2. The contractor shall notify "Miss Utility" at 1-800-257-7777 at least 48 hours prior to any excavation work.

3. The contractor is to notify the following utilities or agencies at least five days before starting work on these drawings:

1-800-257-7777 (410) 754-6281 (410) 313-2366 393-3553 Verizon Telephone Company: Howard County Bureau of Utilities: AT&T Cable Location Division: 850-4620 B.G.E. Co. Contractor Services: B.G.E. Co. Underground Damage Control: State Highway Administration:

4. Site analysis:

PARCEL 'A': 7.00± AC Present zoning: B-1 Use of structure: Offices Total building area: 98,000 sf

Building coverage on site: 2.250 acs. or 29% of gross area (Includes future building) Paved parking lot/area: 2.28 Ac. or 29% of gross area

There are no steep slopes on-site

Project background: Location: Ellicott City, Md.; Tax Map 37, Parcel 604.

Zoning : B-1 Section/Area : N/A

Site Area: 7.807 Acres OPZ references: F-78-37, WP-02-87, F-03-01

Note: WP-02-87 pertains to the resubdivision of the Carrie Norman property (lots 1,2, and parcel 472), requesting an exemption from sketch and preliminary plan, and to proceed as an "originals only" final subdivision plat. Approval from DPZ was given April 30, 2002, under the condition that the petitioner and the owner of parcel 472 submit a final plan application showing the proposed parcels A & B, and the required road right-of-way dedication. The resubdivision plat will be recorded, "only after" both site development plans (SDP-02-52 and SDP-02-89) are technically complete, or the access points and road improvements have been satisfactorily addressed.

- 6. The contractor or developer shall notify the Department of Public Works/Bureau of Engineering/ Construction Inspection Division at (410) 313-1880 at least 24 hours in advance of commencement of work.
- 7. Any damage to public right-of-ways, paving, or existing utilities will be corrected at the contractor's expense.
- 8. Existing utilities located from Field Surveys and available record drawings. Approximate location of existing utilities are shown for the contractors information. Contractor shall locate existing utilities well in advance of construction activities and take all necessary precautions to protect the existing utilities and to
- maintain uninterrupted service. Any damage incurred due to contractor's operation shall be repaired immediately at the contractor's expense. 9. All reinforced concrete for storm drain structures shall have a minimum of 28 days
- 10. Traffic control devices, markings and signing shall be in accordance with the latest edition of the Manual on Uniform Traffic Control Devices (MUTCD). All street and regulatory signs shall be in place prior to the placement of any asphalt.
- 11. Estimates of earthwork quantities are provided soley for the purpose of calculating fees.
- 12. Soil compaction specifications, requirements, methods and materials are to be in accordance with the recommendations of the project Geotechnical Engineer. Geotechnical Engineer to confirm acceptability of proposed paving section, based on soil test.
- 13. All storm drain pipe bedding shall be Class 'C'. 14. The coordinates shown hereon are based upon the Howard County Geodetic Control which is
- and 43A1 were used for this project. 15. A noise study is not required for this project. 16. Existing topography is based on field run information performed by Frederick Ward & Associates, Inc.

based upon the Maryland State Plane Coordinate System. Howard County Monument Nos. 37GB

- 17. See sheet 11 for paying section details. Paying section to be confirmed by geotechnical engineer
- 18. All curb and gutter to be Howard County Standard concrete Detail R3.01 unless otherwise specified.
- 19. There are no wetlands, streams, floodplains, or their buffer's located within the limit of disturbance.
- 20. Where drainage flows away from curb, contractor to reverse the gutter pan.
- 21. All elevations are to flowline/bottom of curb unless otherwise noted.
- 22. All dimensions are to face of curb unless otherwise noted.
- 23. Contractor to connect roof drains to storm drain system, as shown.
- 24. Contractor to sod all areas within 10' of proposed building. All other areas to be seeded and mulched.
- 25. Proposed Water Main to to be public.

Existing Conditions and Demolition Plan
Site Layout and Utilities Plan
Site Layout and Utilities Plan
Storm Drain Profiles

<u>Stormwater Management Drainage Area Map</u>

Stormwater Management Drainage Area Map

Stormwater Management Details and Specifications
Stormceptor Notes and Details

Forest Stand Delineation and Forest Conservation Notes

CHIEF, DIVISION OF LAND DEVELOPMENT H

Grading, Stormwater Management, Erosion & Sediment Control Plan

Grading, Stormwater Management, Erosion & Sediment Control Plan Bioretention Details and Specifications Erosion & Sediment Control Details and Specifications

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

Storm Drain Drainage Area Map

Storm Drain Profiles
Storm Drain Profiles

Sewer Profiles Sewer Profiles

Site Details

Site Details

Landscape Plan

Retaining Wall Details

Retaining Wall Details

Retaining Wall Details

Bioretention Planting Details

- 26. Stormwater Management in accordance with 2000 Maryland Stormwater Management Manual. Cpv, is provided by a stormwater managment pond onsite. WQv provided partially by a wet extended detention SWM pond the remainder of the WQv and all of the Rev are provided by bioretention and
- 27. All curbs fillets shall be 5' radii, unless otherwise noted on plans.
- 28. APFO Traffic Study performed by the Traffic Group on November 16, 2001.
- 29. All exterior lighting to comply with Section 134 of the Zoning Regulations. See sheet 3 for detail.
- 30. This plan shall comply with the Zoning Regulations as amended by Council Bill 50-2001.

SHEET INDEX

31. There are no historic sites on the subject parcel.

DESCRIPTION

32. The forest conservation obligation of 137214 s.f. will be fulfilled by fee-in-lieu of reforestation in the amount of \$68,607.00.

10/16/02

8

29'-3"

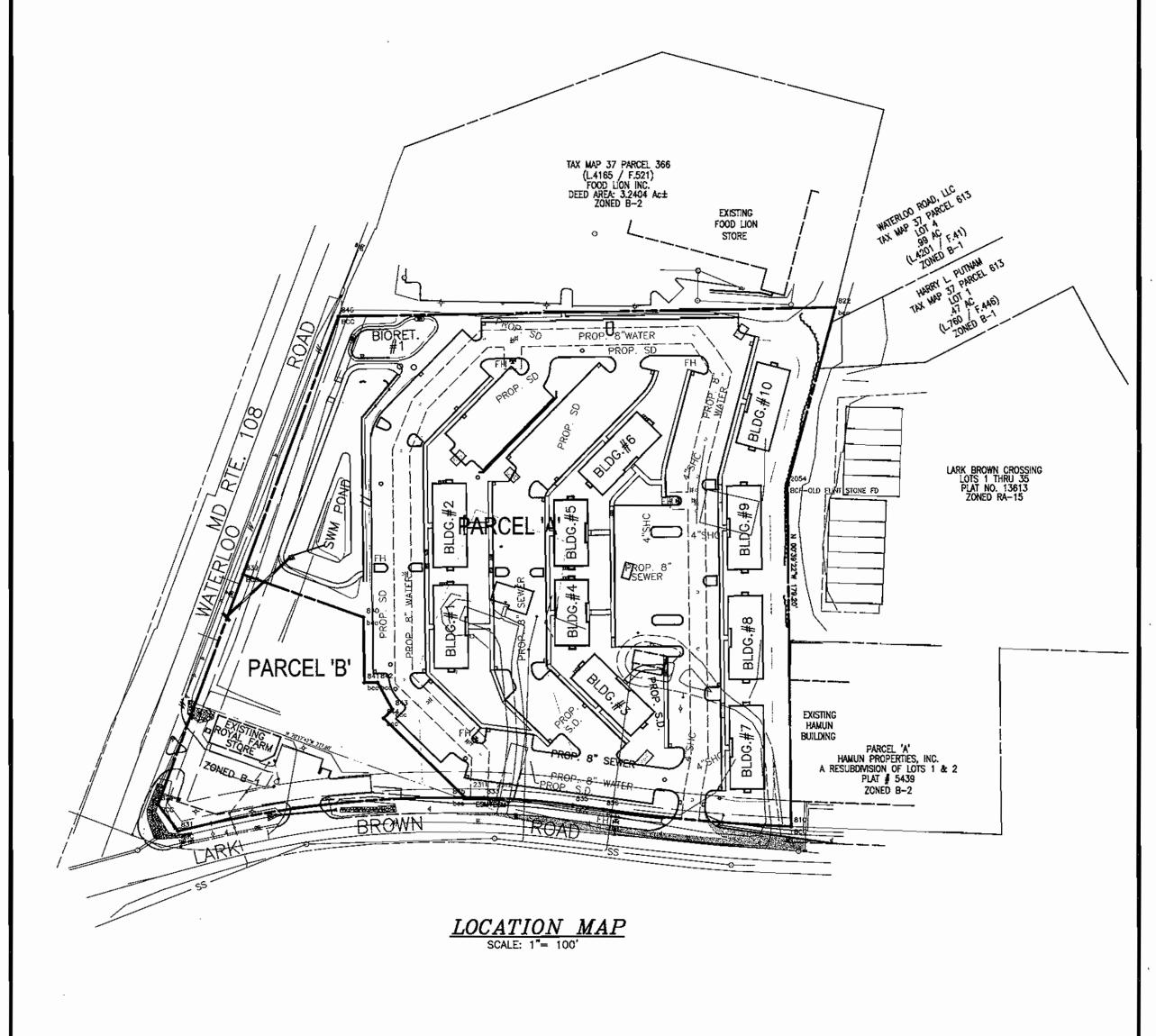
29'-3"

29'--3"

33. Reference WP-02-87, approved April 30, 2002, to waive the requirement to submit a sketch and preliminary plan.

GATEWAY OFFICE PARK PARCEL 'A'

HOWARD COUNTY, MARYLAND



BENCHMARKS

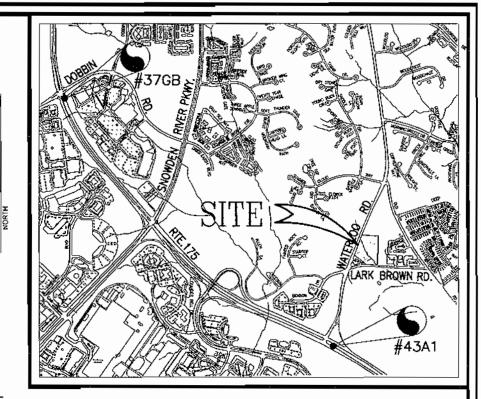
HOWARD COUNTY MONUMENT #37GB N 553452.821 E 1368503.167 ELEV. 325.937 REBAR & CAP - 3.1' FROM NORTH SIDE OF RTE. 175, 6.7' FROM CORNER OF PKG LOT AND 44.9' SW OF INLET.

HOWARD COUNTY MONUMENT #43A1 N 552081.823 E 1370625.811 ELEV. 307.471

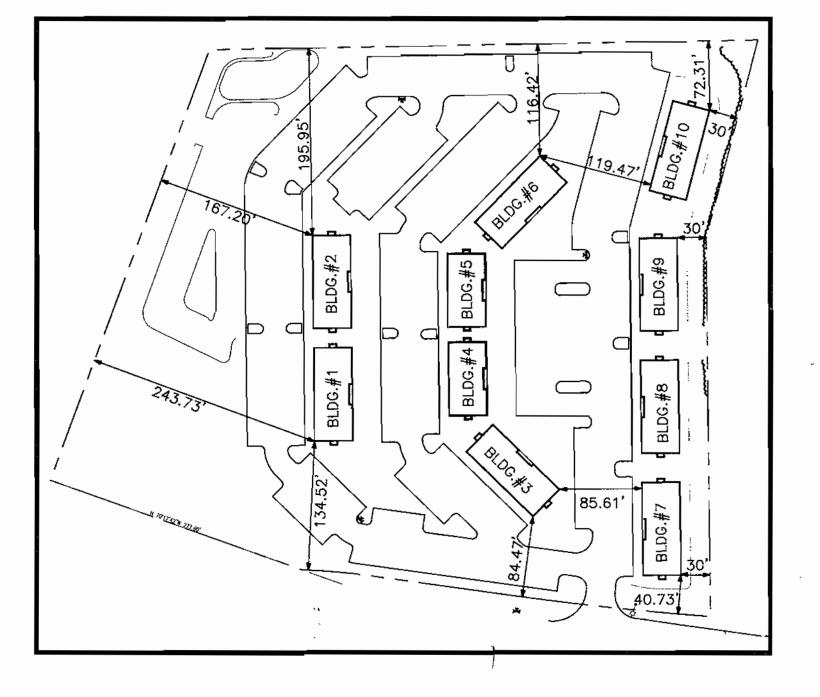
STD. CONC. MON- IN THE MEDIAN OFF RTE. 175, AT THE TURN AROUND LOCATED 0.25 MI. FROM THE TRAFFIC LIGHT AT RTE. 108.

HORIZONTAL CONTROL IS IN NAD83. VERTICAL CONTROL IS IN NGVD29.

<u>LEGE</u>ND Existing Contour Proposed Contour Spot Elevation Direction of Flow



VICINITY MAP SCALE: 1"=2000'



PARKING TABULATION

3.3 SPC. PER 1000 SF OFFICE AREA 98,000 TOTAL SF OFFICES@ 3.3 SPC/1000 SF= 324 SPCS. PARKING PROVIDED: 404 SPCS

INCLUDING: 11 HANDICAP SPCS* * ONE VAN-ACCESSIBLE SPACE

16'-18"

PARCEL NUMBER

3450000

PARCEL 'A'

RIGHT SIDE ELEVATION 3 STORY 4000 S.F.

STREET ADDRESS

8186 Lark Brown Road, Ellicott City, MD 21042

8188 Lark Brown Road, Ellicott City, MD 21042

8178 Lark Brown Road, Ellicott City, MD 21042

8180 Lark Brown Road, Ellicott City, MD 21042

8182 Lark Brown Road, Ellicott City, MD 21042

8184 Lark Brown Road, Ellicott City, MD 21042

8170 Lark Brown Road, Ellicott City, MD 21042

8172 Lark Brown Road, Ellicott City, MD 21042

8174 Lark Brown Road, Ellicott City, MD 21042

8176 Lark Brown Road, Eilicott City, MD 21042

BLOCK NO. ZONE TAX/ZONE ELECT. DIST. CENSUS TI

SECTION/AREA

N/A

SEWER CODE:

ADDRESS CHART

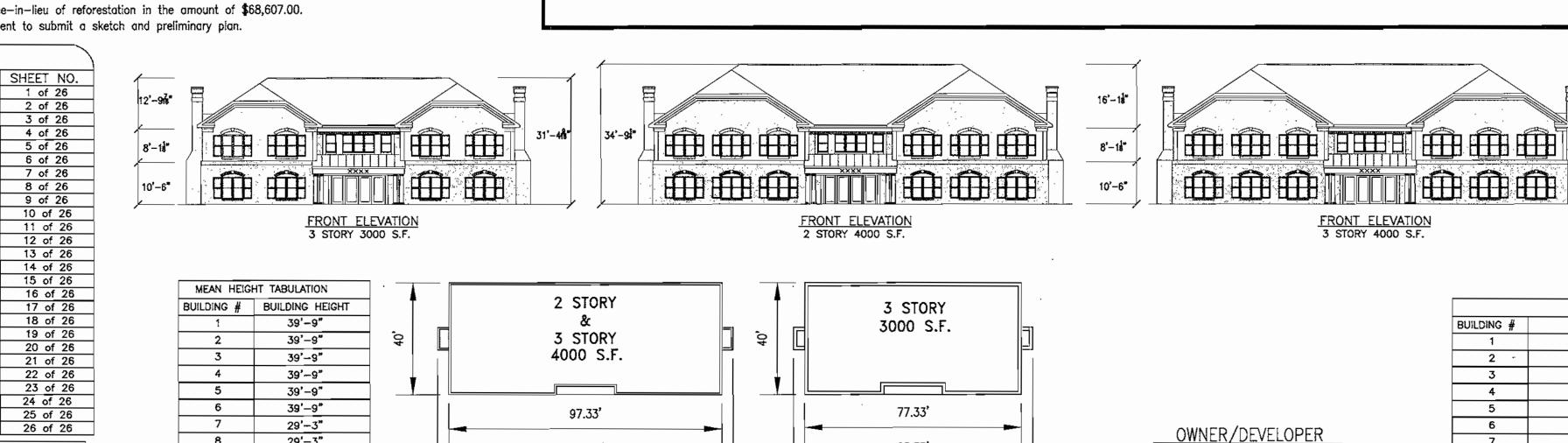
8

WATER CODE:

Gateway Office Park

20

E08



105.33

PLAN VIEW N.T.S.

85.33'

PLAN VIEW N.T.S.

CHARTWELL PROFESSIONAL PARK, L.L.C.

c/o JAMES M. JOST & CO., INC.

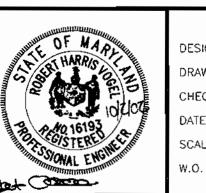
7370 GRACE DRIVE, SUITE A

COLUMBIA, MD 21044

Attn.: MR. JAMES JOST

(443) 535-9200

TITLE SHEET SITE DEVELOPMENT PLAN GATEWAY OFFICE PARK **PARCEL** A RESUBDIVISION OF LOTS 1 & 2, CARRIE NORMAN PROPERTY TAX MAP 37 GRID 20 6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND FREDERICK WARD ASSOCIATES, INC. 7125 Riverwood Drive Columbia, Maryland 21046-2354



