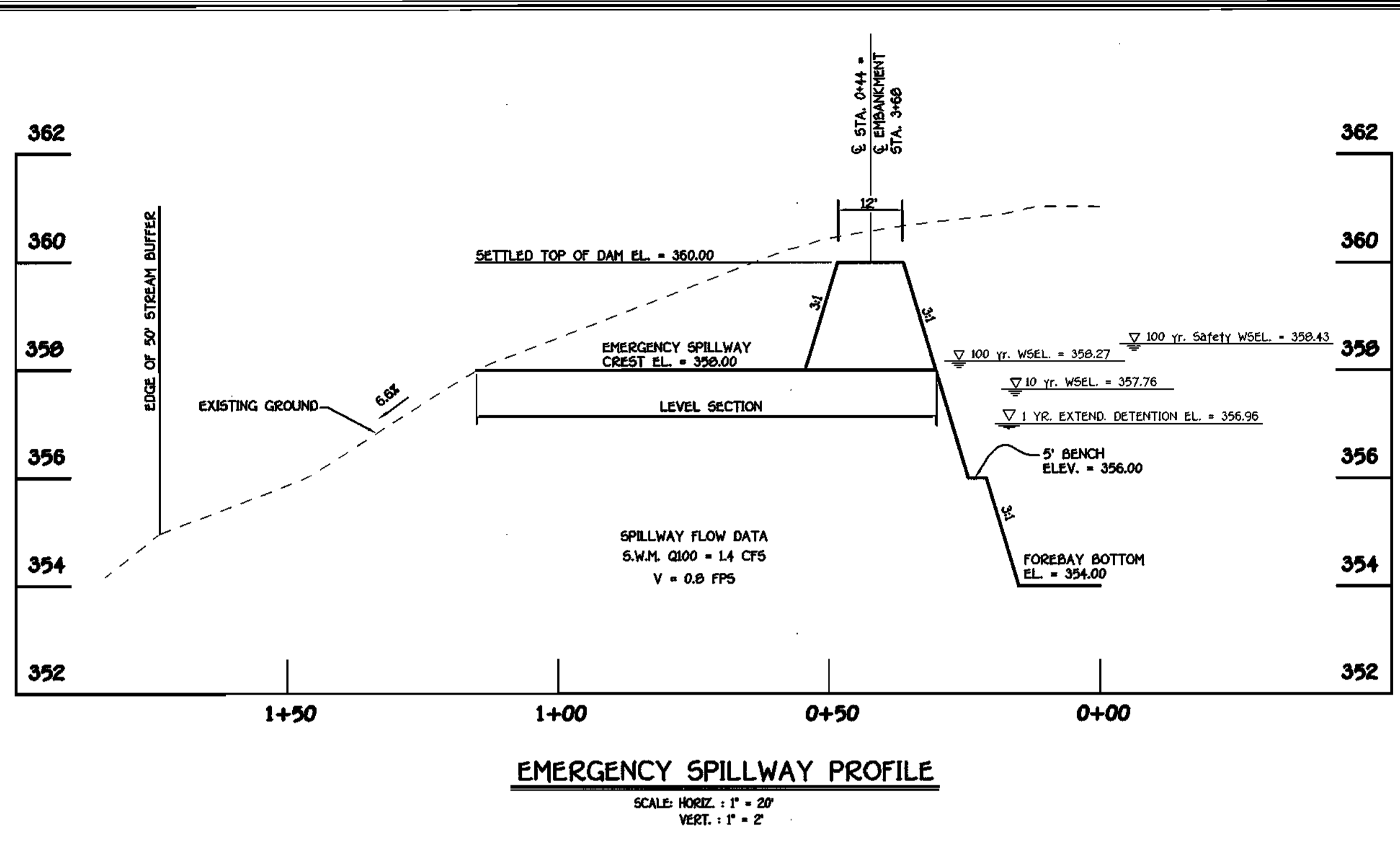
APPROVED: DEPARTMENT OF PLANNING AND ZONING
Joseph S. Smith Director - Department of Planning and Zoning Date *10/3/02*
Andy Hemata Chief, Division of Land Development Date *10/3/02*
William J. ... Chief, Development Engineering Division Date *9/20/02*

SUBDIVISION: THE COURTYARDS AT THE TIMBERS
 SECTION/AREA: 617
 PARCEL NO.: 617
 DEED REF.: 5609 / 611
 BLOCK NO.: 3
 ZONE: POR
 TAX/ZONE: 37
 ELEC. DIST.: 1st
 CENSUS TR.: 6030
 WATER CODE: D 04
 SEWER CODE: 2810000

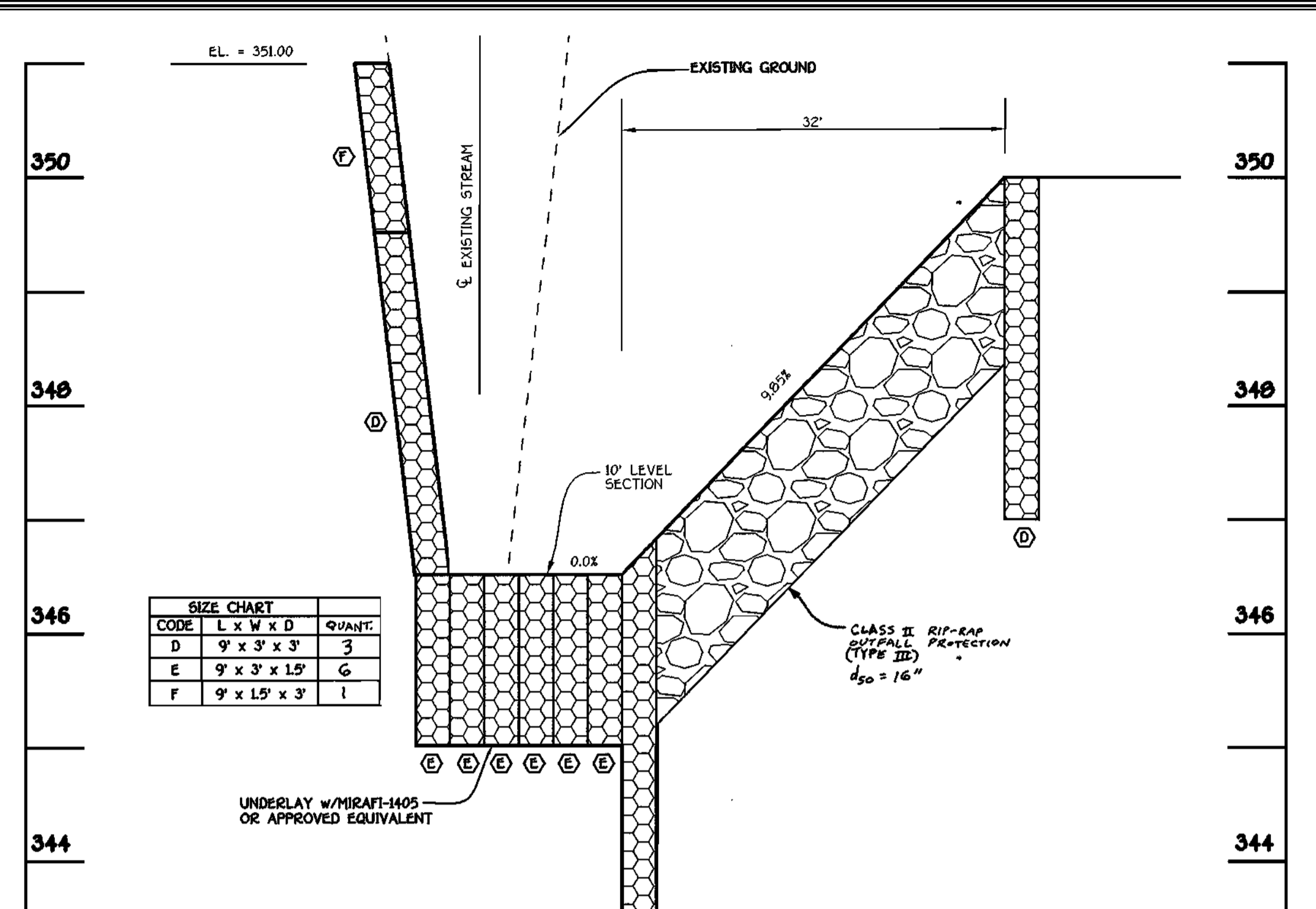
PRIVATE SEWER MAIN PROFILES
THE COURTYARDS AT THE TIMBERS
 TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS
 ZONED: POR
 TAX MAP No.: 37 PARCEL No.: 617 GRID No.: 3
 FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND
 SCALE: AS SHOWN DATE: AUGUST 21, 2002
 SHEET 14 OF 24 **SDP 02-55**



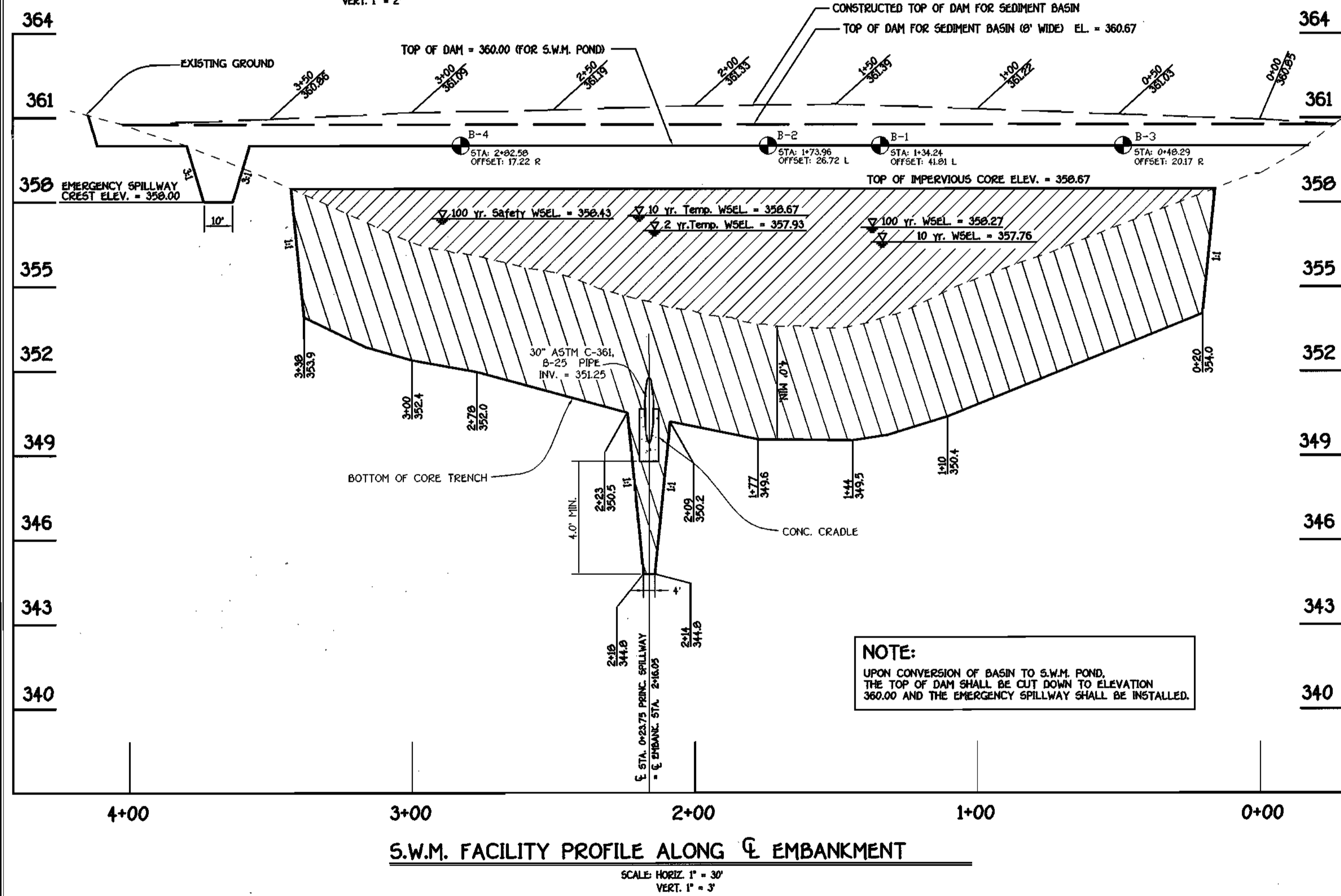
BMP FACILITY NO. 1 - GABION FOREBAY PROFILE
SCALE: HORIZ. 1" = 20'
VERT. 1" = 2'



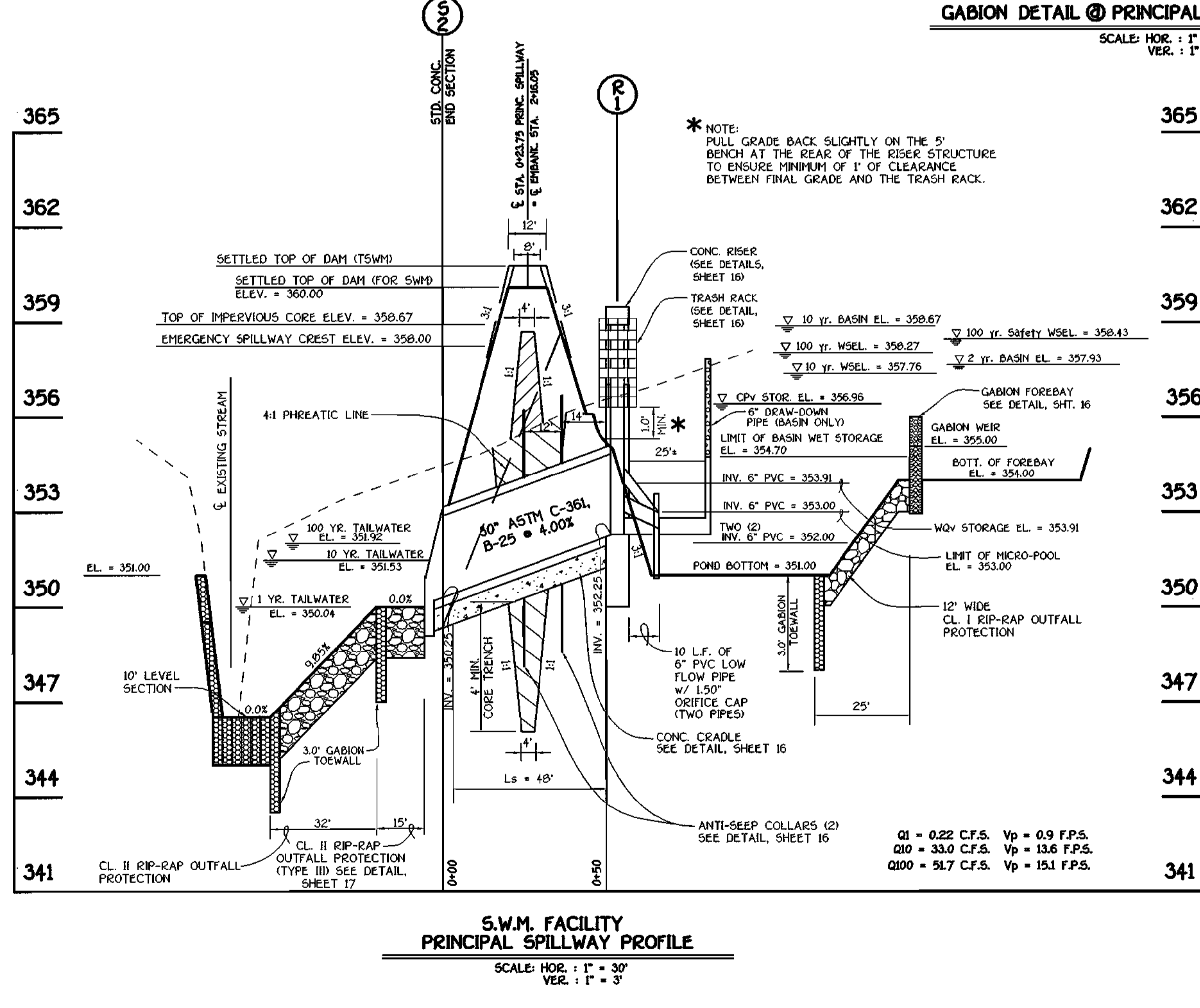
EMERGENCY SPILLWAY PROFILE
SCALE: HORIZ. 1" = 20'
VERT. 1" = 2'



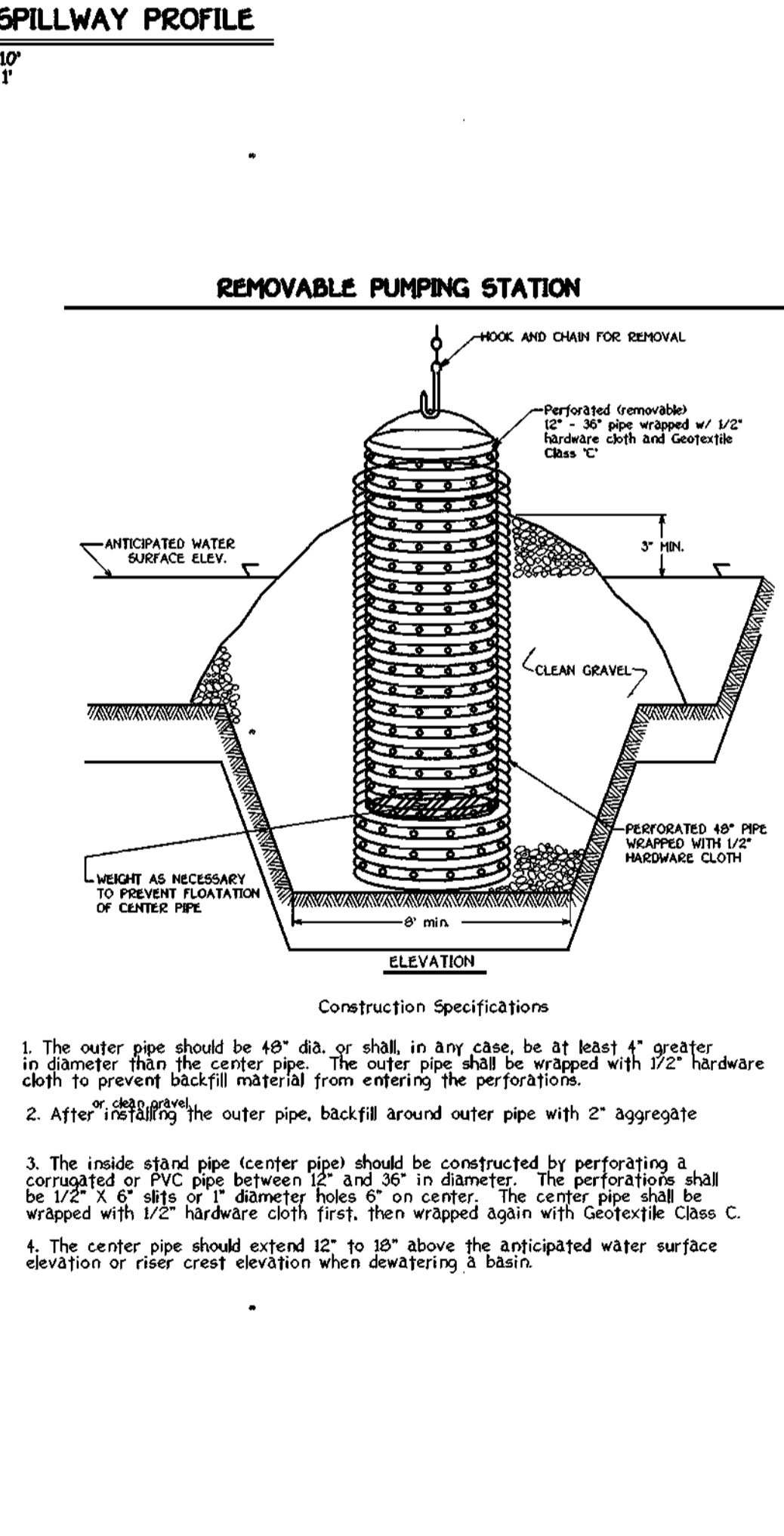
GABION DETAIL @ PRINCIPAL SPILLWAY PROFILE
SCALE: HOR. 1" = 10'
VERT. 1" = 1'



S.W.M. FACILITY PROFILE ALONG EMBANKMENT
SCALE: HORIZ. 1" = 30'
VERT. 1" = 3'



S.W.M. FACILITY PRINCIPAL SPILLWAY PROFILE
SCALE: HOR. 1" = 30'
VERT. 1" = 3'



REMOVABLE PUMPING STATION
Construction Specifications

FISHER, COLLINS & CARTER, INC.
CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
CENTRAL SQUARE OFFICE PARK - 10272 BALTIMORE NATIONAL PIKE
ELLICOTT CITY, MARYLAND 21042
4100 441 - 2395

DATE	DESCRIPTION	REVISION BLOCK



ENGINEER'S CERTIFICATE
I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.

CHARLES J. CROVO, SR., P.E., L.S. 8/2/02
Signature of Engineer (print name below signature) Date

DEVELOPER'S CERTIFICATE
I/We certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize periodic on-site inspection by the Howard Soil Conservation District.

DONALD R. REUMER, JR. 8/2/02
Signature of Developer (print name below signature) Date

Reviewed for HOWARD SCD and meets Technical Requirements.

U.S.D.A.-Natural Resources Conservation Service
This development plan is approved for soil erosion and sediment control by the HOWARD SOIL CONSERVATION DISTRICT.

