

GENERAL NOTES

- ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST STANDARDS AND SPECIFICATIONS OF HOWARD COUNTY PLUS MSHA STANDARDS AND SPECIFICATIONS IF APPLICABLE.
- THE BOUNDARY SHOWN IS BASED ON A MONUMENTED FIELD-RUN SURVEY PERFORMED BY BENCHMARK ENGINEERING, INC., DATED JANUARY, 2012.
- THE COORDINATES SHOWN HEREON ARE BASED UPON THE HOWARD COUNTY GEODETIC CONTROL MONUMENTS 40FA & 40FB, WHICH ARE BASED ON THE MARYLAND STATE PLANE COORDINATE SYSTEM.
- EXISTING TOPOGRAPHY IS TAKEN FROM FIELD RUN BY BENCHMARK ENGINEERING, INC., ON OR ABOUT JANUARY 2012. CONTOUR INTERVAL IS 2 FEET.
- THE CONTRACTOR SHALL NOTIFY "MISS UTILITY" AT 1-800-257-7777 AT LEAST 48 HOURS PRIOR TO ANY EXCAVATION WORK BEING DONE.
- THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS/BUREAU OF ENGINEERING/CONSTRUCTION INSPECTION DIVISION AT (410) 313-1880 AT LEAST FIVE (5) WORKING DAYS PRIOR TO THE START OF WORK.
- FOREST STAND DELINEATION PLAN WAS PREPARED BY ECO-SCIENCE PROFESSIONALS, INC., DATED MARCH 14, 2012, AND APPROVED UNDER SP-12-004.
- SPECIMEN TREES WERE FOUND ON-SITE AS PER FOREST STAND DELINEATION PLAN PREPARED BY ECO-SCIENCE PROFESSIONALS, INC., DATED MARCH 14, 2012, AND APPROVED UNDER SP-12-004. NO SPECIMEN TREES WILL BE REMOVED FOR THIS DEVELOPMENT.
- WETLANDS EXIST WITHIN THE LIMITS OF DISTURBANCE AS SHOWN ON THE WETLAND DELINEATION REPORT PREPARED BY ECO-SCIENCE PROFESSIONALS, INC., DATED MARCH 14, 2012, AND APPROVED UNDER SP-12-004.
- A NOISE STUDY IS NOT REQUIRED FOR THIS PROJECT.
- A.P.F.O. TRAFFIC STUDY WAS PREPARED BY THE MARS GROUP, INC., DATED MARCH, 2012, AND APPROVED UNDER SP-12-004.
- THE EXISTING PERCOLATION TEST LOCATIONS WILL PROVIDE ADEQUATE INFORMATION FOR SEVERAL OF THE MICRO-SCALE PRACTICES USED FOR STORMWATER MANAGEMENT. A GEOTECHNICAL REPORT FOR THIS PROJECT WAS PREPARED BY GEOTECHNICAL LABORATORIES, INC., DATED APRIL, 2012 FOR THE REMAINING MICRO-SCALE PRACTICES.
- THERE ARE EXISTING STRUCTURES LOCATED ON-SITE TO BE REMOVED. ONE EXISTING DWELLING ON SITE IS TO REMAIN.
- TO THE BEST OF OUR KNOWLEDGE, THERE ARE NO CEMETERY LOCATIONS ON-SITE.
- THIS PLAN IS SUBJECT TO THE AMENDED 5th EDITION OF THE HOWARD COUNTY SUBDIVISION REGULATIONS.
- STORMWATER MANAGEMENT SHALL BE PROVIDED FOR THIS PROJECT BASED ON GUIDELINES ESTABLISHED BY THE 2000 MARYLAND STORMWATER DESIGN MANUAL VOLUMES I & II AS AMENDED BY THE STORMWATER MANAGEMENT ACT OF 2007. ENVIRONMENTAL SITE DESIGN METHODS SHALL BE UTILIZED AS FOLLOWS: THE ROADS ARE TREATED BY NON-ROOFTOP DISCONNECT, GRASS SWALES AND MICRO-BIORETENTION FACILITIES. DRIVEWAY AREAS MAY BE TREATED BY DISCONNECT, OR ON-LOT MICRO-BIORETENTION. SOME SHARED DRIVEWAY SECTION WILL BE TREATED IN MICRO-BIORETENTION FACILITIES C & D. ALL ON-LOT STORMWATER MANAGEMENT FACILITIES AND CREDITS ARE SUBJECT TO THE REQUIREMENTS OF RECORDED DOC.*
- THE TOTAL FOREST CONSERVATION OBLIGATION AMOUNT OF 14.73 ACRES SHALL BE MET BY THE ON-SITE RETENTION OF 6.93 AC. WITHIN A FOREST CONSERVATION EASEMENT AND THE ON-SITE AFForestation AND REFORESTATION OF 7.80 AC. WITHIN A FOREST CONSERVATION EASEMENT. THE DEVELOPER SHALL BOND IN ACCORDANCE WITH THE DEPARTMENT OF PUBLIC WORKS DEVELOPER'S AGREEMENT WITH SURETY IN THE AMOUNT OF \$169,884.00 (339,768SF*0.50).
- STREET LIGHTS ARE NOT REQUIRED.
- ALL LANDSCAPING REQUIREMENTS SHALL BE FULFILLED IN ACCORDANCE WITH THE HOWARD COUNTY CODE, SECTION 16.124 AND THE LANDSCAPE MANUAL.
- FOR FLAG OR PIPESTEM LOTS, REFUSE COLLECTION, SNOW REMOVAL AND ROAD MAINTENANCE ARE PROVIDED TO THE JUNCTION OF THE FLAG OR PIPESTEM AND ROAD RIGHT-OF-WAY AND NOT TO THE PIPESTEM LOT DRIVEWAY.
- DRIVEWAYS SHALL BE PROVIDED PRIOR TO ISSUANCE OF A USE AND OCCUPANCY PERMIT FOR ANY NEW DWELLINGS FOR FIRE AND EMERGENCY VEHICLES PER THE FOLLOWING MINIMUM REQUIREMENTS:
 - WIDTH - 12' (16' SERVING MORE THAN ONE RESIDENCE).
 - SURFACE - 6" OF COMPACT CRUSHER RUN BASE WITH TAR AND CHIP COATING.
 - GEOMETRY - MAX. 1.5% GRADE. MAX. 10% GRADE CHANGE & MIN. 45' TURNING RADIUS.
 - STRUCTURES (CULVERTS/BRIDGES) - CAPABLE OF SUPPORTING 25 GROSS TONS (H2S LOAD).
 - DRAINAGE ELEMENTS - CAPABLE OF SAFELY PASSING 100 YEAR FLOODPLAIN WITH NO MORE THAN 1 FOOT DEPTH OVER DRIVEWAY.
 - STRUCTURE CLEARANCES - MINIMUM 12 FEET.
 - MAINTENANCE - SUFFICIENT TO INSURE ALL WEATHER USE.
- TRAFFIC CONTROL DEVICES, MARKINGS AND SIGNING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). ALL STREET AND REGULATORY SIGNS SHALL BE IN PLACE PRIOR TO THE PLACEMENT OF ANY ASPHALT.
- SUBJECT PROPERTY IS ZONED RR-DEO PER OCTOBER 23, 2013 COMPREHENSIVE ZONING PLAN.
- APPLICABLE HOWARD COUNTY DPZ FILE REFERENCES FOR THIS PROJECT INCLUDE: ECP-12-045, WP-13-025, SP-12-004, F-11-114
- FINANCIAL SURETY IN THE AMOUNT OF \$33,000.00 FOR THE REQUIRED STREET TREES AND \$11,400.00 FOR THE REQUIRED PERIMETER LANDSCAPING SHALL BE POSTED AS PART OF THE DPW DEVELOPER'S AGREEMENT.
- WRITTEN AUTHORIZATION BY BGE OF THE PROJECT LANDSCAPE PLAN WILL BE OBTAINED PRIOR TO SIGNATURE APPROVAL OF ANY ASPHALT.
- THE INTENDED USE OF THE PRESERVATION PARCELS: BUILDABLE PRESERVATION PARCEL "A" IS A PRIVATELY OWNED RESIDENCE, PRIVATELY OWNED WITH HOWARD COUNTY AND THE HOA AS EASEMENT HOLDERS. NON-BUILDABLE PRESERVATION PARCELS "B", "C" AND "D" ARE FOR THE PROTECTION OF ENVIRONMENTAL FEATURES AND TO BE PRIVATELY OWNED WITH HOWARD COUNTY AND THE HOA AS EASEMENT HOLDERS. NON-BUILDABLE PRESERVATION PARCEL "E" IS A PUBLIC OWNED SEWERAGE EASEMENT, TO BE OWNED BY HOWARD COUNTY, MARYLAND, WITH THE HOA AS AN EASEMENT HOLDER.
- THERE IS A FLOODPLAIN LOCATED ON-SITE BASED ON A FLOODPLAIN ANALYSIS PREPARED BY BENCHMARK ENGINEERING, INC., DATED MARCH 2012, REVISED JULY, 2012.
- THE EXISTING 22' PAVED SECTION OF POINT RIDGE DRIVE, AS SHOWN ON F-95-06, WAS CARRIED THROUGH TO THE INTERSECTION WITH PLEASANT SPRINGS COURT FROM STATION 0+00 TO STATION 2+73 EXCEEDS 200' BUT NO EXPANSION TO THE WIDTH IS PROPOSED AS MOST OF THIS ROADWAY IS EXISTING.
- THIS PROPERTY IS NOT WITHIN THE METROPOLITAN DISTRICT. WATER AND SEWER SHALL BE PRIVATE FOR LOTS 1-10, LOTS 17-22 AND PRESERVATION PARCEL "A". WATER SHALL BE PRIVATE AND SEWER SHALL BE PUBLIC FOR LOTS 11-16.
- EXISTING UTILITIES SHOWN HEREON ARE BASED ON FIELD LOCATIONS AND RECORD DRAWINGS.
- A PRIVATE RANGE OF ADDRESS SIGN SHALL BE FABRICATED AND INSTALLED BY HOWARD COUNTY BUREAU OF HIGHWAYS AT THE DEVELOPERS/OWNERS EXPENSE FOR EACH OF THE USE-IN-COMMON DRIVEWAYS. CONTACT HOWARD COUNTY TRAFFIC DIVISION AT 401-313-2430 FOR DETAILS AND COST ESTIMATE.
 - THE R1-1 (STOP) SIGNS AND THE STREET NAME SIGN (SNS) ASSEMBLIES FOR THIS DEVELOPMENT MUST BE INSTALLED BEFORE THE BASE PAVING IS COMPLETE. B) THE TRAFFIC CONTROL SIGNS SHOWN ON THESE PLANS ARE APPROXIMATE AND MUST BE FIELD APPROVED BY HOWARD COUNTY TRAFFIC DIVISION (410-313-2430) PRIOR TO THE INSTALLATION OF ANY OF THE TRAFFIC CONTROL DEVICES. C) ALL TRAFFIC CONTROL DEVICES ARE THEIR LOCATIONS SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). D) ALL SIGN POSTS USED FOR TRAFFIC CONTROL SIGNS INSTALLED IN THE CURB RIGHT-OF-WAY SHALL BE MOUNTED ON A 2" GALVANIZED STEEL TUBULAR (QUICK PUNCH), SQUARE TUBE POST (14 GAUGE) INSERTED INTO A 2-1/2" GALVANIZED STEEL PERFORATED SQUARE TUBE SLEEVE (12 GAUGE) - 3' LONG. THE ANCHOR SHALL NOT EXTEND THAN TWO QUICK PUNCH HOLES ABOVE GROUND LEVEL. A GALVANIZED STEEL POLE CAP SHALL BE MOUNTED ON TOP OF EACH POST.
- AN INVESTIGATION OF DESIGN ALTERNATIVES HAS BEEN PROVIDED AT THE FINAL PLAN SUBMISSION FOR EXTENDING POINT RIDGE ROAD AND PLEASANT SPRINGS COURT IN ORDER TO ELIMINATE OR SHORTEN THE LOT PIPESTEMS. NO VIABLE ALTERNATIVES WERE FOUND.
- WAVER PETITION WP-13-025. A REQUEST TO WAIVE SUBSECTIONS 16.115(C) AND 16.116(A) OF THE HOWARD COUNTY SUBDIVISION AND LANDSCAPING REGULATIONS. THE PLANNING DIRECTOR APPROVED THE WAVER REQUEST ON FEBRUARY 8, 2013 SUBJECT TO THE FOLLOWING CONDITIONS:
 - THE PETITIONER SHALL OBTAIN AUTHORIZATION BY THE MARYLAND DEPARTMENT OF THE ENVIRONMENT AND U.S. ARMY CORPS OF ENGINEERS FOR ACTIVITIES PROPOSED IN STREAMS, WETLANDS, THE WETLAND BUFFER AND THE 100-YEAR FLOODPLAIN.
 - THE PETITIONER SHALL NOT STORE OR DISCARD BUILDING MATERIALS OR OTHER DEBRIS WITHIN THE 100-YEAR FLOODPLAIN WITHOUT PRIOR WRITTEN AUTHORIZATION BY THE HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING AND THE MARYLAND DEPARTMENT OF THE ENVIRONMENT.
 - THE PETITIONER SHALL ENSURE THAT ALL GRADING AND ROAD CONSTRUCTION ACTIVITIES WITHIN THE WETLANDS, STREAMS, BUFFERS AND 100-YEAR FLOODPLAIN THE MINIMUM NECESSARY TO ACCOMMODATE THE PROPOSED ROAD CROSSINGS AND SIDE SLOPE GRADING. THE PETITIONER SHALL ENSURE THAT BEST MANAGEMENT PRACTICES ARE USED WHEN CONSTRUCTING THE ROAD CROSSING AND THAT STABILIZATION IS INITIATED IMMEDIATELY UPON COMPLETION OF THE ROAD CONSTRUCTION ACTIVITIES.
 - THE PETITIONER SHALL COMPLY WITH THE DEVELOPMENT ENGINEERING DIVISION COMMENTS DATED AUGUST 13, 2012.
- LOTS 12 thru 17 OF THIS SUBDIVISION ARE CONNECTED TO THE SHARED SEWERAGE DISPOSAL FACILITY COVERED BY SECTIONS 18.1200 ET SEQ. OF THE HOWARD COUNTY CODE. THE DEVELOPER IS OBLIGATED TO CONSTRUCT THE FACILITY UNDER THE PROVISIONS OF THE DEVELOPER AGREEMENT TO BE EXECUTED WITH THIS PLAN (F-13-112). A BUILDING PERMIT FOR LOTS 12 thru 17 MAY NOT BE ISSUED UNTIL THE CONSTRUCTION OF THE FACILITY IS COMPLETED. ACTIVITY ON THESE LOTS IS RESTRICTED AND IS SUBJECT TO THE DEGRADATION CONDITIONS, RIGHTS-OF-ENTRY AND RESTRICTIONS FOR SHARED SEWERAGE DISPOSAL FACILITY INTENDED TO BE RECORDED AMONG THE LAND RECORDS OF HOWARD COUNTY, MARYLAND. LOTS 12 thru 17 SHALL BE ASSESSED SHARED SEWERAGE DISPOSAL FACILITIES CHARGES AND ASSESSMENTS PURSUANT TO SECTIONS 20.800 ET SEQ. OF THE HOWARD COUNTY CODE. THE SHARED SEWERAGE DISPOSAL FACILITY IS TO OWNED AND MAINTAINED BY HOWARD COUNTY.
- NO GRADING, REMOVAL OF VEGETATIVE COVER OR TREES, PAVING AND NEW STRUCTURES SHALL BE PERMITTED WITHIN THE LIMITS OF WETLANDS, STREAMS, THEIR REQUIRED BUFFERS OR 100YR FLOODPLAIN EXCEPT THAT ASSOCIATED WITH THE ROAD AND DRIVEWAY CROSSINGS PER WP-13-025.
- THIS DEVELOPMENT IS SUBJECT TO MDE PERMIT NO. 12-NF-0278/201261105
- A PRE-SUBMISSION COMMUNITY MEETING FOR THIS PROJECT WAS HELD ON FEBRUARY 29, 2012.
- THIS PROJECT IS EXEMPT FROM SB-236 GROWTH TIER LEGISLATION. PERCOLATION CERTIFICATION PLAN WAS APPROVED PRIOR TO JULY 1, 2012.
- THE USE-IN-COMMON DRIVEWAY MAINTENANCE AGREEMENT FOR LOTS 3-5, 7-10, 17-23 AND PRESERVATION PARCEL "A" WERE RECORDED SIMULTANEOUSLY WITH THIS PLAN.

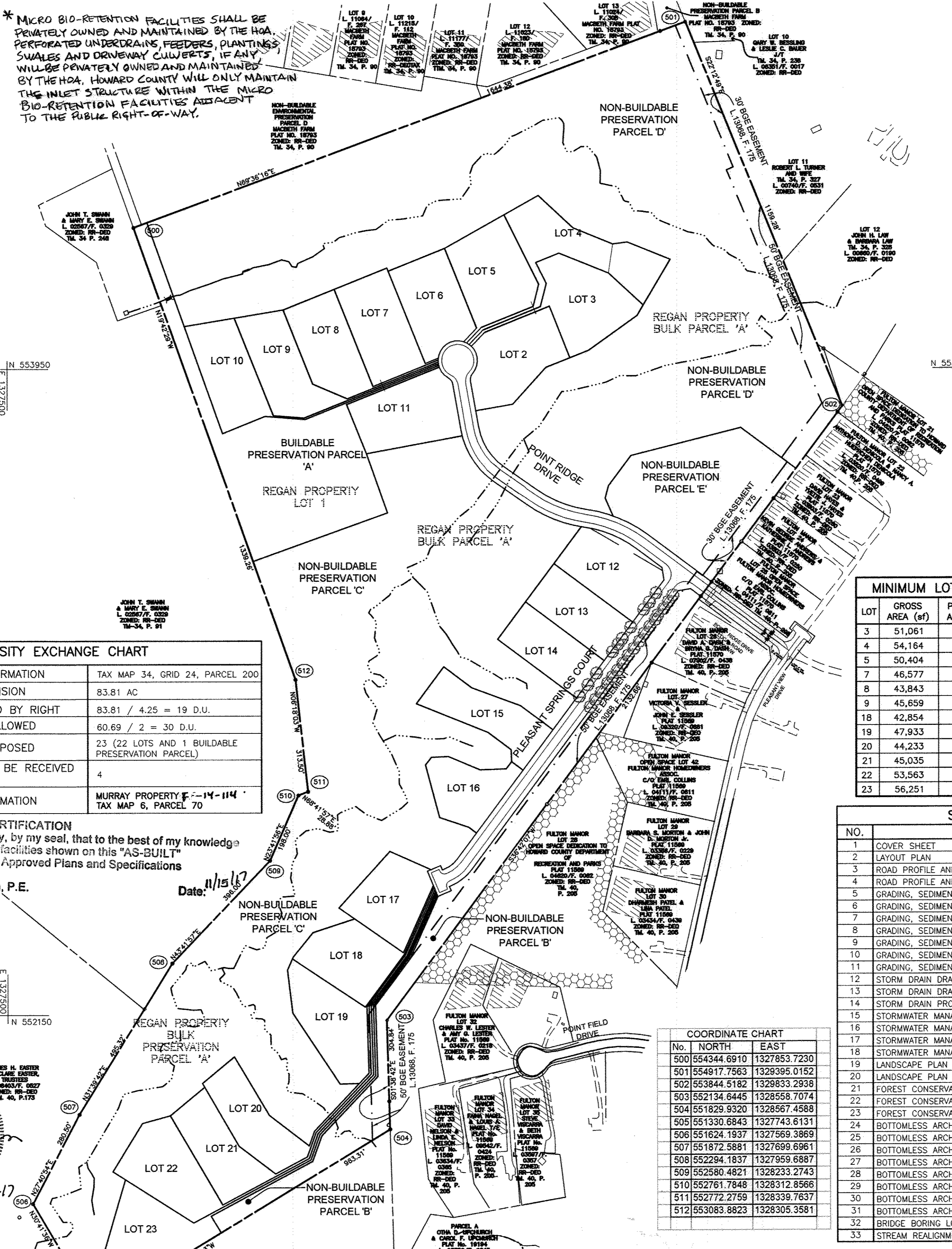
FINAL ROAD CONSTRUCTION PLANS

REGAN PROPERTY

LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL 'A' and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

5th ELECTION DISTRICT

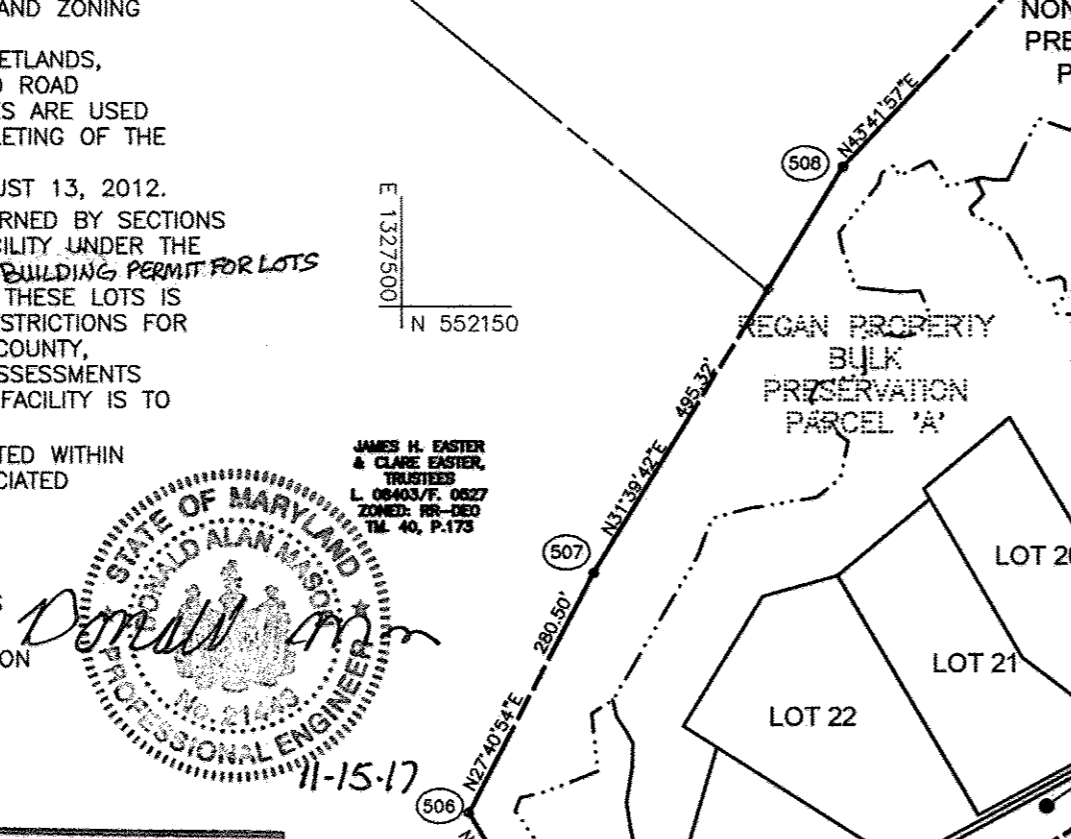
HOWARD COUNTY, MARYLAND



DENSITY EXCHANGE CHART	
RECEIVING PARCEL INFORMATION	TAX MAP 34, GRID 24, PARCEL 200
TOTAL AREA OF SUBDIVISION	83.81 AC
DENSITY UNITS ALLOWED BY RIGHT	83.81 / 4.25 = 19 D.U.
MAXIMUM DEO UNITS ALLOWED	60.69 / 2 = 30 D.U.
NUMBER OF UNITS PROPOSED	23 (22 LOTS AND 1 BUILDABLE PRESERVATION PARCEL)
DEO DENSITY UNITS TO BE RECEIVED FROM SENDING PARCEL	4
SENDING PARCEL INFORMATION	MURRAY PROPERTY F-11-114 TAX MAP 6, PARCEL 70

AS-BUILT CERTIFICATION
I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications

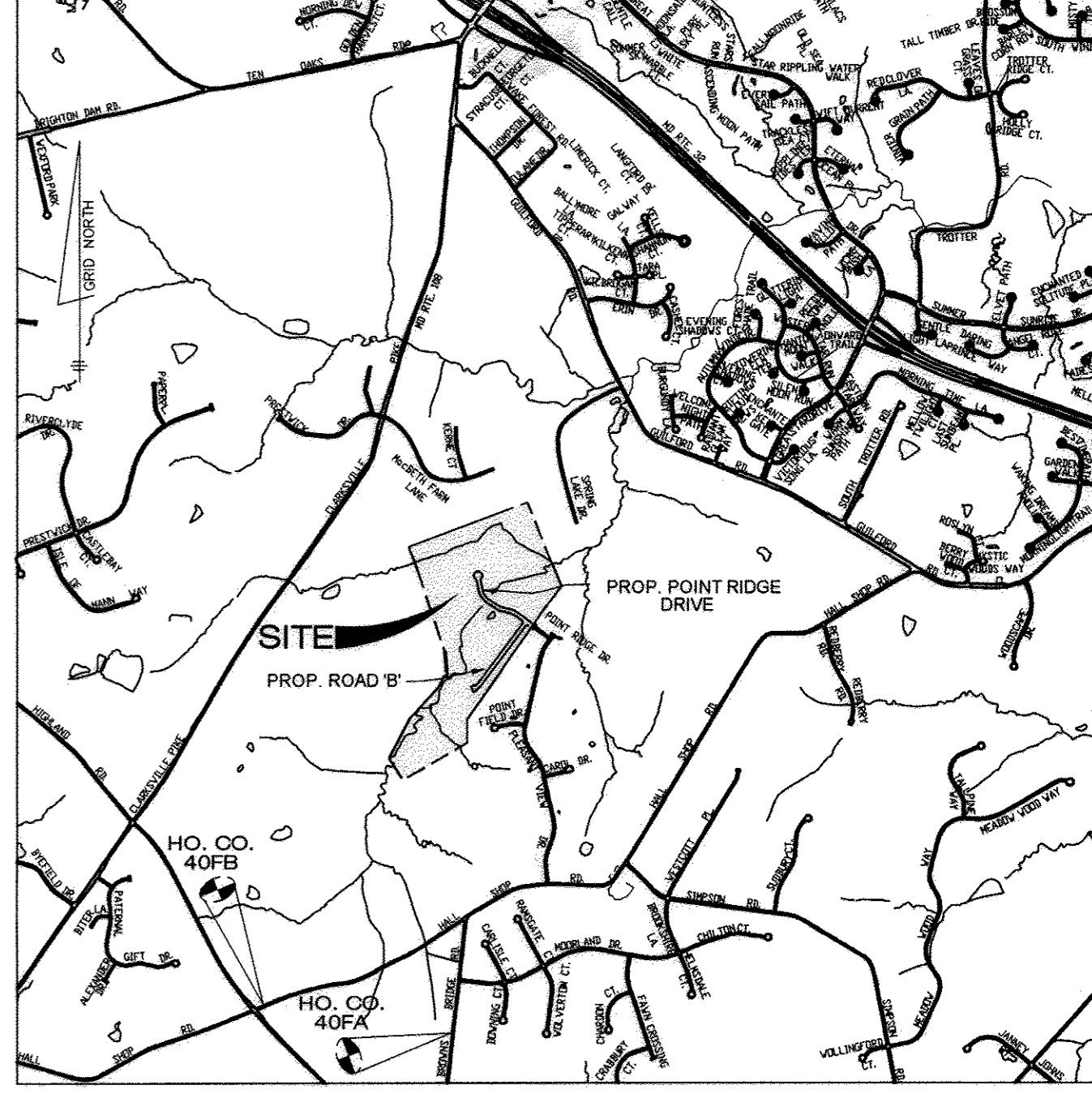
Donald Mason, P.E. Date: 11/15/14



BENCH MARKS	
HO. CO. #40FA (NAD '83) CONCRETE MONUMENT 0.25' BELOW SURFACE ON BROWN BRIDGE ROAD 0.3 MILE NORTH OF ROUTE 216	ELEV. 497.10 E 1,328,421.39' N 548,106.926'
HO. CO. #40FB (NAD '83) MONUMENT BEING 10.5' OFF EDGE OF PAVEMENT 2.0' BELOW SURFACE AT THE INTERSECTION OF HALL SHOP ROAD AND ROUTE 216	ELEV. 504.438' E 1,328,000.81' N 548,470.381'

STORMWATER MANAGEMENT PRACTICES		
LOT NUMBER	DISCONNECT OF NON-ROOFTOP RUNOFF (NUMBER)	MICRO-BIORETENTION (NUMBER)
LOT 2	1	1
LOT 3	1	1
LOT 4	1	1
LOT 5	1	1
LOT 6	1	1
LOT 7	1	1
LOT 8	1	1
LOT 9	1	1
LOT 10	1	1
LOT 11	1	1
LOT 12	1	1
LOT 13	1	1
LOT 14	1	1
LOT 15	1	1
LOT 16	1	1
LOT 17	1	1
LOT 18	1	1
LOT 19	1	1
LOT 20	1	1
LOT 21	1	1
LOT 22	1	1
LOT 23	1	1

MICRO-BIORETENTION FACILITIES ON LOTS 3, 5, 6, 7, 8, 10 AND 20 MUST INCLUDE AN IMPERMEABLE LINER.



VICINITY MAP
SCALE: 1" = 2000'
ADC MAP PAGE: 4933, GRID: K10 & 505, GRID: J1 & K1

- AS-BUILT NOTES:**
- VERTICAL DATUM FOR THIS AS-BUILT IS BASED ON THE MARYLAND STATE REFERENCE SYSTEM NAD 83 AS PROJECTED FROM NAD 83 GEODETIC CONTROL POINTS AND VERTICAL DATUM. THE AS-BUILT IS NOT AN ORIGINAL VERTICAL DATUM. ALSO AS REQUESTED FROM THE ABOVE MENTIONED HOWARD COUNTY GEODETIC CONTROL STATIONS.
 - THE ELEVATIONS USED IN PREPARING THIS AS-BUILT WERE A TOTAL STATION AND PRECISION LEVELING.
 - THIS AS-BUILT WAS PREPARED BY BENCHMARK ENGINEERING, INC. 75 THOMAS JOHNSON DRIVE SUITE E FREDERICK, MARYLAND 21702
 - RIGHT OF WAY BOUNDARIES SHOWN ON THIS PLAN ARE BASED ON RECORD DRAWINGS AND FIELD SURVEY.

LEGEND

SOLS CLASSIFICATION	ChB2
SOLS DELINEATION	
EXISTING CONTOURS	
PROPOSED CONTOURS	999
LIMIT OF WETLANDS	
25' WETLANDS BUFFER	
CENTERLINE OF STREAM	
STREAM BUFFER	
EXISTING WOODS LINE	
PROPOSED WOODS LINE	
EXISTING STRUCTURE	
PROPOSED STRUCTURE	
SLOPES 15% TO 24.9%	
SLOPES 25% OR GREATER	
100 YEAR FLOODPLAIN	
PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)	
PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)	
TREE PROTECTION FENCE	
FCE PERMANENT SIGNAGE	
PROP. STREET TREE	POINT RIDGE DRIVE
PROP. WELL LOCATION	ROAD 'B'
PRIVATE SEWAGE DISPOSAL AREA	
PRIVATE WELL AREA	
BOUNDARY COORDINATE	502

GRAPHIC SCALE
1 inch = 200 ft

SITE DATA TABULATION

- GENERAL SITE DATA
 - PRESENT ZONING: RR-DEO
 - LOCATION: TAX MAP 34 - GRID 24 - PARCEL 200
 - APPLICABLE DPZ FILE REFERENCES: ECP-12-045, SP-12-004, WP-13-025
 - DEED REFERENCE: L11915 / F.00086
 - PROPOSED USE OF SITE: 22 SFD HOMES; 1 BUILDABLE PRESERVATION PARCEL; 4 NON-BUILDABLE PRESERVATION PARCEL
 - PROPOSED WATER AND SEWER SYSTEMS: PRIVATE; LOTS 1-10, 17-22 AND PRESERVATION PARCEL "A" PRIVATE WATER PUBLIC SEWER; LOTS 11-16
- AREA TABULATION
 - TOTAL AREA OF SITE 83.81 Ac.±
 - AREA OF 100 YEAR FLOODPLAIN (EXISTING) 21.97 Ac.±
 - AREA OF STEEP SLOPES (25% OR GREATER) 1.30 Ac.± (1.15 Ac.± OUTSIDE FLOODPLAIN)
 - NET AREA OF SITE 60.69 Ac.±
 - AREA OF THIS PLAN SUBMISSION 83.81 Ac.±
 - LIMIT OF DISTURBANCE 8.02 Ac.±
 - AREA OF PROPOSED BUILDABLE LOTS 26.11 Ac.±
 - AREA OF PRESERVATION PARCEL 55.07 Ac.±
 - AREA OF PROPOSED PUBLIC ROAD 2.63 Ac.±
 - AREA OF PROPOSED PUBLIC R/W DEDICATION N/A
- DENSITY TABULATION
 - NET AREA OF SITE 60.69 Ac.±
 - TOTAL NUMBER OF LOTS ALLOWED PER ZONING
1 UNIT PER 4.25 GROSS ACRES 19
1 UNIT PER 2 NET ACRES (MAX) 30
- UNIT/LOT TABULATION
 - TOTAL NUMBER OF BUILDABLE LOTS PROPOSED ON THIS SUBMISSION 22
 - TOTAL NUMBER OF PRESERVATION PARCELS PROPOSED ON THIS SUBMISSION 4 (1 BUILDABLE)

MINIMUM LOT SIZE CHART			
LOT	GROSS AREA (sqft)	PIPESTEM AREA (sqft)	MINIMUM LOT SIZE (sqft)
3	51,061	1,813	49,248
4	54,164	4,324	49,840
5	50,404	446	49,958
7	46,577	86	46,491
8	43,843	935	42,908
9	45,659	1,309	44,350
18	42,854	813	42,041
19	47,933	1,538	46,395
20	44,233	4,229	40,004
21	45,035	5,010	40,025
22	53,563	6,064	47,499
23	56,251	5,346	49,905

SHEET INDEX	
NO.	DESCRIPTION
1	COVER SHEET
2	LAYOUT PLAN
3	ROAD PROFILE AND DETAILS (POINT RIDGE DRIVE)
4	ROAD PROFILE AND DETAILS (PLEASANT SPRINGS COURT)
5	GRADING, SEDIMENT AND EROSION CONTROL PLAN AND SOILS MAP
6	GRADING, SEDIMENT AND EROSION CONTROL PLAN AND SOILS MAP
7	GRADING, SEDIMENT AND EROSION CONTROL PLAN AND SOILS MAP
8	GRADING, SEDIMENT AND EROSION CONTROL PLAN AND SOILS MAP
9	GRADING, SEDIMENT AND EROSION CONTROL PLAN AND SOILS MAP
10	GRADING, SEDIMENT AND EROSION CONTROL PLAN AND SOILS MAP
11	GRADING, SEDIMENT AND EROSION CONTROL DETAIL AND NOTES
12	STORM DRAIN DRAINAGE AREA MAP
13	STORM DRAIN DRAINAGE AREA MAP
14	STORM DRAIN PROFILES
15	STORMWATER MANAGEMENT PLAN
16	STORMWATER MANAGEMENT PLAN
17	STORMWATER MANAGEMENT PLAN
18	STORMWATER MANAGEMENT PLAN
19	LANDSCAPE PLAN
20	LANDSCAPE PLAN
21	FOREST CONSERVATION PLAN
22	FOREST CONSERVATION PLAN
23	FOREST CONSERVATION NOTES AND DETAILS
24	BOTTOMLESS ARCH CROSSING #1
25	BOTTOMLESS ARCH CROSSING #1
26	BOTTOMLESS ARCH CROSSING #1
27	BOTTOMLESS ARCH CROSSING #1
28	BOTTOMLESS ARCH CROSSING #2
29	BOTTOMLESS ARCH CROSSING #2
30	BOTTOMLESS ARCH CROSSING #2
31	BOTTOMLESS ARCH CROSSING #2
32	BRIDGE BORING LOGS
33	STREAM REALIGNMENT PLAN

APPROVED: DEPARTMENT OF PUBLIC WORKS
 [Signature] 4-29-14
 CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 [Signature] 5-21-14
 CHIEF, DIVISION OF LAND DEVELOPMENT

[Signature] 5-9-14
 CHIEF, DEVELOPMENT ENGINEERING DIVISION

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21A13 Expiration Date: 12-21-18

PLAN VIEW
SCALE: 1" = 200'

BENCHMARK ENGINEERING, INC.
 ENGINEERS & LAND SURVEYORS & PLANNERS
 8480 BALTIMORE AVENUE SUITE 315 A ELLOTT CITY, MARYLAND 21043
 (410) 465-6105 (F) 410-465-6844
 75 THOMAS JOHNSON DRIVE SUITE E FREDERICK, MARYLAND 21702
 301-710-5888
 WWW.BE-ENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 28559, Expiration Date: 7-23-2015.

OWNER/DEVELOPER:
 MB HIGHLAND RESERVE, LLC
 1686 EAST GUIDE DRIVE
 ROCKVILLE, MD 20850
 301-762-9511

PROJECT: REGAN PROPERTY
 LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL 'A' and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

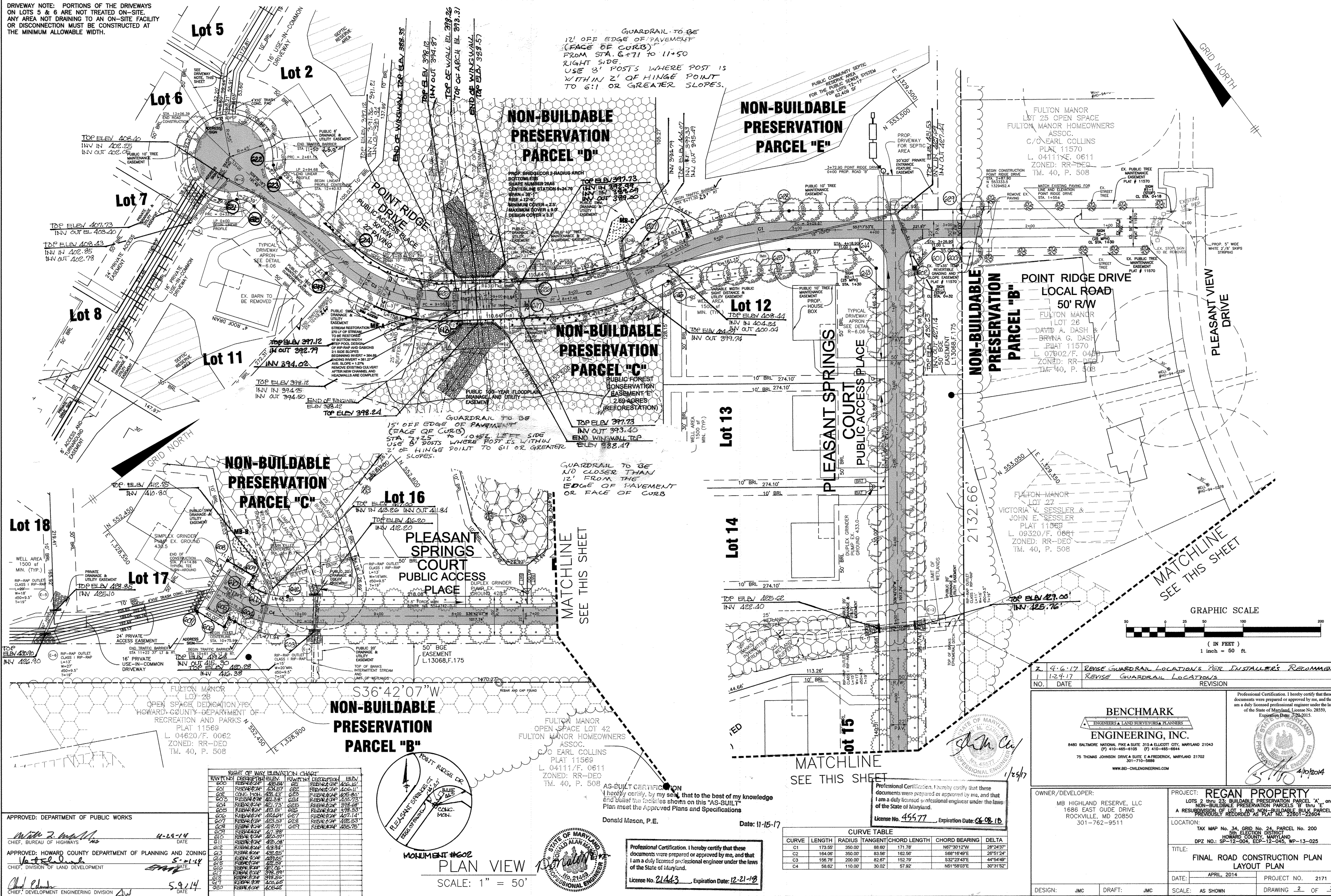
LOCATION:
 TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
 FINAL ROAD CONSTRUCTION PLAN
 COVER SHEET

DATE: APRIL, 2014 **PROJECT NO.:** 2171

DESIGN: JMC **DRAFT:** JMC **SCALE:** AS SHOWN **DRAWING:** 1 OF 33

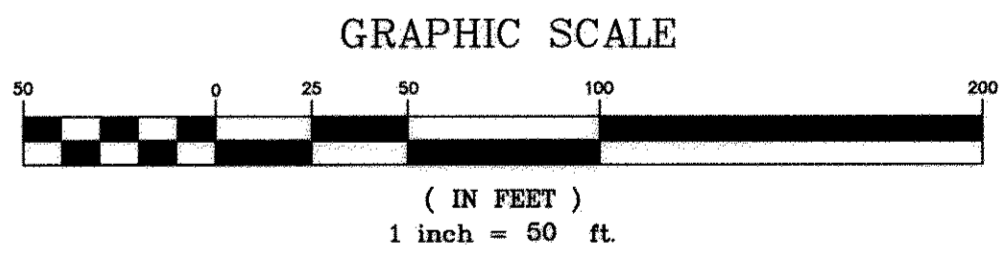
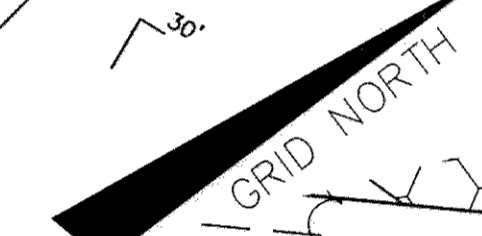
DRIVEWAY NOTE: PORTIONS OF THE DRIVEWAYS ON LOTS 5 & 6 ARE NOT TREATED ON-SITE. ANY AREA NOT DRAINING TO AN ON-SITE FACILITY OR DISCONNECTION MUST BE CONSTRUCTED AT THE MINIMUM ALLOWABLE WIDTH.



GUARDRAIL TO BE 12' OFF EDGE OF PAVEMENT (FACE OF CURB) FROM STA. 6+71 TO 11+50 RIGHT SIDE. USE 8' POSTS WHERE POST IS WITHIN 2' OF HINGE POINT TO 6:1 OR GREATER SLOPES.

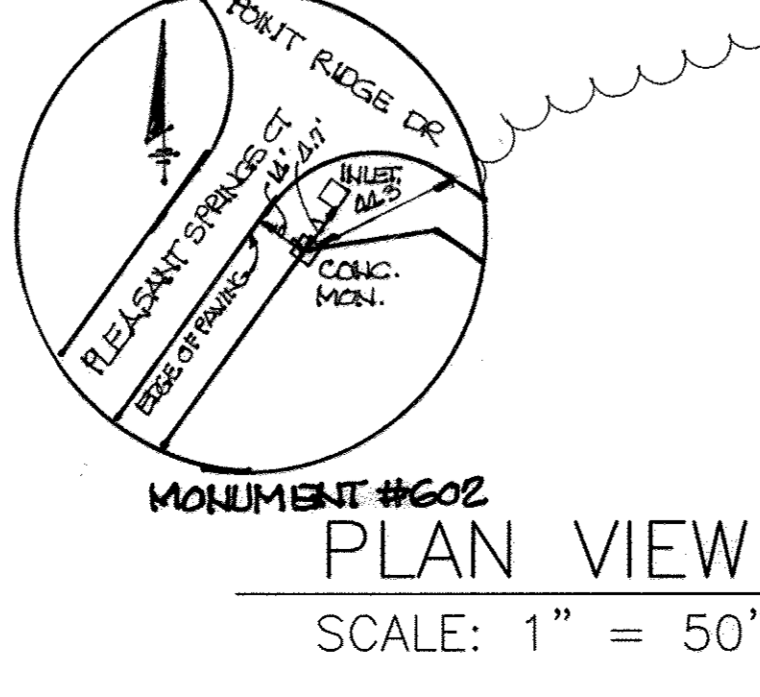
GUARDRAIL TO BE 15' OFF EDGE OF PAVEMENT (FACE OF CURB) STA. 7+25 TO 10+52 LEFT SIDE USE 8' POSTS WHERE POST IS WITHIN 2' OF HINGE POINT TO 6:1 OR GREATER SLOPES.

GUARDRAIL TO BE NO CLOSER THAN 12' FROM THE EDGE OF PAVEMENT OR FACE OF CURB



RIGHT OF WAY ELEVATION CHART

R/W (IN)	DESCRIPTION	ELEV.	R/W (IN)	DESCRIPTION	ELEV.
600	REBAR 24"	426.24	602	REBAR 24"	426.12
601	REBAR 24"	426.27	603	REBAR 24"	426.11
602	REBAR 24"	426.30	604	REBAR 24"	426.10
603	REBAR 24"	426.33	605	REBAR 24"	426.09
604	REBAR 24"	426.36	606	REBAR 24"	426.08
605	REBAR 24"	426.39	607	REBAR 24"	426.07
606	REBAR 24"	426.42	608	REBAR 24"	426.06
607	REBAR 24"	426.45	609	REBAR 24"	426.05
608	REBAR 24"	426.48	610	REBAR 24"	426.04
609	REBAR 24"	426.51	611	REBAR 24"	426.03
610	REBAR 24"	426.54	612	REBAR 24"	426.02
611	REBAR 24"	426.57	613	REBAR 24"	426.01
612	REBAR 24"	426.60	614	REBAR 24"	426.00
613	REBAR 24"	426.63	615	REBAR 24"	425.99
614	REBAR 24"	426.66	616	REBAR 24"	425.98
615	REBAR 24"	426.69	617	REBAR 24"	425.97
616	REBAR 24"	426.72	618	REBAR 24"	425.96
617	REBAR 24"	426.75	619	REBAR 24"	425.95
618	REBAR 24"	426.78	620	REBAR 24"	425.94
619	REBAR 24"	426.81	621	REBAR 24"	425.93
620	REBAR 24"	426.84	622	REBAR 24"	425.92



APPROVED: DEPARTMENT OF PUBLIC WORKS
 Chief, Bureau of Highways
 Date: 4-25-14

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 Chief, Division of Land Development
 Date: 5-21-14

Chief, Development Engineering Division
 Date: 5-9-14

AS-BUILT CERTIFICATION
 I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications
 Donald Mason, P.E. Date: 11-15-17



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443, Expiration Date: 12-21-18

CURVE TABLE

CURVE	LENGTH	RADIUS	TANGENT	CHORD LENGTH	CHORD BEARING	DELTA
C1	173.55'	350.00'	88.89'	171.78'	N67°30'12"W	28°29'37"
C2	164.05'	350.00'	83.88'	162.67'	S88°16'42"E	28°51'24"
C3	156.78'	200.00'	82.87'	152.79'	S32°23'43"E	44°54'48"
C4	58.62'	110.00'	30.02'	57.92'	N61°58'03"E	30°31'52"

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 45577, Expiration Date: 06-08-18



NO.	DATE	REVISION
2	4-6-17	REVISE GUARDRAIL LOCATIONS PER INSTALLER'S RECOMMENDATION
1	1-24-17	REVISE GUARDRAIL LOCATIONS

BENCHMARK ENGINEERING, INC.
 ENGINEERS & LAND SURVEYORS & PLANNERS
 6480 BALTIMORE NATIONAL PIKE & SUITE 315 & ELLICOTT CITY, MARYLAND 21043
 (P) 410-485-6100 (F) 410-485-6844
 75 THOMAS JOHNSON DRIVE SUITE E & FREDERICK, MARYLAND 21702
 301-710-5888
 WWW.BE-CVLENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 28559, Expiration Date: 7-29-2015.

OWNER/DEVELOPER: MB HIGHLAND RESERVE, LLC
 1688 EAST GUDE DRIVE ROCKVILLE, MD 20850
 301-762-9511

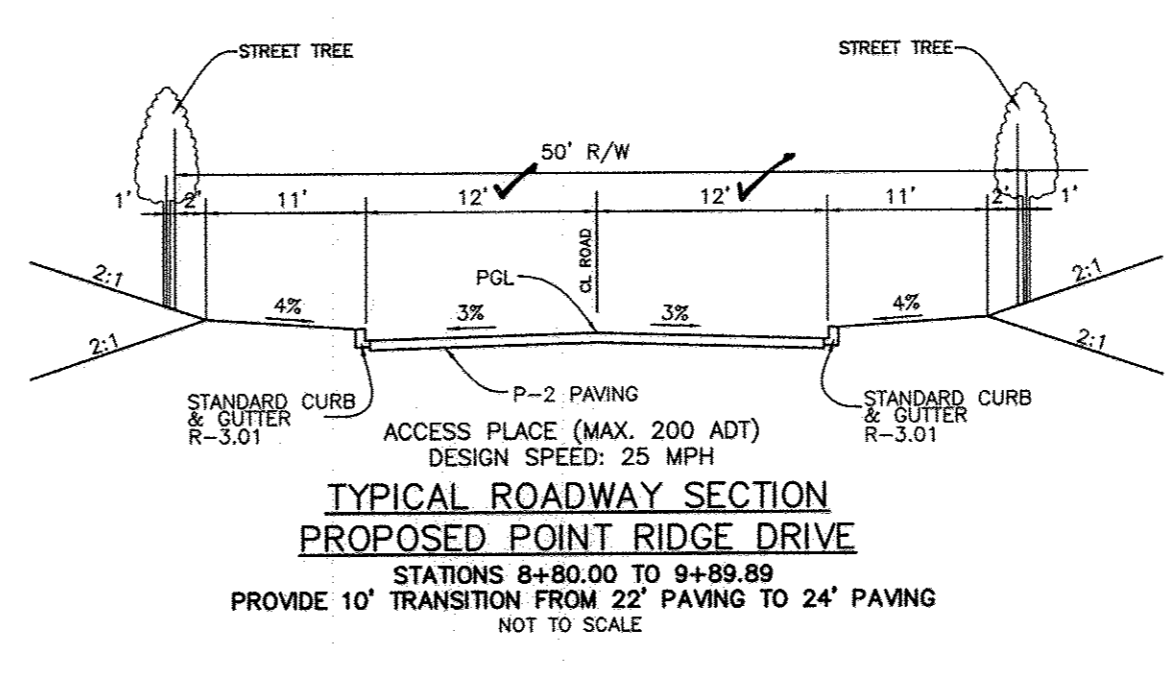
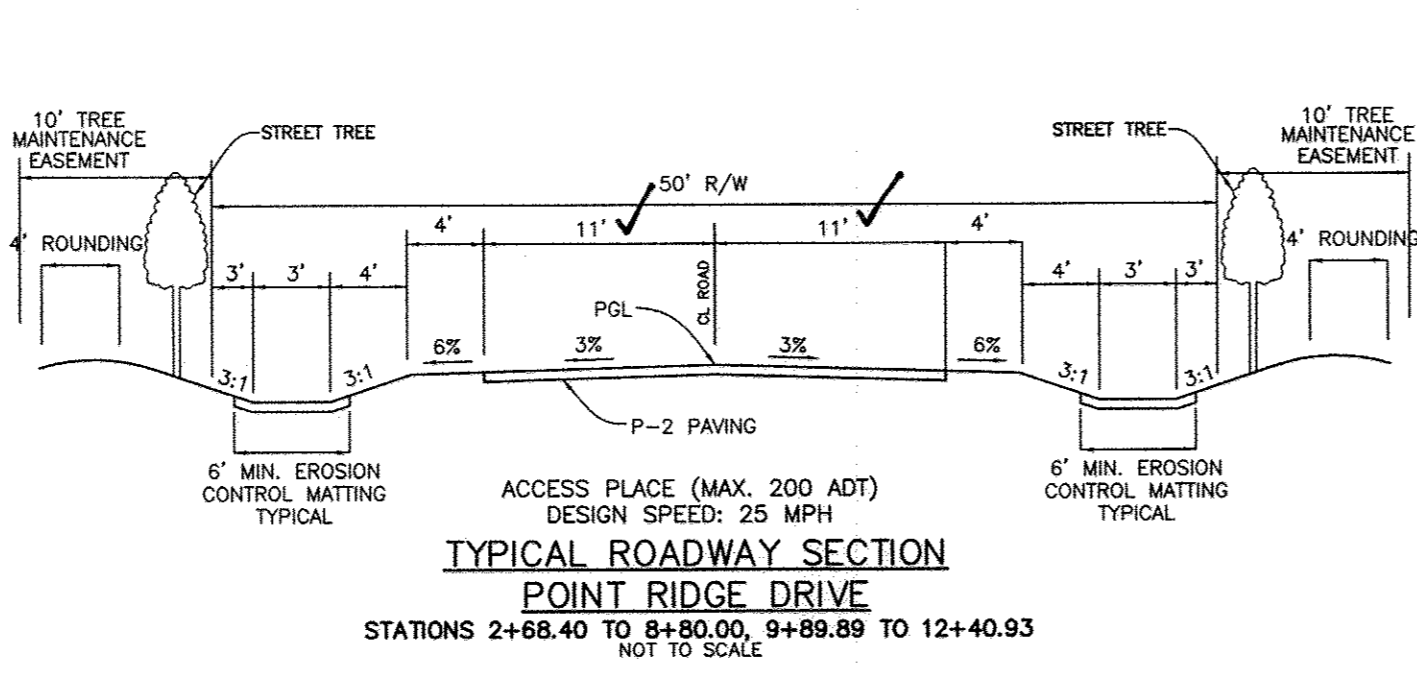
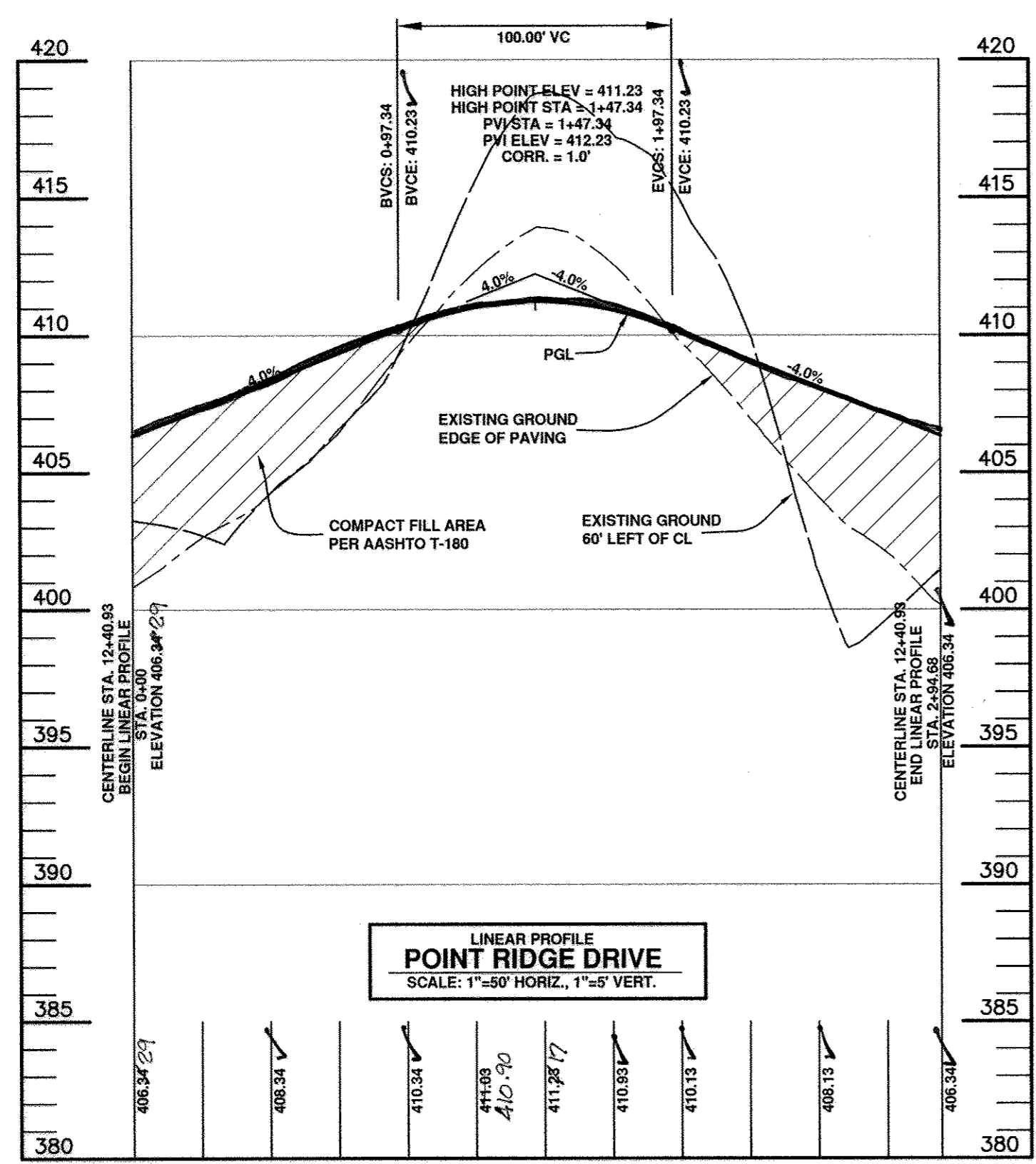
PROJECT: REGAN PROPERTY
 LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL "A", and NON-BUILDABLE PRESERVATION PARCELS "B" thru "E" A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE PARCEL "A" PREVIOUSLY RECORDED AS PLAT NO. 22807-22808

LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE: FINAL ROAD CONSTRUCTION PLAN LAYOUT PLAN

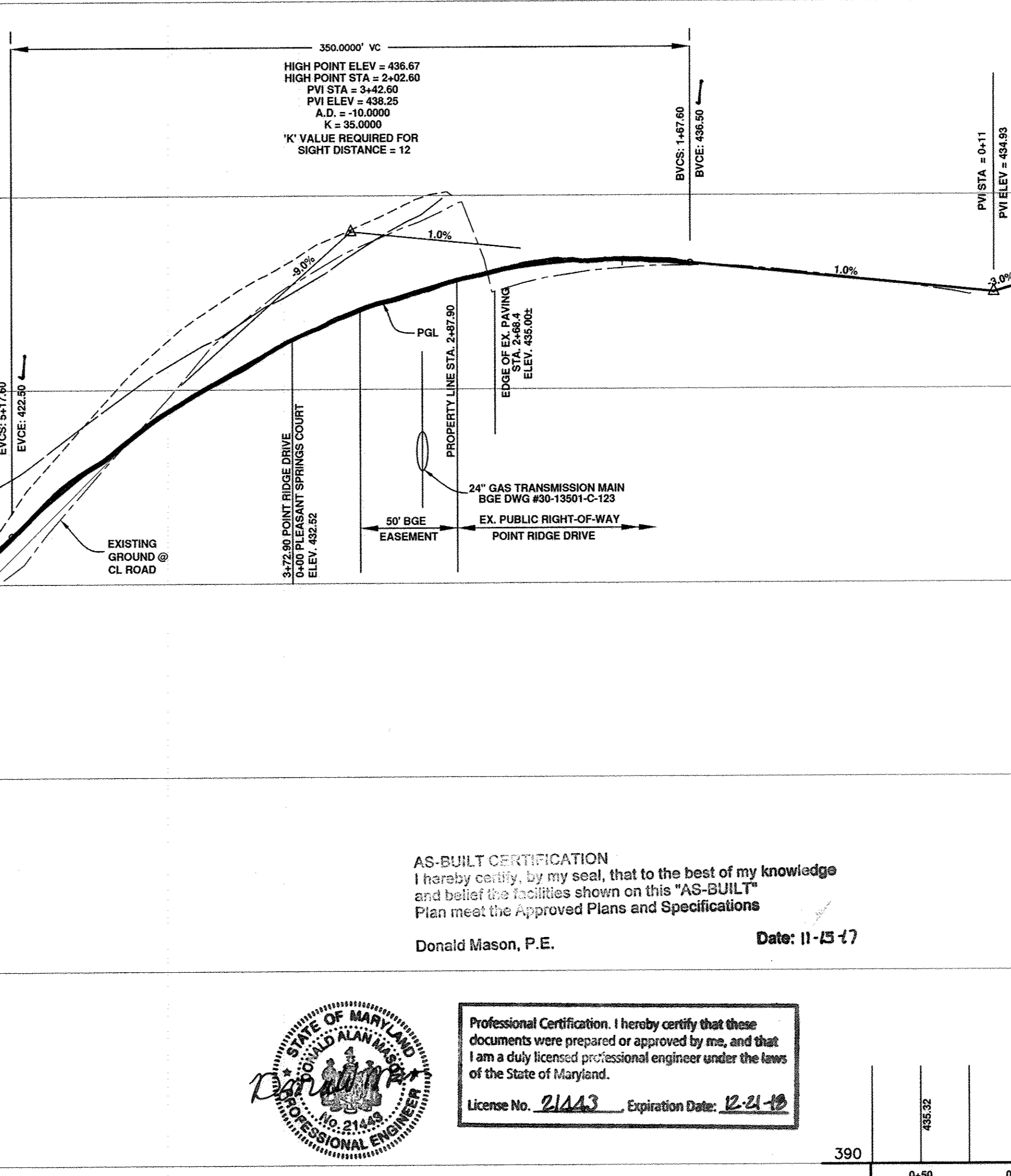
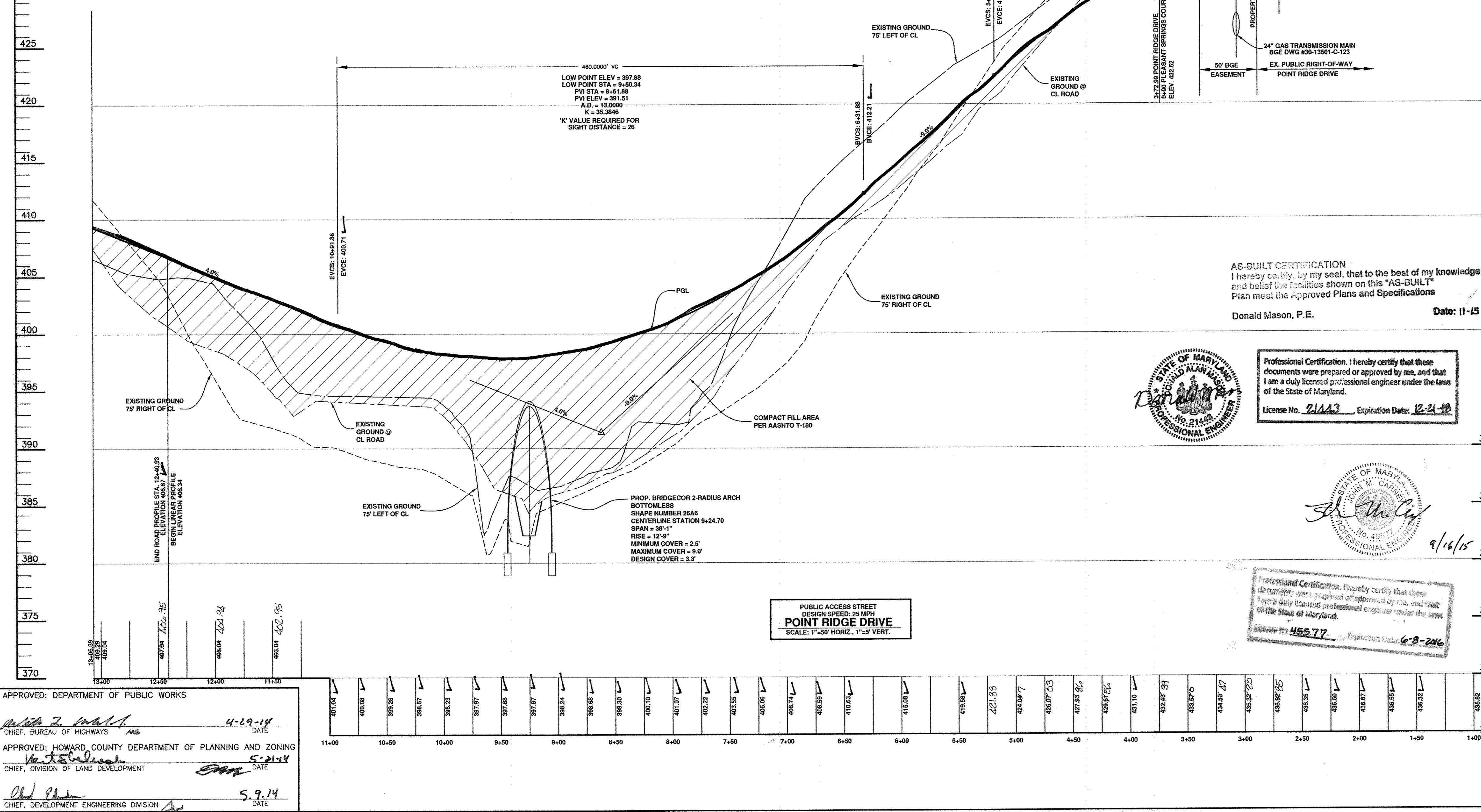
DATE: APRIL, 2014 PROJECT NO. 2171
 SCALE: AS SHOWN DRAWING 2 OF 33

DESIGN: JMC DRAFT: JMC



SECTION NUMBER	ROAD AND STREET CLASSIFICATION	CALIFORNIA BEARING RATIO (CBR)						
		PAVEMENT MATERIAL (INCHES)	3 TO <5	5 TO <7	7 TO <10	10 TO <15	15 TO <20	20 TO <30
P-2	PARKING DRIVE AISLES: RESIDENTIAL AND NON-RESIDENTIAL WITH NO MORE THAN 10 HEAVY TRUCKS PER DAY LOCAL ROADS: ACCESS PLACE, ACCESS STREET CUL-DE-SAC, RESIDENTIAL	HMA SUPERPAVE FINAL SURFACE	1.5	1.5	1.5	1.5	1.5	1.5
		HMA SUPERPAVE INTERMEDIATE SURFACE	1.0	1.0	1.0	1.0	1.0	1.0
		HMA SUPERPAVE BASE	2.0	2.0	2.0	3.5	2.0	2.0
		GRADED AGGREGATE BASE (GAB)	8.0	4.0	3.0	4.0	4.0	4.0

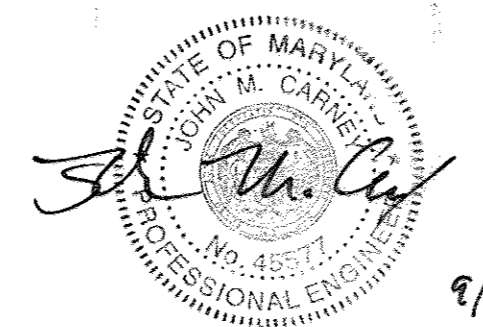
P-2 PAVING DETAIL



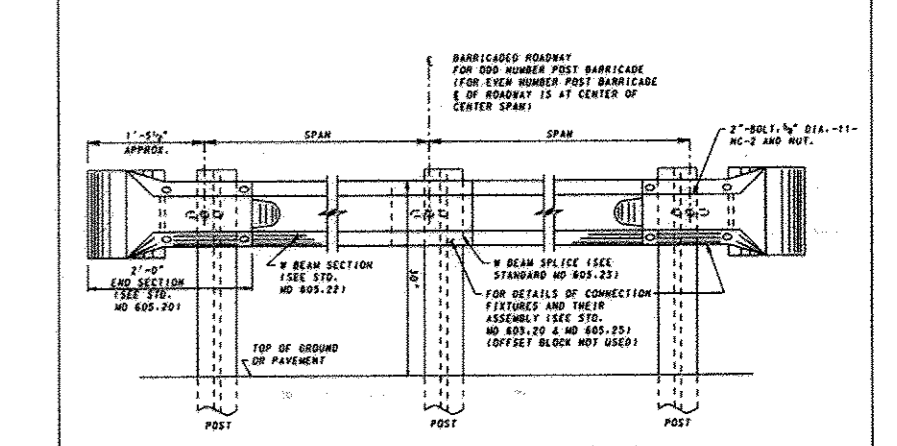
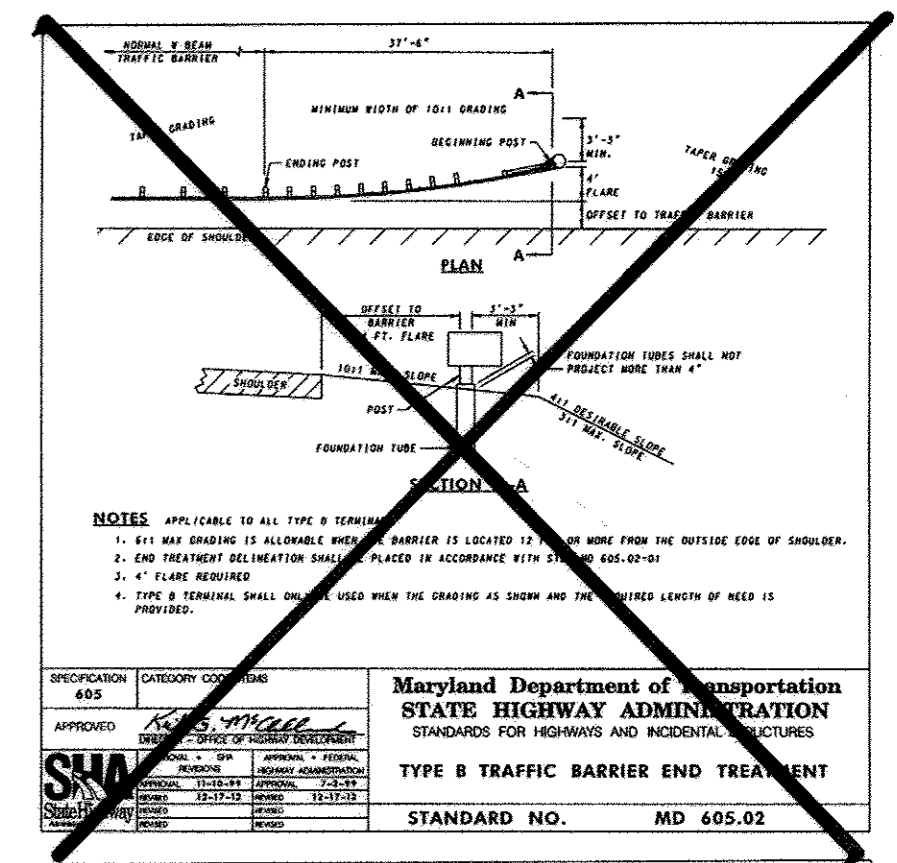
AS-BUILT CERTIFICATION
I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications.
Donald Mason, P.E. Date: 11-13-17



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443, Expiration Date: 12-21-18

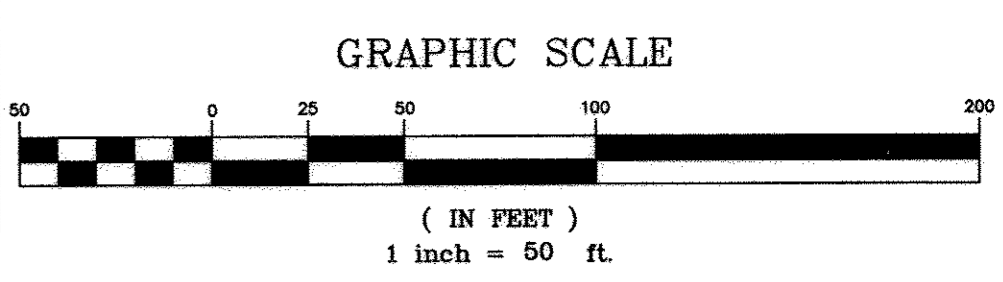


Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 45577, Expiration Date: 6-8-2016



USE OF END TREATMENT AS SHOWN ON STATE HIGHWAY ADMINISTRATION STANDARD DETAIL MD 605.20 HAS BEEN APPROVED BY BUREAU OF HIGHWAYS. EMAIL DATED 6/9/15.

USE OF END TREATMENT AS SHOWN ON STATE HIGHWAY ADMINISTRATION STANDARD DETAIL MD 605.27 HAS BEEN APPROVED BY BUREAU OF HIGHWAYS. EMAIL DATED 6/9/15.



NO.	DATE	REVISION
1	9-16-15	ADD NOTE STATING STANDARD DETAIL MD 605.20 APPROVED AND REMOVE MD 605.02 DETAIL

BENCHMARK ENGINEERING, INC.
ENGINEERS & LAND SURVEYORS & PLANNERS
8480 BALTIMORE NATIONAL PIKE & SUITE 315 & ELICOTT CITY, MARYLAND 21043
(P) 410-465-6105 (F) 410-465-6444
75 THOMAS JOHNSON DRIVE & SUITE E FREDERICK, MARYLAND 21702
301-710-5686
WWW.BE-CVLENGINEERING.COM

OWNER/DEVELOPER:
RONALD R. REGAN
568 ORCHARD BEACH BLVD
PORT WASHINGTON, NY 11050

PROJECT:
REGAN PROPERTY
LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF A NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22801-22804

LOCATION:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200
HOWARD COUNTY, MARYLAND
DPZ No.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
POINT RIDGE DRIVE
FINAL ROAD CONSTRUCTION PLAN
ROAD PROFILE AND DETAILS

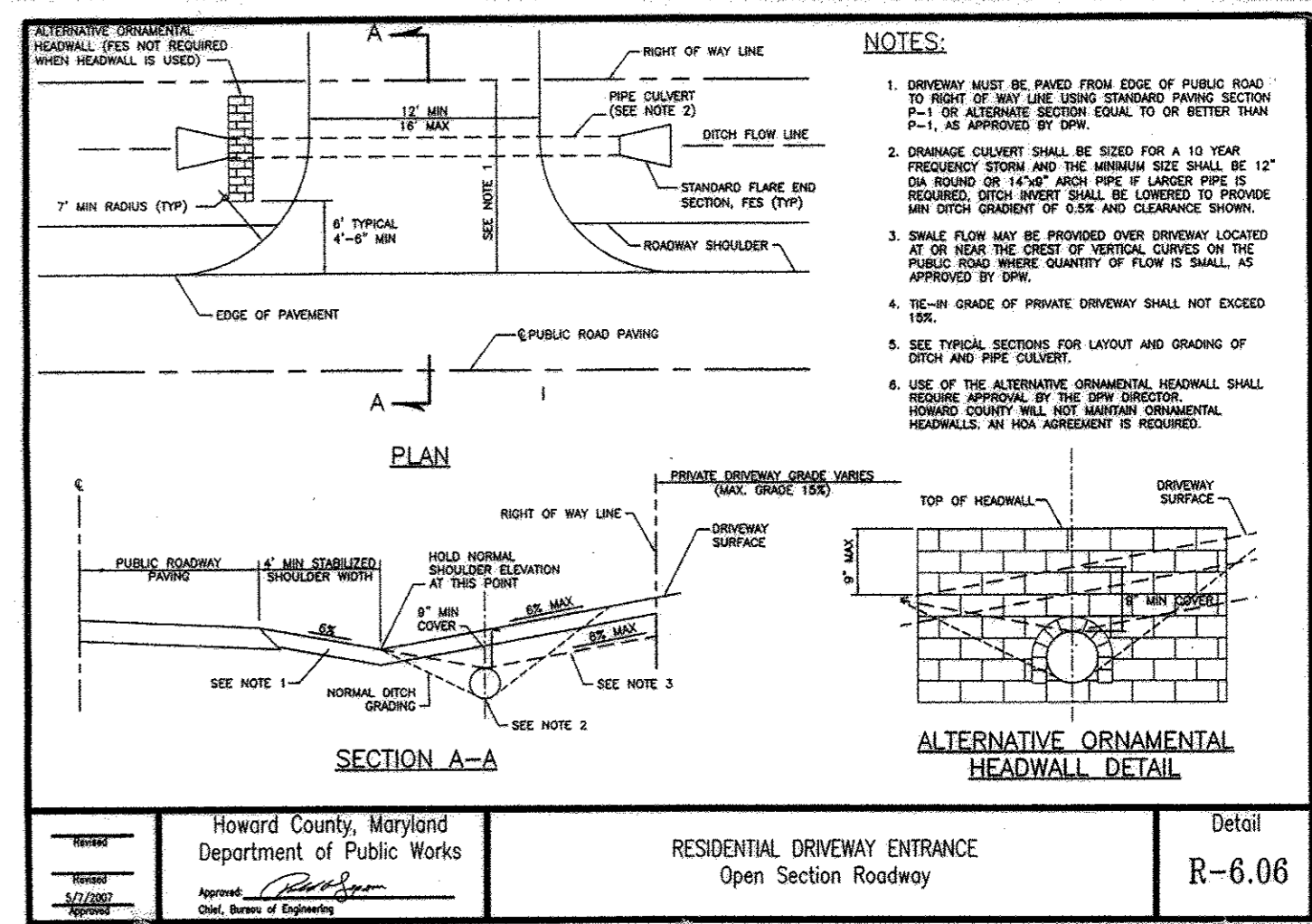
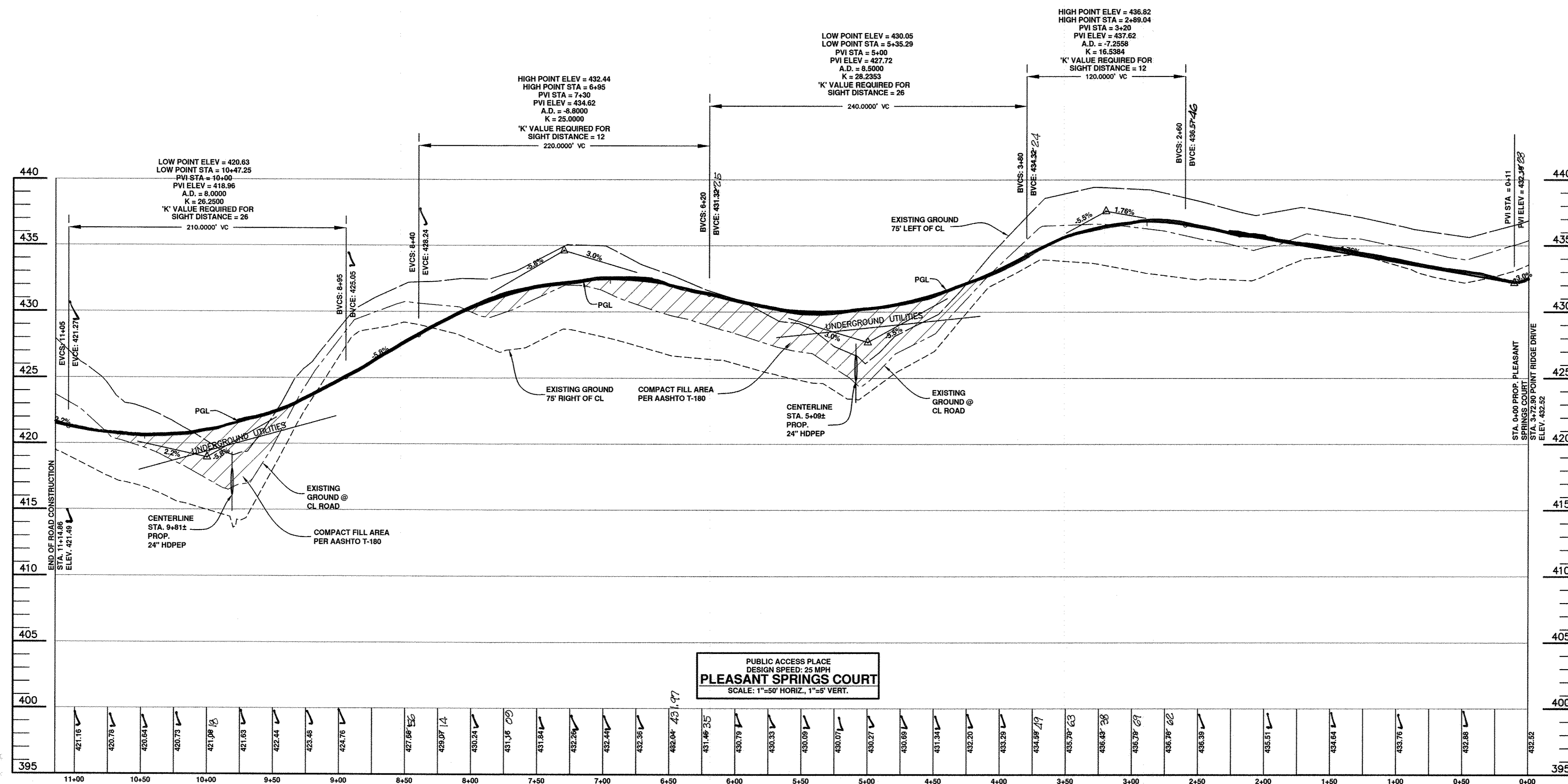
DATE: MARCH, 2014
PROJECT NO.: 2171

DESIGN: JMC
DRAFT: JMC
SCALE: AS SHOWN
DRAWING: 3 OF 33

APPROVED: DEPARTMENT OF PUBLIC WORKS
4-29-14 DATE

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
5-21-14 DATE

5-9-14 DATE



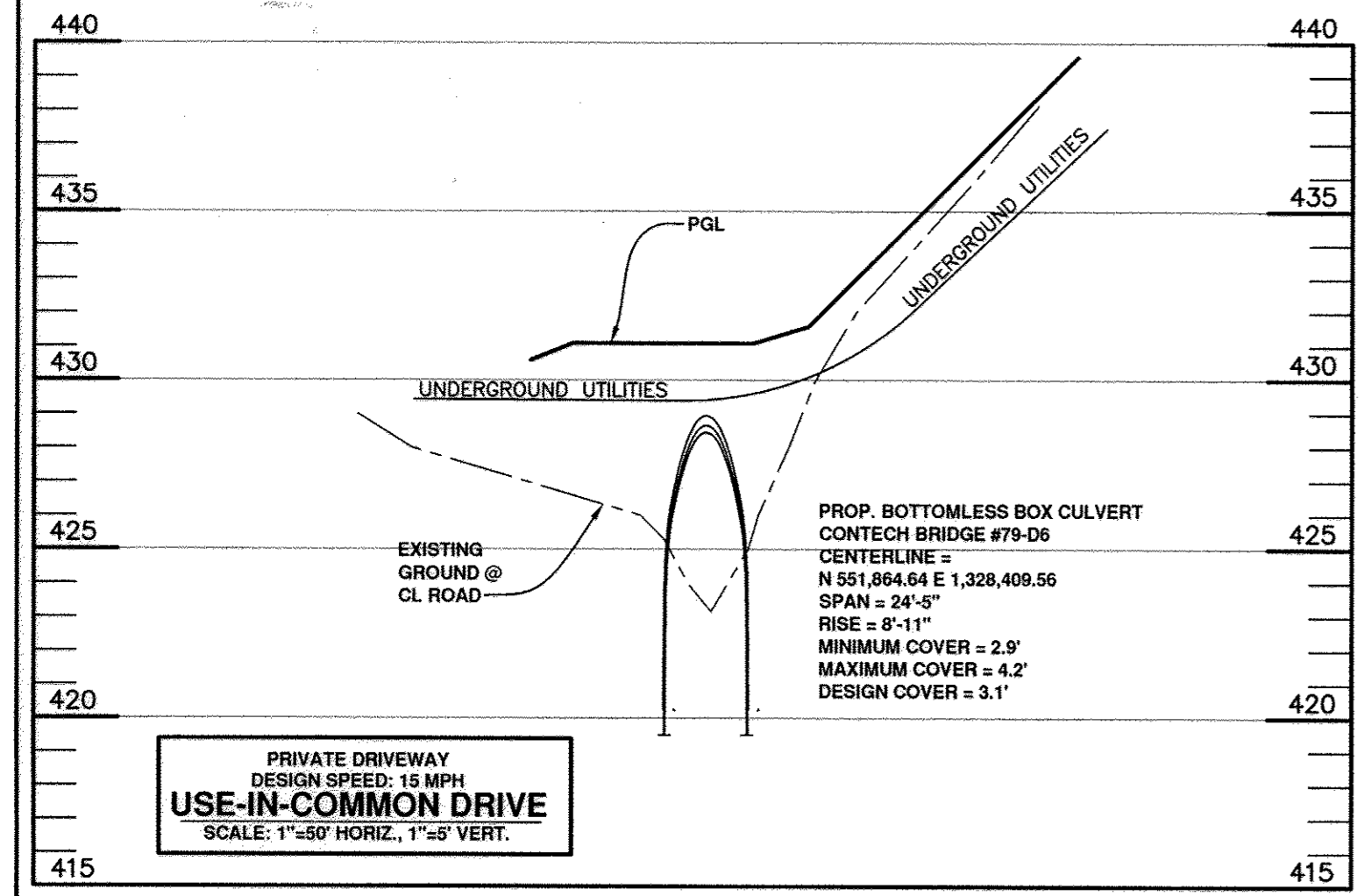
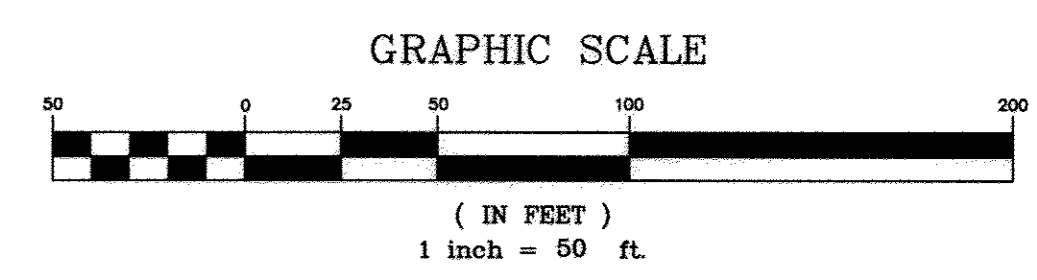
Howard County, Maryland
Department of Public Works
RESIDENTIAL DRIVEWAY ENTRANCE
Open Section Roadway
Detail
R-6.06

PUBLIC ACCESS PLACE
DESIGN SPEED: 25 MPH
PLEASANT SPRINGS COURT
SCALE: 1"=50' HORIZ., 1"=5' VERT.

AS-BUILT CERTIFICATION
I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications
Donald Mason, P.E. Date: 11-15-17



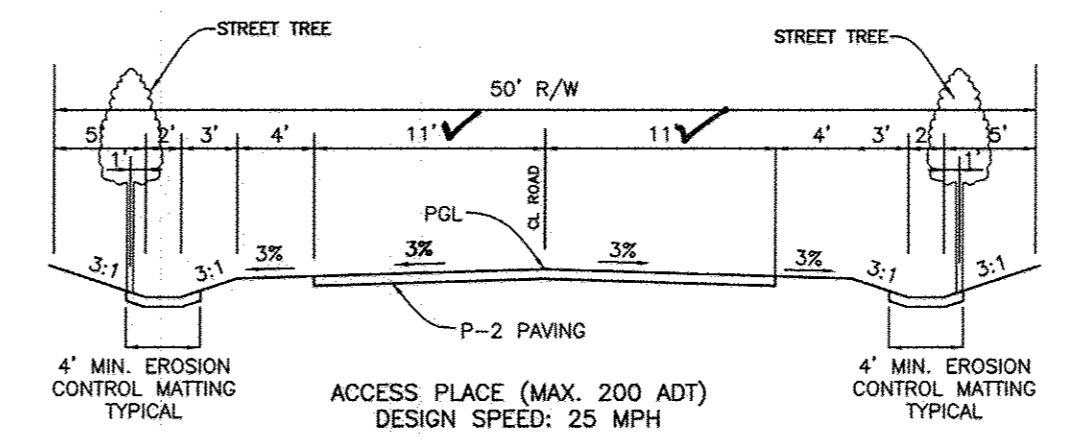
Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443, Expiration Date: 12-21-19



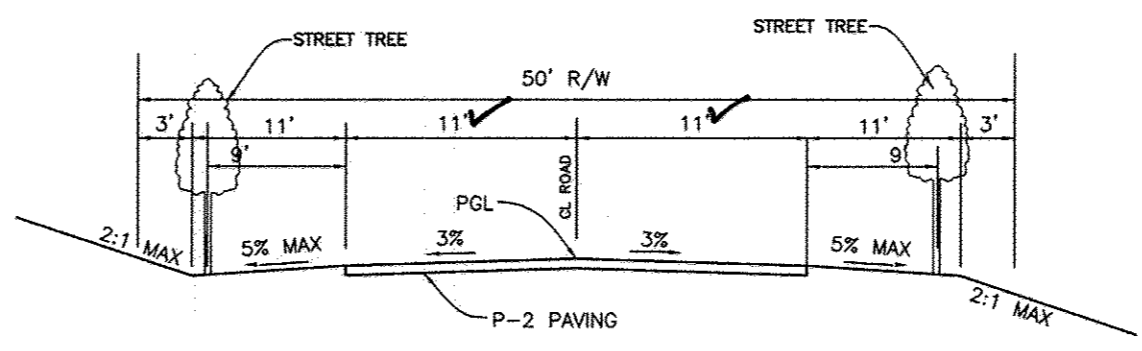
APPROVED: DEPARTMENT OF PUBLIC WORKS
4-29-14
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
5-21-14
5-9-14

SECTION NUMBER	ROAD AND STREET CLASSIFICATION	CALIFORNIA BEARING RATIO (CBR)	3 TO <5 TO <7			3 TO <5 TO <7		
			MIN HMA WITH GAB	HMA WITH CONSTANT GAB	MIN HMA WITH GAB	HMA WITH CONSTANT GAB		
P-2	PARKING DRIVE AISLES: RESIDENTIAL AND NON-RESIDENTIAL WITH NO MORE THAN 10 HEAVY TRUCKS PER DAY LOCAL ROADS: ACCESS PLACE, ACCESS STREET CUL-DE-SAC: RESIDENTIAL	PAVEMENT MATERIAL (INCHES)						
		HMA SUPERPAVE FINAL SURFACE	1.5	1.5	1.5	1.5	1.5	
		HMA SUPERPAVE INTERMEDIATE SURFACE	1.0	1.0	1.0	1.0	1.0	
		HMA SUPERPAVE BASE	2.0	2.0	2.0	2.0	2.0	
		GRADED AGGREGATE BASE (GAB)	8.0	4.0	3.0	4.0	4.0	

P-2 PAVING DETAIL



TYPICAL ROADWAY SECTION
PLEASANT SPRINGS COURT
STATIONS 8+25 TO 9+75
NOT TO SCALE



TYPICAL ROADWAY SECTION
PLEASANT SPRINGS COURT
STATIONS 0+00 TO 8+25 AND
STATIONS 9+75 TO 10+76
NOT TO SCALE

NO.	DATE	REVISION

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 28359, Expiration Date: 7-22-2015.

BENCHMARK ENGINEERING, INC.
ENGINEERS & LAND SURVEYORS & PLANNERS
8480 BALTIMORE NATIONAL PIKE & SUITE 315 A ELLICOTT CITY, MARYLAND 21043
(P) 410-485-6105 (F) 410-485-6644
75 THOMAS JOHNSON DRIVE & SUITE E FREDERICK, MARYLAND 21702
301-710-5686
WWW.BEI-CIVILENGINEERING.COM

OWNER/DEVELOPER:
RONALD R. REGAN
56B ORCHARD BEACH BLVD
PORT WASHINGTON, NY 11050

PROJECT: **REGAN PROPERTY**
LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

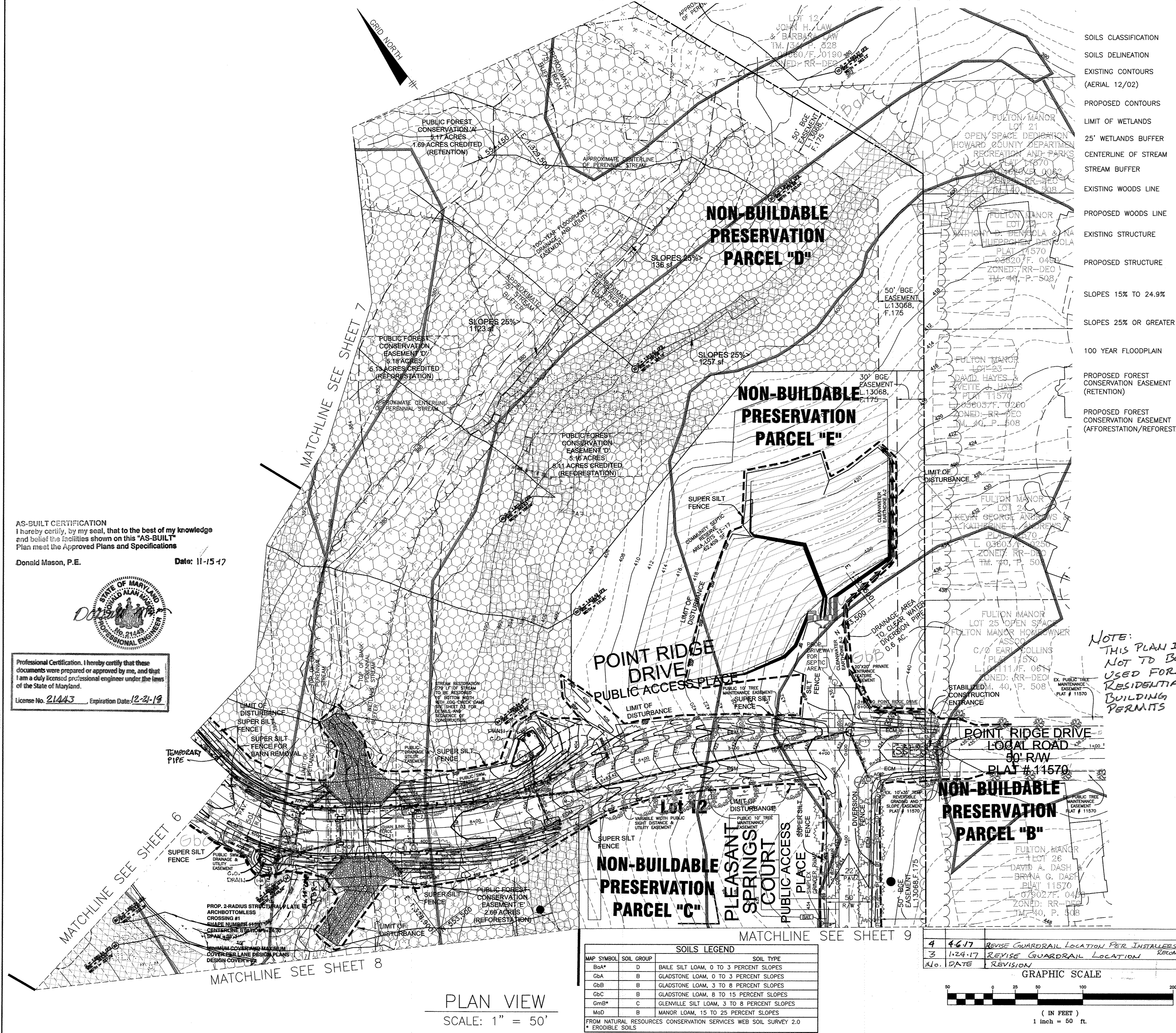
LOCATION:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200
6TH ELECTION DISTRICT
HOWARD COUNTY, MARYLAND
DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
**PLEASANT SPRINGS COURT
FINAL ROAD CONSTRUCTION PLAN
ROAD PROFILE AND DETAILS**

DATE: MARCH, 2014 PROJECT NO. 2171
DESIGN: JMC DRAFT: JMC SCALE: AS SHOWN DRAWING 4 OF 33

AS-BUILT

F-13-112



LEGEND

SOILS CLASSIFICATION	ChB2	TREE PROTECTION FENCE	—//—
SOILS DELINEATION	—480—	FCE PERMANENT SIGNAGE	⬢
EXISTING CONTOURS (AERIAL 12/02)	—478—	PROP. STREET TREE	⊙
PROPOSED CONTOURS	—999—	NON-ROOFTOP DISCONNECTION AND RECEIVING AREA	▨
LIMIT OF WETLANDS	—	PRIVATE SEWAGE DISPOSAL AREA	▨
25' WETLANDS BUFFER	—	PRIVATE WELL AREA	▨
CENTERLINE OF STREAM	—	LIMIT OF DISTURBANCE	—
STREAM BUFFER	—	SUPER SILT FENCE	—
EXISTING WOODS LINE	—	STABILIZED CONSTRUCTION ENTRANCE	—
PROPOSED WOODS LINE	—	SLOPES 15% TO 24.9%	▨
EXISTING STRUCTURE	—	SLOPES 25% OR GREATER	▨
PROPOSED STRUCTURE	—	100 YEAR FLOODPLAIN	—
SLOPES 15% TO 24.9%	▨	PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)	▨
SLOPES 25% OR GREATER	▨	PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)	▨
100 YEAR FLOODPLAIN	—		
PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)	▨		
PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)	▨		

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

Professional Engineer
 License No. 45577 Expiration Date: 6/16/2016

AS-BUILT CERTIFICATION
 I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications

Donald Mason, P.E. Date: 11-15-17



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

License No. 21443 Expiration Date: 12-21-19

ENGINEER'S CERTIFICATE

I HEREBY CERTIFY THAT THIS PLAN FOR SEDIMENT AND EROSION CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.

4/10/2014
 DATE

DEVELOPER'S CERTIFICATE

I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN OF DEVELOPMENT FOR SEDIMENT AND EROSION CONTROL, AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZED PERIODIC ON-SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT.

4/13/14
 DATE

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.

4/29/14
 DATE

APPROVED: DEPARTMENT OF PUBLIC WORKS

4-29-14
 DATE

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

5-2-14
 DATE

5-9-14
 DATE

NO.	DATE	REVISION
2	9-16-15	REVISE GUARDRAIL ALONG POINT RIDGE DRIVE
1	2-9-15	REMOVE BARN LOD. BARN PREVIOUSLY REMOVED BY PRIOR OWNERS

BENCHMARK ENGINEERING, INC.

8480 BALTIMORE NATIONAL PIKE A SUITE 315 A ELICOTT CITY, MARYLAND 21043
 (P) 410-465-8105 (F) 410-465-8844
 75 THOMAS JOHNSON DRIVE A SUITE E A FREDERICK, MARYLAND 21702
 301-770-5685
 WWW.BE-CVLENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 28559, Expiration Date: 02-23-2015.

4/10/2014
 DATE

OWNER/DEVELOPER: MB HIGHLAND RESERVE, LLC
 1686 EAST GUDE DRIVE
 ROCKVILLE, MD 20850
 301-762-9511

PROJECT: **REGAN PROPERTY**
 LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E'. A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BUILDING PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. _____

LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE: **FINAL ROAD CONSTRUCTION PLAN GRADING, SEDIMENT AND EROSION CONTROL PLAN AND SOILS MAP**

DATE: MARCH, 2014 PROJECT NO. 2171

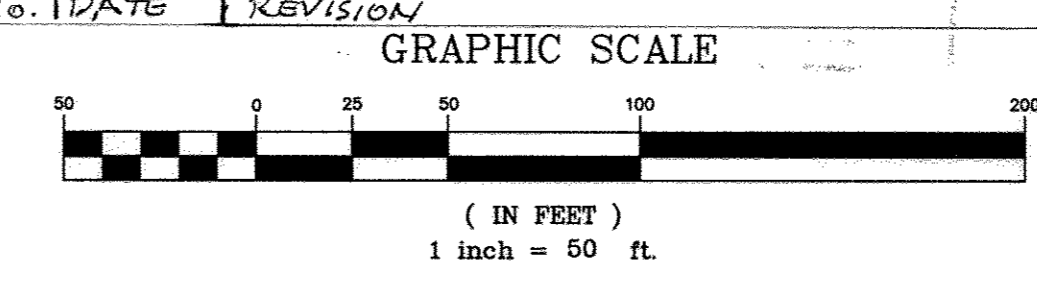
SCALE: AS SHOWN DRAWING 5 OF 33

DESIGN: JCO DRAFT: JCO

SOILS LEGEND

MAP SYMBOL	SOIL GROUP	SOIL TYPE
Ba*	D	BAILE SILT LOAM, 0 TO 3 PERCENT SLOPES
GbA	B	GLADSTONE LOAM, 0 TO 3 PERCENT SLOPES
GbB	B	GLADSTONE LOAM, 3 TO 8 PERCENT SLOPES
GbC	B	GLADSTONE LOAM, 8 TO 15 PERCENT SLOPES
GmB*	C	GLENVILLE SILT LOAM, 3 TO 8 PERCENT SLOPES
MaD	B	MANOR LOAM, 15 TO 25 PERCENT SLOPES

FROM NATURAL RESOURCES CONSERVATION SERVICES WEB SOIL SURVEY 2.0 * ERODIBLE SOILS

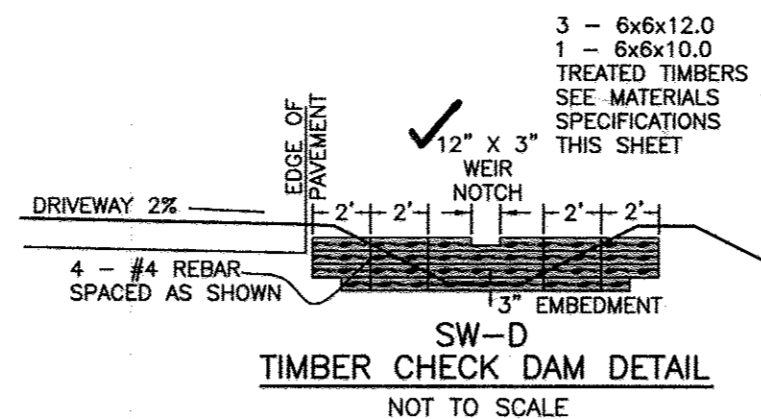


PLAN VIEW
 SCALE: 1" = 50'

SOILS LEGEND		
MAP SYMBOL	SOIL GROUP	SOIL TYPE
BaA*	D	BAILE SILT LOAM, 0 TO 3 PERCENT SLOPES
GbA	B	GLADSTONE LOAM, 0 TO 3 PERCENT SLOPES
GbB	B	GLADSTONE LOAM, 3 TO 8 PERCENT SLOPES
GbC	B	GLADSTONE LOAM, 8 TO 15 PERCENT SLOPES
GmB*	C	GLENVILLE SILT LOAM, 3 TO 8 PERCENT SLOPES
MaD	B	MANOR LOAM, 15 TO 25 PERCENT SLOPES

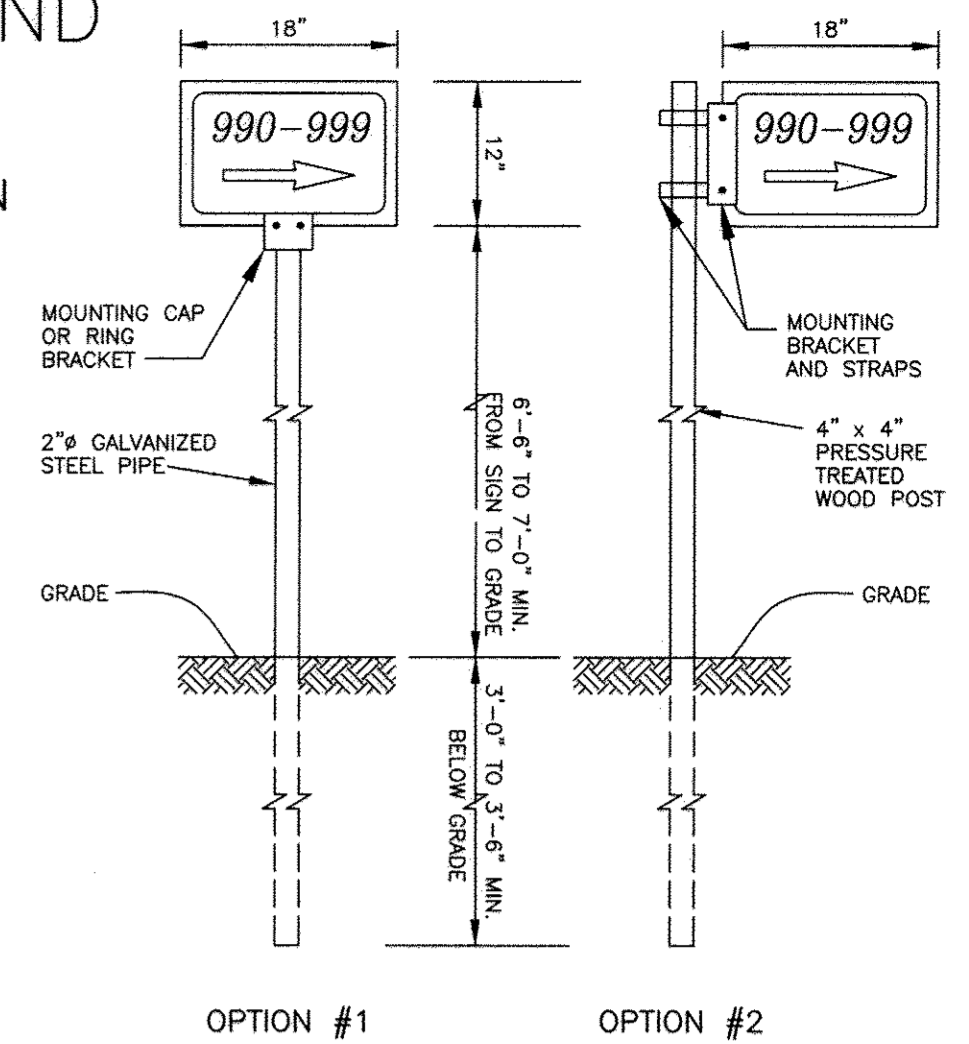
FROM NATURAL RESOURCES CONSERVATION SERVICES WEB SOIL SURVEY 2.0
* ERODIBLE SOILS

DRIVEWAY NOTE: PORTIONS OF THE DRIVEWAYS ON LOTS 5 & 6 ARE NOT TREATED ON-SITE. ANY AREA NOT DRAINING TO AN ON-SITE FACILITY OR DISCONNECTION MUST BE CONSTRUCTED AT THE MINIMUM ALLOWABLE WIDTH.



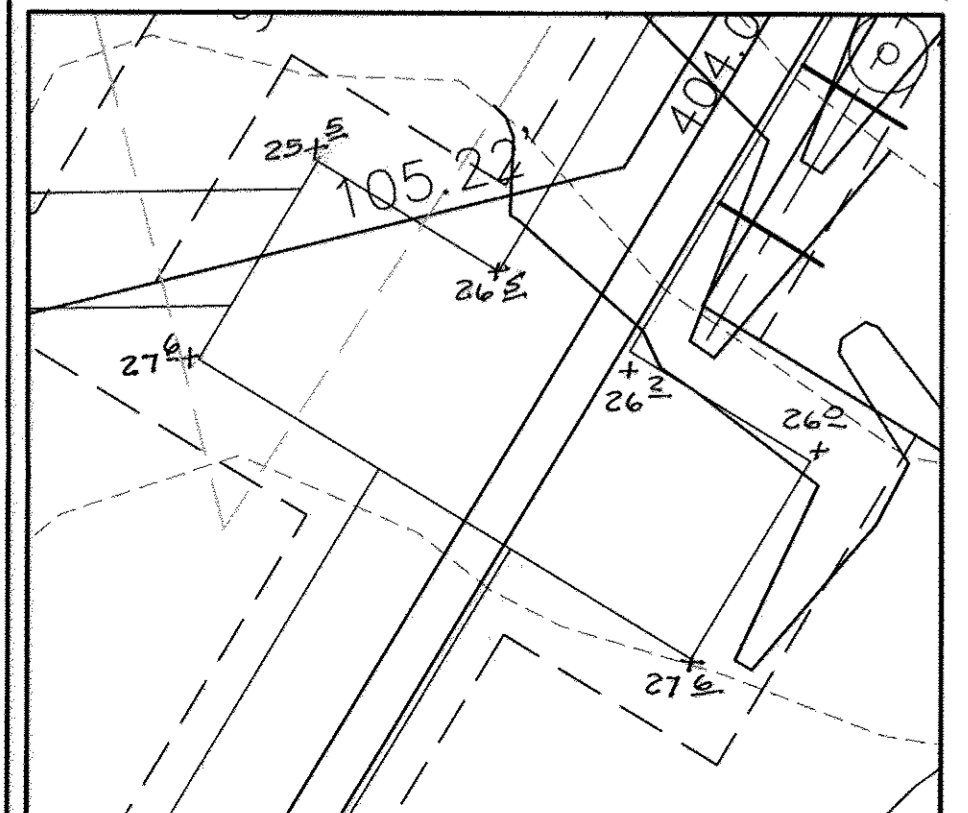
SEE PREVIOUS SHEET FOR LEGEND

- NOTE:
1. SHOULD THE PROPOSED STOCKPILE AREA EXCEED 15' IN HEIGHT, BENCHING OF THE STOCKPILE IS REQUIRED.
 2. CONSTRUCTION ON LOT 2 IS TO BE DELAYED UNTIL PERMISSION IS RECEIVED FROM THE SEDIMENT CONTROL INSPECTOR TO RELOCATE OR REMOVE THE PROPOSED STOCKPILE.

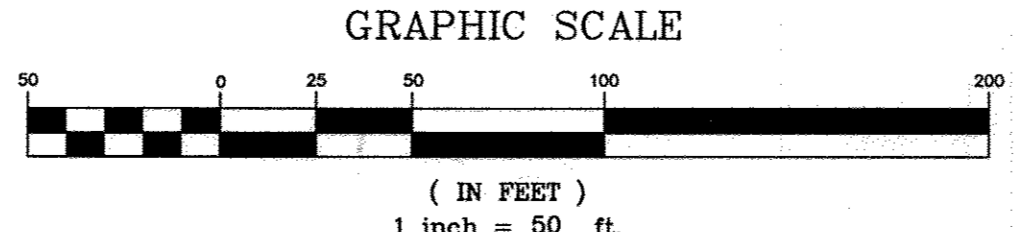


HO. CO. APPROVED SIGN DESIGN AND INSTALLATION DETAIL
NOT TO SCALE

- THE FOLLOWING STANDARD SIGN DESIGN SPECIFICATIONS SHALL APPLY:
1. THE SIGN SIZE SHALL BE 12" x 18".
 2. THE SIGN MATERIAL SHALL BE 0.080 GAUGE THICKNESS ANODIZED ALUMINUM.
 3. THE SIGN SHALL HAVE A GREEN BACKGROUND WITH 3" HIGH REFLECTIVE NUMBERS AND ARROW WITH A WHITE REFLECTIVE BORDER.
 4. WHERE A PRIVATE ROAD NAME IS IN USE OR PART OF A PRIVATE HOMEOWNER'S ARTICLES OF INCORPORATION AGREEMENT THE SIGN SIZE WILL BE ENLARGED TO ACCOMMODATE THE NECESSARY LETTERING BUT REMAIN PROPORTIONAL TO THE ABOVE DESIGN LIMITS.
 5. THE SIGN WILL BE INSTALLED WITHIN THE COMMON DRIVEWAY EASEMENT AREA AS NOTED ON THE FINAL PLAN.
 6. ADDRESS NUMBER IDENTIFICATION SIGNS ARE TO BE PROVIDED UNDER THE TENANTS OF THE HOMEOWNER'S ASSOCIATION INCORPORATION OR A PROPERTY MANAGEMENT COMPANY FOR INSTALLATION AND MAINTENANCE IN ACCORDANCE WITH THE DEPARTMENT OF PLANNING AND ZONING ADDRESS NUMBERING SYSTEM AND PER SECTION 3.503(C) OF THE HOWARD COUNTY CODE - PUBLIC SIGNS. MAINTENANCE/REPAIR AND REPLACEMENT OF THE ADDRESS NUMBER DIRECTIONAL SIGNS WILL BE THE RESPONSIBILITY OF THE HOMEOWNER'S ASSOCIATION OR A PROPERTY MANAGEMENT COMPANY.
 7. COMPLIANCE REGARDING THE INSTALLATION OF THE NEW ADDRESS NUMBER DIRECTIONAL SIGNS WILL BE ENFORCED BY THE DEPARTMENT OF INSPECTIONS, LICENSES AND PERMITS AT THE TIME OF FINAL APPROVAL FOR ISSUANCE OF THE USE AND OCCUPANCY PERMITS.



PLAN VIEW
SCALE: 1" = 50'



AS-BUILT CERTIFICATION
I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications
Donald Mason, P.E. Date: 11-15-17



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443, Expiration Date: 12-21-18

NOTE:
THIS PLAN IS NOT TO BE USED FOR RESIDENTIAL BUILDING PERMITS.

REGAN PROPERTY WETLAND MITIGATION PLAN PHASE I OVERVIEW

To mitigate for the proposed impacts required for the stream and wetland crossing, the applicant is proposing to provide onsite stream channel restoration and wetland mitigation.
The stream channel restoration will be accomplished by removing an existing culvert system and creating a stable open channel through the proposed road crossing. The channel will be constructed through the proposed arch crossing. To minimize impact to the stream, the flow will be maintained through the existing culvert system while the new channel and crossing are being constructed. Once the new channel is in place, the flow will be redirected into the new channel and the old culvert will be removed and filled. Streambank plantings will be incorporated into the stream design to provide shading and habitat along the new channel.
Wetland mitigation will be accomplished by expanding existing wetland system on the site. The mitigation site location was selected because it is adjacent to existing wetlands, has low slope and is currently dominated by herbaceous upland species. To obtain the appropriate hydrologic input to the wetland, the grades within the mitigation area will be lowered to mimic the grade of the adjacent wetland. Monitoring of hydrologic conditions within the adjacent wetlands will be performed using a nearby growing season to determine the hydrologic nature of these wetlands. Typically the grades in the constructed wetlands will be slightly below the adjacent wetland grades to help insure proper hydrologic input. Should elevations be too low, resulting in standing water in the wetland area, a small drainage channel can be constructed to move the excess water from the site. The mitigation area will contain lower basins that will collect and hold standing water for extended periods to help provide additional input into the wetland system.
Topsoil should be salvaged and replaced whenever possible to a depth of 6 inches. Site should be graded to below 6 inches of final grade, then 6 inches topsoil, reclaimed or imported, shall be spread over the site. Soil and substrate amendments need to meet hydric soil characteristics and maintain the specified plant species. A minimum of 60 cubic yards of organic matter per acre is required. The addition of supplemental large woody debris will also be recommended. Upon completion of grading, the soil must be disked or chisel plowed to a depth of at least 8 inches to avoid soil compaction. Side slopes extending out of the wetland will be graded to 3:1 where possible and no greater than 2:1.
Though the impacted wetlands are emergent, we would propose a mix of trees, shrubs and herbaceous species within the mitigation area to maximize function and habitat value.
Species to be used may include:
Black willow, Red maple, Pin oak, Winterberry, Buttonbush, Smooth alder, Duck potato, Soft rush, Wool grass, New York ironweed, Swamp beggar ticks, Joe-pye weed.
Installation of tree and shrub stock will be based on the use of 23 foot tall, container grown plants installed at 9 foot spacing. Herbaceous plantings will be comprised of 2" plugs installed on 2 foot spacing. Standard MDE mitigation survival requirements and monitoring protocol will be followed.
Native wetland and upland seed mixes will be utilized to further enhance the vegetative community development of the wetlands and to stabilize adjacent uplands.