SURVEYORS | Bel Air, Maryland

DESIGN BY: CLS DRAWN BY: __JAJ/CLY CHECKED BY: RHV DATE: APR. 19, 2002 AS SHOWN W.O. NO.: 2017165

Columbia, Maryland

Phone: 410-290-9550 Fax: 410-720-6226

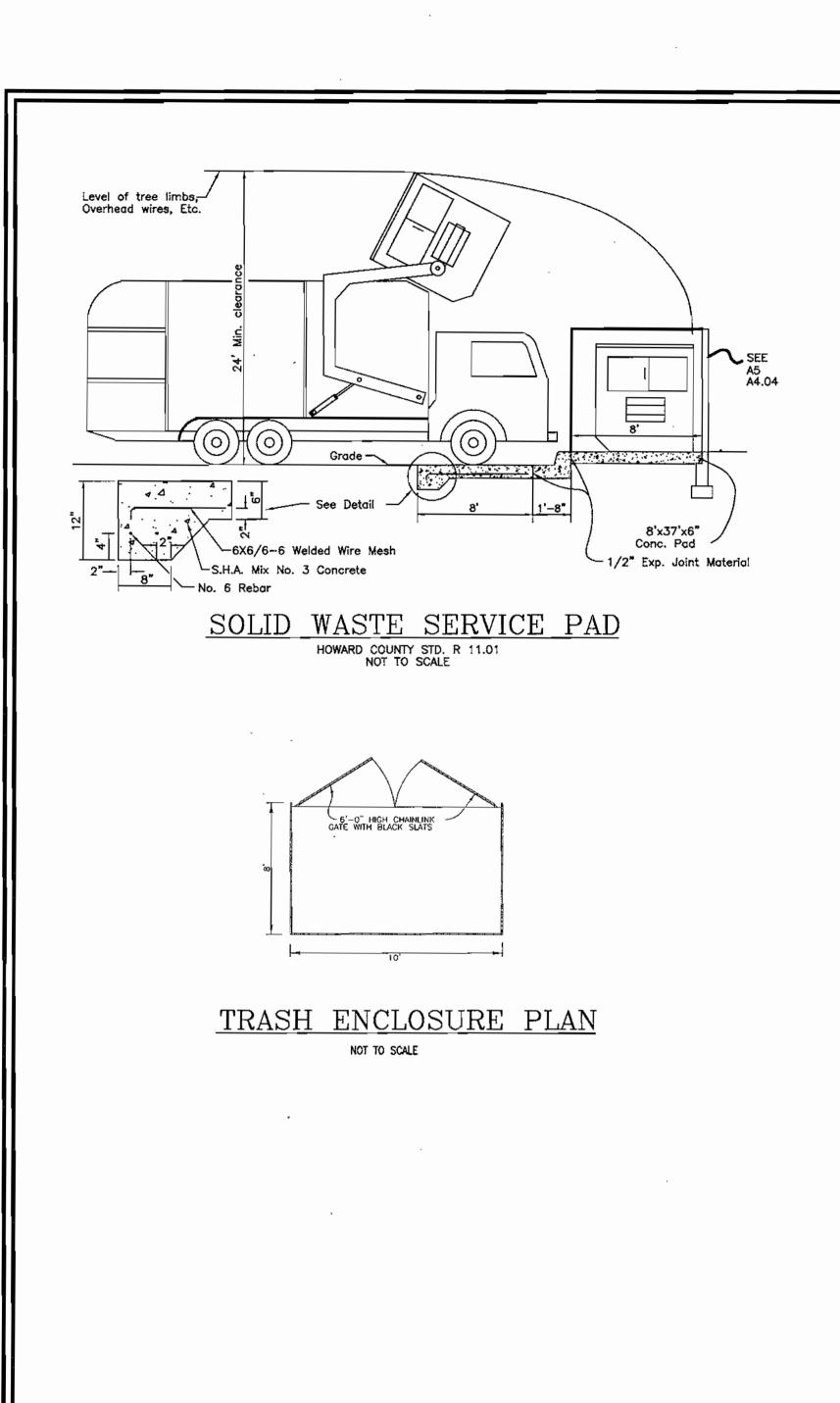
SHEET 26

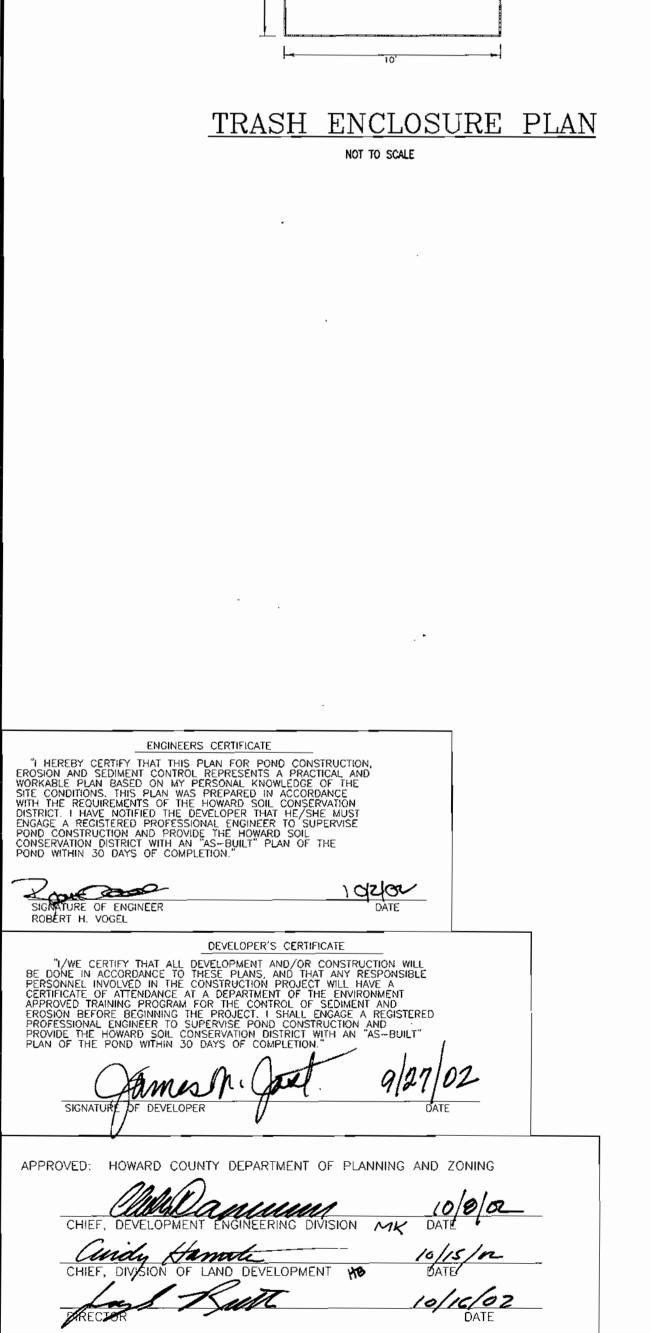
PARCEL 604

Warrenton, Virginia

HOWARD SCD

DATE







NOT TO SCALE

HANDICAP PARKING SIGNS



of the additional sign designating the fine can be no lower than 7'-0"

COMPACTED SUBGRADE

FULL-DEPTH BITUMINOUS CONCRETE ALTERNATE

--- PRIME --- COMPACTED SUBGRADE

GRANULAR BASE ALTERNATE

LARK BROWN ROAD WIDENING PAVING SECTION (P5)

NOT TO SCALE

BITUMINOUS SURFACE COURSE (BAND SC OR SF)

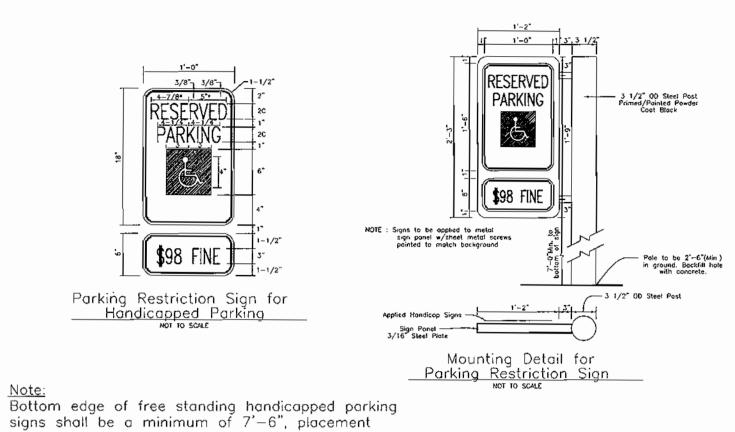
BITUMINOUS CONCRETE BASE

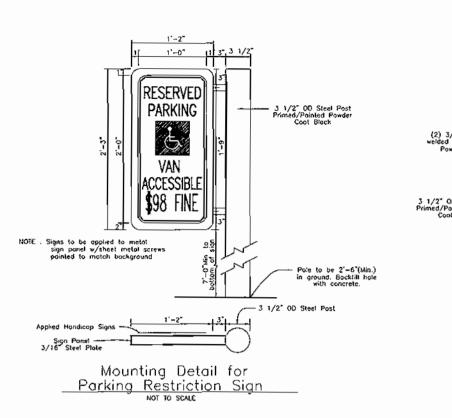
BITUMINOUS CONCRETE BASE COURSE (BAND BF- PLACED -IN TWO EQUAL LAYERS)

BITUMINOUS CONCRETE BASE COURSE (BAND SC OR SF)

BITUMINOUS CONCRETE BASE COURSE (BAND BF- PLACED -IN TWO EQUAL LAYERS)

BITUMINOUS CONCRETE BASE COURSE (BAND BF- PLACED IN TWO EQUAL LAYERS)





- SURFACE COURSE ASPHALT

- SURFACE COURSE ASPHALT

GRADED AGGREGATE BASE

---- BASE COURSE ASPHALT

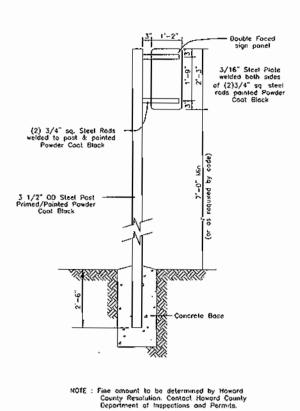
- BASE COURSE ASPHALT GRADED AGGREGATE BASE

LIGHT DUTY PAVING SECTION

HEAVY DUTY PAVING SECTION

SITE PAVING SECTION

NOT TO SCALE



* Reduce Spocing 50%

Legend and Border – Green White Symbol on Blue Background Background – White

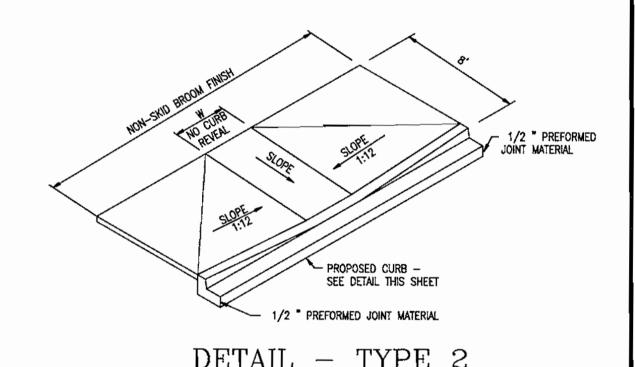
HOWARD COUNTY STD DETAIL R-3.01 NOT TO SCALE 2500 PSI CONCRETE (BROOM FINISH) 6 x 6 WELDED WIRE FABRIC CR-6 OR CR-467 ----- COMPACTED SUBGRADE

> CONCRETE WALK SECTION NOT TO SCALE

MIX 2 CONCRETE

BASE COURSE

<u>DETAIL - CURB AND GUTTER</u>



DETAIL - TYPE 2 SIDEWALK RAMP FENCING MATERIAL - FINISHED GRADE

> TEMPORARY CONSTRUCTION FENCE DETAIL
> NOT TO SCALE

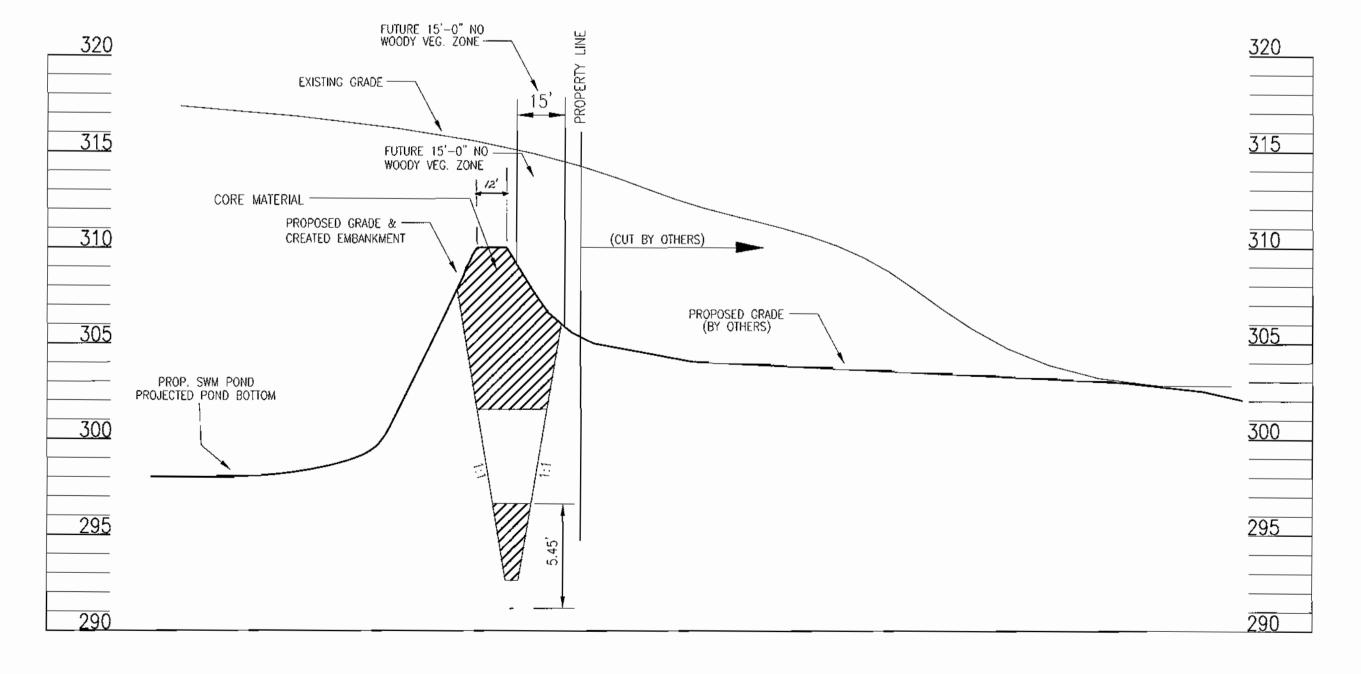


A RESUBDIVISION OF LOTS 1 & 2, CARRIE NORMAN PROPERTY TAX MAP 37 GRID 20 PARCEL 604 6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND

SURVEYORS | Bel Air, Maryland

FREDERICK WARD ASSOCIATES, INC. ENGINEERS | 7125 Riverwood Drive Columbia, Maryland 21046-2354 Phone: 410-290-9550 Fax: 410-720-6226 Columbia, Maryland Warrenton, Virginia

DESIGN BY: ____CLS CHECKED BY: DATE: _APR. 19, 2002 AS SHOWN SCALE: W.O. NO.:



CREATED EMBANKMENT PROFILE

SCALE: (HOR.) 1:30 (VERT.) 1:5

OWNER/DEVELOPER CHARTWELL PROFESSIONAL PARK, L.L.C. c/o JAMES M. JOST & CO., INC. 7370 GRACE DRIVE, SUITE A COLUMBIA, MD 21044 Attn.: MR. JAMES JOST (443) 535-9200

11 SHEET 26

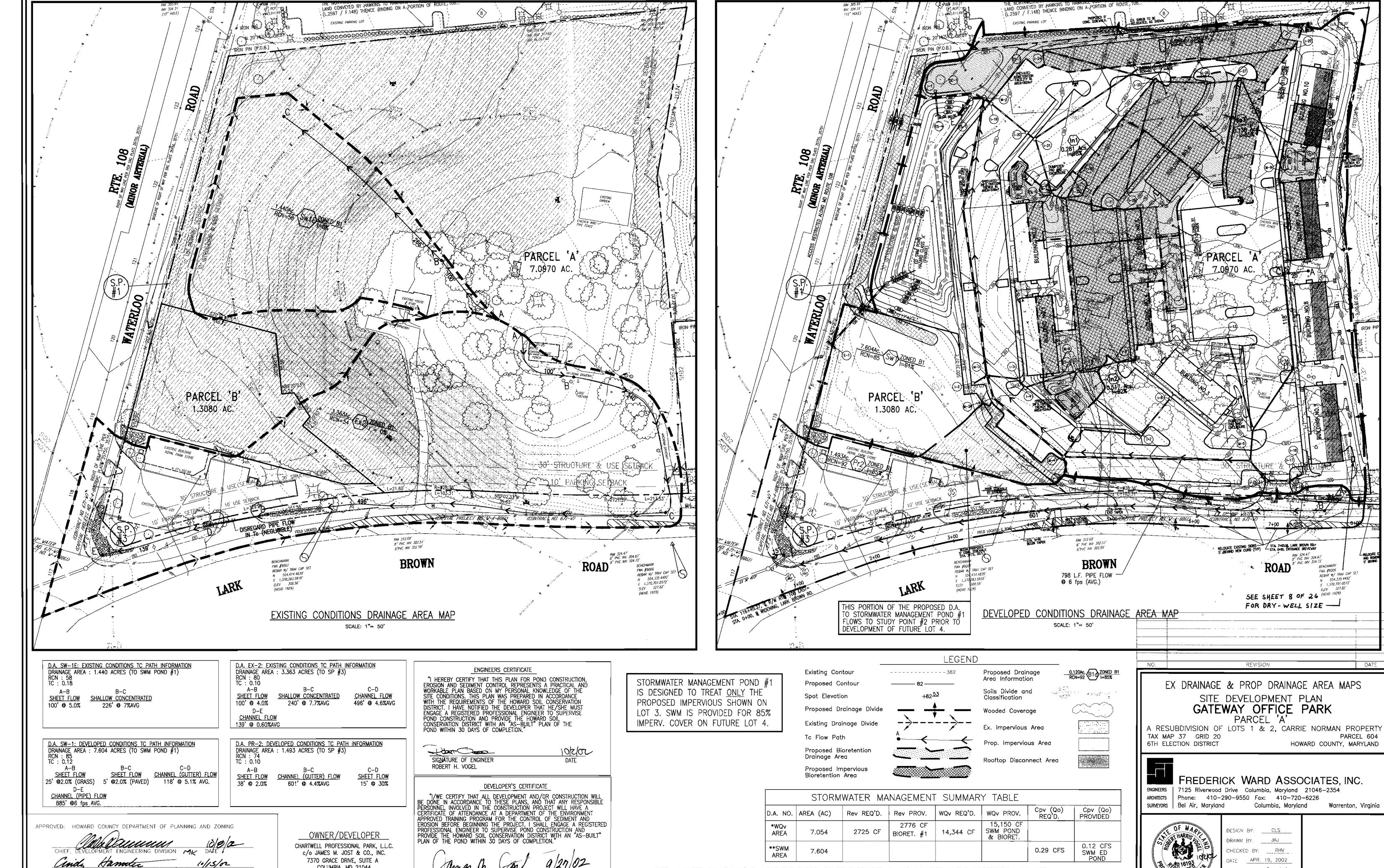
CHIEF, DIVISION OF LAND DEVELOPMENT 18

10/16/02

COLUMBIA, MD 21044

Attn.: MR. JAMES JOST

(443) 535-9200



* WATER QUALITY AND Rev FOR THE PARCEL TO BE SUBDIVIDED IS NOT INCLUDED IN THE BIORETENTION AND WET POND. THE WQv AREA IS THE TOTAL 7.807 Acs. LESS THE 0.753 Acs. OF THE PROPOSED PARCEL TO BE SUBDIVIDED.