HOWARD SCD

DEVELOPER/OWNER
288K, L.L.C.
c/o LAND DESIGN AND DEVELOPMENT, INC.
8000 MAIN STREET
ELLICOTT CITY, MARYLAND 21043

BUILDER
RYAN HOMES
11460 CROWBRIDGE DRIVE
SUITE 129
OWINGS HILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Chief, Department of Planning and Zoning
Chief, Division of Land Development
Chief, Development Engineering Division

SUBDIVISION	SECTION/AREA	PARCEL NO.
THE COURTYARDS AT THE TIMBERS		617

DEED REF.	BLOCK NO.	ZONE	TAX/ZONE	ELEC. DIST.	CENSUS TR.
5609 / 611	3	POR	37	1st	6030

WATER CODE: D 04
SEWER CODE: 2610000

STORMWATER MANAGEMENT NOTES AND DETAILS

THE COURTYARDS AT THE TIMBERS
TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

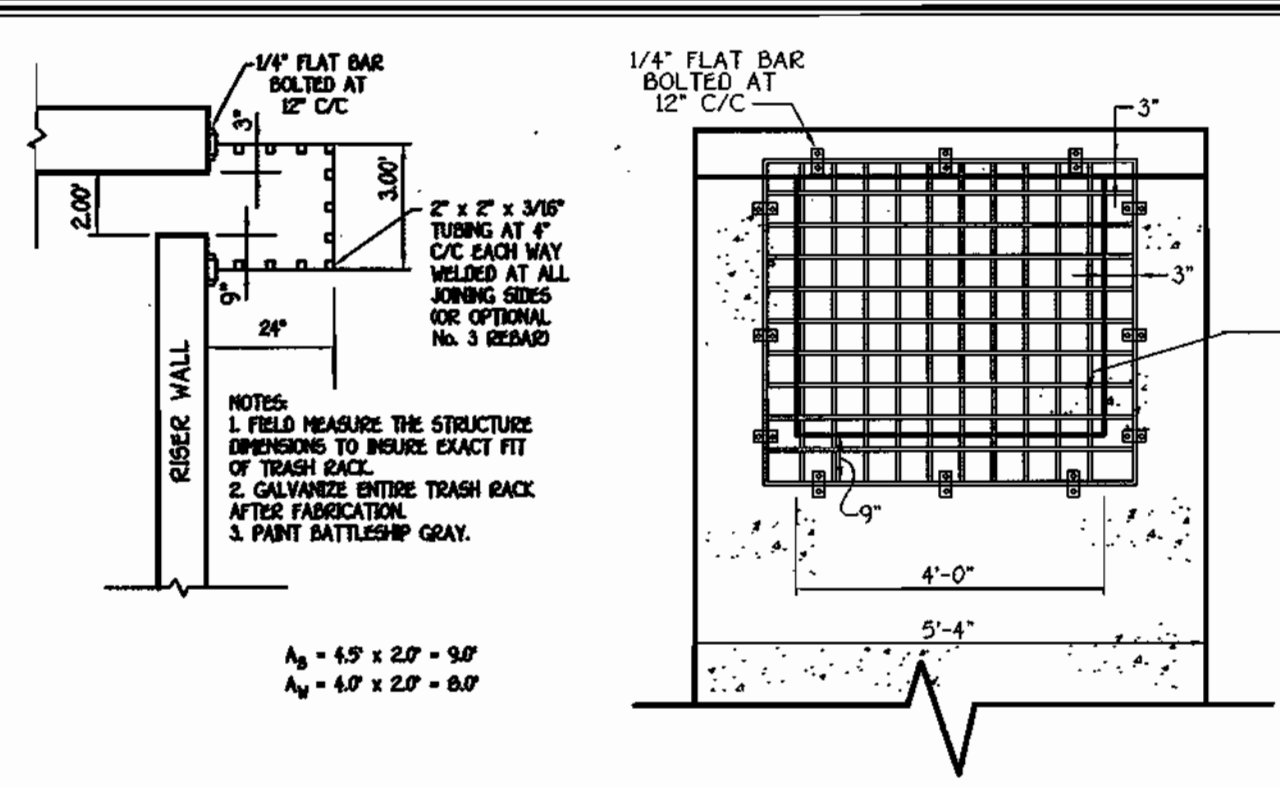
ZONED: POR

TAX MAP NO.: 37
FIRST ELECTION DISTRICT: HOWARD COUNTY, MARYLAND

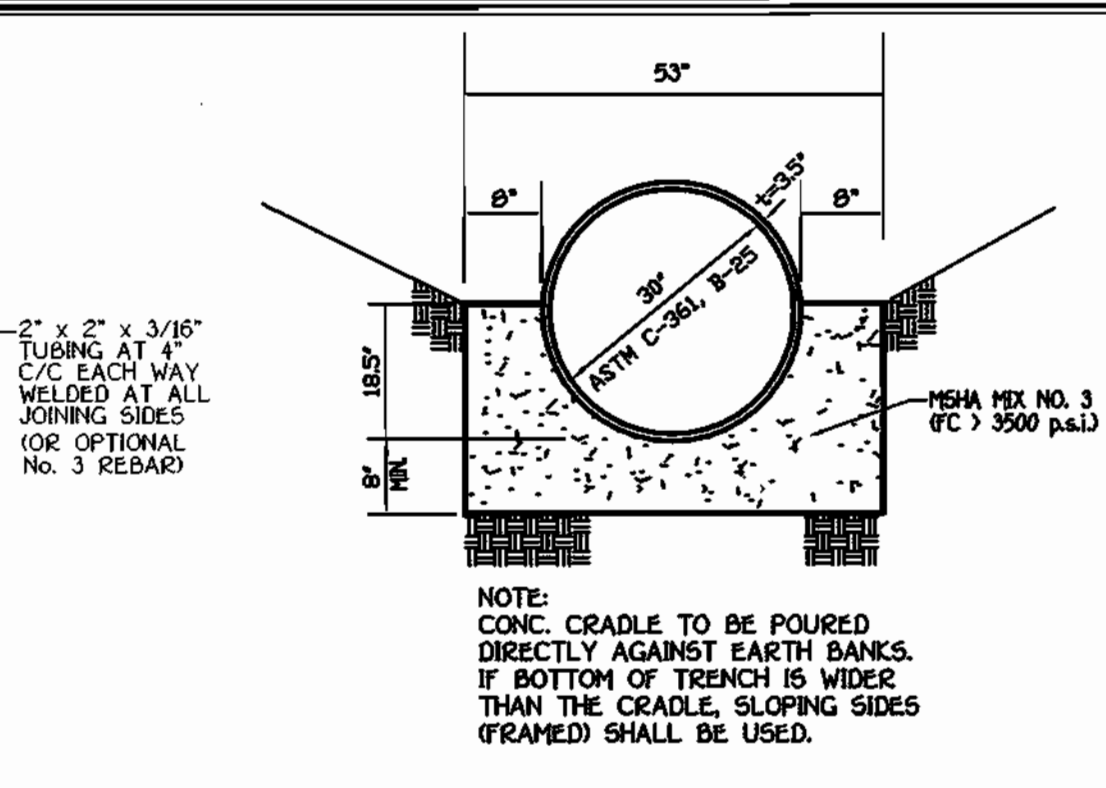
PARCEL NO.: 617
GRID NO.: 3

SCALE: AS SHOWN
DATE: AUGUST 21, 2002

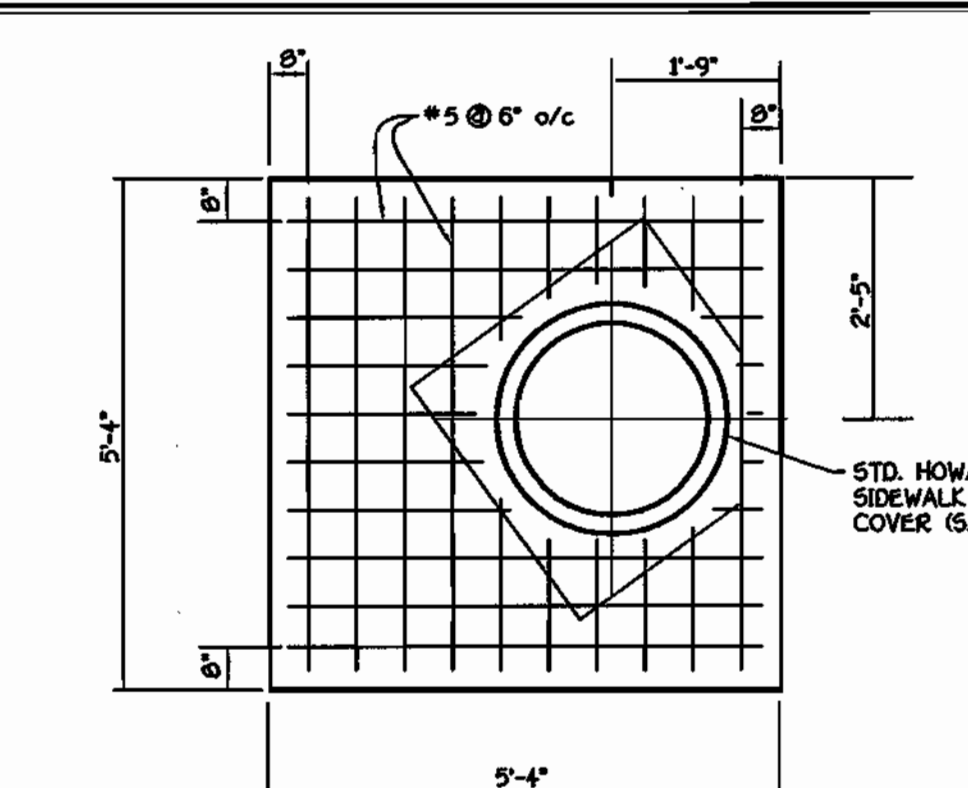
SHEET 15 OF 24
SDP 02-55



TRASH RACK 'A' DETAIL
NO SCALE

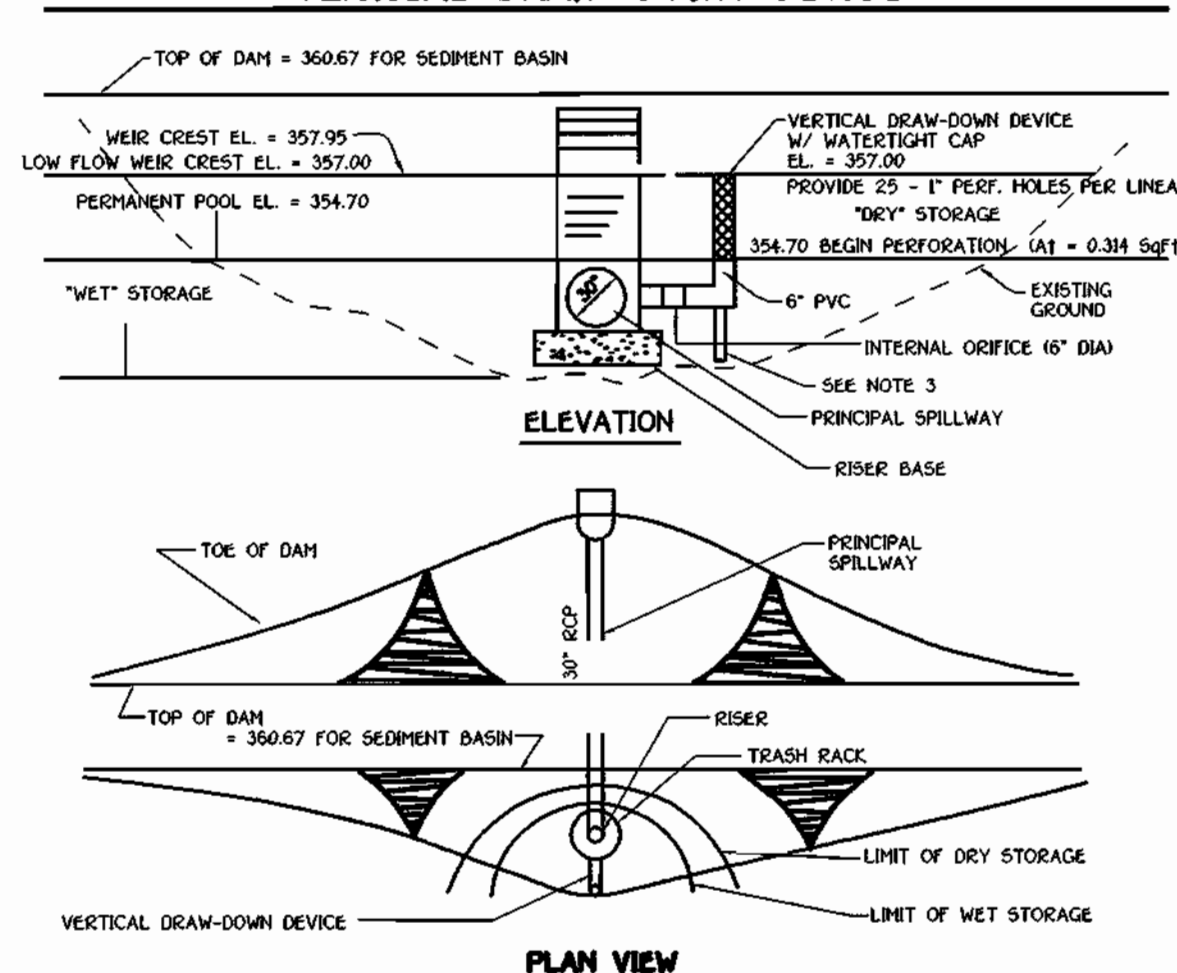


A2 CRADLE
NOT TO SCALE



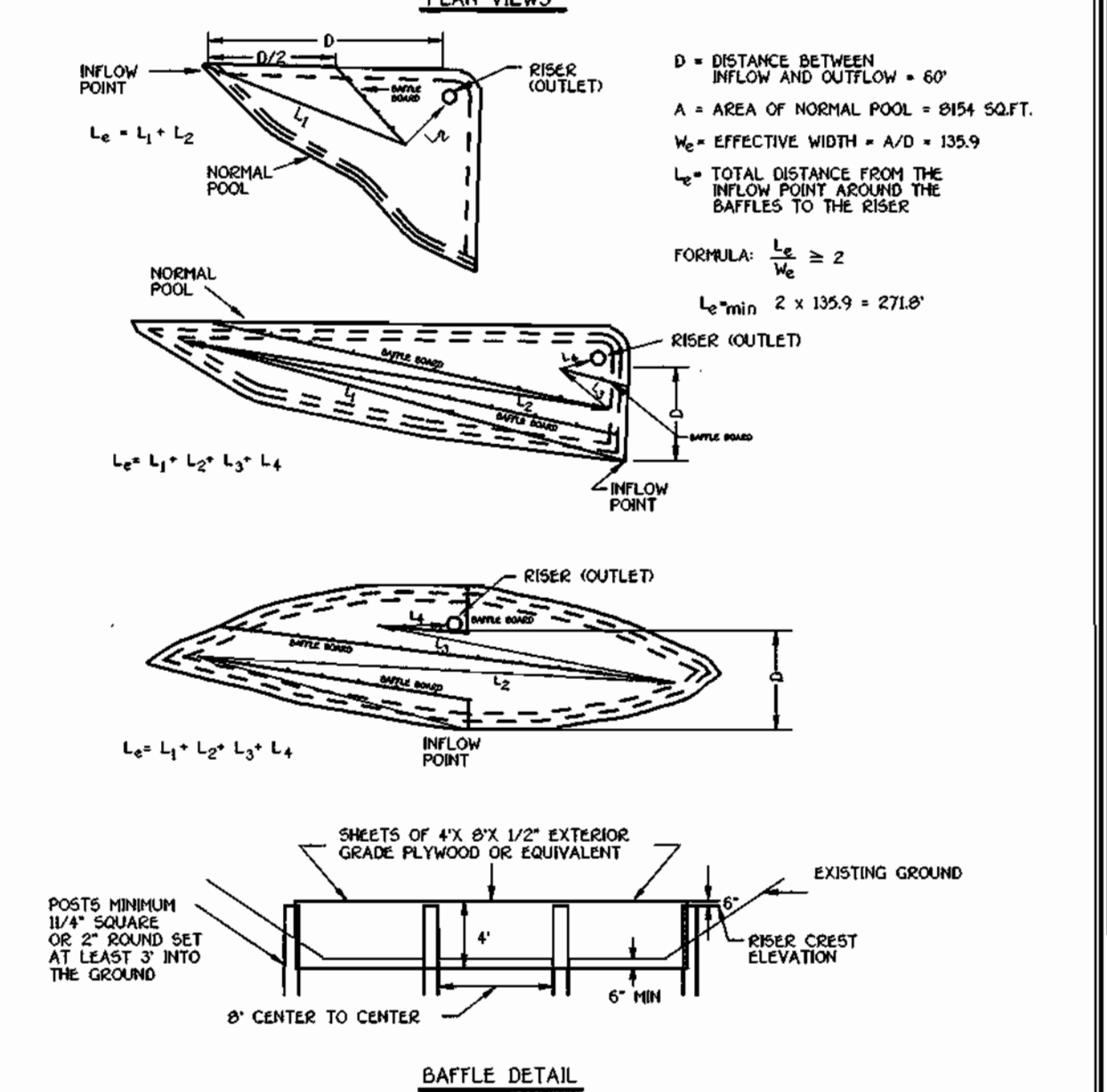
TOP SLAB DETAIL

**S.W.M./SEDIMENT BASIN
VERTICAL DRAW-DOWN DEVICE**



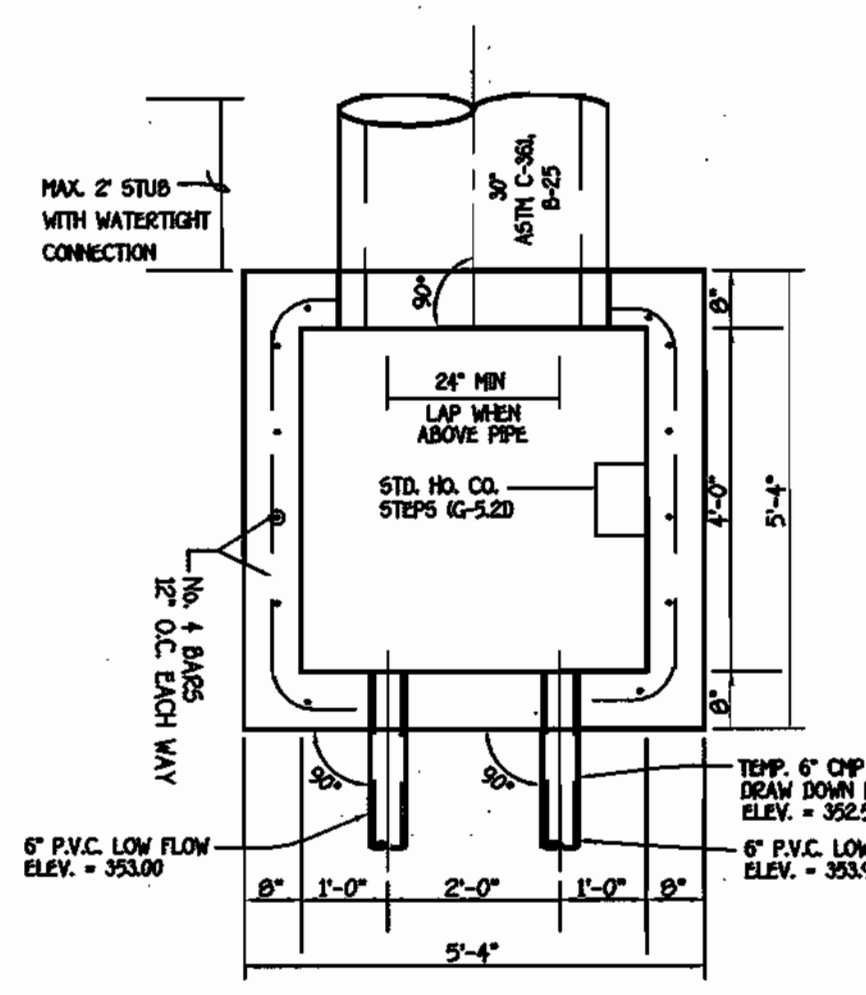
- CONSTRUCTION SPECIFICATIONS**
- PERFORATIONS IN THE DRAW-DOWN DEVICE MAY NOT EXTEND INTO THE WET STORAGE.
 - THE TOTAL AREA OF THE PERFORATIONS MUST BE GREATER THAN 2 TIMES THE AREA OF THE INTERNAL ORIFICE.
 - THE PERFORATED PORTION OF THE DRAW-DOWN DEVICE SHALL BE WRAPPED WITH 1/2\"/>

SEDIMENT BASIN Baffles

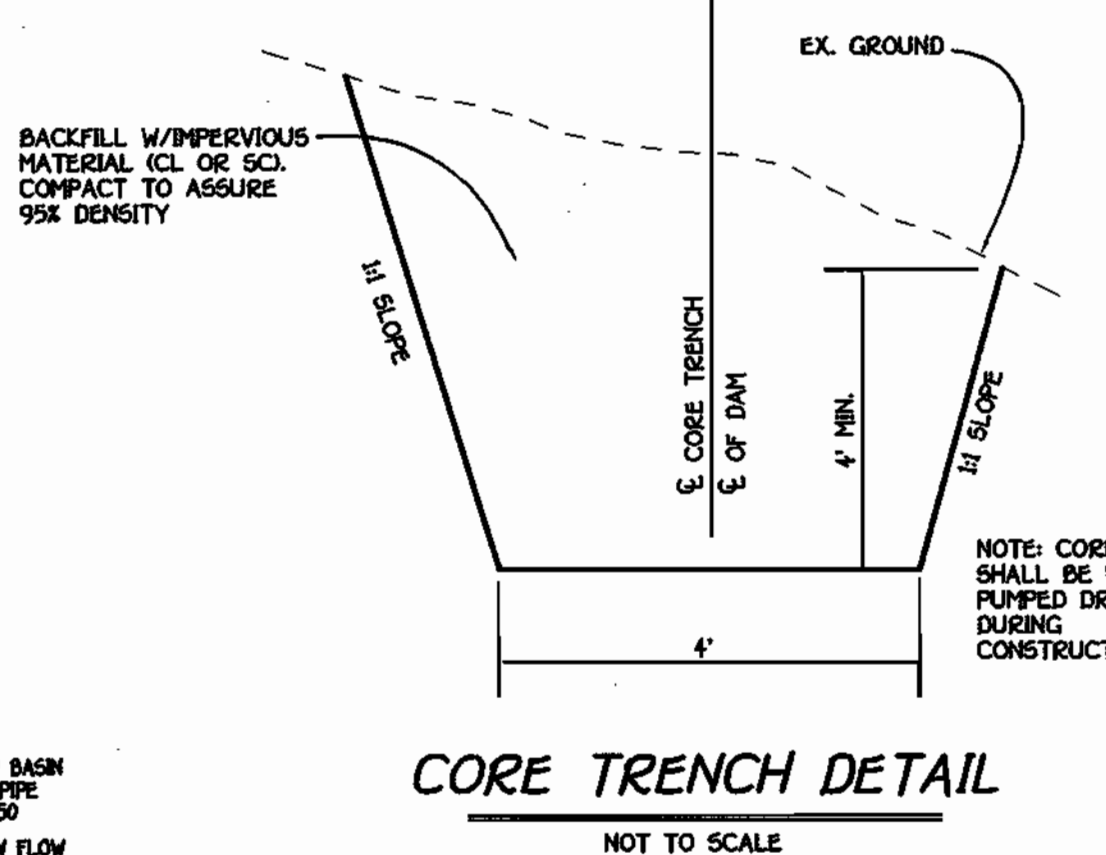


BAFFLE COMPUTATIONS

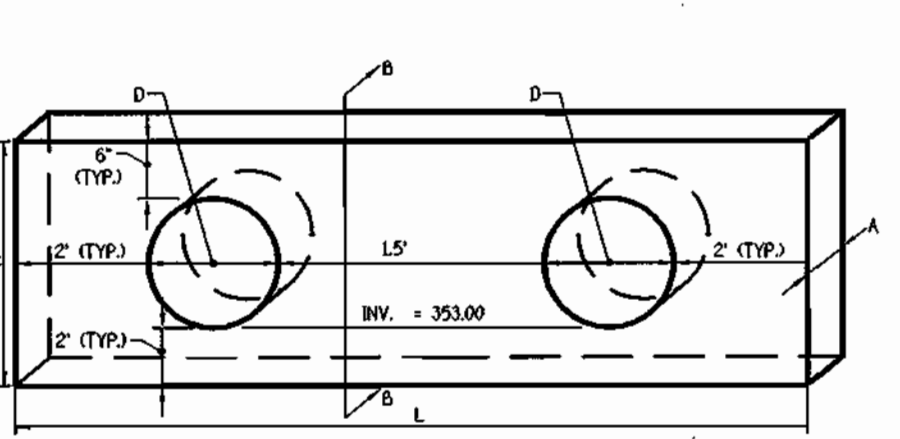
D = 145 FT.
A = 16,600 SQ. FT. • ELEV. 354.70
We = A/D = 16,600 SQ. FT. / 145 FT. = 114.5 FT.
Le REQUIRED = We x 2 = 114.5 FT. x 2 = 229 FT.
Le PROVIDED = 75' + 65' + 65' + 35' = 240 FT.