ENGINEER'S CERTIFICATE
I, HEREBY CERTIFY THAT THIS PLAN FOR SEDIMENT AND EROSION CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.
BRIAN F. CLEARY
ENGINEER - BRIAN F. CLEARY
4/10/2014
DATE

DEVELOPER'S CERTIFICATE
I, HEREBY CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN OF DEVELOPMENT FOR SEDIMENT AND EROSION CONTROL AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT.
Martin J. Mitchell
DEVELOPER
4/13/14
DATE

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.
John R. Robertson
HOWARD SCD
4/8/14
DATE

APPROVED: DEPARTMENT OF PUBLIC WORKS
WILLIAM T. WALL
CHIEF, BUREAU OF HIGHWAYS
4-29-14
DATE

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
KATHY J. BROWN
CHIEF, DIVISION OF LAND DEVELOPMENT
5-21-14
DATE
5-9-14
DATE

NO.	DATE	REVISION
1	2-9-15	EXPAND WETLAND MITIGATION & SOIL AREA; REMOVE LOD FOR BARN

BENCHMARK ENGINEERING, INC.
ENGINEERS & LAND SURVEYORS & PLANNERS
8480 BALTIMORE NATIONAL PIKE SUITE 315 ELLICOTT CITY, MARYLAND 21143
(P) 410-465-6105 (F) 410-465-6844
75 THOMAS JOHNSON DRIVE SUITE E FREDERICK, MARYLAND 21702
WWW.BE-COMENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 28559, Expiration Date: 12-31-2015.
STATE OF MARYLAND PROFESSIONAL ENGINEER
4/10/2014

OWNER/DEVELOPER: MB HIGHLAND RESERVE, LLC 1686 EAST GUDE DRIVE ROCKVILLE, MD 20850 301-762-9511	PROJECT: REGAN PROPERTY LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' THROUGH A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 200
LOCATION: TAX MAP NO. 34, GRID NO. 24, PARCEL NO. 200 3RD ELECTION DISTRICT HOWARD COUNTY, MARYLAND DPZ NO.: SP-12-004, ECP-12-045, WP-13-025	TITLE: FINAL ROAD CONSTRUCTION PLAN GRADING, SEDIMENT AND EROSION CONTROL PLAN AND SOILS MAP
DATE: MARCH, 2014	PROJECT NO. 2171
DESIGN: JCO	DRAFT: JCO
SCALE: AS SHOWN	DRAWING 8 OF 33

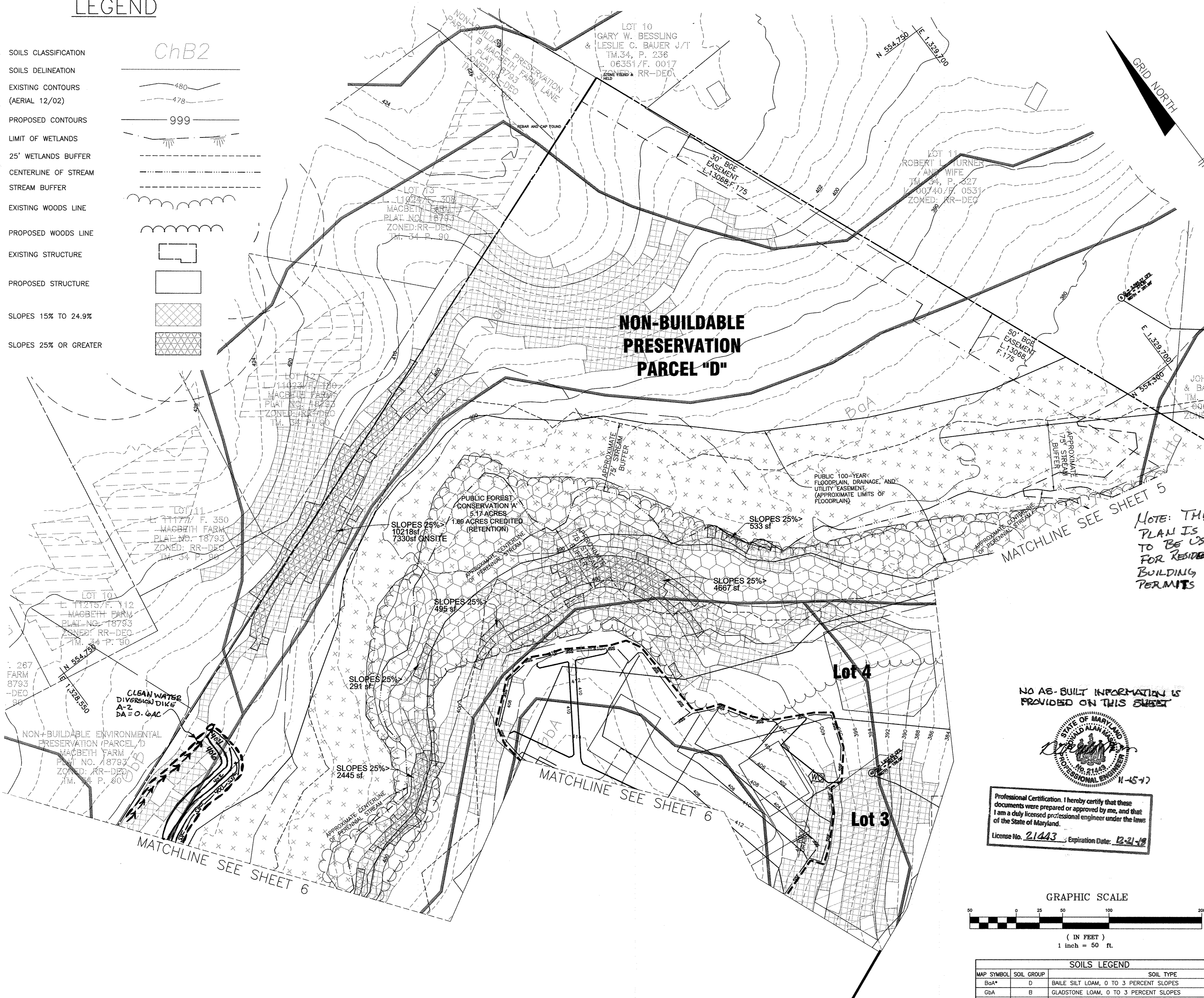
AS-BUILT

LEGEND

- SOILS CLASSIFICATION ChB2
- SOILS DELINEATION
- EXISTING CONTOURS (AERIAL 12/02)
- PROPOSED CONTOURS
- LIMIT OF WETLANDS
- 25' WETLANDS BUFFER
- CENTERLINE OF STREAM
- STREAM BUFFER
- EXISTING WOODS LINE
- PROPOSED WOODS LINE
- EXISTING STRUCTURE
- PROPOSED STRUCTURE
- SLOPES 15% TO 24.9%
- SLOPES 25% OR GREATER

LEGEND

- 100 YEAR FLOODPLAIN
- PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)
- PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)
- TREE PROTECTION FENCE
- FCE PERMANENT SIGNAGE
- PROP. STREET TREE
- NON-ROOFTOP DISCONNECTION AND RECEIVING AREA
- PRIVATE SEWAGE DISPOSAL AREA
- PRIVATE WELL AREA
- LIMIT OF DISTURBANCE
- SUPER SILT FENCE
- STABILIZED CONSTRUCTION ENTRANCE
- STORMWATER MANAGEMENT BORING LOCATION
- EROSION CONTROL MATTING
- EARTH DIKE
- INLET PROTECTION

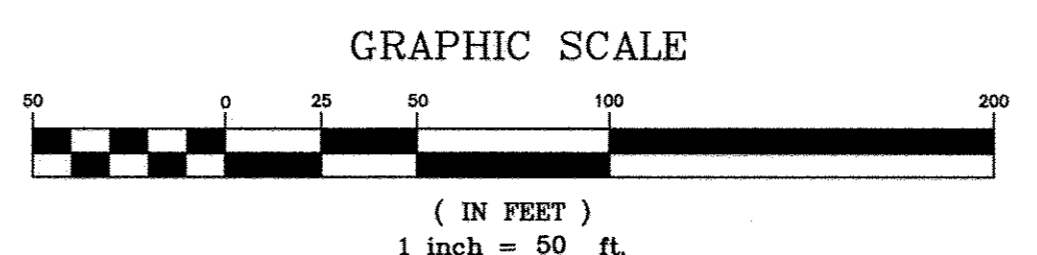


NOTE: THIS PLAN IS NOT TO BE USED FOR RESIDENTIAL BUILDING PERMITS

NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443 Expiration Date: 12-31-18



SOILS LEGEND		
MAP SYMBOL	SOIL GROUP	SOIL TYPE
Ba*	D	BAILE SILT LOAM, 0 TO 3 PERCENT SLOPES
GbA	B	GLADSTONE LOAM, 0 TO 3 PERCENT SLOPES
GbB	B	GLADSTONE LOAM, 3 TO 8 PERCENT SLOPES
GbC	B	GLADSTONE LOAM, 8 TO 15 PERCENT SLOPES
GmB*	C	GLENVILLE SILT LOAM, 3 TO 8 PERCENT SLOPES
MgD	B	MANOR LOAM, 15 TO 25 PERCENT SLOPES

FROM NATURAL RESOURCES CONSERVATION SERVICES WEB SOIL SURVEY 2.0
 * ERODIBLE SOILS

PLAN VIEW
 SCALE: 1" = 50'

ENGINEER'S CERTIFICATE
 I HEREBY CERTIFY THAT THIS PLAN FOR SEDIMENT AND EROSION CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.
 ENGINEER - BRIAN F. CLEARY 4/10/2014 DATE

DEVELOPER'S CERTIFICATE
 I HEREBY CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN OF DEVELOPMENT FOR SEDIMENT AND EROSION CONTROL AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZED A FIELD ON-SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT.
 DEVELOPER - MARTIN MITCHELL 4/3/14 DATE

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.
 JOHN R. ROBERTSON 4/9/14 DATE
 HOWARD SCD

APPROVED: DEPARTMENT OF PUBLIC WORKS
 WILSON 4-29-14 DATE
 CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 [Signature] 5-21-14 DATE
 CHIEF, DIVISION OF LAND DEVELOPMENT

APPROVED: [Signature] 5-9-14 DATE
 CHIEF, DEVELOPMENT ENGINEERING DIVISION

NO.	DATE	REVISION
1	2-9-15	SHOW SOIL AREA, LOD + SEC FOR EARTH WORKS

BENCHMARK ENGINEERING, INC.
 ENGINEERS • LAND SURVEYORS • PLANNERS
 8480 BALTIMORE NATIONAL PIKE & SUITE 315 & ELLICOTT CITY, MARYLAND 21043
 (P) 410-465-6105 (F) 410-465-6644
 75 THOMAS JOHNSON DRIVE & SUITE E & FREDERICK, MARYLAND 21702
 WWW.BE-ONLINEENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443 Expiration Date: 12-31-18

OWNER/DEVELOPER:
 MB HIGHLAND RESERVE, LLC
 1686 EAST GUDE DRIVE
 ROCKVILLE, MD 20850
 301-762-9511

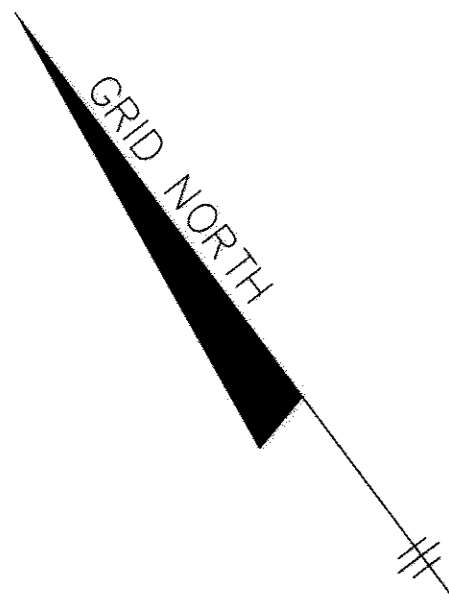
PROJECT: **REGAN PROPERTY**
 LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 200

LOCATION:
 TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 5th ELECTION DISTRICT
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
**FINAL ROAD CONSTRUCTION PLAN
 GRADING, SEDIMENT AND EROSION
 CONTROL PLAN AND SOILS MAP**

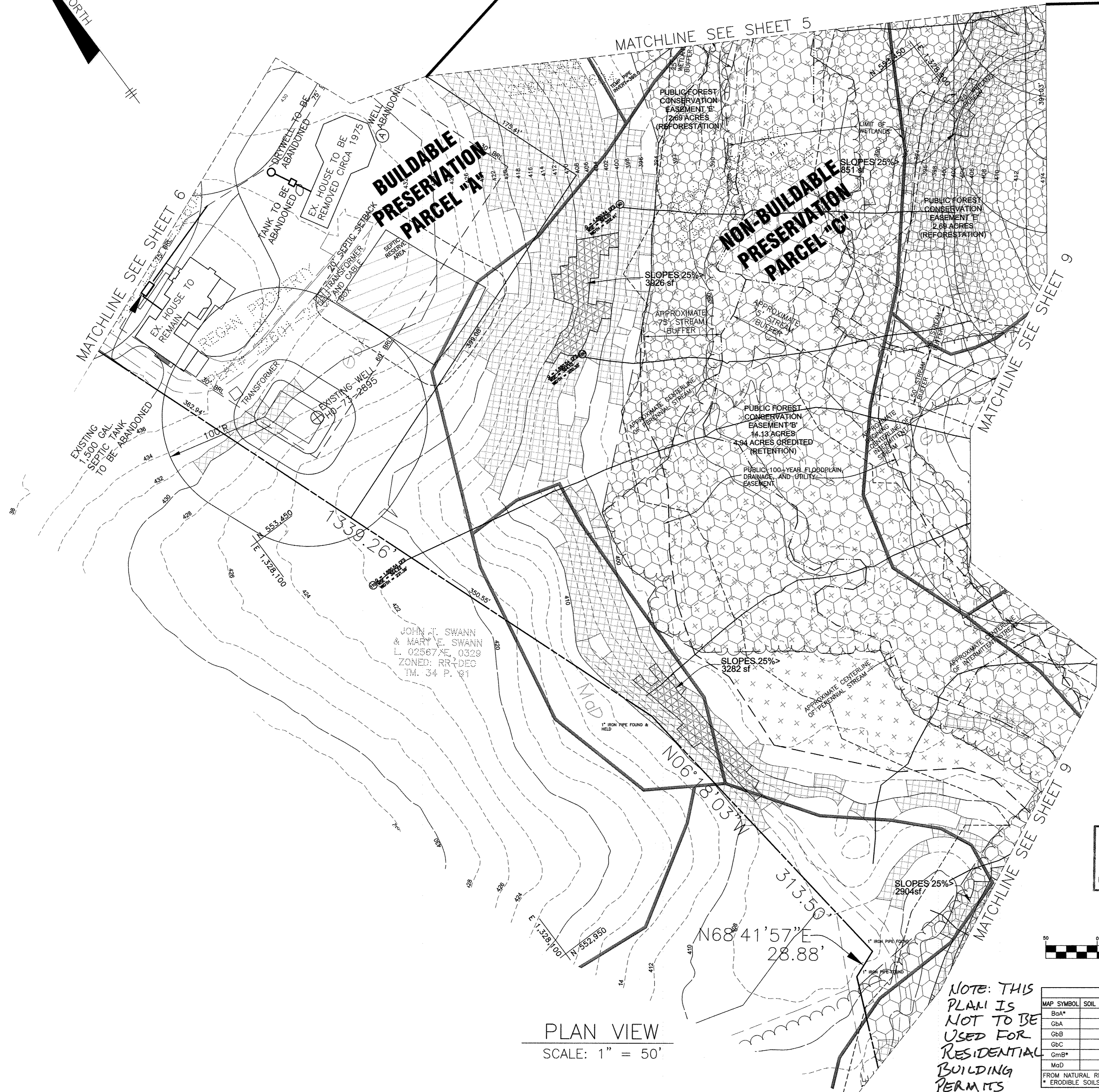
DATE: MARCH, 2014 PROJECT NO. 2171

DESIGN: JCO DRAFT: JCO SCALE: AS SHOWN DRAWING 7 OF 33



LEGEND

- SOILS CLASSIFICATION ChB2
- SOILS DELINEATION
- EXISTING CONTOURS (AERIAL 12/02)
- PROPOSED CONTOURS
- LIMIT OF WETLANDS
- 25' WETLANDS BUFFER
- CENTERLINE OF STREAM
- STREAM BUFFER
- EXISTING WOODS LINE
- PROPOSED WOODS LINE
- EXISTING STRUCTURE
- PROPOSED STRUCTURE
- SLOPES 15% TO 24.9%
- SLOPES 25% OR GREATER
- 100 YEAR FLOODPLAIN
- PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)
- PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)
- TREE PROTECTION FENCE
- FCE PERMANENT SIGNAGE
- PROP. STREET TREE
- NON-ROOFTOP DISCONNECTION AND RECEIVING AREA
- PRIVATE SEWAGE DISPOSAL AREA
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- LIMIT OF DISTURBANCE
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- STABILIZED CONSTRUCTION ENTRANCE
- STORMWATER MANAGEMENT BORING LOCATION
- EROSION CONTROL MATTING
- EARTH DIKE
- INLET PROTECTION



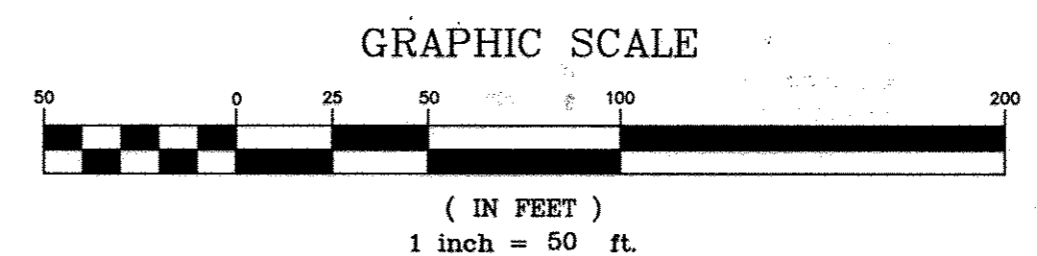
PLAN VIEW
SCALE: 1" = 50'

NOTE: THIS PLAN IS NOT TO BE USED FOR RESIDENTIAL BUILDING PERMITS

NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. _____ Expiration Date: _____



MAP SYMBOL	SOIL GROUP	SOIL TYPE
Ba*	D	BAILE SILT LOAM, 0 TO 3 PERCENT SLOPES
GbA	B	GLADSTONE LOAM, 0 TO 3 PERCENT SLOPES
GbB	B	GLADSTONE LOAM, 3 TO 8 PERCENT SLOPES
GbC	B	GLADSTONE LOAM, 8 TO 15 PERCENT SLOPES
Gmb*	C	GLENNVILLE SILT LOAM, 3 TO 8 PERCENT SLOPES
MaD	B	MANOR LOAM, 15 TO 25 PERCENT SLOPES

FROM NATURAL RESOURCES CONSERVATION SERVICES WEB SOIL SURVEY 2.0
* ERODIBLE SOILS

ENGINEER'S CERTIFICATE
I HEREBY CERTIFY THAT THIS PLAN FOR SEDIMENT AND EROSION CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.
Brian F. Cleary
ENGINEER - BRIAN F. CLEARY
DATE: 4/10/2014

DEVELOPER'S CERTIFICATE
I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN OF DEVELOPMENT FOR SEDIMENT AND EROSION CONTROL, AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE SOILS ON-SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT.
Martin J. Mitchell
DEVELOPER
DATE: 4/3/14

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.
John R. Reardon
HOWARD SCD
DATE: 4/8/14

APPROVED: DEPARTMENT OF PUBLIC WORKS
Will T. ...
CHIEF, BUREAU OF HIGHWAYS
DATE: 4-29-14

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
...
CHIEF, DIVISION OF LAND DEVELOPMENT
DATE: 5-2-14

...
CHIEF, DEVELOPMENT ENGINEERING DIVISION
DATE: 5-9-14

NO.	DATE	REVISION

BENCHMARK ENGINEERING, INC.
ENGINEERS & LAND SURVEYORS & PLANNERS
8480 BALTIMORE NATIONAL PIKE & SUITE 315 & ELLICOTT CITY, MARYLAND 21043
(P) 410-465-6105 (F) 410-465-6844
75 THOMAS JOHNSON DRIVE & SUITE E & FREDERICK, MARYLAND 21702
301-710-5888
WWW.BE-ONLINEENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 28559, Expires 03/31/2015.
...
DATE: 4/10/2014

OWNER/DEVELOPER:
MB HIGHLAND RESERVE, LLC
1686 EAST GUDE DRIVE
ROCKVILLE, MD 20850
301-762-9511

PROJECT: **REGAN PROPERTY**
LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. ...

LOCATION:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200
5th ELECTION DISTRICT
HOWARD COUNTY, MARYLAND
DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
FINAL ROAD CONSTRUCTION PLAN
GRADING, SEDIMENT AND EROSION
CONTROL PLAN AND SOILS MAP

DATE: MARCH, 2014 PROJECT NO. 2171

DESIGN: JCO DRAFT: JCO SCALE: AS SHOWN DRAWING 8 OF 33

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. **21443**, Expiration Date: **12-21-18**



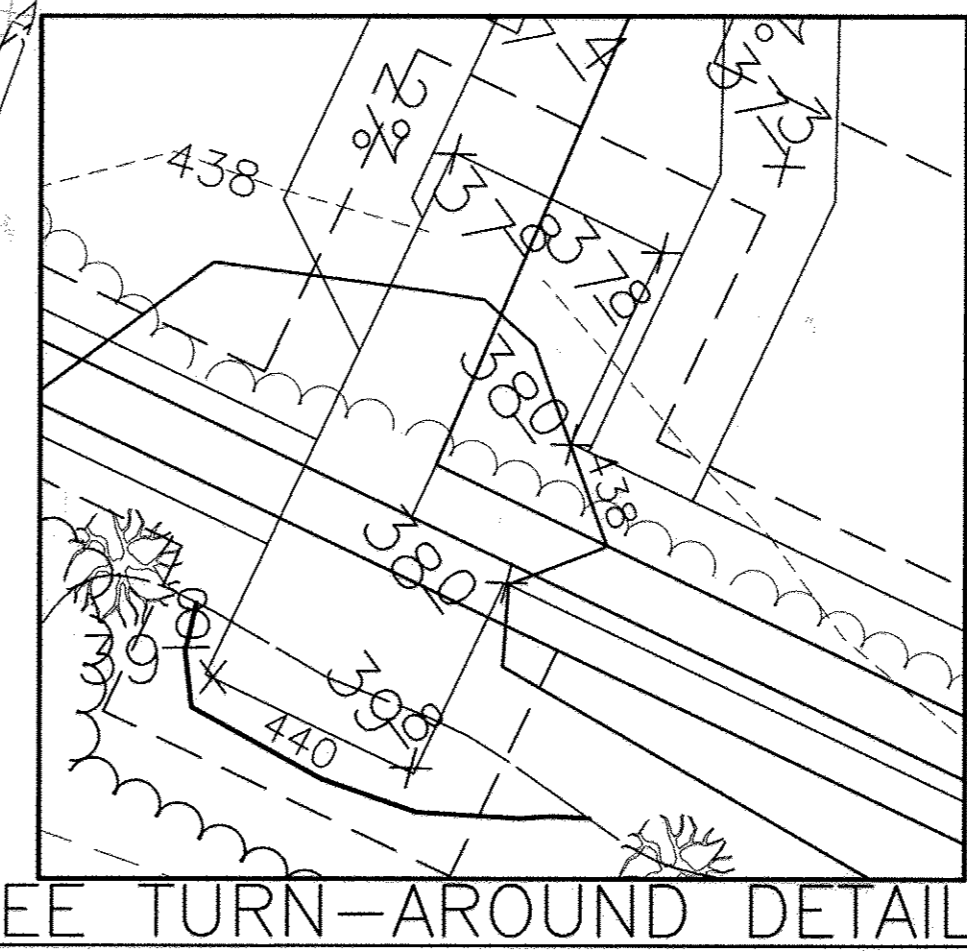
AS-BUILT CERTIFICATION
 I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications
 Donald Mason, P.E.
 Date: **11-15-17**

LEGEND

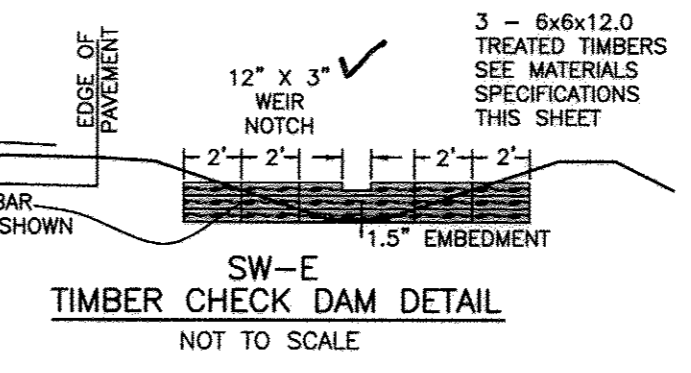
SOILS CLASSIFICATION	ChB2	TREE PROTECTION FENCE	//
SOILS DELINEATION		FCE PERMANENT SIGNAGE	⬢
EXISTING CONTOURS (AERIAL 12/02)	480 478	PROP. STREET TREE	⊙
PROPOSED CONTOURS	999	NON-ROOFTOP DISCONNECTION AND RECEIVING AREA	▨
LIMIT OF WETLANDS		PRIVATE SEWAGE DISPOSAL AREA	▩
25' WETLANDS BUFFER		PRIVATE WELL AREA	▩
CENTERLINE OF STREAM		LIMIT OF DISTURBANCE	-----
STREAM BUFFER		SUPER SILT FENCE	-----
EXISTING WOODS LINE		STABILIZED CONSTRUCTION ENTRANCE	SCS
PROPOSED WOODS LINE		STORMWATER MANAGEMENT BORING LOCATION	B-8 437.0
EXISTING STRUCTURE	▭	EROSION CONTROL MATTING	ECM
PROPOSED STRUCTURE	▭	EARTH DIKE	←←←←←
SLOPES 15% TO 24.9%	▨	INLET PROTECTION	AGIP
SLOPES 25% OR GREATER	▨		
100 YEAR FLOODPLAIN	+		
PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)	▨		
PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)	▨		



NOTE: THIS PLAN IS NOT TO BE USED FOR RESIDENTIAL BUILDING PERMITS.



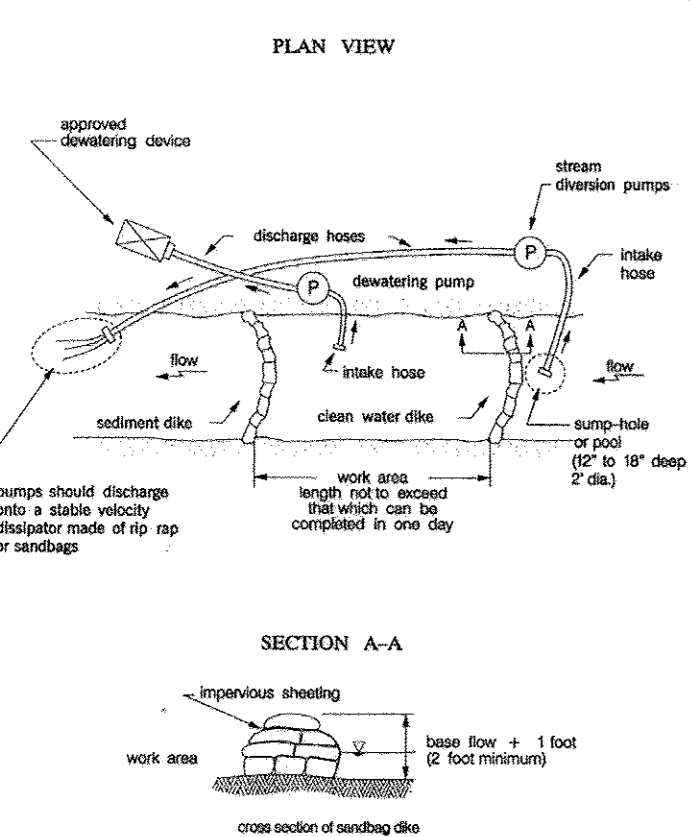
SCALE: 1" = 20'



NOT TO SCALE

NON-BUILDABLE PRESERVATION PARCEL "B"

Maryland's Guidelines to Waterway Construction
DETAIL 1.2: PUMP-AROUND PRACTICE



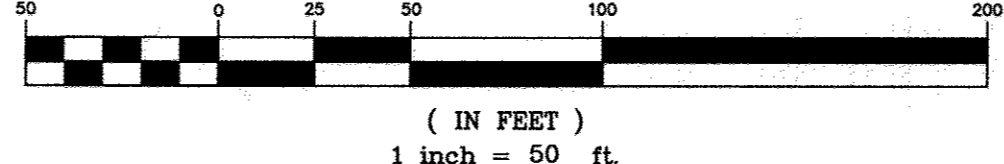
MGWC 1.2: PUMP-AROUND PRACTICE

DESCRIPTION
 Temporary measure for diverting in-channel construction sites.
IMPLEMENTATION SEQUENCE
 1. Construction activities including the installation of erosion and sediment control measures should not begin until all necessary easements and permits have been obtained. All existing utilities should be located in the field prior to construction. The contractor is responsible for any damage to existing utilities that may result from construction and should repair the damage at his/her own expense to the county's or utility company's satisfaction.
 2. The contractor should notify the Maryland Department of the Environment or WMA sediment control inspector at least 5 days before beginning construction. Additionally, the contractor should inform the local environmental protection and resource management inspection and enforcement division and the provider of local utilities a minimum of 48 hours before starting construction.
 3. The contractor should conduct a pre-construction meeting on site with the WMA sediment control inspector, the county project manager, and the engineer to review final plans, discuss erosion and sediment control requirements, and the sequence of construction. The contractor should make out all limits of disturbance prior to pre-construction meeting so they may be reviewed. The participants will designate the contractor's staging area and flag all areas within the limit of disturbance which will be removed for construction access. These should not be removed within the limit of disturbance without approval from the WMA or local authority.
 4. Construction should not begin until all sediment and erosion control measures have been installed and approved by the engineer and the sediment control inspector. The contractor should stay within the limits of the disturbance as shown on the plans and maintain disturbance within the work area whenever possible.
 5. Upon installation of all sediment control measures and approval by the sediment control inspector and the local environmental protection and resource management inspection and enforcement division, the contractor should begin work at the upstream section and proceed downstream beginning with the establishment of stabilized construction easements. In some cases, work may begin downstream if appropriate. The sequence of construction must be followed and when the contractor gets written approval for the work from the WMA or local authority. The contractor should only begin work in an area which can be completed by the end of the day including grading adjacent to the channel. At the end of each work day, the work area must be stabilized and the pump-around removed from the channel. Work should not be conducted in the channel during rain events.
 6. Sandbag dikes should be situated at the upstream and downstream ends of the work area as shown on the plans, and stream flow should be stopped across the work area. The pump should discharge into a stable velocity dissipater made of riprap or sandbags.

MGWC 1.3: PUMP-AROUND PRACTICE

DESCRIPTION
 Temporary measure for diverting in-channel construction sites.
IMPLEMENTATION SEQUENCE
 1. Water from the work area should be pumped to a sediment filtering measure such as a downspout, silt, sediment bag, or other approved source. The measure should be located such that the water drains back into the channel before the downstream section of the dike.
 2. Traversing a channel reach with equipment within the work area where no work is proposed should be avoided. If equipment has to traverse such a reach for access to another area, then timber mats or similar measures should be used to minimize disturbance to the channel. Temporary stream crossings should be used only when necessary and only where noted on the plans or specified. (See Section 4. Stream Crossings, Maryland Guidelines to Waterway Construction).
 3. All stream restoration measures should be installed as indicated by the plans and all beds graded in accordance with the grading plans and typical cross-sections. All grading must be stabilized at the end of each day with seed and mulch or soil and mulch as specified on the plans.
 4. After an area is completed and stabilized, the stream water dikes should be removed. After the final sediment flush, a new clear water dike should be established upstream from the old sediment dike. Finally, upon completion of a new sediment dike below the old one, the old sediment dike should be removed.
 5. A pump-around must be installed on any tributary or stream reach which contributes flow to the work area. This should be accomplished by locating a sandbag dike at the downstream end of the tributary or stream velocity dissipater used for the main river pump-around. This water should discharge onto the same velocity dissipater used for the main river pump-around.
 6. If a tributary is to be restored, construction should take place on the tributary before work on the main stream reaches the tributary confluence. Construction in the tributary, including pump-around practices, should follow the same sequence as for the main stem of the river or stream. When construction on the tributary is completed, work on the main stem should resume. Water from the tributary should continue to be pumped around the work area in the main stem.
 7. The contractor is responsible for providing access to and maintaining all erosion and sediment control devices until the sediment control inspector approves their removal.
 8. After construction, all disturbed areas should be regraded and revegetated as per the planting plan.

PLAN VIEW
 SCALE: 1" = 50'



SOILS LEGEND

MAP SYMBOL	SOIL GROUP	SOIL TYPE
BaA*	D	BAILE SILT LOAM, 0 TO 3 PERCENT SLOPES
GbA	B	GLADSTONE LOAM, 0 TO 3 PERCENT SLOPES
GbB	B	GLADSTONE LOAM, 3 TO 8 PERCENT SLOPES
GbC	B	GLADSTONE LOAM, 8 TO 15 PERCENT SLOPES
GmB*	C	GLENVILLE SILT LOAM, 3 TO 8 PERCENT SLOPES
MoD	B	MANOR LOAM, 15 TO 25 PERCENT SLOPES

FROM NATURAL RESOURCES CONSERVATION SERVICES WEB SOIL SURVEY 2.0
 * ERODIBLE SOILS

ENGINEER'S CERTIFICATE
 I HEREBY CERTIFY THAT THIS PLAN FOR SEDIMENT AND EROSION CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.
 ENGINEER - **Brian F. Keary** DATE: **4/10/2014**

DEVELOPER'S CERTIFICATE
 I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN OF DEVELOPMENT FOR SEDIMENT AND EROSION CONTROL, AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT.
 DEVELOPER - **Martin J. Michaels** DATE: **4/2/14**

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.
John K. Redman DATE: **4/9/14**
 HOWARD SCD

APPROVED: DEPARTMENT OF PUBLIC WORKS
William T. Hall DATE: **4-29-14**
 CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
Veit Schmalzer DATE: **5-21-14**
 CHIEF, DIVISION OF LAND DEVELOPMENT

Michael J. Hall DATE: **5-9-14**
 CHIEF, DEVELOPMENT ENGINEERING DIVISION

BENCHMARK ENGINEERING, INC.
 ENGINEERS & LAND SURVEYORS & PLANNERS
 8480 BALTIMORE NATIONAL PIKE & SUITE 315 A ELLICOTT CITY, MARYLAND 21043
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 WWW.BEI-ENGINEERING.COM

OWNER/DEVELOPER:
 MB HIGHLAND RESERVE, LLC
 1686 EAST GUDE DRIVE
 ROCKVILLE, MD 20850
 301-762-9511

PROJECT: REGAN PROPERTY
 LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL "A", and NON-BUILDABLE PRESERVATION PARCELS "B" thru "E" A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL "A" PREVIOUSLY RECORDED AS PLAT NO. _____

LOCATION:
 TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 ELECTION DISTRICT
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

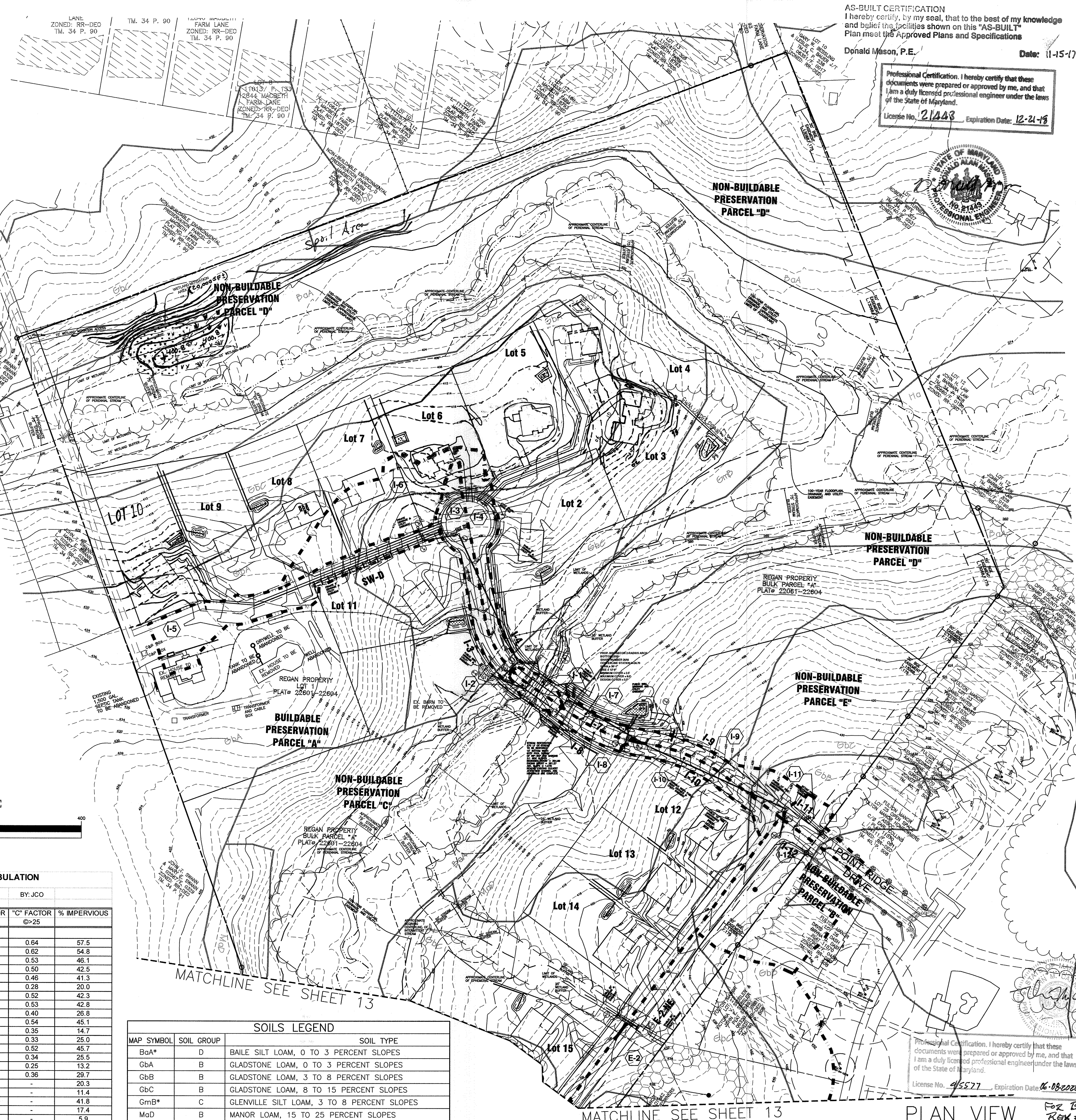
TITLE:
 FINAL ROAD CONSTRUCTION PLAN
 GRADING, SEDIMENT AND EROSION CONTROL PLAN AND SOILS MAP

DATE: MARCH, 2014
PROJECT NO.: 2171

DESIGN: JCO **DRAFT:** JCO **SCALE:** AS SHOWN **DRAWING 10 OF 33**

DRIVEWAY NOTE: PORTIONS OF THE DRIVEWAYS ON LOTS 5 & 6 ARE NOT TREATED ON-SITE. ANY AREA NOT DRAINING TO AN ON-SITE FACILITY OR DISCONNECTION MUST BE CONSTRUCTED AT THE MINIMUM ALLOWABLE WIDTH.

GRID NORTH

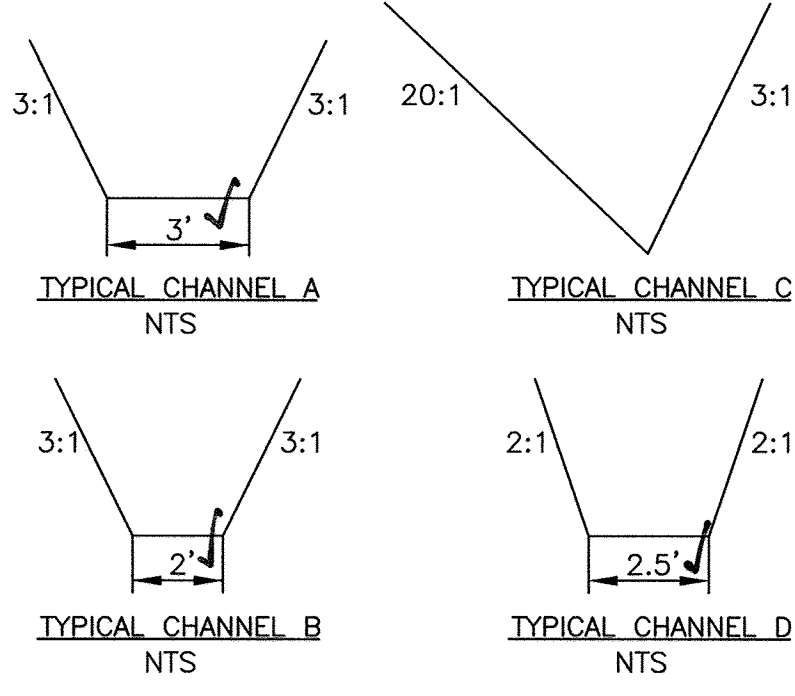


AS-BUILT CERTIFICATION
I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications.
Donald Mason, P.E.
Date: 11-15-17

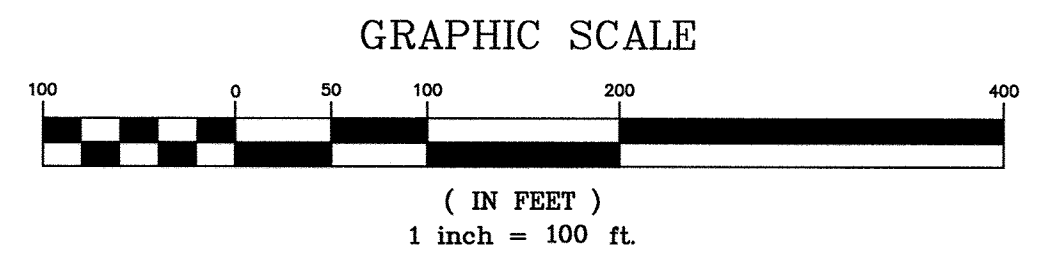
Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443 Expiration Date: 12-21-18



<p>1-3 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 1.0600 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.3301 ft Velocity: 0.8468 fps Full Flowrate: 16.0680 cfs Flow area: 1.3171 ft² Flow perimeter: 5.0878 ft Hydraulic radius: 0.2589 ft Top width: 4.9806 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 2.48176%</p>	<p>1-4 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 1.1300 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.3419 ft Velocity: 0.8208 fps Full Flowrate: 16.0680 cfs Flow area: 1.3707 ft² Flow perimeter: 5.1629 ft Hydraulic radius: 0.2666 ft Top width: 5.0519 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 25.7104%</p>	<p>1-5 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.2800 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1550 ft Velocity: 0.5213 fps Full Flowrate: 16.0680 cfs Flow area: 0.5371 ft² Flow perimeter: 3.9805 ft Hydraulic radius: 0.1349 ft Top width: 2.9302 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 11.6594%</p>	<p>1-6 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.2900 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1582 ft Velocity: 0.5276 fps Full Flowrate: 16.0680 cfs Flow area: 0.5497 ft² Flow perimeter: 4.0006 ft Hydraulic radius: 0.1374 ft Top width: 3.9493 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 11.8947%</p>	<p>1-7 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.7500 cfs Slope: 0.0900 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.2163 ft Velocity: 0.9500 fps Full Flowrate: 24.1020 cfs Flow area: 0.7895 ft² Flow perimeter: 4.3684 ft Hydraulic radius: 0.1807 ft Top width: 4.2982 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 16.2667%</p>	<p>1-8 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.2900 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1582 ft Velocity: 0.5276 fps Full Flowrate: 16.0680 cfs Flow area: 0.5497 ft² Flow perimeter: 4.0006 ft Hydraulic radius: 0.1374 ft Top width: 3.9493 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 11.8947%</p>	<p>1-9 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.7500 cfs Slope: 0.0900 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.2163 ft Velocity: 0.9500 fps Full Flowrate: 24.1020 cfs Flow area: 0.7895 ft² Flow perimeter: 4.3684 ft Hydraulic radius: 0.1807 ft Top width: 4.2982 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 16.2667%</p>	<p>1-10 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.2800 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1550 ft Velocity: 0.5213 fps Full Flowrate: 16.0680 cfs Flow area: 0.5371 ft² Flow perimeter: 3.9805 ft Hydraulic radius: 0.1349 ft Top width: 2.9302 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 11.6594%</p>	<p>1-11 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.2800 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1550 ft Velocity: 0.5213 fps Full Flowrate: 16.0680 cfs Flow area: 0.5371 ft² Flow perimeter: 3.9805 ft Hydraulic radius: 0.1349 ft Top width: 2.9302 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 11.6594%</p>	<p>1-12 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.2800 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1550 ft Velocity: 0.5213 fps Full Flowrate: 16.0680 cfs Flow area: 0.5371 ft² Flow perimeter: 3.9805 ft Hydraulic radius: 0.1349 ft Top width: 2.9302 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 11.6594%</p>	<p>1-13 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.2800 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1550 ft Velocity: 0.5213 fps Full Flowrate: 16.0680 cfs Flow area: 0.5371 ft² Flow perimeter: 3.9805 ft Hydraulic radius: 0.1349 ft Top width: 2.9302 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 11.6594%</p>	<p>1-14 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.2800 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1550 ft Velocity: 0.5213 fps Full Flowrate: 16.0680 cfs Flow area: 0.5371 ft² Flow perimeter: 3.9805 ft Hydraulic radius: 0.1349 ft Top width: 2.9302 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 11.6594%</p>	<p>1-15 Swale: CHANNEL A 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.2800 cfs Slope: 0.0400 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 3.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1550 ft Velocity: 0.5213 fps Full Flowrate: 16.0680 cfs Flow area: 0.5371 ft² Flow perimeter: 3.9805 ft Hydraulic radius: 0.1349 ft Top width: 2.9302 ft Area: 9.2972 ft² Perimeter: 11.4124 ft Percent full: 11.6594%</p>	<p>E-3 NE: CHANNEL B 10-year Storm Data: Shape: Trapezoidal Solving for: Depth of Flow Flowrate: 0.3300 cfs Slope: 0.0500 ft/ft Manning's n: 0.1500 Height: 1.3300 ft Bottom width: 2.0000 ft Left slope: 0.3333 ft (V/H) Right slope: 0.3333 ft (V/H) Results: Depth: 0.1972 ft Velocity: 0.6457 fps Full Flowrate: 14.7647 cfs Flow area: 0.5111 ft² Flow perimeter: 4.3475 ft Hydraulic radius: 0.1574 ft Top width: 3.1833 ft Area: 7.9672 ft² Perimeter: 10.4124 ft Percent full: 14.8266%</p>
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SWALE DETAILS
SEE SHEET 13 FOR SWALE COMPUTATIONS



AREA AND "C" FACTOR TABULATION						
PROJECT:	Regan Property	DATE:	11/27/2013	BY:	JCO	
INLET #	ZONING (Z)	SUBAREA (B)	AREA (Ac) (A)	"C" FACTOR (C)<25	"C" FACTOR (C)>25	% IMPERVIOUS
I-1	RR-DEO		0.06	0.56	0.64	57.5
I-2	RR-DEO		0.07	0.54	0.62	54.8
I-3	RR-DEO		0.35	0.46	0.53	46.1
I-4	RR-DEO		0.41	0.42	0.50	42.5
I-5	RR-DEO		0.75	0.43	0.46	41.3
I-6	RR-DEO		0.88	0.21	0.28	20.0
I-7	RR-DEO		0.09	0.46	0.52	42.3
I-8	RR-DEO		0.09	0.46	0.53	42.8
I-9	RR-DEO		0.33	0.35	0.40	26.8
I-10	RR-DEO		0.23	0.48	0.54	45.1
I-11	RR-DEO		0.22	0.28	0.35	14.7
I-12	RR-DEO		1.26	0.26	0.33	25.0
I-14	RR-DEO		0.56	0.45	0.52	45.7
E-2	RR-DEO		6.30	0.27	0.34	25.5
E-4	RR-DEO		7.41	0.19	0.25	13.2
E-6	RR-DEO		7.16	0.29	0.36	29.7
SW-D	RR-DEO		0.77	0.30	-	20.3
E-2 NE	RR-DEO		0.38	0.24	-	11.4
E-3 NE	RR-DEO		0.11	0.45	-	41.8
E-4 NE	RR-DEO		0.32	0.28	-	17.4
E-4 W	RR-DEO		1.16	0.17	-	5.9
SW-E	RR-DEO		0.08	0.54	-	54.1

SOILS LEGEND		
MAP SYMBOL	SOIL GROUP	SOIL TYPE
BaA*	D	BAILE SILT LOAM, 0 TO 3 PERCENT SLOPES
GbA	B	GLADSTONE LOAM, 0 TO 3 PERCENT SLOPES
GbB	B	GLADSTONE LOAM, 3 TO 8 PERCENT SLOPES
GbC	B	GLADSTONE LOAM, 8 TO 15 PERCENT SLOPES
GmB*	C	GLENVILLE SILT LOAM, 3 TO 8 PERCENT SLOPES
MaD	B	MANOR LOAM, 15 TO 25 PERCENT SLOPES

FROM NATURAL RESOURCES CONSERVATION SERVICES WEB SOIL SURVEY 2.0 * ERODIBLE SOILS

PLAN VIEW
SCALE: 1" = 100'

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 45577 Expiration Date: 06-08-2020

APPROVED: DEPARTMENT OF PUBLIC WORKS
4-29-14
CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
5-21-14
CHIEF, DIVISION OF LAND DEVELOPMENT

CHIEF, DEVELOPMENT ENGINEERING DIVISION

2-6-11-18 SHOW HOUSE GRADING FOR LOTS 5, 6 & 10
1-2-9-15 SHOW FINAL WETLAND MITIGATION AREA (20,000 SF PERMITS) & SPILL AREA

NO. DATE REVISION

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 28559, Expired 06/07/2013.

BENCHMARK ENGINEERING, INC.
8480 BALTIMORE NATIONAL PIKE & SUITE 315 & ELLICOTT CITY, MARYLAND 21043
(P) 410-465-6105 (F) 410-465-8644
75 THOMAS JOHNSON DRIVE & SUITE E AFFREDERICK, MARYLAND 21702
301-710-9686
WWW.BEI-CIVILENGINEERING.COM

OWNER/DEVELOPER:
RONALD R. REGAN
56B ORCHARD BEACH BLVD
PORT WASHINGTON, NY 11050

SCOTT T. REGAN
10509 TWIN CEDAR COURT
LAUREL, MARYLAND 20723

KELLY R. REGAN
12859 ROUTE 108
HIGHLAND, MARYLAND 20777
301.672.4820

DESIGN: JCO DRAFT: JCO

PROJECT: **REGAN PROPERTY**
LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL "A" and A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL "A" PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

LOCATION: TAX MAP NO. 34, GRID NO. 24, PARCEL NO. 200
HOWARD COUNTY, MARYLAND
DPZ NO.: SP-12-004, EOP-12-045, WP-13-025

TITLE: **FINAL ROAD CONSTRUCTION PLAN**
STORM DRAIN DRAINAGE AREA MAP

DATE: DECEMBER 2013 PROJECT NO. 2171

SCALE: AS SHOWN DRAWING 12 OF 33

AREA AND "C" FACTOR TABULATION						
PROJECT	Regan Property	DATE:	11/27/2013	BY:	JCO	
INLET #	ZONING (Z)	SUBAREA (B)	AREA (Ac) (A)	"C" FACTOR (C)-25	"C" FACTOR (C)-25	% IMPERVIOUS
I-1	RR-DEO		0.06	0.56	0.64	57.5
I-2	RR-DEO		0.07	0.54	0.62	54.8
I-3	RR-DEO		0.35	0.46	0.53	46.1
I-4	RR-DEO		0.41	0.42	0.50	42.5
I-5	RR-DEO		0.75	0.43	0.46	41.3
I-6	RR-DEO		0.88	0.21	0.28	20.0
I-7	RR-DEO		0.09	0.46	0.52	42.3
I-8	RR-DEO		0.09	0.46	0.53	42.8
I-9	RR-DEO		0.33	0.35	0.40	26.8
I-10	RR-DEO		0.23	0.48	0.54	45.1
I-11	RR-DEO		0.22	0.28	0.35	14.7
I-12	RR-DEO		1.29	0.26	0.33	25.0
I-14	RR-DEO		0.56	0.45	0.52	45.7
E-2	RR-DEO		6.30	0.27	0.34	25.5
E-4	RR-DEO		7.41	0.19	0.25	13.2
E-6	RR-DEO		7.16	0.29	0.35	29.7
SW-D	RR-DEO		0.77	0.30	-	20.3
E-2 NE	RR-DEO		0.38	0.24	-	11.4
E-3 NE	RR-DEO		0.11	0.45	-	41.8
E-4 NE	RR-DEO		0.32	0.28	-	17.4
E-4 W	RR-DEO		1.16	0.17	-	5.9
SW-E	RR-DEO		0.08	0.54	-	54.1

LEGEND

- SOILS CLASSIFICATION ChB2
- SOILS DELINEATION —————
- EXISTING CONTOURS (AERIAL 12/02) - - - - - 480
- - - - - 478
- PROPOSED CONTOURS ————— 999
- LIMIT OF WETLANDS - - - - -
- 25' WETLANDS BUFFER - - - - -
- CENTERLINE OF STREAM —————
- STREAM BUFFER —————
- EXISTING WOODS LINE ~~~~~
- PROPOSED WOODS LINE ~~~~~
- EXISTING STRUCTURE []
- PROPOSED STRUCTURE []
- SLOPES 15% TO 24.9% []
- SLOPES 25% OR GREATER []
- 100 YEAR FLOODPLAIN []
- PROPOSED FOREST CONSERVATION EASEMENT (RETENTION) []
- PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION) []
- TREE PROTECTION FENCE //
- PRIVATE SEWAGE DISPOSAL AREA []
- STORM DRAIN DRAINAGE DIVIDE []
- DRAINAGE AREA DESIGNATION (I-11)



SWALE COMPUTATIONS														
PROJECT: Regan Property		DATE: #####		DESIGN BY: am										
D.A. #	Channel Slope (%)	Q _s (cfs)	V _s (ft)	R (ft)	Sw (ft/ft)	Shear Stress	2-yr Stabilization Requirement	Q _s (cfs)	V _s (cfs)	R (ft)	Sw (ft/ft)	Shear Stress	10-yr Stabilization Requirement	Final Stabilization Requirement
		Bare Earth												
I-3	4.0	0.72	2.68	0.076	0.040	0.19	Temporary	1.06	0.805	0.259	0.04	0.65	None	Temporary
I-4	4.0	0.77	2.74	0.079	0.040	0.20	Temporary	1.13	0.821	0.267	0.04	0.67	None	Temporary
I-7	4.0	0.19	1.62	0.036	0.040	0.09	None	0.28	0.521	0.135	0.04	0.34	None	None
I-8	4.0	0.20	1.66	0.037	0.040	0.09	None	0.29	0.528	0.137	0.04	0.34	None	None
I-9 *	9.0	0.51	3.04	0.050	0.090	0.28	Temporary	0.75	0.950	0.181	0.09	1.01	None	Temporary
I-10 *	9.0	0.50	3.02	0.050	0.090	0.28	Temporary	0.73	0.942	0.178	0.09	1.00	None	Temporary
I-11	3.0	0.28	1.72	0.049	0.030	0.09	None	0.41	0.539	0.176	0.03	0.33	None	None
I-12	3.0	1.53	3.19	0.123	0.030	0.23	Temporary	2.25	0.911	0.387	0.03	0.72	None	Temporary
SW-D	8.0	1.05	4.13	0.087	0.080	0.43	Temporary	1.54	1.242	0.295	0.08	1.47	None	Temporary
E-2 NE *	5.0	0.41	2.51	0.059	0.050	0.18	Temporary	0.59	0.609	0.144	0.05	0.45	None	Temporary
E-3 NE *	5.0	0.22	2.01	0.045	0.050	0.14	None	0.33	0.646	0.157	0.05	0.49	None	None
E-4 NE *	5.0	0.41	2.64	0.064	0.050	0.20	Temporary	0.60	0.777	0.207	0.05	0.65	None	Temporary
E-4 W *	8.3	0.91	4.12	0.085	0.083	0.44	Temporary	1.34	1.170	0.264	0.08	1.37	None	Temporary
SW-E	10.0	0.20	2.53	0.035	0.100	0.22	Temporary	0.30	0.79	0.127	0.10	0.79	None	Temporary

Shear Stress Equation: $\tau = \gamma \cdot R \cdot S_w$ where:

τ = shear stress (lb/ft²)
 γ = weight density of water (62.4 lb/ft³)
 R = average water depth (hydraulic radius) (ft)
 S_w = water surface slope (ft/ft)

Note: The swale computation were prepared for sediment control matting determination. A bare earth Manning's coefficient for the 2-year storm was used. Since the Q-10 flows resulted in velocities under 1.0 fps in all stormwater treatment swales except SW-D the flows were not recomputed for the Pe.

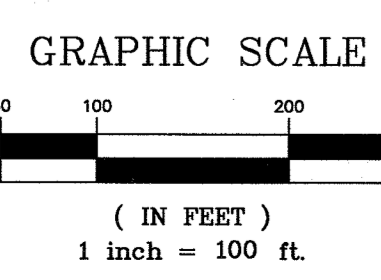
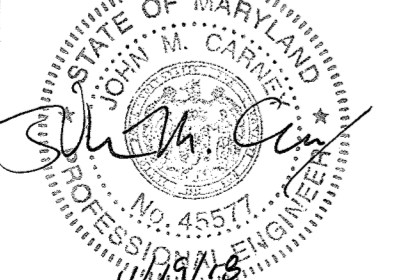
* SWALES NOT USED FOR SWM

JAMES H. EASTER
 CLARE EASTER, TRUSTEES
 L. 08403/F. 0527
 ZONED: RR-DEO
 TM. 40, P. 173

NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443, Expiration Date: 12-31-19

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 45577, Expiration Date: 06-02-20



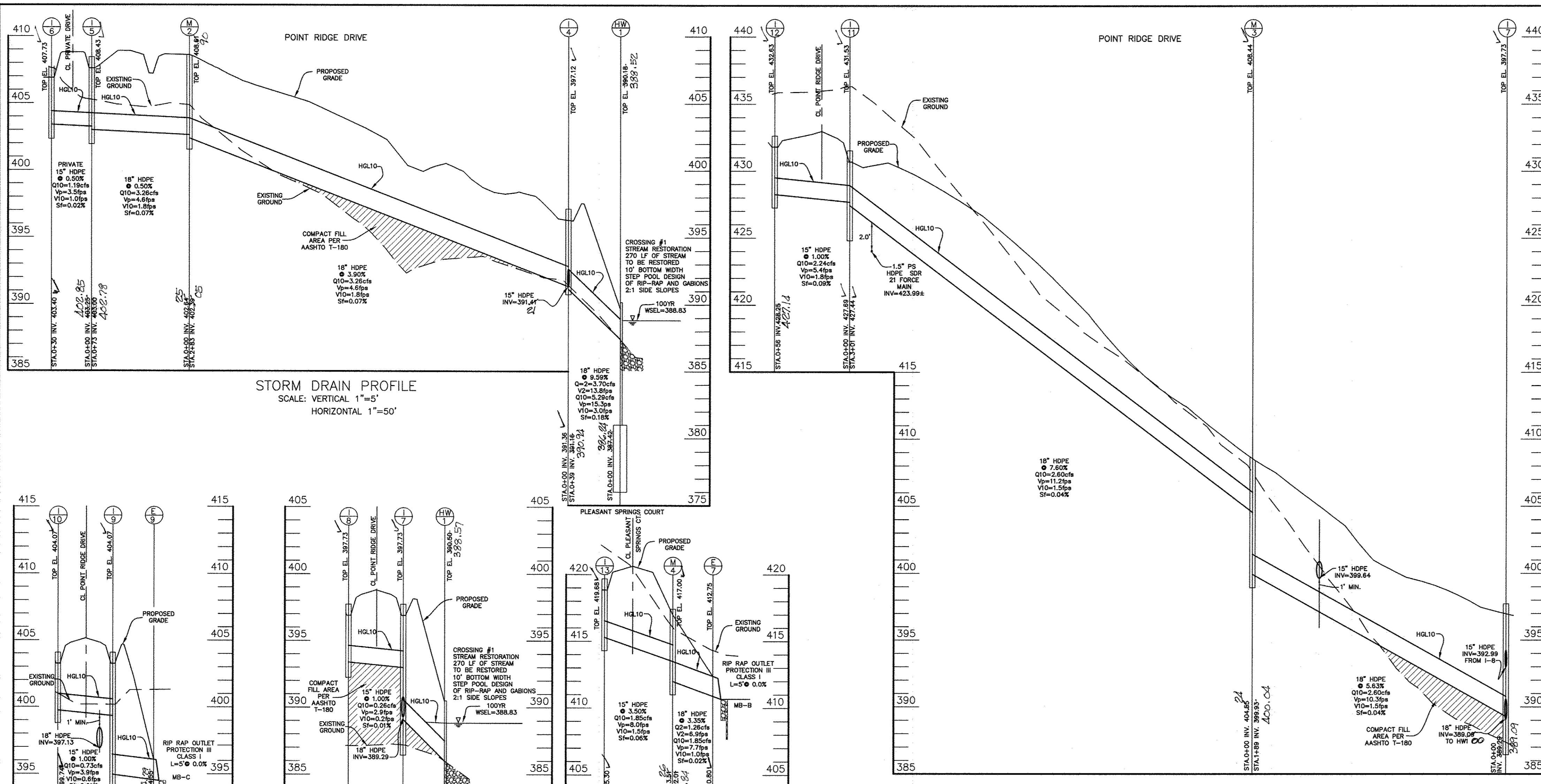
PLAN VIEW
 SCALE: 1" = 100'

4	11-12-14	REVISE LOTS 21 & 22	4-29-14
3	6-9-14	REVISE SWM LOT 17	5-31-14
APPROVED: DEPARTMENT OF PUBLIC WORKS			
[Signature]			4-29-14
CHIEF, BUREAU OF HIGHWAYS			DATE
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING			
[Signature]			5-31-14
CHIEF, DIVISION OF LAND DEVELOPMENT			DATE
[Signature]			5-9-14
CHIEF, DEVELOPMENT ENGINEERING DIVISION			DATE
2	1/16/19	REVISE LOT 16 FOR SPECIFIC HOUSE	
1	11-13-18	REVISE LOT 17 FOR SPECIFIC HOUSE	
NO.	DATE	REVISION	

BENCHMARK ENGINEERING, INC.
 ENGINEERS & LAND SURVEYORS & PLANNERS
 8480 BALTIMORE NATIONAL PIKE A SUITE 315 A ELLEWOOD CITY, MARYLAND 21043
 (P) 410-465-8108 (F) 410-465-8544
 75 THOMAS JOHNSON DRIVE A SUITE E ABERDEER, MARYLAND 21702
 301-710-5686
 WWW.BE-CVLENGINEERING.COM

Professional Certification: I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 28559 Expiration Date: 7-22-2015

OWNER/DEVELOPER: RONALD R. REGAN 56B ORCHARD BEACH BLVD PORT WASHINGTON, NY 11050		PROJECT: REGAN PROPERTY LOTS 2 THRU 23, NON-BUILDABLE PRESERVATION PARCEL 'A', and A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604	
SCOTT T. REGAN 10509 TWIN CEDAR COURT LAUREL, MARYLAND 20723		LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200 5TH ELECTION DISTRICT HOWARD COUNTY, MARYLAND DPZ NO.: SP-12-004, ECP-12-045, WP-13-025	
KELLY R. REGAN 12859 ROUTE 108 HIGHLAND, MARYLAND 20777 301.672.4820		TITLE: FINAL ROAD CONSTRUCTION PLAN STORM DRAIN DRAINAGE AREA MAP	
DESIGN: JCO	DRAFT: JCO	DATE: JANUARY 2014	PROJECT NO. 2171
SCALE: AS SHOWN		DRAWING 13 OF 33	

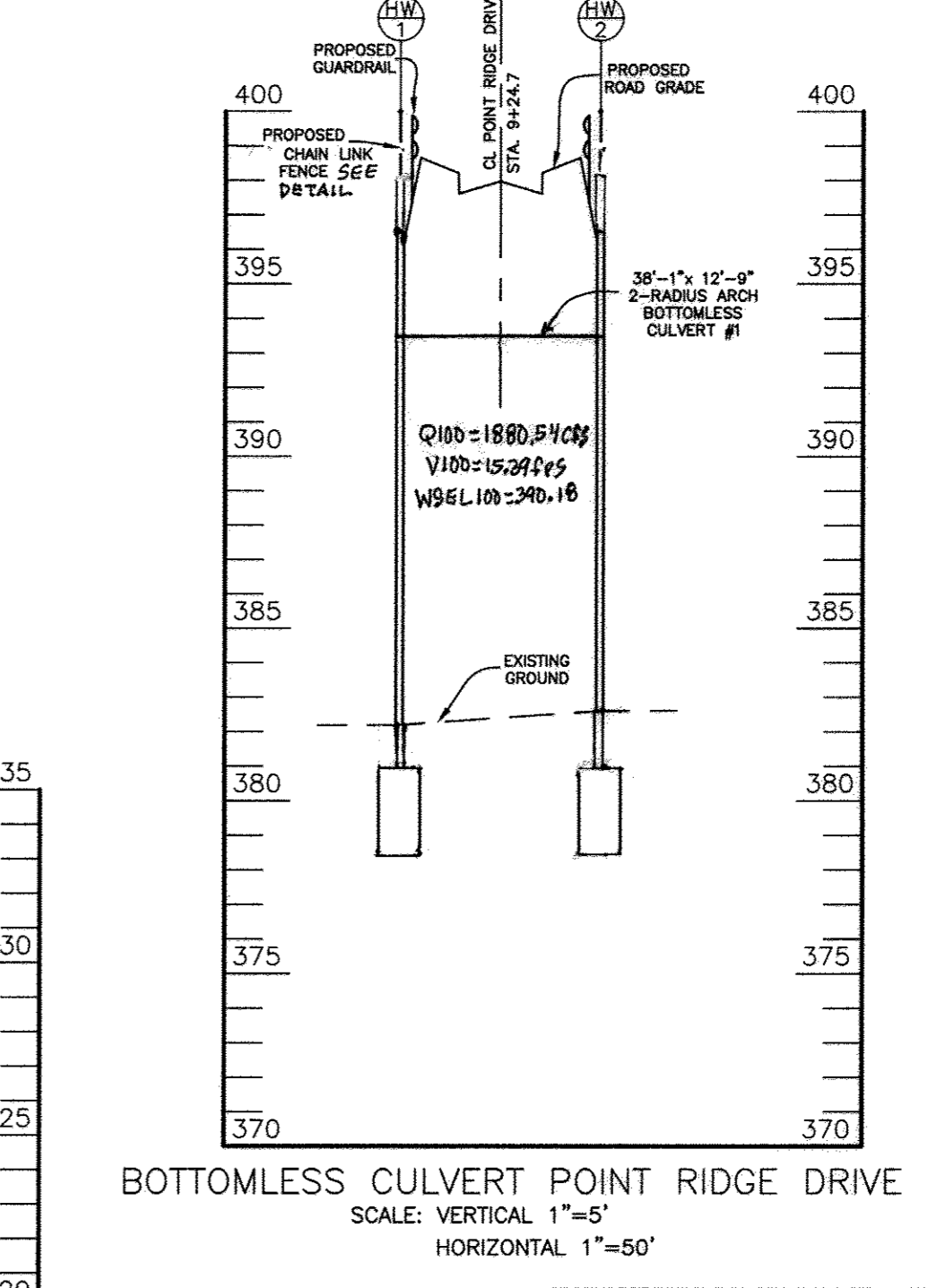
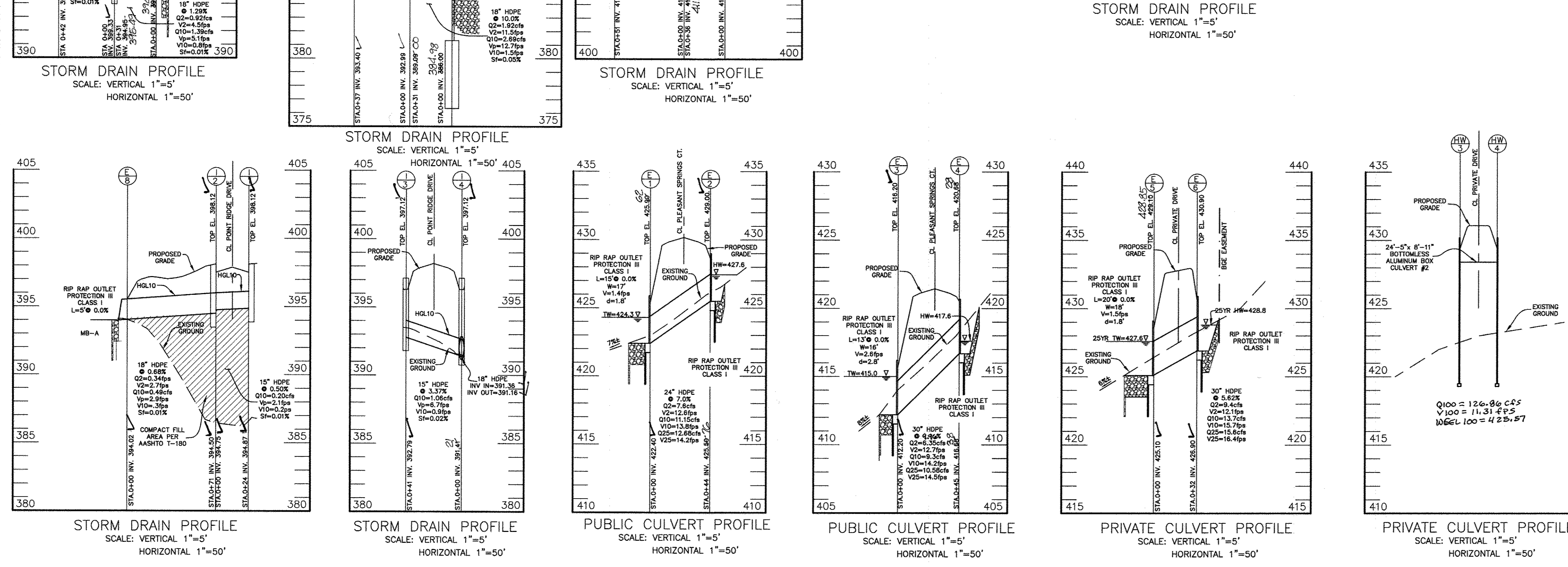


STRUCTURE SCHEDULE									
NO.	TYPE	LOCATION	INV. IN	INV. OUT	TOP ELEV.	THROAT ELEV.	OWNERSHIP	HO. CO. STD.	
END SECTIONS									
E-1	24" TYPE C HEADWALL	55299.6 1329058.9	-	422.40	425.00	-	-	D-5.21	
E-2	24" TYPE C HEADWALL	55296.5 1329098.8	-	425.50	429.00	-	-	D-5.21	
E-3	30" TYPE C HEADWALL	55261.3 1328771.3	-	412.20	416.20	-	-	D-5.21	
E-4	30" TYPE C HEADWALL	55258.1 1328813.1	-	416.00	421.00	-	-	D-5.21	
E-5	30" TYPE C HEADWALL	55235.1 1328591.9	-	425.10	429.10	-	-	D-5.21	
E-6	30" TYPE C HEADWALL	55230.6 1328607.9	-	426.90	430.90	-	-	D-5.21	
E-7	18" CONC. END SECTION	55257.9 1328703.5	-	410.80	412.30	-	-	D-5.51, D-5.52	
E-8	18" CONC. END SECTION	55368.8 1328807.8	-	394.02	395.52	-	-	D-5.51, D-5.52	
E-9	18" CONC. END SECTION	55358.1 1329118.1	-	398.55	400.05	-	-	D-5.51, D-5.52	
INLETS									
I-1	TYPE A-5	POINT RIDGE DRIVE 9+59.3 12'R	-	394.87	398.12	-	-	D-4.01	
I-2	TYPE A-5	POINT RIDGE DRIVE 9+59.3 12'L	394.75	394.50	398.12	-	-	D-4.01	
I-3	TYPE D	POINT RIDGE DRIVE 9+97.5 20'L	-	392.79	397.12	-	-	D-4.10	
I-4	TYPE D	POINT RIDGE DRIVE 9+98.1 21'R	391.16	391.16	397.12	-	-	D-4.10	
I-5	TYPE D	55393.8 13218668.5	403.25	403.00	408.43	-	-	D-4.10	
I-6	TYPE D	55396.5 1328657.5	-	403.40	407.73	-	-	D-4.10	
I-7	TYPE D	POINT RIDGE DRIVE 8+69.7 21'R	389.69	389.69	397.73	-	-	D-4.10	
I-8	TYPE D	POINT RIDGE DRIVE 8+69.7 21'L	393.40	393.40	397.73	-	-	D-4.10	
I-9	TYPE D	POINT RIDGE DRIVE 7+23.6 20.6'L	399.33	394.95	404.07	-	-	D-4.10	
I-10	TYPE D	POINT RIDGE DRIVE 7+23.6 20.6'R	399.74	399.74	404.07	-	-	D-4.10	
I-11	TYPE D	POINT RIDGE DRIVE 3+71.0 20.5'R	402.69	402.44	431.53	-	-	D-4.10	
I-12	TYPE D	PLEASANT SPRINGS CT. 0+32.6 22'L	402.11	402.28	432.63	-	-	D-4.10	
I-13	TYPE D	PLEASANT SPRINGS CT. 10+47.0 21'L	-	415.30	419.65	-	-	D-4.10	
MANHOLES									
M-2	4'-0" MANHOLE	12+88.4 23.1' L	402.64	402.29	408.50	-	-	G-4.12	
M-3	4'-0" MANHOLE	6+74.6 5.8' L	404.59	399.93	408.44	-	-	G-4.12	
M-4	4'-0" MANHOLE	10+47.0 30.1' R	413.51	412.01	417.00	-	-	G-4.12	

- STRUCTURE LOCATION FOR MANHOLES IS AT THE CENTER OF THE MANHOLE.
- STRUCTURE LOCATION FOR INLETS IS AT THE CENTER OF THE INLET TOP OF CURB.
- STRUCTURE ELEVATION AND LOCATION FOR END SECTIONS IS AT THE MIDDPOINT OF THE END OF PIPE BEGINNING OF END SECTION.
- PRECAST STRUCTURES MEETING HS-20 LOADING MAY BE USED.
- STRUCTURE LOCATION FOR CURB OR COMBINATION INLETS IS AT THE CENTER FACE OF CURB.
- STRUCTURE LOCATION FOR TYPE D INLETS IS AT THE CENTER TOP OF SLAB.
- OUTLET PROTECTION TO BE PER MDE RIP-RAP OUTLET PROTECTION III, D-4-1C (SEE DETAIL SHEET 11)

DRIVEWAY CULVERT SCHEDULE					
LOT	SIZE	TYPE	PIPE	IN	OUT
1	12"	HDPE	22LF@4%	408.4	407.7
10	12"	HDPE	22LF@3%	403.7	402.8
UIC 2-4	12"	HDPE	26LF@3%	409.5	408.9
UIC 6-9	12"	HDPE	26LF@4%	408.4	407.4

STORMDRAIN PIPE SCHEDULE				
PIPE SIZE	LENGTH	TYPE	TYPE	
15"	281'	HDPE	PUBLIC	
18"	1056'	HDPE	PUBLIC	
24"	44'	HDPE	PUBLIC	
30"	32'	HDPE	PRIVATE	
30"	45'	HDPE	PUBLIC	



NOTES:

- EXISTING GRADE TO BE OBTAINED IN SUCH A MANNER TO PROVIDE A SAFE WORK AREA.
- STORM DRAIN COVER SHALL BE PRECAST CONCRETE, AS RECOMMENDED BY PIPE MANUFACTURER, TO BE INSTALLED TO THE TOP OF THE COVER.
- ALL WORK CONNECTIONS SHALL BE CHECKED TO ALLOW ADEQUATE WORK AREA TO BE MAINTAINED THROUGHOUT THE PROJECT.
- IF THE PIPE MANUFACTURER'S RECOMMENDATIONS FOR COVER CONNECTIONS ARE NOT MET, THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ADEQUATE CONNECTIONS TO THE MANHOLE.
- CONNECTIONS TO BE MADE PER MDE SPECIFICATIONS.