** STORMWATER MANAGEMENT (CPv) FOR THE SITE AREA (INCLUDING THE 0.753 Acs TO BE SOLD) IS PROVIDED BY

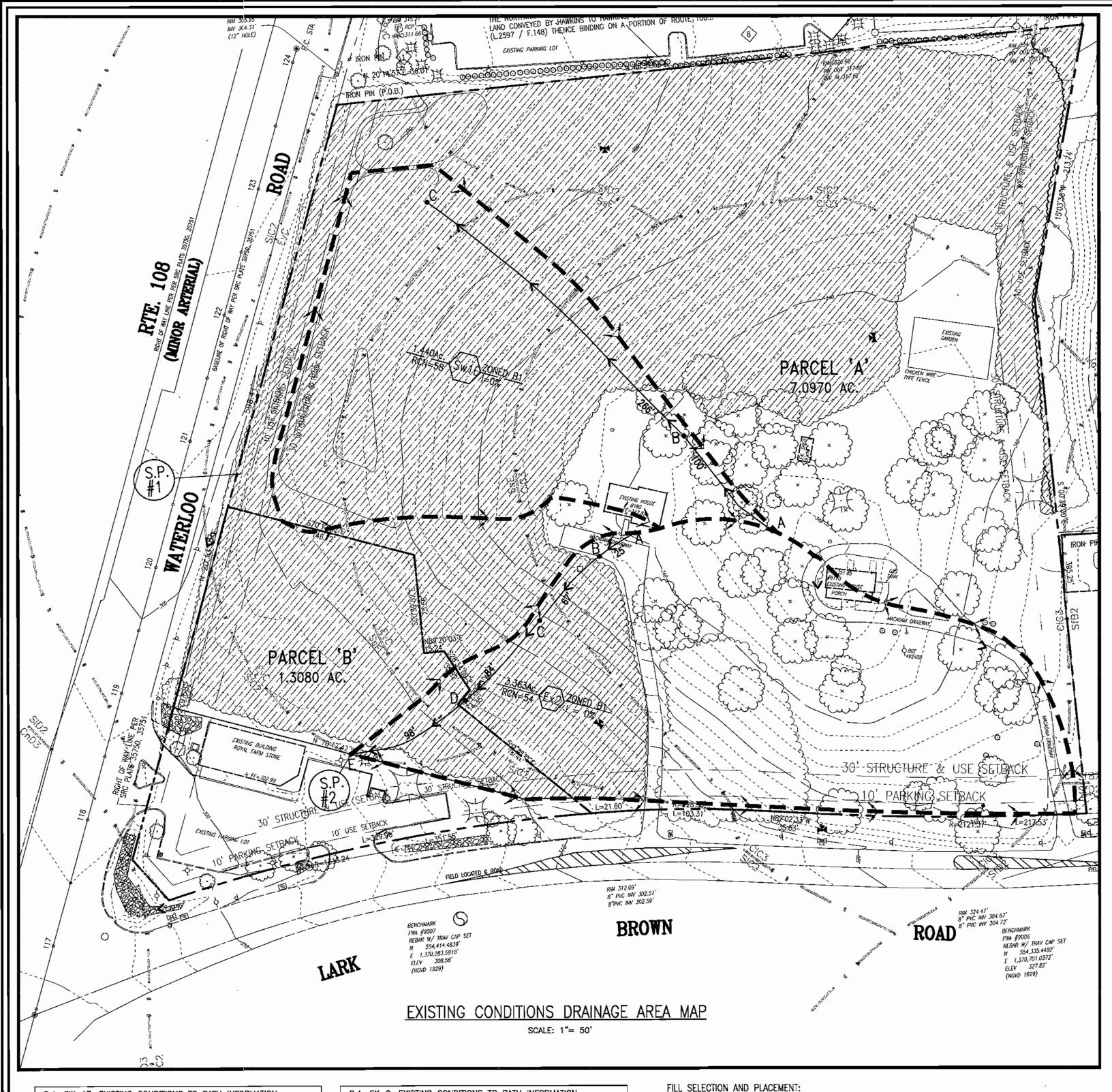
THE PROPOSED SWM POND. (SEE NOTE AT LEFT)

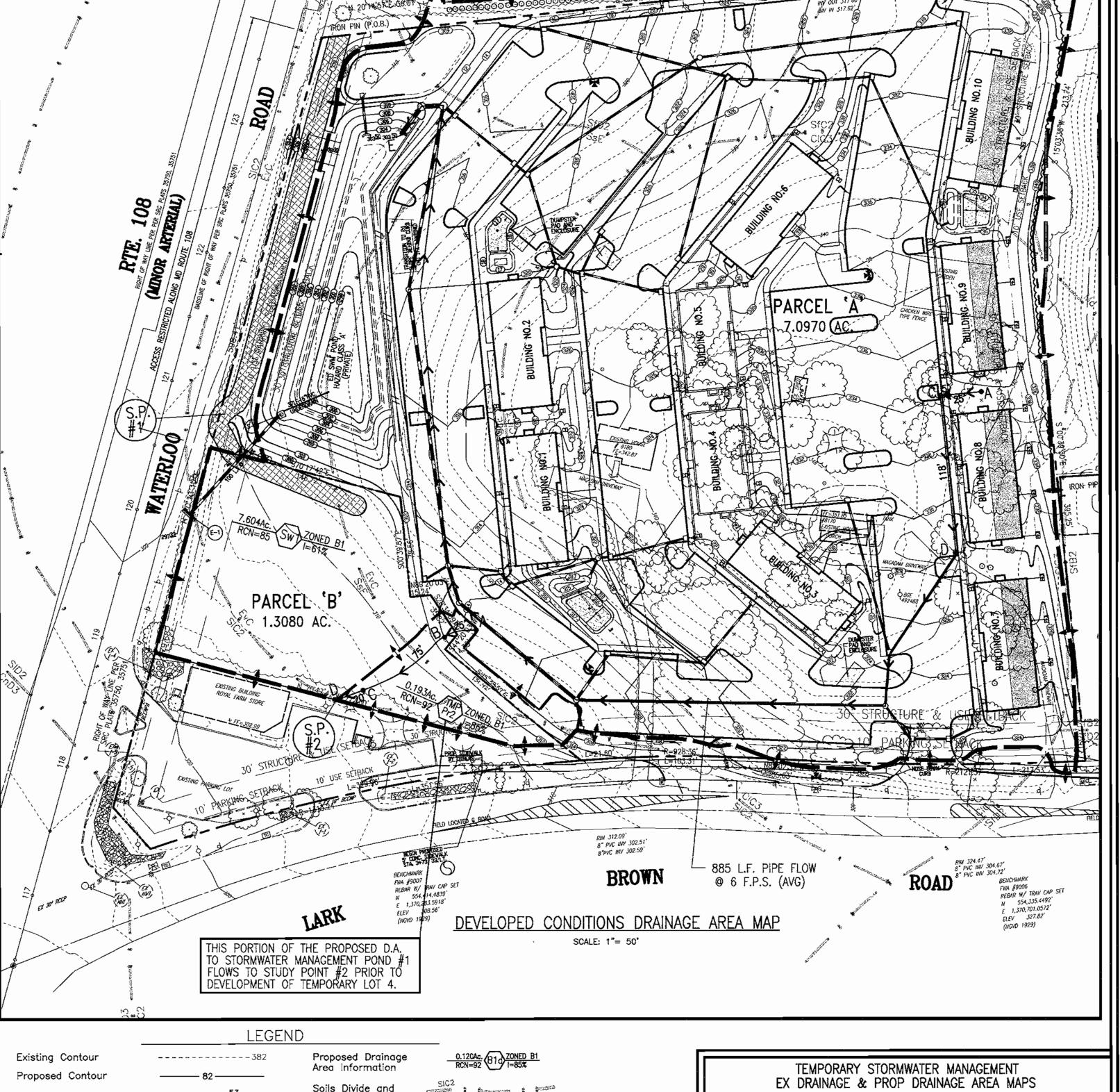
13 SHEET 26

1" = 50'

W.O. NO.: 2017165

SCALE





D.A. SW-1E: EXISTING CONDITIONS TO PATH INFORMATION DRAINAGE AREA: 1.440 ACRES (TO SWM POND #1) RCN: 58
TC: 0.18

SHALLOW CONCENTRATED
226' @ 7% AVG 100' @ 5.0%

D.A. SW-1: DEVELOPED CONDITIONS TO PATH INFORMATION DRAINAGE AREA: 7.604 ACRES (TO SWM POND #1)
RCN: 85

C-D CHANNEL (GUTTER) FLOW SHEET FLOW SHEET FLOW
25' @ 2.0%(GRASS) 5' @ 2%(PAVED) 118' @ 5.1% AVG D-E CHANNEL (PIPE) FLOW 885' @ 6fps AVG

STORMWATER MANAGEMENT POND #1 IS DESIGNED TO TREAT ONLY THE PROPOSED IMPERVIOUS SHOWN ON LOT 3. SWM IS PROVIDED FOR 85% IMPERVIOUS COVER ON LOT 4.

D.A. EX-2: EXISTING CONDITIONS TO PATH INFORMATION DRAINAGE AREA: 3.363 ACRES (TO SP #2)
RCN: 54

98' @ 5.6% (GRASS)

SHEET FLOW

25' @ 3.3%

C-D
SHALLOW CONCENTRATED
84' @ 16% AVG SHEET FLOW SHEET FLOW 25' 12.5% (PAVED) 67' 16% AVG (UNPAVED) SHALLOW CONCENTRATED

The on—site soils were generally found to have moisture contents ranging from 2 to 13 percent, which is expected to be within 5 percent dry of the optimum moisture content for compaction to 10 percent wet. However, the moisture content of these soils may vary with the seasons and expect that the soils may require wetting or drying prior to compaction. D.A. TMP PR-2: TEMPORARY DEVELOPED COND. TC PATH INFO.
DRAINAGE AREA: 0.193 ACRES (TEMP. FLOW TO SP #2)
RCN: 92
TC: 0.14 If imported fill is required at this site, the imported soils should consist of material classified as SM, SC, SW, SP, GC, and GM per ASTM D-2487. The imported soils should be free of deleterious

material, debris, or fragments larger than 6-inches in diameter. The plasticity index of the soil should be less than 15 when tested in accordance with ASTM D_4318. C-D SHALLOW CONCENTRATED 75' @ 18% AVG 20' @ 5.6% Earthwork should be conducted during the drier periods of the year, typically between June and October. Additional earthwork costs should be anticipated if earthwork operations occur during (GRASS)

> All structural fill and/or back fill should be placed in horizontal loose lifts not in excess of 8-inches thick and compacted to at least 95 percent of the maximum dry density as determined by the Standard Proctor (ASTM D-698). The top one foot of pavement subgrade should be compacted to 100 percent of the maximum dry density as determined by ASTM D-698. The moisture content of the fill should be maintained within 2% of the optimum moisture content as determined by ASTM D-698. All compacted fill in the building areas should extend at least 5 feet outside the proposed building footprint. All lift depths, compaction equipment, moisture levels, and compactive effort should be monitored and approved by the geotechnical engineer.

Compacted fill will be required below the proposed buildings, parking areas, and as back fill in utility trenches. On—site soils were generally classified as SAND (SP), silty SAND (SP—SM), clayey

SAND (SP-SC), and sandy SILT (ML), and are considered suitable for use as compacted fill. The

lean CLAY (CL) is not recommended to be used as fill below footings or pavements. Cemented rock fragments larger than 6 inches in diameter should be crushed or removed from the fill soils.

Area Information Proposed Contour Soils Divide and Spot Elevation Classification Proposed Drainage Divide Wooded Coverage Existing Drainage Divide Ex. Impervious Area To Flow Path Prop. Impervious Area Proposed Bioretention Drainage Area Rooftop Disconnect Area Proposed Impervious Bioretention Area

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING CHIEF, DEVELOPMENT ENGINEERING DIVISION MK DATE

and the second of the second o

A RESUBDIVISION OF LOTS 1 & 2, CARRIE NORMAN PROPERTY

OWNER/DEVELOPER

CHARTWELL PROFESSIONAL PARK, L.L.C.

c/o JAMES M. JOST & CO., INC.

7370 GRACE DRIVE, SUITE A

COLUMBIA, MD 21044

Attn.: MR. JAMES JOST

(443) 535-9200

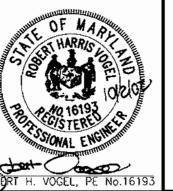
PARCEL 604 TAX MAP 37 GRID 20 6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND FREDERICK WARD ASSOCIATES, INC.

SITE DEVELOPMENT PLAN

GATEWAY OFFICE PARK

PARCEL 'A'

ENGINEERS | 7125 Riverwood Drive Columbia, Maryland 21046-2354 ARCHITECTS Phone: 410-290-9550 Fax: 410-720-6226 SURVEYORS Bel Air, Maryland Columbia, Maryland Warrenton, Virginia



DESIGN BY: CLS DRAWN BY: CHECKED BY: RHV APR. 19, 2002 1" = 50SCALE: W.O. NO.: 2017165

14 SHEET 26

GENERAL NOTES

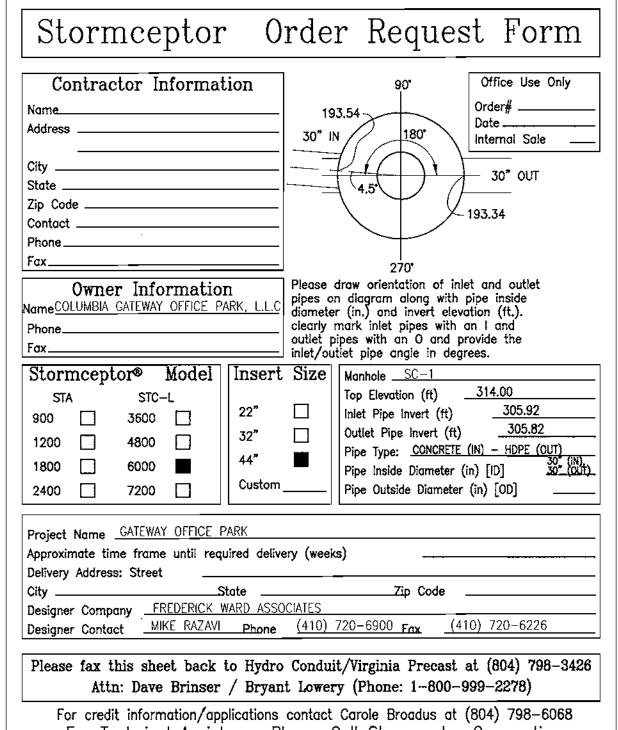
- 1. ALL STRUCTURES DETAILED ON THIS SHEET SHALL BE CAPABLE OF SUPPORTING AASHTO HS-20 LOADING, AND SHALL BE CONSTRUCTED IN FULL ACCORDANCE WITH ACI 318, LATEST EDITION.
- 2. CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH fc $^{\prime}$ = 3,500 psi.
- 3. ALL REINFORCING SHALL CONFORM TO ASTM A-615
- 4. CONTRACTOR SHALL PRE-CAST STRUCTURE IN WHOLE OR IN PART, SUBJECT TO THE PRIOR APPROVAL OF PRE-CAST SHOP DRAWING SUBMITTED TO HOWARD COUNTY BUREAU OF ENGINEERING PRIOR TO MANUFACTURING.
- 5. SILT AND DEBRIS SHALL NOT BE ALLOWED TO ENTER THE STRUCTURES UNTIL THE CONTRIBUTING DRAINAGE AREA HAS BEEN PERMENENTLY STABILIZED.
- 6. ALL OPENINGS TO THE STRUCTURE SHALL BE PROTECTED WITH THE APPROPRIATE SEDIMENT CONTROL MEASURES DURING CONSTRUCTION.

STORMWATER MANAGEMENT AND WATER QUALITY MAINTENANCE NOTES (PRIVATE OWNERSHIP AND MAINTENANCE)

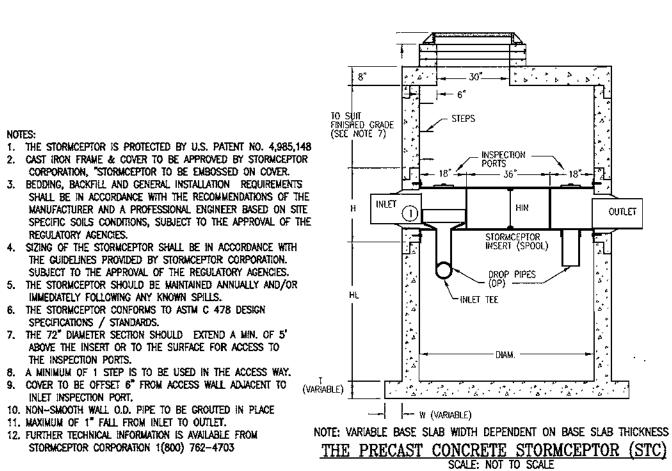
- INSPECT THE STORMCEPTOR ON A MONTHLY BASIS AND NOTE THE SEDIMENT AND OIL ACCUMULATIONS. MORE FREQUENT INSPECTIONS ARE APPROPRIATE WHERE OIL SPILLS OCCUR REGULARLY. STORMCEPTOR CANADA INC. CAN PROVIDE ADVICE ON SAMPLING EQUIPMENT.
- SEDIMENT SHOULD BE REMOVED ANNUALLY, OR WHENEVER THE ACCUMULATION REACHES 50% OF THE OPERATING DEPTH, FROM BASE TO DRAIN INVERT. IN AREAS OF NEW CONSTRUCTION, OR WHERE VEGETATION HAS NOT YET BEEN ESTABLISHED, MORE FREQUENT REMOVAL MAY BE NECESSARY.
- VACUUM TRUCKS ARE USED TO REMOVE THE SEDIMENT AND OIL FROM THE STORMCEPTOR TREATMENT CHAMBER. OIL LEVELS GREATER THAN 2.5 CM SHOULD BE REMOVED IMMEDIATELY BY A LICENSED WASTE MANAGEMENT FIRM, AND SIGNIFICANT SPILLS MUST BE REPORTED TO THE APPROPRIATE REGULATORY AGENCY.
- THE PIPES AND STRUCTURAL PARTS SHALL BE CLEANED AND REPAIRED AS NEEDED.
- ALL MAINTENANCE SHALL BE PERFORMED BY THE OWNER OR BY THE OWNER'S REPRESENTATIVE AT THE OWNER'S EXPENSE.
- MINIMIZE SURFACE EROSION FROM PERVIOUS SURFACES AT ALL TIMES. MAINTAIN GRASS, SOD, AND/OR MULCH COVERAGE UPON LANDSCAPING.
- REMOVE DEBRIS ON TOP OF INLET GRATES AFTER EVERY RAINFALL. REMOVE OILS, SILTS AND AGGREGATES, AND DEBRIS FROM ALL INLETS AND STRUCTURES.
- 8. THE DISPOSAL OF THE LIQUID AND SOLID MATTER SHOULD BE AS FOLLOWS:
 - ALL LIQUID MATERIAL IN THE SEPARATOR INLET SHALL BE PUMPED INTO A SUITABLE TANK TRUCK AND DISPOSED OF AT AN APPROVED SANITARY DISTRICT DISCHARGE MANHOLE OR BE TAKEN TO AN APPROVED SEWAGE TREATMENT PLANT FOR DISCHARGE.
 - THE SOLID MATERIAL SHALL BE LANDFILLED IN AN APPROVED SANITARY

OPERATION AND MAINTENANCE SCHEDULE STORMCEPTOR WATER QUALITY DEVICE

- A. THE STORMCEPTOR WATER QUALITY STRUCTURE SHALL BE PERIODICALLY INSPECTED AND CLEANED TO MAINTAIN OPERATION AND FUNCTION. THE OWNER SHALL INSPECT THE STORMCEPTOR UNIT YEARLY AT A MINIMUM, UTILIZING THE STORMCEPTOR INSPECTION/MONITORING FORM. INSPECTIONS SHALL BE DONE BY USING A CLEAR PLEXIGLAS TUBE ('SLUDGE JUDGE') TO EXTRACT A WATER COLUMN SAMPLE. WHEN THE SEDIMENTS DEPTHS EXCEED THE LEVEL SPECIFIED IN TABLE 6 OF THE STORMCEPTOR TECHNICAL MANUAL, THE UNIT MUST BE CLEANED.
- B. THE STORMCEPTOR WATER QUALITY STRUCTURE SHALL BE CHECKED AND CLEANED IMMEDIATELY AFTER PETROLEUM SPILLS. THE OWNER SHALL CONTACT THE APPROPRIATE REGULATORY AGENCIES.
- C. THE MAINTENANCE OF THE STORMCEPTOR UNIT SHALL BE DONE USING A VACUUM TRUCK WHICH WILL REMOVE THE WATER, SEDIMENT, DEBRIS, FLOATING HYDROCARBONS, AND OTHER MATERIALS IN THE UNIT. PROPER CLEANING AND DISPOSAL OF THE REMOVED MATERIALS AND LIQUID MUST BE FOLLOWED BY THE OWNER.
- D. THE INLET AND OUTLET PIPES SHALL BE CHECKED FOR ANY OBSTRUCTIONS AT LEAST ONCE EVERY SIX MONTHS. IF OBSTRUCTIONS ARE FOUND THE OWNER SHALL HAVE THEM REMOVED. STRUCTURAL PARTS OF THE STORMCEPTOR UNIT SHALL BE REPAIRED AS NEEDED.
- E. THE OWNER SHALL RETAIN AND MAKE THE STORMCEPTOR INSPECTION/MONITORING FORMS AVAILABLE TO THE HOWARD COUNTY OFFICIALS UPON THEIR REQUEST.



For Technical Assistance Please Call Stormceptor Corporation at (301) 762-8361 or toll free at 1-800-762-4703



GENERAL NOTES FOR STORM DRAIN CONSTRUCTION

- 1. MANHOLE LIDS AND INLET GRATES SHALL BE TRAFFIC BEARING
- AND BICYCLE SAFE. 2. ALL MANHOLE LIDS, INLET GRATES, AND CLEANOUT TOPS TO BE ADJUSTED TO MATCH CROSS-PITCH AND SLOPE OF EXISTING AND PROPOSED GRADES.