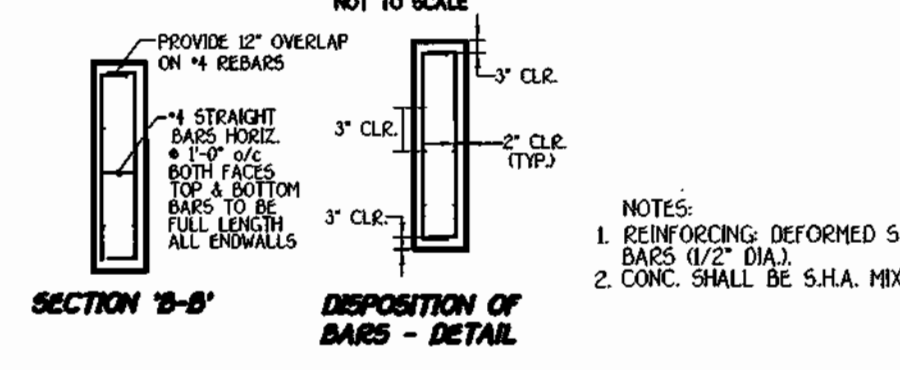
RISER PLAN VIEW
NO SCALE



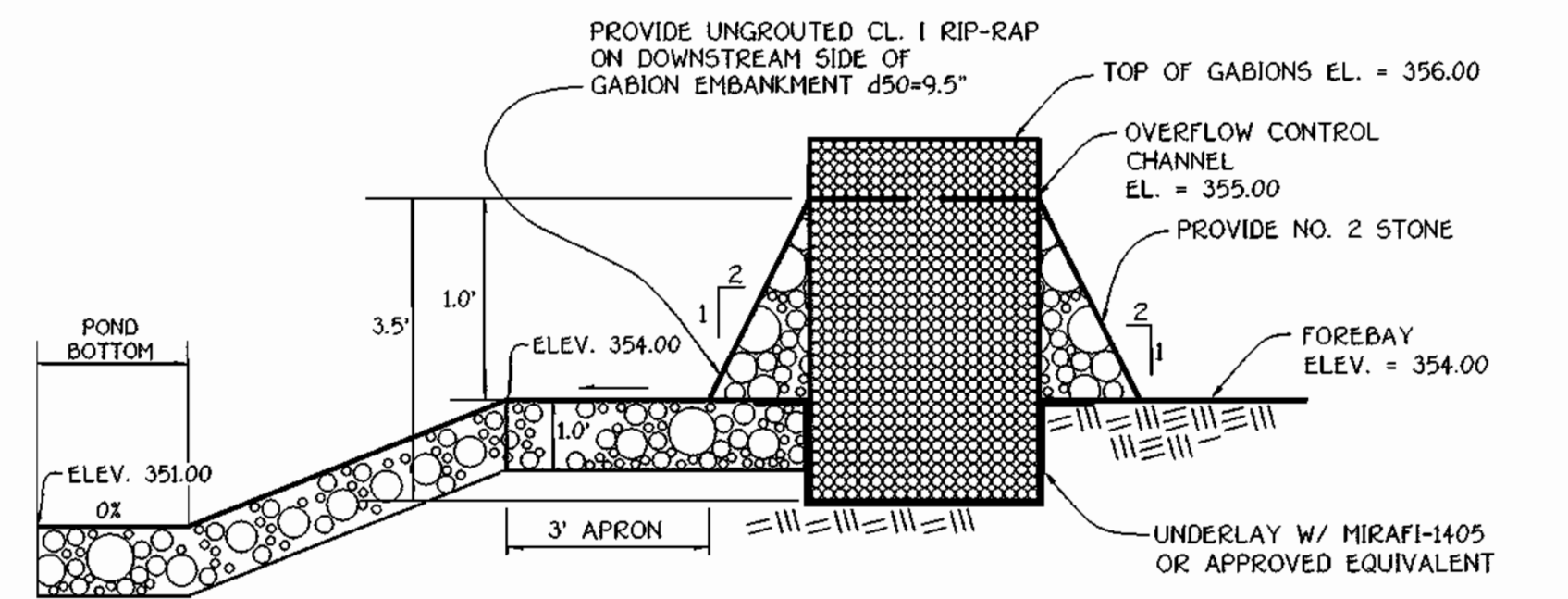
CORE TRENCH DETAIL
NOT TO SCALE



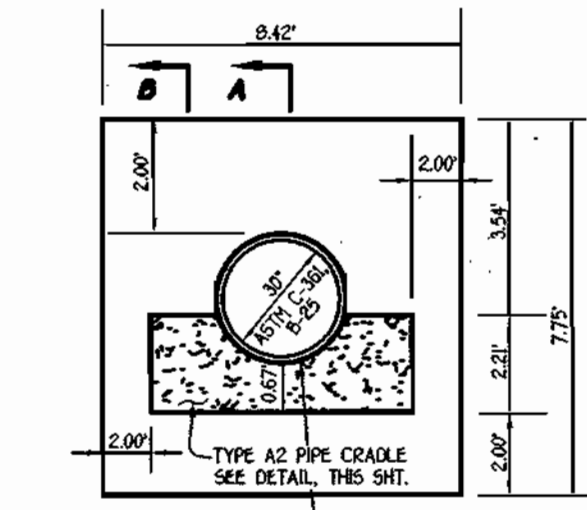
CONCRETE HEADWALL DETAIL
NOT TO SCALE



ANTI-SEEP COLLAR
NOT TO SCALE



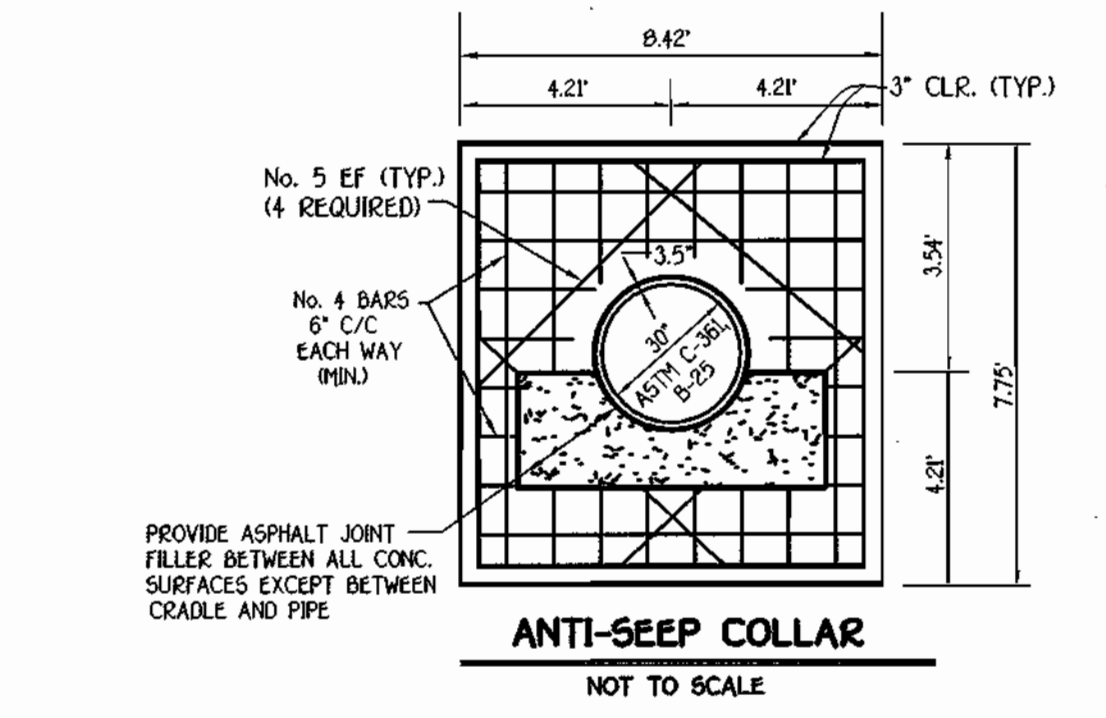
GABION FILTER AT FOREBAY
NO SCALE



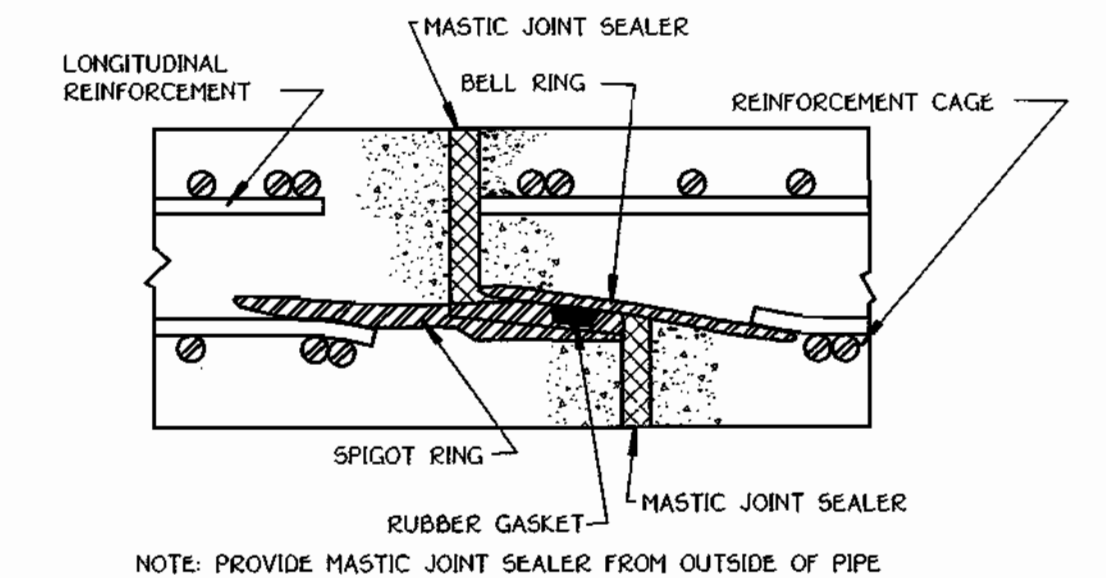
TYPICAL SECTION THROUGH BARREL, CRADLE AND ANTI-SEEP COLLAR
NO SCALE

- NOTES:**
- CONCRETE SHALL BE MESA MIX NO. 3 (FC = 3500 P.S.I.)
 - REINFORCING STEEL GRADE 60
 - FOR WALLS OF STRUCTURE SHALL UTILIZE L#1 SCORLEID CO. T-9955 FISH LINES RANDOM SPLIT-FACE ROCKS OPTIONAL
 - PROVIDE ROUGH BROWN FINISH
 - ANCHOR BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH SECTION 6.07.03.59 OF THE MESA STANDARDS AND SPECS.
 - ALL REINFORCING SPLICES SHALL BE LAP SPLICES OF 30 BAR DIA. UNLESS OTHERWISE SHOWN.

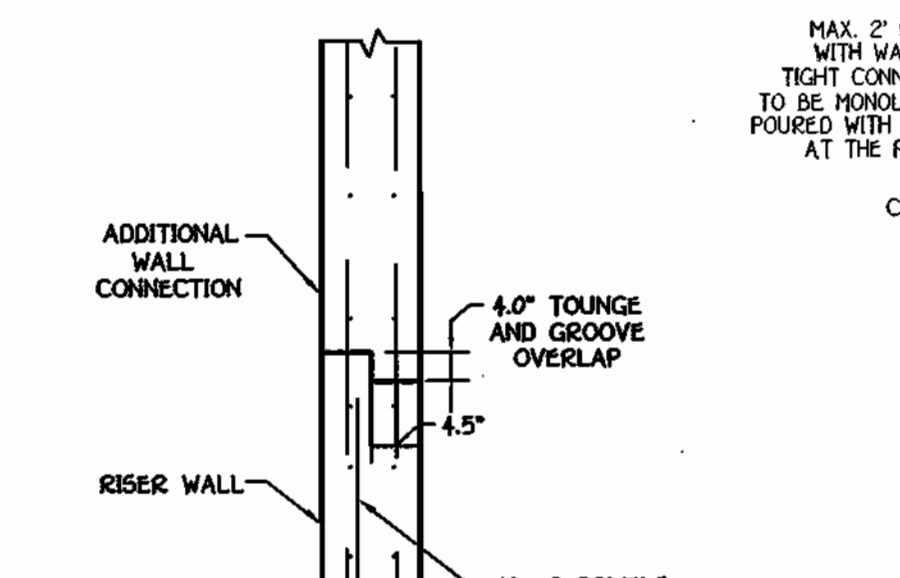
TYPICAL BOLLARD DETAIL
NOT TO SCALE



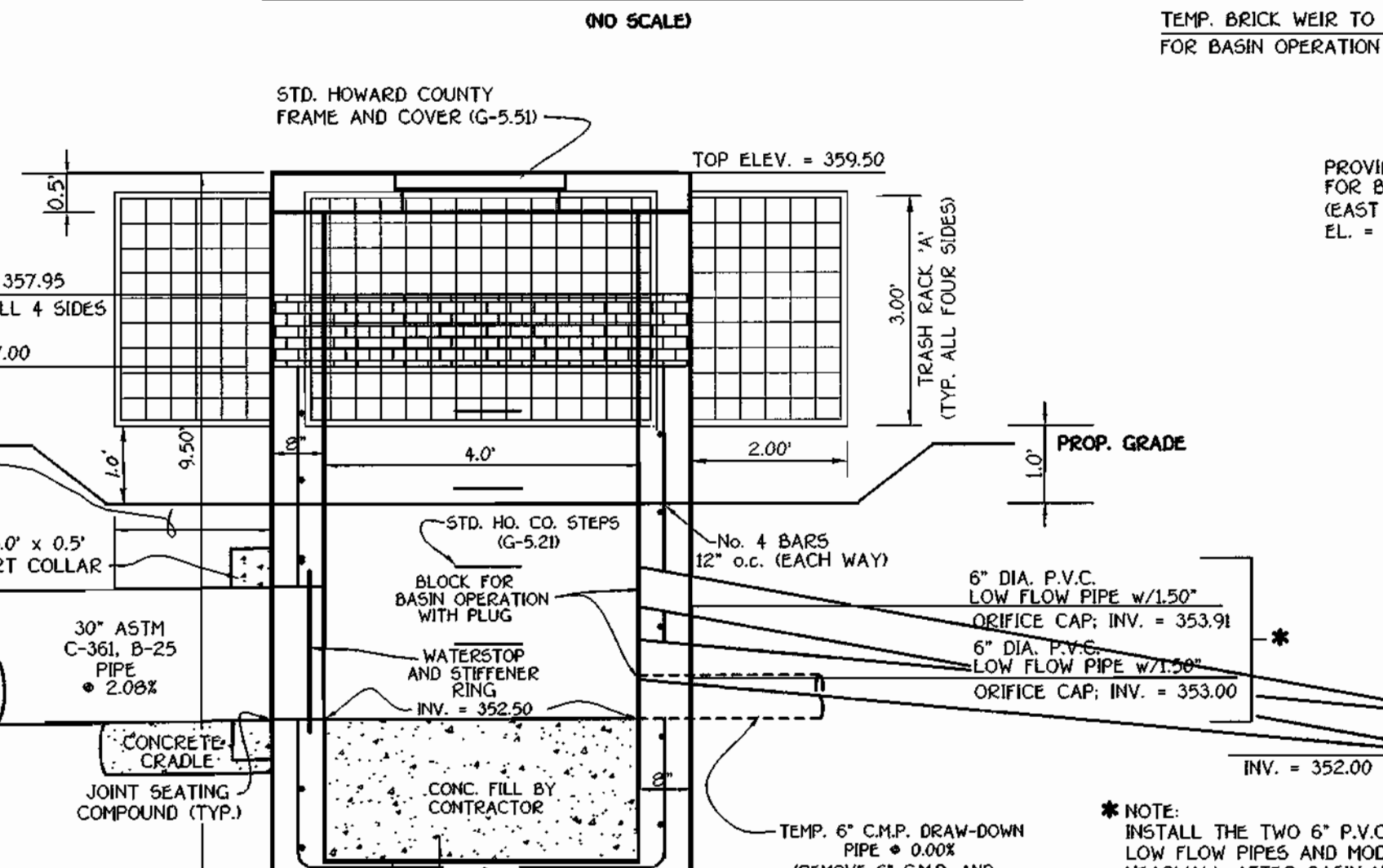
ANTI-SEEP COLLAR
NOT TO SCALE



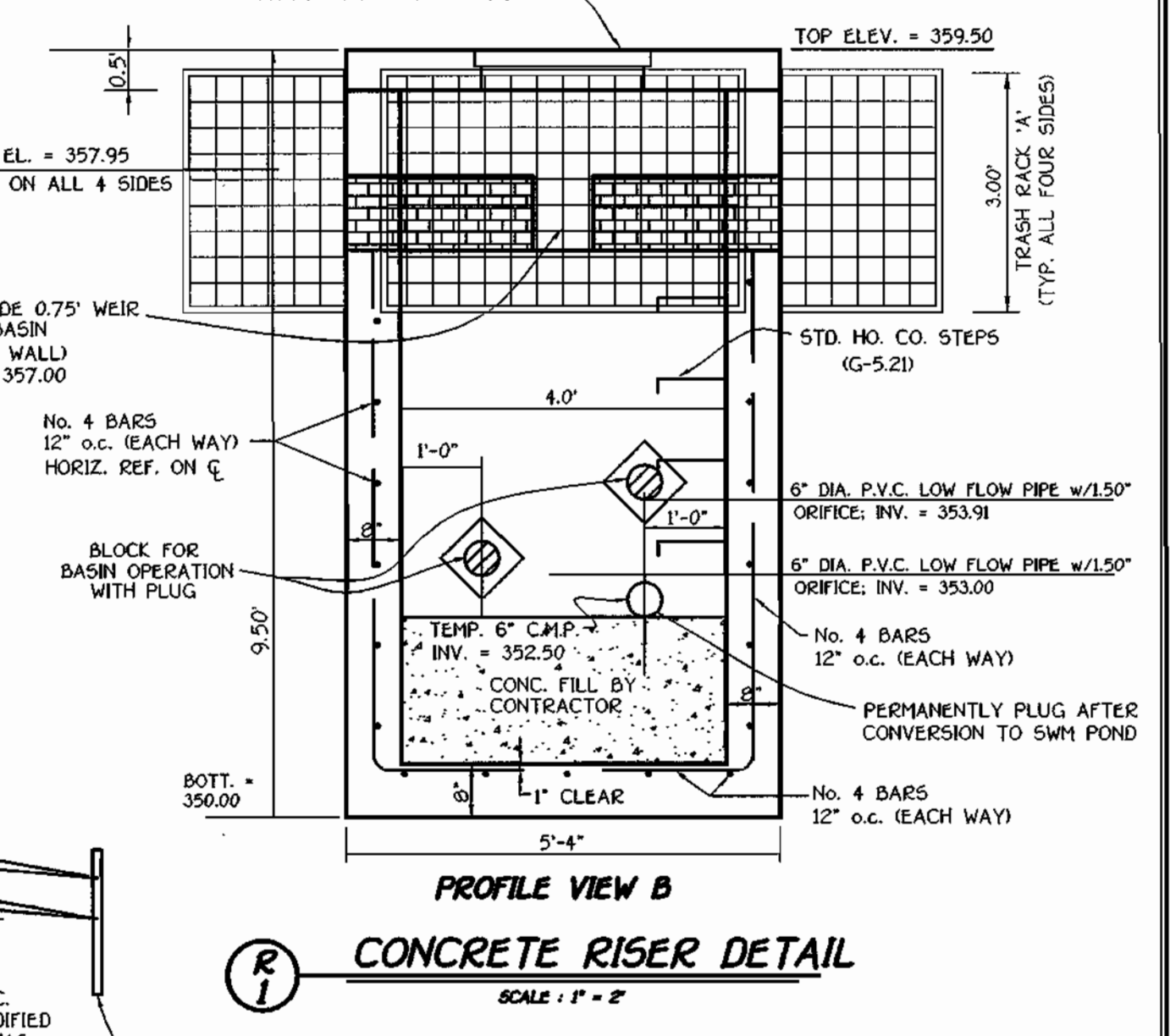
CONCRETE PIPE JOINT DETAIL
NO SCALE



**KEYED JOINT DETAIL
WALL SECTION TO WALL SECTION**
NO SCALE



CONCRETE RISER DETAIL
SCALE: 1" = 2"



CONCRETE RISER DETAIL
SCALE: 1" = 2"

DATE	DESCRIPTION

ENGINEER'S CERTIFICATE

I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.

CHARLES J. CROVO, SR., P.E., L.S. 8/28/02
Signature of Engineer (print name below signature) Date

DEVELOPER'S CERTIFICATE

I/We certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize persons on site for inspection by the Howard Soil Conservation District.

DONALD R. REUWER, JR. 8/28/02
Signature of Developer (print name below signature) Date

Reviewed for HOWARD SCD and meets Technical Requirements.

J. M. ... 9/14/02
U.S.D.A. Natural Resources Conservation Service Date

This development plan is approved for soil erosion and sediment control by the HOWARD SOIL CONSERVATION DISTRICT.

Howard SCD 9/16/02
Date

DEVELOPER/OWNER
RSEK, L.L.C.
c/o LAND DESIGN AND DEVELOPMENT, INC.
8000 MAIN STREET
ELLICOTT CITY, MARYLAND 21043

BUILDER
RYAN HOMES
11460 KENNEDY DRIVE
SUITE 120
OWINGS MILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Director: Department of Planning and Zoning 10/3/02
Date

Chief, Division of Land Development 10/3/02
Date

Chief, Development Engineering Division 9/20/02
Date

SUBDIVISION	SECTION/AREA	PARCEL NO.
THE COURTYARDS AT THE TIMBERS		617
DEED REF. 5609 / 611	BLOCK NO. 3	ZONE POR
TAX/ZONE 37	ELEC. DIST. 1st.	CENSUS TR. 6030
WATER CODE D 04	SEWER CODE	2610000

STORMWATER MANAGEMENT NOTES AND DETAILS

THE COURTYARDS AT THE TIMBERS

TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

ZONED: POR

TAX MAP No.: 37 PARCEL No.: 617 GRID No.: 3

FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND

SCALE: AS SHOWN DATE: AUGUST 21, 2002

SHEET 16 OF 24 **SDP 02-55**

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STORM WATER MANAGEMENT POND CONSTRUCTION SPECIFICATIONS

These specifications are appropriate to all ponds within the scope of the Standard for Practice MD-376. All references to ASTM and AASHTO specifications apply to the most recent version.

Site Preparation

Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the toe of the embankment.

Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet structure shall be cleared.

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

EARTH FILL

Material - The fill material shall be taken from approved designated borrow areas. It shall be free of roots, stumps, wood, rubbish, stones greater than 6", frozen or other objectionable materials. Fill material for the center of the embankment and cut-off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer. Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment.

Placement - Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 9-inch thick before compaction layers which will be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment.

Compaction - The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not be so wet that water can be squeezed out.

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within +2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

Cut Off Trench - The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation with the minimum width being four feet. The depth shall be at least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability.

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the core shall be a minimum of four feet. The height shall extend up to at least 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed concurrently with the outer shell of the embankment.

Structure Backfill

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi, 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2,000 ohm-cm. Material shall be placed such that a minimum of 6" measured perpendicular to the outside of the pipe of flowable fill shall be under (bedding), over and, on the sides of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24" or greater over the structure or pipe. Backfill material outside the structural backfill (flowable fill zone shall be of the type and quality conforming to the specified for the core of the embankment or other embankment materials.

Pipe Conduits

All pipes shall be circular in cross section.

Corrugated Metal Pipe - All of the following criteria shall apply for corrugated metal pipe:

1. **Materials** - (Polymer Coated Steel Pipe) - Steel pipes with polymeric coatings shall have a minimum coating thickness of 0.01 inch (0.2 mil) on both sides of the pipe. This pipe and its appearance shall conform to the requirements of AASHTO Specifications M-245 & M-246 with watertight coupling bands or flanges.