DATE	4/10/2014
PROJECT	REGAN PROPERTY
DESIGNER	JCO
DRAWN	JCO

APPROVED: DEPARTMENT OF PUBLIC WORKS
 CHIEF, BUREAU OF HIGHWAYS
 DATE: 4-29-14

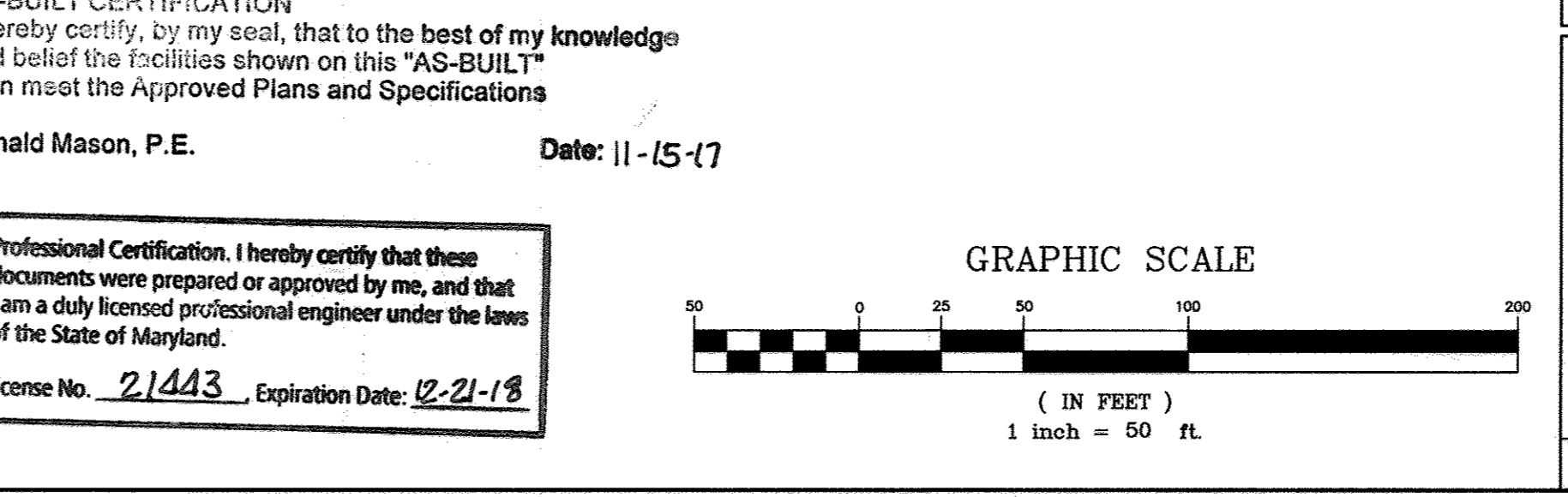
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 CHIEF, DIVISION OF LAND DEVELOPMENT
 DATE: 5-11-14

CHIEF, DEVELOPMENT ENGINEERING DIVISION
 DATE: 5-9-14

AS-BUILT CERTIFICATION
 I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications

Donald Mason, P.E.
 License No. 21443, Expiration Date: 12-21-18

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.



BENCHMARK ENGINEERING, INC.
 ENGINEERS & LAND SURVEYORS & PLANNERS
 8480 BALTIMORE NATIONAL PRICE & SUITE 315 & ELLICOTT CITY, MARYLAND 21043
 (P) 410-465-8105 (F) 410-465-8844
 75 THOMAS JOHNSON DRIVE & SUITE E & FREDERICK, MARYLAND 21702
 301-710-5888
 WWW.BE-CIVILENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 28559, Exp. 12/31/2015.

OWNER/DEVELOPER:
 RONALD R. REGAN
 56B ORCHARD BEACH BLVD
 PORT WASHINGTON, NY 11050

PROJECT: REGAN PROPERTY
 LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL "A", and NON-BUILDABLE PRESERVATION PARCELS "B" thru "E" A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL "A" PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

LOCATION:
 TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 5th ELECTION DISTRICT
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE: FINAL ROAD CONSTRUCTION PLAN
 STORM DRAIN PROFILES

DATE: MARCH, 2014 PROJECT NO. 2171
 SCALE: AS SHOWN DRAWING 14 OF 33

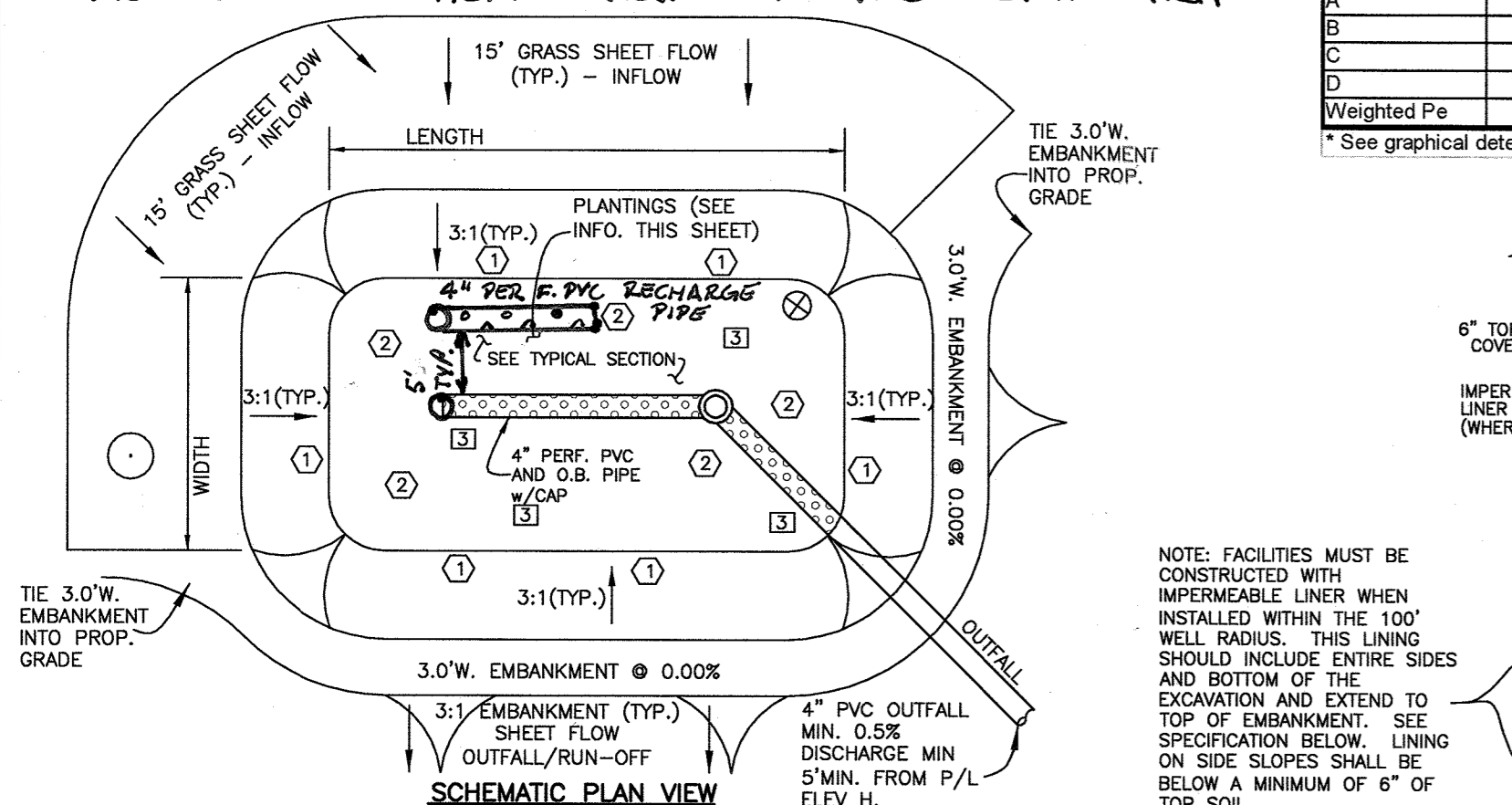
DESIGN: JCO DRAFT: JCO

Lot No.	Drainage Area	Imperious (%)	Rv (%)	ESDv (cf)	Notes	Drainage Area	IMP	PE	ESDv
2	11590	37.00	32%	0.34		1291	572	1.0	43
3	6021	44.61	82%	0.77		2406	1192	1.0	89
4	10164	45.65	46%	0.46		4257	1928	1.0	145
5	6153	85.57	52%	0.52		3145	1221	1.0	92
6	4498	33.51	76%	0.73		1843	842	1.0	63
7	4528	38.50	85%	0.81					
8	12052	52.58	44%	0.44					
9	11444	5174	45%	0.45					
10	4740	3992	62%	0.61					
11	19283	5215	27%	0.29					
12	8812	5342	61%	0.59					
13	8263	5350	65%	0.63					
14	8371	5392	64%	0.63					
15	8520	5725	67%	0.65					
16	8307	5057	61%	0.59					
17	6576	3855	59%	0.58					
18	3606	3104	55%	0.55					
19	9425	5212	56%	0.58					
20	5102	3860	76%	0.73					
21	7830	4754	63%	0.62					
22	10827	4934	43%	0.41					
23	5738	3850	66%	0.64					
MB-A	6195	3040	49%	0.49					
MB-B	18672	7486	40%	0.41					
MB-C	32822	9187	28%	0.30					
SW-D	33666	6849	20%	0.23					

*Micro-Bioretenation facilities within the 100' well radius must be provided with an impermeable liner.

FACILITY	MICRO-BIORETENTION GEOMETRY										PLANTING SCHEDULE			
	LENGTH*	WIDTH*	DEPTH	A	B	C	D	E	F	G	H	(1)	(2)	(3)
MB-1	30.0	10.0	0.75	364.00	393.25	393.00	391.00	390.67	390.17	389.67	389.00	18	14	14
MB-2	27.0	17.0	1.00	588.00	587.00	588.75	594.75	594.75	594.75	594.75	594.75	83	83	92
MB-3	27.0	18.0	1.00	470.00	468.00	465.41	467.75	465.41	467.75	465.41	467.75	84	84	92
MB-4	27.0	18.0	1.00	416.50	416.50	416.50	416.50	416.50	416.50	416.50	416.50	10	14	14
MB-5	21.0	14.0	1.00	412.00	412.00	412.00	412.00	412.00	412.00	412.00	412.00	13	9	9
MB-6	17.0	9.0	1.00	417.00	416.00	415.75	413.75	413.42	412.92	412.42	413.00	14	11	11
MB-7	28.0	9.5	1.00	420.00	419.00	418.75	418.75	418.42	417.92	417.42	418.00	21	16	16
MB-8	22.0	11.0	1.00	423.00	422.00	421.75	419.75	419.42	418.92	418.42	419.00	20	15	15
MB-9	27.0	11.0	1.00	422.00	421.00	420.75	418.75	418.42	417.92	417.42	418.00	20	15	15
MB-10	21.0	13.0	0.75	369.00	368.00	367.75	365.75	365.42	364.92	364.42	365.00	20	14	14
MB-11	27.0	9.0	1.00	422.00	421.00	420.75	418.75	418.42	417.92	417.42	418.00	20	15	15
MB-12	28.0	9.0	1.00	421.00	420.00	419.75	417.75	417.42	416.92	416.42	417.00	20	15	15
MB-13	27.0	9.0	1.00	422.00	421.00	420.75	418.75	418.42	417.92	417.42	418.00	20	15	15
MB-14	28.0	9.0	1.00	418.00	417.00	416.75	414.75	414.42	413.92	413.42	414.00	21	16	16
MB-15	22.5	10.5	1.00	410.00	409.00	408.75	406.75	406.42	405.92	405.42	406.00	20	19	19
MB-16	45.0	10.0	1.00	416.00	415.00	414.75	412.75	412.42	411.92	411.42	412.00	40	40	20
MB-17	19.6	13.6	1.00	427.00	426.00	425.75	423.75	423.42	422.92	422.42	423.00	51	51	90
MB-18	24.0	10.0	1.00	430.00	429.00	428.75	426.75	426.42	425.92	425.42	426.00	20	15	15
MB-19	35.0	11.0	1.00	425.00	424.00	423.75	421.75	421.42	420.92	420.42	421.00	47	42	23
MB-20	44.0	17.0	1.00	439.00	438.00	437.75	435.75	435.42	434.92	434.42	435.00	61	61	61
MB-21	44.0	17.0	1.00	428.00	427.00	426.75	424.75	424.42	423.92	423.42	424.00	61	61	61
MB-22	20.0	7.5	1.00	428.00	427.00	426.75	424.75	424.42	423.92	423.42	424.00	15	11	11

* THESE ARE LOT MICRO-BIO FACILITIES ARE BEING AS-BUILT AS PART OF THE HOME CONSTRUCTION GRADE CERTIFICATION

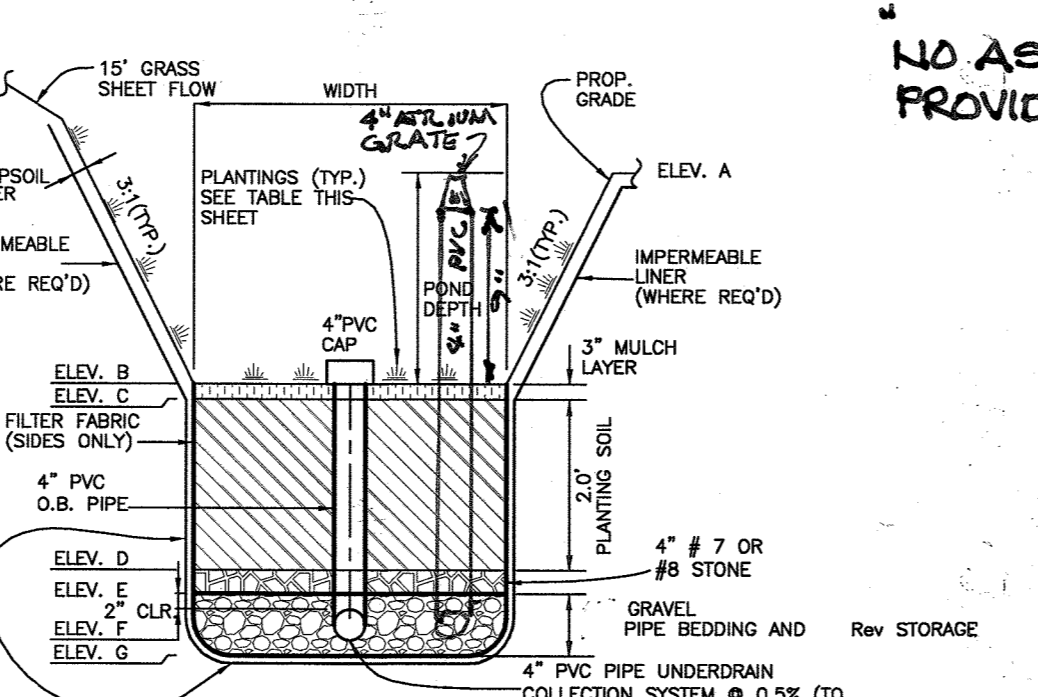


MATERIAL	SPECIFICATION	SIZE	NOTES:
PLANTINGS	SEE APPENDIX A; TABLE A.4	N/A	PLANTINGS ARE SITE SPECIFIC
PLANTING SOIL (2.0' TO 4.0' DEEP)	LOAMY SAND 80-85% COMPOST; 35-40% SANDY LOAM 30% COARSE SAND 30% COMPOST 40%	N/A	USDA SOIL TYPES: LOAMY SAND OR SANDY LOAM; CLAY CONTENT <5%
ORGANIC CONTENT	MIN. 10% BY DRY WEIGHT ASTM D 2974		
MULCH	SHREDDED HARDWOOD	N/A	AGED 6 MONTHS, MINIMUM, NO PINE OR WOOD CHIPS
GEOTEXTILE (CLASS 'C')		N/A	PE TYPE 1 NONWOVEN
GEOTEXTILE (1/4" WIRE MESH)		1/4" WIRE MESH	
UNDERDRAIN GRAVEL	ASHTO M-43	NO. 57 OR NO. 8 0.375" TO 0.750"	
UNDERDRAIN PIPING	F758, TYPE PS28 OR ASHTO M-278	4" TO 6" RIGID SCH 40 GRVEL OVER PIPES, NOT NECESSARY UNDERNEATH PIPES. PIPES SHALL BE WRAPPED WITH 1/4"-INCH GALVANIZED HARDWARE CLOTH	
IMPERVIOUS LINER	ASTM-D-4833 (THICKNESS) ASTM-D-412 (TENSILE STRENGTH 1,100 LB., ELONGATION 200%) ASTM-D-624 (TEAR RESISTANCE - 150 LB./IN) ASTM-D-471 (WATER ADSORPTION: +8 TO -2% MASS)	30 MIL THICK	LINER TO BE ULTRAVIOLET RESISTANT. A GEOTEXTILE FABRIC SHOULD BE USED TO PROTECT THE LINER FROM PUNCTURE.
GEOTEXTILE (BELOW IMPERV. LINER)	ASTM-D-4833 (PUNCTURE STRENGTH 125LB.) ASTM-D-4632 (TENSILE STRENGTH 300 LB.)		

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS
 DATE: 4-29-14
 APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 DATE: 5-21-14
 APPROVED: CHIEF, DIVISION OF LAND DEVELOPMENT
 DATE: 5-9-14

DEPTH	DESCRIPTION OF MATERIALS	TEMPERATURE	REMARKS
0.0	Soil with rock (organic) matter and organic soil		Boring dry during drilling and at completion.
0.0-1.0	Brown to reddish-brown clayey SILT with some fine to medium sand and little rock fragments, moist (ML)		Bedrock was not encountered in this boring.
1.0-2.0	Brown to orange-brown fine to medium sandy SILT with some to little rock fragments, moist (ML)		
2.0-3.0	Light brown to brown micaceous silty fine to medium SAND with little rock fragments, moist (SM)		
3.0-4.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
4.0-5.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
5.0-6.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
6.0-7.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
7.0-8.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
8.0-9.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
9.0-10.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
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79.0-80.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
80.0-81.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
81.0-82.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
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85.0-86.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
86.0-87.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
87.0-88.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
88.0-89.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
89.0-90.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
90.0-91.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
91.0-92.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
92.0-93.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
93.0-94.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
94.0-95.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
95.0-96.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
96.0-97.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
97.0-98.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
98.0-99.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		
99.0-100.0	Dark brown micaceous fine to medium sandy SILT, damp, (ML)		

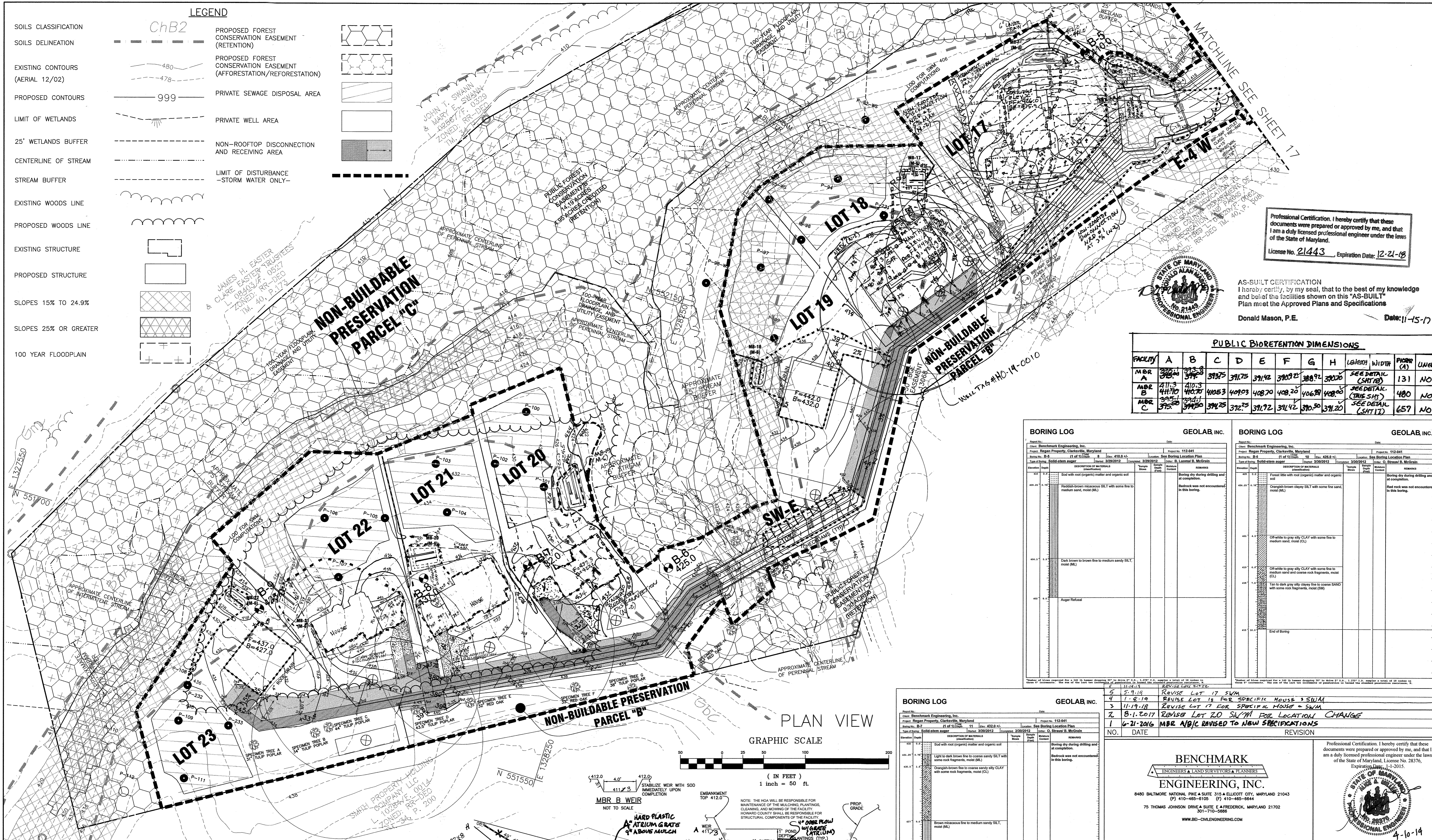
REGAN PROPERTY		Project Number
Determine ESD Implementation Goals		2171
Soil Conditions for "Woods in Good Condition"		
RSC	RCN	Area (ft ²)
A	B	55
B	C	1,347,856
C	D	70
D	E	77
E	F	16,548
F	G	55
G	H	1386270
H	I	212928
I	J	15
J	K	11
K	L	11



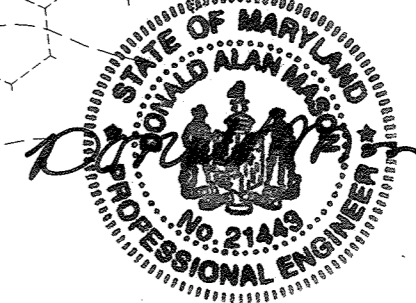
MATERIAL	SPECIFICATION	SIZE	NOTES:
PLANTINGS	SEE APPENDIX A; TABLE A.4	N/A	PLANTINGS ARE SITE SPECIFIC
PLANTING SOIL (2.0' TO 4.0' DEEP)	LOAMY SAND 80-85% COMPOST; 35-40% SANDY LOAM 30% COARSE SAND 30% COMPOST 40%	N/A	USDA SOIL TYPES: LOAMY SAND OR SANDY LOAM; CLAY CONTENT <5%
ORGANIC CONTENT	MIN. 10% BY DRY WEIGHT ASTM D 2974		
MULCH	SHREDDED HARDWOOD	N/A	AGED 6 MONTHS, MINIMUM, NO PINE OR WOOD CHIPS
GEOTEXTILE (CLASS 'C')		N/A	PE TYPE 1 NON

LEGEND

- SOILS CLASSIFICATION **ChB2**
- SOILS DELINEATION
- EXISTING CONTOURS (AERIAL 12/02)
- PROPOSED CONTOURS
- LIMIT OF WETLANDS
- 25' WETLANDS BUFFER
- CENTERLINE OF STREAM
- STREAM BUFFER
- EXISTING WOODS LINE
- PROPOSED WOODS LINE
- EXISTING STRUCTURE
- PROPOSED STRUCTURE
- SLOPES 15% TO 24.9%
- SLOPES 25% OR GREATER
- 100 YEAR FLOODPLAIN
- PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)
- PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)
- PRIVATE SEWAGE DISPOSAL AREA
- PRIVATE WELL AREA
- NON-ROOFTOP DISCONNECTION AND RECEIVING AREA
- LIMIT OF DISTURBANCE -STORM WATER ONLY-



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443, Expiration Date: 12-21-18



AS-BUILT CERTIFICATION
 I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications
 Donald Mason, P.E. Date: 11-15-17

PUBLIC BIORETENTION DIMENSIONS

FACILITY	A	B	C	D	E	F	G	H	LENGTH	WIDTH	FLOOR (A)	USER
MBR A	395.1	393.8	393.75	391.75	391.92	390.92	388.92	390.70	SEE DETAIL (SHEET B)	131	NO	NO
MBR B	411.3	410.3	410.63	409.03	408.70	408.20	406.59	408.00	SEE DETAIL (THIS SHEET)	480	NO	NO
MBR C	395.1	394.1	394.25	392.25	391.92	391.42	390.50	391.20	SEE DETAIL (SHEET D)	657	NO	NO

BORING LOG GEOLAB, INC.

Elevation	Depth	Description of Materials	Remarks
418.0	0.0	Soil with root (organic) matter and organic soil	Boring dry during drilling and at completion.
418.0	1.0	Reddish-brown micaceous SILT with some fine to medium sand, moist (ML)	Bedrock was not encountered in this boring.
418.0	1.5	Orange-brown clayey SILT with some fine sand, moist (ML)	Bedrock was not encountered in this boring.
418.0	2.0	Off-white to gray silty CLAY with some fine to medium sand and coarse rock fragments, moist (CL)	Bedrock was not encountered in this boring.
418.0	2.5	Off-white to gray silty CLAY with some fine to medium sand and coarse rock fragments, moist (CL)	Bedrock was not encountered in this boring.
418.0	3.0	Tan to dark gray silty clayey fine to coarse SAND with some rock fragments, moist (SM)	Bedrock was not encountered in this boring.
418.0	3.5	Aggr Refusal	Bedrock was not encountered in this boring.
418.0	4.0	End of Boring	

BORING LOG GEOLAB, INC.

Elevation	Depth	Description of Materials	Remarks
418.0	0.0	Soil with root (organic) matter and organic soil	Boring dry during drilling and at completion.
418.0	0.5	Light to dark brown fine to coarse sandy SILT with some rock fragments, moist (ML)	Bedrock was not encountered in this boring.
418.0	1.0	Orange-brown fine to coarse sandy CLAY with some rock fragments, moist (CL)	Bedrock was not encountered in this boring.
418.0	1.5	Brown micaceous fine to medium sandy SILT, moist (ML)	Bedrock was not encountered in this boring.
418.0	2.0	End of Boring	

BENCHMARK ENGINEERING, INC.
 8480 BALTIMORE NATIONAL PIKE SUITE 315 & ELLICOTT CITY, MARYLAND 21043
 (P) 410-465-6105 (F) 410-465-6644
 75 THOMAS JOHNSON DRIVE SUITE E & FREDERICK, MARYLAND 21702
 301-710-5686
 WWW.BE-CIVILENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 28376, Expiration Date: 1-1-2015.

OWNER/DEVELOPER:
 RONALD R. REGAN
 56B ORCHARD BEACH BLVD
 LAUREL, MARYLAND 20723
 11050

PROJECT: **REGAN PROPERTY**
 LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 1ST ELECTION DISTRICT
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE: **STORMWATER MANAGEMENT PLANS**

DATE: JANUARY 2014 PROJECT NO. 2171
 SCALE: AS SHOWN DRAWING .18. OF .33.

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS
 [Signature] 4-29-14
 CHIEF, BUREAU OF HIGHWAYS

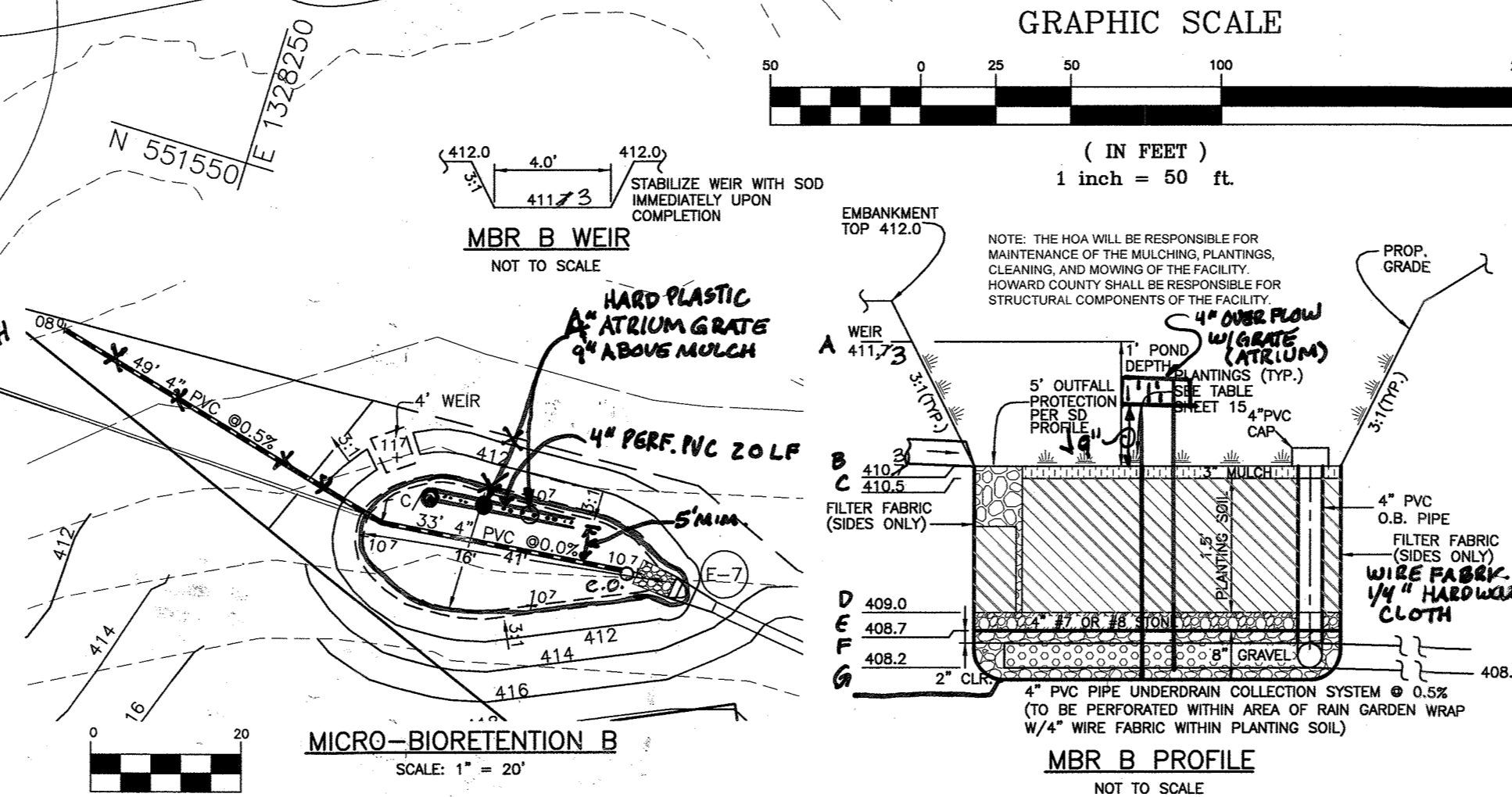
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 [Signature] 5-21-14
 CHIEF, DIVISION OF LAND DEVELOPMENT

[Signature] 5-9-14
 CHIEF, DEVELOPMENT ENGINEERING DIVISION

SOILS LEGEND

MAP SYMBOL	SOIL GROUP	SOIL TYPE
Ba*	D	BAILIE SILT LOAM, 0 TO 3 PERCENT SLOPES
GbA	B	GLADSTONE LOAM, 0 TO 3 PERCENT SLOPES
GbB	B	GLADSTONE LOAM, 3 TO 8 PERCENT SLOPES
GbC	B	GLADSTONE LOAM, 8 TO 15 PERCENT SLOPES
GmB*	C	GLENVILLE SILT LOAM, 3 TO 8 PERCENT SLOPES
MoD	B	MANOR LOAM, 15 TO 25 PERCENT SLOPES

FROM NATURAL RESOURCES CONSERVATION SERVICES WEB SOIL SURVEY 2.0 * ERODIBLE SOILS



LEGEND

SOILS CLASSIFICATION: ChB2

SOILS DELINEATION: [Symbol]

EXISTING CONTOURS (AERIAL 12/02): [Symbol]

PROPOSED CONTOURS: [Symbol]

LIMIT OF WETLANDS: [Symbol]

25' WETLANDS BUFFER: [Symbol]

CENTERLINE OF STREAM: [Symbol]

STREAM BUFFER: [Symbol]

EXISTING WOODS LINE: [Symbol]

PROPOSED WOODS LINE: [Symbol]

EXISTING STRUCTURE: [Symbol]

PROPOSED STRUCTURE: [Symbol]

SLOPES 15% TO 24.9%: [Symbol]

SLOPES 25% OR GREATER: [Symbol]

100 YEAR FLOODPLAIN: [Symbol]

PROPOSED FOREST CONSERVATION EASEMENT (RETENTION): [Symbol]

PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION): [Symbol]

PRIVATE SEWAGE DISPOSAL AREA: [Symbol]

PRIVATE WELL AREA: [Symbol]

NON-ROOFTOP DISCONNECTION AND RECEIVING AREA: [Symbol]

LIMIT OF DISTURBANCE - STORM WATER ONLY: [Symbol]

BORING LOG GEOLAB, INC.

Client: Benchmark Engineering, Inc. Project No: 112-041
 Project: Regan Property, Clarksville, Maryland
 Date: 11/11/14 Rev: 423.0-41
 Location: See Boring Location Plan
 Date of Boring: 3/29/2012
 Log No: 3/29/2012
 Log Title: D. Straub & McGrain

Elevation	Depth	Description of Materials	Remarks
423.2	0-1.0	Sod with root (organic) matter and organic soil	Boring dry during drilling and at completion.
422.28	0-1.0	Reddish-brown micaceous SILT with some fine to medium sand, root (ML)	Bedrock was not encountered in this boring.
417.2	5.0	Dark brown to brown fine to medium sandy SILT, root (ML)	
417	5.1	End of Boring	

Notes: 1. All borings were drilled to a depth of 10 feet below the proposed foundation level. 2. The soil was found to be consistent with the proposed foundation level.

BORING LOG GEOLAB, INC.

Client: Benchmark Engineering, Inc. Project No: 112-041
 Project: Regan Property, Clarksville, Maryland
 Date: 11/11/14 Rev: 393.0-41
 Location: See Boring Location Plan
 Date of Boring: 3/29/2012
 Log No: 3/29/2012
 Log Title: B. Leonard & McGrain

Elevation	Depth	Description of Materials	Remarks
393.2	0-1.0	Sod with root (organic) matter and organic soil	Boring dry during drilling and at completion.
390.20	0-1.0	Brown micaceous fine to medium sandy SILT with little rock fragments, root (ML)	Bedrock was not encountered in this boring.
388.2	4.0	Light orange, gray and tan elastic SILT with little fine sand, root (ML)	Large diameter rock fragments encountered.
387	4.2	Asper Refusal	

Notes: 1. All borings were drilled to a depth of 10 feet below the proposed foundation level. 2. The soil was found to be consistent with the proposed foundation level.

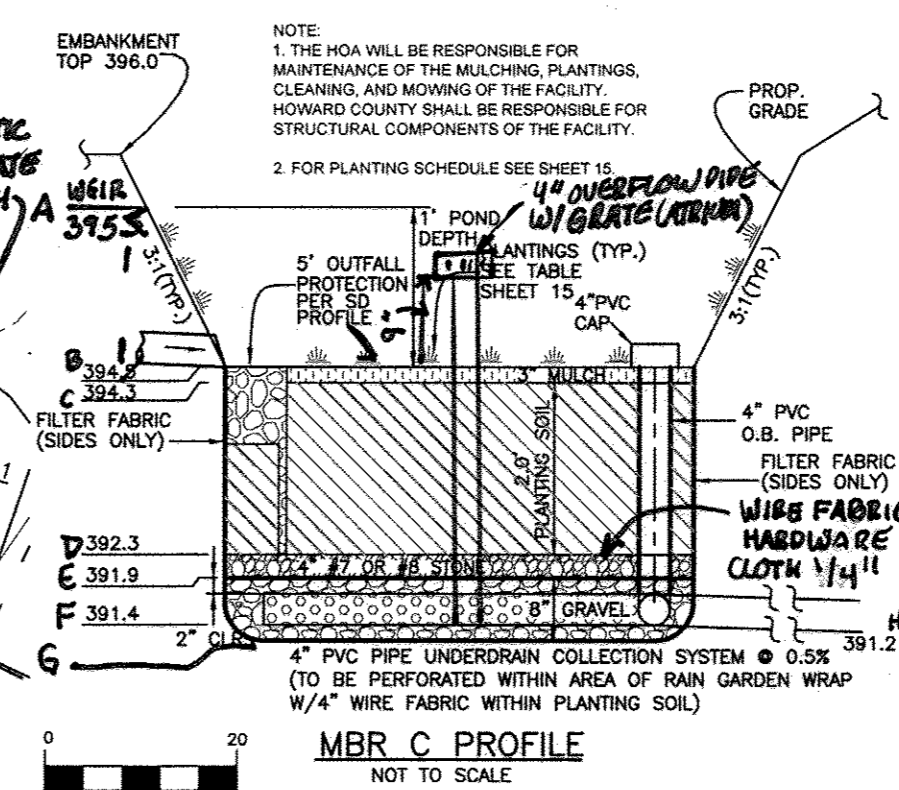
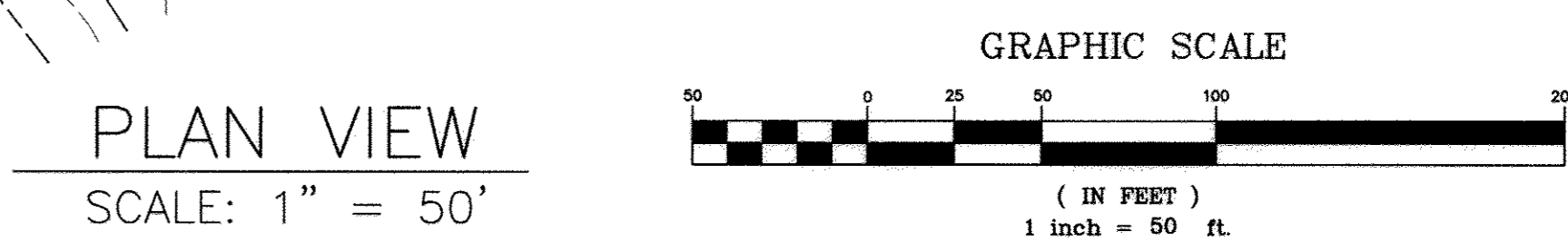
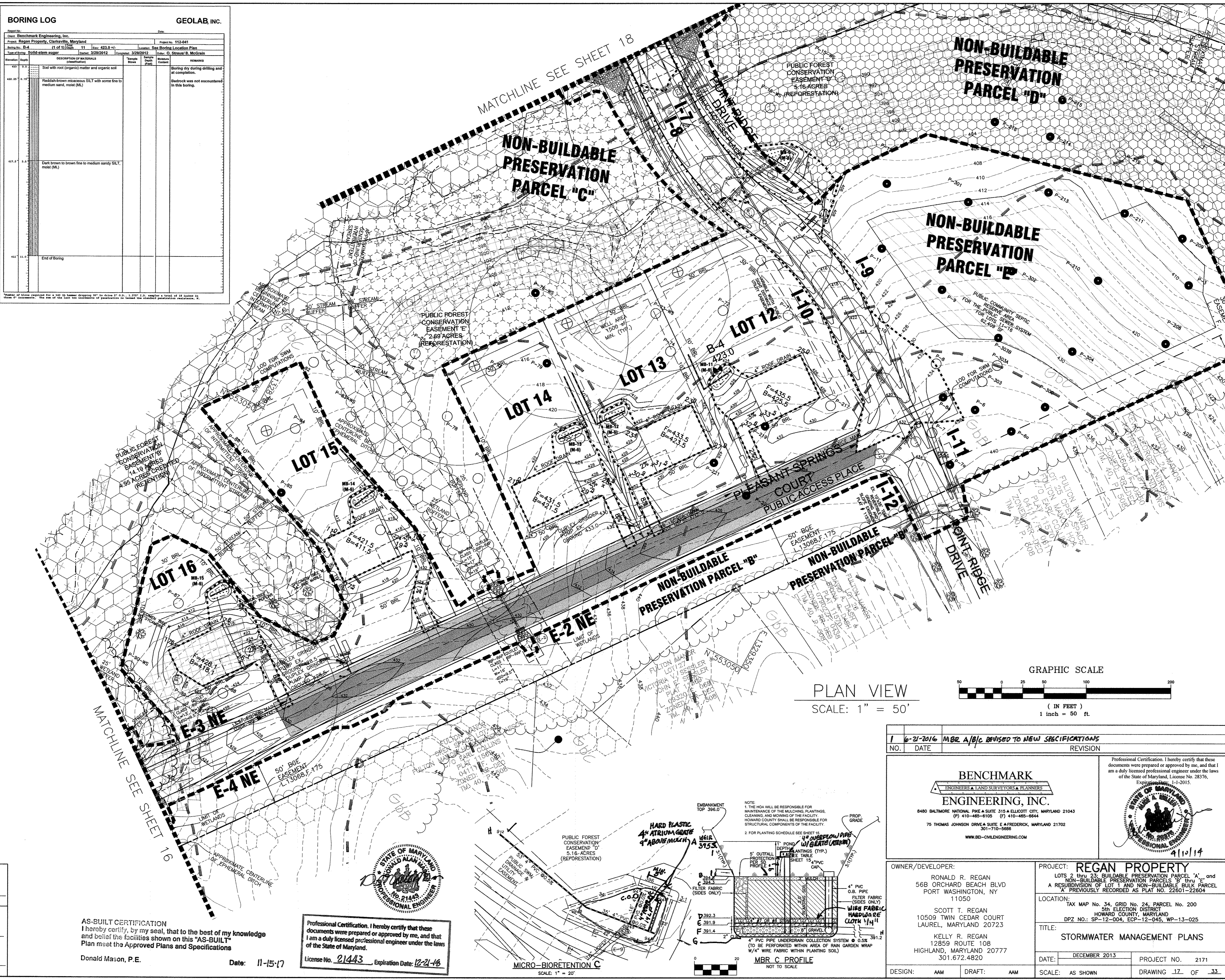
APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS
 [Signature] 4-29-14
 CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 [Signature] 5-21-14
 CHIEF, DIVISION OF LAND DEVELOPMENT

[Signature] 5-9-14
 CHIEF, DEVELOPMENT ENGINEERING DIVISION

AS-BUILT CERTIFICATION
 I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications
 Donald Mason, P.E. Date: 11-15-17

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443 Expiration Date: 12-21-16



NO.	DATE	REVISION
1	6-21-2016	MBR A/B/C REVISED TO NEW SPECIFICATIONS

BENCHMARK ENGINEERING, INC.
 8480 BALTIMORE NATIONAL PIKE SUITE 315 A ELICOTT CITY, MARYLAND 21043
 (P) 410-465-6105 (F) 410-465-6644
 75 THOMAS JOHNSON DRIVE SUITE E FREDERICK, MARYLAND 21702
 WWW.BE-CIVILENGINEERING.COM

OWNER/DEVELOPER:
 RONALD R. REGAN
 56B ORCHARD BEACH BLVD
 PORT WASHINGTON, NY 11050

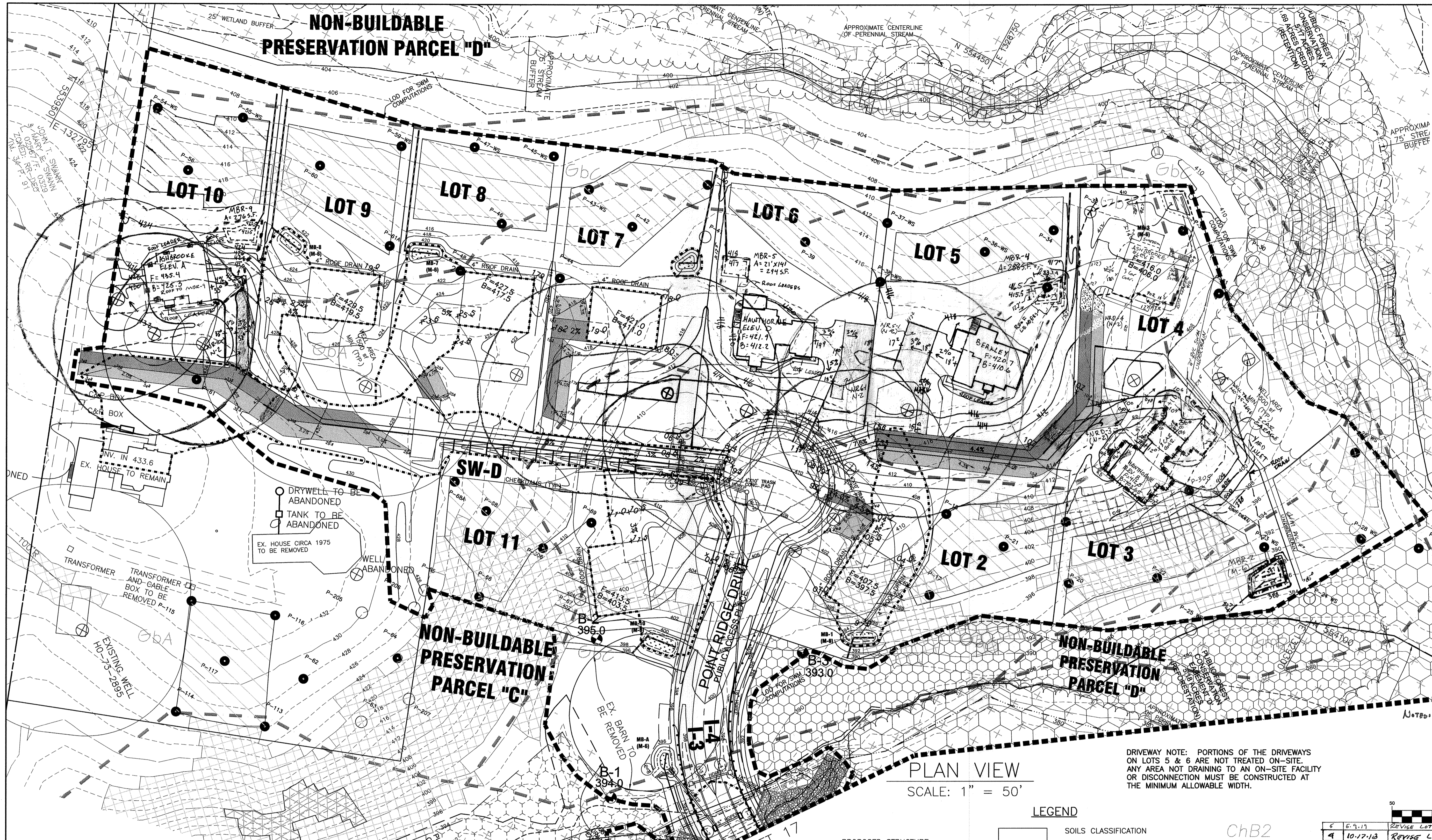
PROJECT: REGAN PROPERTY
 LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E'. A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULKY PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

LOCATION:
 TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 4th ELECTION DISTRICT
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
 STORMWATER MANAGEMENT PLANS

DATE: DECEMBER 2013 **PROJECT NO.:** 2171

DESIGN: AAM **DRAFT:** AAM **SCALE:** AS SHOWN **DRAWING 17 OF 33**

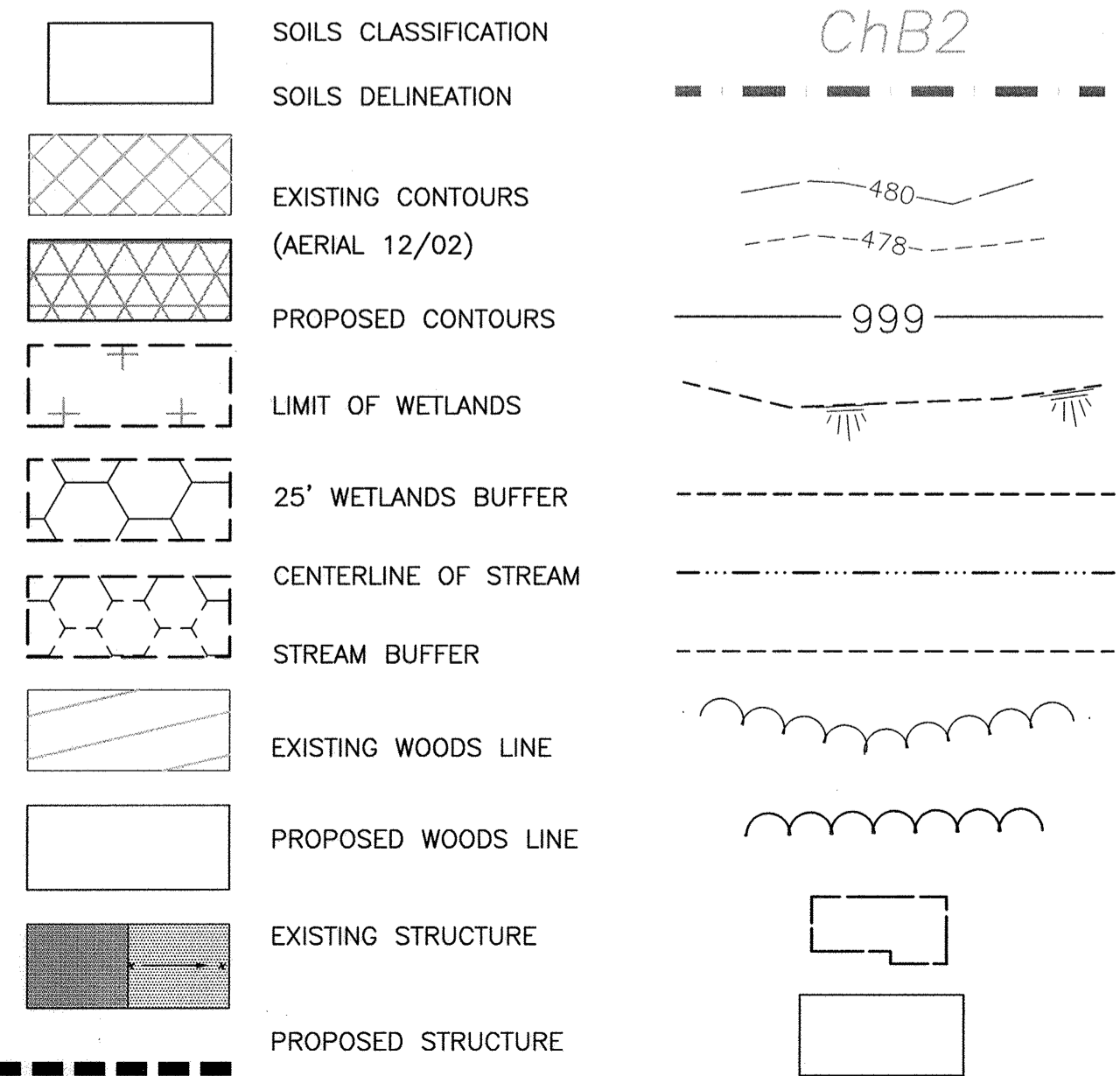


PLAN VIEW
SCALE: 1" = 50'

DRIVEWAY NOTE: PORTIONS OF THE DRIVEWAYS ON LOTS 5 & 6 ARE NOT TREATED ON-SITE. ANY AREA NOT DRAINING TO AN ON-SITE FACILITY OR DISCONNECTION MUST BE CONSTRUCTED AT THE MINIMUM ALLOWABLE WIDTH.

LEGEND

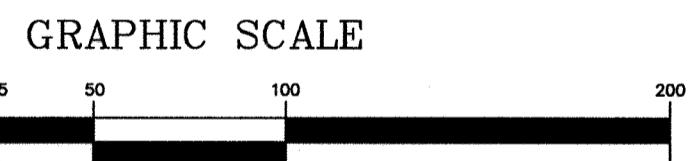
- PROPOSED STRUCTURE
- SLOPES 15% TO 24.9%
- SLOPES 25% OR GREATER
- 100 YEAR FLOODPLAIN
- PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)
- PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)
- PRIVATE SEWAGE DISPOSAL AREA
- PRIVATE WELL AREA
- NON-ROOFTOP DISCONNECTION AND RECEIVING AREA
- LIMIT OF DISTURBANCE - STORM WATER ONLY -



BORING LOG		GEOLAB, INC.	
Client: Benchmark Engineering, Inc. Project No: 112-041			
Project: Regan Property, Clarksville, Maryland			
Location: See Boring Location Plan			
Date: 3/29/2012			
Drawn by: L. L. McGrain			
Checked by: L. L. McGrain			
Elevation	Depth	Description of Materials	Remarks
395.0	0.0	Soil with root (organic) matter and organic soil	Groundwater encountered at a depth of 8.0 feet at completion.
393.0	2.0	Clay with gravelly fine to coarse SAND with little sil. clump (SM)	After 24 hours, the water was encountered at a depth of 7.5 feet.
391.0	4.0	Brown micaceous fine to medium sandy SILT with little gravel, moist (ML)	Bedrock was not encountered in this boring.
389.0	6.0	Dark brown and gray silty CLAY with little fine sand and little to trace gravel, moist to saturated (CL)	
387.0	8.0	End of Boring	

BORING LOG		GEOLAB, INC.	
Client: Benchmark Engineering, Inc. Project No: 112-041			
Project: Regan Property, Clarksville, Maryland			
Location: See Boring Location Plan			
Date: 3/29/2012			
Drawn by: L. L. McGrain			
Checked by: L. L. McGrain			
Elevation	Depth	Description of Materials	Remarks
395.0	0.0	Soil with root (organic) matter and organic soil	Groundwater encountered at a depth of 8.0 feet during drilling.
393.0	2.0	Brown micaceous fine to medium sandy SILT with trace to little rock fragments, moist to saturated (ML)	At completion, cave-in depth of 3.5 feet.
391.0	4.0	End of Boring	Bedrock was not encountered in this boring.