STORMCEPTOR CORPORATION 1(800) 762-4703

- 3. ROOF DRAIN PIPE TO BE HDPE, OR PVC SCH. 40, UNLESS
- OTHERWISE NOTED. 4. ALL STORM DRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST EDITION OF THE HOWARD COUNTY STANDARDS AND
- 5. INFORMATION IS UNAVAILABLE REGARDING THE LOCATION OF ON-SITE TELEPHONE, GAS, AND ELECTRIC.

CONTRACTOR INSTALLATION INSTRUCTIONS: PRECAST CONCRETE STORMCEPTOR

- STAKE-OUT THE LOCATION OF THE STORMCEPTOR AND EXCAVATE HOLE. EXCAVATE ADEQUATE SPACE TO CONNECT INLET AND OUTLET PIPES TO UNIT. INSTALL A 12" DEEP (OR AS REQUIRED) LAYER OF COMPACTED (95% STANDARD PROCTOR DENSITY OR LOCAL AND STATE REQUIREMENTS, AS DIRECTED BY THE INSPECTOR) AGGREGATE SUBBASE AT BOTTOM OF EXCAVATION. INSTALL MULE OR SHORING, AS NEEDED. CHECK ELEVATION OF UNIT BY MEASURING ITS SECTIONS FROM BASE OF THE STORAGE CHAMBER (BOTTOM OF UNIT'S SLAB) TO THE INVERT OF STORMCEPTOR BYPASS CHAMBER INLET ELEVATION (FIBERGLASS INSERT). SUBTRACT THIS DISTANCE FROM DESIGN INVERT
- SUBBASE AND ADJUST AS NEEDED. SECURE INSPECTOR APPROVAL OF SUBGRADE AND SUBBASE. INSTALL STORAGE CHAMBER, INSTALL SCREW INSERTS INTO BASE OF STORAGE CHAMBER. ATTACH CABLES OR CHAINS TO ALL 3 LIFTING LUGS ON THE BASE SLAB. USING LARGE EQUIPMENT OR CRANE LIFT AND PLACE THE BASE SECTION OF THE STORAGE CHAMBER IN THE EXCAVATED HOLE ON THE SUBBASE. MAKE SURE THAT THE BASE IS LEVEL. SPECIFIC ALIGNMENT OF THIS PART IS NOT REQUIRED. INSTALL RUBBER GASKET ON BASE UNIT AND COAT WITH LUBRICATING GREASE (PROVIDED IN SHIPMENT), IF NOT PRELUBRICATED. INSTALL ADDITIONAL STORAGE CHAMBER SECTIONS, AS REQUIRED

ELEVATION TO DETERMINE TOP OF SUBBASE ELEVATION. CHECK ELEVATION OF INSTALLED

- (PROCEDURE IS SAME AS STEP 8.). INSTALL REDUCING SLAB. (STORMCEPTOR MODELS STC-2400, STC-3600, STC-4800, STC-8000 AND STC-7200) CHECK THAT SECTION IS SET FLUSH, LEVEL AND IS AT THE PROPER ELEVATION. INSTALL RUBBER GASKET ON THE TRANSITION SLAB SPIGOT
- AND COAT WITH LUBRICATING GREASE (PROVIDED IN SHIPMENT). 6. INSTALL BYPASS CHAMBER OF STORMCEPTOR WITH FACTORY INSTALLED STORMCEPTOR INSERT. LIFT BYPASS SECTION AND INSTALL, WHILE CHECKING ALIGNMENT AND GRADE OF INLET AND OUTLET DRAINAGE PIPES. CHECK TO MAKE SURE THE BYPASS CHAMBE IS SET FLUSH, LEVEL AND IS AT THE PROPER ELEVATION. THE BYPASS CHAMBER MUST BE ORIENTED SUCH THAT INLET PIPE DISCHARGES INTO THE V-SHAPED FIBERGLASS WEIRS (INSIDE INSERT). INSTALL RUBBER GASKET ON TOP OF BYPASS SECTION AND COAT WITH LUBRICATING GREASE, IF NOT PRELUBRICATED.
- INSTALL STORMCEPTOR DROP PIPES ACCORDING TO STC PIPE INSTALLATION PROCEDURE. INSTALL RISER SECTION. LIFT RISER SECTION AND INSTALL, WHILE CHECKING THAT SECTION IS SET FLUSH AND IS AT PROPER ELEVATION AND THAT UNIT IS LEVEL. SPECIFIC ALIGNMENT OF THIS PART IS REQUIRED IF STEP(S) ARE INCLUDED. ALIGN STEPS ABOVE INLET INSPECTION PORT. NOTE: FOR SHALLOW INSTALLATIONS THIS SECTION MAY NOT BE REQUIRED.

INSTALL TOP CAP WITH OPENING FOR STORMCEPTOR COVER. IF OPENING IS OFFSET

- (NOT CENTERED) THE TOP CAP OPENING SHOULD BE ORIENTED ABOVE THE STORMCEPTOR INLET INSPECTION PORT (PLUG). 10. BACKFILL STORMCEPTOR WITH APPROVED BACKFILL MATERIAL (NO ORGANIC OR TOPSOIL IS TO BE USED FOR BACKFILL). BACKFILL AND COMPACT IN 8 INCH LIFTS. BACKFILL SHOULD BE COMPACTED TO 95% OF STANDARD PROCTOR DENSITY, OR LOCAL AND STATE REQUIREMENTS, AS DIRECTED BY THE INSPECTOR.
- 11. INSTALL AND SET GRADE ADJUSTING RINGS, AS NEEDED. 12. INSTALL AND SET STORMCEPTOR FRAME AND COVER. 13. INSTALL INLET AND OUTLET STORM DRAIN PIPES. CONNECT INLET AND OUTLET STORM DRAIN PIPES WITH FLEXIBLE BOOTS (WHEN PROVIDED) AND WITH NON-SHRINK GROUT WHEN NO FLEXIBLE BOOTS ARE PROVIDED. THE INVERT OF THE INLET AND OUTLET PIPE IS TO MATCH WITH THE INVERT OF THE STORMCEPTOR INSERT. FLEXIBLE BOOT INSTALLATION PROCEDURES: CENTER THE PIPE IN THE BOOT OPENING. LUBRICATE TH OUTSIDE OF THE PIPE AND/OR THE INSIDE OF THE BOOT IF THE PIPE OUTSIDE DIAMETER IS THE SAME AS THE INSIDE DIAMETER OF THE BOOT. POSITION THE PIPE CLAMP IN THE GROOVE OF THE BOOT WITH THE SCREW AT THE TOP, TIGHTEN THE PIPE CLAMP SCREW TO 60 INCH POUNDS. IF THE PIPE IS MUCH SMALLER THAN THE BOOT, LIFT THE BOOT SUCH THAT IT CONTACTS THE BOTTOM OF THE PIPE WHILE TIGHTENING THE CLAMP TO ENSURE EVEN CONTRACTION OF THE RUBBER. MOVE THE PIPE HORIZONTALLY AND/OR
- VERTICALLY TO BRING IT TO GRADE. 14. THE STORMCEPTOR SHOULD BE PUMPED OUT WHEN THE SEDIMENT CONTROL MEASURES ARE REMOVED (SITE PERMANENTLY STABILIZED).

15. FINAL INSPECTION.

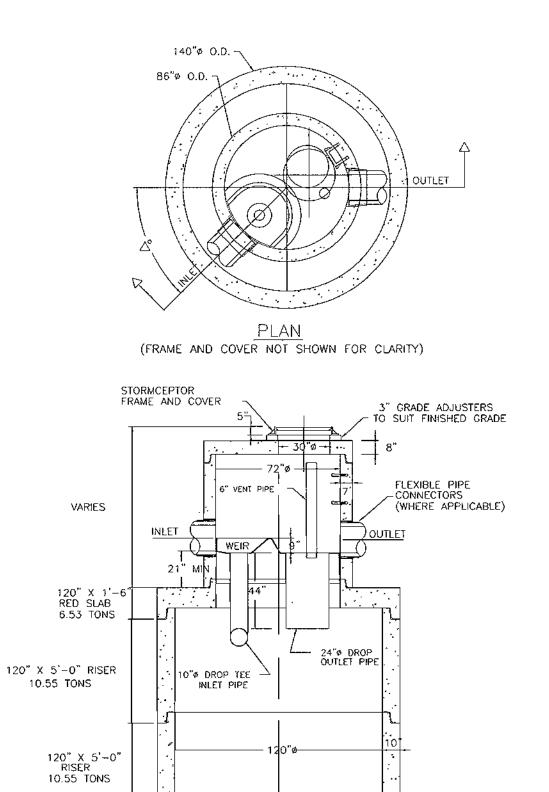
DIMENSIONS*					
MODEL	DIAM (ft)	DROP PIPE DIAM. (in) (DP)	HL (in)	7** (in)	₩** (in)
STC 900	6	6	42	. 8	8
STC 1200	9	6	60	8	8
STC 1800	6	6	96	8	8
STC 2400	8	8	76	8	0
STC 3600	8	8	118	8	0
STC 4800	10	10	106	12	0
STC 6000	10	10	130	12	0
STC 7200	12	12	114	14	0

	DIMENSIONS*			
PIPE DIAMETER D (in)	PIPE MATERIAL	HIN (in)	H (in)	L (in)
10	PVC	22	36	8
10	CONC / PE RIB	22	36	7
12	PVC / PE RIB	22	36	9
12	CONCRETE	22	42	11
15	PVC / PE RIB	22	42	9.5
15	CONCRETE	22	48	11.5
18	PVC / PE RIB	22	48	10
18	CONCRETE	32	48	11
21	PVC / PE RIB	32	48	9.5
21	CONCRETE	32	54	11.5
24	PVC	32	48	9
24	PE RIBBED	32	54	10
24	CONCRETE	32	54	11
27	PVC	32	54	8.5
27	CONCRETE	44	60	11.5
30	CONCRETE	44	60	12
33	CONCRETE	44	60	12.5
36	CONCRETE	44	60	13

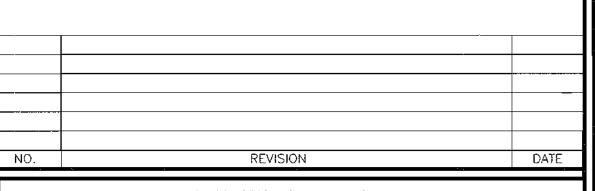
FLOWS AND CAPACITIES*				
MOĐEL	MAX TREATED FLOW RATE (gpm)**	SEDIMENT CAPACITY (U.S. gal.)	OIL CAPACITY (U.S. gal.)	TOTAL CAPACITY (U.S. gal.)
STC 900	285	70	280	910
STC 1200	285	110	280	1230
STC 1800	285	195	280	1860
STC 2400	475	210	560	2405
STC 3600	475	390	560	3720
STC 4800	800	515	675	5030
STC 6000	800	675	675	6205
STC 7200	1110	800	680	7510

 APPROXIMATE ** WITHOUT BY-PASSING

> OWNER/DEVELOPER CHARTWELL PROFESSIONAL PARK, L.L.C. c/o JAMES M. JOST & CO., INC. 7370 GRACE DRIVE, SUITE A COLUMBIA, MD 21044 Attn.: MR. JAMES JOST (443) 535-9200



STC 6000 PRECAST CONCRETE STORMCEPTOR SCALE: NOT TO SCALE



STORMCEPTOR DETAILS, NOTES AND SPECIFICATIONS SITE DEVELOPMENT PLAN GATEWAY OFFICE PARK PARCEL 'A'

A RESUBDIVISION OF LOTS 1 & 2, CARRIE NORMAN PROPERTY TAX MAP 37 GRID 20 PARCEL 604 6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND



120" X 12' 🛓

BASE SLAB

6.56 TONS

FREDERICK WARD ASSOCIATES, INC. ENGINEERS | 7125 Riverwood Drive Columbia, Maryland 21046-2354

ARCHITECTS Phone: 410-290-9550 Fax: 410-720-6226 Columbia, Maryland surveyors | Bel Air, Maryland



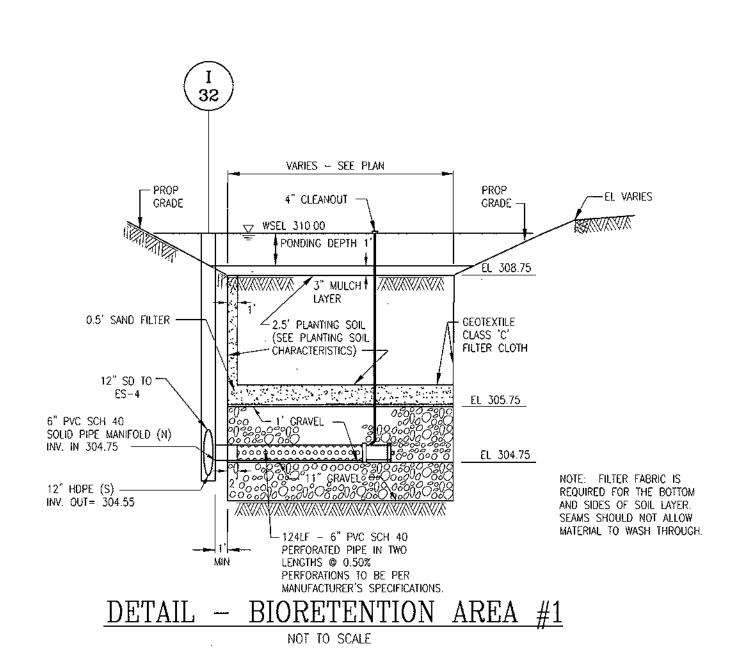
DESIGN BY: ____CLS_ DRAWN BY: CLS CHECKED BY: CLS DATE: APR. 19, 2002 SCALE: AS SHOWN W.O. NO.: 2017165

16 SHEET _

Warrenton, Virginia

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

10/16/02



Appendix B.3. Construction Specifications for Sand Filters, Bioretention and Open Channels

Specifications for Bioretention

1. Material Specifications

The allowable materials to be used in bioretention area are detailed in Table B.3.2.

2. Planting Soil The soil shall be a uniform mix, free of stones, stumps, roots or other similar objects larger than two inches. No other materials or substances shall be mixed or dumped within the bioretention area that may be harmful to plant growth, or prove a hindrance to the planting or maintenance operations. The planting soil shall be free of Bermuda grass, Quackgrass, Johnson grass, or other noxious weeds as specified under COMAR 15.08.01.05.

The planting soil shall be tested and shall meet the following criteria:

pH range 5.2 - 7.0organic matter __ 1.5, - % (by weight)

magnesium 35 lb./ac phosphorus (phosphate - P 20\$ 75 lb./ac potassium (potash - K 20) 85 lb./ac

soluble salts — nit to exceed 500 ppm All bioretention areas shall have a minimum of one test. Each test shall consist of both the standard soil test for pH, phosphorus and potassium and additional test of organic matter, and soluble salts. A textural analysis shall be performed for each location where

Since different lab calibrate their testing equipment differently, all testing results shall come from the same testing facility. Should the pH fall out of the acceptable range, it may be modified (higher) with lime or (lower) with iron sulfate plus sulfur.

It is very important to minimize compaction of both the base of the bioretention area and the required backfill. When possible, use hoes to remove original soil. If bioretention areas are excovated using loader, the contractor should use wide track or marsh track equipment, or light equipment with turn type tire. Use of equipment with narrow tracks or narrow tires, rubber tires with large lugs, or high pressure tires will cause excessive compaction resulting in reduced infiltration rates and is not acceptable. Compaction will significantly contribute to design failure.

Compaction can be alleviated at the base of the bioretention facility by using a primary tilling operation such as chisel plow, ripper, or subsoiler. These tilling operations are to refracture the soil profile through the 12 inch compaction zone. Substitute methods must be approved by the engineer. Rotatillers typically do not till deep enough to reduce the effects of compaction from heavy

Rotatili 2 to 3 inches of sand into the base of the bioretenion facility before backfilling the required sand layer. Pump any ponded water before preparing (rototilling) base.

When backfilling the topsoil over the sand layer, first place 3 to 4 inches of topsoil over the sand, then rotatill the sand/topsoil to create a gradation zone. Backfill the remainder of the topsoil to final grade. When backfilling the bioretention facility, place soil in lifts 12" to 18". Do not use heavy equipment within the bioretention basin. Heavy equipment can be used around the perimeter of the basin to supply soils and sand. Grade bioretention materials with light equipment such as a compact loader or a dozer/loader with marsh tracks.

4. Plant Material

Refer to Plant List on sheet 23 of 26 for plant material in the bioretention areas.

5. Plant Installation

Mulch should be placed to a uniform thickness of 2" to 3". Shredded hardwood much is the only accepted mulch. Pine mulch and wood chips will float and move to the perimeter of the bioretention area during a storm event and are not acceptable. Shredded mulch must be well aged (6 to 12 months) for acceptance.

Root stock of the plant material shall be kept moist during transport and on—site storage. The plant root ball should be planted so 1/8th of the ball is above final grade surface. The diameter of the planting pit shall be at least six inches larger than the diameter of the planting ball. Set and maintain the plant straight during the entire planting process.

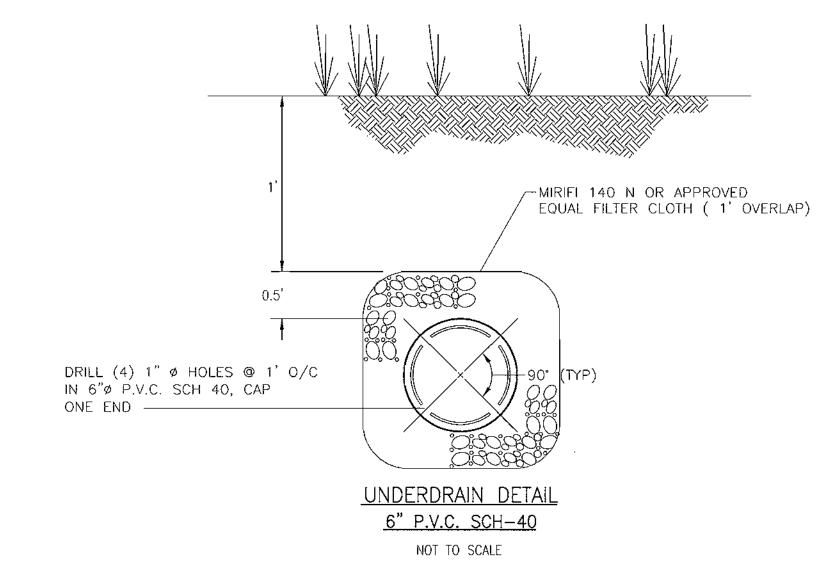
Trees shall be braced using 2" by 2" stakes only as necessary and for the first growing season only. Stakes are to be equally spaced on the outside of the tree ball. Grasses and legume seed should be drilled into the soil to a depth of at least one inch. Grass and legume plugs shall be planted following the non-grass ground cover plnting specifications.

The topsoil specifications provide enough organic material to adequately supply nutrients from natural cycling. The primary function of the bioretention structure is to improve water quality. Adding fertilizers defeats, or at a minimum, impedes this goal. Only add fertilizer if wood chips or mulch are used to amend the soil. Rototill urea fertilizer at a rate of 2 pounds of nitrogen

per 1000 square feet.

Underdrains are to be placed on a 3'-0" wide section filter cloth. Pipe is placed next, followed by the gravel bedding. The ends of underdrain pipes not terminating in an observation well shall be capped. The main collector pipe for underdrain systems shall be constructed at a minimum slope of 0.5%. Observation well and/or clean—out pipes must be provided (one minimum per every 1000 square feet of surface area).

7. Miscellaneous The bioretention facility may not be constructed until all contributing drainage area has been stabilized.