Materials - (Aluminum Coated Steel Pipe) - This pipe and its appearance shall conform to the requirements of AASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts to be used for connections. The pH of the surrounding soils shall be between 4 and 9.

Materials - (Aluminum Pipe) - This pipe and its appearance shall conform to the requirements of AASHTO Specification M-190 Type A with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts to be used for connections. The pH of the surrounding soils shall be between 4 and 9.

2. **Coupling bands, anti-seep collars, end sections, etc.**, must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at least 24 mils in thickness.

3. **Connections** - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are aluminum. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.

All connections shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches in diameter: flanges on both ends of the pipe with a circular 3/8 inch closed cell neoprene gasket, prepared to the flange bolt circle, sandwiched between adjacent flanges; a 12-inch wide standard lap type band with 12-inch wide by 3/8-inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2-inch greater than the corrugation depth. Pipes 24-inches in diameter and larger shall be connected by a 24-inch long annular corrugated band using a minimum of 4 (four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12-inches in the end of each pipe. Flanged joints with 3/8-inch closed cell gaskets the full width of the flange is also acceptable.

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or a neoprene band.

4. **Bedding** - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

5. **Backfilling** shall conform to "Structure Backfill".

6. **Other details** (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete pipe:

1. **Materials** - Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C-303.

2. **Bedding** - Reinforced concrete pipe conduits shall be laid in a concrete bedding/grade for their entire length. This bedding/grade shall consist of high slump concrete placed under the pipe and on the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete grade is not needed for structural reasons, flowable fill may be used as described in the "Structure Backfill" section of this standard. Gravel bedding is not permitted.

3. **Laying pipe** - Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

4. **Backfilling** shall conform to "Structure Backfill".

5. **Other details** (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Plastic Pipe

The following criteria shall apply for plastic pipe:

1. **Materials** - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" - 10" inch pipe shall meet the requirement of AASHTO M252 Type 5, and 12" through 24" inch shall meet the requirement of AASHTO M294 Type 5.

2. **Joints and connections** to anti-seep collars shall be completely watertight.

3. **Bedding** - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

4. **Backfilling** shall conform to "Structure Backfill".

5. **Other details** (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Drainage Diaphragms - When a drainage diaphragm is used, a registered professional engineer will supervise the design and construction inspection.

Concrete

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3.

Rock Riprap

Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 311.

Geotextile shall be placed under all riprap and shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

Care of Water during Construction

All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect the areas to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the excavations, foundations, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water pumps from which the water shall be pumped.

Stabilization

All borrow areas shall be graded to provide proper drainage and left in a slightly rough condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (M-D-342) or as shown on the accompanying drawings.

Erosion and Sediment Control

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures.

OPERATION AND MAINTENANCE

An operation and maintenance plan in accordance with Local or State Regulations will be prepared for all ponds. As a minimum, the dam inspection checklist located in Appendix A shall be included as part of the operation and maintenance plan and performed at least annually. Written records of maintenance and major repairs needs to be retained in a file. The issuance of a Maintenance and Repair Permit for any repairs or maintenance that involves the modification of the dam or spillway from its original design and specifications is required. A permit is also required for any repairs or reconstruction that involve a substantial portion of the structure. All indicated repairs are to be made as soon as practical.

STORMWATER MANAGEMENT POND MAINTENANCE SCHEDULE

A. ROUTINE MAINTENANCE

Facility shall be Inspected Annually After Major Storms. Inspections Should be Performed During Wet Weather To Determine If The Pond is Functioning Properly.

Top and Side Slopes Of The Embankment Shall be Mowed A Minimum Of Two (2) Times A Year, Once In June And Once In September. Other Side Slopes, The Bottom Of The Pond, And Maintenance Access Should be Mowed As Needed.

Debris And Litter Next To The Outlet Structure Shall be Removed During Regular Mowing Operations And As Needed.

Visible Signs Of Erosion In The Pond As Well As Rip-Rap Outlet Area Shall be Repaired As Soon As It is Noticed.

Sediment Should be Removed When Its Accumulation Reaches 6".

The Low Flow PVC Pipes Shall be Visually Inspected For Clogging A Minimum Of Two (2) Times A Year, Once In June And Once In September. This Should be Accomplished At The Same Time As The Mowing Of The Embankment.

B. NON-ROUTINE MAINTENANCE

Structural Components Of The Pond such As The Dam, Riser Structure And The Pipes Shall be Inspected Upon The Occurrence Of Any Damage. The Components Should be Inspected During Routine Maintenance Operations.

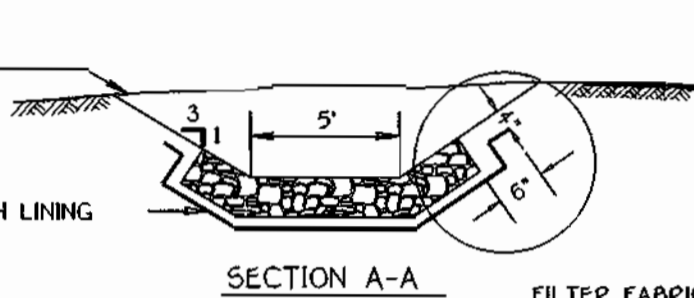
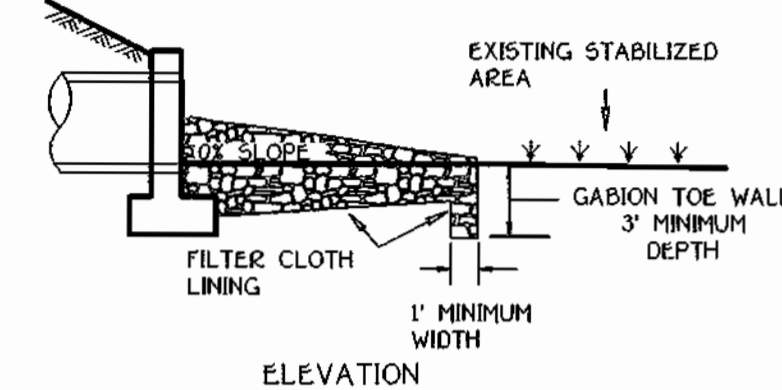
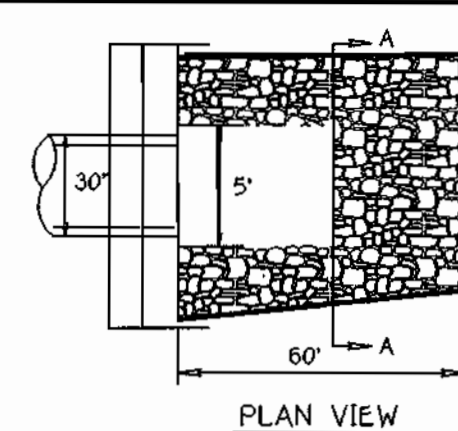
Sediment Should be Removed When Its Accumulation Significantly Reduces The Design Storage, Interferes With The Function Of The Riser, When Deemed Necessary For Aesthetic Reasons, Or When Deemed Necessary By The Howard County Department Of Public Works.

Embankment and Cut-off Trench Construction

THE AREA OF THE PROPOSED SWM POND SHOULD BE STRIPPED OF TOPSOIL AND ANY OTHER UNSUITABLE MATERIALS FROM THE EMBANKMENT OR STRUCTURE AREA IN ACCORDANCE WITH SOIL CONSERVATION GUIDELINES. AFTER STRIPPING OPERATIONS HAVE BEEN COMPLETED, THE EXPOSED SURFACE MATERIALS SHOULD BE PROTECTED WITH A LOADED DUMP TRUCK OR SIMILAR EQUIPMENT IN THE PRESENCE OF A GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE UTILIZING A DYNAMIC CONE PENETROMETER. ANY EXCESSIVELY SOFT OR LOOSE MATERIALS IDENTIFIED BY PROFFOLING OR PENETROMETER TESTING SHOULD BE EXCAVATED TO SUITABLE FIRM SOIL, AND THEN GRADES RE-ESTABLISHED BY BACKFILLING WITH SUITABLE SOIL.

A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER SHOULD BE PRESENT TO MONITOR PLACEMENT AND COMPACTION OF FILL FOR THE EMBANKMENT AND CUT-OFF TRENCH. IN ACCORDANCE WITH HANDBOOK SOIL CONSERVATION SPECIFICATION 370 SOILS CONSIDERED SUITABLE FOR THE CENTER OF EMBANKMENT AND CUT-OFF TRENCH SHALL CONFORM TO UNIFIED SOIL CLASSIFICATION GC, SC, CH, OR CL. IT IS OUR PROFESSIONAL OPINION THAT IN ADDITION TO THE SOIL MATERIALS DESCRIBED ABOVE A FINE GRAINED SOIL, INCLUDING SILT (SH) WITH A PLASTICITY INDEX OF 10 OR MORE CAN BE UTILIZED FOR THE CENTER OF THE EMBANKMENT AND CORE TRENCH. BASED ON OUR VISUAL CLASSIFICATIONS IT APPEARS THAT SOME OF THE ON-SITE SOILS, ESPECIALLY THE NEAR SURFACE SOILS, WILL BE SUITABLE FOR USE AS CORE TRENCH MATERIAL. IT IS RECOMMENDED THAT ADDITIONAL EXPLORATION AND LABORATORY TESTING BE PERFORMED PRIOR TO POND CONSTRUCTION TO IDENTIFY AND QUANTIFY POTENTIAL BODILY AREAS FOR CORE TRENCH MATERIAL. ALL FILL MATERIALS MUST BE PLACED AND COMPACTIONED WITH MD 953 370 SPECIFICATIONS.

ROCK OUTLET PROTECTION III • 5-2

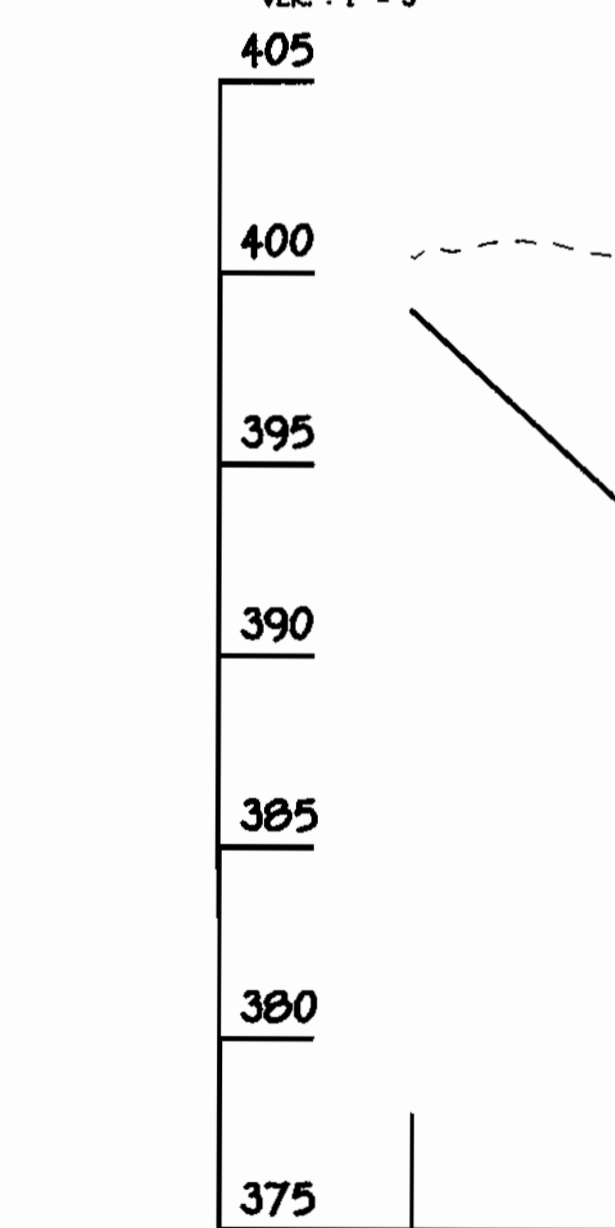


FILTER FABRIC LINING SHALL BE EMBEDDED A MINIMUM OF 4" AND SHALL EXTEND AT LEAST 6" BEYOND THE EDGE OF THE RIP-RAP

366	366	356	356
363	363	353	353
360	360	350	350
357	357	347	347
354	354	344	344

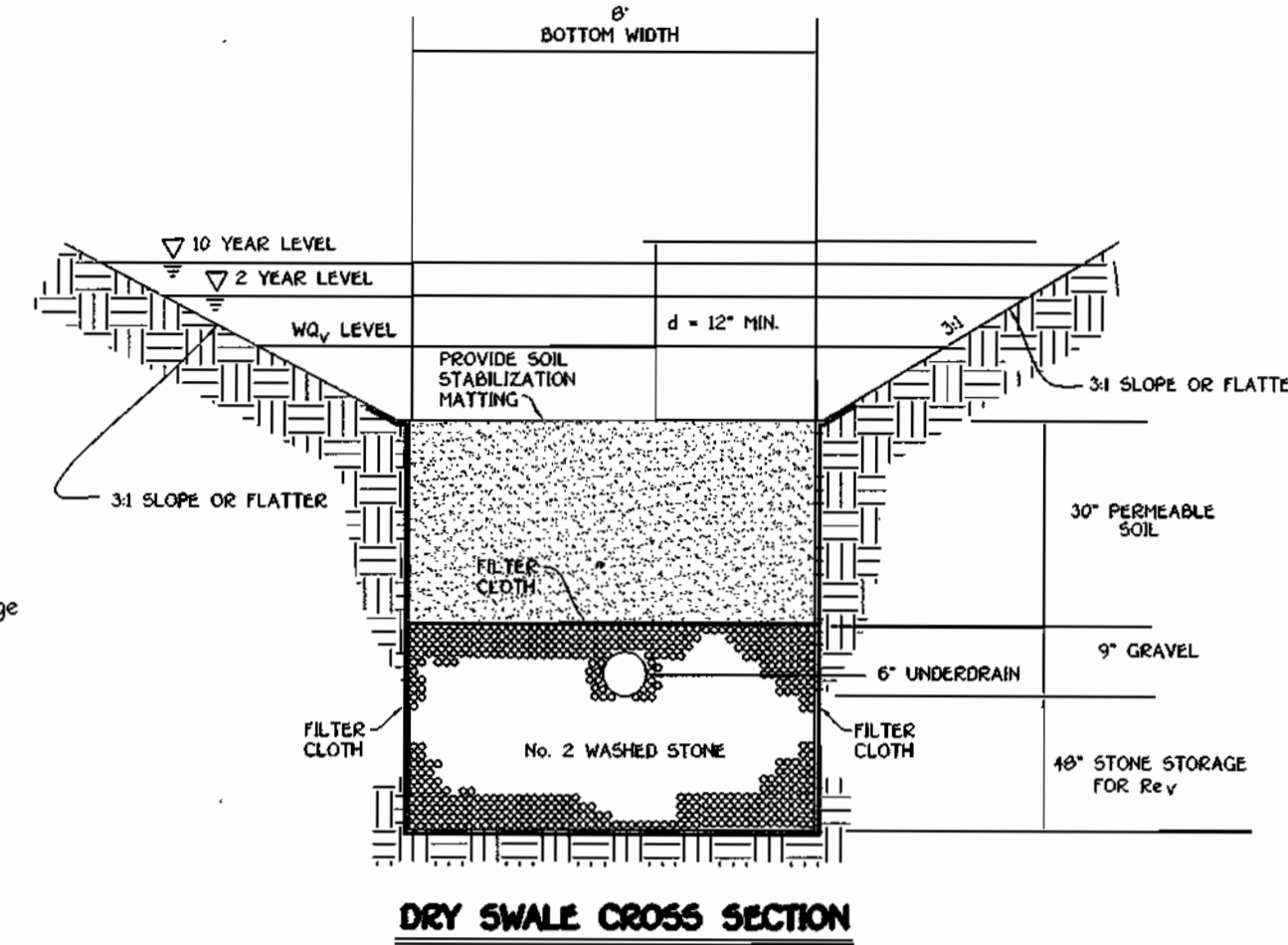
CROSS-SECTION THRU CHANNEL 35' FROM 5-3

SCALE: HORZ. 1" = 30' VER. 1" = 3'



Construction Specifications

- The subgrade for the filter, rip-rap, or gabion shall be prepared to the required lines and grades. Any fill required in the subgrade shall be compacted to a density of approximately that of the surrounding undisturbed material.
- The rock or gravel shall conform to the specified grading limits when installed respectively in the rip-rap or filter.
- Geotextile shall be protected from punching, cutting, or tearing. Any damage other than an occasional small hole shall be repaired by placing another piece of geotextile over the damaged part or by completely replacing the geotextile. All overlaps whether for repairs or for joining two pieces of geotextile shall be a minimum of one foot.
- Stone for the rip-rap or gabion outlets may be placed by equipment. They shall be constructed to the full course thickness in one operation and in such a manner as to avoid displacement of underlying materials. The stone for rip-rap or gabion outlets shall be delivered and placed in a manner that will ensure that it is reasonably homogeneous with the smaller stones and spalls filling the voids between the larger stones. Rip-rap shall be placed in a manner to prevent damage to the filter blanket or geotextile. Hand placement will be required to the extent necessary to prevent damage to the permanent works.
- The stone shall be placed so that it blends in with the existing ground. If the stone is placed too high then the flow will be forced out of the channel and scour adjacent to the stone will occur.



NOTES: FILTER CLOTH SHALL BE GEOTEXTILE CLASS C

RIP-RAP TO BE CLASS II
 450 = 16"
 d_{max} = 24"
 Thickness = 32"
 Q₁₀ = 33.0 C.F.S.
 V₁₀ = 2.52 F.P.S.

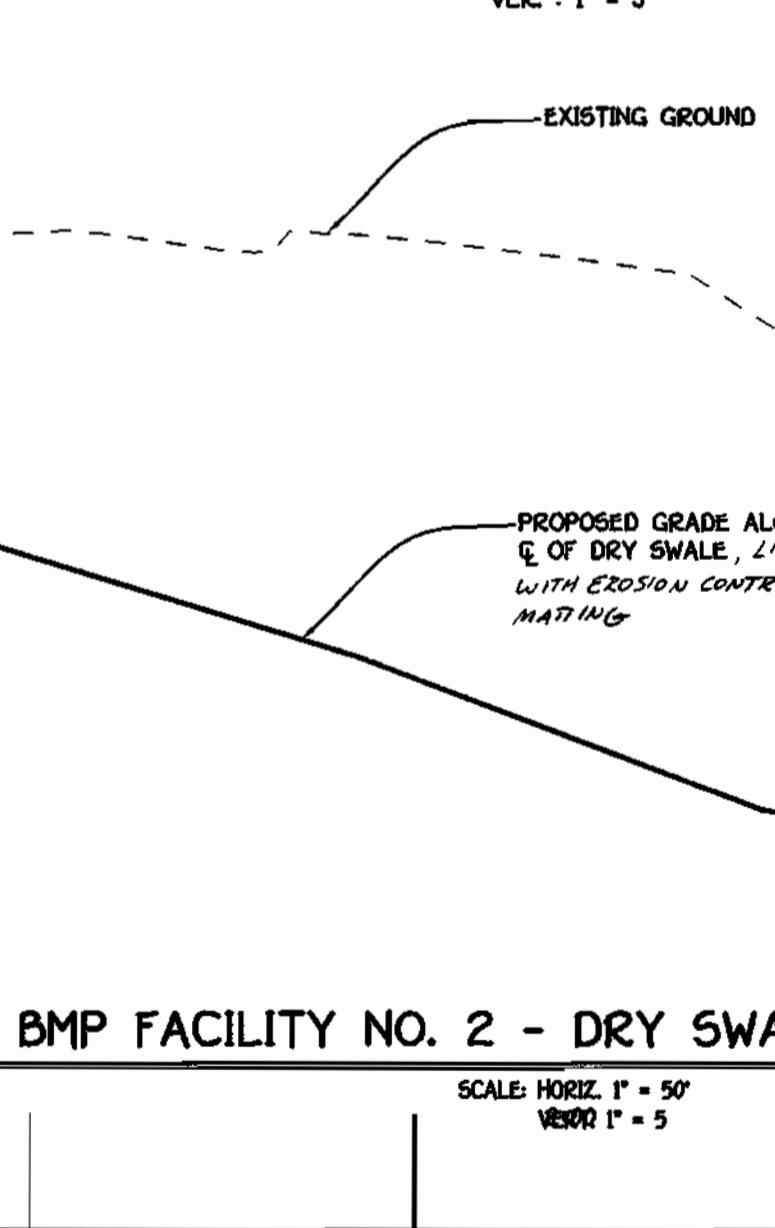
366	366	356	356
363	363	353	353
360	360	350	350
357	357	347	347
354	354	344	344

CROSS-SECTION THRU CHANNEL 35' FROM 5-2

SCALE: HORZ. 1" = 30' VER. 1" = 3'

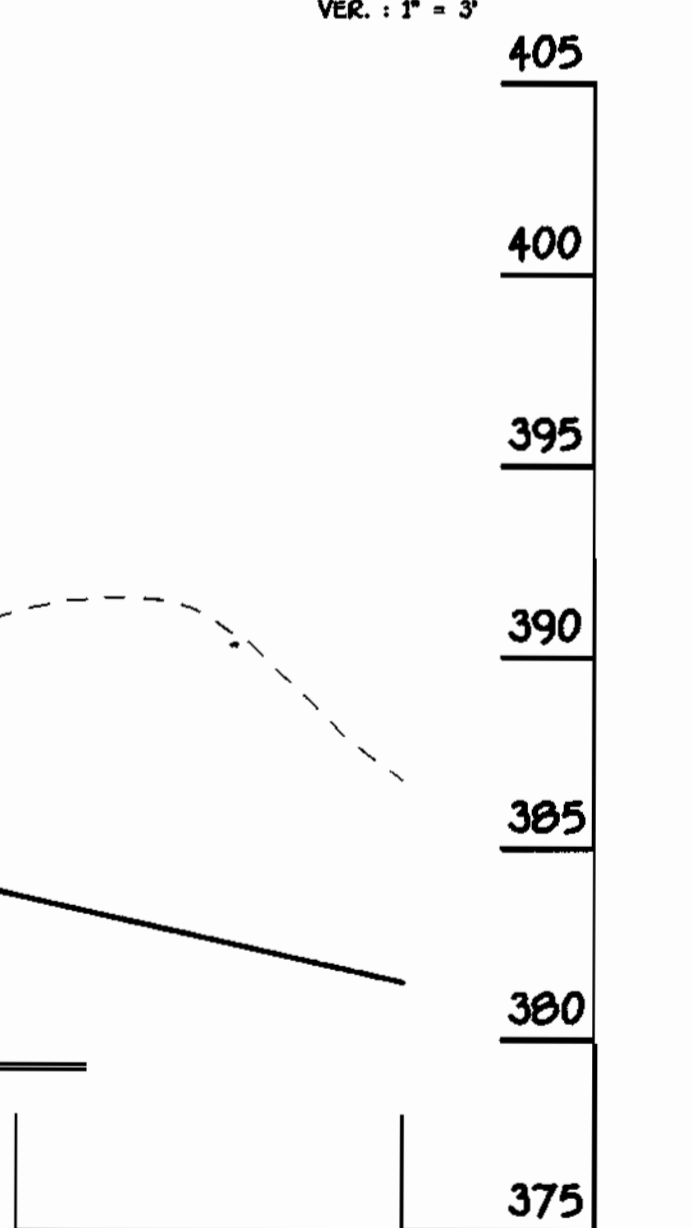
CROSS-SECTION THRU CHANNEL 35' FROM 5-2

SCALE: HORZ. 1" = 30' VER. 1" = 3'



CROSS-SECTION THRU DRY SWALE STA. 3+62

SCALE: HORZ. 1" = 30' VER. 1" = 3'



BMP FACILITY NO. 2 - DRY SWALE PROFILE

SCALE: HORZ. 1" = 50' VERT. 1" = 5'

0+00	1+00	2+00	3+00	4+00	5+00.16
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FISHER, COLLINS & CARTER, INC.
 CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
 CENTRAL SQUARE OFFICE PARK - 18272 BALTIMORE NATIONAL FREE
 ELLICOTT CITY, MARYLAND 21114
 (410) 484 - 2855

STATE OF MARYLAND
 PROFESSIONAL ENGINEER
 No. 10000

DATE: _____ DESCRIPTION: REVISION BLOCK

ENGINEER'S CERTIFICATE
 I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.