NOTES:
 1) ROOF LEADERS ARE TO BE 4" PVC TO THE FIRST FLOW JUNCTION AND THEN 6" PVC TO THE MBR.
 2) FOR WELL AND SEPTIC INFORMATION SEE THE MOST RECENT PERCOLATION CERTIFICATION PLAN.



NO.	DATE	REVISION
1	6-21-2016	MBR A/B/C REVISED TO NEW SPECIFICATIONS
2	5-25-18	REVISE LOTS 5, 6, 10
3	7/12/19	REVISE LOT 3
4	10/17/18	REVISE LOT 3
5	6-9-17	REVISE LOT 4

BENCHMARK ENGINEERING, INC.
 ENGINEERS & LAND SURVEYORS & PLANNERS
 8480 BALTIMORE NATIONAL PIKE & SUITE 315 A ELLICOTT CITY, MARYLAND 21043
 (P) 410-465-6105 (F) 410-465-6644
 75 THOMAS JOHNSON DRIVE & SUITE E A FREDERICK, MARYLAND 21702
 WWW.BEI-CVLENGINEERING.COM

OWNER/DEVELOPER:
 RONALD R. REGAN
 56B ORCHARD BEACH BLVD
 PORT WASHINGTON, NY 11050

PROJECT: **REGAN PROPERTY**
 LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

LOCATION:
 TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 5th ELECTION DISTRICT
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
STORMWATER MANAGEMENT PLANS

DATE: DECEMBER 2013 PROJECT NO. 2171
 SCALE: AS SHOWN DRAWING NO. 18 OF 33

AS-BUILT CERTIFICATION
 I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications

Donald Mason, P.E. Date: 11-15-17

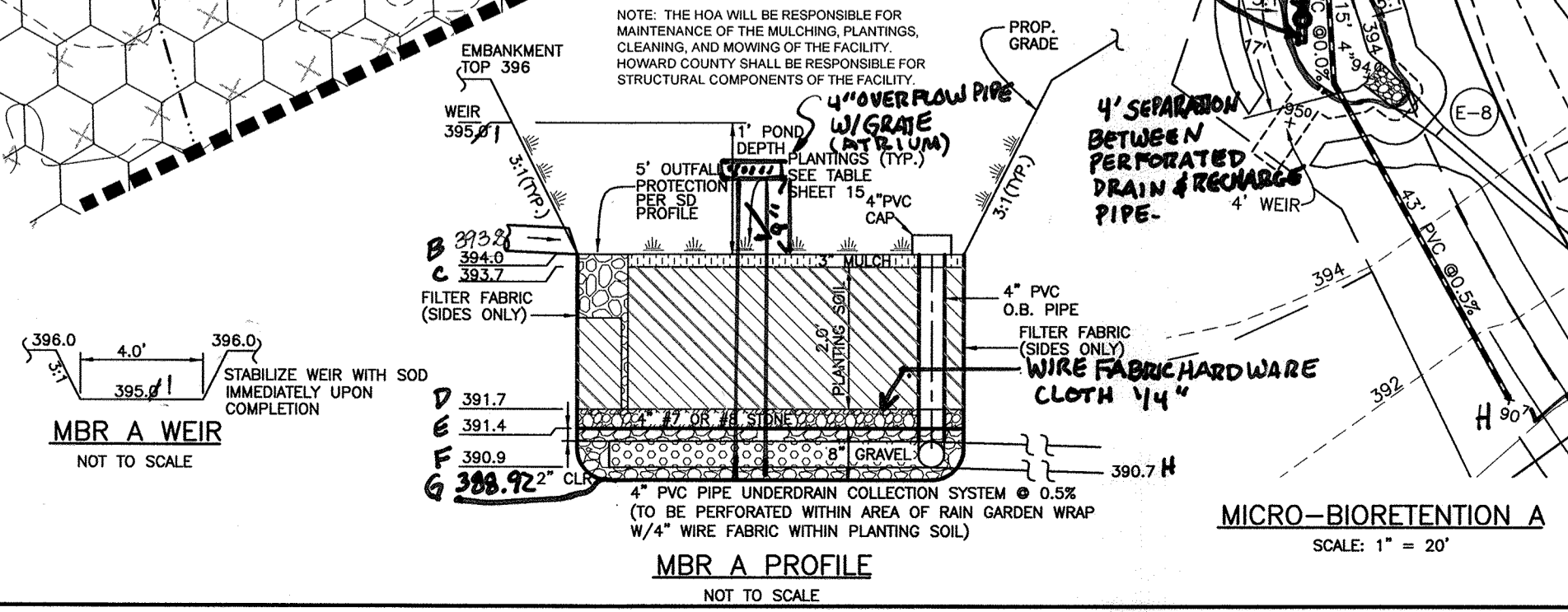
Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443 Expiration Date: 12-31-18



APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS
 [Signature] 4-23-14
 CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 [Signature] 5-24-14
 CHIEF, DIVISION OF LAND DEVELOPMENT

[Signature] 5-9-14
 CHIEF, DEVELOPMENT ENGINEERING DIVISION



Key	Species, Size (dbh)	Comments *
A	Tulip poplar, 31"	
B	Tulip poplar, 34"	Fair condition, poor trunk
C	Tulip poplar, 44"	Good condition, twin stems
D	Tulip poplar, 43.5"	Twin stems
E	Red oak, 32"	Fair, limb dieback
F	Red oak, 33"	
G	Tulip poplar, 38"	Poor condition, trunk rot
H	Red oak, 32"	
I	Tulip poplar, 32"	
J	Tulip poplar, 35"	
K	Sycamore, 31"	
L	Tulip poplar, 38"	
M	Red maple, 34"	

ALL SPECIMEN TREES ARE TO REMAIN. * Good unless otherwise noted.

LANDSCAPE NOTES:

- THIS PLAN HAS BEEN PREPARED IN ACCORDANCE WITH THE PROVISIONS OF SECTION 16.124 OF THE HOWARD COUNTY CODE AND THE HOWARD COUNTY LANDSCAPE MANUAL.
- STREET TREE LOCATIONS:**
 - WHEN THE DISTANCE BETWEEN THE CURB AND SIDEWALK IS 6 FEET OR GREATER, THE TREES SHALL BE LOCATED WITHIN THE RIGHT-OF-WAY AND SHALL BE CENTERED BETWEEN THE CURB AND SIDEWALK.
 - WHEN THE DISTANCE BETWEEN THE CURB AND SIDEWALK IS LESS THAN 6 FEET, TREES MAY BE PLANTED 3 FEET FROM THE SIDEWALK IN THE DIRECTION AWAY FROM THE ROAD. A 10-FOOT WIDE TREE MAINTENANCE EASEMENT SHALL BE REQUIRED IF THE RIGHT-OF-WAY IS LIMITED.
 - TREES SHALL BE PLANTED 6 FEET BEHIND CURB WHEN THERE ARE NO SIDEWALKS.
 - TREES TO BE PLANTED MINIMUM 30 FEET FROM SIGNS AND INTERSECTIONS WHEN PLANTED BETWEEN SIDEWALK AND CURB. TREES MAY NOT BE PLANTED WITHIN 5 FEET OF A STORM DRAIN INLET, OPEN SPACE ACCESS STRIP, OR 10 FEET OF A DRIVEWAY.
- AT THE TIME OF INSTALLMENT, ALL SHRUBS AND OTHER PLANTINGS HEREWITH LISTED AND APPROVED FOR THIS SITE, SHALL BE OF THE PROPER HEIGHT REQUIREMENTS IN ACCORDANCE WITH THE HOWARD COUNTY LANDSCAPE MANUAL. IN ADDITION, NO SUBSTITUTIONS OR RELOCATION OF REQUIRED PLANTINGS MAY BE MADE WITHOUT PRIOR REVIEW AND APPROVAL FROM THE DEPARTMENT OF PLANNING AND ZONING. ANY DEVIATION FROM THIS APPROVED LANDSCAPE PLAN MAY RESULT IN DENIAL OR DELAY IN RELEASE OF LANDSCAPE SURETY UNTIL SUCH TIME AS ALL REQUIRED MATERIALS ARE PLANTED AND/OR REVISIONS ARE MADE TO APPLICABLE PLANS AND CERTIFICATIONS.
- THE OWNER, TENANTS AND/OR THEIR AGENTS SHALL BE RESPONSIBLE FOR MAINTENANCE OF THE REQUIRED LANDSCAPING INCLUDING BOTH PLANT MATERIALS AND BERMS, FENCES AND WALLS. ALL PLANT MATERIALS SHALL BE MAINTAINED IN GOOD GROWING CONDITION, AND WHEN NECESSARY, REPLACED WITH NEW MATERIALS TO ENSURE CONTINUED COMPLIANCE WITH APPLICABLE REGULATIONS. ALL OTHER REQUIRED LANDSCAPING SHALL BE PERMANENTLY MAINTAINED IN GOOD CONDITION, AND WHEN NECESSARY, REPAIRED OR REPLACED.
- FINANCIAL SURETY IN THE AMOUNT OF \$33,000.00 FOR THE REQUIRED STREET TREES AND \$11,400.00 FOR THE REQUIRED PERIMETER LANDSCAPING SHALL BE POSTED AS PART OF THE DPW DEVELOPER'S AGREEMENT.

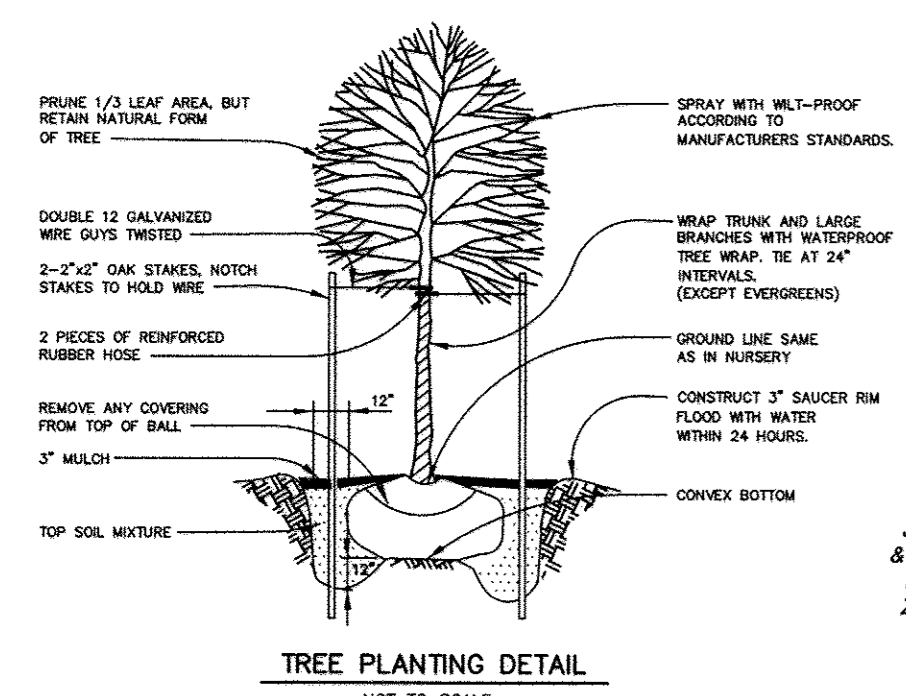
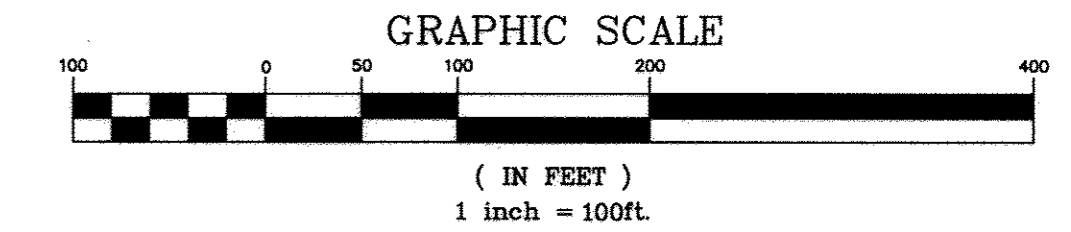
CATEGORY	SCHEDULE A PERIMETER LANDSCAPE EDGE						TOTALS
	ADJACENT TO PERIM. PROPERTY ①	ADJACENT TO PERIM. PROPERTY ②	ADJACENT TO PERIM. PROPERTY ③	ADJACENT TO PERIM. PROPERTY ④	ADJACENT TO PERIM. PROPERTY ⑤	ADJACENT TO ROAD (LOT 12) ⑥	
LANDSCAPE TYPE	A	A	A	A	A	B	
LINEAR FEET OF ROADWAY FRONTAGE/PERIMETER	696 LF	1432 LF	2406 LF	1550 LF	3302 LF	257 LF	
CREDIT FOR EXISTING VEGETATION (YES, NO, LINEAR FEET) (DESCRIBE BELOW IF NEEDED)	YES* 696 LF	YES* 1432 LF	YES* 2406 LF	YES* 928 LF	YES 1464 LF**	NO 725 LF**	
CREDIT FOR WALL, FENCE OR BERM (YES, NO, LINEAR FEET) (DESCRIBE BELOW IF NEEDED)	NO	NO	NO	NO	NO	NO	
NUMBER OF PLANTS REQUIRED	0	0	0	622 LF	1113 LF	257 LF	35
SHADE TREES	0	0	0	11	19	5	35
EVERGREEN TREES	0	0	0	0	0	0	6
OTHER TREES (2:1 SUBSTITUTE)	0	0	0	0	0	0	0
SHRUBS	0	0	0	0	0	0	0
NUMBER OF PLANTS PROVIDED	0	0	0	11	19	5	35
SHADE TREES	0	0	0	0	0	0	6
EVERGREEN TREES	0	0	0	0	0	0	6
OTHER TREES (2:1 SUBSTITUTE)	0	0	0	0	0	0	0
SHRUBS (10:1 SUBSTITUTE)	0	0	0	0	0	0	0

* CREDIT TAKEN IS FOR ON-SITE FOREST CONSERVATION ** CREDIT TAKEN IS FOR EXISTING TREES NOT IN FCE EASEMENT (RETENTION)

SYMBOL	QUANTITY	NAME	REMARKS	DESCRIPTION
	35	FAGUS GRANDIFOLIA (American Beech)	2.5" - 3" cal.	SHADE TREES ALONG PERIMETER EDGES TO BE PROVIDED BY THE DEVELOPER
	6	ILEX OPACA (American Holly)	5' - 6' ht.	EVERGREEN TREES ALONG PERIMETER EDGES TO BE PROVIDED BY THE DEVELOPER

LINEAR FEET OF RIGHT-OF-WAY	POINT RIDGE DRIVE	PLEASANT SPRINGS CT	TOTAL
LINEAR FEET OF CREDIT	2109	2287	
LINEAR FEET OF REQUIRED PLANTING	0	0	
TREE SIZE	LARGE 1:40 LF	LARGE 1:40 LF	
TREES REQUIRED	53	57	110

SYMBOL	QUANTITY	NAME	REMARKS	DESCRIPTION
	53	TILIA CORDATA (Greenspire Littleleaf Linden)	2.5" - 3" cal.	TO BE PLANTED ALONG POINT RIDGE DRIVE (PROVIDED BY THE DEVELOPER)
	30	ACER RUBRUM (Red Sunset Red Maple)	2.5" - 3" cal.	TO BE PLANTED ALONG PLEASANT SPRINGS COURT (PROVIDED BY THE DEVELOPER)
	27	QUERCUS COCCINEA (Scarlet Oak)	2.5" - 3" cal.	TO BE PLANTED ALONG PLEASANT SPRINGS COURT (PROVIDED BY THE DEVELOPER)



DEVELOPER'S/BUILDER'S CERTIFICATE

I/WE CERTIFY THAT THE LANDSCAPING SHOWN ON THIS PLAN WILL BE DONE ACCORDING TO THE PLAN, SECTION 16.124 OF THE HOWARD COUNTY SUBDIVISION AND LAND DEVELOPMENT REGULATIONS AND LANDSCAPE MANUAL. I/WE FURTHER CERTIFY THAT UPON COMPLETION OF A LETTER OF LANDSCAPE INSTALLATION, ACCOMPANIED BY AN EXECUTED ONE-YEAR GUARANTEE OF PLANT MATERIALS, WILL BE SUBMITTED TO THE DEPARTMENT OF PLANNING AND ZONING.

Matthew J. Mitchell 4/3/14
DEVELOPER DATE

APPROVED: DEPARTMENT OF PUBLIC WORKS
W. R. ... 4-29-14
 CHIEF, BUREAU OF HIGHWAYS DATE

APPROVED: DEPARTMENT OF PLANNING AND ZONING
K. ... 5-2-14
 CHIEF, DIVISION OF LAND DEVELOPMENT DATE

Chad ... 5-9-14
 CHIEF, DEVELOPMENT ENGINEERING DIVISION DATE



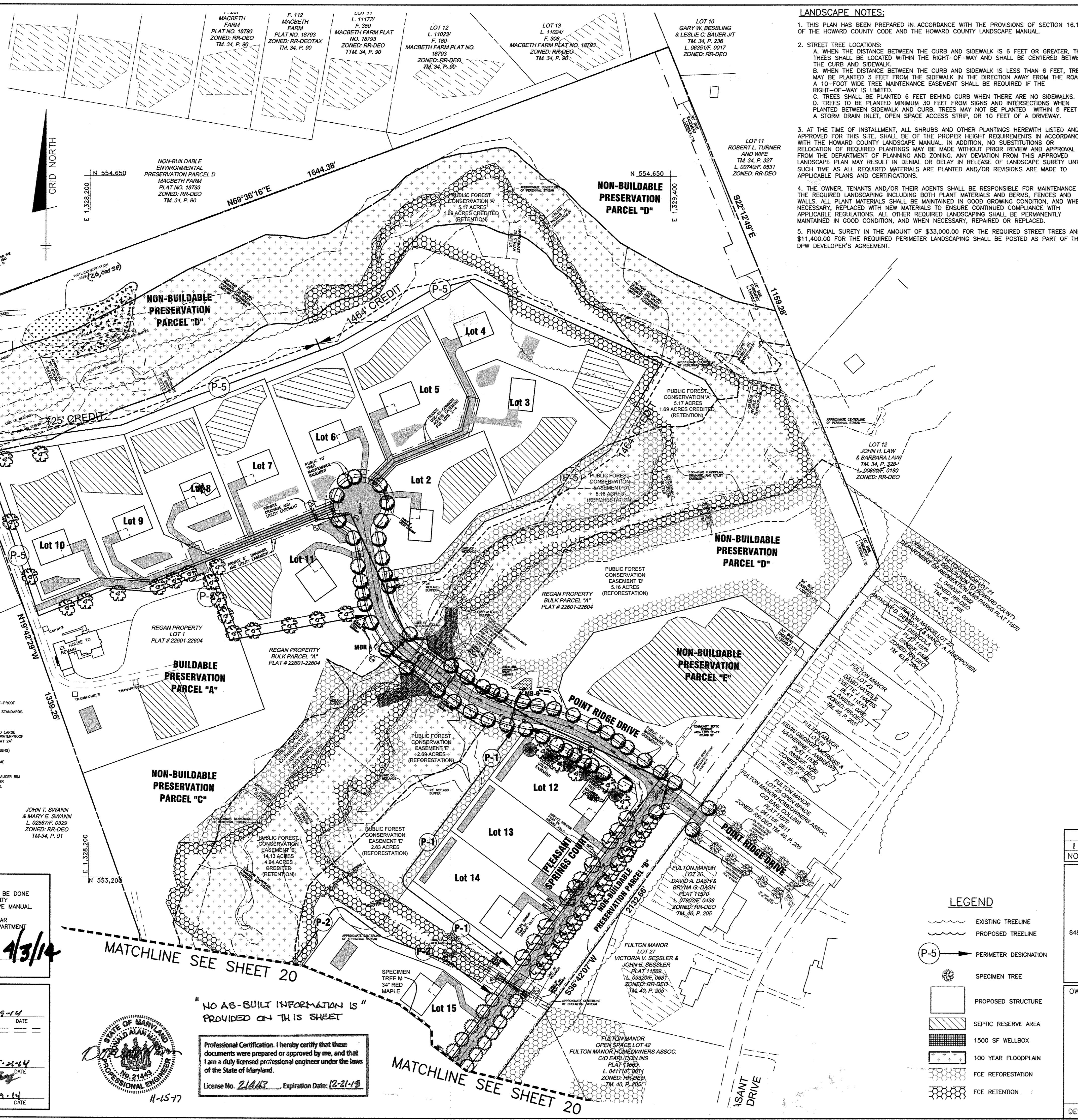
"NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET"

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

License No. 21423, Expiration Date: 12-21-18

LEGEND

- EXISTING TREELINE
- PROPOSED TREELINE
- PERIMETER DESIGNATION (P-5)
- SPECIMEN TREE
- PROPOSED STRUCTURE
- SEPTIC RESERVE AREA
- 1500 SF WELLBOX
- 100 YEAR FLOODPLAIN
- FCE REFORESTATION
- FCE RETENTION



1 2-9-15 SHOW FIDAL WETLAND MITIGATION AREA (20,000 SF PER MDE)

NO. DATE REVISION

BENCHMARK ENGINEERING, INC.
 ENGINEERS & LAND SURVEYORS & PLANNERS
 8480 BALTIMORE NATIONAL PIKE SUITE 315 & ELLIOTT CITY, MARYLAND 21043
 (P) 410-465-6105 (F) 410-465-6644
 75 THOMAS JOHNSON DRIVE SUITE E & FREDERICK, MARYLAND 21702
 301-710-5686
 WWW.BEI-CIVILENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 28559, Matthew J. Mitchell, 12-22-2015.

STATE OF MARYLAND PROFESSIONAL ENGINEER

OWNER/DEVELOPER: MB HIGHLAND RESERVE, LLC
 1686 EAST GUDE DRIVE
 ROCKVILLE, MD 20850
 301-762-9511

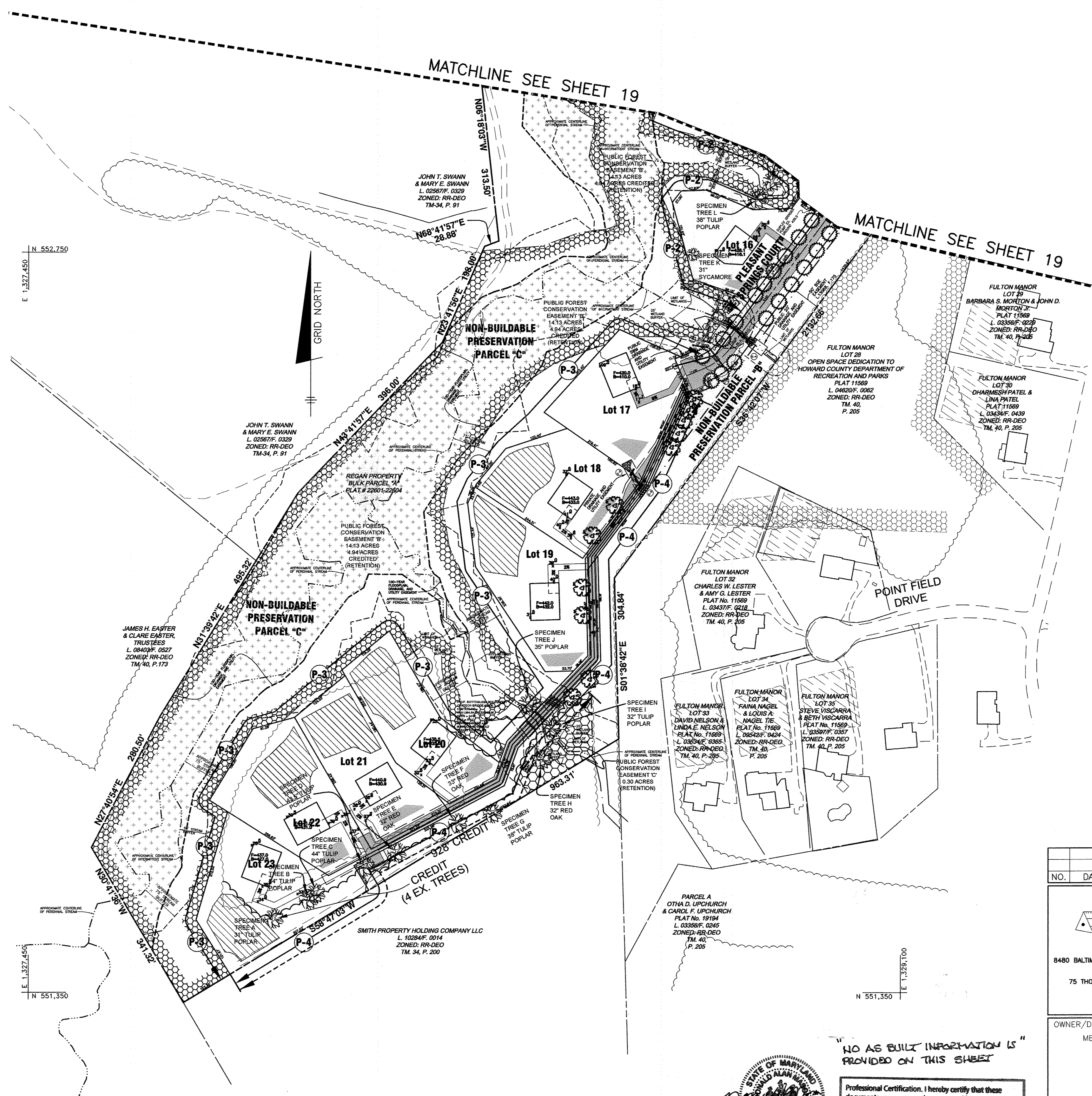
PROJECT: REGAN PROPERTY
 LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL "A", and NON-BUILDABLE PRESERVATION PARCELS "B" thru "E" A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL "A" PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

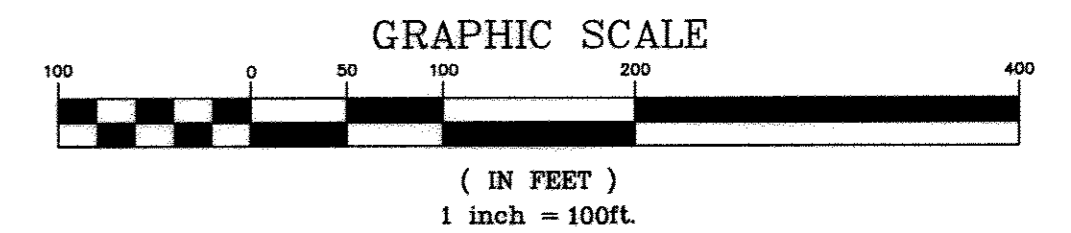
PERIMETER LANDSCAPE AND STREET TREE PLAN

DATE: APRIL, 2014 PROJECT NO. 2171
 SCALE: AS SHOWN DRAWING 19 OF 33

DESIGN: JMC DRAFT: JMC



- LEGEND**
- EXISTING TREELINE
 - PROPOSED TREELINE
 - PERIMETER DESIGNATION
 - SPECIMEN TREE
 - PROPOSED STRUCTURE
 - SEPTIC RESERVE AREA
 - 1500 SF WELLBOX
 - 100 YEAR FLOODPLAIN
 - FCE REFORESTATION
 - FCE RETENTION



DEVELOPER'S/BUILDER'S CERTIFICATE

I/WE CERTIFY THAT THE LANDSCAPING SHOWN ON THIS PLAN WILL BE DONE ACCORDING TO THE PLAN, SECTION 16.124 OF THE HOWARD COUNTY SUBDIVISION AND LAND DEVELOPMENT REGULATIONS AND LANDSCAPE MANUAL. I/WE FURTHER CERTIFY THAT UPON COMPLETION OF A LETTER OF LANDSCAPE INSTALLATION, ACCOMPANIED BY AN EXECUTED ONE-YEAR GUARANTEE OF PLANT MATERIALS, WILL BE SUBMITTED TO THE DEPARTMENT OF PUBLIC WORKS AND ZONING.

Jonathan J. Mitchell 4/3/14
DEVELOPER DATE

APPROVED: DEPARTMENT OF PUBLIC WORKS

Walter J. ... 4-29-18
CHIEF, BUREAU OF HIGHWAYS DATE

APPROVED: DEPARTMENT OF PLANNING AND ZONING

... 5-21-14
CHIEF, DIVISION OF LAND DEVELOPMENT DATE

... 5-2-14
CHIEF, DEVELOPMENT ENGINEERING DIVISION DATE



"NO AS BUILT INFORMATION IS PROVIDED ON THIS SHEET"

Professional Certification: I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

License No. 21443 Expiration Date: 12-31-19

NO.	DATE	REVISION

BENCHMARK ENGINEERING, INC.

ENGINEERS & LAND SURVEYORS & PLANNERS

8480 BALTIMORE NATIONAL PIKE & SUITE 315 A ELLICOTT CITY, MARYLAND 21043
(P) 410-465-8105 (F) 410-465-8644

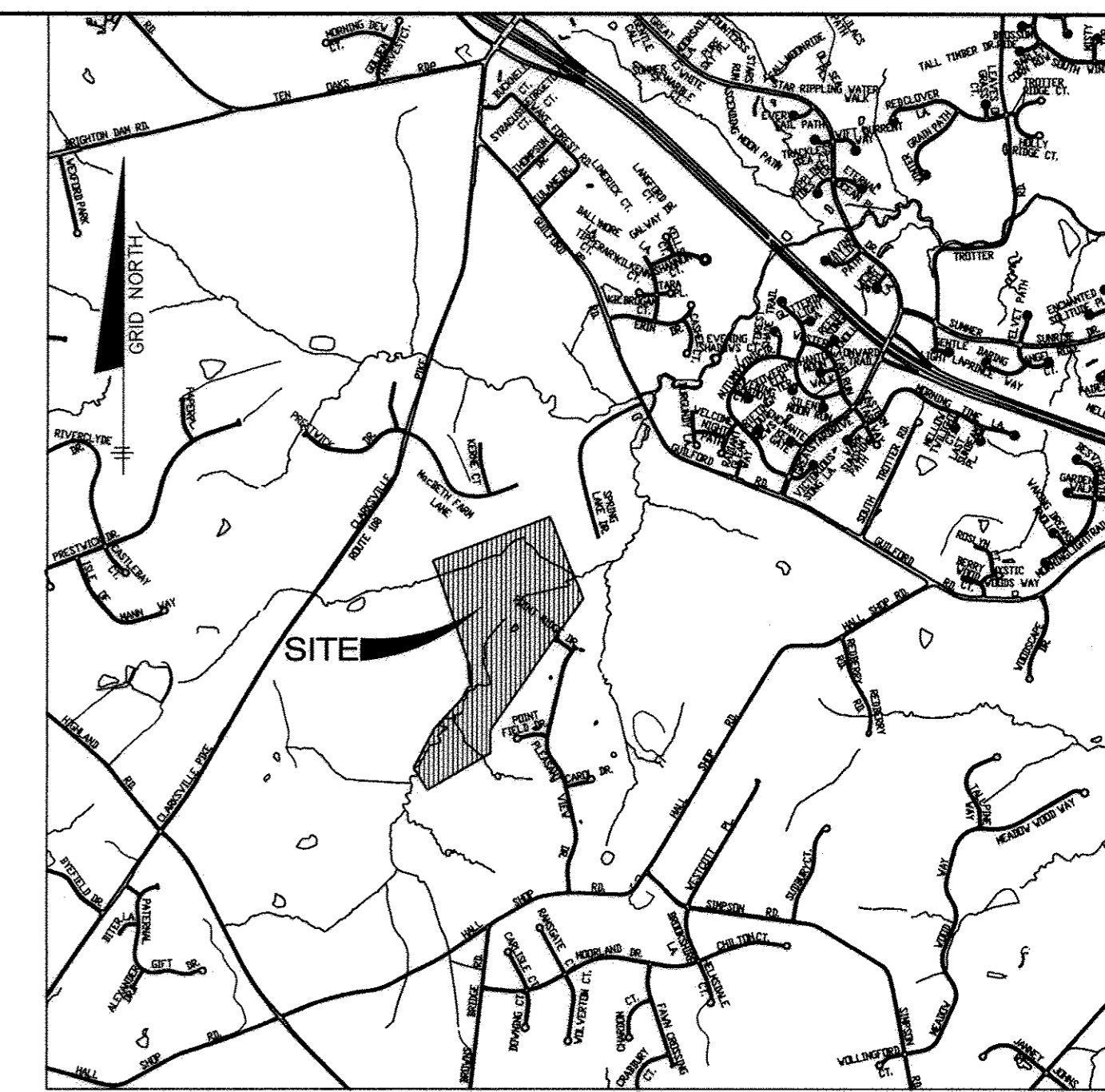
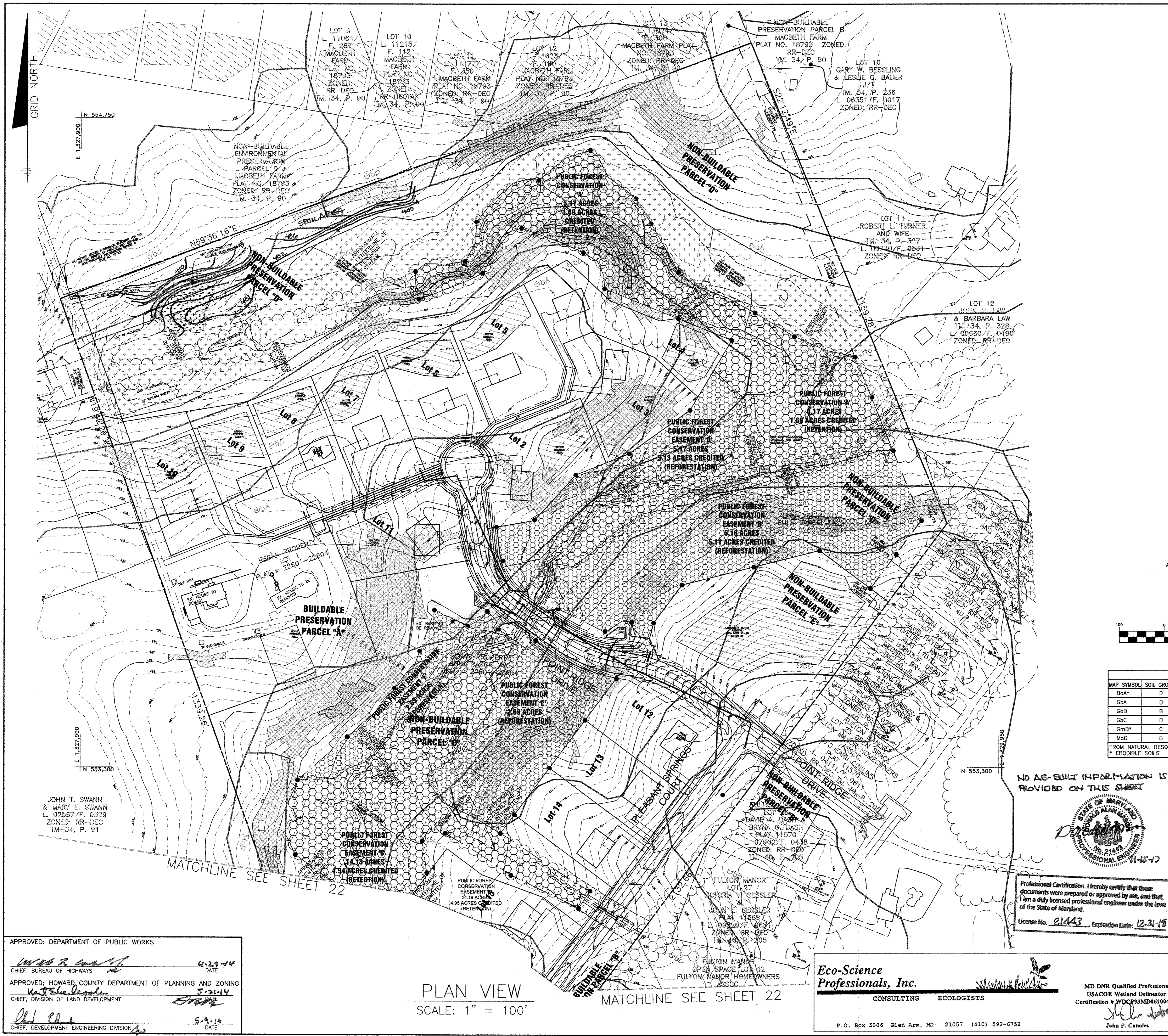
75 THOMAS JOHNSON DRIVE & SUITE E & FREDERICK, MARYLAND 21702
301-710-5686

WWW.BEI-CVLEENGINEERING.COM

Professional Certification: I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

License No. 28559 Expiration Date: 7-22-2015.

OWNER/DEVELOPER:	PROJECT:
MB HIGHLAND RESERVE, LLC 1686 EAST GUDE DRIVE ROCKVILLE, MD 20850 301-762-9511	REGAN PROPERTY LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL "A", and NON-BUILDABLE PRESERVATION PARCELS OF THIS A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL "A" PREVIOUSLY RECORDED AS PLAT NO. 22601-22604
LOCATION:	TITLE:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200 5th ELECTION DISTRICT HOWARD COUNTY, MARYLAND DPZ NO.: SP-12-004, ECP-12-045, WP-13-025	PERIMETER LANDSCAPE AND STREET TREE PLAN
DATE:	PROJECT NO.:
APRIL, 2014	2171
DESIGN:	DRAWING:
JMC	JMC
SCALE:	OF:
AS SHOWN	20 OF 33

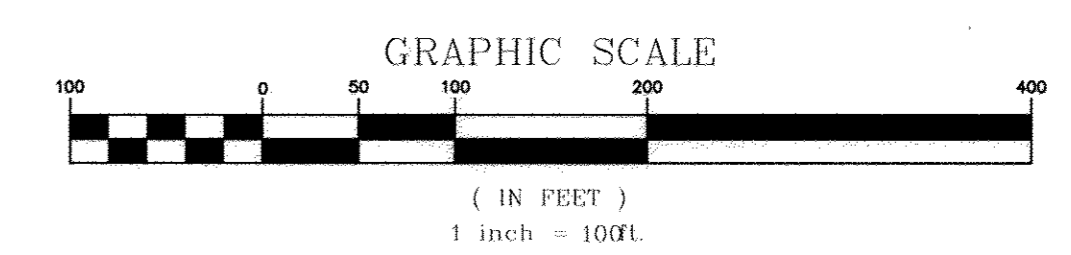


VICINITY MAP
SCALE: 1" = 2000'
ADC MAP PAGE: 31 GRID: D4

Specimen Trees		
Key	Species, Size (dbh)	Comments *
A	Tulip poplar, 31"	
B	Tulip poplar, 34"	Fair condition, poor trunk
C	Tulip poplar, 44"	Good condition, twin stems
D	Tulip poplar, 43.5"	Twin stems
E	Red oak, 32"	Fair, limb dieback
F	Red oak, 33"	
G	Tulip poplar, 38"	Poor condition, trunk rot
H	Red oak, 32"	
I	Tulip poplar, 32"	
J	Tulip poplar, 35"	
K	Sycamore, 31"	
L	Tulip poplar, 38"	
M	Red maple, 34"	

* Good unless otherwise noted

ALL SPECIMEN TREES ARE TO REMAIN.



MAP SYMBOL	SOIL GROUP	SOIL TYPE
Ba*	D	BAILE SILT LOAM, 0 TO 3 PERCENT SLOPES
GbA	B	GLADSTONE LOAM, 0 TO 3 PERCENT SLOPES
GbB	B	GLADSTONE LOAM, 3 TO 8 PERCENT SLOPES
GbC	B	GLADSTONE LOAM, 8 TO 15 PERCENT SLOPES
GmB*	C	GLENVILLE SILT LOAM, 3 TO 8 PERCENT SLOPES
MoD	B	MANOR LOAM, 15 TO 25 PERCENT SLOPES

FROM NATURAL RESOURCES CONSERVATION SERVICES WEB SOIL SURVEY 2.0
* ERODIBLE SOILS

LEGEND	
EXISTING CONTOURS (AERIAL 12/02)	
PROPOSED CONTOURS	
SOIL BOUNDARY	
LIMIT OF WETLANDS	
25' WETLANDS BUFFER	
CENTERLINE OF STREAM	
STREAM BUFFER	
EXISTING WOODS LINE	
PROPOSED WOODS LINE	
EXISTING STRUCTURE	
PROPOSED STRUCTURE	
SLOPES 15% TO 24.9%	
SLOPES 25% OR GREATER	
EX. 100 YEAR FLOODPLAIN	
PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)	
PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)	
TREE PROTECTION FENCE	
FCE PERMANENT SIGNAGE	
PROP. STREET TREE	
PRIVATE SEWAGE DISPOSAL AREA	

THE WATERSHED FOR THIS DRAINAGE AREA IS THE MIDDLE PATUXENT RIVER, DNR LISTING NUMBER 2111306.

NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443, Expiration Date: 12-31-18

NO.	DATE	REVISION
1	2-9-18	SHOW FINAL MITIGATION FOR WETLANDS & SPILL AREA FOR EARTH WORKS & REMOVE B&B

BENCHMARK ENGINEERING, INC.
ENGINEERS • LAND SURVEYORS • PLANNERS
8480 BALTIMORE NATIONAL PIKE & SUITE 315 • ELICOTT CITY, MARYLAND 21043
(P) 410-465-6105 (F) 410-465-6644
75 THOMAS JOHNSON DRIVE & SUITE E • FREDERICK, MARYLAND 21702
301-710-5686
WWW.BE-ENR.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
Expiration Date: 7-22-2015.

APPROVED: DEPARTMENT OF PUBLIC WORKS
WILL R. SWANN
CHIEF, BUREAU OF HIGHWAYS
4-29-18 DATE

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
M. J. Swann
CHIEF, DIVISION OF LAND DEVELOPMENT
5-21-18 DATE

APPROVED: JOHN T. SWANN & MARY E. SWANN
L. 02567/F. 0329
ZONED: RR-DEC
TM-34, P. 91

APPROVED: JOHN P. CANOLES
CHIEF, DEVELOPMENT ENGINEERING DIVISION
5-9-18 DATE

PLAN VIEW
SCALE: 1" = 100'

MATCHLINE SEE SHEET 22

Eco-Science Professionals, Inc.
CONSULTING ECOLOGISTS
P.O. Box 5006 Glen Arm, MD 21057 (410) 592-6752

MD DNR Qualified Professional
USACE Wetland Delinator
Certification # WDCP33MD061004432
John P. Canoles

OWNER/DEVELOPER:
RONALD R. REGAN
56B ORCHARD BEACH BLVD
PORT WASHINGTON, NY 11050

PROJECT: **REGAN PROPERTY**
LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E'. A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A', PREVIOUSLY RECORDED AS PLAT NO. _____

LOCATION:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200
HOWARD COUNTY, MARYLAND
DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
FINAL ROAD CONSTRUCTION PLAN
FOREST CONSERVATION PLAN

DATE: DECEMBER, 2013 PROJECT NO. 2171
SCALE: AS SHOWN DRAWING 21 OF 33

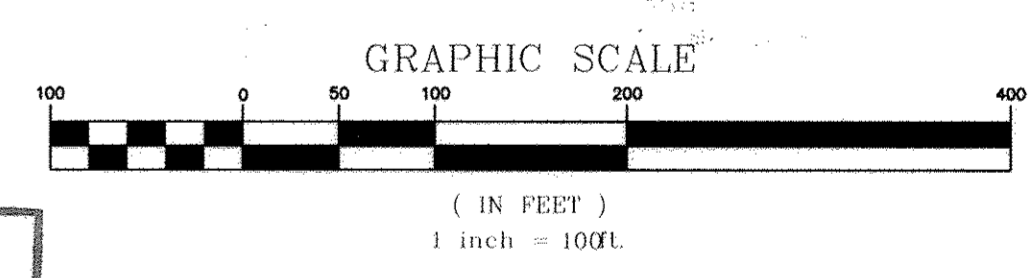
SOILS LEGEND		
MAP SYMBOL	SOIL GROUP	SOIL TYPE
BaA	D	BAILE SILT LOAM, 0 TO 3 PERCENT SLOPES
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GmB*	C	GLENVILLE SILT LOAM, 3 TO 8 PERCENT SLOPES
MoD	B	MANOR LOAM, 15 TO 25 PERCENT SLOPES

FROM NATURAL RESOURCES CONSERVATION SERVICES WEB SOIL SURVEY 2.0
* ERODIBLE SOILS

Specimen Trees		
Key	Species, Size (dbh)	Comments *
A	Tulip poplar, 31"	
B	Tulip poplar, 34"	Fair condition, poor trunk
C	Tulip poplar, 44"	Good condition, twin stems
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E	Red oak, 32"	Fair, limb dieback
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G	Tulip poplar, 38"	Poor condition, trunk rot
H	Red oak, 32"	
I	Tulip poplar, 32"	
J	Tulip poplar, 35"	
K	Sycamore, 31"	
L	Tulip poplar, 38"	
M	Red maple, 34"	

* Good unless otherwise noted
ALL SPECIMEN TREES ARE TO REMAIN.

LEGEND	
EXISTING CONTOURS (AERIAL 12/02)	
PROPOSED CONTOURS	
SOIL BOUNDARY	
LIMIT OF WETLANDS	
25' WETLANDS BUFFER	
CENTERLINE OF STREAM	
STREAM BUFFER	
EXISTING WOODS LINE	
PROPOSED WOODS LINE	
EXISTING STRUCTURE	
PROPOSED STRUCTURE	
SLOPES 15% TO 24.9%	
SLOPES 25% OR GREATER	
EX. 100 YEAR FLOODPLAIN	
PROPOSED FOREST CONSERVATION EASEMENT (RETENTION)	
PROPOSED FOREST CONSERVATION EASEMENT (AFFORESTATION/REFORESTATION)	
TREE PROTECTION FENCE	
FCE PERMANENT SIGNAGE	
PROP. STREET TREE	
PRIVATE SEWAGE DISPOSAL AREA	
PRIVATE WELL AREA	



NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET

Professional Certification: I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21113 Expiration Date: 12-31-18

THE WATERSHED FOR THIS DRAINAGE AREA IS THE MIDDLE PATUXENT RIVER, DNR LISTING NUMBER 2111306.

NO.	DATE	REVISION

BENCHMARK ENGINEERING, INC.
ENGINEERS • LAND SURVEYORS • PLANNERS
8480 BALTIMORE NATIONAL PIKE SUITE 3154 ELICOTT CITY, MARYLAND 21043
(P) 410-465-8105 (F) 410-465-6644
75 THOMAS JOHNSON DRIVE SUITE E A FREDERICK, MARYLAND 21702
301-672-4820
WWW.BE-CIVILENGINEERING.COM

Professional Certification: I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 28559 Expiration Date: 7-22-2015

OWNER/DEVELOPER:
RONALD R. REGAN
568 PORT WASHINGTON BLVD
PORT WASHINGTON, NY 11050

PROJECT: **REGAN PROPERTY**
LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E', A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO.

LOCATION:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200
5th ELECTION DISTRICT
HOWARD COUNTY, MARYLAND
DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
**FINAL ROAD CONSTRUCTION PLAN
FOREST CONSERVATION PLAN**

DATE: JANUARY 2014 PROJECT NO. 2171
SCALE: AS SHOWN DRAWING 22 OF 33

Eco-Science Professionals, Inc.
CONSULTING ECOLOGISTS

MD DNR Qualified Professional
USACOE Wetland Delinerator
Certification # WD093MD0610044B2

John P. Canolis

P.O. Box 5006 Glen Arm, MD 21057 (410) 592-6752

APPROVED: DEPARTMENT OF PUBLIC WORKS
4-29-14 DATE

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
5-11-14 DATE

APPROVED: CHIEF, DEVELOPMENT ENGINEERING DIVISION
5-9-14 DATE

PLAN VIEW
SCALE: 1" = 100'



FOREST CONSERVATION WORKSHEET

REGAN PROPERTY

Computations by: JCO BEI JOB NO. 2171 Date: 6/4/2013

NET TRACT AREA:

A. Total tract area	83.81 ac.
B. Land Dedication acres (parks, county facility, etc.)	0.00 ac.
C. Area within underground transmission lines but not floodplain	3.46 ac.
D. Area to remain in Commercial Agricultural Production/Use	0.00 ac.
E. Other deductions: (floodplain)	21.82 ac.
F. Net Tract Area	58.53 ac.

LAND USE CATEGORY:

Select category (RLD, RMD, Sub., CIO, Inst.) RMD

G. Afforestation Threshold 20% x "F" = 11.70 ac.

H. Conservation threshold 25% x "F" = 14.60 ac.

EXISTING FOREST COVER:

I. Existing forest cover 9.92 ac.

J. Area of forest above afforestation threshold 0.00 ac.

K. Area of forest above conservation threshold 0.00 ac.

BREAK EVEN POINT:

L. Forest retention above threshold with no mitigation	0.00 ac.
M. Clearing permitted without mitigation	0.00 ac.
N. Break Even Point	13.70 ac.

PROPOSED FOREST CLEARING:

N. Total area of forest to be cleared	2.99 ac.
O. Total area of forest to be retained	6.93 ac.

PLANTING REQUIREMENTS:

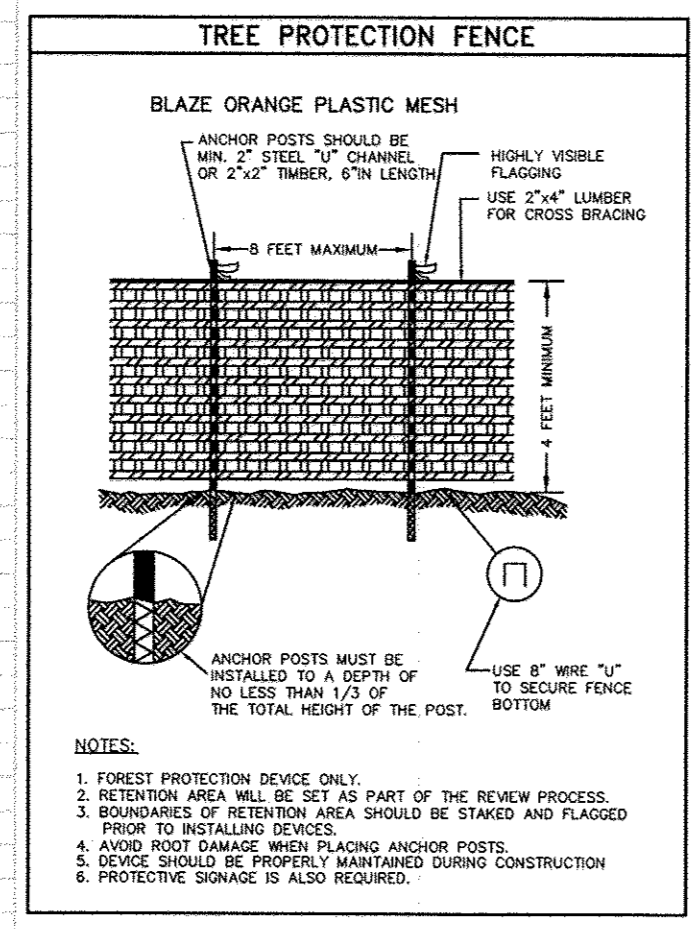
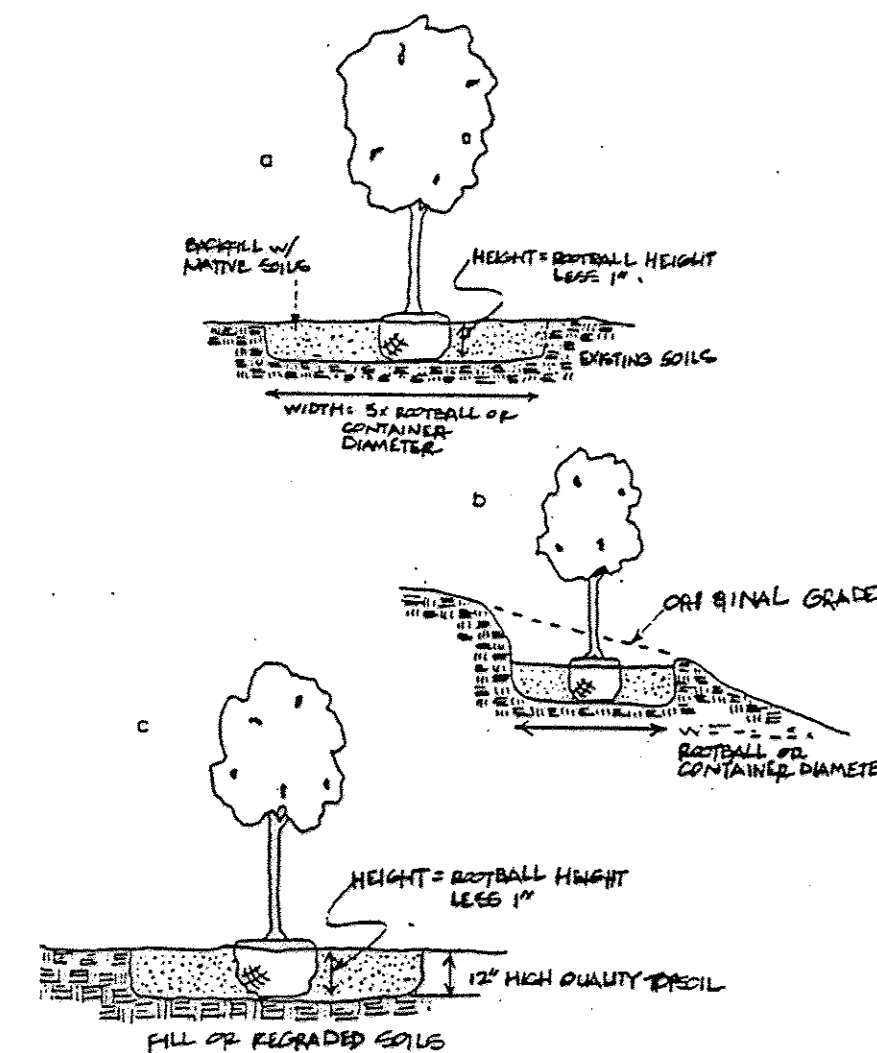
P. Reforestation for clearing above conservation threshold	0.00 ac.
Q. Reforestation for clearing below conservation threshold	6.00 ac.
R. Credit for retention above conservation threshold	0.00 ac.
S. Total reforestation required	6.00 ac.
T. Total afforestation required	1.80 ac.
U. Credit for landscaping - may not exceed 20% of "S."	0.00 ac.
V. Total reforestation and afforestation required	7.80 ac.

The Forest Conservation Obligations for this project will be met by:
6.93 ac. of ex. forest retained within an on-site Forest Conservation Easement,
7.80 of total reforestation and afforestation.

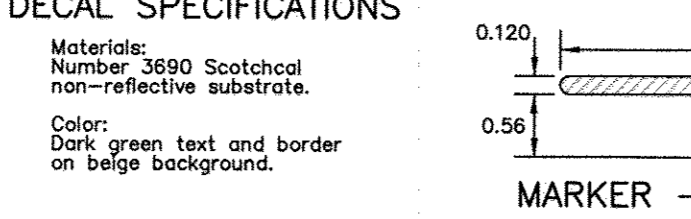
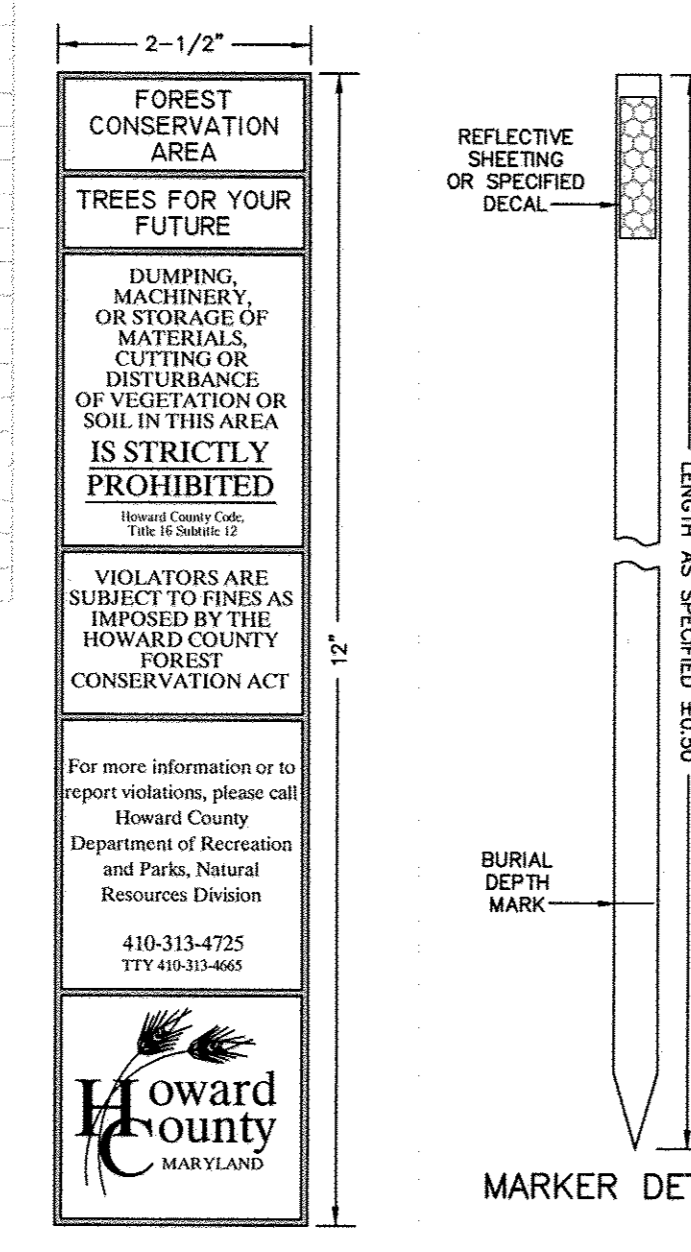
** THE DEVELOPER MAY BREAK THE PRESERVATION PARCEL INTO MULTIPLE PARCELS WITH DIFFERENT USES AND/OR OWNERS. THIS MAY HAVE AN AFFECT ON THE NTA.

NOTE: IN ACCORDANCE WITH THE FOREST CONSERVATION MANUAL, APPENDIX L, RURAL CLUSTER OPTION "B" HAS BEEN USED TO CALCULATE THE FOREST CONSERVATION OBLIGATIONS.

Planting Specifications:
Container Grown and Balled and Burlapped Stock



NOTES:
1. FOREST PROTECTION FENCE ONLY.
2. RETENTION AREA WILL BE SET AS PART OF THE REVIEW PROCESS.
3. BOUNDARIES OF PROTECTION AREA SHOULD BE STAKED AND FLAGGED PRIOR TO INSTALLING DEVICES.
4. ANCHOR POSTS MUST BE PLACED AT 10' INTERVALS.
5. DEVICES SHOULD BE PROPERLY MAINTAINED DURING CONSTRUCTION.
6. PROTECTIVE SIGNAGE IS ALSO REQUIRED.



FCE D - Afforestation Area - 5.18 acres

Qty	Species	Size	Spacing	Total FCA Units
300	Acer rubrum - Red maple	2-3 whip	11' o.c.	11.0
200	Acer saccharinum - Silver maple	2-3 whip	11' o.c.	7.3
150	Juglans nigra - Black walnut	2-3 whip	11' o.c.	5.5
250	Liriodendron tulipifera - Tulip poplar	2-3 whip	11' o.c.	8.3
150	Nyssa sylvatica - Black gum	2-3 whip	11' o.c.	5.5
250	Platanus occidentalis - Sycamore	2-3 whip	11' o.c.	8.3
200	Prunus serotina - Black Cherry	2-3 whip	11' o.c.	7.3
150	Quercus palustris - Pin oak	2-3 whip	11' o.c.	5.5
150	Viburnum prunifolium - Blackhaw	2-3 whip	11' o.c.	5.5
1813				62.7
Total whip plantings x 2 units/tree = FCA unit credit				3626
Total Unit Credit				3626

FCE E - Afforestation Area - 2.63 acres

Qty	Species	Size	Spacing	Total FCA Units
190	Acer rubrum - Red maple	2-3 whip	11' o.c.	6.7
75	Acer saccharinum - Silver maple	2-3 whip	11' o.c.	2.7
75	Juglans nigra - Black walnut	2-3 whip	11' o.c.	2.7
125	Liriodendron tulipifera - Tulip poplar	2-3 whip	11' o.c.	4.5
150	Nyssa sylvatica - Black gum	2-3 whip	11' o.c.	5.5
125	Platanus occidentalis - Sycamore	2-3 whip	11' o.c.	4.5
100	Prunus serotina - Black Cherry	2-3 whip	11' o.c.	3.7
75	Quercus palustris - Pin oak	2-3 whip	11' o.c.	2.7
81	Viburnum prunifolium - Blackhaw	2-3 whip	11' o.c.	2.9
921				34.0
Total whip plantings x 2 units/tree = FCA unit credit				1842
Total Unit Credit				1842

Planting Notes:
Planting density based spacing requirements: whips with shelter @ 11' on center.
* - these species should not be planted in wetlands.

Planting may be made in a curvilinear fashion along contour. The planting should avoid a grid appearance but should be spaced to facilitate maintenance.

Multiflora rose/heavy brush removal/control may be required prior to installation of planting.

All whips are required to be installed with tree shelters per Howard County FCA requirements.