OPERATION AND MAINTENANCE SCHEDULE FOR BIORETENTION AREAS

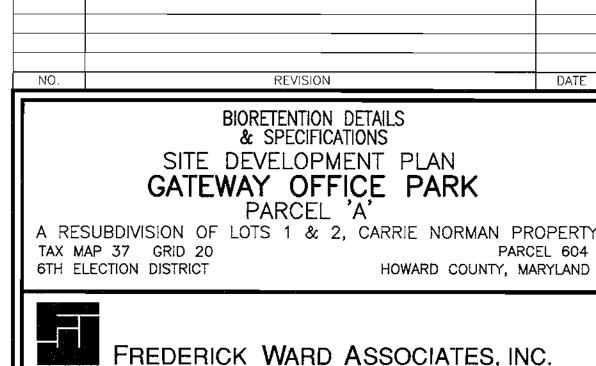
1. annual maintenance of plant material, mulch layer and soil Layer is required, maintenance of mulch and soil is limited To correcting areas of erosion or wash out, any mulch Replacement shall be done in the spring, plant material Shall be checked for disease and insect infestation and Maintenance will address dead material and pruning.

2. schedule of plant inspection will be twice a year in spring And fall, this inspection will include removal of dead and Diseased vegetation considered beyond treatment, treatment of All deficient stakes and wires.

3. mulch shall be inspected each spring, remove previous mulch Layer before applying new layer once every 2 to 3 years.

4. soil erosion to be addressed on an as needed basis, with a Minimum of once per month and after heavy storm events.

MATERIAL	SPECIFICATION	SIZE	NOTES
PLANTINGS	SEE APPENDIX A, TABLE A.4	N/A	PLANTINGS ARE SITE-SPECIFIC
PLANTING SOIL [2.5' TO 4' DEEP]	SAND 35-60% SILT 30-55% CLAY 10-25%	N/A	USDA SOIL TYPES LOAMY SAND, SANDY LOAM OR LOAM
MULCH	SHREDDED HARDWOOD		AGED 6 MONTHS, MINIMUM
PEA GRAVEL DIAPHRAGM AND CURTAIN DRAIN	PEA GRAVEL: ASTM-D-448 ORNAMENTAL STONE: WASHED COBBLES	PEA GRAVEL: NO. 6 STONE: 2" TO 5"	
GEOTEXTILE	CLASS "C"- APPARENT OPENING SIZE (ASTM-D-4751), GRAB TENSILE STRENGTH (ASTM-D-4632), PUNCTURE RESISTANCE (ASTM-D-4833)	N/A	FOR USE AS NECESSARY BENEATH UNDERDRAINS ONLY
UNDERDRAIN GRAVEL	AASHTO M-43	0.375" TO 0.75"	
UNDERDRAIN PIPING	F 758, TYPE PS 28 OR AASHTO M-278	4" TO 6" RIGID SCHEDULE 40 PVC OR SDR35	3/8" PERF. @ 6" ON CENTER, 4 HOLES PER ROW; MINIMUM OF 3" OF GRAVEL OVER PIPES: NOT NECESSARY UNDERNEATH PIPES
POURED IN PLACE CONCRETE (IF REQUIRED)	MSHA MIX NO. 3; F'c=3500 PSI@ 28 DAYS, NORMAL WEIGHT, AIR-ENTRAINED; REINFORCING TO MEET ASTM-615-60	N/A	ON-SITE TESTING OF POURED-IN-PLACE CONCRETE REQUIRED: 28 DAY STRENGTH AND SLUMP TEST; ALL CONCRETE DESIGN (CAST-IN-PLACE OR PRE-CAST) NOT USING PREVIOUSLY APPROVED STATE OR LOCAL STANDARDS REQUIRES DESIGN DRAWINGS SEALED AND APPROVED BY A PROFESSIONAL STRUCTURAL ENGINEER LICENSED IN THE STATE OF MARYLAND - DESIGN TO INCLUDE MEETING ACI CODE 350.R/89; VERTICAL LOADING [H-10 OR H-20]; ALLOWABLE HORIZONTAL LOADING (BASED ON SOIL PRESSURES); AND ANALYSIS OF POTENTIAL CRACKING
SAND [1' DEEP]	AASHTO-M-6 OR ASTM-C-33	0.02" TO 0.04"	SAND SUBSTITUTIONS SUCH AS DIABASE AND GRAYSTONE #10 ARE NOT ACCEPTABLE. NO CALCIUM CARBONATED OR DOLOMITIC SAND SUBSTITUTIONS ARE ACCEPTABLE. NO "ROCK DUST" CAN BE USED FOR SAND.



ENGINEERS | 7125 Riverwood Drive Columbia, Maryland 21046-2354 ARCHITECTS | Phone: 410-290-9550 Fax: 410-720-6226 SURVEYORS | Bel Air, Maryland

Columbia, Maryland Warrenton, Virginia DESIGN BY. CLS DRAWN BY: CHECKED BY: RHV

DATE: APR. 19, 2002 AS SHOWN SCALE: W.O. NO.: 2017165

ROBERT H VOGEL, PE No.16193

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

THESE PLANS HAVE BEEN REVIEWED FOR HOWARD SOIL CONSERVATION DISTRICT AND MEET THE TECHNICAL REQUIREMENTS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL. USDA-NATURAL RESOURCES CONSERVATION SERVICE THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT Ges HOWARD SOIL CONSERVATION DISTRICT

DEVELOPER'S CERTIFICATE "I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE IN ACCORDANCE TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

ENGINEERS CERTIFICATE "I HEREBY CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS—BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

SIGNATURE OF ENGINEER 10/2/02 ROBERT H. VOCEL

19 SHEET 26

DATE

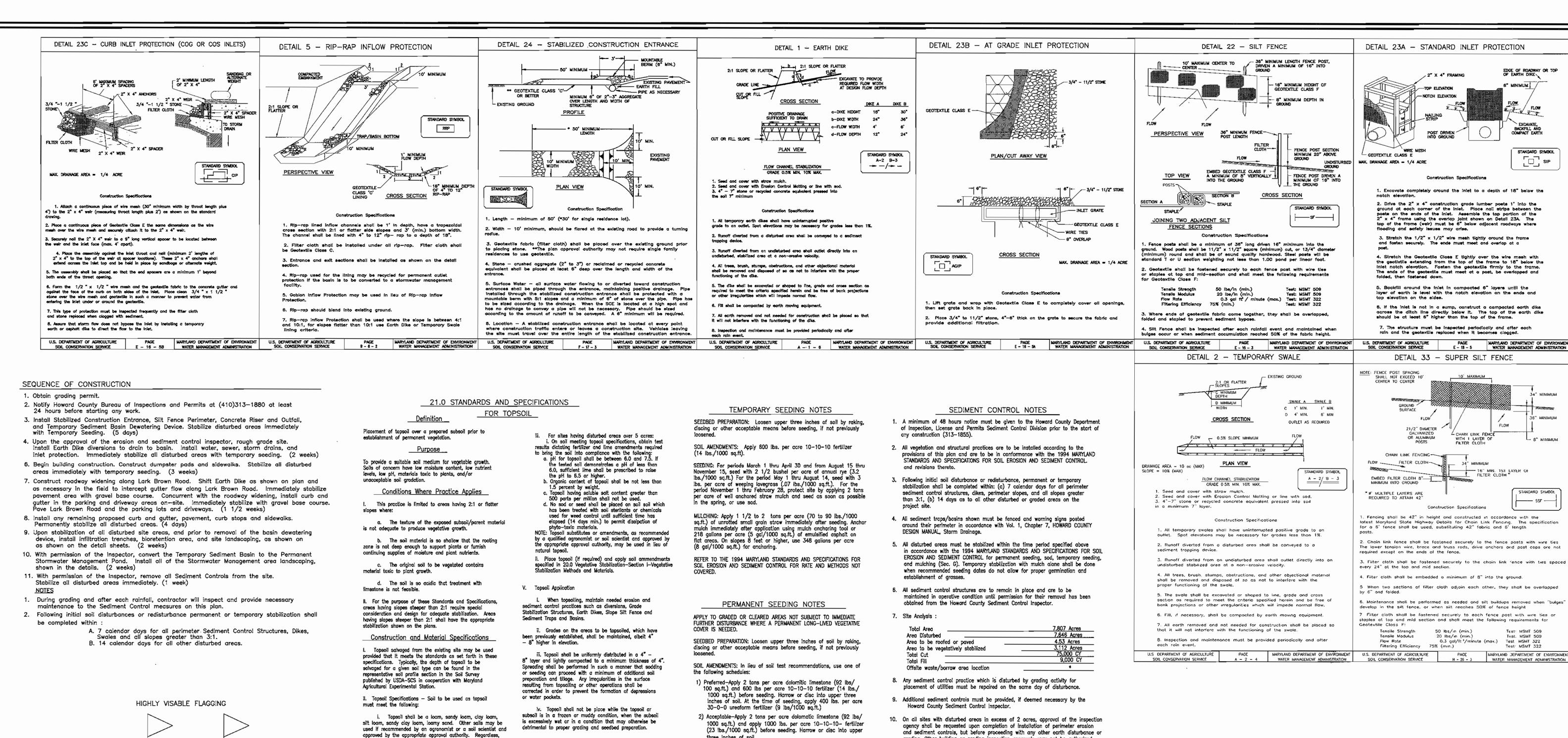
PARCEL 604

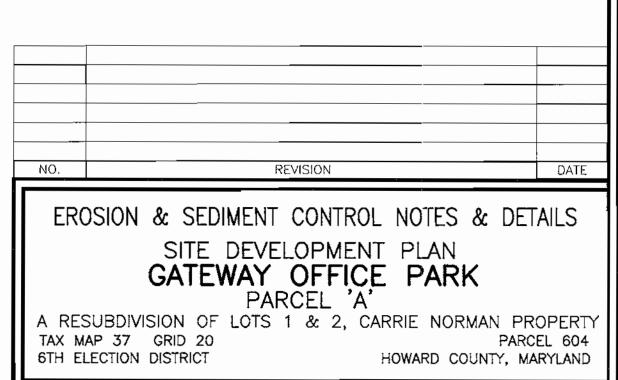


ANCHOR POSTS SHOULD BE

MINIMUM 2" STEEL "U" CHANNEL OR 2" X 2" TIMBER, 6' IN LENGTH.

USE 8' WIRE "U" TO SECURE FENCE BOTTOM.





Construction Specifications

20 lbs/in (min.)

Construction Specifications

PAGE MARYLAND DEPARTMENT OF ENVIRONMENT E - 18 - 5 WATER MANAGEMENT ADMINISTRATION

4" MINIMUM

36" MINIMUM

____ SSF ---

Test, MSMT 509

Test MSMT 322

WATER MANAGEMENT ADMINISTRATION

0.3 gal/ft //minute (max.) Test MSMT 322



FREDERICK WARD ASSOCIATES, INC. 7125 Riverwood Drive Columbia, Maryland 21046-2354

Phone: 410-290-9550 Fax: 410-720-6226 SURVEYORS Bel Air, Maryland

Columbia, Maryland

DESIGN BY: DRAWN BY: CHECKED BY: APR. 19, 2002 AS SHOWN SCALE: 2017165 W.O. NO.:

ROBERT H. VOCEL, PE No.16193

20 SHEET 26

TECHNICAL REQUIREMENTS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL.

THESE PLANS HAVE BEEN REVIEWED FOR HOWARD SOIL CONSERVATION DISTRICT AND MEET THE

tonsoil shall not be a mixture of contrasting textured

subsoils and shall contain less than 5% by volume of

cinders, stones, slag, coarse fragments, gravel, sticks,

roots, trash, or other materials larger that 1 and 1/2" in

ii. Topsoil must be free of plants or plant parts such

as Bermuda grass, quackgrass, Johnsongrass, nutsedge, poison

iii. Where the subsoil is either highly acidic or composed of heavy clays, ground limestone shall be spread at

distributed uniformly over designated areas and worked into

the soil in conjunction with tillage operations as described

i. Place topsoil (if required) and apply soil
 amendments as specified in 20.0 Vegetative Stabilization —
 Section 1 — Vegetative Stabilization Methods and Materials.

If. For sites having disturbed areas under 5 acres:

in the following procedures.

the rate of 4-8 tons/acre (200-400 pounds per 1,000 square feet) prior to the placement of topsoil. Lime shall be

USE 2" X 4" LUMBER FOR

- ANCHOR POSTS MUST BE

INSTALLED TO A DEPTH OF

NO LESS THAN 1/3 OF THE OTAL HEIGHT OF THE POST

CROSS BEARING

MAXIMUM 8 FEET

Forest protection device only.
 Retention area will be set as part of the review process.

5. Protection signage should be used.6. Device should be maintained throughout construction.

NO SCALE

10/15/02 DATE

BLAZE ORANGE PLASTIC MESH TYPICAL TREE PROTECTION FENCE DETAIL

prior to installing device.

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

Mollowaning

Roof damage should be avoided.

. Boundaries of retention area should be staked and flagged

USDA-NATURAL RESOURCES CONSERVATION SERVICE THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. HOWARD SOIL CONSERVATION DISTRICT

DEVELOPER'S CERTIFICATE "I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE IN ACCORDANCE TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS—BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

SEEDING: For the periods March 1 thru April 30, and August 1 thru October 15, seed with 60 lbs. per acre (1.4 lbs/1000 sq.ft.) of

Kentucky 31 Tall Fescue. For the period May 1 thru July 31, seed

with 60 lbs. Kentucky 31 Tall Fescue per acre and 2 lbs. per acre (.05 lbs./1000 sq.ft.) of weeping lovegrass. During the period of

October 16 thru February 28, protect site by: Option (1) 2 tons

per acre well anchored straw mulch and seed as soon as possible in the spring. Option (2) Use sod. Option (3) Seed with 60 lbs/acre

Kentucky 31 Tall Fescue and mulch with 2 tons/acre well anchored

MULCHING: Apply 1 1/2 to 2 tons per acre (70 to 90 lbs/1000

sq. ft.) of unrotted small grain straw immediately after seeding.

Anchor mulch immediately after application using mulch anchoring tool or 218 gallons per acre (5 gal/1000 sq.ft.) of emulsified

asphalt on flat areas. On slapes 8 feet or higher, use 348 gallons per acre (8 gal/1000 sq.ft.) for anchoring.

MAINTENANCE: Inspect all seeded areas and make needed repairs,

"I HEREBY CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS—BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

grading. Other building or grading inspection approvals may not be authorized

11. Trenches for the construction of utilities is limited to three pipe lengths or that

* To be determined by contractor, with pre-approval of the Sediment Control Inspector

which shall be back-filled and stabilized within one working day, whichever is shorter.

until this initial approval by the inspection agency is made.

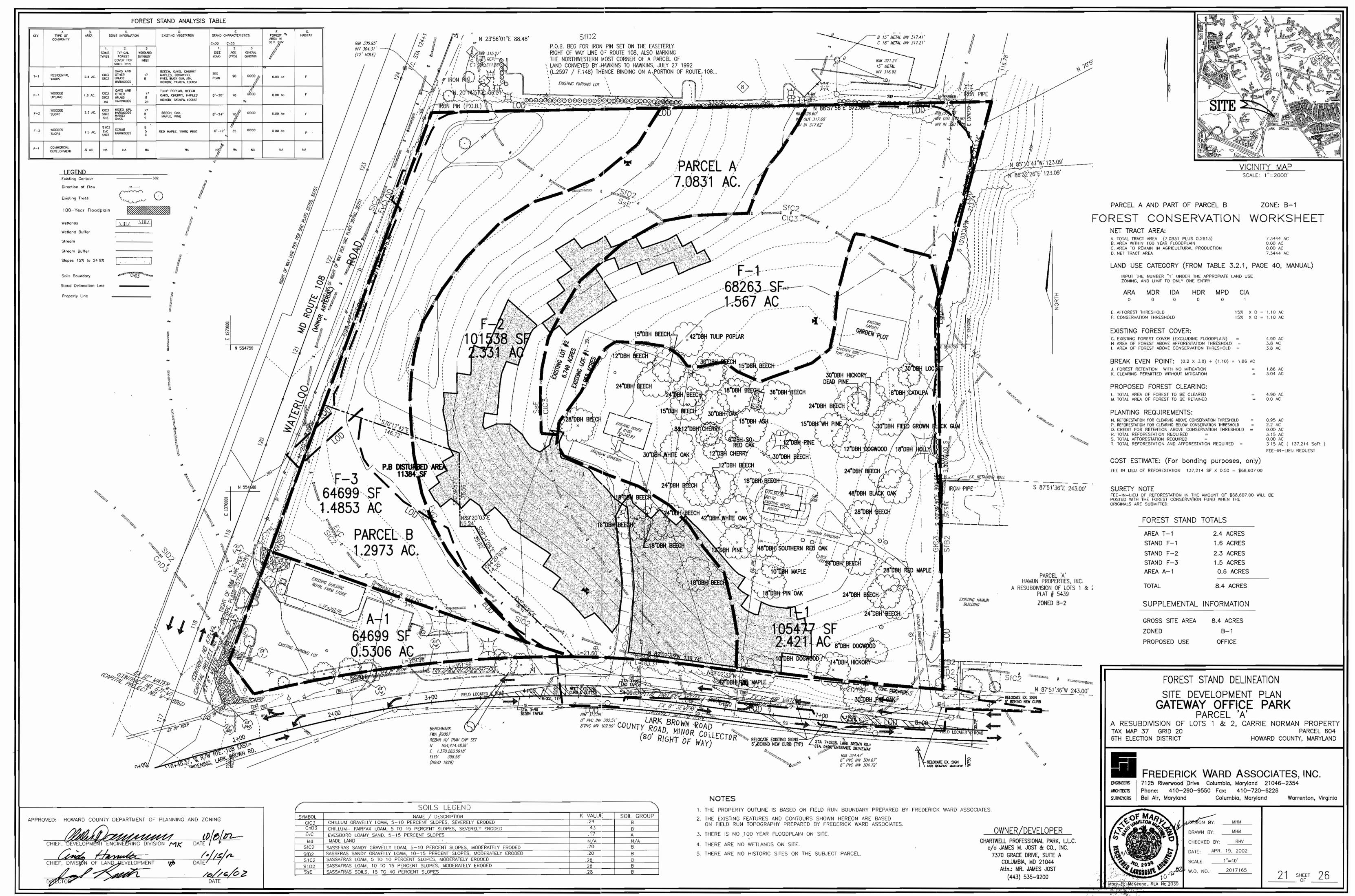
with an approved and active grading permit