CHARLES J. CROVO, SR., P.E., L.S.
 Signature of Engineer (print name below signature) Date: 8/21/02

DEVELOPER'S CERTIFICATE
 I/We certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project will have a certificate of Attendance at a Department of the Environment Approved Training Program on the Control of Sediment and Erosion before beginning the project. I also authorize periodic on-site inspection by the Howard Soil Conservation District.

DONALD R. REIJWER, JR.
 Signature of Developer (print name below signature) Date: 8/21/02

Reviewed for HOWARD SCD and meets Technical Requirements.

U.S.D.A. - Natural Resources Conservation Service
 Date: 8/16/02

This development plan is approved for soil erosion and sediment control by the HOWARD SOIL CONSERVATION DISTRICT.
 Date: 8/16/02

Howard SCD

DEVELOPER/OWNER
 RRSC, L.L.C.
 c/o LAND DESIGN AND DEVELOPMENT, INC.
 8000 MAIN STREET
 ELLICOTT CITY, MARYLAND 21104

BUILDER
 RYAN HOMES
 11460 CROWBRIDGE DRIVE
 SUITE 122
 OWINGS MILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Director - Department of Planning and Zoning
 Date: 10/13/02

Chief, Division of Land Development
 Date: 10/15/02

Chief, Development Engineering Division
 Date: 9/20/02

SUBDIVISION	SECTION/AREA	PARCEL NO.
THE COURTYARDS AT THE TIMBERS		617
DEED REF. 5609 / 611	BLOCK NO. 3	ZONE POR
TAX/ZONE 37	ELEC. DIST. 1st	CENSUS TR. 6030
WATER CODE D 04	SEWER CODE 261000	

STORMWATER MANAGEMENT NOTES AND DETAILS

THE COURTYARDS AT THE TIMBERS
 TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

ZONED: POR

TAX MAP No: 37 PARCEL No: 617 GRID No: 3
 FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND

SCALE: AS SHOWN DATE: AUGUST 21, 2002

SHEET 17 OF 24 **SOP 02-55**

PLANTING/SOIL SPECIFICATIONS

1. PLANTING OF NURSERY STOCK SHALL TAKE PLACE BETWEEN MARCH 15th AND APRIL 30th OR SEPTEMBER 15th AND NOVEMBER 15th.
2. A TWO (2) INCH LAYER OF TOPSOIL SHALL BE SPREAD OVER ALL FORESTATION AREAS IMPACTED BY SITE GRADING TO ASSURE A SUITABLE PLANTING AREA. DISTURBED AREAS SHALL BE SEEDED AND STABILIZED AS PER GENERAL CONSTRUCTION PLAN FOR PROJECT. PLANTING AREAS NOT IMPACTED BY SITE GRADING SHALL HAVE NO ADDITIONAL TOPSOIL INSTALLED.
3. ALL BAREROOT PLANTING STOCK SHALL HAVE THEIR ROOT SYSTEMS DIPPED INTO AN ANTI-DESICCANT GEL PRIOR TO PLANTING.
4. PLANTS SHALL BE INSTALLED SO THAT THE TOP OF ROOT MASS IS LEVEL WITH THE TOP OF EXISTING GRADE. BACKFILL IN THE PLANTING PITS SHALL CONSIST OF 3 PARTS EXISTING SOIL TO 1 PART FINE FINES OR EQUIVALENT.
5. FERTILIZER SHALL CONSIST OF AGRIFORM 22-0-2, OR EQUIVALENT, APPLIED AS PER MANUFACTURER'S SPECIFICATIONS.
6. A TWO (2) INCH LAYER OF HARDWOOD MULCH SHALL BE PLACED OVER THE ROOT AREA OF ALL PLANTINGS.
7. PLANT MATERIAL SHALL BE TRANSPORTED TO THE SITE IN A TARPED OR COVERED TRUCK. PLANTS SHALL BE KEPT MOIST PRIOR TO PLANTING.
8. ALL NON-ORGANIC DEBRIS ASSOCIATED WITH THE PLANTING OPERATION SHALL BE REMOVED FROM THE SITE BY THE CONTRACTOR.

SEQUENCE OF CONSTRUCTION

1. SEDIMENT CONTROL AND TREE PROTECTION DEVICES SHALL BE INSTALLED IN ACCORDANCE WITH GENERAL CONSTRUCTION PLAN FOR SITE. SITE SHALL BE GRADED IN ACCORDANCE WITH GENERAL CONSTRUCTION PLANS.
2. PROPOSED FORESTATION AREAS IMPACTED BY SITE GRADING SHALL BE TOPSOILED AND STABILIZED AS PER NOTE NO. 2 OF PLANTING/SOIL SPECIFICATIONS FOR PROJECT.
3. PLANTS SHALL BE INSTALLED AS PER PLANT SCHEDULE AND THE PLANTING/SOIL SPECIFICATIONS FOR THE PROJECT.
4. UPON COMPLETION OF THE PLANTING, SIGNAGE SHALL BE INSTALLED AS PER THE FOREST PROTECTION DEVICES SHOWN ON THE FOREST CONSERVATION PLAN.
5. PLANTINGS SHALL BE MAINTAINED AND GUARANTEED IN ACCORDANCE WITH THE MAINTENANCE AND GUARANTEE REQUIREMENTS FOR PROJECT.

MAINTENANCE OF PLANTINGS

1. MAINTENANCE OF PLANTINGS SHALL LAST FOR A PERIOD OF 26 MONTHS.
2. ALL PLANT MATERIAL SHALL BE WATERED TWICE A MONTH DURING THE 1st GROWING SEASON. WATERING MAY BE MORE OR LESS FREQUENT DEPENDING ON WEATHER CONDITIONS DURING 2nd GROWING SEASON ONCE A MONTH DURING MAY-SEPTEMBER, IF NEEDED.
3. INVASIVE EXOTICS AND NOXIOUS WEEDS WILL BE REMOVED FROM FORESTATION AREAS. OLD FIELD SUCCESSIONAL SPECIES WILL BE RETAINED.
4. PLANTS WILL BE EXAMINED A MINIMUM TWO TIMES DURING THE GROWING SEASON FOR SERIOUS PLANT PESTS AND DISEASES. SERIOUS PROBLEMS WILL BE TREATED WITH THE APPROPRIATE AGENT.
5. DEAD BRANCHES WILL BE PRUNED FROM PLANTINGS.

GUARANTEE REQUIREMENTS

1. A 75 PERCENT SURVIVAL RATE OF FORESTATION PLANTINGS WILL BE REQUIRED AT THE END OF THE 24 MONTH MAINTENANCE PERIOD. ALL PLANT MATERIAL BELOW THE 75 PERCENT THRESHOLD WILL BE REPLACED AT THE BEGINNING OF THE NEXT GROWING SEASON.

SURETY FOR FORESTATION

1. THE DEVELOPER SHALL POST A SURETY BOND, LETTER OF CREDIT TO ENSURE THAT FORESTATION PLANTINGS ARE COMPLETED. AT THE END OF THE POST-CONSTRUCTION MANAGEMENT AND PROTECTION PERIOD, THE DESIGNATED RESPONSIBLE PROFESSIONAL SHALL CONVEY TO THE DEPARTMENT OF PLANNING AND ZONING CERTIFICATION THAT ALL FOREST CONSERVATION AREAS HAVE REMAINED INTACT OR HAVE BEEN ACHIEVED, AND THAT ANY PERMANENT PROTECTION MEASURES REQUIRED BY THE PLAN ARE IN PLACE. UPON REVIEW OF THE FINAL CERTIFICATION DOCUMENT FOR COMPLETENESS AND ACCURACY, THE COUNTY WILL NOTIFY THE DEVELOPER OF THE RELEASE OF SURETY AND ALL FUTURE OBLIGATIONS. THE DEVELOPER'S LAST OFFICIAL RESPONSIBILITY WILL BE TO TRANSMIT A COPY OF THIS NOTIFICATION TO THE OWNERS OF THE PROPERTIES. SUCH TRANSMITTAL WILL SERVE AS OFFICIAL NOTICE TO OWNERS OF THE ASSUMPTION OF FULL RESPONSIBILITY FOR ALL FUTURE FOREST CONSERVATION OBLIGATIONS.

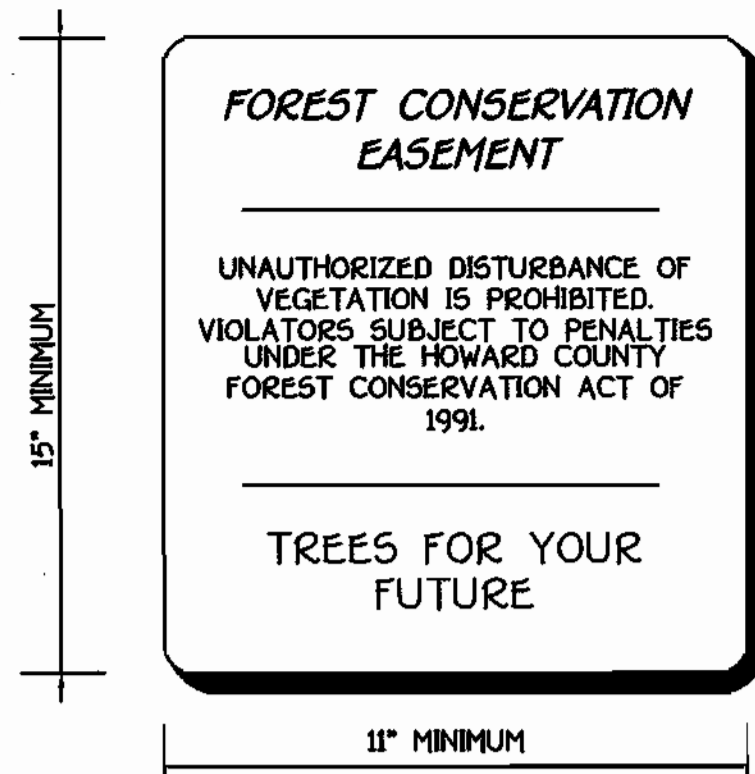
2. SURETY FOR ON-SITE RETENTION (3.0 AC. x 0.20 = \$6,592.00), AND OFF-SITE AFFORESTATION (3.0 AC. x 0.50 = \$15,276.00) IS POSTED WITH THE DEVELOPER'S AGREEMENT FOR THIS SUBDIVISION. TOTAL FOREST CONSERVATION SURETY AMOUNT FOR THIS SUBDIVISION IS \$21,868.00.

FCP NOTES

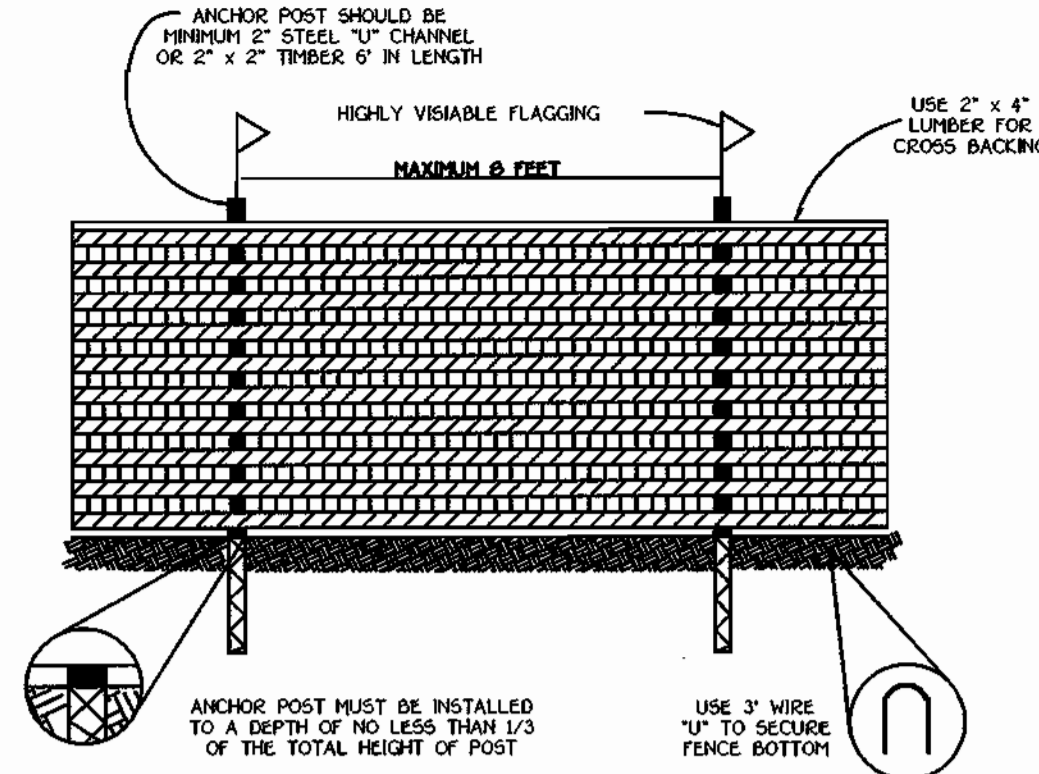
1. ANY FOREST CONSERVATION EASEMENT (FCE) AREA SHOWN HEREON IS SUBJECT TO PROTECTIVE COVENANTS WHICH MAY BE FOUND IN THE LAND RECORDS OF HOWARD COUNTY WHICH RESTRICT THE DISTURBANCE AND USE OF THESE AREAS.
2. THE FOREST CONSERVATION EASEMENTS HAVE BEEN ESTABLISHED TO FULFILL THE REQUIREMENTS OF SECTION 18.1200 OF THE HOWARD COUNTY CODE, FOREST CONSERVATION ACT. NO CLEARING, GRADING, OR CONSTRUCTION IS PERMITTED WITHIN THE FOREST CONSERVATION EASEMENTS. HOWEVER, FOREST MANAGEMENT PRACTICES AS DEFINED IN THE DEED OF FOREST CONSERVATION EASEMENT ARE ALLOWED.
3. FORESTED AREAS OCCURRING OUTSIDE OF THE FCE SHALL NOT BE CONSIDERED PART OF THE FCE AND SHALL NOT BE SUBJECT TO PROTECTIVE LAND COVENANTS.
4. LIMITS OF DISTURBANCE SHALL BE RESTRICTED TO AREAS OUTSIDE THE LIMIT OF TEMPORARY FENCING OR THE FCE BOUNDARY, WHICHEVER IS GREATER.
5. THERE SHALL BE NO CLEARING, GRADING, CONSTRUCTION OR DISTURBANCE OF VEGETATION IN THE FOREST CONSERVATION EASEMENT, EXCEPT AS PERMITTED BY HOWARD COUNTY DPZ.
6. NO STOCKPILES, PARKING AREAS, EQUIPMENT CLEANING AREAS, ETC. SHALL OCCUR WITHIN AREAS DESIGNATED AS FOREST CONSERVATION EASEMENTS.
7. TEMPORARY FENCING SHALL BE USED TO PROTECT FOREST RESOURCES DURING CONSTRUCTION. THE FENCING SHALL BE PLACED ALONG ALL FCE BOUNDARIES WHICH OCCUR WITHIN 15 FEET OF THE PROPOSED LIMITS OF DISTURBANCE.
8. PERMANENT SIGNAGE SHALL BE PLACED 50' - 100' APART ALONG BOUNDARIES OF ALL AREAS INCLUDED IN FOREST CONSERVATION EASEMENTS.
9. THE OUTSTANDING FOREST CONSERVATION REFORESTATION OBLIGATION SHALL BE MET THROUGH OFF-SITE PLANTING (3.0 AC.) ON LOT 5 IN "CHASE FARM", PREVIOUSLY RECORDED AS PLAT NO. 12067.

Eco-Science Professionals, Inc. CONSULTING ECOLOGISTS
 MD DNR Qualified Professional
 USACE Wetland Delimited
 Certification # WDCP3400610044B
 JOHN P. CANOLES

ON-SITE SIGNAGE



BLAZE ORANGE PLASTIC MESH



- NOTES:**
1. FOREST PROTECTION DEVICE ONLY.
 2. RETENTION AREA WILL BE SET AS PART OF THE REVIEW PROCESS.
 3. BOUNDARIES OF RETENTION AREA SHOULD BE STAKED AND FLAGGED PRIOR TO INSTALLING DEVICE.
 4. ROOT DAMAGE SHOULD BE AVOIDED.
 5. PROTECTIVE SIGNAGE MAY ALSO BE USED.
 6. DEVICE SHOULD BE MAINTAINED THROUGHOUT CONSTRUCTION.

TREE PROTECTION DETAIL

NOT TO SCALE

FOREST DATA	
GROSS AREA:	16.4
NET TRACT AREA (NTA):	16.4
EXISTING FOREST (NTA):	7.2
CONSERVATION THRESHOLD:	3.3
FOREST TO BE CLEARED (NTA):	5.3
FOREST TO BE RETAINED IN FCE (NTA):	1.9
REFORESTATION REQUIRED:	3.0
OFFSITE FORESTATION PROPOSED:	3.0

Forest Conservation Worksheet

BASIC SITE DATA	ACRES
Gross Site Area:	16.4
Area within 100 year floodplains:	
Area within agricultural use or preservation parcel:	
Area within overhead transmission lines:	
Net Tract Area (NTA):	16.4
Land Use Category:	POR
INFORMATION FOR CALCULATIONS	
Net Tract Area (NTA):	16.4
Forest conservation threshold (20% x NTA):	3.3
Afforestation threshold (15% x NTA):	2.5
Existing forest on NTA:	7.2
Existing forest above conservation threshold:	3.9
Break even point (if applicable):	4.1
Forest to be cleared:	5.3
Forest to be retained:	1.9
AFFORESTATION CALCULATIONS	
No forest clearing: Afforestation Threshold - Existing Forest	
Forest clearing: (Aff. thresh. - ex. forest) + (forest to be cleared x 2)	
REFORESTATION CALCULATIONS	
Clearing above threshold	
a. Forest cleared above threshold _____ x 1/4 _____ acres	
b. Forest retained above threshold: _____ acres	
Reforestation Required (a-b): _____	
Clearing below threshold	
a. Forest cleared above threshold - 3.9 x 1/4 = 1.0 acres	
b. Forest cleared below threshold - 1.4 x 2 = 2.8 acres	
Reforestation Required (a+b): _____ 3.8	

FISHER, COLLINS & CARTER, INC.
 CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
 CENTRAL SQUARE OFFICE BLDG. - 1822 BALTIMORE NATIONAL PLZ
 ELICOTT CITY, MARYLAND 21142
 (410) 461-2255



ENGINEER'S CERTIFICATE
 I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.
 Signature of Engineer (print name below signature) _____ Date 8/22/02
 CHARLES J. CROVO, SR., P.E., L.S.

DEVELOPER'S CERTIFICATE
 I/we certify that all development and construction will be done according to this plan for sediment and erosion control and that all responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize periodic on-site inspection by the Howard Soil Conservation District.
 Signature of Developer (print name below signature) _____ Date 8/22/02
 DONALD R. REUWER, JR.

Prepared By: _____
 John Canoles - Certification # WDCP3400610044B
 Date 8/21/02

DEVELOPER/OWNER
 RRSK, L.L.C.
 c/o LAND DESIGN AND DEVELOPMENT, INC.
 8000 MARY STREET
 ELICOTT CITY, MARYLAND 21143

BUILDER
 RYAN HOMES
 11460 CROWNBOCKE DRIVE
 SUITE 125
 OWINGS MILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Director - Department of Planning and Zoning _____ Date 10/3/02
 Chief, Division of Land Development _____ Date 10/3/02
 Chief, Development Engineering Division _____ Date 9/20/02

SUBDIVISION	SECTION/AREA	PARCEL NO.
THE COURTYARDS AT THE TIMBERS		617
DEED REF.	BLOCK NO.	ZONE
5609 / 611	3	POR
TAX/ZONE	ELEC. DIST.	CENSUS TR.
37	1st.	6030
WATER CODE	SEWER CODE	
D 04	2610000	

FOREST CONSERVATION PLAN NOTES AND DETAILS

THE COURTYARDS AT THE TIMBERS TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

ZONED: POR
 TAX MAP No.: 37 PARCEL No.: 617 GRID NO.: 3
 FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND
 SCALE: AS SHOWN DATE: AUGUST 21, 2002
 SHEET 21 OF 24 **SDP 02-55**

HILLS-CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Page 1 of 1

Project Name: Khan Property Office Building Location: Howard County, Maryland

DATE	TIME	DEPTH (FT)	SOIL DESCRIPTION	WATER	REMARKS
08/21/02	08:30	0.0	Topsoil		1' Topsoil
08/21/02	08:35	1.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	08:40	2.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	08:45	3.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	08:50	4.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling

HILLS-CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Page 1 of 1

Project Name: Khan Property Office Building Location: Howard County, Maryland

DATE	TIME	DEPTH (FT)	SOIL DESCRIPTION	WATER	REMARKS
08/21/02	08:55	5.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:00	6.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:05	7.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:10	8.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:15	9.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling

HILLS-CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Page 1 of 1

Project Name: Khan Property Office Building Location: Howard County, Maryland

DATE	TIME	DEPTH (FT)	SOIL DESCRIPTION	WATER	REMARKS
08/21/02	09:20	10.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:25	11.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:30	12.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:35	13.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:40	14.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling

HILLS-CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Page 1 of 1

Project Name: Khan Property Office Building Location: Howard County, Maryland

DATE	TIME	DEPTH (FT)	SOIL DESCRIPTION	WATER	REMARKS
08/21/02	09:45	15.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:50	16.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	09:55	17.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:00	18.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:05	19.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling

HILLS-CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Page 1 of 1

Project Name: Khan Property Office Building Location: Howard County, Maryland

DATE	TIME	DEPTH (FT)	SOIL DESCRIPTION	WATER	REMARKS
08/21/02	10:10	20.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:15	21.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:20	22.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:25	23.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:30	24.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling

HILLS-CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Page 1 of 1

Project Name: Khan Property Office Building Location: Howard County, Maryland

DATE	TIME	DEPTH (FT)	SOIL DESCRIPTION	WATER	REMARKS
08/21/02	10:35	25.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:40	26.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:45	27.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:50	28.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	10:55	29.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling

HILLS-CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Page 1 of 1

Project Name: Khan Property Office Building Location: Howard County, Maryland

DATE	TIME	DEPTH (FT)	SOIL DESCRIPTION	WATER	REMARKS
08/21/02	11:00	30.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	11:05	31.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	11:10	32.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	11:15	33.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	11:20	34.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling

HILLS-CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Page 1 of 1

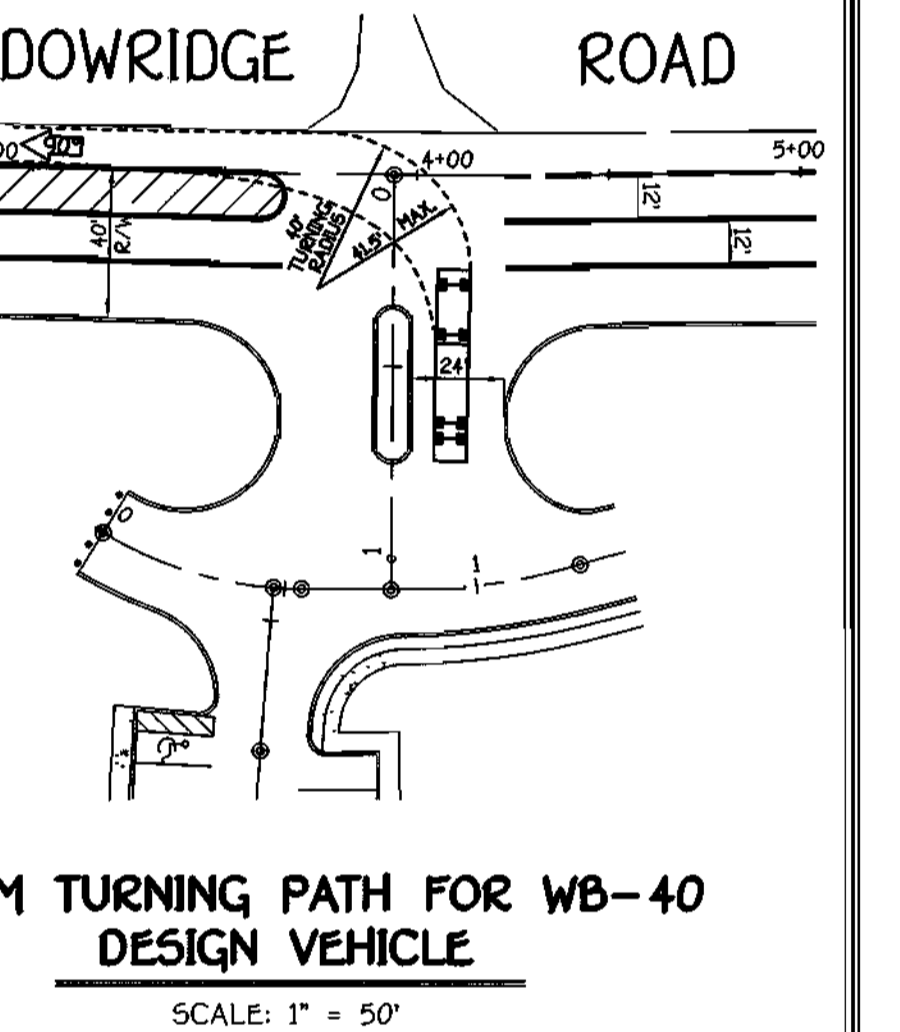
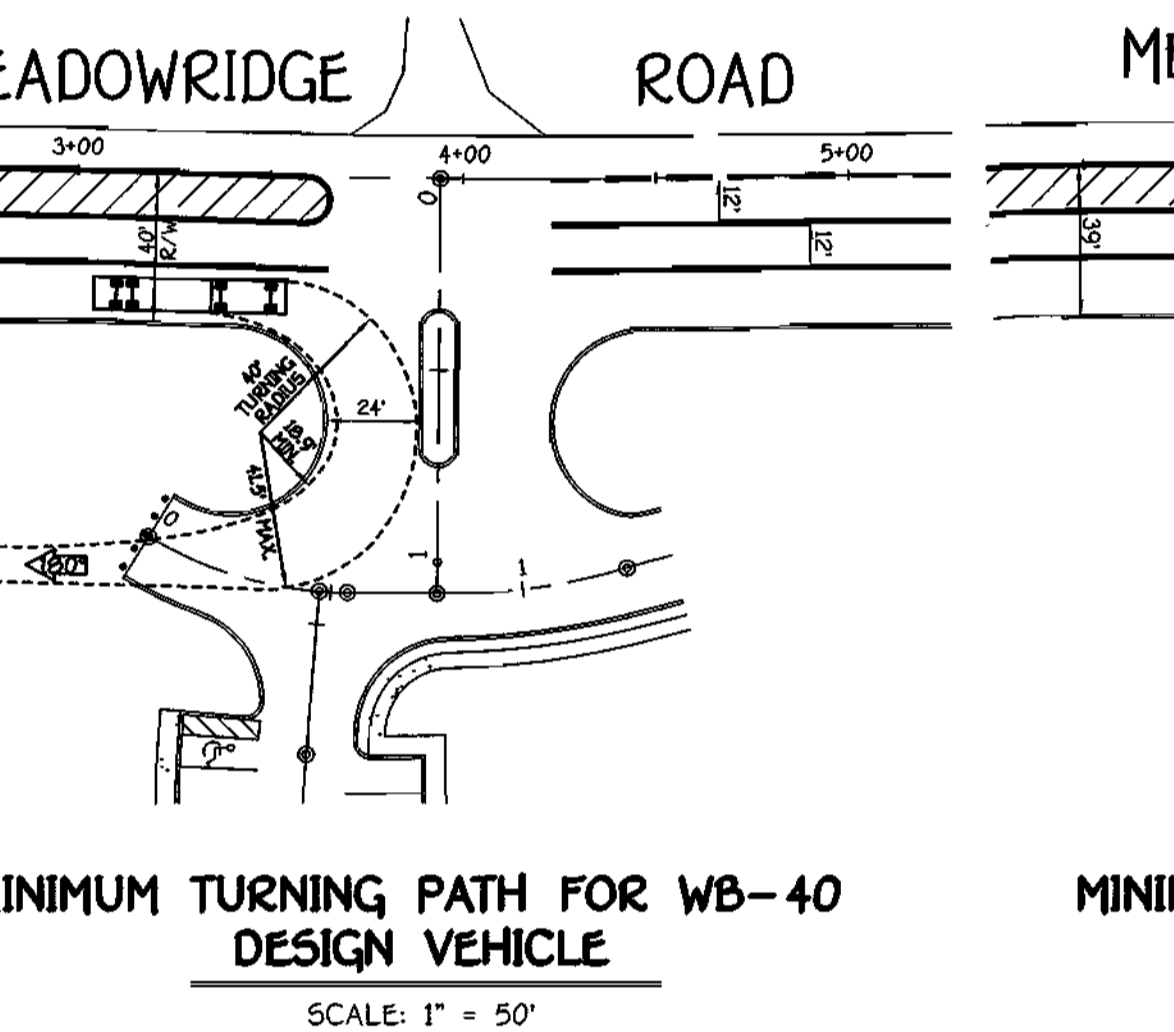
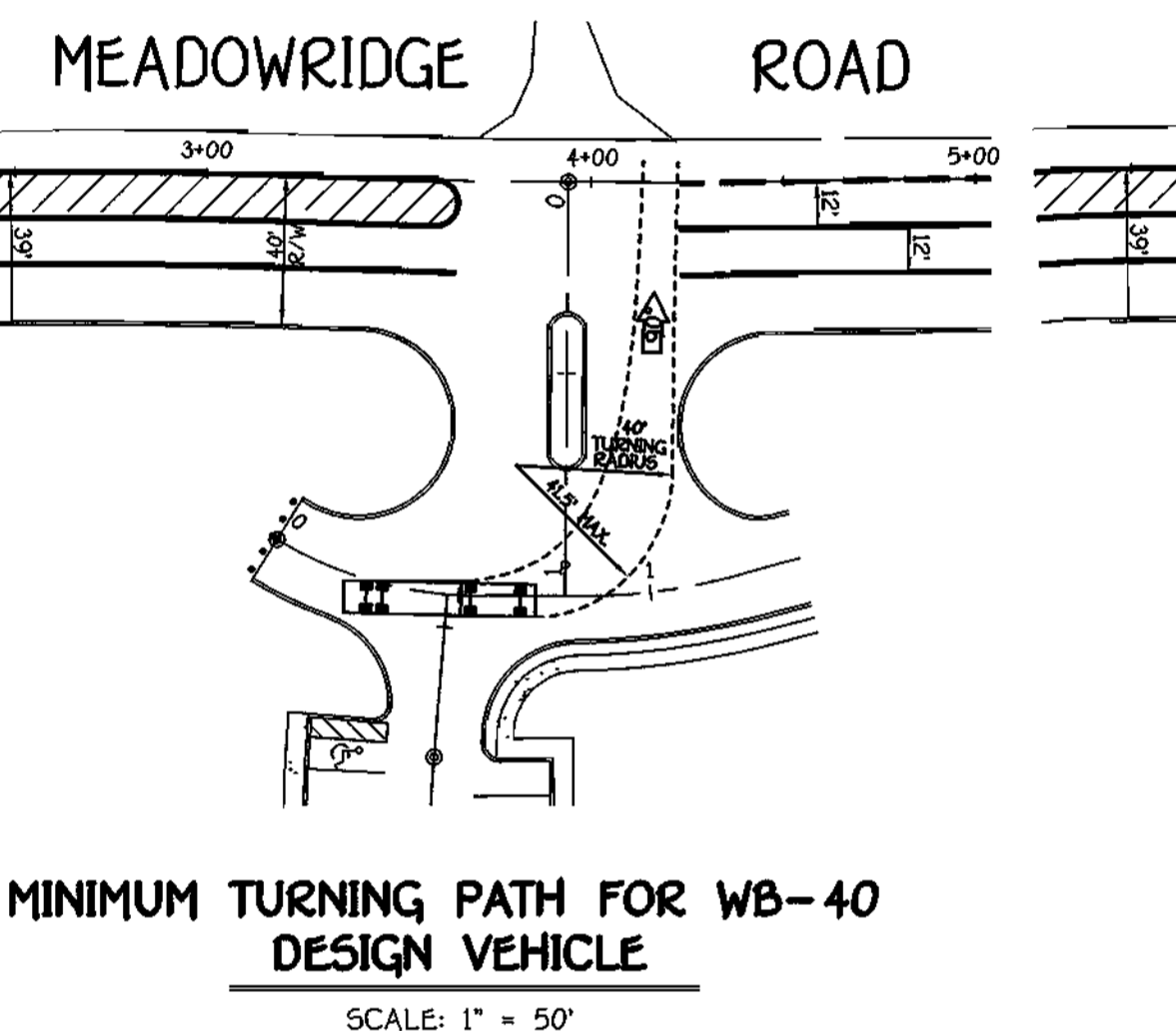
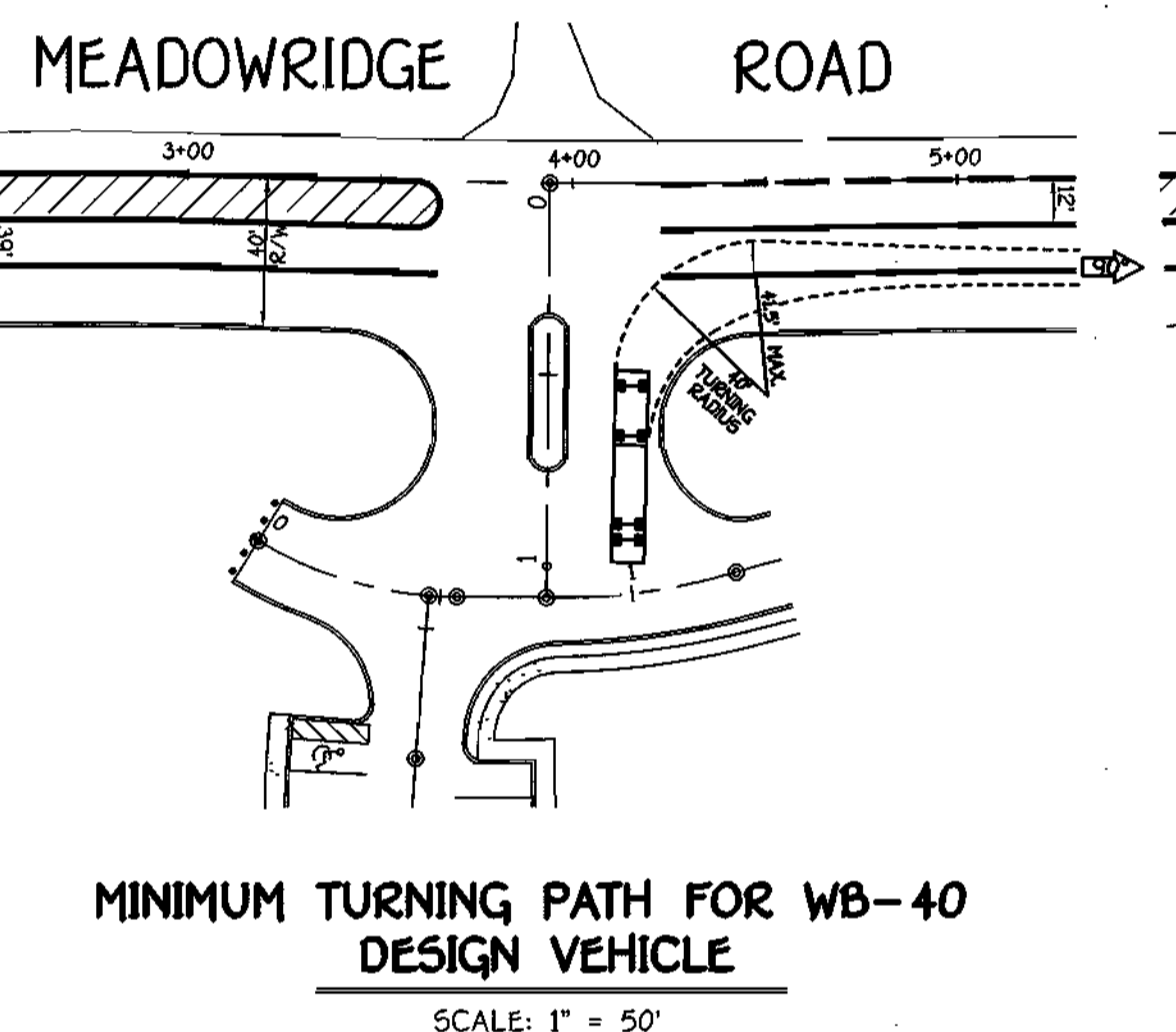
Project Name: Khan Property Office Building Location: Howard County, Maryland

DATE	TIME	DEPTH (FT)	SOIL DESCRIPTION	WATER	REMARKS
08/21/02	11:25	35.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	11:30	36.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	11:35	37.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	11:40	38.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	11:45	39.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling

HILLS-CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Page 1 of 1

Project Name: Khan Property Office Building Location: Howard County, Maryland

DATE	TIME	DEPTH (FT)	SOIL DESCRIPTION	WATER	REMARKS
08/21/02	11:50	40.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	11:55	41.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	12:00	42.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	12:05	43.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling
08/21/02	12:10	44.0	Dark brown, silty clay loam with some sand, some organic matter (SM-C)		No groundwater encountered while drilling



4.5 Groundwater

Groundwater levels were monitored in the open borings. In the proposed building area, groundwater was only encountered on the sampling spoon at a depth of 29.2 ft in Boring TB-13.

A more accurate determination of the hydrostatic water table would require the installation of piezometers or packers which could be monitored over an extended period of time. The actual level of the hydrostatic water table and the amount and level of perched water should be anticipated to fluctuate throughout the year, depending on variations in precipitation, surface runoff, infiltration, site topography, and drainage.

5.0 EVALUATIONS AND RECOMMENDATIONS

Our findings indicate that the site can be developed for the proposed construction utilizing conventional spread footings and ground-supported slab construction. Special consideration should be given to the proper proportioning of fill operations, footing excavations, and concrete placement in all structural areas. Footings should not be constructed on or over any existing fill materials, should such materials be encountered at the site.

The following recommendations have been developed on the basis of the previously described project characteristics and subsurface conditions. If there are any changes to the project characteristics or if different subsurface conditions are encountered during construction, HCEA should be consulted so that the recommendations of this report can be reviewed and revised, if necessary.

5.1 General Site Preparation

The initial step in the development of the site should be the controlled removal of surficial topsoil, wet or soft soils, and deleterious materials from the areas to be developed. Stripper operations should be performed in a manner consistent with good erosion and sediment control practices.

After the initial stripping process is completed, areas of the site to receive fill, or areas of the site already where structures will be located, should be protected. The protection operations should be performed using a 20-ton fully-loaded dump truck. The purpose of the protection will be to provide sufficient stabilization to locate any materials placed on top of the soils requiring underpinning. A Geotechnical Engineer or experienced Soils Inspector should witness the protection operations and should determine which areas need further underpinning and/or stabilization.

5.2 Fill Selection, Placement and Compaction

All material to be used as fill or backfill should be inspected, tested and approved by the Geotechnical Engineer. In general, the on-site soils which are free from organic and other deleterious components can be re-used as general site fill. Materials suitable for various construction purposes can be identified by an experienced Soils Inspector during grading operations.

Extensive moisture conditioning (that is, wetting or drying) of the clayey soils should be anticipated to achieve proper compaction. The moisture content of the soils should be controlled properly to avoid excessive construction delays. If imported fill material is required, those materials should have Unified Soil Classification of SM or better.

It is recommended that the more granular materials encountered be utilized in the upper 24 feet of the pavement areas, if possible. Selectively placing these materials could result in higher subgrade California Bearing Ratio (CBR) values, thereby reducing paving costs.

All fill should be placed in relatively horizontal 8-inch (maximum) loose lifts and should be compacted to a minimum of 95 percent of the Standard Proctor (ASTM D-1556) maximum dry density. Fill materials in landscape and other non-structural areas should be compacted to at least 90 percent of the Standard Proctor maximum dry density if significant subsidence of the fill under its own weight is to be avoided. Field moisture contents should be maintained within 2 percentage points of the optimum moisture content in order to provide adequate compaction.

Structural fill should extend a minimum of ten feet beyond building lines where floor slabs are to be constructed on the fill. Fill slopes no steeper than 2:1 (H:V), or flatter, should be used. A sufficient number of in-place density tests should be performed by an experienced Engineering Technician on a full-time basis to verify that the proper degree of compaction is being obtained.

5.3 Foundations

Based on the plan provided to this office, it appears that the proposed finished floor elevations will result in the building being located predominantly in cut. As much as 18 ft of cut will be encountered on the western side of the building. The southeast corner of the building will be located roughly near existing grades. As such, it is anticipated that the proposed building will be supported on natural soils.

5.4 Ground-Supported Slabs

Floor slabs should be supported on approved, firm natural soils, or on new compacted fill. The slab subgrade should be prepared in accordance with the procedures outlined in Sections 5.1 and 5.2 of this report. In particular, the slab subgrade should be heavily protected to delineate any soft or loose areas requiring underpinning or stabilization.

It is recommended that the slab be directly supported on a minimum 4-inch layer of clean granular material, such as washed sand, clean sand and gravel, or screened crushed stone. Those materials will require acquisition from an off-site source. A suitable moisture/water barrier (that is, polyethylene sheathing) should also be provided. These procedures will provide a moisture break that will help to prevent capillary rise, dampness of the floor slabs and also help to cure the slab concrete. It is also recommended that construction joints on the slab surface and isolation joints on the slab and structural walls be provided such that the slab would be ground-supported.

On most projects, there is a significant time lag between initial grading and a point when the contractor is ready to pour the full-grade. Environmental conditions and construction traffic often disturb the subgrade soils. Provisions should be made in the construction specifications for the restoration of the subgrade soils to a stable condition prior to the placement of the concrete for the floor slabs.

5.5 Groundwater and Drainage

In the proposed building area, groundwater was only encountered on the sampling spoon at a depth of 29.2 ft in Boring TB-13. Therefore, major groundwater-related problems are not anticipated during the building construction.

Any water infiltration resulting from precipitation, surface runoff, or perched water should be able to be controlled by means of sump pits and pumps, or by gravity ditching procedures. If conditions are encountered that cannot be handled in such a manner, the Geotechnical Engineer should be consulted.

Adequate drainage should be provided to the site to minimize any increases in the moisture contents of the foundation soils. All pavement or parking areas should be sloped away from the structure to prevent ponding of water around the building. The site drainage should also be such that run-off onto adjacent properties is controlled properly.

5.6 Retain-Grade and Retaining Walls

The magnitude of lateral earth pressure against subsurface walls is dependent on the type of backfill soil, drainage provisions, and whether the walls are permitted to yield during and/or after placement of the backfill. If the walls are designed as free-standing walls with unrestricted rotation at the top, then an equivalent fluid pressure distribution considering an equivalent fluid weight of 45 lb/ft³ can be used for design purposes. For walls that are designed such that movement of the top of the wall is prohibited, an equivalent fluid pressure distribution considering an equivalent fluid weight of 80 lb/ft³ should be used for design purposes. Any surcharge loadings must also be considered in the wall designs.

Generally, backfill materials behind the walls should consist of granular soils having classifications of SM or better. Because of a potential for swelling, cohesive materials should not be used as wall backfill, except, perhaps, in the upper most 1 ft where a relatively impervious layer will be desirable in order to minimize the infiltration of the subsurface drainage into the granular backfill behind the wall. It is considered essential that all backfill materials be inspected and approved by the Geotechnical Engineer prior to their use.

Wall backfill materials should be compacted to dry densities on the order of 95 percent of the Standard Proctor maximum dry density. We wish to point out that it may be necessary to use smaller walk-behind compaction equipment near the walls to obtain the proper compaction but to avoid damaging the walls. Also, the walls should be properly braced during backfilling operations.

An adequate drainage system should be provided behind walls such that any surface infiltration or groundwater is intercepted and disposed. Otherwise, hydrostatic pressures should also be considered in the wall design.

5.7 Stormwater Management by Infiltration

We have evaluated the site subsurface conditions at the test boring locations drilled in the vicinity of the proposed SWM facilities in accordance with the State of Maryland, "2000 Maryland Stormwater Design Manual, Volumes I & II". The following information is provided for planning stormwater management measures:

1. Location of seasonal high groundwater table.

Groundwater was monitored during the drilling of the borings. Groundwater was encountered at the following locations:

Boring	Approximate Depth to Groundwater (ft)
TB-1	7.0
TB-2	6.0
TB-3	8.0
TB-4	6.5
TB-5	6.7
TB-6	6.75
TB-7	5.375
TB-8	4.125
TB-9	5.150
TB-C	2.003

Information pertaining to the soil and groundwater conditions encountered in the SWM area borings can be found on the Records of Soil Exploration in the Appendix.

2. Backlog

Backlog was not encountered within the depths explored in the SWM area borings during this exploration. However, seams of debris material were encountered at various locations within the borings.

3. Remarks

This report has been prepared to aid in the evaluation of the site for the proposed office building construction. It is considered that adequate recommendations have been provided to serve as a basis for design and preparation of plans and specifications. Additional recommendations can be provided as needed.

water should be anticipated to fluctuate throughout the year, depending on variations in precipitation, surface runoff, infiltration, site topography, and drainage. Site grading operations at other parts of the site can also influence the level of the groundwater at the stormwater management area specifically. HCEA cannot be responsible for changes in groundwater conditions at the site due to seasonal variation and changes caused by other factors such as table and/or bedrock.

2. Subsurface Conditions

The soils encountered in the borings consisted predominantly of silty sand and gravel (SM-GM), silt sand (SM), and silt sand with clay (SC-SM). In-situ infiltration testing was performed at locations other than each boring, except for Boring TB-6 where groundwater was encountered at depths shallower than the proposed in-situ infiltration test depths in the boring. We wish to point out that the in-situ infiltration tests obtained have had no factor of safety applied to them. The results of the in-situ tests are as follows:

Boring	Measured In-Situ Infiltration Rate (in/hr)
TB-1	0.000
TB-2	0.126
TB-3	2.188
TB-4	0.000
TB-5	2.063
TB-6	6.750
TB-7	5.375
TB-8	4.125
TB-9	5.150
TB-C	2.003

Information pertaining to the soil and groundwater conditions encountered in the SWM area borings can be found on the Records of Soil Exploration in the Appendix.