Planting units defined by the spacing requirements established in the FCA Manual. One plant unit is defined as 1 seedling or whip without shelter. The Manual states that 700 seedlings/whips which is defined as 1 seedling or whip without shelter, or 200 1" caliper trees, or 100 2" caliper trees. By conversion it has been determined that a seedling or whip without shelter = 1 unit, whip with shelter = 2 units, 1" caliper tree = 3.5 units and 2" caliper tree = 7 units. The use of plant units simplifies the plant density calculations when mixing stock size.

FOREST CONSERVATION NOTES:

- ANY FOREST CONSERVATION EASEMENT (FCE) AREA SHOWN HEREON IS SUBJECT TO PROTECTIVE COVENANTS WHICH MAY BE FOUND IN THE LAND RECORDS OF HOWARD COUNTY WHICH RESTRICT THE DISTURBANCE AND USE OF THESE AREAS.
- THE FOREST CONSERVATION EASEMENTS HAVE BEEN ESTABLISHED TO FULFILL THE REQUIREMENTS OF SECTION 16-1209 OF THE HOWARD COUNTY CODE. FOREST CONSERVATION ACT. NO CLEARING, GRADING, OR CONSTRUCTION IS PERMITTED WITHIN THE FOREST CONSERVATION EASEMENTS; HOWEVER, FOREST MANAGEMENT PRACTICES AS DEFINED IN THE DEED OF FOREST CONSERVATION EASEMENT ARE ALLOWED.
- LIMITS OF DISTURBANCE SHALL BE RESTRICTED TO AREAS OUTSIDE THE LIMIT OF TEMPORARY FENCING OR THE FCE BOUNDARY, WHICHEVER IS GREATER.
- THERE SHALL BE NO CLEARING, GRADING, CONSTRUCTION OR DISTURBANCE OF VEGETATION IN THE FOREST CONSERVATION EASEMENT, EXCEPT AS PERMITTED BY HOWARD COUNTY DPZ.
- NO STOCKPILES, PARKING AREAS, EQUIPMENT CLEANING AREAS, ETC. SHALL OCCUR WITHIN AREAS DESIGNATED AS FOREST CONSERVATION EASEMENTS.
- TEMPORARY FENCING SHALL BE USED TO PROTECT FOREST RESOURCES DURING CONSTRUCTION. THE FENCING SHALL BE PLACED ALONG ALL FCE BOUNDARIES WHICH OCCUR WITHIN 15 FEET OF THE PROPOSED LIMITS OF DISTURBANCE.
- PERMANENT SIGNAGE SHALL BE PLACED 50-100' APART ALONG THE BOUNDARIES OF ALL AREAS INCLUDED IN FOREST CONSERVATION EASEMENTS.
- THE TOTAL FOREST CONSERVATION OBLIGATION AMOUNT OF 14.73 ACRES SHALL BE MET BY THE ON-SITE RETENTION OF 6.93 AC. WITHIN A FOREST CONSERVATION EASEMENT AND THE ON-SITE AFFORESTATION AND REFORESTATION OF 7.80 AC. WITHIN A FOREST CONSERVATION EASEMENT. THE DEVELOPER SHALL BOND IN ACCORDANCE WITH THE PUBLIC WORKS DEVELOPER'S AGREEMENT WITH SURETY IN THE AMOUNT OF \$169,884.00 (339,768.00 SF x 0.50).

FOREST CONSERVATION EASEMENT CHART

EASEMENT	TOTAL AREA	TYPE	CREDITED AREA
A	5.17 AC.	RETENTION	1.89 AC.
B	14.13 AC.	RETENTION	4.94 AC.
C	0.30 AC.	RETENTION	0.30 AC.
D	5.17 AC.	AFFORESTATION/REFORESTATION	5.11 AC.
E	2.69 AC.	AFFORESTATION/REFORESTATION	2.69 AC.

REFORESTATION PLAN

The reforestation area will be placed into a Forest Conservation Easement.

A. Planting Plan and Methods

Plant species selection was based on our knowledge regarding plant communities in Maryland's Piedmont Plateau and information provided in the soil survey on typical vegetation for the soil type on the planting site. Species selection was also based on our knowledge of plant availability in the nursery industry.

Reforestation will be accomplished through a mixed planting of whips and branched transplants. Container grown stock is recommended but bareroot stock may be used to help control afforestation costs. If bareroot stock is used the root systems of all plants will be dipped in an anti-desiccant gel prior to planting to improve moisture retention in the root systems.

Prior to planting the proposed Forest Conservation Easements all multiflora rose in the planting area shall be removed. Removal of the rose may be performed with mowing and herbicide treatments. Physical removal of all top growth following by a periodic herbicide treatment of stump sprouts is recommended. Native tree and shrub species occurring within the rose thickets should be retained wherever possible. Herbicide treatments shall occur on 2 month intervals during the first growing season and once each in the spring and fall for subsequent years. Herbicide used shall be made specifically to address woody plant material and shall be applied as per manufacturers specifications. Care should be taken not to spray planted trees or naturally occurring native tree/shrub seedlings. It is recommended that initiation of rose removal begin at least six months prior to planting.

D. Sequence of Construction

- The following timetable represents the proposed timetable for development. The items outlined in the Forest Conservation Plan will be enacted within two (2) years of subdivision approval.
- Install all signage and sediment control devices.
- Hold pre-construction meeting between developer, contractor and County inspector.
- Build access roads, install well and septic systems, and construct houses. Stabilize all disturbed areas accordingly.
- Begin multiflora rose removal. Install permanent protective signage for Easements and initiate plantings in accordance with Forest Conservation Plan. Plantings will be completed within two (2) years of subdivision approval.
- Remove sediment control.
- Hold post-construction meeting with County inspectors to assure compliance with FCP. Submit Certification of Installation.
- Monitor and maintain plantings for 2 years.

E. Construction Monitoring

Eco-Science Professionals, or another qualified professional designated by the developer, will monitor construction of the project to ensure that all activities are in compliance with the Forest Conservation Plan.

F. Post-Construction Meeting

Upon completion of construction, Eco-Science Professionals, or another qualified professional designated by the developer, will notify the County that construction has been completed and arrange for a post-construction meeting to review the project site. The meeting will allow the County inspector to verify that afforestation plantings have been installed.

C. Maintenance of Plantings

For information regarding maintenance of the reforestation plantings, see Section VIII B.

D. Guarantee Requirements

A 90 percent survival rate of the reforestation plantings will be required after one growing season. All plant material below the 90 percent survival threshold will be replaced at the beginning of the second growing season. At the end of the second growing season, a 75 percent survival rate will be required. All plant material below the 75 percent survival threshold will be replaced by the beginning of the next growing season.

E. Security for Reforestation

Section 16-1209 of the Howard County Forest Conservation Act requires that a developer shall post a security (bond, letter of credit, etc.) with the County to insure that all work is done in accordance with the FCP.

CONSTRUCTION PERIOD PROTECTION PROGRAM

A. Forest Protection Techniques

1. Soil Protection Area (Critical Root Zone)

The soil protection area, or critical root zone, of a tree is that portion of the soil column where most of a tree's roots are found. The majority of roots responsible for water and nutrient uptake are located just below the soil surface. Temporary fencing shall be placed around the critical root zone of the forest in areas where the forest limits occur within 25 feet of the limit of disturbance.

2. Fencing and Signage

Existing forest limits occurring within 25 feet of the limits of disturbance shall be protected using temporary protective fencing. Permanent signage shall be placed around the afforestation area prior to plant installation, as shown on the plan.

B. Pre-Construction Meeting

Upon staking of limits of disturbance a pre-construction meeting will be held between the developer, contractor and appropriate County inspector. The purpose of the meeting will be to verify that all sediment control is in order, and to notify the contractor of possible penalties for non-compliance with the FCP.

C. Storage Facilities/Equipment Cleaning

All equipment storage, parking, sanitary facilities, material stockpiling, etc. associated with construction of the project will be restricted to those areas outside of the proposed Forest Conservation Easement. Cleaning of equipment will be limited to areas within the L.O.D. of the proposed homesites. Wastewater resulting from equipment cleaning will be controlled to prevent runoff into environmentally sensitive areas.

FOREST PROTECTION PROCEDURES - Preconstruction Phase

- The edge of the woods to be protected will be marked (staked or flagged) in the field per the limits of forest conservation easement shown in the approved site development plan prior to the start of construction activity. All areas within protective easement area to be considered "off limits" to any construction activities. The optional protective fencing shall be installed at the outside edge of forested areas and should be combined with sediment control devices when possible. The limit of the critical root zone and therefore the location of the protective devices is to be determined as follows:
Edge of Forested Area - 1 foot of protective radius/inch of DBH or an eight foot protective radius, whichever is greater.

Critical Root Zone for the forest on this site is an average of 51' 12 feet from the trunk of the tree. Critical root zones for Specimen Trees:

- Construction activities expressly prohibited within the preservation areas are:
Placing or stockpiling backfill or top soil in protected areas
Felling trees into protected areas
Driving construction equipment into or through protected areas
Burning in or in close proximity to protected areas
Stocking or storing supplies of any kind
Concrete wash-off areas
Conducting trenching operations
Grading beyond the limits of disturbance
Parking vehicles or construction equipment
Removal of root mat or topsoil
Siting and construction of:
Utility lines
Access roads
Impervious surfaces
Stormwater management devices
Staging areas

Protective fencing (see Figure "Protective Fencing") shall be the responsibility of the general contractor. The general contractor shall affix signs to the fencing at 25' minimum intervals indicating that these areas are "Forest Retention Area" (see Figure "Signage"). The general contractor shall take great care to assure the restricted areas are not violated and that root systems are protected from smothering, flooding, excessive wetting from dewatering operations, off-site runoff, spillage, and drainage or solutions containing materials hazardous to tree roots.

The general contractor shall be responsible for any tree damaged or destroyed within the preservation areas whether caused by the contractor, his agents, employees, subcontractors, or licensees.

Foot traffic shall be kept to a minimum in the protective areas.

All trees which are not to be preserved within fifty feet of any tree preservation areas are to be removed in a manner that will not damage those trees that are designated for preservation. It is highly recommended that tree stumps within this fifty foot area be ground out with a stump grinding machine to minimize damage.

The general contractor shall designate a "wash out" area onsite for concrete trucks which will not drain toward a protected area.

A pre-construction meeting shall be held with local authorities before any disturbance has taken place on site.

FOREST PROTECTION PROCEDURES - Construction Phase

Forest and tree conditions should be monitored during construction and corrective measures taken when appropriate. The following shall be monitored:

- Soil compaction
- Root injury - prune and monitor; consider crown reduction
- Limb injury - prune and monitor
- Flooded conditions - drain and monitor; correct problem
- Drought conditions - water and monitor; correct problem
- Other stress signs - determine reason, correct, and monitor

FOREST PROTECTION PROCEDURES - Post Construction Phase

The following measures shall be taken:

- Corrective measures if damages were incurred due to negligence:
 - Stress reduction
 - Removal of dead or dying trees. This may be done only if trees pose an immediate safety hazard.
- Removal of temporary structures:
 - No burial of discarded materials will occur onsite within the conservation area.
 - No open burning within 100 feet of a wooded area.
 - All temporary forest protection structures will be removed after construction.
 - Remove temporary roads by removing stone or broadcasting mulch; pre-construction elevation should be maintained.
 - Aerate compacted soil.
 - Replant disturbed sites with trees, shrubs and/or herbaceous plants.
 - Retain signs for retention areas or specimen trees.
 - A County official shall inspect the entire site.
- Future protection measures:
 - Howard County and the developer shall arrange for the dedication of an appropriate forest conservation easement at a later date.

FOREST PROTECTION PROCEDURES - Preconstruction Phase

Stress Reduction and Protection of Specimen Trees Isolated from Forest Retention Areas and General Forest Retention Areas (as they may apply)

Isolated specimen trees that are to be preserved will be examined to determine if stress reduction techniques are needed. Protective measures and their evaluation criteria are provided on this plan only if they are employed herein.

Root Pruning

Will the critical root zone be affected by construction activities such as grade changes, digging for foundations and roads or utility installation?

Design Considerations

- Prune prior to construction as shown on the plan (see Figure "Root Pruning Detail.")
- Prune root with a clean cut using proper pruning equipment such as a vibratory knife.
- Exact location of pruning trench should be identified, and immediately backfilled to cover exposed roots after pruning with soil removed from topsoil, peat moss, or other suitable material or with other high organic soil.
- For trees over 15" in diameter, root pruning may be done up to one year in advance of construction.
- Tree(s) will be monitored for signs of stress.

Crown Reduction or Pruning

Has the root system been significantly reduced (>30%) or are there dead, damaged, or diseased limbs?

Evaluation Criteria

Design Considerations

- Reduce only at specified times of the year:
Flowering trees - only after flowering and before bud set
Non-Flowering trees - in late winter, early spring or mid summer
- No more than 1/3 of the crown should be removed at one time using acceptable pruning methods (see Figure "Crown Reduction Detail")
- Monitor for signs of stress

Watering

Will construction activities alter the hydrology of the site? Has or will root pruning occur?

Design Considerations

- Water only as necessary
- Monitor for signs of stress (see Figure "Tree Planting and Maintenance Calendar")

Fertilizing

Is or will be tree(s) be under stressful conditions? Has or will root pruning occur?

Design Considerations

- Use low nitrogen and slow release fertilizers.
- Apply in late fall or early spring (see Figure "Tree Planting and Maintenance Calendar")
- For small trees (<3" in diameter), use punch hole method or pressurized injection method (see Figure "Application of Fertilizers by Injection.")
- For larger trees (>3" diameter), use punch hole method or pressurized injection method (see Figure "Application of Fertilizers by Injection.")
- Do not apply fertilizer any closer than 3' from tree trunk for pressurized injection method.
- Monitor for signs of stress.

POST-CONSTRUCTION MANAGEMENT PLAN

Howard County requires a two year post-construction management plan be prepared as part of the forest conservation plan. The plan goes into effect upon acceptance of the construction certification of completion by the County. Eco-Science Professionals, or another qualified professional designated by the developer, will be responsible for implementation of the post-construction management plan.

The following items will be incorporated into the plan:

A. Fencing and Signage

Permanent signage indicating the limits of the retention/reforestation area shall be maintained.

B. General Site Inspections/Maintenance of Plantings

Site inspections will be performed a minimum of three times during the growing season. The purpose of the inspections will be to assess the health of the afforestation plantings. Appropriate measures will be taken to rectify any problems which may arise.

In addition, maintenance of the afforestation plantings will involve the following steps:

- Watering - All plant material shall be watered twice a month during the 1st growing season, more or less frequently depending on weather conditions. During the second growing season, once a month during May-September, if needed.
- Removal of invasive exotics and noxious weeds. Old field successional species will be retained.
- Identification of serious plant pests and diseases, treatment with appropriate agent.
- Pruning of dead branches.
- After 12 and 24 months, replacement of plants, if required, in accordance with the Guarantee Requirements shown on the FCP.

C. Education

The developer will provide appropriate materials to property owners informing them of the location and purpose of the afforestation area. Materials may include site plans and information explaining the intent of the forest conservation law.

D. Final Inspection

At the end of the two year post-construction management period, Eco-Science Professionals, or another qualified professional, will submit to the administrator of the Howard County Forest Conservation Program certification that all retention/afforestation requirements have been met. Upon acceptance of this certification, the County will release the developer from all future obligations and release the developer's bond.

AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 21443. Expiration Date: 12-24-18

Professional Engineer Seal: STATE OF MARYLAND, ENGINEERS & LAND SURVEYORS & PLANNERS, No. 21443, PROFESSIONAL ENGINEER, 11-15-17

Professional Engineer Seal: STATE OF MARYLAND, PROFESSIONAL ENGINEER, 11-15-17

BENCHMARK ENGINEERS, INC.

8480 BALTIMORE NATIONAL PIKE & SUITE 315 & ELLICOTT CITY, MARYLAND 21043
(P) 410-465-6105 (F) 410-465-6644
75 THOMAS JIMSON DRIVE & SUITE A FREDERICK, MARYLAND 21702
301-710-5686
WWW.BEI-ENGINEERING.COM

OWNER/DEVELOPER:
RONALD R. REGAN
56B ORCHARD BEACH BLVD
PORT WASHINGTON, NY
11050

PROJECT: **REGAN PROPERTY**
LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22604

LOCATION:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200
HOWARD COUNTY, MARYLAND
DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE: **FINAL ROAD CONSTRUCTION PLAN**

DATE: DECEMBER 2013 PROJECT NO. 2171

SCALE: AS SHOWN DRAWING 23 OF 33

DESIGN: JMC DRAFT: JMC

THE WATERSHED FOR THIS DRAINAGE AREA IS THE MIDDLE PATUXENT RIVER, DNR LISTING NUMBER 2111306.

Eco-Science Professionals, Inc. CONSULTING ECOLOGISTS
P.O. Box 5006 Glen Arm, MD 21057 (410) 592-6752

MD DNR Qualified Professional USACOE Wetland Delineator Certification # WPCP03MD061004422
John P. Canoles

FOREST PROTECTION PROCEDURES - Preconstruction Phase

Stress Reduction and Protection of Specimen Trees Isolated from Forest Retention Areas and General Forest Retention Areas (as they may apply)

Isolated specimen trees that are to be preserved will be examined to determine if stress reduction techniques are needed. Protective measures and their evaluation criteria are provided on this plan only if they are employed herein.

Root Pruning

Will the critical root zone be affected by construction activities such as grade changes, digging for foundations and roads or utility installation?

Design Considerations

- Prune prior to construction as shown on the plan (see Figure "Root Pruning Detail.")
- Prune root with a clean cut using proper pruning equipment such as a vibratory knife.
- Exact location of pruning trench should be identified, and immediately backfilled to cover exposed roots after pruning with soil removed from topsoil, peat moss, or other suitable material or with other high organic soil.
- For

BRIDGE PLATE ARCH CULVERT DESIGN

38' SPAN 12'-6.54" RISE 7 GAGE (0.187")

63'-3.0625" LONG

GENERAL NOTES:

Scope: The plate arch culvert & retaining wall engineer's (Ryan & Associates) scope consists of preparing the culvert & culvert retaining walls designs to enable the contractor to obtain necessary permits and properly construct the arch and walls. The design considers the internal and local stability and is in accordance with acceptable engineering practice and these specifications. A scour analysis has been performed and the riprap designed by Benchmark Engineering on their sheet 33 provides adequate protection. Professional Engineering oversight and certification for the arch construction is also a requirement of these plans. Services outside this scope such as investigation of failed or non-conforming construction or any other services may be provided on a time & materials basis or for a negotiated fee. The scope of Ryan & Associates (RA) for this project does not include arch stakeout or any other civil engineering/surveying. Reinforcing shop drawings are required to be reviewed and approved by the design engineer.

INSTALLATION MUST CONFORM TO THE ATTACHED "Ryan & Associates Structural Specifications and Guidelines".

CONSTRUCTION REVIEW & CERTIFICATION: CONSTRUCTION INSPECTION, TESTING AND CERTIFICATION BY A MD LICENSED PROFESSIONAL STRUCTURAL & GEOTECHNICAL ENGINEER IS A REQUIREMENT OF THESE PLANS. ACCEPTANCE OF THE USE OF THESE PLANS INDICATES AGREEMENT FOR PROFESSIONAL ENGINEERING CONSTRUCTION REVIEW AND CERTIFICATION

THESE DRAWINGS ARE THE PROPERTY OF RYAN & ASSOCIATES. UNAUTHORIZED REPRODUCTION FOR ANY PURPOSE IS AN INFRINGEMENT UPON COPYRIGHT LAWS. VIOLATORS WILL BE SUBJECT TO PROSECUTION BY THE FULLEST EXTENT OF THE LAW.

WRITTEN DIMENSIONS ON THE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALE DIMENSIONS. CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THIS OFFICE MUST BE NOTIFIED OF ANY VARIATION FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THESE DRAWINGS.

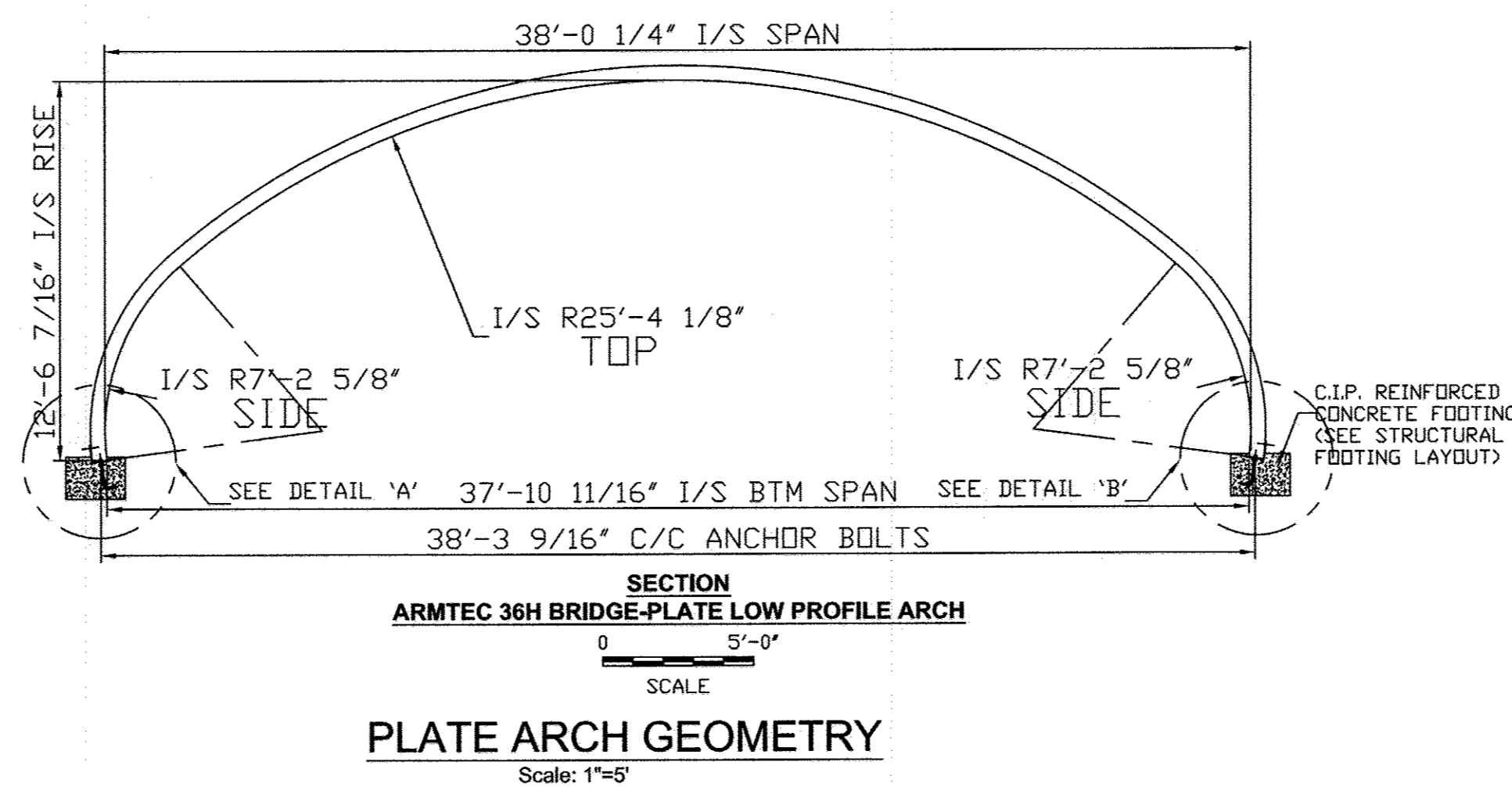
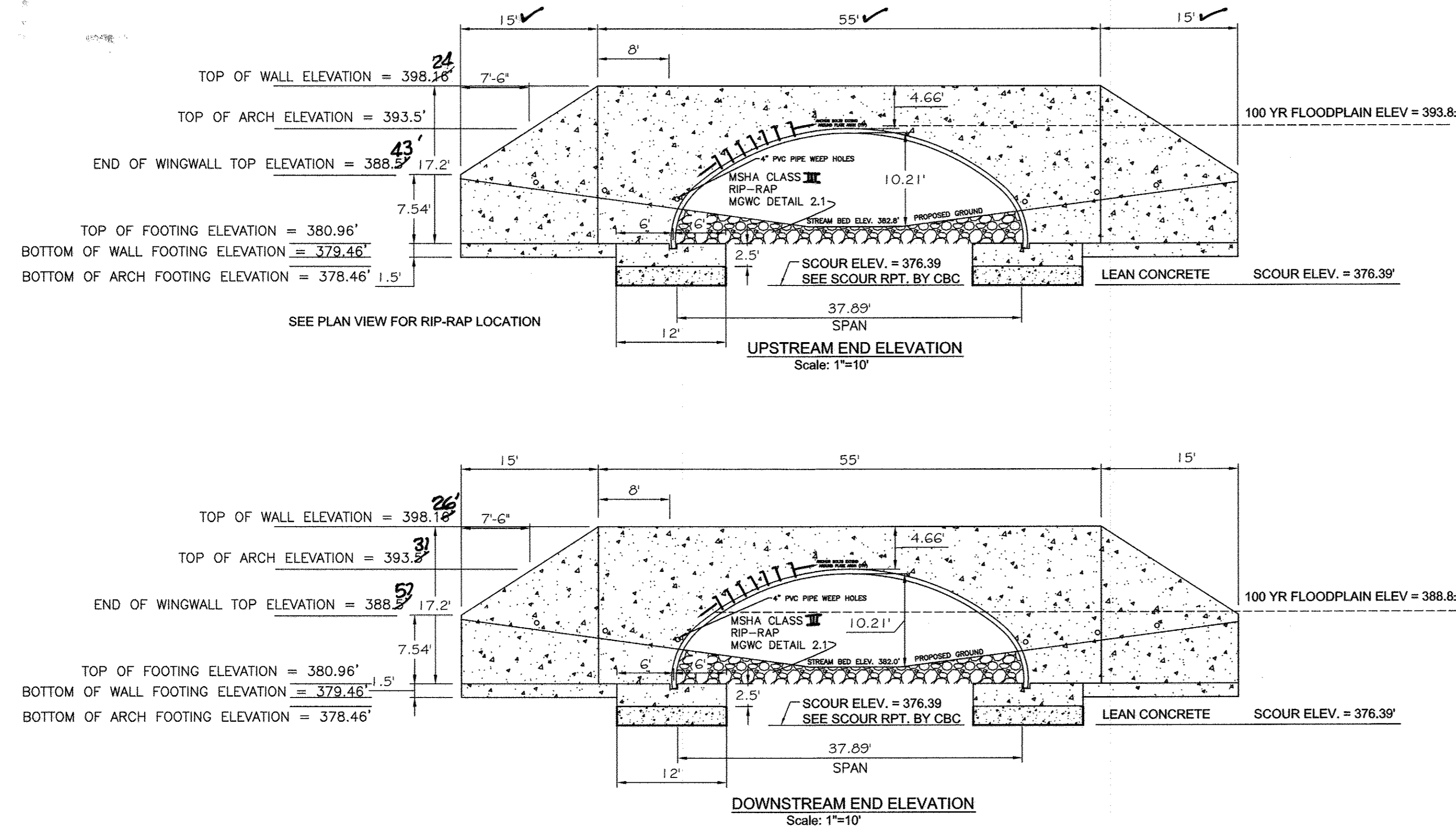
DESIGN DATA

DESIGN LOADING:
 ARMTEC BRIDGE PLATE STRUCTURE: HS25
 HEADWALLS: EARTH PRESSURE ONLY
 WINGWALLS: EARTH PRESSURE ONLY
 DESIGN FILL HEIGHT: 3'-6" MIN - 5'-0" MAX FROM CROWN OF PLATE TO BOTTOM OF FLEXIBLE PAVEMENT
 NET ALLOWABLE SOIL BEARING PRESSURE: 4,000 PSF *
 GROSS ALLOWABLE SOIL BEARING PRESSURE: 4,000 PSF *

* FOUNDATION EXCAVATION AND SUBGRADE PREPARATION SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL RECOMMENDATIONS FOR THIS PROJECT PREPARED BY GEOLAB GEOTECHNICAL LABORATORIES, INC., DATED 05/24/13 AND 11/29/13 AND SUBSEQUENT RECOMMENDATIONS TO ACHIEVE 4,000 PSF ALLOWABLE BEARING CAPACITY.

GEOTECH RECOMMENDATIONS TO ACHIEVE 4,000 PSF ALLOWABLE BEARING CAPACITY:
 IF THE FOOTERS FOR THIS STRUCTURE NEED TO HAVE SUBGRADE LOWERED TO ACHIEVE 4000PSF BEARING THEN EXCAVATE DOWN TO 4000PSF BEARING AND IMMEDIATELY BACKFILL WITH LEAN CONCRETE TO BOTTOM OF FOOTING ELEVATION.

PROVIDE A FENCE ALONG THE END OF THE WALL. END OF THE FENCING MUST BE TAPERED TO AVOID ACCESS TO THE LEDGE.



DRAWING INDEX	
Sheet 24 - Upstream & Downstream Elevations, Plate Geometry	
Sheet 25 - Structural Elevation, Combined Footing Layout	
Sheet 26 - Arch, Head & Wingwall Details	
Sheet 27 - Manufacturer "Armtec" Specifications	
Sheet 28 - Structural Specifications	

NO.	DATE	REVISION

BENCHMARK
 ENGINEERS & LAND SURVEYORS & PLANNERS
 8480 BALTIMORE NATIONAL PIKE & SUITE 315 & ELLIOTT CITY, MARYLAND 21043
 (P) 410-465-6105 (F) 410-465-6644
 75 THOMAS JOHNSON DRIVE & SUITE E & FREDERICK, MARYLAND 21702
 301-710-5686
 WWW.BEI-CIVILENGINEERING.COM

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, William K. Ryan, P.E. License No. 21586, Expiration Date: 5-9-2015.

OWNER/DEVELOPER: RONALD R. REGAN 56B ORCHARD BEACH BLVD PORT WASHINGTON, NY 11050	PROJECT: REGAN PROPERTY LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL A PREVIOUSLY RECORDED AS PLAT NO. 22601-22604
LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200 5th ELECTION DISTRICT HOWARD COUNTY, MARYLAND DPZ No.: SP-12-004, ECP-12-045, WP-13-025	TITLE: PLATE ARCH CULVERT CROSSING #1 UPSTREAM/DOWNSTREAM ELEVATIONS, PLATE GEOMETRY
DESIGNER: WKR DRAFTER: WKR	DATE: APRIL 4, 2014 PROJECT NO. 2171 SCALE: AS SHOWN DRAWING 24 OF 33



CALL "MISS UTILITY"
 TELEPHONE 1-800-257-7777 FOR UTILITY LOCATIONS AT LEAST 48 HOURS BEFORE CONSTRUCTION.

AS-BUILT CERTIFICATION
 I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications
 Donald Mason, P.E. Date: 11-15-17



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443, Expiration Date: 12-21-19

APPROVED: DEPARTMENT OF PUBLIC WORKS <i>W. J. ...</i> CHIEF, BUREAU OF HIGHWAYS DATE: 4-29-14	APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING <i>W. K. ...</i> CHIEF, DIVISION OF LAND DEVELOPMENT DATE: 5-21-14
APPROVED: CHIEF, DEVELOPMENT ENGINEERING DIVISION <i>Ch. ...</i> DATE: 5-9-14	

DRAWN BY: WKR	CLIENT: LANE/LSB&C
DESIGN BY: WKR	OWNER: MB Highland Reserve, LLC
CHECKED BY: WKR	JOB No: 1101-14-03
DATE: 03/27/2014	

DO NOT SCALE THIS DRAWING. DIMENSIONS AND NOTES HAVE PRECEDENCE OVER DRAWING

Ryan & Associates
 A Division of WKR Consulting Inc.
 SPECIALIZING IN STRUCTURAL & GEOTECHNICAL ENGINEERING
 e-mail: info@ryanandassociates.net

Frederick, MD Office
 2412 Wynfield Ct.
 Frederick, MD 21702

301-360-9534 (ph)
 301-360-9574 (fx)

www.ryanandassociates.net

LANE LONGSPAN

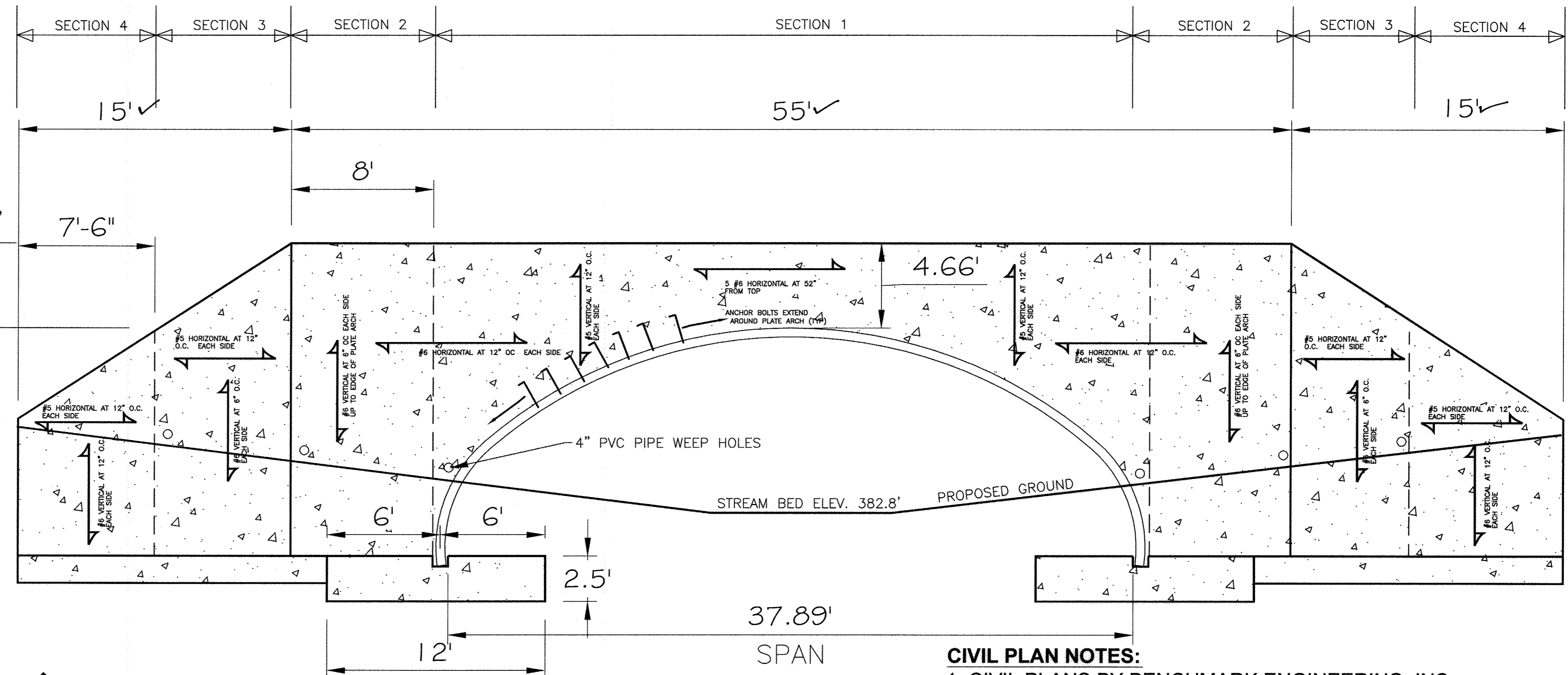
LANE ENTERPRISES, INC. - LSB&C DIVISION
 8271 MERCER ST., PULASKI, PA 16143
 LANE: 724-652-7747
 LSB&C: 888-949-5722

AS-BUILT CERTIFICATION
I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications
Donald Mason, P.E. Date: 11-15-17



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443, Expiration Date: 12-21-18

TOP OF WALL ELEVATION = 398.46'²⁶
TOP OF ARCH ELEVATION = 393.5'[✓]
END OF WINGWALL TOP ELEVATION = 388.5'[✓]
TOP OF FOOTING ELEVATION = 380.96'
BOTTOM OF WALL FOOTING ELEVATION = 379.46'
BOTTOM OF ARCH FOOTING ELEVATION = 378.46'^{1.5'}



HEADWALL/WINGWALL ELEVATION - TYPICAL BOTH SIDES
SCALE: 3/8"=1'

CIVIL PLAN NOTES:
1. CIVIL PLANS BY BENCHMARK ENGINEERING, INC. AND THEY ARE TO BE FOLLOWED AND HOLD PRECEDENCE OVER THESE PLANS FOR LOCATION INFORMATION;
2. FENCH ALONG HEADWALLS & WINGWALLS AS SHOWN ON SHEET 26 AND IN ACCORDANCE WITH HOWARD COUNTY CHAIN LINK FENCE STANDARD DETAIL G-7.21.

PLATE ARCH STRUCTURAL NOTES:
1. SUBGRADE BEARING CAPACITY 4000PSF TO BE VERIFIED BY OVERSITE ENGINEER OF RECORD;
2. FOOTING CONCRETE 4000 PSI;
3. ALL BACKFILL AROUND ARCH TO BE A-1-a MIN 19' PAST ARCH AND COMPACTED TO 95% STD. PROCTOR MAX. DRY DENSITY;
4. SCOUR ANALYSIS HAS BEEN COMPLETED BY CBC ENGINEERS. SCOUR IS NEXT TO SIDES OF CULVERT. FOOTING EXTENDING 6' INSIDE CULVERT MEETS SCOUR PROTECTION REQUIREMENTS.;
5. 2000 PSI LEAN CONCRETE OR VIBRATED #57 STONE TO BE USED BELOW FOOTING SUBGRADE TO ACHIEVE BEARING CAPACITY IF NEEDED.

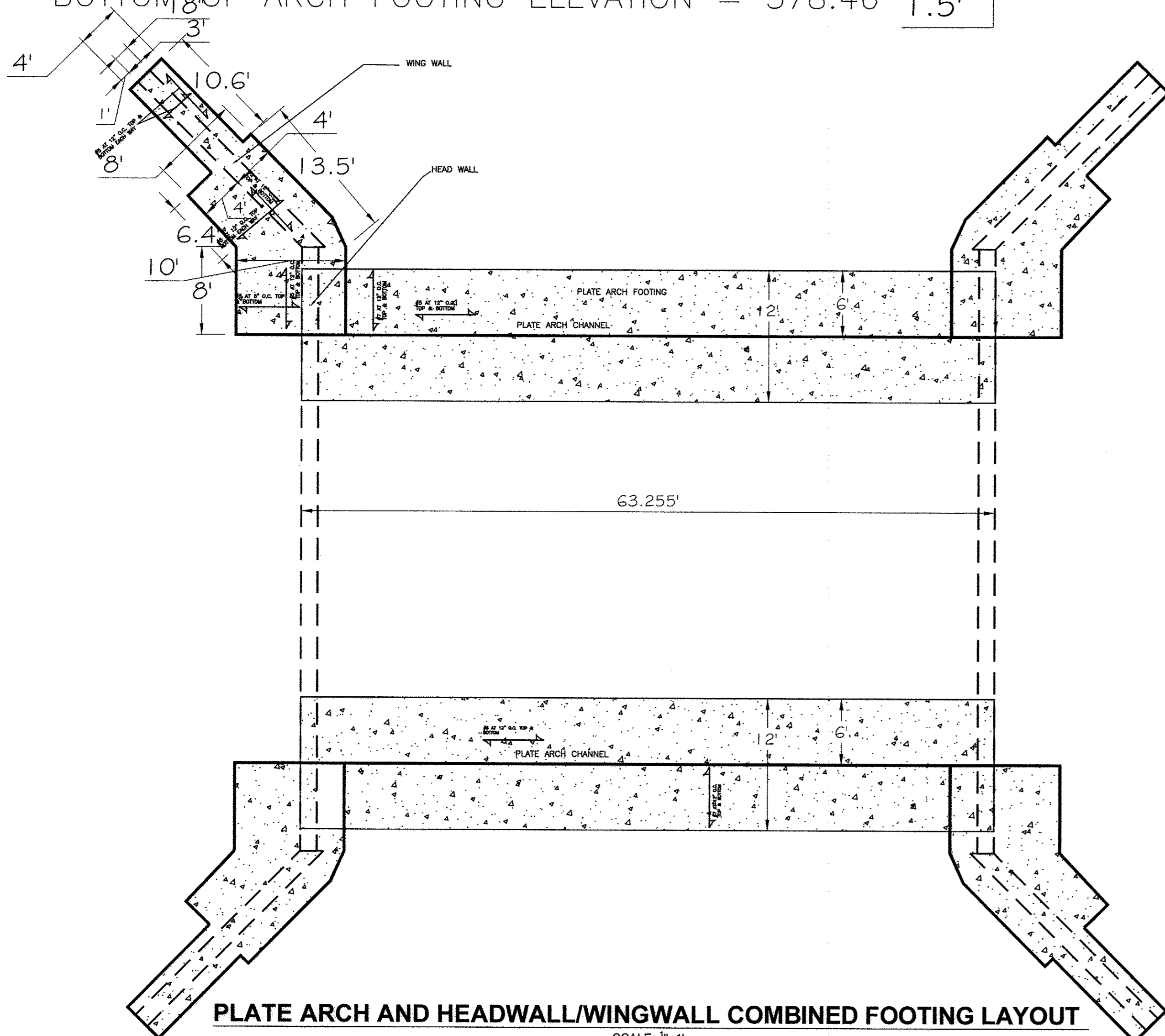
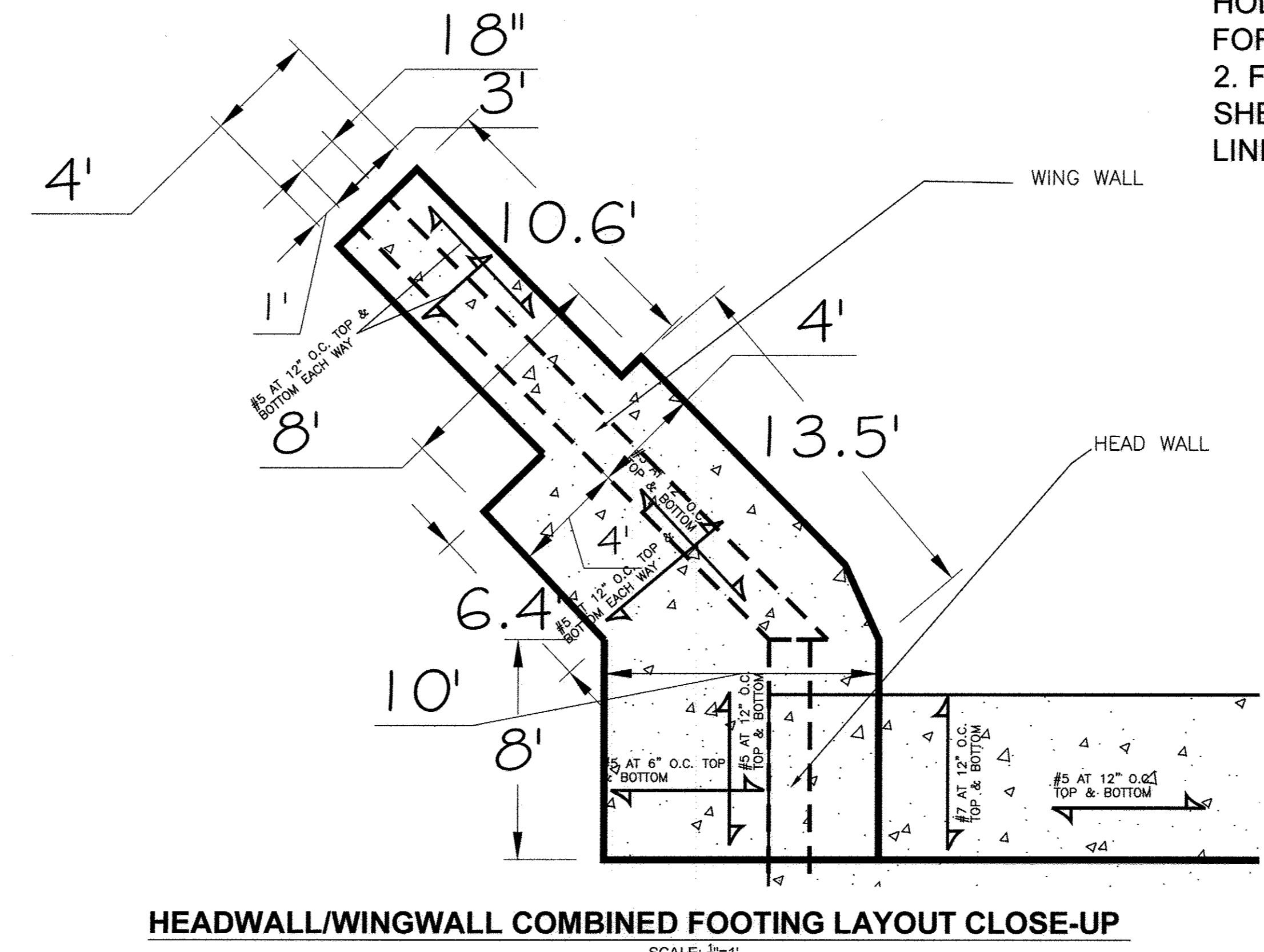


PLATE ARCH AND HEADWALL/WINGWALL COMBINED FOOTING LAYOUT
SCALE: 1/2"=1'



HEADWALL/WINGWALL COMBINED FOOTING LAYOUT CLOSE-UP
SCALE: 3/8"=1'

APPROVED: DEPARTMENT OF PUBLIC WORKS
4-29-14
DATE
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
5-21-14
DATE
APPROVED: CHIEF, DEVELOPMENT ENGINEERING DIVISION
5-9-14
DATE

DRAWN BY: WKR
DESIGN BY: WKR
CHECKED BY: WKR
DATE: 03/27/2014
CLIENT: LANE/LSB&C
OWNER: MB Highland Reserve, LLC
JOB No: 1101-14-03
DO NOT SCALE THIS DRAWING. DIMENSIONS AND NOTES HAVE PRECEDENCE OVER DRAWINGS.

Ryan & Associates
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SPECIALIZING IN STRUCTURAL & GEOTECHNICAL ENGINEERING
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301-360-9574 (fx)
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LANE **LONGSPAN**
LANE ENTERPRISES, INC. - LSB&C DIVISION
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LANE: 724-652-7747
LSB&C: 888-949-5722

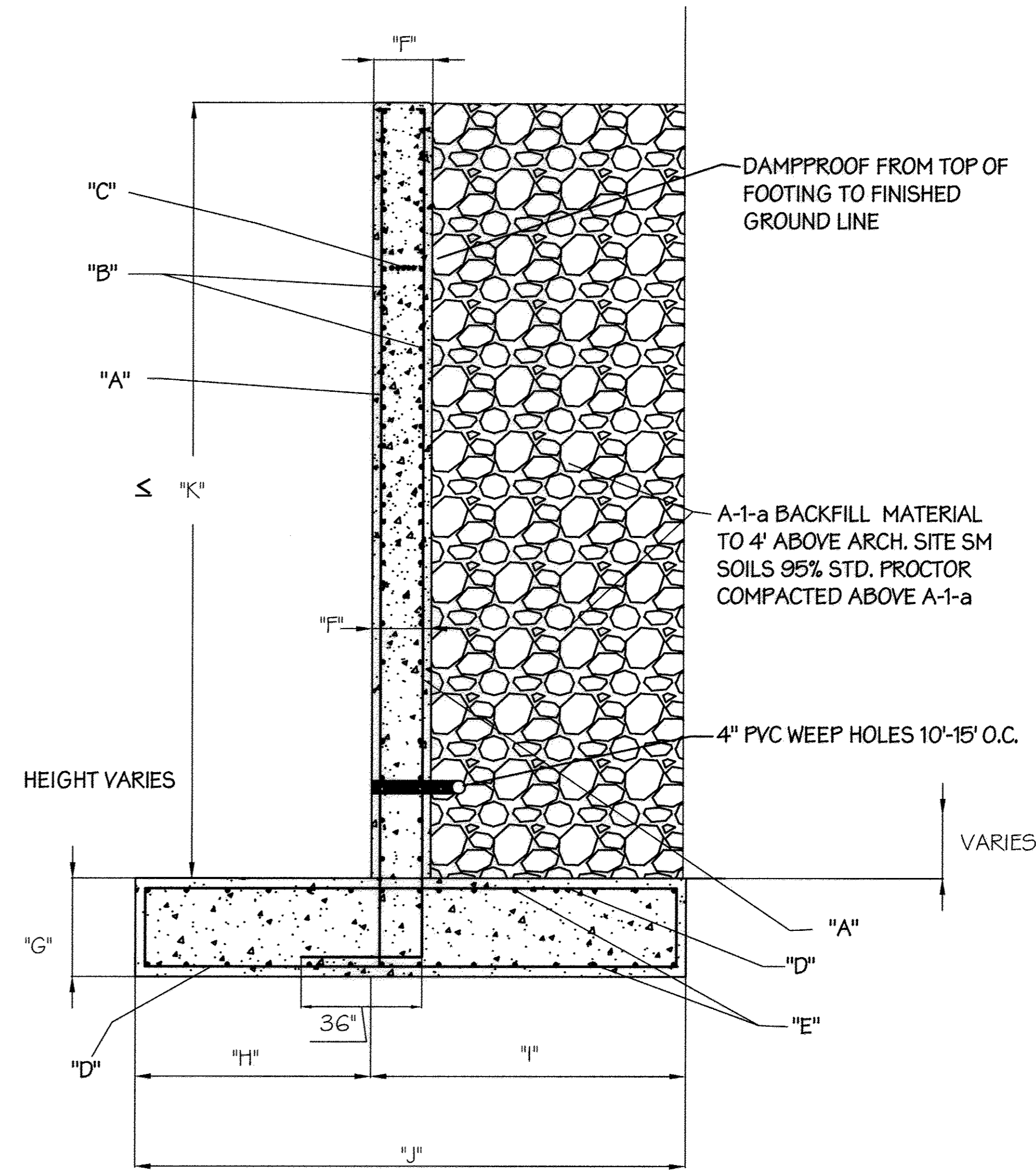
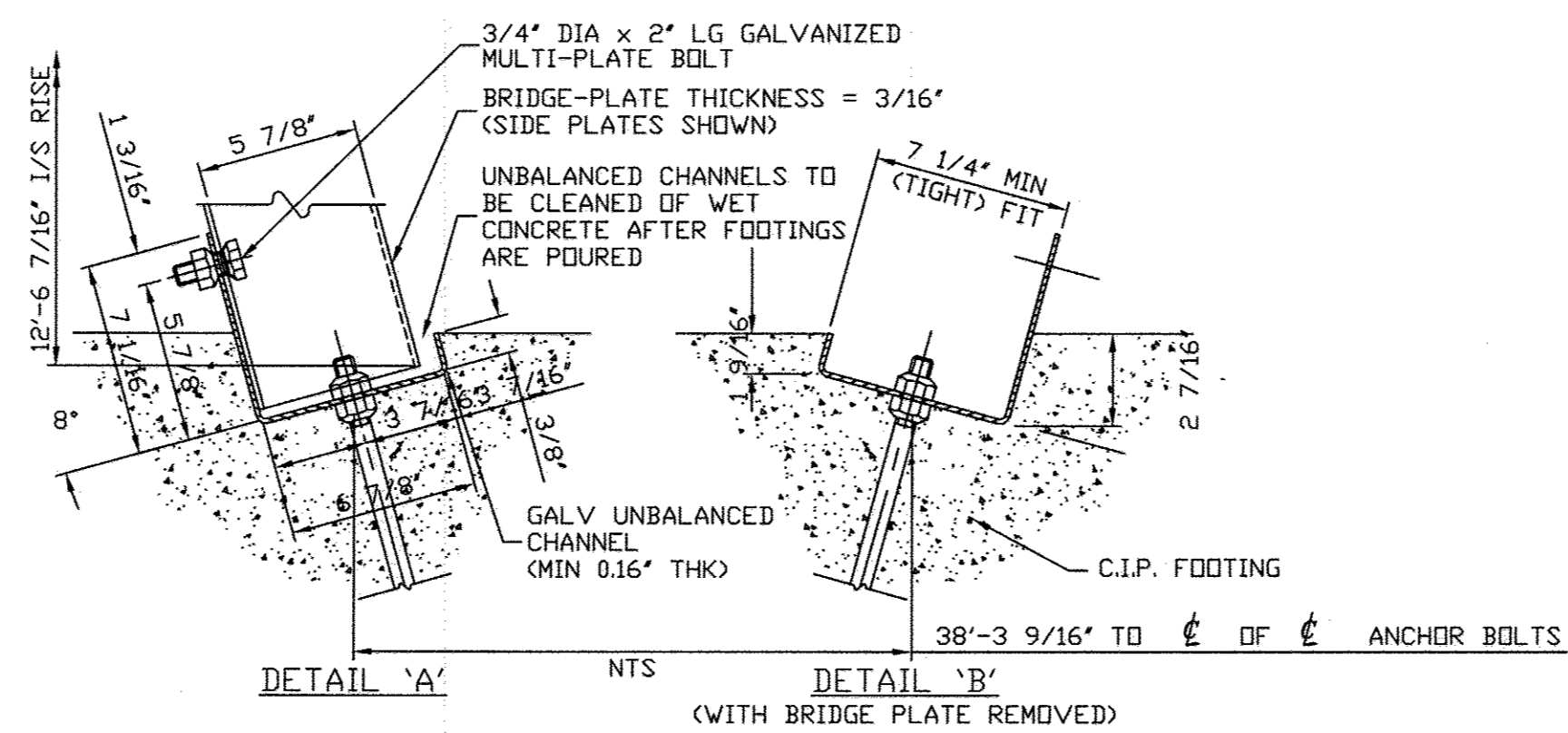
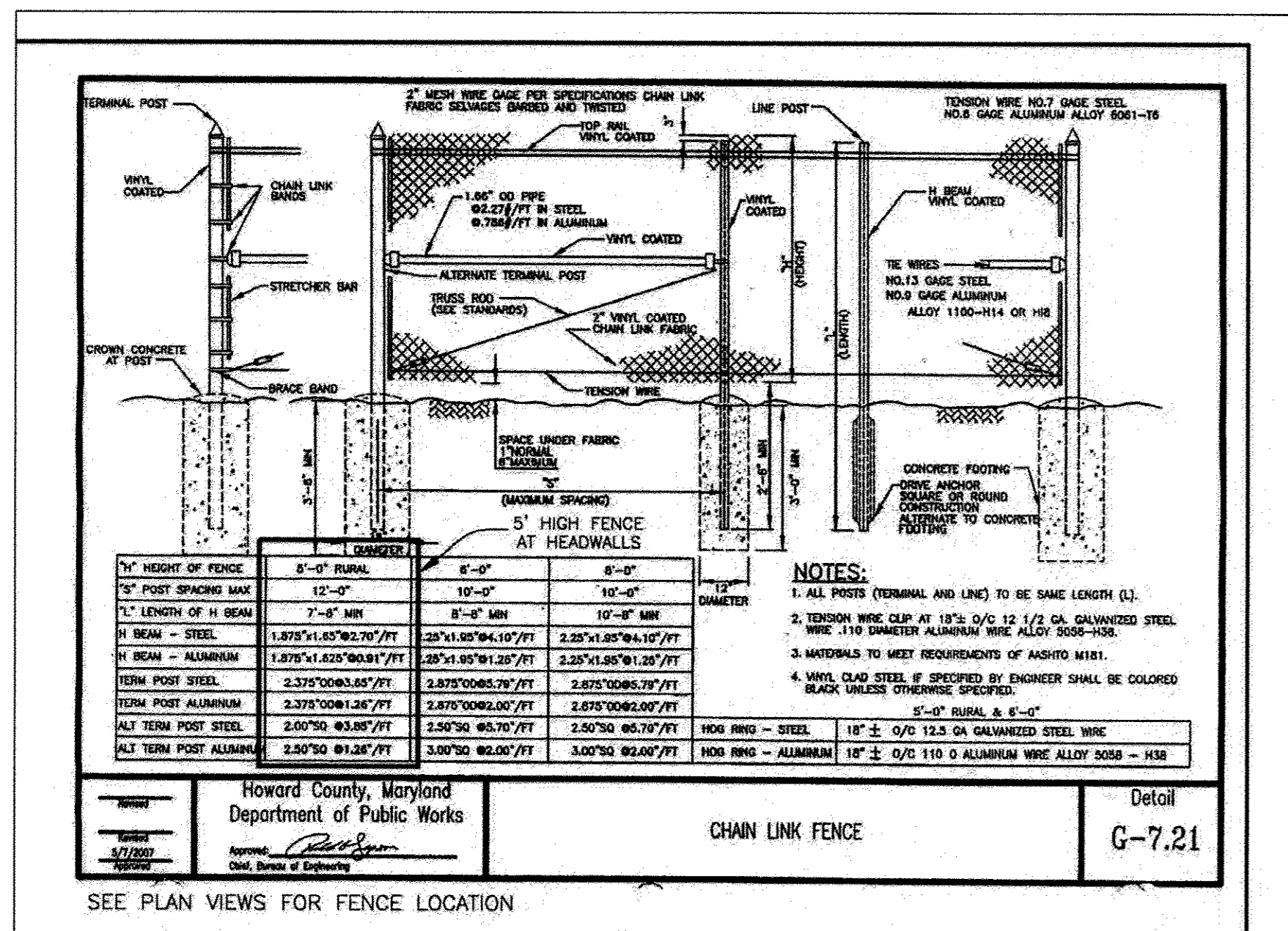
NO.	DATE	REVISION

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, William K. Ryan, P.E. License No. 21586, Expiration Date: 5-9-2015.