SIGNATURE OF ENGINEER ROBERT H. VOGEL

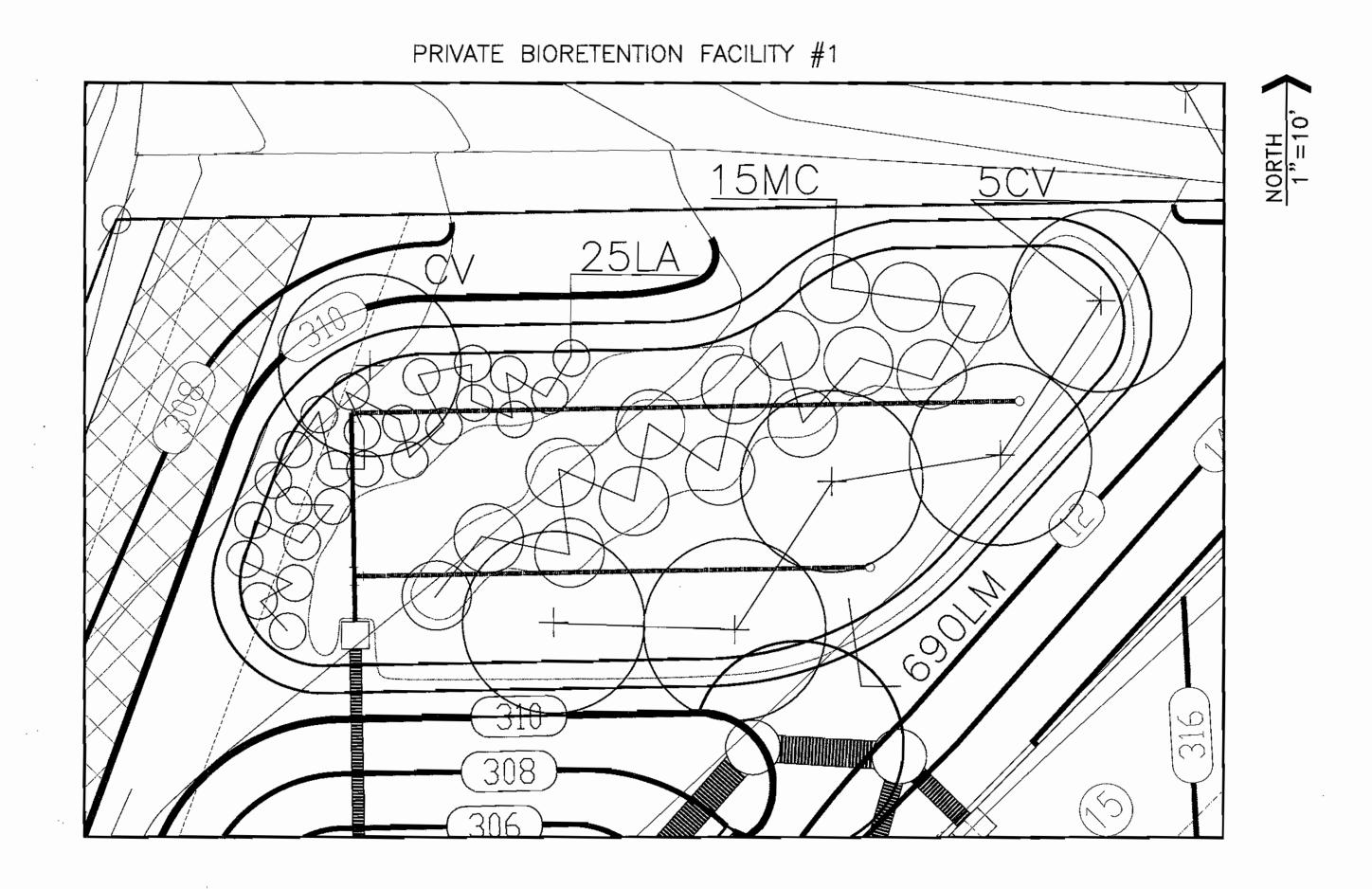
ENGINEERS CERTIFICATE

OWNER/DEVELOPER CHARTWELL PROFESSIONAL PARK, L.L.C c/o JAMES M. JOST & CO., INC. 7370 GRACE DRIVE, SUITE A COLUMBIA, MD 21044 Attn.: MR. JAMES JOST (443) 535-9200

Warrenton, Virginia



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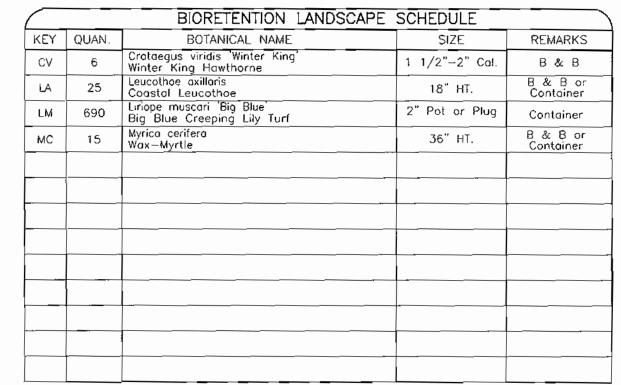
DEVELOPER'S BUILDER'S CERTIFICATE

I/WE CERTIFY THAT THE LANDSCAPING SHOWN ON THIS PLAN WILL BE DONE ACCORDING TO THE PLAN, SECTION 16.124 OF THE

YEAR GUARANTEE OF PLANT MATERIALS, WILL BE SUBMITTED TO THE DEPARTMENT OF PLANNING AND ZONING.

HOWARD COUNTY CODE AND THE HOWARD COUNTY LANDSCAPE MANUAL. I/WE FURTHER CERTIFY THAT UPON COMPLETION, A CERTIFICATION OF LANDSCAPE INSTALLATION, ACCOMPANIED BY AN EXECUTED ONE(1)

Existing Contour Proposed Contour Spot Elevation Direction of Flow



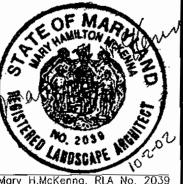
REVISION

BIORETENTION PLANTING DETAILS SITE DEVELOPMENT PLAN GATEWAY OFFICE PARK PARCEL 'A'

A RESUBDIVISION OF LOTS 1 & 2, CARRIE NORMAN PROPERTY TAX MAP 37 GRID 20 PARCEL 604 6TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND

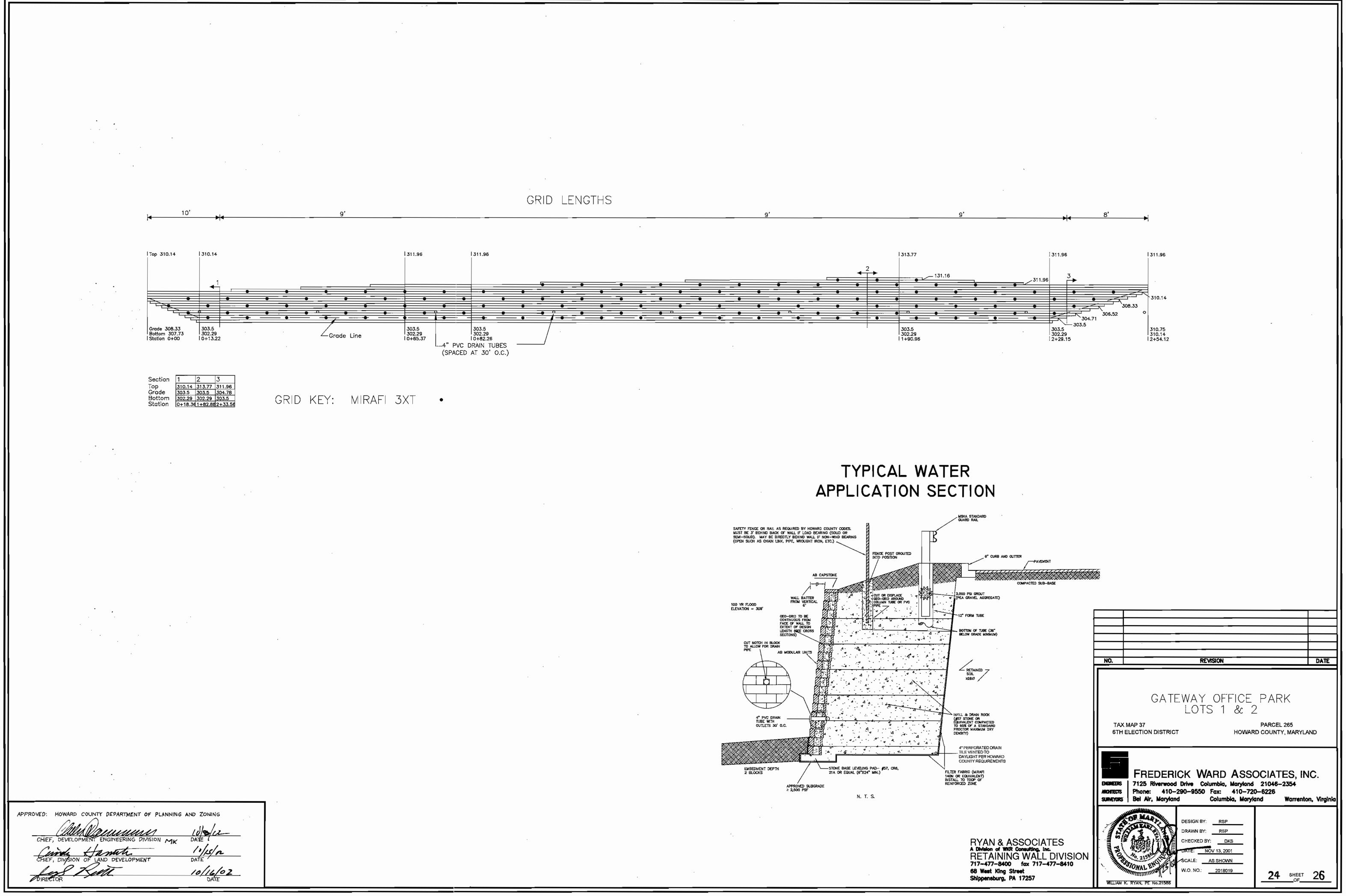
FREDERICK WARD ASSOCIATES, INC. ENGINEERS 7125 Riverwood Drive Columbia, Maryland 21046-2354

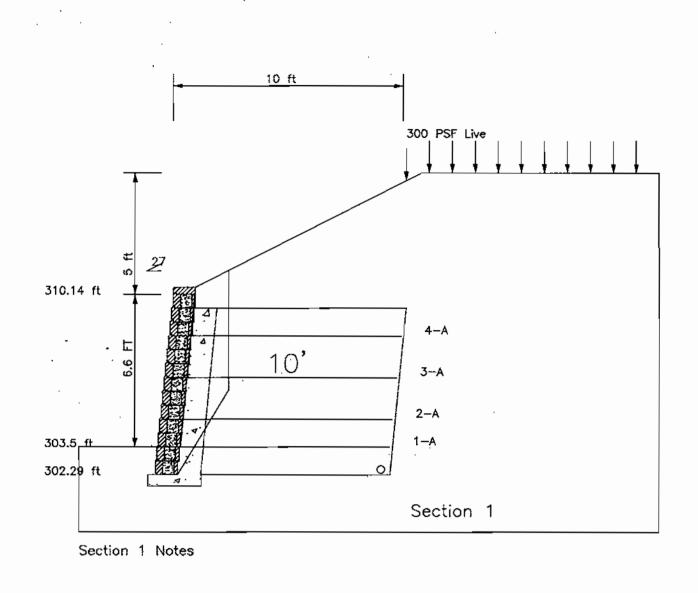
ARCHITECTS | Phone: 410-290-9550 Fax: 410-720-6226 SURVEYORS | Bel Air, Maryland | Columbia, Maryland | Warrenton, Virginia



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⋪	DESIGN BY.	MHM
١	DRAWN BY:	МНМ
ı	CHECKED BY:	RHV
I	DATE. APR.	19, 2002
١	SCALE:	1"=10'
1	WO NO:	2017165

23 SHEET 26





BLOCK DIMENSIONS

Total Wall Height = 7.85' Block Height = .604' Angle of Setback = 6° Depth of Block = .97' -Length of Block - 1.469'

SOIL PARAMETERS

Infill: Friction Angle = 28° Unit Weight = 120 PCF Retained: Friction Angle = 28° Unit weight = 120 PCF Foundation: Friction Angle = 28° Unit Weight = 120 PCF

BEARING CAPACITY FACTOR OF SAFETY = 3.35

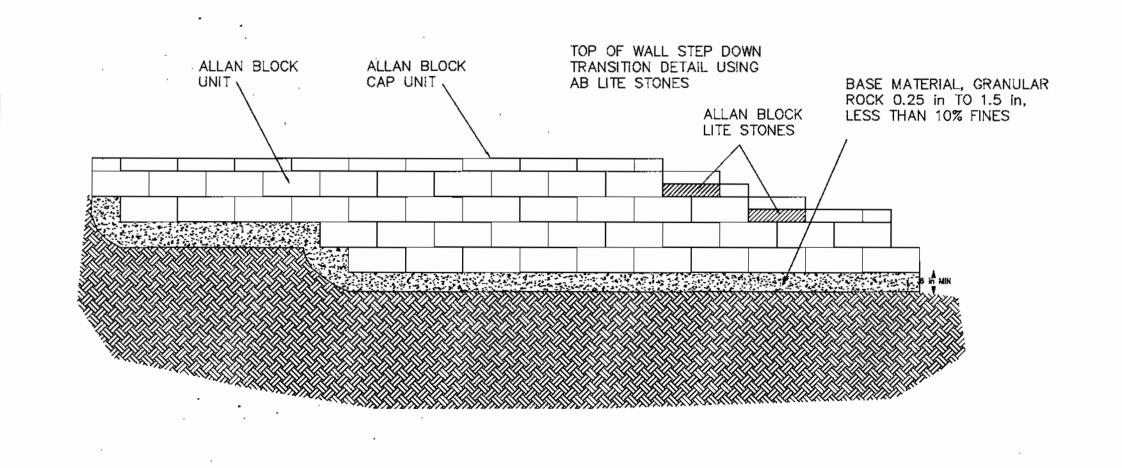
SAFETY FACTORS STATIC & SEISMIC

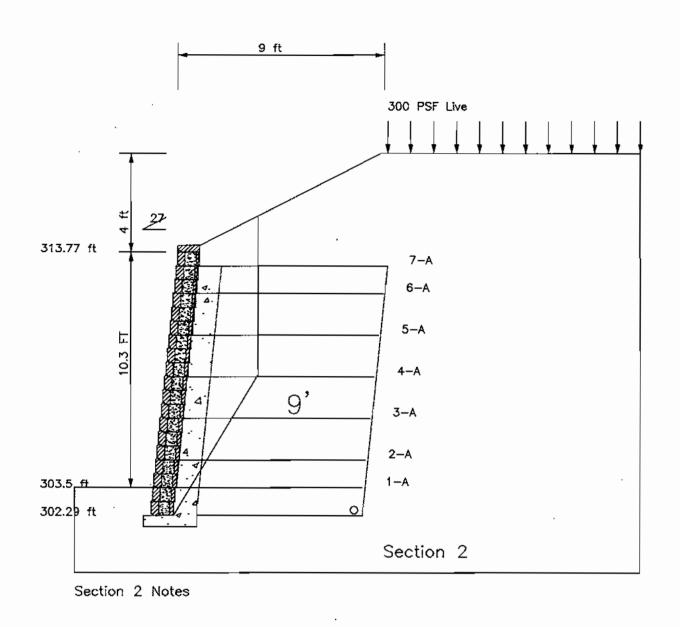
Minimum Sliding =1.5 Actual Sliding = 1.783 Minimum Overturning = 2.0 Actual Overturning = 4.956

A-Miragrid 3XT B-Miragrid 5XT C-Miragrid 7XT

MAXIMUM BEARING PRESSURE = 1,304 PSF

ALLAN BLOCK STEP DOWN TYPICAL DETAIL





BLOCK DIMENSIONS

Total Wall Height = 11.48' Block Height = .604' Angle of Setback = 6° Depth of Block = .97' Length of Block - 1.469'

SOIL PARAMETERS

Unit Weight = 120 PCF Infill: Friction Angle = 28° Retained: Friction Angle = 28° Unit weight = 120 PCF Foundation: Friction Angle = 28° Unit Weight = 120 PCF

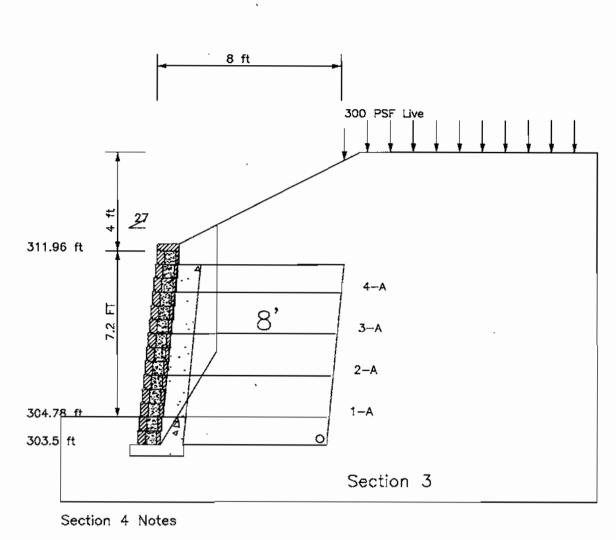
BEARING CAPACITY FACTOR OF SAFETY = 2.19

SAFETY FACTORS STATIC & SEISMIC

Minimum Sliding =1.5 Actual Sliding = 1.823 Minimum Overturning = 2.0 Actual Overturning = 3.921

A-Miragrid 3XT B-Miragrid 5XT C-Miragrid 7XT

MAXIMUM BEARING PRESSURE = 1,988 PSF



BLOCK DIMENSIONS Total Wall Height = 8.46' Block Height = .604' Depth of Block = .97' Angle of Setback = 6° Length of Block - 1.469'

SOIL PARAMETERS

Infill: Friction Angle = 28* Unit Weight = 120 PCF Retained: Friction Angle = 28° Unit weight = 120 PCF Foundation: Friction Angle = 28° Unit Weight = 120 PCF

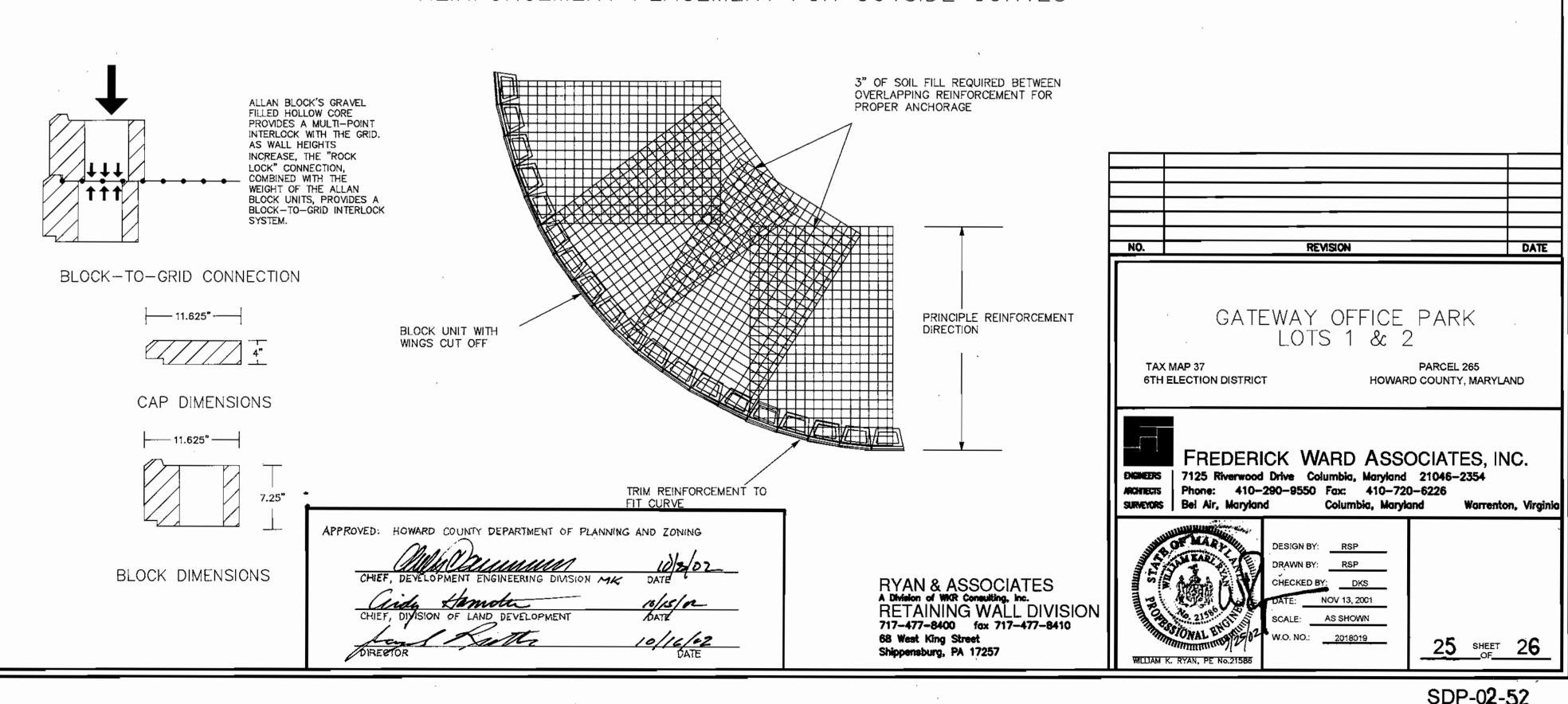
BEARING CAPACITY FACTOR OF SAFETY = 2.94 SAFETY FACTORS STATIC & SEISMIC

Minimum Stiding =1.5 Actual Sliding = 1.744 Minimum Overturning = 2.0 Actual Overturning = 4.114

A-Miragrid 3XT B-Miragrid 5XT C-Miragrid 7XT

MAXIMUM_BEARING PRESSURE = 1,485 PSF

REINFORCEMENT PLACEMENT FOR OUTSIDE CURVES



DATE

SDP-02-52

SPECIFICATIONS FOR SEGMENTAL RETAINING WALL SYSTEMS

PART 1: GENERAL

1.01 Description

A. Work includes furnishing and installing segmental retaining wall (SRW) units to the lines and grades designated on the construction drawings. Also included is furnishing and installing appurtenant materials required for construction of the retaining wall as shown on the construction drawings.