7.0 REMARKS

This report has been prepared to aid in the evaluation of the site for the proposed office building construction. It is considered that adequate recommendations have been provided to serve as a basis for design and preparation of plans and specifications. Additional recommendations can be provided as needed.

Based on the State of Maryland's "2000 Maryland Stormwater Design Manual, Volumes I & II", infiltration basins and trenches are not acceptable practices when an infiltration rate of less than 0.02 inches per hour is obtained. Bioretention facilities in areas with in-situ infiltration rates of less than 0.02 inches per hour require underdrains. Also, the bottom of the facility should be located a minimum of 4 ft above the seasonally high water table and/or bedrock.

Specific details pertaining to the design of the proposed SWM facilities were not available at the time this report was being prepared. HCEA is therefore unable to comment on the feasibility of utilizing infiltration methods of DWM for the various facilities at the site.

6.0 RECOMMENDED ADDITIONAL SERVICES

Additional soil and foundation engineering, testing, and consulting services recommended for this project are summarized below:

Site Preparation and Foundation: A Geotechnical Engineer or experienced Soils Inspector should inspect the site after it has been stripped and excavated. The inspector should determine if any undercutting or in-place densification is necessary to prepare a subgrade for fill placement or for slab support.

Fill Placement and Compaction: A Geotechnical Engineer or experienced Soils Inspector should witness any required filling operations and should take sufficient in-place density tests to verify that the specified degree of fill compaction is achieved. He should observe and approve borrow materials used and should determine if their existing moisture contents are suitable.

Footings: A Geotechnical Engineer or experienced Soils Inspector should inspect the footing excavations for the building foundations. He should verify that the design bearing pressure is available and that no loose pockets exist beneath the bearing surfaces of the footing excavations. Based on the inspection, the Inspector would either approve the bearing surfaces or recommend that loose or soft soils be undercut to expose satisfactory bearing materials.

4.5 Groundwater

Groundwater levels were monitored in the open borings. In the proposed building area, groundwater was only encountered on the sampling spoon at a depth of 29.2 ft in Boring TB-13.

A more accurate determination of the hydrostatic water table would require the installation of piezometers or packers which could be monitored over an extended period of time. The actual level of the hydrostatic water table and the amount and level of perched water should be anticipated to fluctuate throughout the year, depending on variations in precipitation, surface runoff, infiltration, site topography, and drainage.

5.0 EVALUATIONS AND RECOMMENDATIONS

Our findings indicate that the site can be developed for the proposed construction utilizing conventional spread footings and ground-supported slab construction. Special consideration should be given to the proper proportioning of fill operations, footing excavations, and concrete placement in all structural areas. Footings should not be constructed on or over any existing fill materials, should such materials be encountered at the site.

The following recommendations have been developed on the basis of the previously described project characteristics and subsurface conditions. If there are any changes to the project characteristics or if different subsurface conditions are encountered during construction, HCEA should be consulted so that the recommendations of this report can be reviewed and revised, if necessary.

5.1 General Site Preparation

The initial step in the development of the site should be the controlled removal of surficial topsoil, wet or soft soils, and deleterious materials from the areas to be developed. Stripper operations should be performed in a manner consistent with good erosion and sediment control practices.

After the initial stripping process is completed, areas of the site to receive fill, or areas of the site already where structures will be located, should be protected. The protection operations should be performed using a 20-ton fully-loaded dump truck. The purpose of the protection will be to provide sufficient stabilization to locate any materials placed on top of the soils requiring underpinning. A Geotechnical Engineer or experienced Soils Inspector should witness the protection operations and should determine which areas need further underpinning and/or stabilization.

FISHER, COLLINS & CENTER, INC.
CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
CENTRAL SQUARE OFFICE PARK - 10772 BALDWIN NATIONAL PIKE
ELLCOTT CITY, MARYLAND 21042
410-486-2000

HILLS-CARNES ENGINEERING ASSOCIATES, INC.
10772 BALDWIN NATIONAL PIKE
ELLCOTT CITY, MARYLAND 21042
410-486-2000

STATE OF MARYLAND
PROFESSIONAL ENGINEER

ENGINEER'S CERTIFICATE

I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.

CHARLES J. CROWD, SR., P.E., L.S. 8/22/02
Signature of Engineer (print name below signature) Date

DEVELOPER'S CERTIFICATE

I/we certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project will have Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize periodic inspection by the Howard Soil Conservation District.

DONALD R. REIJER, JR. 8/22/02
Signature of Developer (print name below signature) Date

DEVELOPER/OWNER

RESK, L.L.C.
c/o LAND DESIGN AND DEVELOPMENT, INC.
8000 MAIN STREET
ELLCOTT CITY, MARYLAND 21043

BUILDER

RYAN HOMES
11460 CROWNDRIVE
SUITE 100
OWINGS MILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Director: Department of Planning and Zoning
Chief, Division of Land Development
Chief, Development Engineering Division

10/3/02
10/5/02
9/20/02

DATE DATE DATE

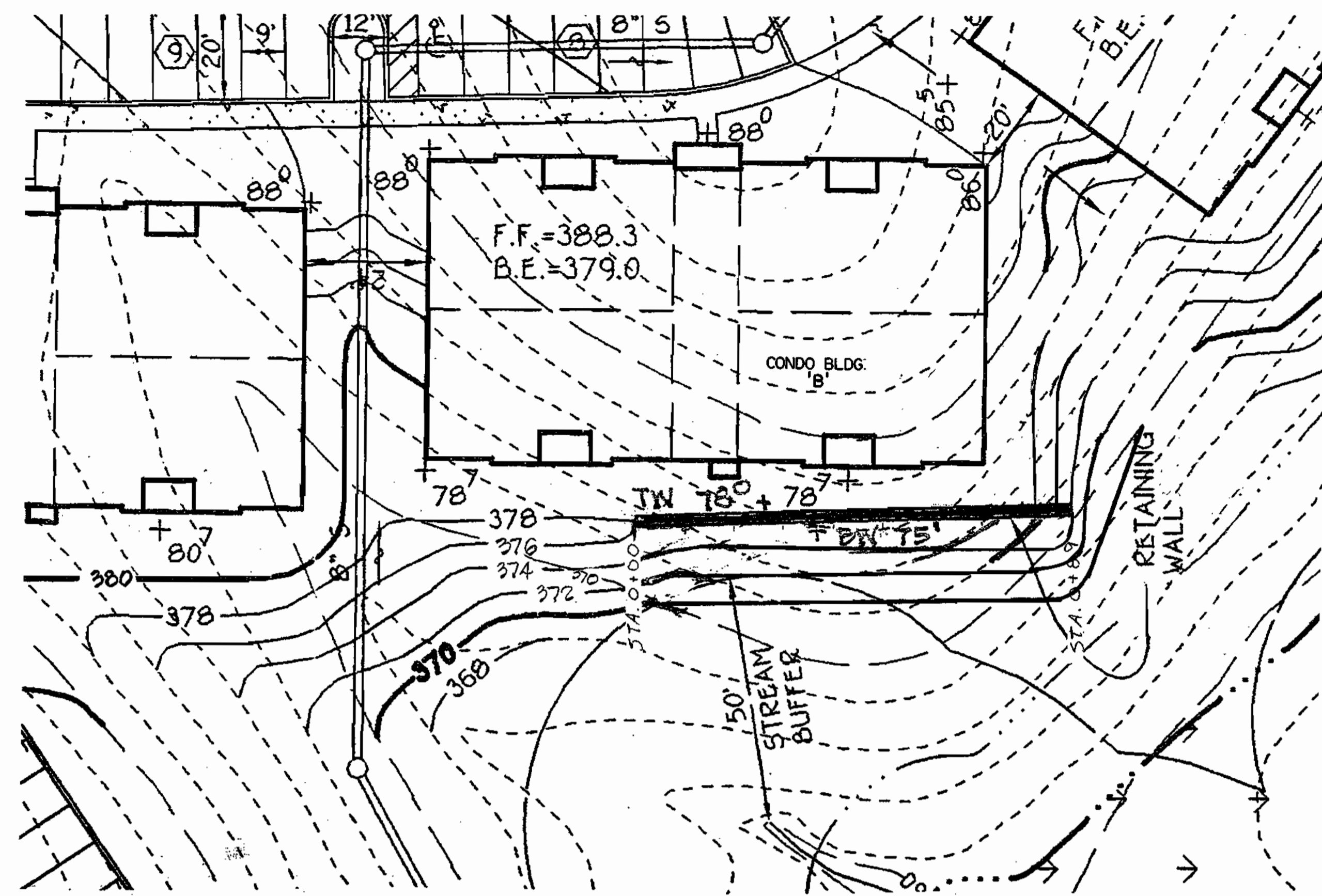
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DEED REF.: 5609 / 611
BLOCK NO.: 3
ZONE: POR
TAX/ZONE: 37
ELEC. DIST.: 1st
CENSUS TR.: 6030
WATER CODE: D 04
SEWER CODE: 2610000

SOIL BORINGS AND DETAILS

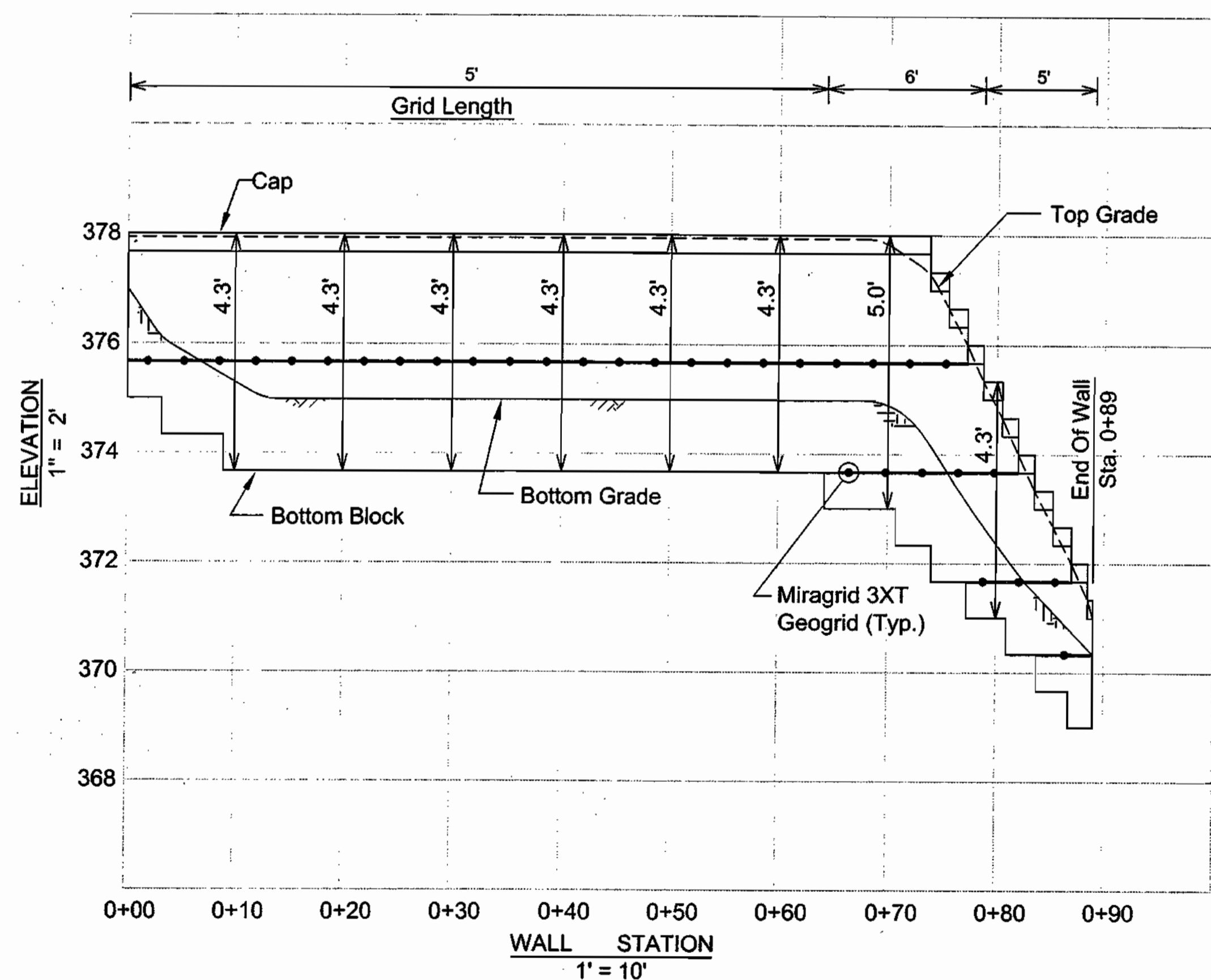
THE COURTYARDS AT THE TIMBERS
TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

ZONED: POR
FIRST ELECTION DISTRICT: HOWARD COUNTY, MARYLAND
SCALE: AS SHOWN
DATE: AUGUST 21, 2002

TAX MAP No: 37
PARCEL No: 617
GRID No: 3
SCALE: AS SHOWN
DATE: AUGUST 21, 2002
SHEET 22 OF 24
SDP 02-55



RETAINING WALL LOCATION PLAN
1" = 20'



WALL ELEVATION
1" = 10'

SPECIFICATIONS
KEYSTONE MODULAR CONCRETE BLOCK RETAINING WALL

PART 1: GENERAL

- 1.01 Description**
- A. Work shall consist of furnishing and construction of a KEYSTONE Retaining Wall System in accordance with these specifications and in reasonably close conformity with the lines, grades, design, and dimensions shown on the plans.
 - B. Work includes preparing foundation soil, furnishing and installing leveling pad, unit drainage fill and backfill to the lines and grades shown on the construction drawings.
 - C. Work includes furnishing and installing geogrid soil reinforcement of the type, size, location, and lengths designated on the construction drawings.

1.02 Delivery, Storage and Handling

- A. Contractor shall check all materials upon delivery to assure that the proper type, grade, color, and certification has been received.
- B. Contractor shall protect all materials from damage due to job site conditions and in accordance with manufacturer's recommendations. Damaged materials shall not be incorporated into the work.

PART 2: PRODUCTS

2.01 Modular Concrete Retaining Wall Units

- A. Modular concrete units shall conform to the following architectural requirements:
 - Face color - concrete gray - standard manufacturer's color may be specified by the Owner.
 - Face finish - sculptured rock face in angular tri-planer configuration. Other face finishes will be allowed with written approval of Owner.
 - Bond configuration - running with bonds nominally located at midpoint of vertically adjacent units, in both straight and curved alignments.
 - Exposed surfaces of units shall be free of chips, cracks or other imperfections when viewed from a distance of 10 feet under diffused lighting.
- B. Modular concrete materials shall conform to the requirements of ASTM C1372 - Standard Specifications for Segmental Retaining Wall Units.
- C. Modular concrete units shall conform to the following structural and geometric requirements measured in accordance with appropriate references:
 - Compressive strength = 3000 psi minimum;
 - Absorption = 8% maximum
 - Dimensional tolerances = ± 1/8" from nominal unit dimensions not including rough split unit dimensions, ± 1/16" unit height - top and bottom planes;
 - Unit size - As specified on plan.

- D. Modular concrete units shall conform to the following constructability requirements:
 - Vertical setback = 1/8"± per course (near vertical) or 1"± per course per the design;
 - Alignment and grid positioning mechanism - fiberglass pins, two per unit minimum;
 - Maximum gap between erected units shall be 1/2 inch.

2.02 Shear Connectors

- A. Shear connectors shall be 1/2 inch diameter fibroresin polyester resin-impregnated fiberglass reinforcement rods or equivalent to provide connection between vertically and horizontally adjacent units. Strength of shear connectors between vertical adjacent units shall be applicable over a design temperature of 10 degrees F to + 100 degrees F.
- B. Shear connectors shall be capable of holding the geogrid in the proper design position during grid pre-tensioning and backfilling.

2.03 Base Leveling Pad Material

- A. Material shall consist of a compacted #57 crushed stone base as shown on the construction drawings.

2.04 Unit Drainage Fill

- A. Unit drainage fill shall consist of #57 crushed stone
- B. One cubic foot, minimum, of drainage fill shall be used for each square foot of wall face. Drainage fill shall be placed within cores of, between, and behind units to meet this requirement.

2.05 Reinforced Backfill

- A. Reinforced backfill shall be type SM, be free of debris and meet the following gradation tested in accordance with ASTM D-422 and meet other properties shown on the plan:

Sieve Size	Percent Passing
2 inch	100-75
3/4 inch	100-75
No. 40	0-60
No. 200	0-35
- B. Plasticity Index (PI) <15 and Liquid Limit <40 per ASTM D-4318.
- C. Material can be site excavated soils where the above requirements can be met. Unsuitable soils for backfill (high plastic clays or organic soils) shall not be used in the reinforced soil mass.

2.06 Geogrid Soil Reinforcement

- A. Geosynthetic reinforcement shall consist of geogrids manufactured specifically for soil reinforcement

applications and shall be manufactured from high tenacity polyester yarn, or HDPE material.

- 2.07 Drainage Pipe
 - A. The drainage pipe shall be perforated corrugated HDPE pipe manufactured in accordance with ASTM D-1248.

PART 3: EXECUTION

3.01 Excavation

- A. Contractor shall excavate to the lines and grades shown on the construction drawings. Owner's representative shall be responsible for inspecting and approving the excavation prior to placement of leveling material or fill soils.

3.02 Base Leveling Pad

- A. Leveling pad material shall be placed to the lines and grades shown on the construction drawings, to a minimum thickness of 6 inches and extend laterally a minimum of 6" in front and behind the modular wall unit.
- B. Leveling pad shall be prepared to insure full contact to the base surface of the concrete units.

3.03 Modular Unit Excavation

- A. First course of units shall be placed on the leveling pad at the appropriate line and grade. Alignment and level shall be checked in all directions and insure that all units are in full contact with the base and properly seated.
- B. Place the front of units side-by-side. Do not leave gaps between adjacent units. Layout of corners and curves shall be in accordance with manufacturer's recommendations.
- C. Install shear/connecting devices per manufacturer's recommendations.
- D. Place and compact drainage fill within and behind wall units. Place and compact backfill soil behind drainage fill.
- E. Maximum stacked vertical height of wall units, prior to unit drainage fill and backfill placement and compaction, shall not exceed manufacturer's recommendations.

3.04 Structural Geogrid Installation

- A. Geogrid shall be oriented with the highest strength axis perpendicular to the wall alignment.
- B. Geogrid reinforcement shall be placed at the strengths, lengths, and elevations shown on the construction design drawings or as directed by the Engineer.
- C. The geogrid shall be laid horizontally on compacted backfill and attached to the modular wall units. Place the next course of modular concrete units over the geogrid. The geogrid shall be pulled taut, and anchored prior to backfill placement on the geogrid.

- D. Geogrid reinforcements shall be continuous throughout their embedment lengths and placed side-by-side to provide 100% coverage at each level. Spliced connections between shorter pieces of geogrid or gaps between adjacent pieces of geogrid are not permitted.

3.05 Reinforced Backfill Placement

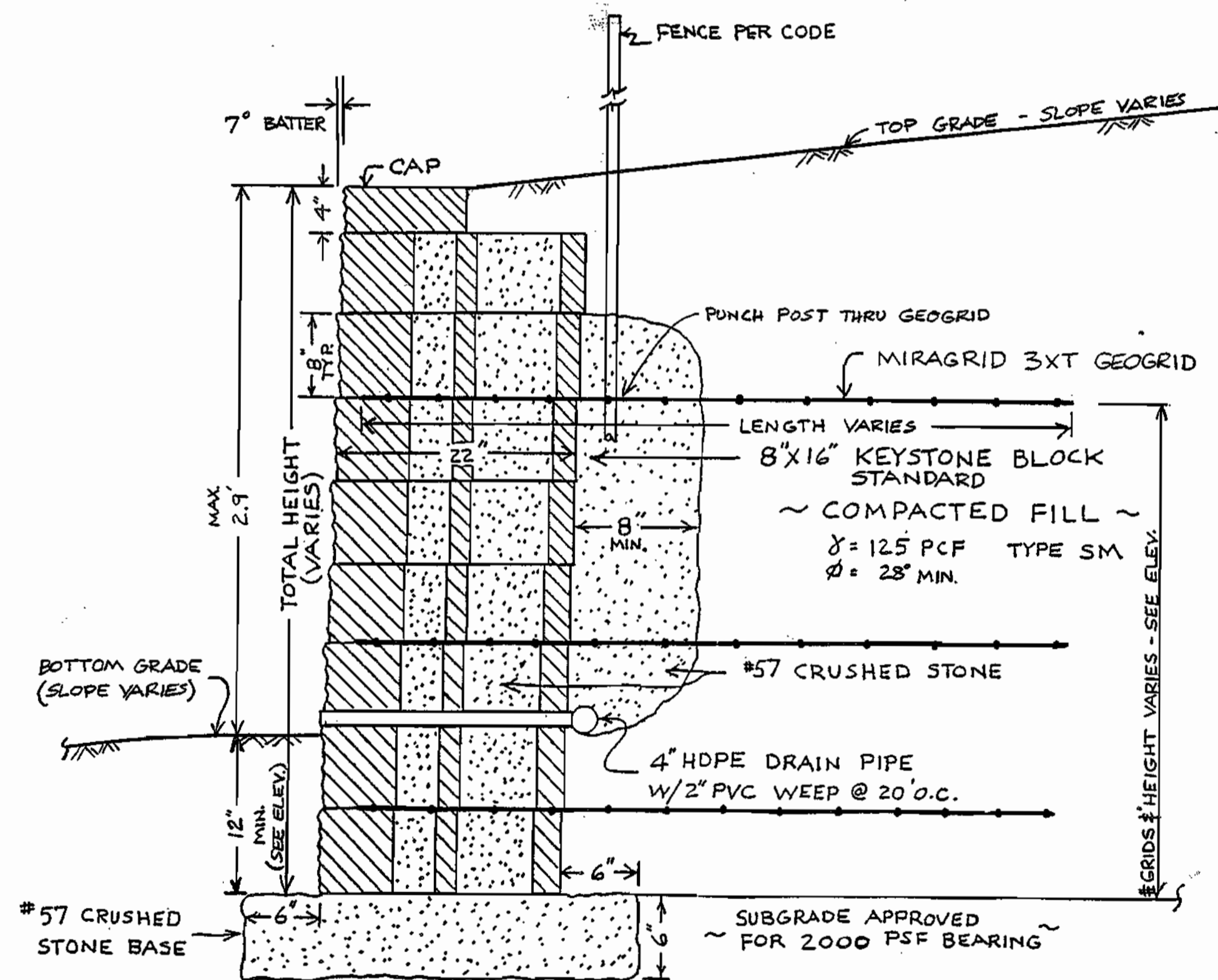
- A. Reinforced backfill shall be placed, spread, and compacted in such a manner that minimizes the development of stack in the geogrid and installation damage.
- B. Reinforced backfill shall be placed and compacted in lifts not to exceed 6 inches where hand compaction is used, or 8 - 10 inches where heavy compaction equipment is used. Lift thickness shall be decreased to achieve the required density as required.
- C. Reinforced backfill shall be compacted to 95% of the maximum density as determined by ASTM D698. The moisture content of the backfill material prior to and during compaction shall be uniformly distributed throughout each layer and shall be + 3% to - 3% of optimum.
- D. Only lightweight hand-operated equipment shall be allowed within 3 feet from the tail of the modular concrete unit.
- E. Tracked construction equipment shall not be operated directly upon the geogrid reinforcement. A minimum fill thickness of 6 inches is required prior to operation of tracked vehicles over the geogrid. Tracked vehicle turning should be kept to a minimum to prevent tracks from displacing the fill and damaging the geogrid.
- F. Rubber tired equipment may pass over geogrid reinforcement at slow speeds, less than 10 MPH. Sudden braking and sharp turning shall be avoided.
- G. At the end of each day's operation, the Contractor shall slope the last lift of reinforced backfill away from the wall units to direct runoff away from wall face. The Contractor shall not allow surface runoff from adjacent areas to enter the wall construction site.