BENCHMARK ENGINEERS, LAND SURVEYORS & PLANNERS
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56B ORCHARD BEACH BLVD
PORT WASHINGTON, NY 11050
SCOTT T. REGAN
10509 TWIN CEDAR COURT
LAUREL, MARYLAND 20723
KELLY R. REGAN
12859 ROUTE 108
HIGHLAND, MARYLAND 20777
301.672.4820

PROJECT: REGAN PROPERTY
LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E', A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAN No. 22601-22604
LOCATION:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200
5th ELECTION DISTRICT
HOWARD COUNTY, MARYLAND
DPZ No.: SP-12-004, ECP-12-045, WP-13-025
TITLE: PLATE ARCH CULVERT CROSSING #1
HEADWALL/WINGWALL FOOTING LAYOUT AND ELEVATIONS
DATE: APRIL 4, 2014
PROJECT NO.: 2171
DESIGN: WKR
DRAFT: WKR
SCALE: AS SHOWN
DRAWING: 25 OF 33



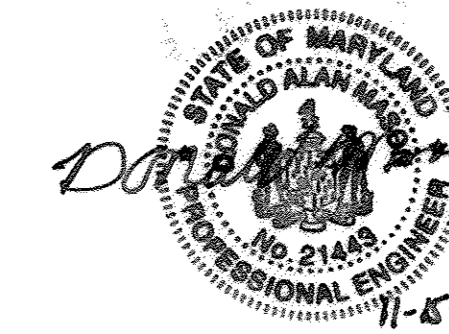
HOWARD COUNTY CHAINLINK FENCE DETAIL FOR FENCE CONSTRUCTION ALONG WINWALLS AND HEADWALLS

SCALE: NS

TYPICAL WALL SECTION - SEE CHART FOR DIMENSIONS

SCALE: NS

NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET



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License No. 21443, Expiration Date: 12-21-19

SECTION	MAX. HEIGHT H	VERTICAL REINFORCING A	HORIZONTAL REINFORCING B	SECTION 1 BEAM ADDITIONAL STEEL C	FOOTING STEEL D	FOOTING STEEL E	WALL WIDTH F	FOOTING THICKNESS G	FOOTING TOE H	FOOTING HEEL I	FOOTING TOTAL LENGTH J	WALL MAX HEIGHT K
1	-	#5 @ 12" O.C.	#6 @ 12" O.C.	5 - #6 @ 52"	-	-	18"	-	-	-	-	4'
2	17'-6"	#6 @ 6" O.C.	#6 @ 12" O.C.	-	#5 @ 6" O.C.	#5 @ 12" O.C.	18"	18"	6'	4'	10'	17'-6"
3	17'-6"	#6 @ 6" O.C.	#5 @ 12" O.C.	-	#5 @ 6" O.C.	#5 @ 12" O.C.	18"	18"	4'	4'	8'	17'-6"
4	7'-6"	#6 @ 12" O.C.	#5 @ 12" O.C.	-	#5 @ 12" O.C.	#5 @ 12" O.C.	18"	12"	1'	3'	4'	7'-6"
PLATE ARCH	-	-	-	-	#7 @ 12" O.C.	#5 @ 12" O.C.	-	2.5'	6'	6'	12'	-

HEADWALL & WINGWALL & PLATE ARCH DIMENSIONS & REINFORCING DETAILS CHART

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APPROVED: DEPARTMENT OF PUBLIC WORKS
W. R. ... 4-29-14
CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
V. ... 5-21-14
CHIEF, DIVISION OF LAND DEVELOPMENT

CHIEF, DEVELOPMENT ENGINEERING DIVISION
A. ... 5-9-14

DRAWN BY: WKR
DESIGN BY: WKR
CHECKED BY: WKR
DATE: 03/27/2014

CLIENT: LANE/LSB&C
OWNER: MB Highland Reserve, LLC
JOB No: 1101-14-03

DO NOT SCALE THIS DRAWING. DIMENSIONS AND NOTES HAVE PRECEDENCE OVER DRAWINGS

Ryan & Associates
A Division of WKR Consulting Inc.
SPECIALIZING IN STRUCTURAL & GEOTECHNICAL ENGINEERING
e-mail: info@ryanandassociates.net

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LANE LONGSPAN

LANE ENTERPRISES, INC. - LSB&C DIVISION
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LANE: 724-652-7747
LSB&C: 888-949-5722

OWNER/DEVELOPER:
RONALD R. REGAN
56B ORCHARD BEACH BLVD
PORT WASHINGTON, NY 11050

PROJECT: REGAN PROPERTY
LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL "A", and NON-BUILDABLE PRESERVATION PARCELS "B" thru "E", A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL "A", PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

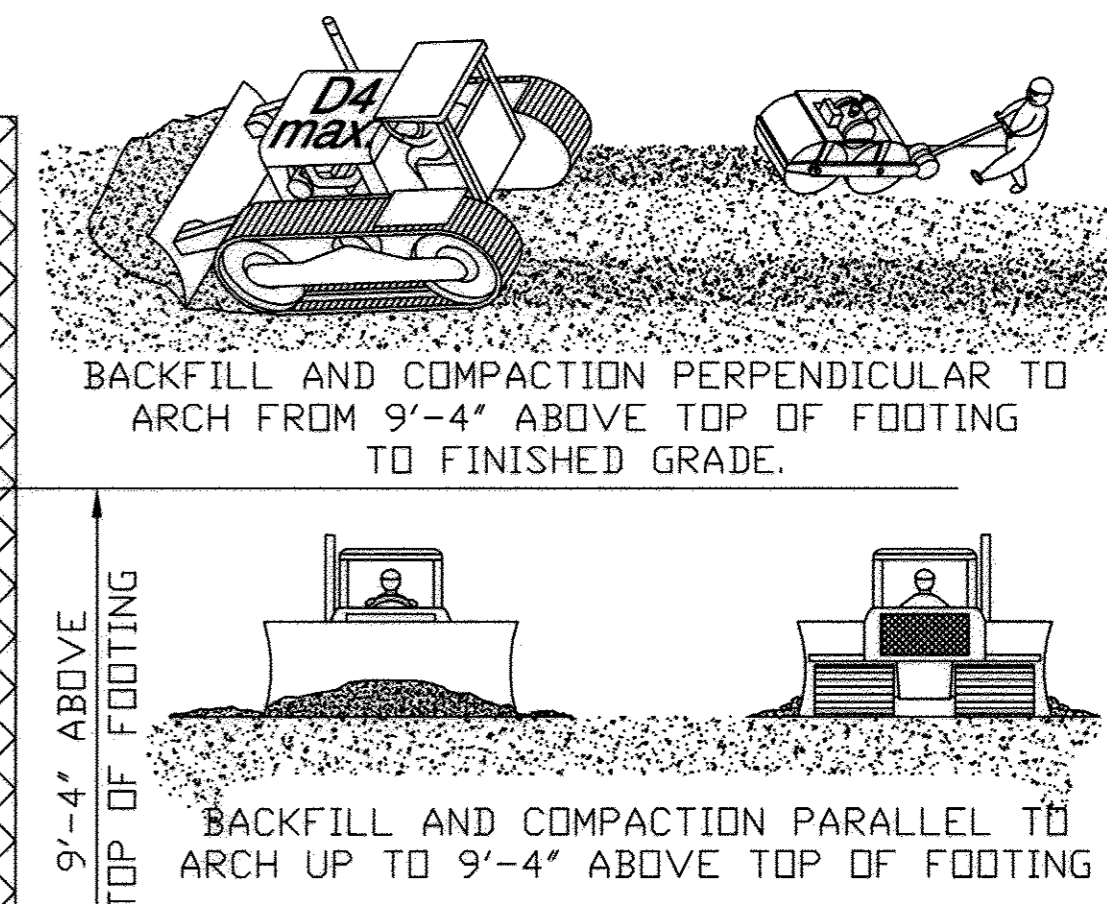
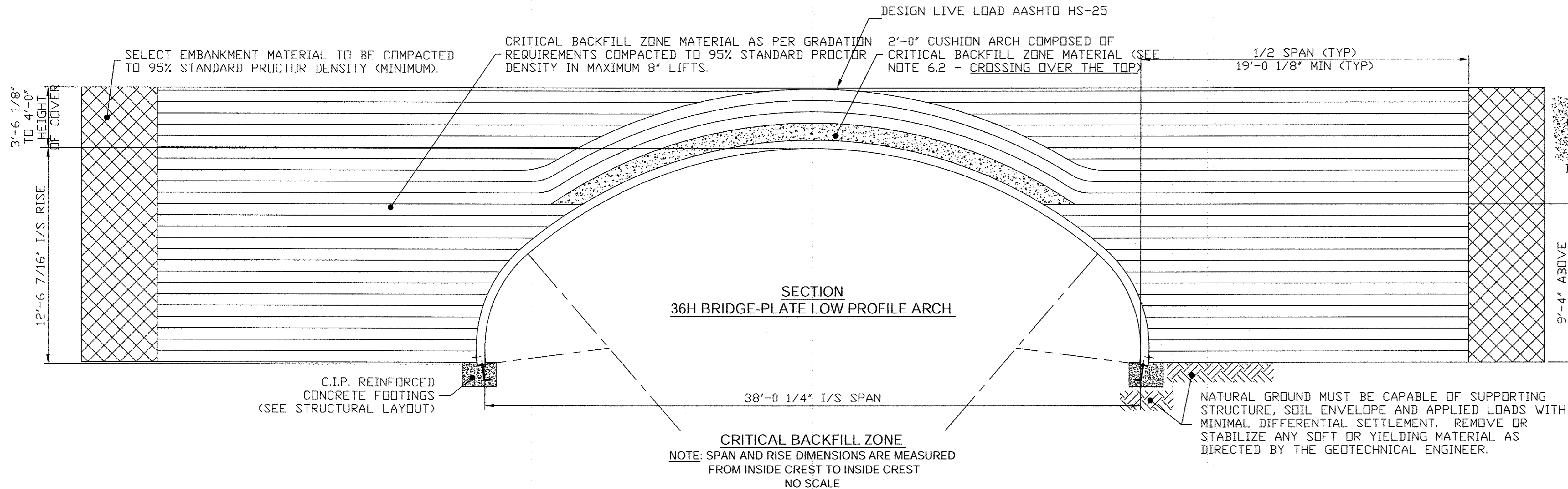
LOCATION:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200
5th ELECTION DISTRICT
HOWARD COUNTY, MARYLAND
DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE: PLATE ARCH CULVERT CROSSING #1
ARMTEC PLATE DETAILS, WALL & FOOTINGS GEOMETRY & DETAILS

DATE: APRIL 4, 2014
PROJECT NO. 2171

DESIGN: WKR
DRAFT: WKR
SCALE: AS SHOWN
DRAWING 26 OF 33

AS-BUILT



- NOTES:**
- ALL DIMENSIONS IN FT-IN (IMPERIAL UNITS) UNLESS NOTED
 - BRIDGE-PLATE IS MANUFACTURED IN METRIC UNITS [mm]

INSTALLATION INSTRUCTIONS

THESE INSTALLATION INSTRUCTIONS ARE INTENDED TO BE USED IN CONJUNCTION WITH THE ENGINEER'S SPECIFICATIONS AND ARE NOT TO SUPERSEDE THEM.

1.0 ASSEMBLY OF STRUCTURE

- THE BRIDGE PLATE SHALL BE ASSEMBLED IN ACCORDANCE WITH PLATE ASSEMBLY DRAWING AND INSTALLATION INSTRUCTIONS PROVIDED.
- BOLTING MUST BE DONE WITH THE CURVED SURFACE OF THE NUT AGAINST THE PLATE.
- BEFORE BACKFILLING, ALL BOLTS SHALL BE TIGHTENED TO A TORQUE OF 150 FT-LBS MINIMUM, 250 FT-lbs MAXIMUM.

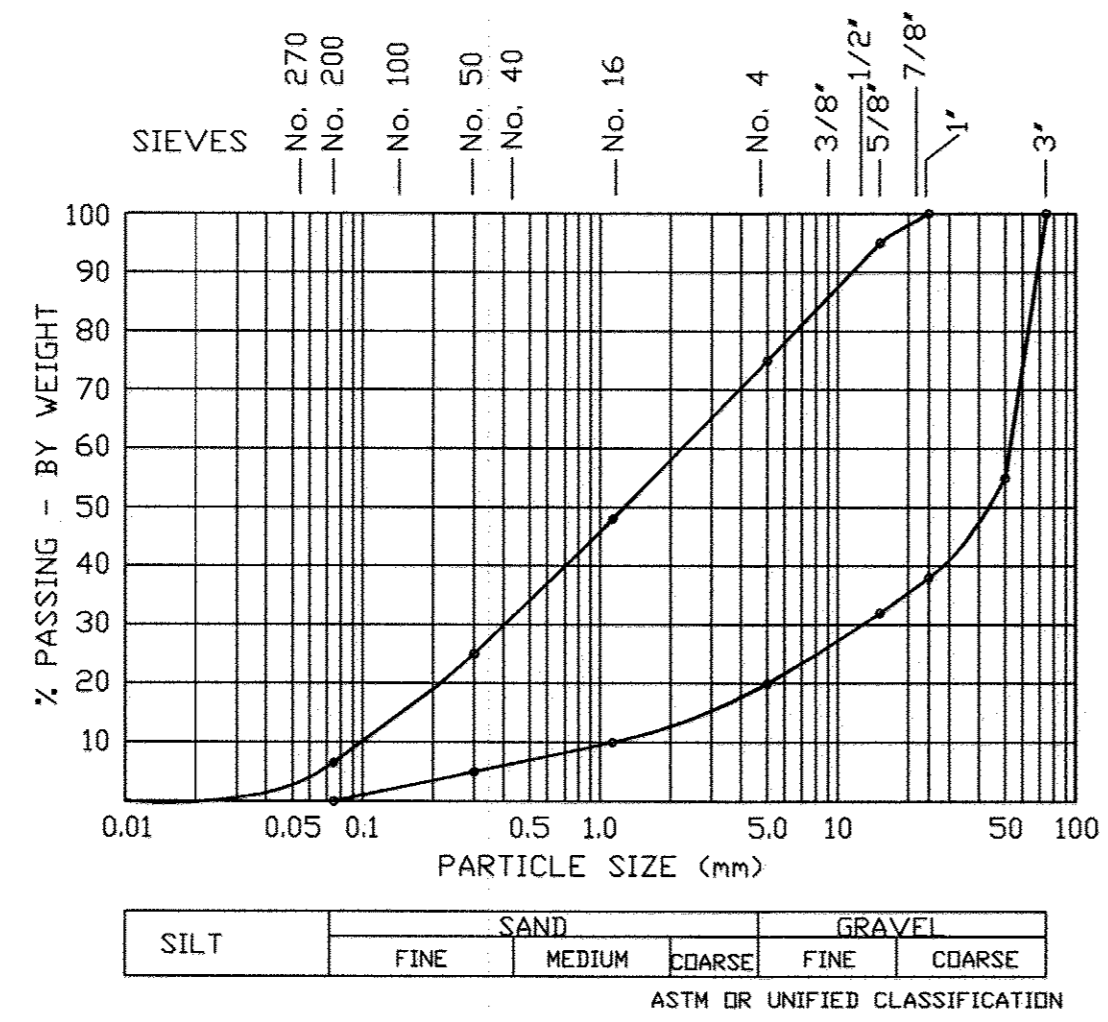
2.0 DIMENSIONAL CHECK OF STRUCTURE

- DURING PLATE ASSEMBLY, THE DIMENSIONS OF THE STRUCTURE MUST BE CHECKED AND RECORDED. ANY VARIATIONS SHALL BE WITHIN RECOMMENDED ARMTEC TOLERANCES.
- A REGULAR CHECK IS TO BE MADE OF THE LINE AND DIMENSION OF THE STRUCTURE WHILE BACKFILLING PROCEEDS. THIS CHECK SHALL BE USED AS AN EVALUATION OF THE INFLUENCE OF THE COMPACTIVE EFFORTS ON THE SHAPE OF THE STRUCTURE. THE PLUMB BOB METHOD OF MONITORING IS RECOMMENDED FOR THIS CHECKING PROCEDURE. COMPACTIVE EFFORTS MAY INCREASE THE RISE OF THE STRUCTURE.

3.0 BACKFILLING

MATERIAL - BACKFILL GRADATION SPECIFICATIONS

- MATERIAL IN THE CRITICAL BACKFILL ZONE SHALL BE GRANULAR WITH ANGULAR GRAINS. SIEVE ANALYSIS FOR THIS MATERIAL MUST FALL WITHIN THE GRADATION ENVELOPE SHOWN BELOW. MATERIAL MUST BE WELL GRADED. MAXIMUM PARTICLE SIZE IS NOT TO EXCEED 3\"/>
- ph OF 5 TO 10 RESISTIVITY NOT LESS THAN 3000 OHM-CMS. CHLORIDES NOT GREATER THAN 100 ppm. SULPHATES NOT GREATER THAN 200 ppm.



3.3 MATERIAL OUTSIDE THE CRITICAL BACKFILL ZONE SHALL BE SELECT EMBANKMENT MATERIAL COMPACTED TO 95% STANDARD PROCTOR DENSITY (MINIMUM).

4.0 SPREADING

- HEAVY EQUIPMENT SHALL OPERATE ONLY AS CLOSE TO THE STRUCTURE AS IS ALLOWED BY THE ENGINEER (AS ADVISED BY THE ARMTEC REPRESENTATIVE).
- MATERIAL IS NOT TO BE DUMPED ON TOP OF THE STRUCTURE BUT SHALL BE DUMPED ON EITHER SIDE AND THEN SPREAD IN LAYERS SUITABLE FOR THE TYPE OF COMPACTION EQUIPMENT BEING USED. TRUCK END-DUMPING OR DOZER PLACEMENT AGAINST THE SIDE OF THE STRUCTURE IS ABSOLUTELY NOT PERMITTED.
- FILL SHALL BE PLACED IN LAYERS WHICH SHALL NOT EXCEED 8\"/>
- TRUCKS CAN UNLOAD IN ROUGH LAYERS STARTING NO CLOSER THAN 5\"/>
- FILL DEPTH SHALL BE MAINTAINED APPROXIMATELY EQUAL ON EACH SIDE OF THE STRUCTURE AT ALL TIMES. THE MAXIMUM DIFFERENCE IN ELEVATION SHALL BE 1'-4\"/>

5.0 COMPACTION

- AREAS CLOSE TO THE STRUCTURE SHALL BE COMPACTED USING VIBRATING OR TAMPING EQUIPMENT RUNNING PARALLEL WITH THE LENGTH OF THE STRUCTURE AT ALL TIMES.
- GRANULAR FILL MATERIAL SHALL BE AT OPTIMUM MOISTURE CONTENT DURING COMPACTION.
- EACH FILL LAYER SHALL BE COMPACTED TO A MINIMUM OF 95% STANDARD PROCTOR DENSITY.

6.0 CROSSING OVER THE TOP

- WHEN THE BACKFILL REACHES AN ELEVATION OF 9'-4\"/>
- THE INITIAL COVERING OF THE TOP IS THE MOST CRITICAL LOADING SITUATION AND MUST BE PERFORMED IN THE PRESENCE OF THE ENGINEER OR AN AUTHORIZED REPRESENTATIVE. 1' OF BACKFILL MUST EXIST AT ALL TIMES BETWEEN THE EQUIPMENT AND THE STRUCTURE. THIS 1' COVER (CUSHION ARCH) MUST BE BUILT UP EVENLY FROM BOTH SIDES. THE EQUIPMENT USED SHALL NOT BE HEAVIER THAN A D-4 CATERPILLAR DOZER (16 000 lbs) FOR SPREADING MATERIAL AND NOT HEAVIER THAN A BUFFALO-BOMAG BW-75S FOR COMPACTION.
- FOR COVER GREATER THAN 1'-8\"/>
- DESIGN VEHICLES/HIGHWAY TRAFFIC MAY ONLY CROSS OVER THE STRUCTURE AFTER FILL IS PLACED AND COMPACTED TO FINISHED HEIGHT OF COVER AS SHOWN ABOVE.
- IF THE BACKFILL IS NOT TO BE PLACED IMMEDIATELY TO THE FINISHED ROAD ELEVATION, A WEARING AND TRAVELLING SURFACE IS TO BE BUILT OVER THE CRITICAL BACKFILL ZONE IN ORDER TO KEEP THE LATTER OPERATIVE AND ADEQUATE TO PERFORM ITS FUNCTION AS A SAFE STRUCTURE AT ALL TIMES.

APPROVED: DEPARTMENT OF PUBLIC WORKS
 [Signature] 4-29-14
 CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 [Signature] 5-21-14
 CHIEF, DIVISION OF LAND DEVELOPMENT

[Signature] 5-9-14
 CHIEF, DEVELOPMENT ENGINEERING DIVISION

DRAWN BY: WKR
 DESIGN BY: WKR
 CHECKED BY: WKR
 DATE: 03/27/2014

CLIENT: LANE/LSB&C
 OWNER: MB Highland Reserve, LLC
 JOB No: 1101-14-03

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NO AS BUILT INFORMATION IS PROVIDED ON THIS SHEET

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443 Expiration Date: 12-31-19

BENCHMARK ENGINEERING, INC.
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 75 THOMAS JOHNSON DRIVE A SUITE E A FREDERICK, MARYLAND 21702
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 WWW.BE-ENGINEERING.COM

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 License No. 21586, Expiration Date: 5-9-2015.

NO.	DATE	REVISION

OWNER/DEVELOPER:
 RONALD R. REGAN
 56B ORCHARD BEACH BLVD
 PORT WASHINGTON, NY 11050

PROJECT: **REGAN PROPERTY**
 LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL A PREVIOUSLY RECORDED AS PLAT NO. 22001-252694

LOCATION:
 TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 5th ELECTION DISTRICT
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE: **PLATE ARCH CULVERT CROSSING #1**
 ARMTEC PLATE SPECIFICATIONS

DATE: APRIL 4, 2014 PROJECT NO. 2171
 SCALE: AS SHOWN DRAWING 27 OF 33

STRUCTURAL NOTES & SPECIFICATIONS

1: DEFINITIONS

- 1.01 Owner: Regan Family
- 1.02 Contractor: TBD
- 1.03 Design Structural Engineer: Ryan & Associates, Frederick Office, MD 21740
- 1.04 Site Civil Engineer: Benchmark Engineering, Inc.
- 1.05 Site Geotechnical Investigation Engineer: Geolab, Inc.

If any of the above responsibilities change it is the owner's responsibility to notify LSBC/LANE prior to the start of the work. It is the owner's responsibility to make sure all parties listed above are aware of their roles, requirements, responsibilities and final submittals.

- 1.06 Reference Standards
 - A. ASTM 3034- Specification for Polyvinyl Chloride (PVC) Plastic Pipe
 - B. ASTM C 140- Sampling and Testing Concrete Masonry Units and related units
 - C. ASTM D 422- Gradation of Soils
 - D. ASTM D 698- (AASHTO T99) Standard Test Methods for Laboratory Compaction Characteristics of Soil using Standard Effort
 - E. ASTM D 1248- Polyethylene Plastics Extrusion Materials for wire and Cable
 - F. ASTM D 1557- (AASHTO T 180) Standard Test Method for Laboratory Compaction Characteristics of Soil using Modified Effort
 - G. ASTM D 1586- Standard Test Method for Penetration Test and Split-Barrel Sampling of Soils
 - H. ASTM D 2166- Unconfined Compressive Strength of Cohesive Soil
 - I. ASTM D 2487- Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System)
 - J. ASTM D 3080- Direct Shear Test of Soils Under Consolidated Drained Conditions
 - K. ASTM D 4318- Liquid Limit, Plastic Limit and Plasticity Index of Soils
 - L. ASTM D 2850- Unconsolidated, Undrained Compressive Strength of Cohesive Soils in Triaxial Compression
 - M. ASTM A 615- Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement
 - N. ACI 318-02- Concrete Building Design and Construction
 - O. ACI 315-99 "Manual of Standard Practice for Detailing Reinforcing Concrete Structures"
 - P. AASHTO LRFD Bridge Design Specifications, 8th Edition, 2012
 - Q. AASHTO Standard Specifications For Highway Bridges, 17th Edition, 2002
 - R. AISI Handbook of Steel Drainage & Highway Construction Products, 1994 Edition

2: GENERAL

- 2.01 Contractor shall notify LSB&C/LANE and the oversite review engineer - RA (Engineer of Record - EOR) responsible for construction certification of any discrepancies, omissions, or conflicts between the various elements of the working drawings and/or specifications before proceeding with any work involved. In all cases, unless otherwise directed by the engineer in writing, the most stringent requirements shall govern and be performed.
- 2.02 Contractor shall verify all conditions, dimensions and elevations, etc., at the site and shall coordinate work performed by all trades. Do not scale drawings.
- 2.03 Shop drawings shall be reviewed and approved by the structural engineer (RA) prior to fabrication.
- 2.04 Sizes, locations, loads, and anchorage of equipment shall be verified in the field with equipment manufacturers (suppliers) prior to fabrication or installation of supporting structures.
- 2.05 Temporary bracing designed by a professional bracing/structural engineer shall be provided wherever necessary to take care of all loads to which the structure may be subjected, including wind. Such bracing shall be left in place as long as may be required for safety or until all the structure elements are complete, as determined by the bracing engineer.
- 2.06 During and after construction the contractor and/or owner shall keep loads on the structure within the limits of the design load until turned over to the county or final owner, then the load restrictions shall be enforced by that entity. Maximum loading is HS 25 as determined by AASHTO.
- 2.07 Contractor shall be responsible for safety and protection within and adjacent to the job site.
- 2.08 Civil engineer is responsible for erosion and sediment control design.
- 2.09 Road pavement design and its appurtenance structure are civil engineer's responsibility. Refer to the civil drawings for all pavement and roadway drainage system information and questions thereby to the civil engineer.

3: CONCRETE

- 3.01 All foundation concrete (footings, walls etc.) shall be normal weight concrete with a compressive strength equal to at least 4,000 psi within 28 days after casting. The water/cement ratio shall be no greater than 0.50 and slump shall be 2-4 inches.
- 3.02 All concrete work shall be placed, cured, stripped and protected as directed by these specifications and ACI standards and practices.
- 3.03 Contractor is responsible for all shoring and formwork.
- 3.04 Concrete design and detailing shall conform to the requirements of ACI 318-08. Contractor shall submit mix designs to the oversite review engineer accompanied by appropriate graphs and background data for approval. Mix design shall indicate 7 and 28-day strengths, cement content, air content, water-cement ratio, amount of fine and coarse aggregates and admixtures.
 - a. Minimum ultimate compressive strength of concrete at 28 days shall be as follows, unless noted otherwise:
 - Footings, Stiffeners and Walls concrete: 4000 PSI
 - Unreinforced concrete: 2000 PSI
- 3.05 All exterior concrete and concrete exposed to weather shall be air-entrained. (All concrete used on this project)
- 3.06 Use of additives shall not be permitted unless specifically approved by the oversite review engineer.
- 3.07 The concrete subcontractor shall not reproduce any portion of the structural contract drawings for utilization as shop drawings.
- 3.08 Concrete shall be consolidated by means of mechanical vibration. Vibrators shall be inserted and removed vertically at regular intervals not to exceed 18" to ensure uniform consolidation. In no case shall vibrators be used to transport the concrete inside the forms.

3.09 Formwork shall follow ACI 347 "Recommended practice for concrete form work". Forms shall conform to the working drawing to shape, line and dimensions members and shall be substantially free from surface defects and sufficiently tight to prevent leakage. They shall be properly braced and tied to maintain position and shape.

3.10 Fresh concrete will be protected from rain, flowing water and mechanical injury, sun, drying winds and freezing for a period of 7 days. The temperature of the concrete must be kept above 50 degrees F for at least 7 days.

3.11 Cure concrete in accordance with ACI Committee 308 "Guide to curing concrete". The minimum cure time for footings, stiffeners and retaining walls is seven (7) days (depends on concrete mix design). Use approved curing compound to keep concrete moist to achieve specified strength of 4,000psi. Keep forms in place for seven (7) days. Verify strength with seven (7) day break test to confirm concrete will achieve required strength.

3.12 Ground water and surface water within the subgrade excavation area must be maintained below the bottoms of the footer elevation and the bottoms of the excavation during preparation of the subgrade.

3.13 No expansion or contraction joints are required unless otherwise noted in these plans.

3.14 Lean concrete for scour protection can be poured in sections both vertically and horizontally as necessary.

4: GEOTECHNICAL NOTES

- 4.01 Geotechnical site information provided by GeoLab, Inc..
- 4.02 All structural fill soils will have a minimum dry density of 135PCF unless indicated otherwise on this drawing set. Fill shall be compacted to at least 95% of the maximum dry density as determined by the standard proctor ASTM D698 (AASHTO T-99) with the exception of the top foot, which will be 100% of the maximum dry density.
- 4.03 All structural fill material will be placed in layers, which, before compaction, will not exceed eight inches. Each layer shall be spread to ensure conformity of materials in each layer.
- 4.04 Virgin/undisturbed soils are defined as soils with a minimum SPT "N" value of 12.

5: FOOTINGS

- 5.01 All footings are based on an allowable soil bearing pressure of 4,000 PSF for retaining wall and 4000 PSF for Arch. Any soil condition encountered during excavation that is contrary to those used for design of footings as outlined in these drawings shall be brought to the attention of the EOR construction oversight engineer for direction before proceeding.
- 5.02 Bottom of footings shall be a minimum of 30" below finished grade, unless a lower elevation is noted. Footing elevations noted are estimated based on available geotechnical and grading information. All footings adjacent to existing footings shall be lowered to match existing footing elevation.
- 5.03 All foundation subgrades shall be inspected and approved under the supervision of the RA EOR or their representative prior to pouring concrete. Footings may be lowered to achieve the minimum footing subgrade bearing capacity of 5,000/7,500 PSF. Undercut footing sub-grade as necessary to achieve 5,000/7,500 psf bearing capacity and fill with unreinforced concrete or stone subgrade in accordance with section 7.05 of these specifications.

6: REINFORCING STEEL

- 6.01 Reinforcing bars shall be deformed billet steel conforming to ASTM A615, grade 60. All welded wire fabric shall conform to ASTM A185. Bars shall be branded by the manufacturer with bar size and grade of steel and certified mill reports shall be submitted to oversite review engineer for approval and record. Reinforcing steel shall be detailed in accordance with the ACI 315-99 "Manual of Standard Practice for Detailing Reinforcing Concrete Structures", latest edition. Provide corner bars at junctions of concrete walls and wall footings and lap 48 x bar diameters.
- 6.02 With wall reinforcing as shown in typical details, size and spacing of corner bars to be same as horizontal wall reinforcing, unless shown otherwise. Where continuous bars are called for, they shall run continuously around corners and lapped as necessary min. 48 x bar diameters. Provide standard hooks at discontinuous ends. Tension and compression lap splices shall not be less than the splice lengths as given in ACI 318. Generally lap top bars at mid span and bottom bars at supports. Provide placing accessories in accordance with ACI recommendations.
- 6.03 Provide the following minimum concrete cover for reinforcement:
 - a. Concrete cast against and permanently exposed to earth ...3"
 - b. Concrete exposed to earth or weather:
 - No. 6 through No. 18 bars ...2"
 - No. 5 bar, W31 or D31 wire, and smaller ...2"
 - c. Concrete slabs, walls and joists not exposed to the earth or weather:
 - No. 14 and No. 18 bars ...1 1/2"
 - No. 11 bar and smaller ...3/4"
 - d. Beams, columns:
 - Primary reinforcement, ties, stirrups and spirals ... 2"

7: EARTHWORK SPECIFICATIONS

- 7.01 The contractor shall furnish all labor, material and equipment for the earthwork. The contractor shall perform all work and services except those set out and furnished by Long Span Bridge & Culvert, LLC. (LSBC)/LANE.
- 7.02 This work shall consist of all clearing and grading, preparation of the land to be filed, filing of the land, spreading and compaction of the fill, and all subsidiary work necessary to complete the grading of the cut and fill areas to conform with the project lines, grades, slopes and specifications.
- 7.03 This work is to be accomplished under the observation of the oversite review engineer or their representative. Placement of the backfill material will not be permitted unless the RA EOR or their representative is on site.
- 7.04 Prior to bidding the work, the contractor shall examine, investigate and inspect the construction site as to the nature and location of the work and local conditions at the construction site including, without limitation, the character of the surface or subsurface conditions and obstacles to be encountered on and around the construction site; and shall make such additional investigation as they may deem necessary for the planning and proper execution of the work.
- 7.05 The EOR or their representative in the field shall verify the subgrade soil condition, gravel, and the rock quality. All stone subgrade shall be compacted with a vibratory plate compactor in no more than 8" lifts and verified by the EOR or their representative.
- 7.06 If conditions other than those indicated by the confirmatory subsurface boring program are encountered by the contractor, Long Span Bridge & Culvert, LLC (LSBC/LANE) and RA should be notified immediately. The material, which the contractor believes to be a changed condition, should not be disturbed so that RA, LSBC/LANE and/or their designated representative can investigate the condition.
- 7.07 The work for clearing and grubbing includes furnishing all labor, materials, transportation, supervision, tools and construction machinery, which may be necessary to accomplish the clearing and grubbing for this project area.
- 7.08 All trees, bushes, etc., shall be removed from the limits of the proposed areas to receive fill or other engineered structures. The areas may be extended outside the actual lines of construction only to the distance required to provide the contractor with sufficient space to perform the work.
- 7.09 All stumps, vegetation, brush, debris or deleterious materials shall be removed from the limits of the fill or other engineered structures.

7.10 The work for stripping includes furnishing all labor, materials, transportation, supervision, tools and construction machinery, which may be necessary to be provided by the contractor.

7.11 When the construction/operation sequence requires, the area of fill or other engineered structures shall be properly stripped. This stripping shall include topsoil and other deleterious materials. Topsoil shall be removed to its full depth and stockpiled for use in the final cover. Any rubbish, organic and objectionable soils and other deleterious material shall be properly disposed of at a site approved by owner or LSBC/LANE.

7.12 The lines and grades shall be established by using control benchmarks provided by licensed surveyors.

7.13 Soft or spongy cohesive or silty materials encountered at the base of the excavation shall be removed at the direction of the EOR or their representative. The excavation for the footing wall foundations shall be observed and subgrade-bearing capacity certified by the EOR upon completion of this task. At the direction of the EOR or their representative, soft material will be removed to a depth directed by the EOR or their representative, and replaced with granular backfill compacted at least 100% of the maximum dry unit weight density at a moisture content within 2% of optimum as determined by AASHTO T-99 method / ASTM D698.

7.14 No select granular backfill may be placed, without being observed by LSBC/LANE's/RA's shape control technician.

7.15 Ground water and surface water within the subgrade excavation area must be maintained at least 3 feet below the footer elevation during preparation of the subgrade. If additional excavation is required to remove unsuitable materials, the water must be maintained 3 feet below the deepest excavation elevation.

7.16 The subgrade shall be compacted with a soil vibratory compactor or equivalent with a dynamic force of 50,000 pounds (min.). The top 1 foot of the subgrade soil shall be compacted to at least 100% of the maximum dry unit weight at a moisture content within 2% of optimum as determined by AASHTO T-99 method (standard proctor). All compaction and subgrade bearing capacity to be verified by the site geotechnical engineer or representative.

7.17 All select granular backfill material around the culvert and above the footing shall consist of AASHTO M 145 A-1-a. Recycled concrete material shall not be allowed. The select backfill material shall have fines (pass no. 200 sieve material) maximum 15% by weight. See typical select backfill chart this sheet.

7.18 The select granular backfill material and site soil backfill for the adjoining embankment material shall be tested in the laboratory for grain size distribution (AASHTO T-27 for granular material; AASHTO T-88 for soil material) and moisture-density relationship (AASHTO T-99). The testing described above is for purposes of verification of site soil backfill parameters and is in addition to the general project specifications for the embankment backfill, but does not supersede project specifications that may be more stringent.

7.19 All backfill operations shall place the material evenly on both sides of the plate arch and each lift shall extend for the entire length of the plate arch prior to placement of the next sequential lift. Fill placement shall begin in the middle of the plate arch length and extend equally on both sides in the upstream and downstream directions.

7.20 The select granular backfill shall be placed in horizontal layers not to exceed 8" loose depth. The lift thickness may be reduced by the EOR or their representative to obtain the required compaction, fill all the voids, achieve the proper seating of the backfill material and achieve the stability of the backfill material and the plate arch. The granular backfill shall be compacted to 95% of the maximum dry unit weight as determined by the standard proctor test (AASHTO T-99). Greater emphasis shall be given to a uniform degree of compaction throughout each lift than to achieving a degree of compaction greater than the minimum specified criteria. The site Geotechnical Engineer shall do testing of select granular backfill.

7.21 All granular material shall be compacted using mechanical devices, vibrating plates or other equipment approved by the EOR. Compaction equipment weighing more than 24,000 pounds shall not be used within 2.5' of the corrugated metal structure. The compaction equipment shall be capable of compacting the material under the haunch of the plate arch (I.E.; below the spring line of the plate arch).

7.22 The soil backfill (compacted normal backfill) within 32'-0" or to natural undisturbed embankment backfill on each side shall be placed in layers not to exceed 8" loose depth. The lift thickness may be reduced by the Site Geotechnical Engineer to obtain the required compaction. The soil backfill shall be compacted to a minimum of 95% of the maximum dry unit weight as determined by the standard proctor test (AASHTO T-99) and to a moisture content within 2% of the optimum moisture content as determined by the same test. Field nuclear density test shall be performed at a minimum frequency of four tests per every other lift and every 25' on the soil backfill on each side of the structure. The testing described above is in addition to the general project specifications for the embankment backfill, but does not supersede project specifications that may be more stringent than those requirements. The Site Geotechnical Engineer is responsible for testing and recording measurements of the soil backfill.

7.23 If at any time longitudinal cracks develop in the backfill surrounding the pipe to a distance of 30' from the spring line of the plate arch, these features must be brought to the immediate attention of the field QA/QC personnel and the EOR.

7.24 While compacting granular backfill material with a vibrator compactor and adjacent to the plate arch, the opposite side of the plate arch should be observed to note if vibrations are loosening the granular material on that side. This may be more prevalent at higher elevations of the backfill with respect to the plate arch. If this condition occurs, the field QA/QC technician and EOR should be notified prior to placement of a sequential lift on either side.

7.25 The structure should not be crossed with equipment heavier than a D4 dozer. No other equipment or highway (HS25) loading shall be allowed to cross the structure until the asphalt pavement is placed unless there is a minimum of 12" of soil cover or span/8 inches of soil cover whichever is greater, covering the plate arch. Top filling should begin at the middle of the structure (lengthwise) with the backfill being pushed up and over the structure with a D4 or preferably smaller type dozer. The fill should be pushed over the structure in a manner 45 to 90 degrees to the axis of the structure. Density test for structural fill are not required as long as the A-1-a material is vibrated with a vibratory smooth drum roller and moisture is +/-2% of optimum of a standard proctor maximum dry density. Density tests for regular soil backfill shall be performed at a minimum frequency of four tests per every lift on the soil backfill on each side of the structure. The testing described above is in addition to the general project specifications for the embankment backfill, but does not supersede project specifications that may be more stringent than those requirements. The contractor shall submit to the owner samples of all proposed soil backfill material for laboratory testing to verify moisture and density relationships (AASHTO T-99/ASTM D698) and grain size relationships (AASHTO T-27/ASTM C136).

7.26 All construction to be certified at the end of the job by a Professional Structural/Geotechnical Engineer (oversite review engineer - the EOR) qualified in the design and construction of plate arch culverts (minimum 10 years experience) that all work performed by contractor meets these design requirements and specifications. Certification to be submitted to LSBC/LANE, RA and the local jurisdiction for record file.

8: REQUIRED SUBMITTALS

- 8.01 The contractor must submit the following items to the oversite review engineer for approval in writing at least 2 weeks prior to use:
 - a. Manufacturer certification for yield strength of reinforcing steel.
 - b. Manufacturer certification for concrete design.
 - c. Shop drawings of all concrete work.
 - d. Plate arch shop drawings.

9: DEWATERING REQUIREMENTS

9.01 Dewater footing excavations using sump pumps or well points as required. Footing excavation must be dewatered and maintained that way for a minimum of seven days or concrete strength of 3,000psi has been reached, whichever is greater

10: CONSTRUCTION OVERSIGHT CERTIFICATIONS

- 9.01 The plate arch construction requires engineering oversight and inspection. The oversite reviewer/EOR, Civil, Structural and Geotechnical Engineers must provide LSBC/LANE certification reports of all footings and retaining wall/headwalls reinforcing placement and the following items:
 - a. Subgrade bearing capacity and backfill (select granular and compacted normal backfill) compaction testing, field reports, testing results, testing locations, and registered professional engineer's certification.
 - b. Field reports of concrete placement review, laboratory test results of concrete cylinder breaks at 7 and 28 days and certified by a Registered Professional Structural Engineer.
 - c. Final report of construction certification that the construction was performed in accordance with the design and the material testing and inspection verifying the same, stamped by a Registered Professional Structural/Geotechnical Engineer.

11: ENVIRONMENTAL PERMITTING

10.01 Environmental permitting must be addressed and applied for with the State (MDE) and Army Corp of Engineers, as required.

12: SAFETY

11.01 All contractors (and owners), their representatives and their crew must be qualified/certified to perform all works within their scope. They must adhere to OSHA's health and safety laws. The General Contractor is solely responsible for all site safety.

13: RA'S RESPONSIBILITY

12.01 RA's scope of work for this project are design of plate arch (minimum steel thickness stress analysis), footings, headwalls and wingwalls. Engineering Construction Review of Arch & Walls installation, footings, steel & concrete forming, and structural backfill placement is required. RA may be contracted for this work also. Acceptance of the plan drawings by our client & the owner means they agree to our scope and responsibilities.

14: LONG SPAN BRIDGE & CULVERT, LLC/LANE SCOPE OF WORK

13.01 Long Span Bridge & Culvert, LLC (LSBC)/LANE will deliver, furnish and assemble the Long Span low profile arch on footings designed by Ryan & Associates and prepared by Site Contractor. The base channel will be furnished by LSBC and installed in the concrete foundations by the Site Contractor in accordance with the plans. Structural plate for this job to be 7ga (.187" thick) steel.

13.02 LSBC/LANE will conduct a pre-construction meeting prior to foundation preparation and arch assembly. Attendance at the pre-construction meeting is mandatory for the owner or the owner's representative (e.g. Site Civil Engineer, Site Contractor and Concrete Contractor) and the oversite review Structural/Geotechnical Engineer. It is the owner's responsibility to have each party in attendance. If a party is not in attendance it is the owner's responsibility to inform that entity of its responsibilities and duties prior to the start of work.

13.03 LSBC/LANE/RA will provide a shape control technician to monitor structure's shape and observe the proper placement and compaction of the select fill material, unless provided otherwise and approved in writing by LSBC/LANE/RA.

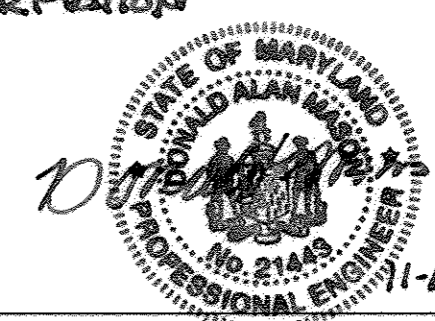
13.04 LSBC/LANE will require the Site Contractor to unload the structure plates and base channel. LSBC/LANE will require the Site Contractor to provide access to the structure for a rubber-tired crane. Parallel access roads shall be within 30' of the centerline of the structure on each side.

BACKFILL CHART

AASHTO M 145- TABLE 2 (MODIFIED*)				
GROUP CLASSIFICATION	A-1		A-2 (MODIFIED)	
SIEVE ANALYSIS, PERCENT PASSING	A-1-a	A-1-b	A-2-4	A-2-5
NO. 10 (2.00 mm)	50 max	----	----	----
NO. 40 (.425 mm)	30 max	50 max	----	----
NO. 100 (.150 mm)	----	----	50 max	50 max
NO. 200 (.075 mm)	15 max	25 max	20 max	20 max
Characteristics of fraction passing No. 40 (0.425 mm)				
Liquid Limit	----	----	40 max	41 max
Plasticity Index	6 max	6 max	10 max	10 max
Usual Material Types	Stone Fragments Gravel and Sand		Silty or Clayey Gravel and Sand	

*Modified to be more select than M-145

NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443, Expiration Date: 12-31-16

NO.	DATE	REVISION
<p>BENCHMARK ENGINEERING, INC. ENGINEERS • LAND SURVEYORS • PLANNERS</p> <p>8480 BELMONT NATIONAL PIKE • SUITE 315 • ELICOTT CITY, MARYLAND 21043 (P) 410-465-6105 (F) 410-465-6644</p> <p>75 THOMAS JOHNSON DRIVE • SUITE E • FREDERICK, MARYLAND 21702 301-710-6686</p> <p>WWW.BEI-CHEMENGINEERING.COM</p>		
Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, William K. Ryan, P.E. License No. 21586, Expiration Date: 5-9-2015.		

APPROVED: DEPARTMENT OF PUBLIC WORKS	9-29-14
<i>Wanda Z. Campbell</i> CHIEF, BUREAU OF HIGHWAYS	DATE
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING	5-21-14
<i>Wanda Z. Campbell</i> CHIEF, DIVISION OF LAND DEVELOPMENT	DATE
<i>Paul P. ...</i> CHIEF, DEVELOPMENT ENGINEERING DIVISION	5-9-14
	DATE

DRAWN BY: WKR	CLIENT: LANE/LSBC
DESIGN BY: WKR	OWNER: MB Highland Reserve, LLC
CHECKED BY: WKR	JOB No: 1101-14-03
DATE: 03/27/2014	
DO NOT SCALE THIS DRAWING. DIMENSIONS AND NOTES HAVE PRECEDENCE OVER DRAWING	

Ryan & Associates
A Division of WKR Consulting Inc.
SPECIALIZING IN STRUCTURAL & GEOTECHNICAL ENGINEERING

Frederick, MD Office
2412 Wynfield Ct.
Frederick, MD 21702

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301-360-9574 (fx)

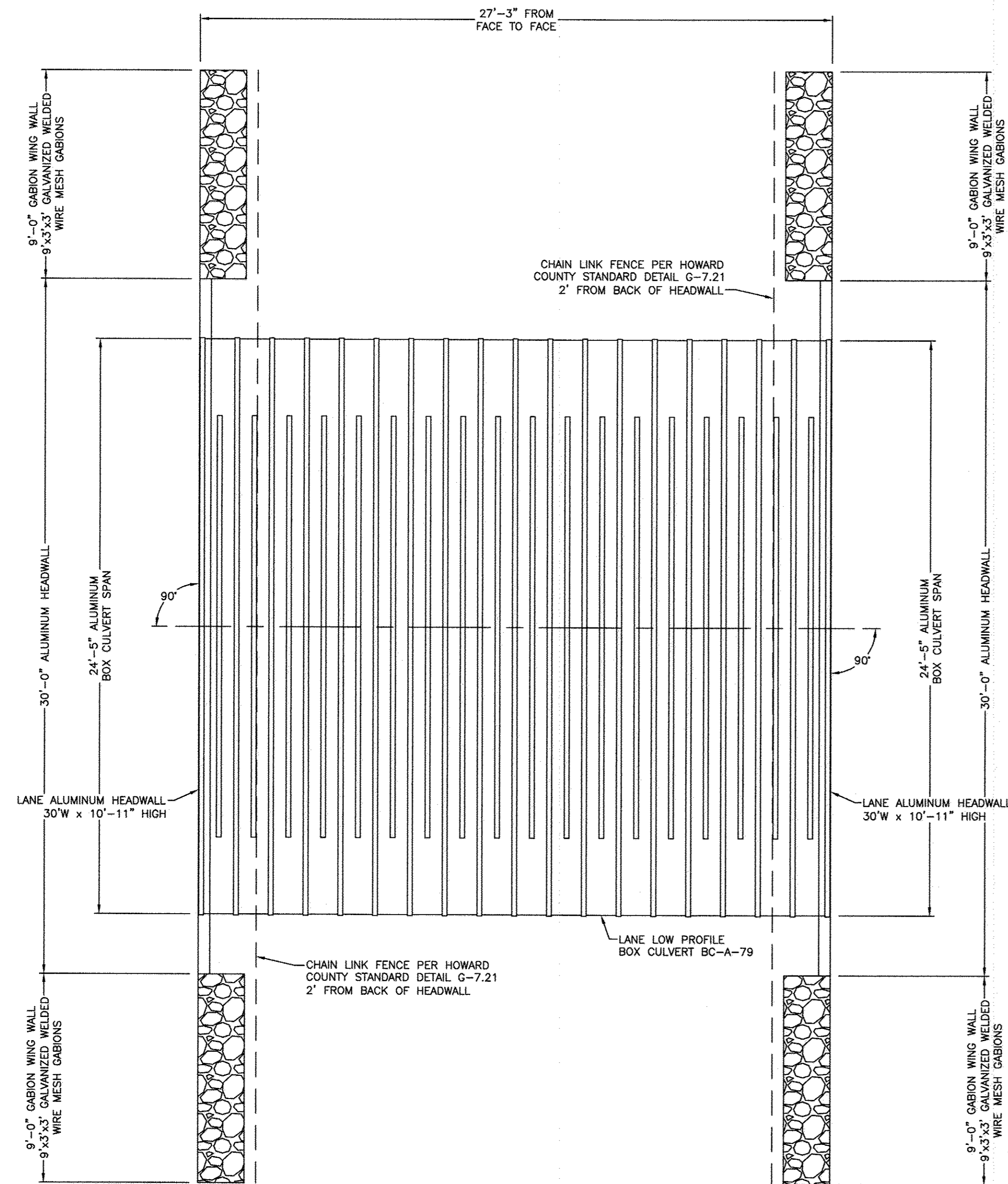
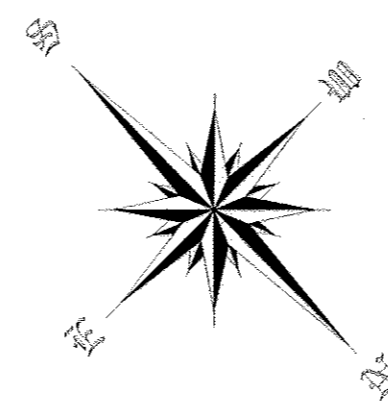
e-mail: info@ryanandassociates.net

www.ryanandassociates.net

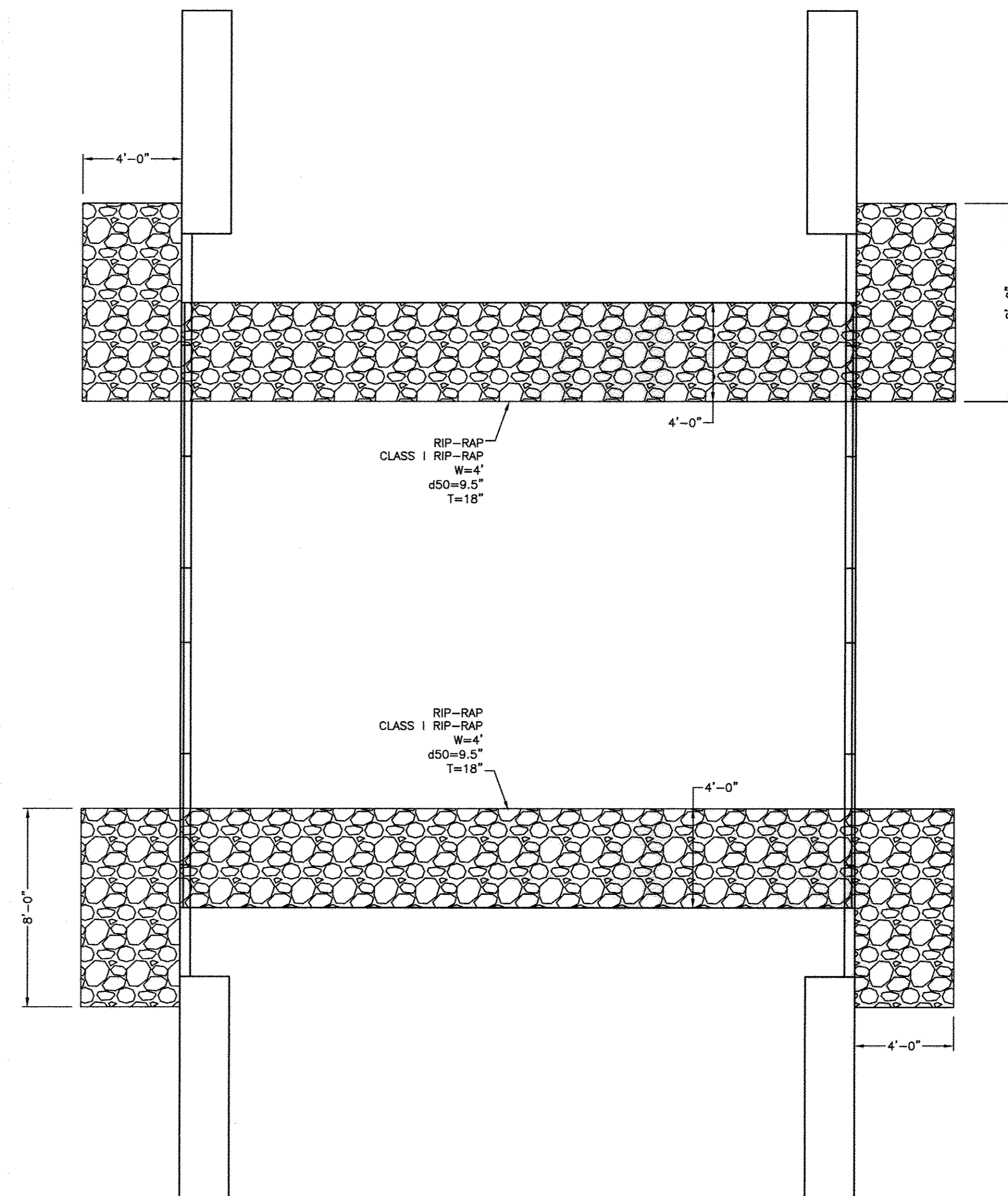
LANE LONGSPAN

LANE ENTERPRISES, INC. - LSB&C DIVISION
8271 MERCER ST., PULASKI, PA 16143
LANE: 724-652-7747
LSB&C: 888-949-5722

OWNER/DEVELOPER: RONALD R. REGAN 56B ORCHARD BEACH BLVD PORT WASHINGTON, NY 11050	PROJECT: REGAN PROPERTY LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'D' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604
SCOTT T. REGAN 10509 TWIN CEDAR COURT LAUREL, MARYLAND 20723	LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200 5th ELECTION DISTRICT HOWARD COUNTY, MARYLAND DPZ NO.: SH-12-004, ECF-12-045, WP-13-025
KELLY R. REGAN 12859 ROUTE 108 HIGHLAND, MARYLAND 20777 301.672.4820	TITLE: PLATE ARCH CULVERT CROSSING #1 STRUCTURAL NOTES & SPECIFICATIONS
DESIGN: WKR	SCALE: AS SHOWN
DRAFT: WKR	PROJECT NO. 2171
	DRAWING 28 OF 33



**BOX CULVERT
PLAN VIEW**
SCALE: 1/4"=1'



**RIP-RAP PROTECTION FOR
CROSSING #2**
SCALE: 1/4"=1'

Culvert Data

Lane Low Profile Aluminum Box Culvert (BC-A-79) w/ Footer Plates*
 Span: 24'-5"
 Rise: 8'-11"
 Thickness: 0.125" (Crown & Haunch)
 External Ribs: Type VI (Crown & Haunch)

Culvert Design Data

Culvert Loading: AASHTO H525
 Culvert Cover Heights: 2' Min. (Top of Crown to bottom of Flexible Pavement)
 3' Max. (Top of Crown to Finish Grade)
 Culvert Footer Plates: Minimum Soil Bearing Pressure Req. 4000 psf. (Confirmed by Others)
 Headwall Design Pressure: Earth Pressure Only

*At time of excavation for footer plates, subgrade bearing to be inspected and verified by Geotechnical Engineer that subgrade meets or exceeds assumed design allowable bearing capacity for footer plates.

APPROVED: DEPARTMENT OF PUBLIC WORKS

Will R. ... 4-29-14
 CHIEF, BUREAU OF HIGHWAYS
 APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
Victor ... 5-21-14
 CHIEF, DIVISION OF LAND DEVELOPMENT
Paul ... 5-9-14
 CHIEF, DEVELOPMENT ENGINEERING DIVISION



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Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443, Expiration Date: 12-21-19

**CONCEPT DRAWINGS
ONLY, PROFESSIONALLY
ENGINEERED SHOP
DRAWINGS REQUIRED
FOR APPROVAL.**

LANE **LONGSPAN**
 LANE ENTERPRISES, INC. - LSB&C DIVISION
 8271 MERCER ST., PULASKI, PA 16143
 LANE: 724-652-7747
 LSB&C: 888-949-5722

NO.	DATE	REVISION

BENCHMARK
 ENGINEERS & LAND SURVEYORS & PLANNERS
ENGINEERING, INC.
 8480 BALTIMORE NATIONAL PIKE & SUITE 215 A ELIJAH CITY, MARYLAND 21043
 (P) 410-465-3108 (F) 410-465-8844
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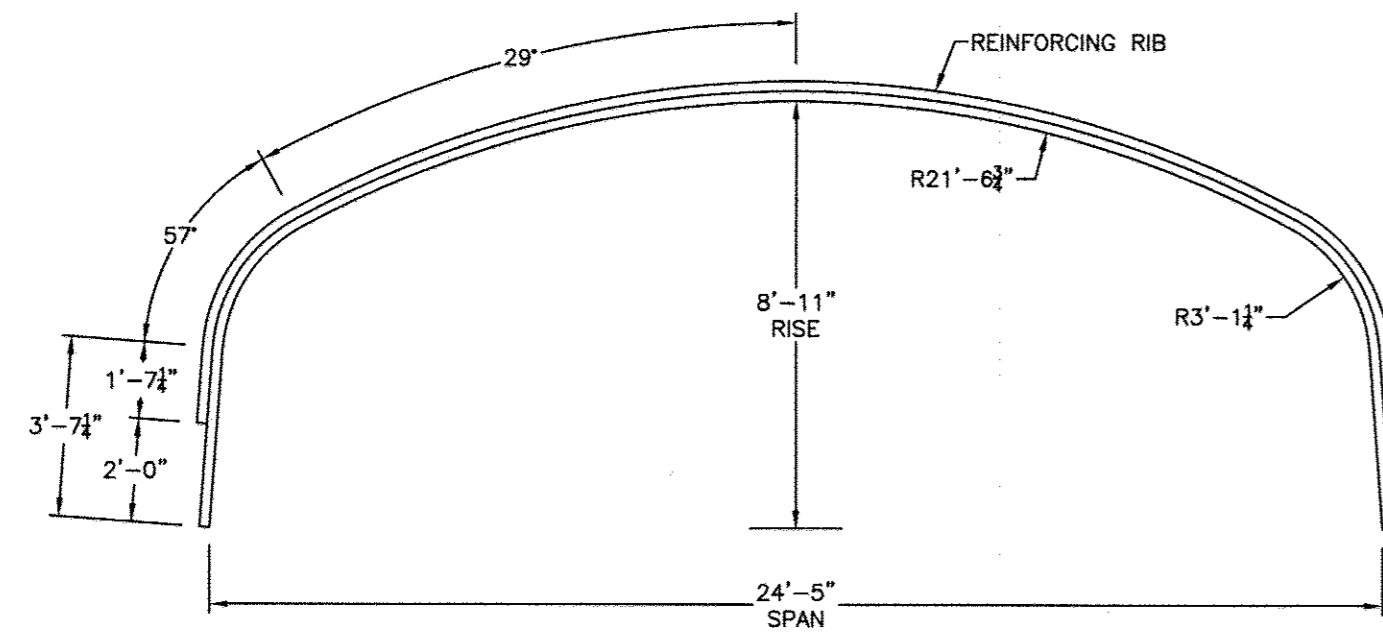
Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, William K. Ryan, P.E.
 License No. 21586, Expiration Date: 5-9-2015.

4/8/2014

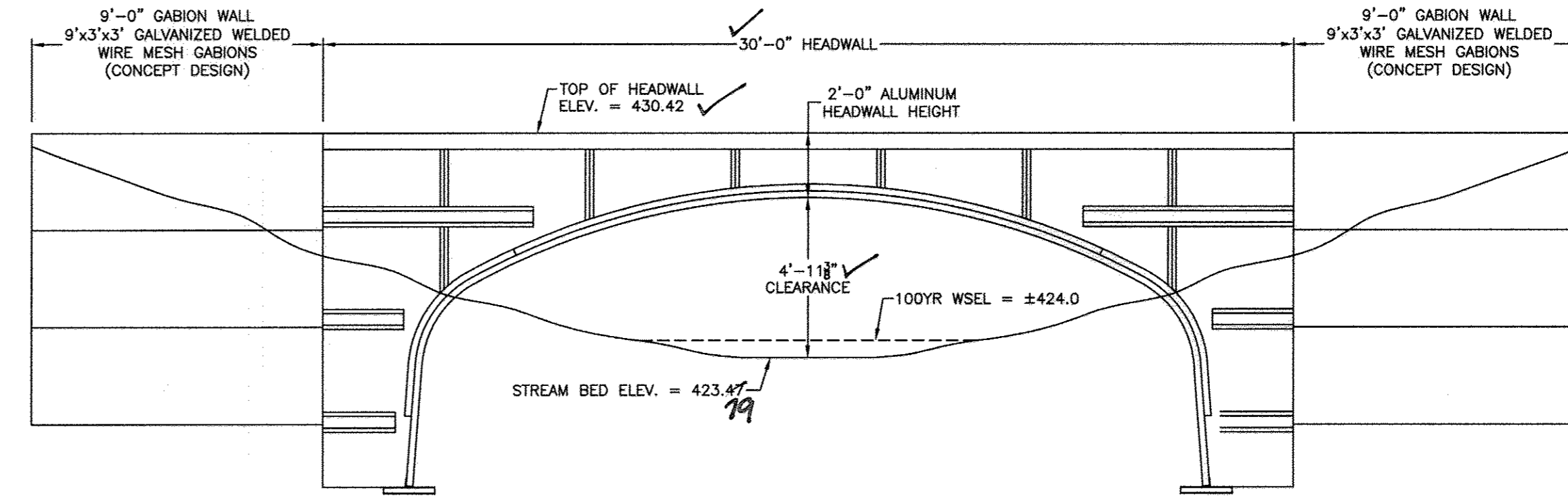
OWNER/DEVELOPER:
 RONALD R. REGAN
 56B ORCHARD BEACH BLVD
 PORT WASHINGTON, NY 11050
 SCOTT T. REGAN
 10509 TWIN CEDAR COURT
 LAUREL, MARYLAND 20723
 KELLY R. REGAN
 12859 ROUTE 108
 HIGHLAND, MARYLAND 20777
 301.672.4820

PROJECT: **REGAN PROPERTY**
 LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E', and A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604
 LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 5th ELECTION DISTRICT
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025
 TITLE: **PLATE BOX CULVERT CROSSING #2**
 CULVERT PLAN VIEWS
 DATE: MARCH, 2014 PROJECT NO. 2171
 DESIGN: JCO DRAFT: JDS SCALE: NO SCALE DRAWING 29 OF 33

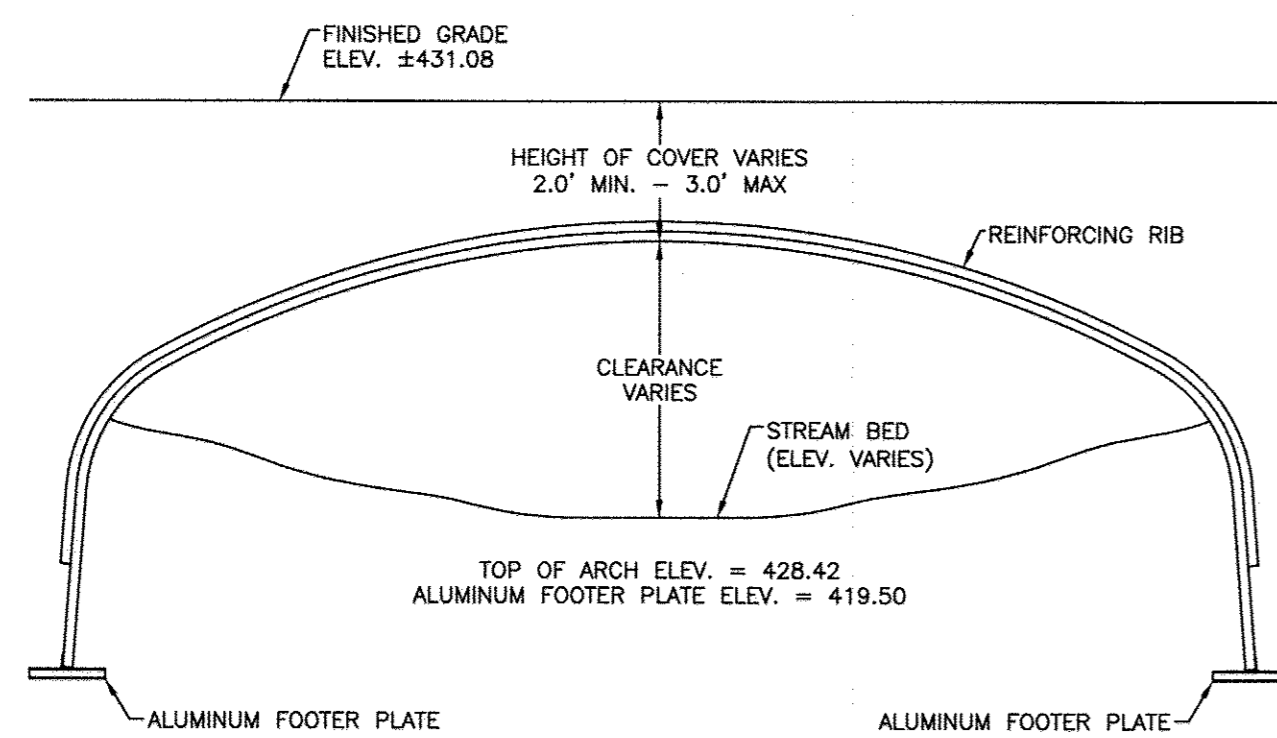
AS-BUILT



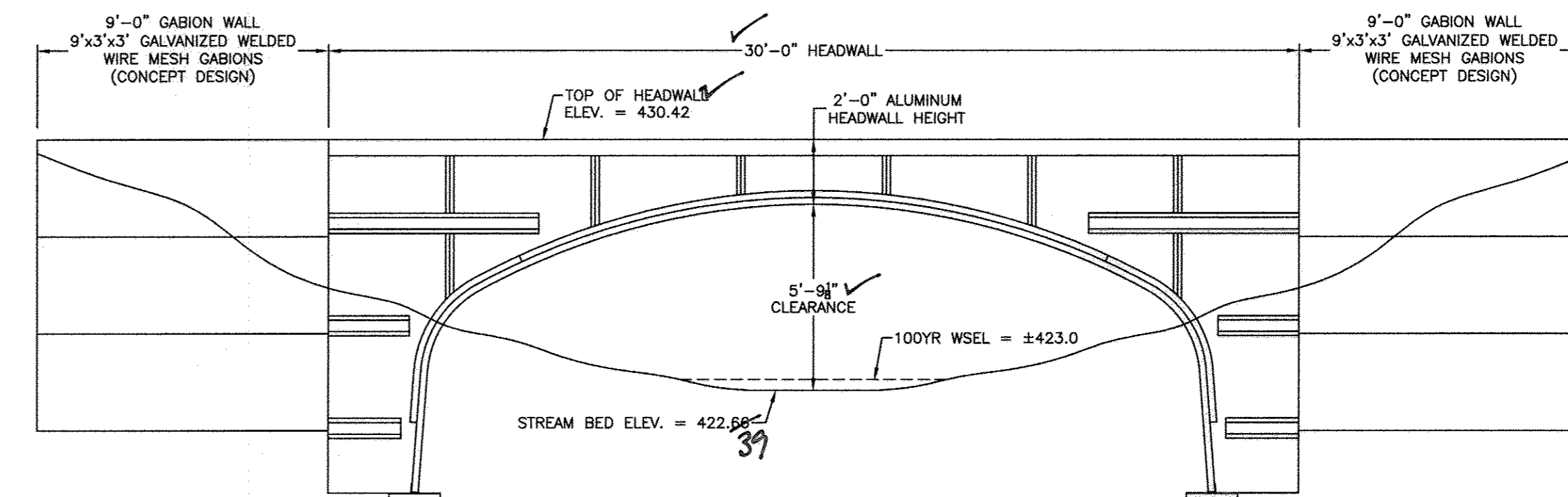
**ALUMINUM BOX CULVERT
SECTION VIEW**
SCALE: 1/4"=1'



**UPSTREAM END
ELEVATION**
SCALE: 1/4"=1'



**TYPICAL BRIDGE
SECTION VIEW**
SCALE: 1/4"=1'



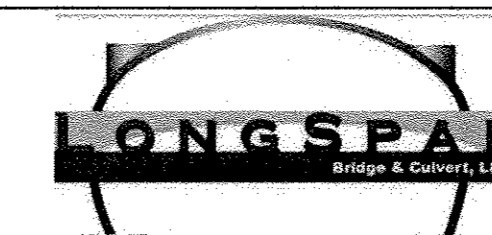
**DOWNSTREAM END
ELEVATION**
SCALE: 1/4"=1'

**CONCEPT DRAWINGS
ONLY, PROFESSIONALLY
ENGINEERED SHOP
DRAWINGS REQUIRED
FOR APPROVAL.**

AS-BUILT CERTIFICATION
I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications
Donald Mason, P.E. Date: 11-15-17



Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443, Expiration Date: 12-31-19



LANE ENTERPRISES, INC. - LSB&C DIVISION
8271 MERCER ST., PULASKI, PA 16143
LANE: 724-652-7747
LSB&C: 888-949-5722

NO.	DATE	REVISION

BENCHMARK ENGINEERING, INC.
ENGINEERS & LAND SURVEYORS & PLANNERS
8480 BALTIMORE NATIONAL PIKE & SUITE 315 • ELICOTT CITY, MARYLAND 21043
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Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, William K. Ryan, P.E. License No. 21586, Expiration Date: 5-9-2015.