1.02 Reference Standards

Α.	ASTM C 140-	Sampling and Testing Concrete Masonry Units
В.	ASTM D 4595-	Tensile Properties of Geotextiles by the Wide-Width Strip Method.
C.	ASTM D 5262-	Test Method for Evaluating the Unconfined Creep Behavior of Geo- Grid
\mathbf{D}_{\cdot}	GRI:GG1-	Single Rib Geogrid Tensile Strength
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GRI:GG5- Geogrid Pullout
 ASTM D 698- Moisture Density Relationship for Soils, Standard Method

G. ASTM D 422- Gradation of Soils

ASTM 4318- Atterberg Limits of Soil
ASTM 3034- Specification for Polyvinyl Chloride (PVC) Plastic Pipe

ASTM D1248- Specification for Corrugated Plastic Pipe

PART 2: MATERIALS

2.01 Segmental Retaining Wall Units

A. SRW units shall be machine formed, Portland Cement concrete blocks specifically designed for retaining wall applications. SRW unit currently approved for this project is:

Allan Block as manufactured by Nitterhouse Masonry Products

NOTE: Where Allan Block specifications and reference documents conflict with these specifications, these specifications hold precedence.

- B. SRW units shall be capable of being erected with the horizontal gap between adjacent units not exceeding 1/8". The units shall be uniformly square and not trapezoidal in shape.
- C. SRW units shall have a minimum 4" overlap of units on each successive course so that walls are interlocked and continuous.
- O. SRW units shall be sound and free of cracks or other defects that would interfere with the proper placing of the unit or significantly impair the strength or permanence of the structure. Cracking or excessive chipping may be grounds for rejection. Units showing cracks longer than 1/2" shall not be used within the wall. Units showing chips visible at a distance of 30 feet from the wall shall not be used within the wall.
- E. Concrete used to manufacture SRW units shall have a minimum 28 days compressive strength of 3,000 psi and a maximum moisture absorption rate, by weight, of 8% as determined in accordance with ASTM C 140. Compressive strength test specimens shall conform to the saw-cut coupon provisions of Section 5.2.4 of ASTM C140 with the following exception: Coupon shall be taken from the least dimension of the unit of a size and shape representing the geometry of the unit as a whole.
- F. SRW units' molded dimensions shall not differ more than $\pm 1/8$ inch from that specified, except height which shall be $\pm 1/16$ inch as measured in accordance with ASTM C140.

2.02 Geosynthetic Reinforcement

A. Geosynthetic reinforcement shall consist of geogrids or geotextiles as indicated on the design plans.

No grid substitutions shall be permitted without the approval of Ryan & Associates.

2.03 Leveling Pad

A. Unless otherwise noted on the cross sections, the leveling pad shall be 6" deep X 24" wide. Material for leveling pad shall consist of compacted sand, gravel, or a combination thereof. (Typical stone used for this pad is #57, CR6, 21A, etc.) The leveling pad should extend laterally at least a distance of 6 inches from the toe and heel of the lowermost SRW unit. In cases of poor bearing capacity or fill soils an enlarged, grid reinforced footer may be required. This typically consists of 1' deep X 4' wide with geogrid under and within the stone. Lean, un-reinforced concrete with strength of 1500 PSI and 6" deep may also be used as for the leveling pad.

2.04 Drainage Aggregate

A. Drainage aggregate shall be angular, clean stone or granular fill consisting of #57 or approved equal (i.e.- median stone size ½" to 1 ½"). Rounded, pea gravel is not permissible.

2.05 Drainage Pipe

- A. The drainage collection pipe shall be a 4" perforated or slotted PVC, or corrugated HDPE pipe.
- B. Drain pipes are mandatory and shall be vented to daylight at the end(s) of the wall or at a central low point of the wall. If this is not possible, vent through the wall above finished grade at maximum intervals of 30' O.C. In no case shall a continuous pipe be run for more than 300' without an outlet to daylight.

2.06 Reinforced (Infill) Soil: the reinforced geo-grid zone

- A. The soil used must meet or exceed the design friction angle noted on the design cross sections. The reinforced material shall be free of debris and organic material (i.e.- no trash, plants or root matter, top soil, etc.). Unless otherwise noted on the plans, the reinforced zone material shall <u>not</u> consist of CH (fat clay), MH (fat silt), or OH (organic) soils.
- B. Rocks may be used as infill material as long as their diameter is 6" or less. NOTE: when all gravel is used as infill the LTDS of the geo-grid must be reduced to account for additional installation damage from the large particles. Recycled concrete is permissible for infill.

2.07 Retained Soil: the area beyond the infill soil and extending to a distance of twice the exposed wall height

A. The soil used must meet or exceed the design friction angle noted on the design cross sections. Unless otherwise noted on the plans, the retained material shall <u>not</u> consist of CH (fat clay), MH (fat silt), or OH (organic) soils.

PART 3: CONSTRUCTION

3:01 Inspection

- The Owner or Owner's Representative is responsible for verifying that the contractor meets all the requirements of the specification. This includes all submittals for materials and design, qualifications, and proper installation of wall system.
- B. Contractor's field construction supervisor shall have demonstrated experience and be qualified to direct all work at the site.

3.02 Excavation

- A. Contractor shall excavate to the lines and grades shown on the project plans. Contractor shall take precautions to minimize over-excavation. Over-excavation shall be filled with compacted infill material or as directed by the site Geo-technical Engineer.
- B. Contractor shall verify location of existing structures and utilities prior to excavation. Contractor shall ensure all surrounding structures are protected from the effects of wall excavation. Excavation support (shoring), if required, is the responsibility of the Contractor

3.03 Foundation Preparation

- A. Following excavation, the foundation soil shall be examined by the Owner's Geotechnical Engineer to assure that the actual foundation soil strength meets or exceeds the allowable design bearing strength (this parameter can be found in the design's General Notes). Soils not meeting the required strength shall be removed and replaced with select structural fill compacted to 95% of a standard proctor for the full depth.
- B. If large deposits of fill are encountered an enlarged, grid-reinforced footer may be required.

4.04 Leveling Pad Construction

- A. Leveling pad shall be placed as shown on the construction drawings with a minimum thickness of 6" and a minimum width of 24". The leveling pad should at a minimum extend laterally at least a distance of 6 inches from the toe and heel of the lower most SRW Unit.
- B. Soil leveling pad material shall be compacted with a vibratory plate compactor to provide a firm, level-bearing surface on which to place the first course of units. Compaction will be with mechanical plate compactors to achieve 95% of maximum standard proctor density (ASTM D 698). A thin layer (not to exceed ½") of well-graded sand or stone dust can be used to smooth the top of the leveling pad.

4.05 SRW Unit Installation

- All SRW units shall be installed at the proper elevation and orientation as shown on the wall profiles and details on the construction plans. The SRW units shall be installed in general accordance with the manufacturer's recommendations. The design engineer of record (Ryan & Associates) specifications and drawings shall govern in any conflict between the two requirements.
- B. First course of SRW units shall be placed on the leveling pad. The units shall be leveled side-to-side, front-to-rear and with adjacent units, and aligned to ensure intimate contact with the leveling pad. The first course is the most important to ensure accurate and acceptable results. No gaps shall be left between the front of adjacent units. Alignment may be done by means of a string line or offset from base line to the back of the units.
- C. Clean all excess debris from top of units and install next course.
- D. Lay out of curves and corners shall be installed in accordance with the plan details or in general accordance with SRW manufacturer's installation guidelines. Walls shall be interlocked by overlapping successive courses. Continuous vertical joints are not permitted unless glued. In general, all tangent angles shown on the civil drawings should be changed into curves to enhance the wall's strength and appearance. Inside and outside corners may be constructed without compromising the wall's integrity.
- E. Repeat procedures to extent of wall height.
- F. The wall face cant shall not differ more than ± 2 degrees from that specified.
- G. Embedment shall follow the general rule of 1" buried for every 1' of wall exposed when the front slope is 4:1 or greater. For 3:1 front slopes a minimum of 21" shall be buried, and for 2:1 front slopes a minimum of 29" shall be buried.

4.06 Geosynthetic Reinforcement Placement

- A. All geosynthetic reinforcement shall be installed at the proper elevation and orientation as shown on the wall profiles and details on the final construction plans. Partial grid coverage is not acceptable- no gaps shall be present between grid sections.
- B. At the elevations shown on the plans, the geosynthetic reinforcement shall be laid horizontally on compacted infill and on top of the concrete SRW units. Embedment of the geosynthetic in the SRW units shall be consistent with SRW manufacturer's recommendations. Correct orientation of the geosynthetic reinforcement shall be verified by the Contractor to be in accordance with the geosynthetic manufacturer's recommendations. The highest strength direction of the geosynthetic must be perpendicular to the wall face.
- C. Geosynthetic reinforcement layers shall be one continuous piece for their entire embedment length. Overlap of the geosynthetic in the design strength direction (perpendicular to the wall face) is not permitted.
- D. Tracked construction equipment shall not be operated directly on the geosynthetic reinforcement. A minimum of 6 inches of backfill is required prior to operation of tracked vehicles over the geosynthetic. Turning should be kept to a minimum. Rubber-tired equipment may pass over the geosynthetic reinforcement at slow speeds (less than 5 mph).
- E. The geosynthetic reinforcement shall be in tension and free of wrinkles prior to placement of soil fill. The nominal tension shall be applied to the reinforcement and secured in place with staples, stakes or by hand tensioning until reinforcement is covered by six inches of fill.

4.07 Drainage Materials

- A. Drainage aggregate shall be installed to the line, grades, and sections shown on the final plans.

 Drainage fill shall be placed to the minimum thickness of 12" as shown on the construction plans behind units. Drainage fill shall also fill all voids between and within (if hollow) the units.
- Drainage collection pipes shall be installed to maintain gravity flow of water outside the reinforced soil zone. The drainage collection pipe shall daylight into a storm sewer manhole or along a slope at an elevation lower than the lowest point of the pipe within the aggregate drain (see section 2.05).
- All drainage zone aggregate, including the stone placed within the block cells shall be compacted with a vibratory plate compactor with a minimum of two passes.

4.08 Backfill Placement

- A. The reinforced backfill shall be placed as shown in the construction plans in the maximum compacted lift thickness of 10 inches and shall be compacted to a minimum of 95% of standard proctor density (ASTM D 698) at a moisture content within 2% of optimum. The backfill shall be placed and spread in such a manner as to eliminate wrinkles or movement of the geosynthetic reinforcement and the SRW units. Compaction testing shall be done at 25%, 50%, 75%, and 100% of the wall height or as specified by the site geo-technical engineer.
- Only a vibratory plate or small-scale vibratory smooth drum compactor equipment shall be allowed within 3 feet of the front of the wall face. Compaction within the 3 feet behind the wall face shall be achieved by at least three (3) passes of the lightweight mechanical plate compactor or roller. Heavy equipment (such as track hoes, ride on rollers, pans, etc.) must be kept back a minimum of 3' from the rear of the wall.
- C. At the end of each day's operation, the Contractor shall slope the last level of backfill away from the wall facing to direct water runoff away from the wall face.

- D. At completion of wall construction if final grading, paving, landscaping, and/or storm drainage installation adjacent to the wall is not placed immediately after wall completion, temporary grading shall be provided to ensure water runoff is not allowed to <u>collect or pond</u> behind the wall until final construction adjacent to the wall is completed.
- E. Filter fabric is neither required nor recommended behind the drainage layer. Installation of filter fabric has proven to result in poor wall construction and its benefit has not been proven when used with clays, silts, and mixed soils. The exception is when all sand is used for infill material since it is non-cohesive and could potentially slough, clogging the drainage layer.

4.09 SRW Caps

A. SRW caps shall be properly aligned and glued to underlying units with a flexible, high-strength concrete adhesive (adhesive should be designed for "concrete to concrete" applications). Rigid adhesive or mortar is not acceptable.

4.10 Water Applications

A. When walls are installed in water applications (such as storm water ponds, streams, bulkheads, areas adjacent to flood plains, etc.) all granular material must be used as infill up to 1' above the 100 year flood elevation or the high water level. This material must be free draining and have less than 10% fines. The leveling pad and the reinforced zone (up to the extent of the stone infill) must be wrapped in filter fabric to prevent migration of fines. Rip rap stone is required in front of the bottom three course on walls installed in tidal waters. Rip rap may also be required to prevent scouring and erosion in front of walls installed in water sources prone to fluctuating water levels, and where pipes that frequently carry water exit through walls.

4.11 Rails, Fences, & Other Structures

- A. Open rails and fences not subject to wind loads may be placed directly behind the wall as long as they are not subject to vehicular impact. Solid or semi-solid fences that are subject to wind loads must be kept back a minimum of 3° from the rear of the wall to prevent loading of the wall.
- B. Guardrails subject to vehicular impact must be kept back a minimum of 3' to prevent loading of the wall. Guardrails may be placed closer than this 3' minimum only if a barrier (such as wheel stops, curbing, etc.) prevents impact.
- Light posts and similar structures subject to wind loads must be kept back a minimum of 3' to prevent loading of the wall.
- D. In cases where this 3' minimum cannot be met due to restraints on the site, additional analyses will need to be done to determine a method of stabilization. Ryan & Associates can be contracted to provide this design for an additional cost.

4.12 Storm Structures

- A. RCP pipes may pass through the wall without compromising the design. The SRW units may be cut to fit around the pipe and the void filled with non-shrink grout or type "M" mortar. A concrete collar may be cast around the structure if desired. When a collar is cast, the top of the collar shall line up with an even block course to maintain proper alignment and neat workmanship. Corrugated steel pipes may not be able to support the wall's weight and may require a concrete beam. Check load capabilities with the pipe manufacturer.
- B. When a pipe is located in or below the leveling pad a grade beam may be required. Ryan & Associates shall be consulted to determine the size, strength and reinforcing of the beam.
- B. Concrete storm structures may be located behind a wall and within the reinforced zone as dictated by the project's civil drawings. If the structure(s) cannot be moved out of the reinforced zone and the grid installed to the full design length the following shall apply. On small structures (such as manholes, collection boxes, concrete pipes less than 20" O.D., etc.) it is acceptable to shorten the grid from the design length and meet the structure. The area between the wall and structure must be filled with #57 stone or equal- not the site soil. On large structures and in cases where pipes parallel the wall for long distances, Ryan & Associates shall be consulted to determine the impact on the wall before allowing this to be done

4.13 Construction Adjacent to Completed Wall

- A. The Owner or Owner's Representative is responsible for ensuring that construction adjacent to the wall by others does not disturb the wall or place temporary construction loads on the wall that exceed design loads, including loads such as water pressure, temporary grades, or equipment loading. Heavy paving or grading equipment shall be kept a minimum of three feet behind the back of the wall face. Equipment with wheel loads in excess of 150 psf live load shall not be operated with 10 feet of the face of the retaining wall during construction adjacent to the wall. Care should be taken by the General Contractor to ensure water runoff is directed away from the wall structure until final grading and surface drainage collection systems are completed.
- Care must be taken when installing appurtenances (such as transformers, generators, etc.) within the reinforced zone of the wall. The compaction integrity of the reinforced zone must be maintained, both below and beside (around) the appurtenance. Neglecting to do so may cause hydrostatic pressure and wall failure.

END OF SECTION

Revised 01-02-01

REVISION DATE GATEWAY OFFICE PARK LOTS 1 & 2 TAX MAP 37 PARCEL 265 **6TH ELECTION DISTRICT** HOWARD COUNTY, MARYLAND FREDERICK WARD ASSOCIATES, INC. ENGNEERS | 7125 Riverwood Drive Columbia, Maryland 21046-2354 Phone: 410-290-9550 Fax: 410-720-6226 SURVEYORS | Bel Air, Maryland Columbia, Maryland Warrenton, Virginia SCALE: AS SHOWN 2018019