3.06 Cap Installation

- A. Cap units shall be glued to underlying units with an all-weather adhesive recommended by the manufacturer.

3.07 Field Quality Control

- A. The Owner shall engage inspection and testing services, including independent laboratories, to provide quality assurance and testing services during construction.
- B. As a minimum, quality assurance testing should include foundation soil inspection, soil bearing and backfill testing, verification of design parameters, and observation of construction for general compliance with design drawings and specifications.



TYPICAL WALL SECTION
N.T.S.

APPROVED: DEPARTMENT OF PLANNING AND ZONING				10/3/02	
Director - Department of Planning and Zoning				Date	
Chief, Division of Land Development				10/3/02	
Chief, Development Engineering Division				Date	
SUBDIVISION		SECTION/AREA		LOT NO.	
THE COURTYARDS AT THE TIMBERS					
PLAT NO.	BLOCK NO.	ZONE	TAX/ZONE	ELEC. DIST.	CENSUS TR.
	3	POR	07	1ST	6030
WATER CODE			SEWER CODE		
DEVELOPER/OWNER			BUILDER		
LAND DESIGN AND DEVELOPMENT, L.L.C.			RYAN HOPES, LLC		
800 MAIN STREET			11460 CROFTBRIDGE DRIVE #128		
ELLCOTT CITY, MARYLAND 20843			OWINGS HILLS, MD. 21117		



HILLIS-CARNES ENGINEERING ASSOCIATES, INC.

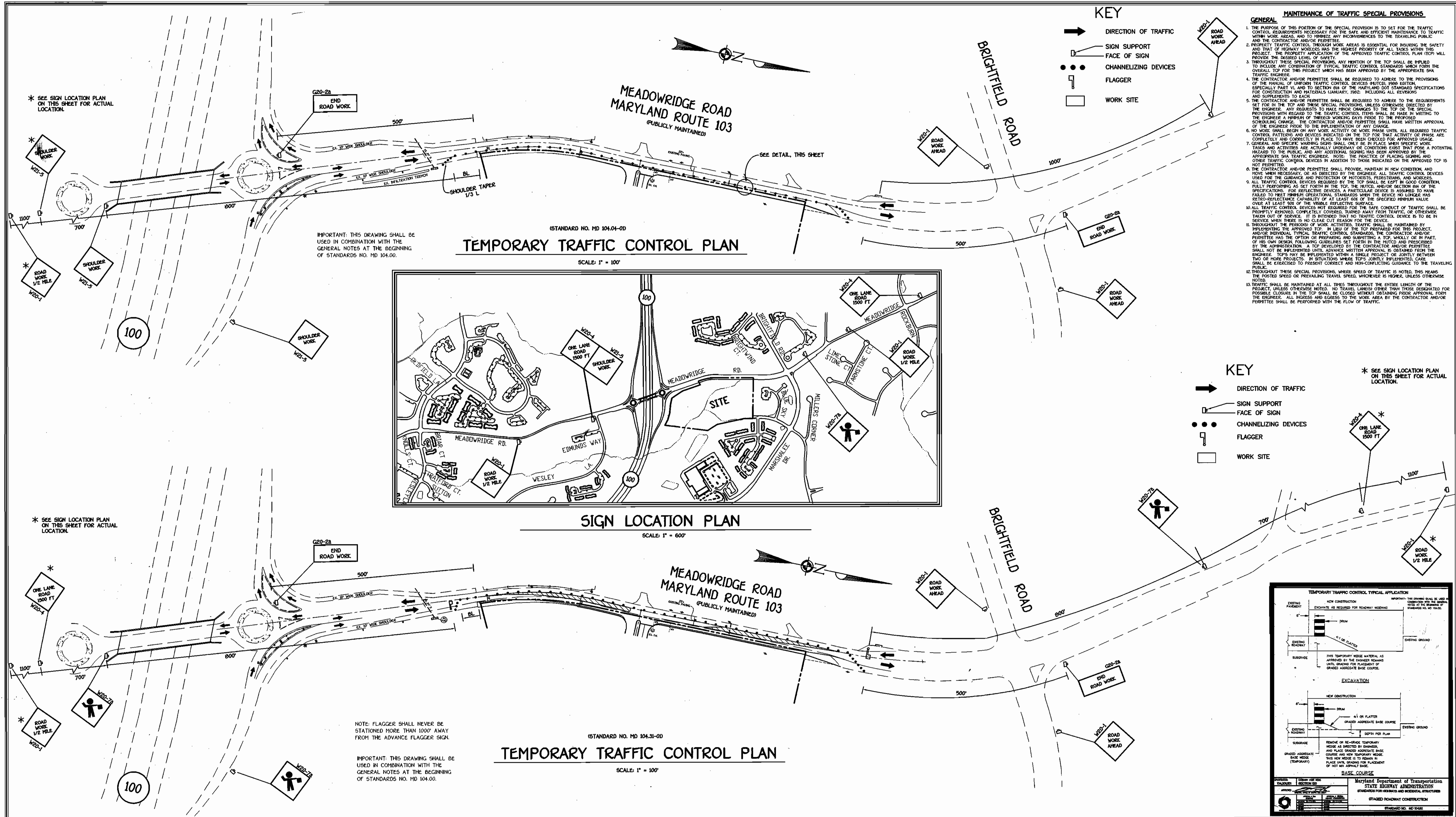
12011 Guilford Road - Suite 106
(410) 880-4788

Annapolis Junction, Maryland
Fax: (410)880-4098

JOB NUMBER: 00188-A
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DATE: 9/7/01
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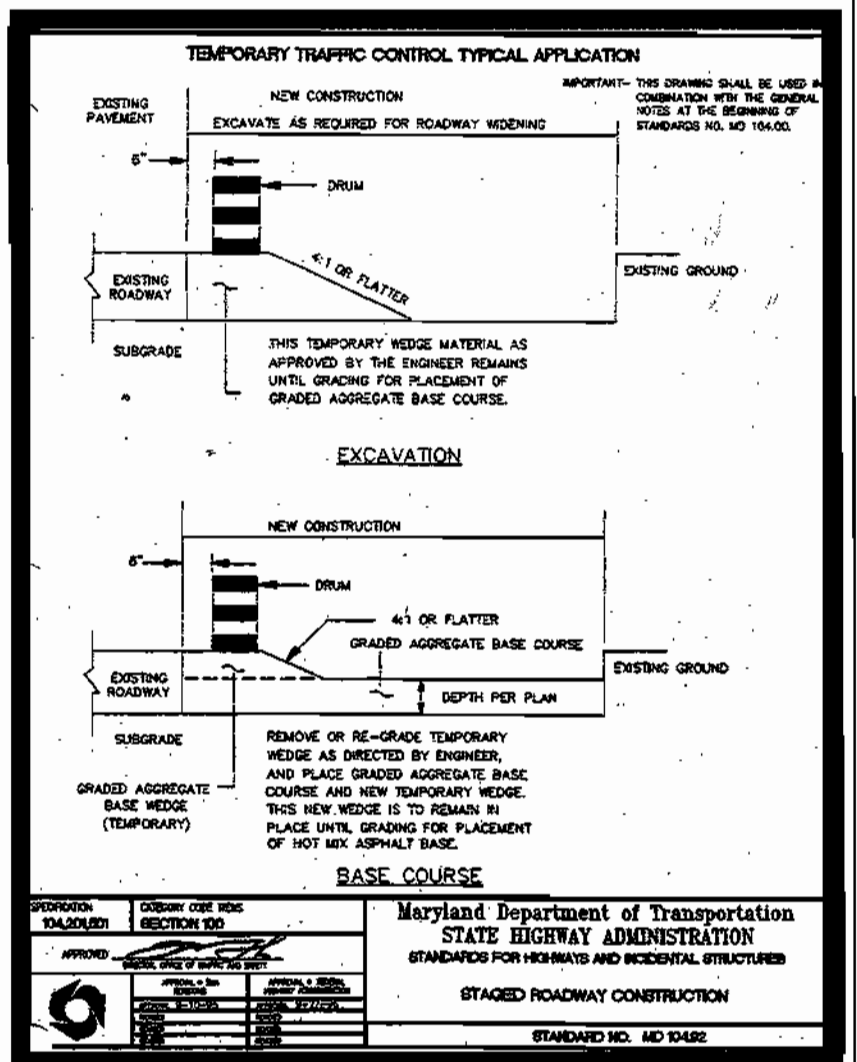
DESIGNED BY: RWS
DRAWN BY: AM
APPROVED BY: RMH
REVISED DATE:

RETAINING WALL CONSTRUCTION DETAIL
THE COURTYARDS AT THE TIMBERS HOWARD COUNTY, MARYLAND
SHEET 23 OF 24



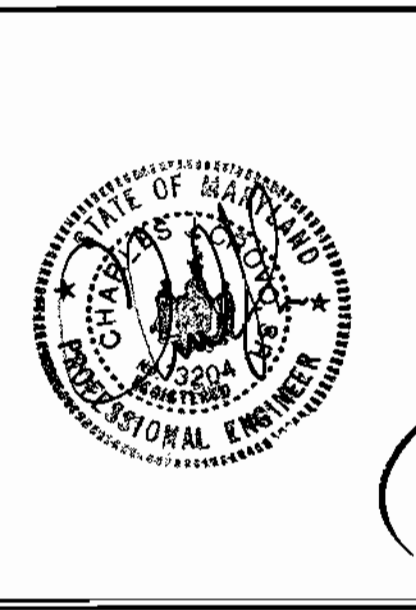
- KEY**
- DIRECTION OF TRAFFIC
 - SIGN SUPPORT
 - FACE OF SIGN
 - CHANNELIZING DEVICES
 - FLAGGER
 - WORK SITE
- MAINTENANCE OF TRAFFIC SPECIAL PROVISIONS**
- GENERAL**
- THE PURPOSE OF THIS PORTION OF THE SPECIAL PROVISIONS IS TO SET FOR THE TRAFFIC CONTROL REQUIREMENTS NECESSARY FOR THE SAFE AND EFFICIENT MAINTENANCE TO TRAFFIC WITHIN WORK AREAS, AND TO MINIMIZE ANY INTERFERENCE TO THE TRAVELING PUBLIC.
 - PROPER TRAFFIC CONTROL THROUGH WORK AREAS IS ESSENTIAL FOR INSURING THE SAFETY AND THAT OF HIGHWAY WORKERS HAS THE HIGHEST PRIORITY OF ALL TASKS WITHIN THIS PROJECT. THE PROPER APPLICATION OF THE APPROVED TRAFFIC CONTROL PLAN MUST PROVIDE THE DESIRED LEVEL OF SAFETY.
 - THROUGHOUT THESE SPECIAL PROVISIONS, ANY PORTION OF THE T.C.P. SHALL BE HELD TO INCLUDE ANY COMBINATION OF TYPICAL TRAFFIC CONTROL STANDARDS WHICH FORM THE OVERALL T.C.P. FOR THIS PROJECT WHICH HAS BEEN APPROVED BY THE APPROPRIATE SHA TRAFFIC ENGINEER.
 - THE CONTRACTOR AND/OR PERMITTEE SHALL BE REQUIRED TO ADHERE TO THE PROVISIONS OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES, 1998 EDITION, ESPECIALLY PART VI AND SECTION 811 OF THE HANDBOOK OF STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, LATEST EDITION, INCLUDING ALL ADDENDUMS AND SUPPLEMENTS TO EACH.
 - THE CONTRACTOR AND/OR PERMITTEE SHALL BE REQUIRED TO ADHERE TO THE REQUIREMENTS SET FORTH IN THE T.C.P. AND THESE SPECIAL PROVISIONS, UNLESS OTHERWISE DIRECTED BY THE ENGINEER. ANY REQUESTS TO MAKE MINOR CHANGES TO THE T.C.P. OR THE SPECIAL PROVISIONS WITH REGARD TO THE TRAFFIC CONTROL ITEMS SHALL BE MADE IN WRITING TO THE ENGINEER A MINIMUM OF THREE WORKING DAYS PRIOR TO THE PROPOSED SCHEDULING CHANGE. THE CONTRACTOR AND/OR PERMITTEE SHALL HAVE WRITTEN APPROVAL OF THE ENGINEER PRIOR TO THE IMPLEMENTATION OF ANY CHANGE.
 - NO WORK SHALL BEGIN ON ANY WORK ACTIVITY OR WORK PHASE UNTIL ALL REQUIRED TRAFFIC CONTROL PATTERNS AND DEVICES INDICATED ON THE T.C.P. FOR THAT ACTIVITY OR PHASE ARE COMPLETELY AND CORRECTLY IN PLACE TO HAVE BEEN CHECKED FOR PROPER USAGE.
 - GENERAL AND SPECIFIC WARNING SIGNS SHALL ONLY BE IN PLACE WHEN SPECIFIC WORK TASKS AND ACTIVITIES ARE ACTUALLY UNDERWAY OR CONDITIONS EXIST THAT POSE A POTENTIAL HAZARD TO THE PUBLIC, AND ANY ADDITIONAL SIGNING HAS BEEN APPROVED BY THE APPROPRIATE SHA TRAFFIC ENGINEER. NOTE: THE PRACTICE OF PLACING SIGNING AND OTHER TRAFFIC CONTROL DEVICES IN ADDITION TO THOSE INDICATED ON THE APPROVED T.C.P. IS NOT PERMITTED.
 - THE CONTRACTOR AND/OR PERMITTEE SHALL PROVIDE, MAINTAIN IN NEW CONDITION, AND REMOVE WHEN NECESSARY, OR AS DIRECTED BY THE ENGINEER, ALL TRAFFIC CONTROL DEVICES USED FOR THE GUIDANCE AND PROTECTION OF MOTORISTS, PEDESTRIANS, AND WORKERS.
 - ALL TRAFFIC CONTROL DEVICES REQUIRED BY THE T.C.P. SHALL BE KEPT IN GOOD CONDITION, FULLY PERFORMING AS SET FORTH IN THE T.C.P. THE T.C.P. AND/OR SECTION 811 OF THE SPECIFICATIONS, FOR REFLECTIVE DEVICES, A PARTICULAR DEVICE IS ASSUMED TO HAVE FAILED TO MEET MINIMUM OPERATIONAL STANDARDS WHEN THE DEVICE NO LONGER HAS RETRO-REFLECTANCE CAPABILITY OF AT LEAST 60% OF THE SPECIFIED MINIMUM VALUE ORS AT LEAST 50% OF THE VISIBLE REFLECTIVE SURFACE.
 - ALL TRAFFIC CONTROL DEVICES NOT REQUIRED FOR THE SAFE CONDUCT OF TRAFFIC SHALL BE PROPERLY REMOVED, COMPLETELY COVERED, TURNED AWAY FROM TRAFFIC, OR OTHERWISE TAKEN OUT OF SERVICE. IT IS INTENDED THAT NO TRAFFIC CONTROL DEVICE IS TO BE IN SERVICE WHEN THERE IS NO CLEAR CUT REASON FOR THE DEVICE.
 - THROUGHOUT THE PERIODS OF WORK ACTIVITIES, TRAFFIC SHALL BE MAINTAINED BY IMPLEMENTING THE APPROVED T.C.P. IN LIEU OF THE T.C.P. PREPARED FOR THIS PROJECT, AND/OR INDIVIDUAL TYPICAL TRAFFIC CONTROL STANDARDS, THE CONTRACTOR AND/OR PERMITTEE HAS THE OPTION OF PREPARING AND SUBMITTING A T.C.P. WHOLLY OR IN PART, OF HIS OWN DESIGN FOLLOWING THE GUIDELINES SET FORTH IN THE T.C.P. AND/OR SECTION 811 OF THE SPECIFICATIONS. A T.C.P. DEVELOPED BY THE CONTRACTOR AND/OR PERMITTEE SHALL NOT BE IMPLEMENTED UNTIL ADVANCE WRITTEN APPROVAL IS OBTAINED FROM THE ENGINEER. T.C.P.'S MAY BE IMPLEMENTED WITHIN A SINGLE PROJECT OR JOINTLY BETWEEN TWO OR MORE PROJECTS, PROVIDED THAT THE T.C.P.'S ARE JOINTLY DEVELOPED, SHALL BE EXERCISED TO PRESENT CORRECT AND NON-CONFLICTING GUIDANCE TO THE TRAVELING PUBLIC.
 - THROUGHOUT THESE SPECIAL PROVISIONS, WHERE SPEED OF TRAFFIC IS NOTED, THIS MEANS THE POSTED SPEED OR PREVAILING TRAVEL SPEED, WHICHEVER IS HIGHER, UNLESS OTHERWISE NOTED.
 - TRAFFIC SHALL BE MAINTAINED AT ALL TIMES THROUGHOUT THE ENTIRE LENGTH OF THE PROJECT, UNLESS OTHERWISE NOTED. NO TRAVEL LANE(S) OTHER THAN THOSE DESIGNATED FOR POSSIBLE CLOSURE IN THE T.C.P. SHALL BE CLOSED WITHOUT OBTAINING PRIOR APPROVAL FROM THE ENGINEER. ALL PROGRESS AND ACCESS TO THE WORK AREA BY THE CONTRACTOR AND/OR PERMITTEE SHALL BE PERFORMED WITH THE FLOW OF TRAFFIC.

- KEY**
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 - ALL TRAFFIC CONTROL DEVICES REQUIRED BY THE T.C.P. SHALL BE KEPT IN GOOD CONDITION, FULLY PERFORMING AS SET FORTH IN THE T.C.P. THE T.C.P. AND/OR SECTION 811 OF THE SPECIFICATIONS, FOR REFLECTIVE DEVICES, A PARTICULAR DEVICE IS ASSUMED TO HAVE FAILED TO MEET MINIMUM OPERATIONAL STANDARDS WHEN THE DEVICE NO LONGER HAS RETRO-REFLECTANCE CAPABILITY OF AT LEAST 60% OF THE SPECIFIED MINIMUM VALUE ORS AT LEAST 50% OF THE VISIBLE REFLECTIVE SURFACE.
 - ALL TRAFFIC CONTROL DEVICES NOT REQUIRED FOR THE SAFE CONDUCT OF TRAFFIC SHALL BE PROPERLY REMOVED, COMPLETELY COVERED, TURNED AWAY FROM TRAFFIC, OR OTHERWISE TAKEN OUT OF SERVICE. IT IS INTENDED THAT NO TRAFFIC CONTROL DEVICE IS TO BE IN SERVICE WHEN THERE IS NO CLEAR CUT REASON FOR THE DEVICE.
 - THROUGHOUT THE PERIODS OF WORK ACTIVITIES, TRAFFIC SHALL BE MAINTAINED BY IMPLEMENTING THE APPROVED T.C.P. IN LIEU OF THE T.C.P. PREPARED FOR THIS PROJECT, AND/OR INDIVIDUAL TYPICAL TRAFFIC CONTROL STANDARDS, THE CONTRACTOR AND/OR PERMITTEE HAS THE OPTION OF PREPARING AND SUBMITTING A T.C.P. WHOLLY OR IN PART, OF HIS OWN DESIGN FOLLOWING THE GUIDELINES SET FORTH IN THE T.C.P. AND/OR SECTION 811 OF THE SPECIFICATIONS. A T.C.P. DEVELOPED BY THE CONTRACTOR AND/OR PERMITTEE SHALL NOT BE IMPLEMENTED UNTIL ADVANCE WRITTEN APPROVAL IS OBTAINED FROM THE ENGINEER. T.C.P.'S MAY BE IMPLEMENTED WITHIN A SINGLE PROJECT OR JOINTLY BETWEEN TWO OR MORE PROJECTS, PROVIDED THAT THE T.C.P.'S ARE JOINTLY DEVELOPED, SHALL BE EXERCISED TO PRESENT CORRECT AND NON-CONFLICTING GUIDANCE TO THE TRAVELING PUBLIC.
 - THROUGHOUT THESE SPECIAL PROVISIONS, WHERE SPEED OF TRAFFIC IS NOTED, THIS MEANS THE POSTED SPEED OR PREVAILING TRAVEL SPEED, WHICHEVER IS HIGHER, UNLESS OTHERWISE NOTED.
 - TRAFFIC SHALL BE MAINTAINED AT ALL TIMES THROUGHOUT THE ENTIRE LENGTH OF THE PROJECT, UNLESS OTHERWISE NOTED. NO TRAVEL LANE(S) OTHER THAN THOSE DESIGNATED FOR POSSIBLE CLOSURE IN THE T.C.P. SHALL BE CLOSED WITHOUT OBTAINING PRIOR APPROVAL FROM THE ENGINEER. ALL PROGRESS AND ACCESS TO THE WORK AREA BY THE CONTRACTOR AND/OR PERMITTEE SHALL BE PERFORMED WITH THE FLOW OF TRAFFIC.



FISHER, COLLINS & CARTER, INC.
 CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS
 CENTENNIAL SQUARE OFFICE PARK - 10222 BALDWIN NATIONAL FREE
 ELLICOTT CITY, MARYLAND 21114
 410-461-3200

DATE	DESCRIPTION	REVISION BLOCK



ENGINEER'S CERTIFICATE
 I certify that this plan for sediment and erosion control represents a practical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District.

CHARLES J. CROVO, SR., P.E., L.S.
 Signature of Engineer (print name below signature) Date: 8/22/02

DEVELOPER'S CERTIFICATE
 I/We certify that all development and construction will be done according to this plan for sediment and erosion control, and that all responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I also authorize periodic on-site inspection by the Howard Soil Conservation District.

DONALD R. REUWER, JR.
 Signature of Developer (print name below signature) Date: 8/22/02

DEVELOPER/OWNER
 2RSK, L.L.C.
 c/o LAND DESIGN AND DEVELOPMENT, INC.
 SUITE 1225
 ELLICOTT CITY, MARYLAND 21143

BUILDER
 RYAN HOMES
 11460 CROWNDRIVE DRIVE
 SUITE 1225
 OWINGS HILLS, MARYLAND 21117

APPROVED: DEPARTMENT OF PLANNING AND ZONING

Director: Department of Planning and Zoning Date: 10/2/02
 Chief, Division of Land Development Date: 10/15/02
 Chief, Development Engineering Division Date: 9/20/02

SUBDIVISION	SECTION/AREA	PARCEL NO.
THE COURTYARDS AT THE TIMBERS		617
DEED REF. 5609 / 611	BLOCK NO. 3	ZONE POR
TAX/ZONE 37	ELEC. DIST. 1st.	CENSUS TR. 6030
WATER CODE D 04	SEWER CODE 2610000	

TRAFFIC CONTROL PLANS
THE COURTYARDS AT THE TIMBERS
 TOWNHOUSE CONDOMINIUMS & CONDOMINIUM BUILDINGS

ZONED: POR
 TAX MAP No.: 37 PARCEL No.: 617 GRID No.: 3
 FIRST ELECTION DISTRICT HOWARD COUNTY, MARYLAND
 SCALE: AS SHOWN DATE: AUGUST 21, 2002
 SHEET 24 OF 24 **SDP 02-55**

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