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

CHIEF, DEVELOPMENT ENGINEERING DIVISION MK DATE

CHIEF, DWISION OF LAND DEVELOPMENT

DIRECTOR

DIRECTOR

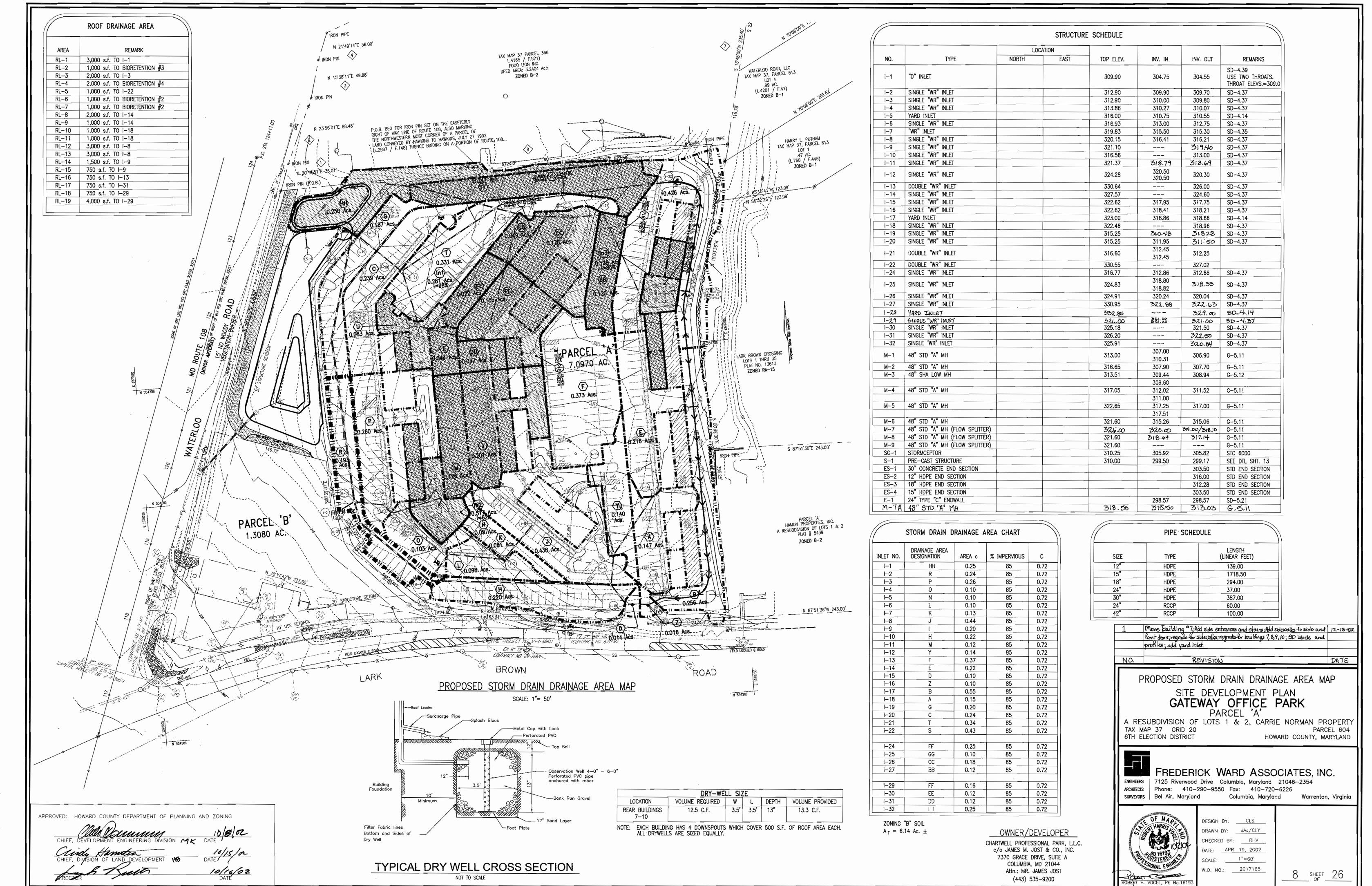
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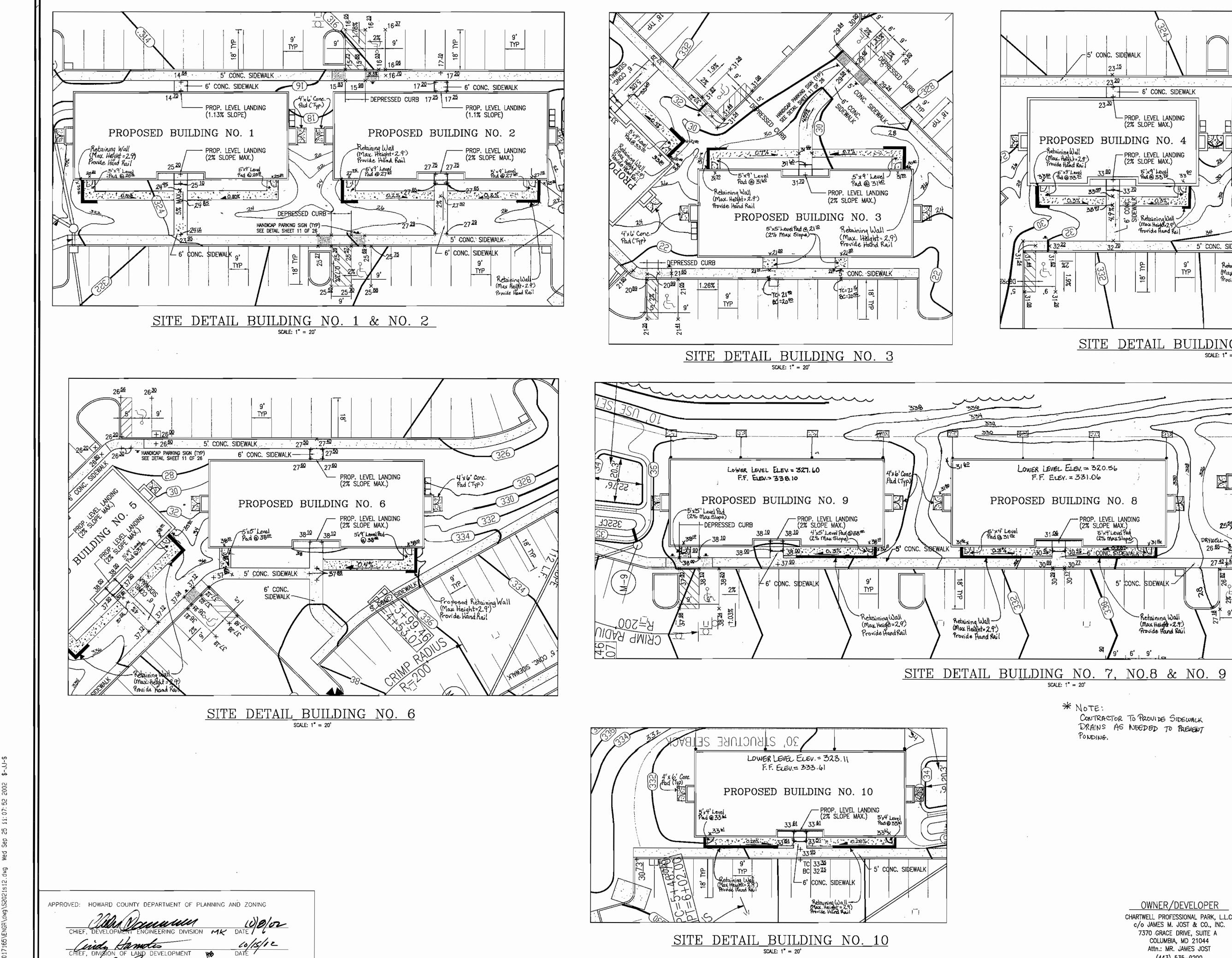
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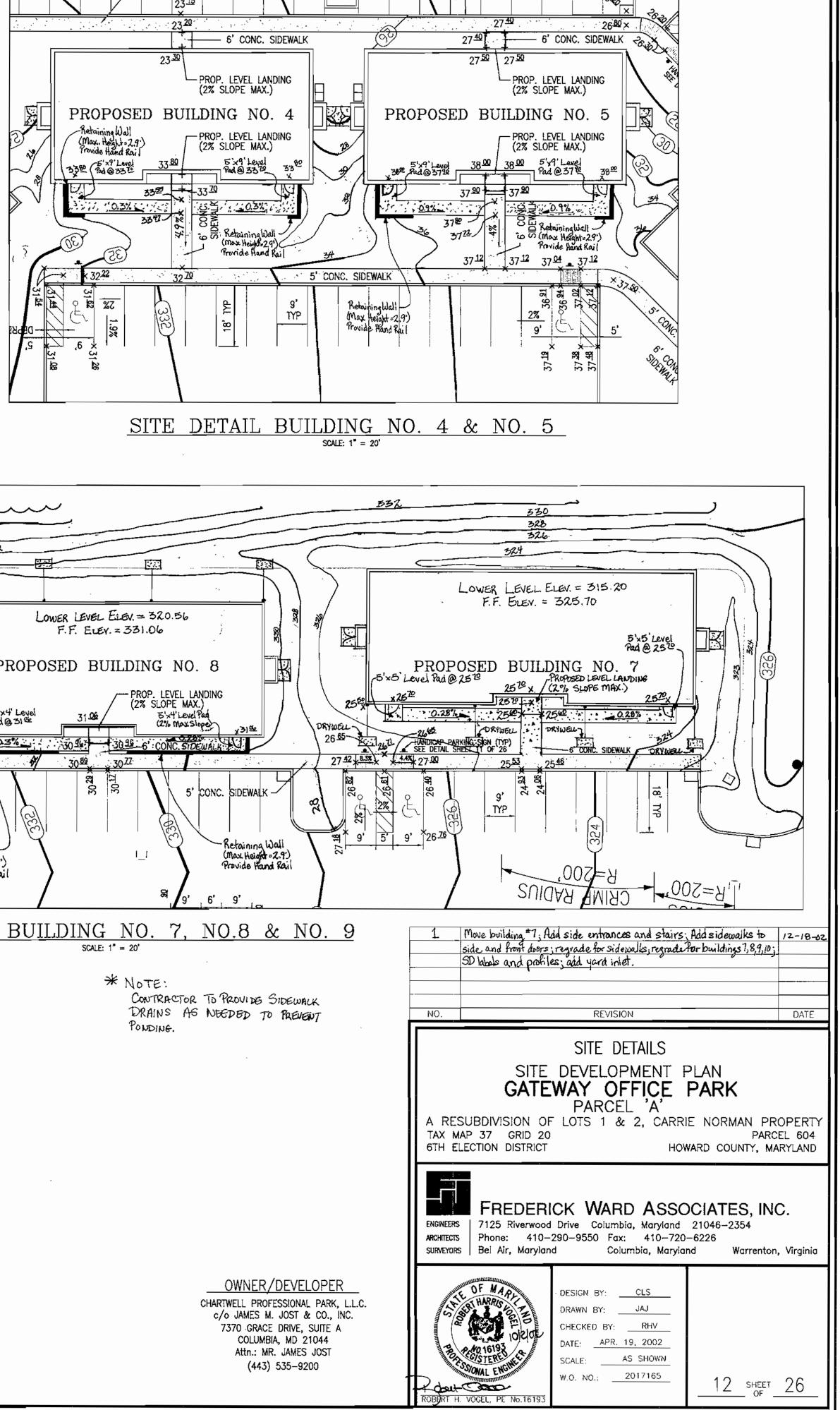
DATE

RYAN & ASSOCIATES
A Division of WKR Consulting, Inc.
RETAINING WALL DIVISION
717-477-8400 fax 717-477-8410
68 West King Street
Shippensburg, PA 17257

SDP-02-52

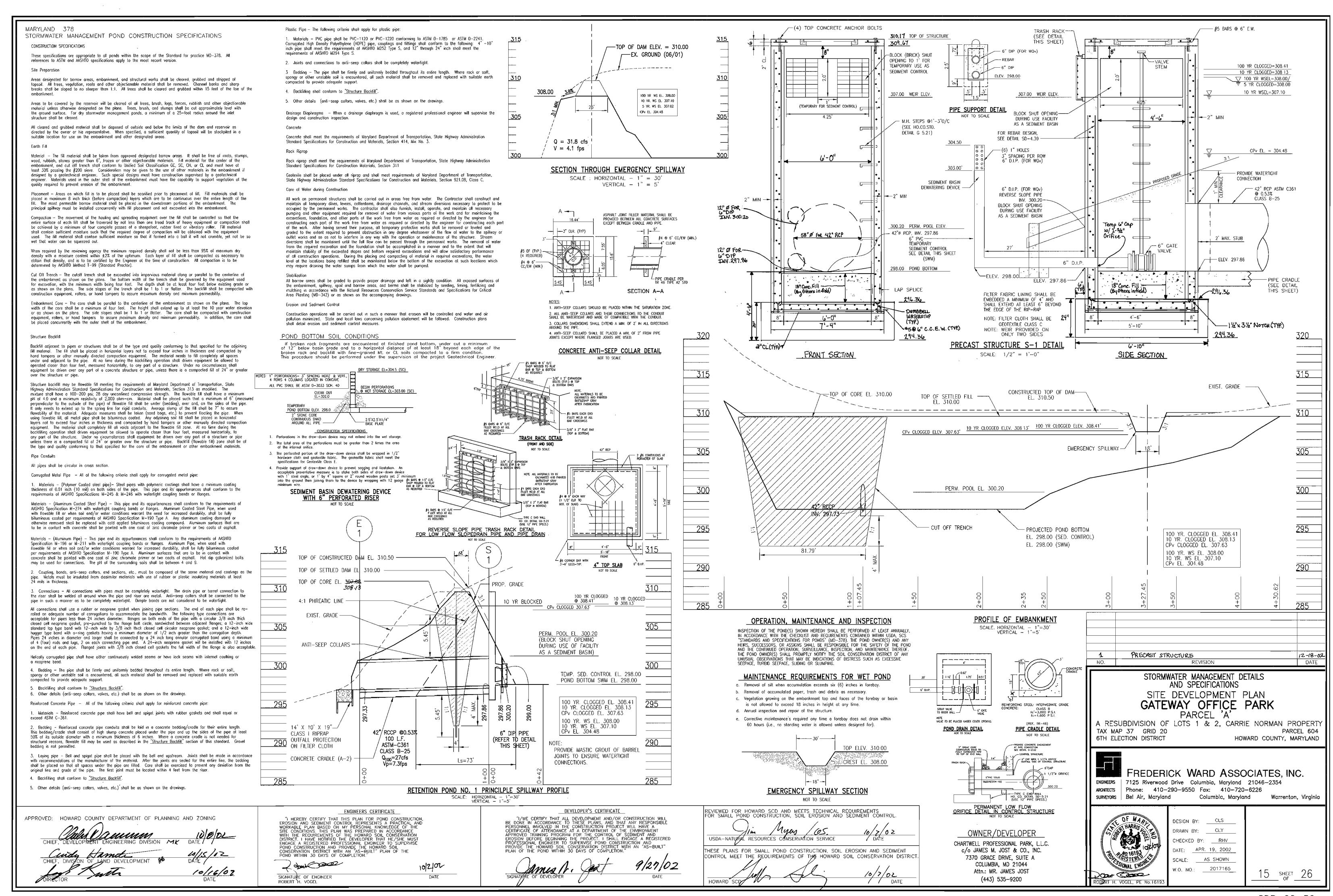


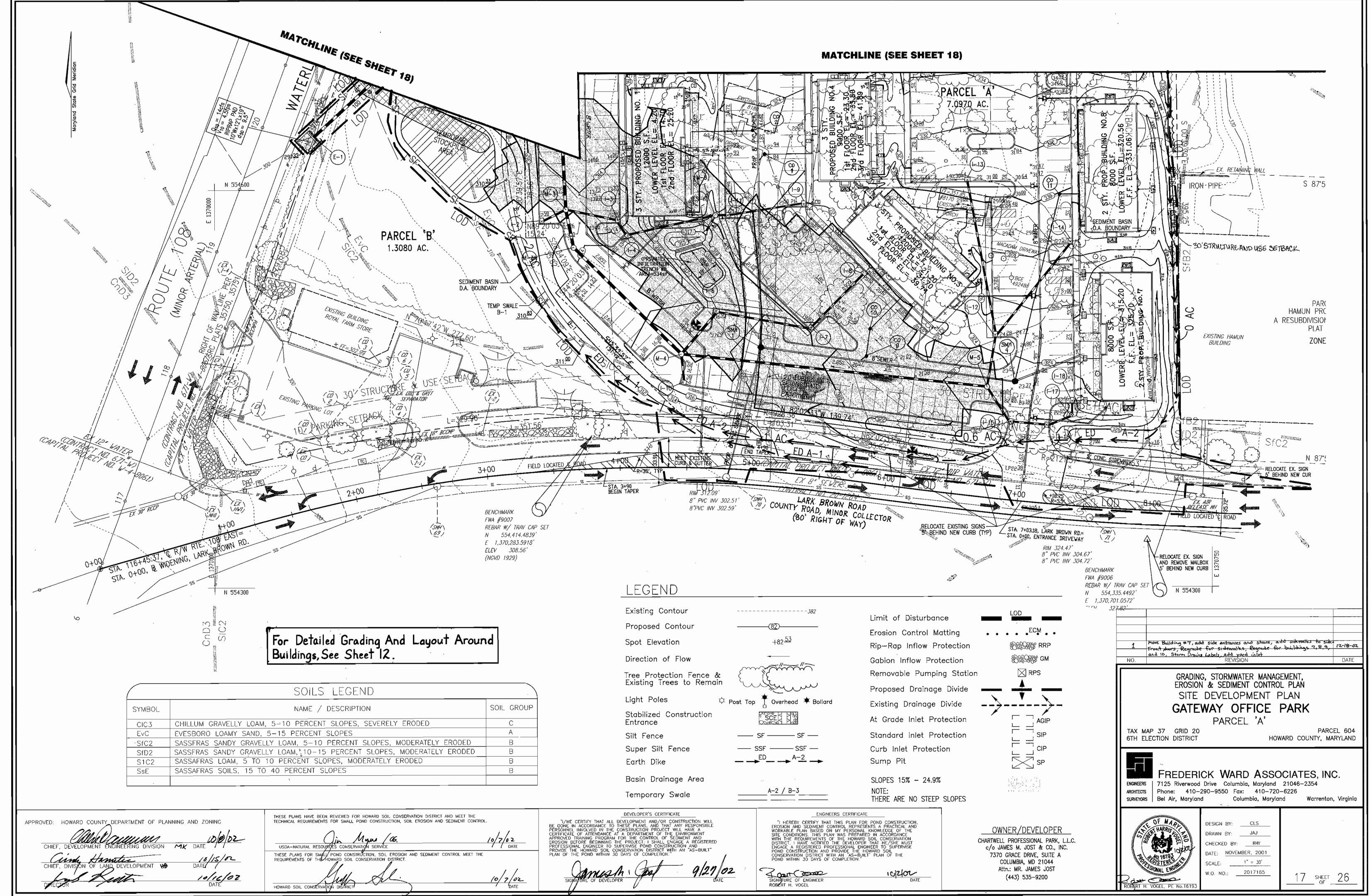




10/16/02







A 18" METAL INV 317.15'

B 15" METAL INV 317.41"

C 18" METAL INV 317.21"

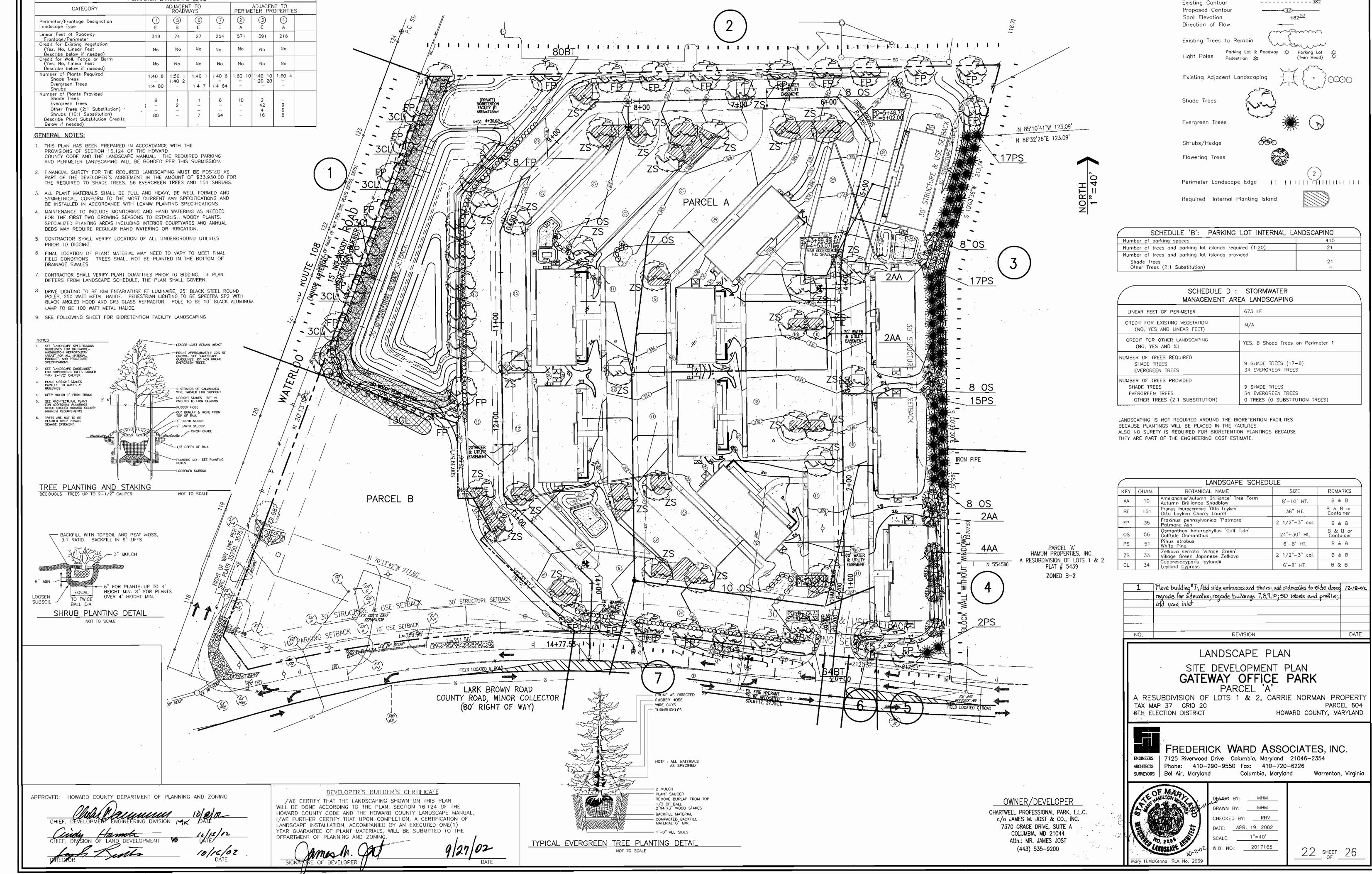
Gateway Office Park LLC

September 12, 2001

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N 23°56'01"E 88.48'

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SCHEDULE A PERIMETER LANDSCAPE EDGE

LEGEND

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