4/8/2014

APPROVED: DEPARTMENT OF PUBLIC WORKS <i>W. J. ...</i> CHIEF, BUREAU OF HIGHWAYS DATE: 4-29-14
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING <i>V. ...</i> CHIEF, DIVISION OF LAND DEVELOPMENT DATE: 5-21-14
<i>...</i> CHIEF, DEVELOPMENT ENGINEERING DIVISION DATE: 5-9-14

OWNER/DEVELOPER: RONALD R. REGAN 56B ORCHARD BEACH BLVD PORT WASHINGTON, NY 11050	PROJECT: REGAN PROPERTY LOTS 2 thru 23, BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604
SCOTT T. REGAN 10509 TWIN CEDAR COURT LAUREL, MARYLAND 20723	LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200 5th ELECTION DISTRICT HOWARD COUNTY, MARYLAND DPZ NO.: SP-12-004, ECP-12-045, WP-13-025
KELLY R. REGAN 12859 ROUTE 108 HIGHLAND, MARYLAND 20777 301.672.4820	TITLE: PLATE BOX CULVERT CROSSING #2 CULVERT SECTION & END VIEWS
DESIGN: JCO	DRAFT: JDS
DATE: MARCH, 2014	PROJECT NO. 2171
SCALE: NO SCALE	DRAWING 30 OF 33

AS-BUILT

A. GENERAL

- This specification covers the manufacture and installation of aluminum structural plate box culverts, consisting of aluminum structural plate, stiffening ribs, and appurtenances. Material and geometric requirements shall conform to AASHTO M219 and ASTM B864. Geometric requirements shall also conform to Figure 12.9.4.1-1 and Table 12.9.4.1-1 of the AASHTO LRFD Bridge Design Specifications. Box culvert dimensions, plate thicknesses, rib type and spacing, end treatment, and foundation (full invert or footings) shall be as indicated on project plans. Bolts and nuts shall conform to the requirements of ASTM A307 or ASTM A449 and shall be galvanized in accordance with ASTM A153.
- Cover heights shown are based on AASHTO LRFD Bridge Design Specifications (5th Edition) as modified by applicable requirements contained in PennDOT Design Manual Part 4. Lane takes full responsibility for engineering theory, calculation correctness, and ensuring that all design assumptions are validated in the contract document either by needed details or construction specifications.
- Footing reactions depend on the actual cover height and shall be calculated in accordance with Section 12.9 of the AASHTO LRFD Bridge Design Specifications and/or PennDOT Design Manual Part 4.
- Standard design is for square end structures. Culvert ends requiring bevels or skews may require additional features to be incorporated into the shop drawings by the manufacturer.
- Contractor is to certify that all information shown on the drawings has been thoroughly checked.

B. PROJECT DESIGN

- Secure Project Engineers review and approval for use of box culvert (size & shape).
- Rib and plate combinations for the various shell configurations not shown in the cover height tables require computational support consistent with the development of said table, signed and sealed by a licensed professional engineer.
- As applicable, provide a suitable footing design for the application, complete with engineering calculations signed and sealed by a professional engineer. The design shall incorporate soil bearing capacities per geotechnical report documents, or otherwise determined by investigation.
- Place footings below scour and frost depths. Place bottom of footing at a minimum depth equal to the prevailing frost depth or scour depth. If non-yielding material is found above the scour depth, the height of non-yielding material shall govern.
- Assembly drawings and footing design shall be incorporated into construction plans.
- The following order of precedence governs any real or apparent conflict between design parameters, stresses or specifications:
 - Design requirements listed in "Special drawings and special design requirements" of the special provisions
 - AASHTO LRFD Bridge Design Specifications
- Do not change the bottom of the footing elevation unless appropriately authorized by both the Design Engineer and Geotech Engineer.
- The type and extent of end treatment on the box culvert should be chosen and designed so as to prevent the loss of backfill due to high flow conditions.
- Headwall construction shall be for vertical orientation only. Any design, other than vertical orientation, must be reviewed by the manufacturer and/or design engineer.

C. Structure Assembly

- The box culvert shall be assembled in accordance with the shop drawings provided by Lane Enterprises.
- Plate laps must be properly mated in a tangent fashion using proper alignment techniques and adequate bolt torque to seat the corrugation. The recommended installation bolt torque for aluminum box culverts is 90-115 ft-lbs for invert plates and 100-150 ft-lbs for all other components.

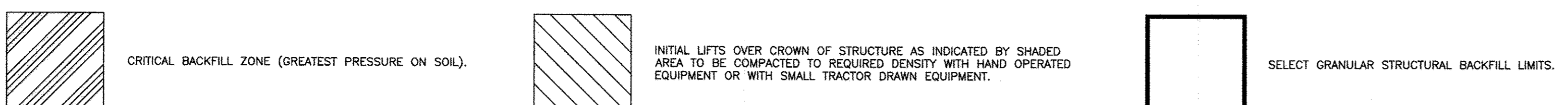
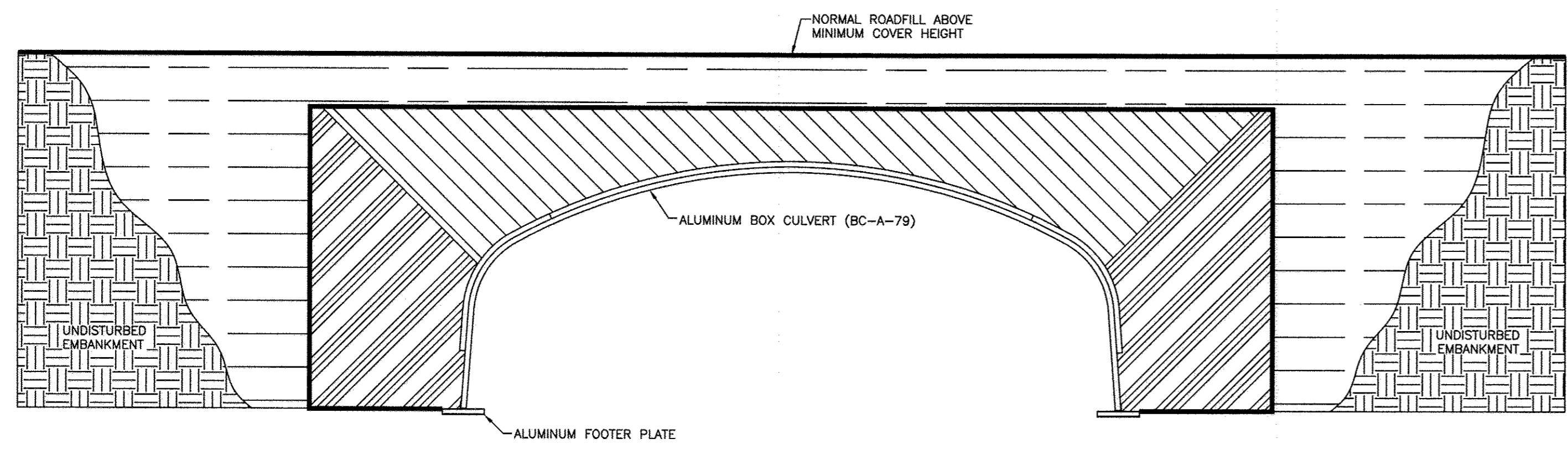
D. Construction

- The work consists of furnishing, erecting and backfilling an aluminum structural plate box culvert on concrete footings, corrugated footer plates, or a corrugated invert plate. The work includes excavation of on-site soils, proper trench construction, and structure placement to the lines, grades and locations shown on approved project drawings.
- The box culvert shall be installed in accordance with the plans and specifications, the manufacturer's recommendations and Section 26 of the AASHTO LRFD Bridge Construction Specifications. The contractor shall take special note that aluminum structural plate box culverts are flexible by nature and therefore derive structural stability from the strength and relative stiffness of the surrounding backfill material. It is the resulting soil-culvert interaction system that defines the ability of the flexible box culvert to withstand the defined service loads.
- The structure shall be backfilled using a clean, well graded granular material that meets the requirements of AASHTO M145 for soil classifications A-1, A-3, A-2-4, or A-2-5. Backfill must be placed symmetrically on each side of the structure in 6 to 8-in lifts. Each lift shall be compacted to a minimum of 90% modified density per AASHTO T180.
- If less than 3-ft of space is available on either side of the box culvert, concrete grout or a flowable fill may be required to ensure compaction against undisturbed soil.
- The equivalent of a Caterpillar D4 bulldozer or smaller to operate near and above the structure during backfilling to finish grade.
- Minimum cover may need to be increased for construction vehicle loads larger than a Caterpillar D4 bulldozer, but shall not exceed the maximum allowable cover for the box culvert design.
- When using a full invert or footer plates, the foundation shall have a minimum of 4,000 psf bearing capacity and include 6-in stable, well-graded granular bedding (lower bearing capacities can be accommodated through special design or the use of concrete footings).

E. Structural Backfill

- Structural backfill to be well graded granular, A-1, A-3, A-2-4, or A-2-5, per AASHTO M145, placed in six- to eight-inch lifts symmetrically on each side compacted to minimum 90% modified density per AASHTO T180.
- For A-2 materials, moisture content must be between -3% to +2% of optimum as defined by AASHTO T180.
- Structural backfill shall be free of organic material, frozen lumps, and rock fragments larger than three inches.
- If less than 3-ft of space is available on either side of the box culvert, concrete grout may be required.
- Equipment used to compact backfill within 3-ft from edge of footing shall be approved by the engineer prior to use.
- Continuous culvert shape monitoring shall be conducted to correct any undue distortions.
- As a minimum, structural backfill shall extend above the crown to the minimum cover specified for the box culvert.
- Minimum cover may need to be increased for construction vehicle loads larger than the D4 dozer, but shall not exceed the maximum allowable cover for the box culvert design.

Sieve Analysis, Percent Passing				
Group Classification	A-1	A-3	A-2-4	A-2-5
No. 10 (2.00 mm)	—	—	—	—
No. 40 (0.425 mm)	50 max	51 max	—	—
No. 100 (0.150 mm)	—	—	50 max	50 max
No. 200 (0.075 mm)	25 max	10 max	20 max	20 max
Characteristics of fraction passing No. 40 (0.425 mm)				
Liquid Limit	—	—	40 max	41 max
Plasticity Index	6 max	Non-plastic	10 max	10 max
Typical Constituent Materials	Stone Fragment, Gravel and Sand	Sand	Silty or Clayey Gravel and Sand	



BACKFILL DETAIL
SCALE: 1/4"=1'

APPROVED: DEPARTMENT OF PUBLIC WORKS
 [Signature] 4-29-14
 CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 [Signature] 5-21-14
 CHIEF, DIVISION OF LAND DEVELOPMENT

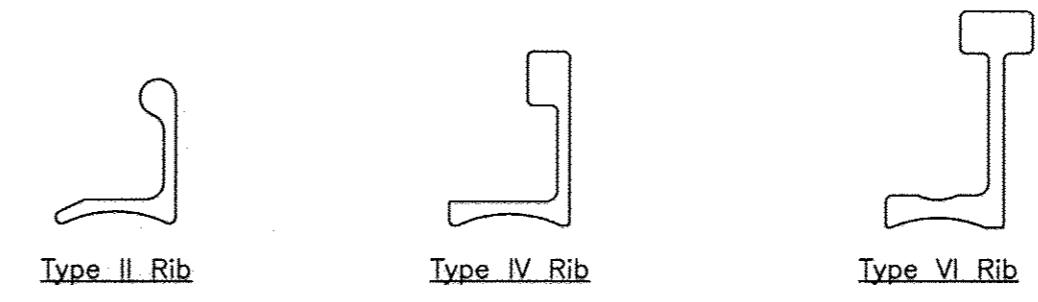
APPROVED: [Signature] 5-9-14
 CHIEF, DEVELOPMENT ENGINEERING DIVISION



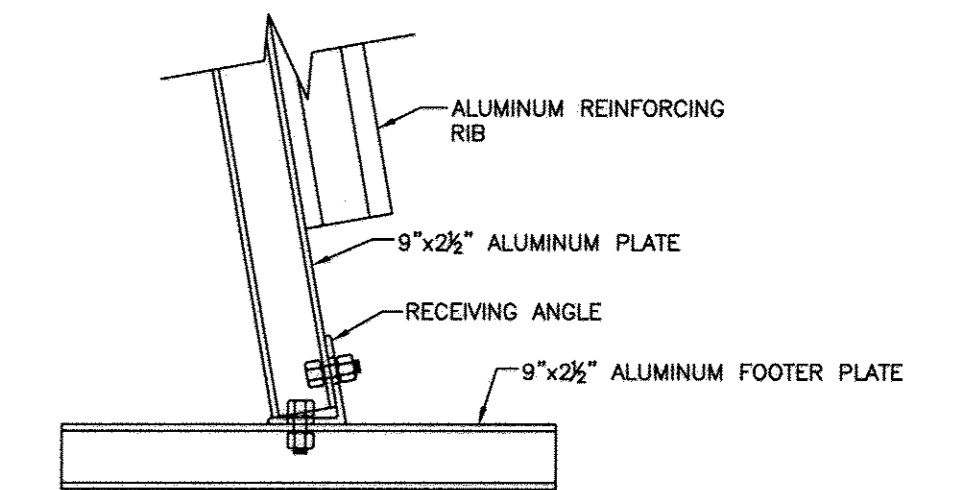
NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET

Professional Certification: I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443 Expiration Date: 12-31-15

TRANSVERSE RIB STIFFENERS

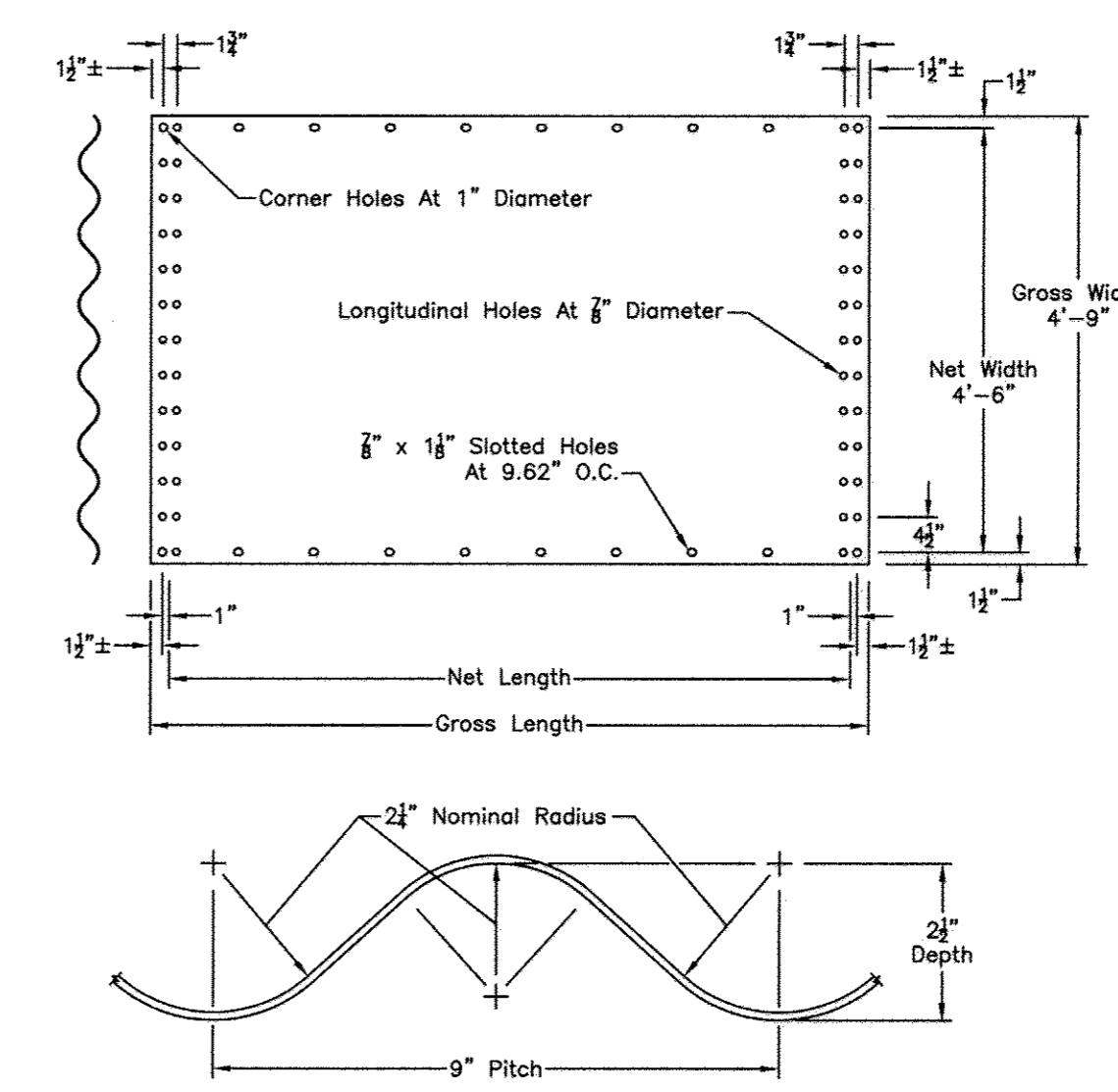


ALUMINUM FOOTER PLATES



STANDARD PLATE DETAIL

(PLATE & APPURTENANCES IN ACCORDANCE WITH AASHTO M219)



Length	Net Length	Gross Length	Weight Per Plate							
			0.100	0.125	0.125	0.150	0.175	0.200	0.225	0.250
*N	in.	in.	0.100	0.125	0.125	0.150	0.175	0.200	0.225	0.250
8N	76.96	81.71	52.7	65.9	65.9	79.1	92.2	104.7	117.8	130.9
9N	86.58	91.33	58.9	73.6	73.6	88.4	103.1	117.8	132.6	147.3
10N	96.20	100.95	65.1	81.4	81.4	97.7	114.0	130.2	146.5	162.8
11N	105.82	110.57	71.3	89.2	89.2	107.0	124.8	142.6	160.5	178.3
12N	115.44	120.19	77.5	96.9	96.9	116.3	135.7	155.1	174.4	193.8
13N	125.06	129.81	83.7	104.7	104.7	125.6	146.5	167.5	188.4	209.3
14N	134.68	139.43	89.9	112.4	112.4	134.9	157.4	179.9	202.4	224.8
15N	144.30	149.05	96.1	120.2	120.2	144.2	168.3	192.3	216.3	240.4
16N	153.92	158.67	102.4	127.9	127.9	153.5	179.1	204.7	230.3	257.8
17N	163.54	168.29	108.6	135.7	135.7	162.8	190.0	217.1	244.2	271.4
18N	173.16	177.91	114.8	143.5	143.5	172.1	200.8	229.5	258.2	286.9

Notes:
 (1) Weights based on nominal thickness.
 (2) Bolt holes have not been deducted.
 (3) *N = 9.62"

CONCEPT DRAWINGS ONLY, PROFESSIONALLY ENGINEERED SHOP DRAWINGS REQUIRED FOR APPROVAL.

LANE **LONGSPAN**

LANE ENTERPRISES, INC. - LSB&C DIVISION
 8271 MERCER ST., PULASKI, PA 16143
 LANE: 724-652-7747
 LSB&C: 888-949-5722

NO. DATE REVISION

BENCHMARK ENGINEERS & LAND SURVEYORS & PLANNERS ENGINEERING, INC.
 8480 BALTIMORE NATIONAL PIKE & SUITE 315 & ELLICOTT CITY, MARYLAND 21043
 (P) 410-465-4100 (F) 410-465-6844
 75 THOMAS JOHNSON DRIVE & SUITE E & FREDERICK, MARYLAND 21702
 301-710-5688
 WWW.BEI-CIVILENGINEERING.COM

Professional Certification: I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, William K. Ryan, P.E. License No. 21586, Expiration Date: 5-9-2015.

4/8/2014

OWNER/DEVELOPER:
 RONALD R. REGAN
 56B ORCHARD BEACH BLVD
 PORT WASHINGTON, NY 11050

PROJECT: REGAN PROPERTY
 LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

LOCATION:
 TAX MAP No. 34, GRID No. 24, PARCEL No. 200
 5TH ELECTION DISTRICT
 HOWARD COUNTY, MARYLAND
 DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
 PLATE BOX CULVERT CROSSING #2
 PLATE SPECIFICATIONS

DATE: MARCH, 2014 PROJECT NO. 2171

DESIGN: JCO DRAFT: JDS SCALE: NO SCALE DRAWING 31 OF 33

BORING LOG

GEOLAB, INC.

Report No.:		Date: 5/20/2013			
Client: Benchmark Engineering, Inc.					
Project: Regan Property, Clarksville, Maryland					
Project No. 112-041					
Boring No. SB-1	(1 of 1)	Depth 15	Elev. 385.5		
Location: See Boring Location Plan					
Type of Boring: Hollow-stem Auger	Started: 5/11/13	Completed: 5/11/13	Driller: Free State Drilling, Inc.		
Elevation	Depth	DESCRIPTION OF MATERIALS (classification)	Sample Blows Sample Depth (Feet)	Moisture Content	REMARKS
385.5	0.0	Sod with root (organic) matter and organic soil.			Groundwater was encountered during drilling at a depth of 6.0 feet. At the end of the day, the water level was at a depth of 2.0 feet.
384.6	0.9	Light brown silty fine to medium SAND, wet, very loose (SM)	0, 0, 3	34.5	
382.5	3.0	Gray micaceous fine to coarse SAND with some gravel and little silt, moist, medium dense (SM)	6, 7, 6	13.1	
380	5.5	Light to dark brown micaceous silty fine to medium SAND, moist, loose to dense to medium dense (SM)	5, 4, 5	26.4	
	7.5		8, 13, 18	19.5	
	10		11, 12, 17	16.6	
	13.5				
	15.0	End of Boring	15		

*Number of blows required for a 140 lb hammer dropping 30" to drive 2" O.D., 1.375" I.D. sampler a total of 18 inches in three 6" increments. The sum of the last two increments of penetration is termed the standard penetration resistance, N.

BORING LOG

GEOLAB, INC.

Report No.:		Date: 5/20/2013			
Client: Benchmark Engineering, Inc.					
Project: Regan Property, Clarksville, Maryland					
Project No. 112-041					
Boring No. SB-2	(1 of 1)	Depth 19.5	Elev. 385.6		
Location: See Boring Location Plan					
Type of Boring: Hollow-stem Auger	Started: 5/11/13	Completed: 5/11/13	Driller: Free State Drilling, Inc.		
Elevation	Depth	DESCRIPTION OF MATERIALS (classification)	Sample Blows Sample Depth (Feet)	Moisture Content	REMARKS
385.6	0.0	Sod with root (organic) matter and organic soil.			Groundwater was encountered during drilling at a depth of 11.0 feet. At end of day, the water level was at 2.9 feet.
384.8	0.8	Dark brown clayey SILT with little fine sand, wet, soft (ML)	1, 2, 2	30.2	
382.6	3.0	Gray micaceous fine to coarse SAND with some gravel and little silt, moist, medium dense (SM)	7, 9, 12	12.9	Boring terminated at a depth of 19.5 feet due to auger refusal.
380.1	5.5	Brown to grayish brown micaceous silty fine to medium SAND, wet, loose (SM)	3, 3, 4	30.2	
	7.5		5, 5, 5	29.1	
	10		9, 9, 15	10.1	
	13.5				
372.6	13.0	Tan fine to coarse sandy GRAVEL (rock fragments) with trace silt, moist, medium dense (GP-GM)	9, 9, 15	10.1	
	15.0				
	18.5		24, 50+	9.9	
266.1	19.5	End of Boring	19.5		

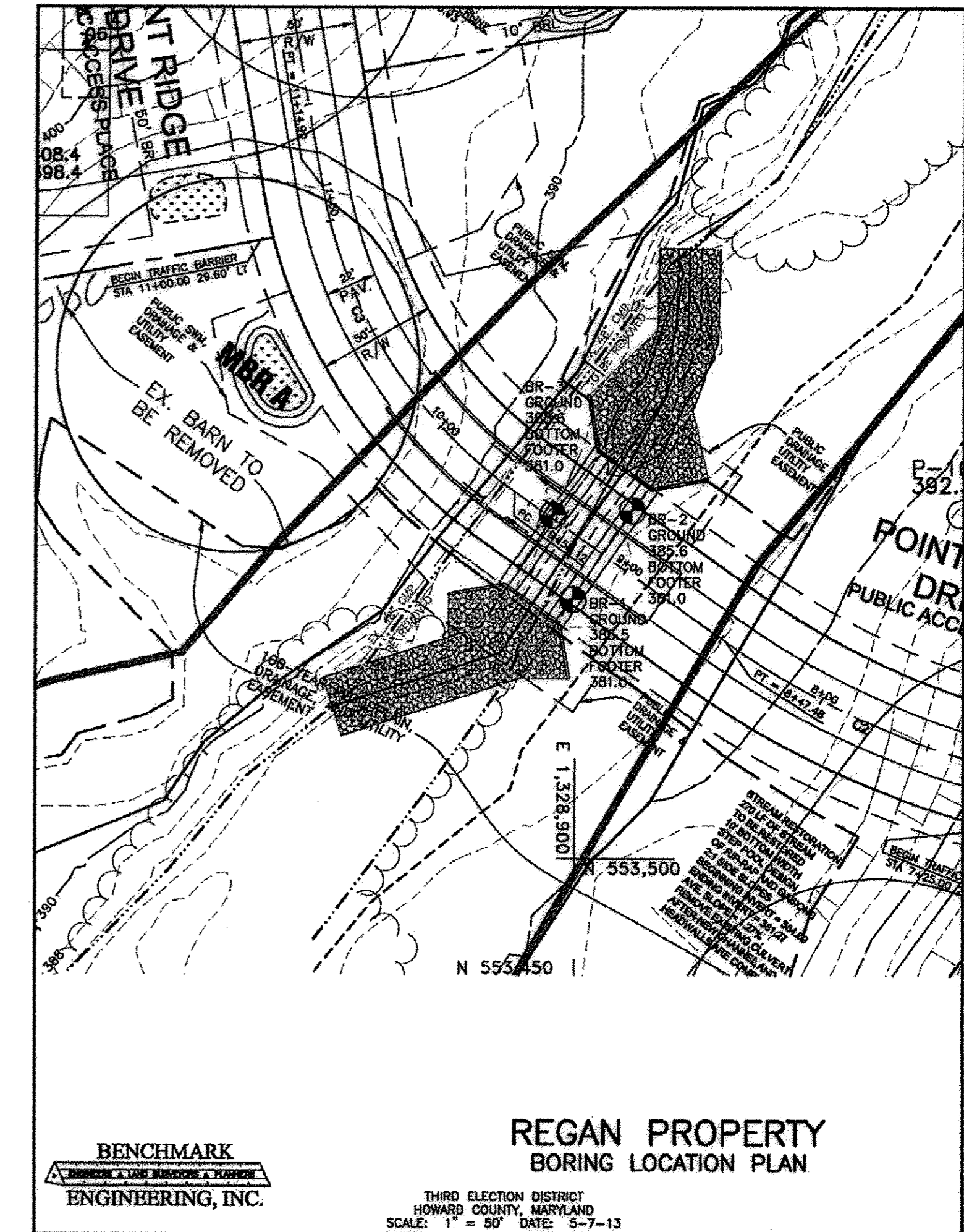
*Number of blows required for a 140 lb hammer dropping 30" to drive 2" O.D., 1.375" I.D. sampler a total of 18 inches in three 6" increments. The sum of the last two increments of penetration is termed the standard penetration resistance, N.

BORING LOG

GEOLAB, INC.

Report No.:		Date: 5/20/2013			
Client: Benchmark Engineering, Inc.					
Project: Regan Property, Clarksville, Maryland					
Project No. 112-041					
Boring No. SB-3	(1 of 1)	Depth 14.5	Elev. 385.6		
Location: See Boring Location Plan					
Type of Boring: Hollow-stem Auger	Started: 5/11/13	Completed: 5/11/13	Driller: Free State Drilling, Inc.		
Elevation	Depth	DESCRIPTION OF MATERIALS (classification)	Sample Blows Sample Depth (Feet)	Moisture Content	REMARKS
385.6	0.0	Sod with root (organic) matter and organic soil.			Groundwater was encountered during drilling at a depth of 6.0 feet. At completion, the water level was recorded at a depth of 5.0 feet.
384.5	1.1	Dark brown SILT with some fine sand, wet, loose (ML)	1, 1, 2	39.1	
382.4	3.0	Gray micaceous fine to coarse SAND with some gravel and little silt, moist, medium dense (SM)	7, 8, 12	7.9	At the end of the day, the water level was at a depth of 3.0 feet.
380.1	5.5	Dark gray micaceous silty fine to medium SAND, moist, medium dense to very dense (SM)	11, 12, 18	20.7	
	7.5		4, 9, 12	17.4	
	10				
	13.5		21, 50+	13.1	
	14.5	End of Boring	14.5		

*Number of blows required for a 140 lb hammer dropping 30" to drive 2" O.D., 1.375" I.D. sampler a total of 18 inches in three 6" increments. The sum of the last two increments of penetration is termed the standard penetration resistance, N.



BENCHMARK ENGINEERING, INC.
THIRD ELECTION DISTRICT
HOWARD COUNTY, MARYLAND
SCALE: 1" = 50' DATE: 5-7-13

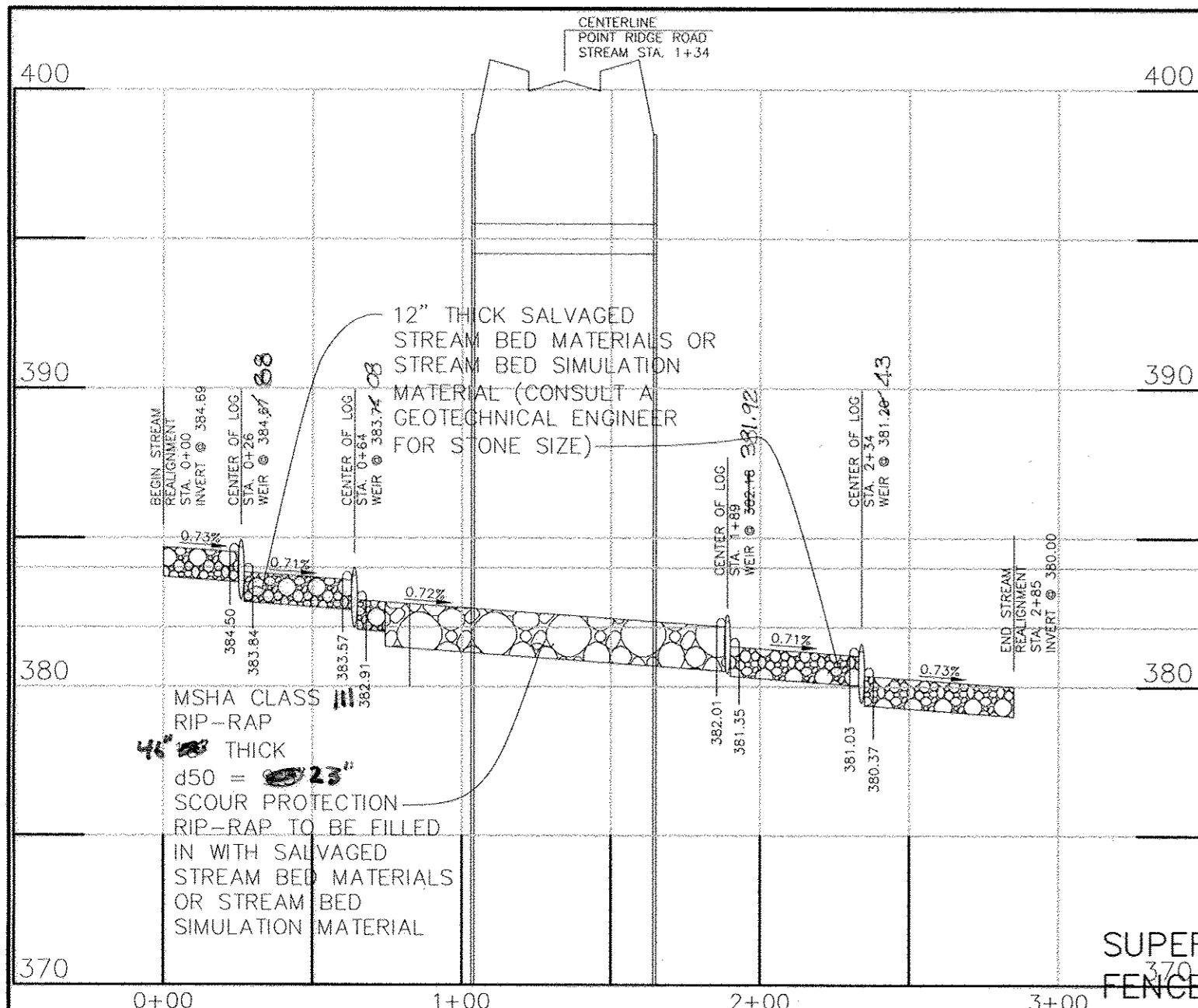
APPROVED: DEPARTMENT OF PUBLIC WORKS
 [Signature] 4-29-14
 CHIEF, BUREAU OF HIGHWAYS
 APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
 [Signature] 5-21-14
 CHIEF, DIVISION OF LAND DEVELOPMENT
 [Signature] 5-9-14
 CHIEF, DEVELOPMENT ENGINEERING DIVISION

NO AS-BUILT INFORMATION IS PROVIDED ON THIS SHEET

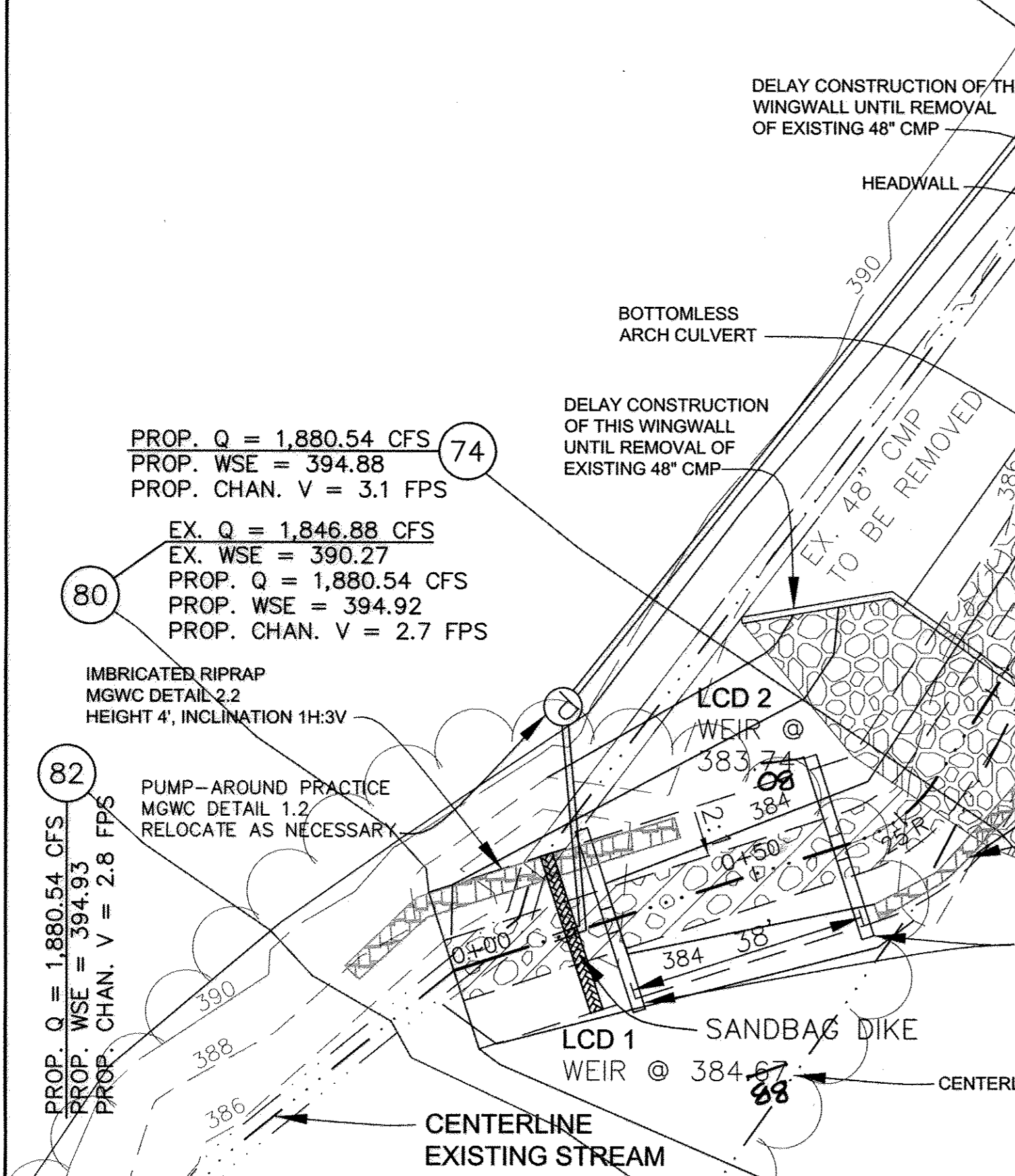


Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
 License No. 21443 Expiration Date: 12-31-19

NO.	DATE	REVISION
<p>BENCHMARK ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE & SUITE 315 A ELLICOTT CITY, MARYLAND 21043 (P) 410-485-6105 (F) 410-485-8644 75 THOMAS JOHNSON DRIVE & SUITE E A FREDERICK, MARYLAND 21702 301-710-5888 WWW.BE-ENGINEERING.COM</p>		
<p>Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 28559, Expiration Date: 12-22-2015.</p> <p>[Signature] 4/10/2014</p>		
OWNER/DEVELOPER:		PROJECT:
RONALD R. REGAN 56B ORCHARD BEACH BLVD PORT WASHINGTON, NY 11050		REGAN PROPERTY LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'E' A RESUBDIVISION OF LOT 1 AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22801-22604
SCOTT T. REGAN 10509 TWIN CEDAR COURT LAUREL, MARYLAND 20723		LOCATION: TAX MAP No. 34, GRID No. 24, PARCEL No. 200 3RD ELECTION DISTRICT HOWARD COUNTY, MARYLAND DPZ NO.: SP-12-004, ECP-12-045, WP-13-025
KELLY R. REGAN 12859 ROUTE 108 HIGHLAND, MARYLAND 20777 301.672.4820		TITLE: FINAL ROAD CONSTRUCTION PLAN BORING LOG PLAN
DESIGN: JCO	DRAFT: JCO	DATE: MARCH, 2014 PROJECT NO. 2171
SCALE: NO SCALE		DRAWING 32 OF 33



PROFILE - STREAM ALIGNMENT
STEP DESIGN WITH LOG CHECK DAMS
SCALE: VERTICALLY 1" = 5'
HORIZONTALLY 1" = 50'



PLAN VIEW
SCALE: 1" = 20'

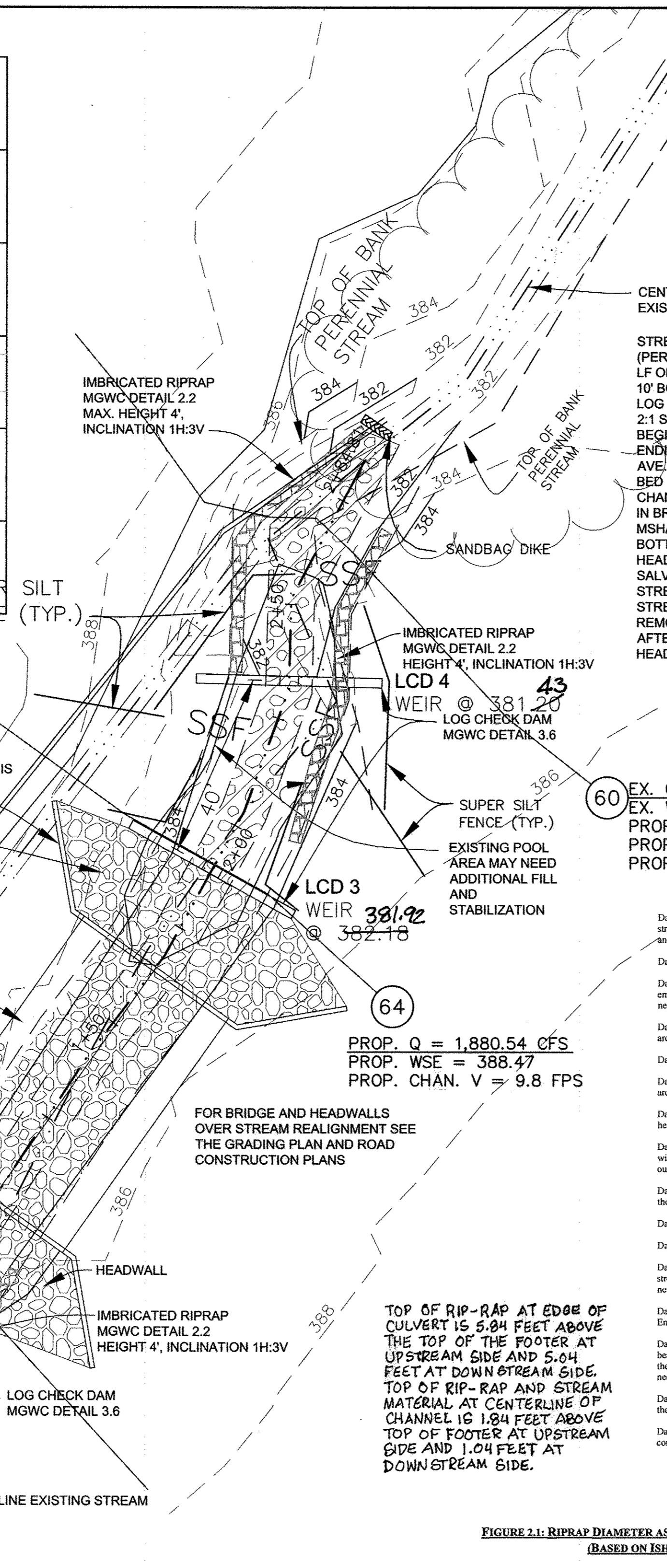
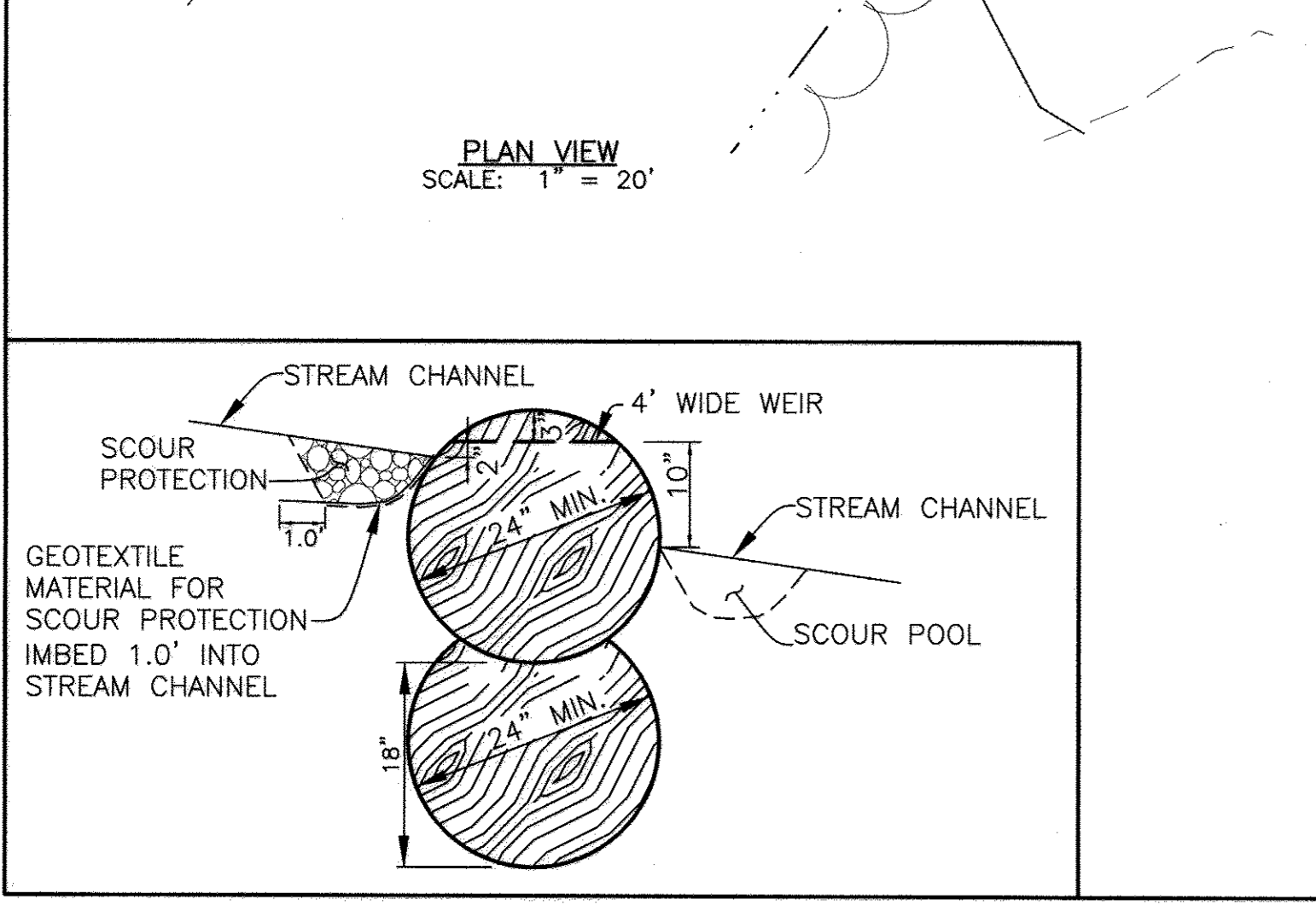
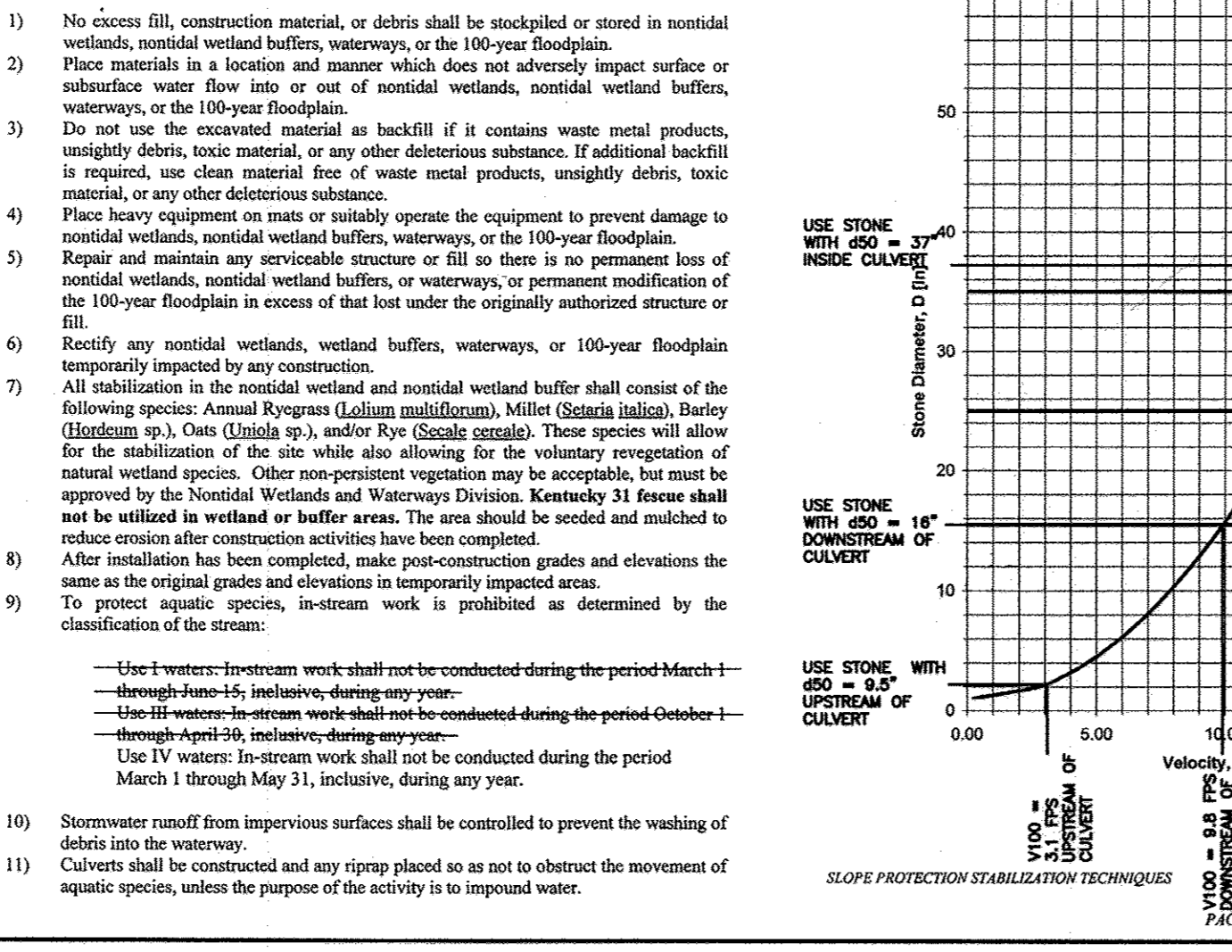
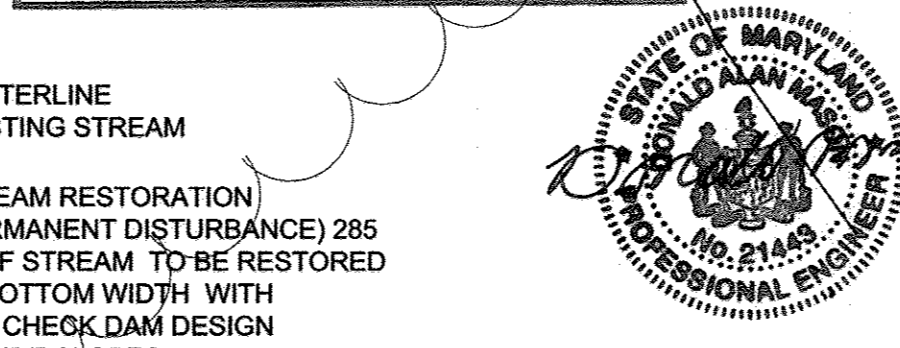


FIGURE 2.1: RIP-RAP DIAMETER AS A FUNCTION OF STREAM VELOCITY
(BASED ON ISHBAH EQUATION)



AS-BUILT CERTIFICATION
I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications.
Donald Mason, P.E.
Date: 11-15-17
Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443 Expiration Date: 12-21-19



STREAM RESTORATION (PERMANENT DISTURBANCE) 285 LF OF STREAM TO BE RESTORED TO BOTTOM WIDTH WITH LOG CHECK DAM DESIGN 2.1 SIDE SLOPES BEGINNING INVERT = 384.69 ENDING INVERT = 380.00 AVE. SLOPE = 1.65% BED SLOPE = 0.7% CHAN VELOCITY IN BRIDGE 15.4 FT/S MAX. MSHA CLASS II RIPRAP WITHIN BOTTOMLESS ARCH AND HEADWALLS SALVAGED OR IMPORTED STREAM MATERIAL IN OTHER AREAS REMOVE EXISTING CULVERT AFTER NEW CHANNEL AND HEADWALLS ARE COMPLETE

EX. Q = 1,884.93 CFS
EX. WSE = 387.19
PROP. Q = 1,912.42 CFS
PROP. WSE = 387.28
PROP. CHAN. V = 8.1 FPS

Sequence of Construction for Stream Realignment

Day 1 - Obtain grading permit. A letter of Authorization from MDE must be obtained prior to disturbances to the stream, floodplains or wetlands for the road and/or utility crossings. Stream closure shall be between March 1st and May 31st.

Day 2 - Install sediment and erosion control measures at the crossing location.

Day 3-5 - With the approval of the sediment control inspector excavate for the new stream bed. Do not breach the embankment (upstream or downstream of the crossing) of the existing channel. Install the pump and filter bag as necessary. Provide erosion control matting on all disturbed embankments that are not stabilized with Rip-Rap.

Day 6-10 - Upon approval of the Howard County Sediment Control Inspector excavate for the footers of the arch crossings. Install pumps and filter bags as necessary.

Day 10-28 - Upon approval of the bearing capacities by the Geotechnical Engineer install the footers.

Day 39-39 - Install the arch culvert. Remove pumps as necessary. Backfill the culvert footings according to the arch manufacturer's specifications or Geotechnical Engineers recommendations.

Day 40-43 - Upon approval of the Howard County Sediment Control Inspector excavate for the footers of both headwalls and the wing walls on the southern side of the crossing. Install pumps and filter bags as necessary.

Day 44-60 - Upon approval of the bearing capacities by the Geotechnical Engineer install the headwalls and the wing walls on the southern side of the stream. Move pumps as necessary to provide dewatering. Provide riprap outlet for same.

Day 61-68 - Begin to backfill overtop of the bridge with the material specified by the bridge manufacturer. See the manufacturer's plans for the back fill specifications.

Day 69-71 - Install the log check dams and imbricated rip-rap.

Day 72-74 - Install the bed material and channel rip-rap.

Day 75-77 - Upon approval of the Howard County Sediment Control Inspector remove the embankment of the stream to allow flow in the new stream bed and under the arch. Dam the existing stream to force flow under the new culvert.

Day 78-82 - Remove the existing 48" CMP and backfill trench with material approved by the Geotechnical Engineer.

Day 83-89 - Excavate for the footers of the wing walls on the northern side of the crossing. Upon approval of the bearing capacities by the Geotechnical Engineer install the wing walls on the northern side of the stream. Attach the storm drain to the wing wall and provide riprap outlet for same. Install and remove pumps and filter bags as necessary.

Day 90-95 - Complete backfill overtop of the bridge with the material specified by the bridge manufacturer. See the manufacturer's plans for the back fill specifications.

Day 96-99 - Upon approval of the Howard County Sediment Control Inspector, remove remaining sediment control devices and stabilize disturbed areas in accordance with the permanent seeded notes.

AS-BUILT CERTIFICATION
I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications.
Donald Mason, P.E.
Date: 11-15-17
Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.
License No. 21443 Expiration Date: 12-21-19

MGWC 3.6: LOG & ROCK CHECK DAMS

Rigid engineering technique for creation of aquatic habitat and channel grade control

DESCRIPTION
Low profile drop structures, such as check dams, are primarily used to create aquatic habitat in the form of scour pools and for grade control on actively incising streams and rivers.

EFFECTIVE USES & LIMITATIONS
Log and rock check dams are best suited to Roanoke stream types B3-B4, C3-C4, E3, and F3-F4. When constructed and spaced properly, check dams can simulate the natural pattern of pools and riffles occurring in undisturbed streams while forming grade control structures which fish use as spawning grounds.

Check dams have also been used to prevent the movement of fine sediments into the main channel, to aerate water, and to raise water levels past culvert invert elevations, thereby allowing fish passage.

Check dams should be avoided in the following areas:
• channels with bedrock beds or unstable bed substrates;
• channels without well developed, stable banks;
• streams with high bedload transport;
• streams with naturally well developed pools-riffle sequences; and
• reaches where the water temperature regime is negatively impacted when the current is slowed.

MATERIAL SPECIFICATIONS
Check dams, when used as stream restoration and grade control measures, are typically made of rocks, logs, or a combination of the two.

Riprap: Riprap for designed stability, bank armoring, and toe protection should be capable of withstanding bankfull flow velocities and abutment according to MGWC 2.1: Riprap.

Log: Native, rot resistant wood such as Sycamore with a minimum diameter of 12 inches (0.30 meters) should be used when available. If more than one layer of logs is to be used, they should be hewn smooth so that they fit against each other. Before installation, the log(s) should be grooved or notched to concentrate low flows. On wider shallow streams with gravelly beds, large flat rocks or boulders sized according to MGWC 2.1: Riprap to resist bankfull flows and sealed with gravel and sand may be used in place of logs.

Approximate Cost (\$1999):
\$355 per log dam

INSTALLATION GUIDELINES
All erosion and sediment control devices, including dewatering basins, should be implemented as the first order of business according to a plan approved by the WMA. The recommended construction procedure for log and rock check dams should be as follows (refer to Detail 3.6):

CHANNEL STABILIZATION AND REHABILITATION TECHNIQUES MARYLAND DEPARTMENT OF THE ENVIRONMENT WATERWAY CONSTRUCTION GUIDELINES REVISED NOVEMBER 2000 PAGE 3.6-1

MGWC 2.1: RIPRAP

Rigid engineering technique for bank stabilization

DESCRIPTION
Riprap is used to protect and stabilize embankment soils from the erosive forces of flowing water and piping forces resulting from groundwater seepage. A well-engineered riprap system should consist of the following:
• a filter layer of gravel or cloth designed to prevent soil movement into or through the riprap layer while allowing water to drain from the embankment, and
• a stone layer of appropriate gradation and thickness to resist the shearing forces of channelized water.

EFFECTIVE USES & LIMITATIONS
When properly designed and installed, riprap is an effective method where soil conditions, water turbulence and velocity, and ground water conditions are such that the soil may erode under the design flow conditions. Some common areas of riprap applicability are:
• diversion channel banks ends/bottoms;
• roadside ditches;
• drop structure outlets; and
• laterally expanding banks threatening infrastructure or personal property.

Additionally, properly graded riprap forms a flexible, self-sealing cover which can be easily repaired in localized areas by the timely replacement of stone. Uniform-grade riprap can also be used with geotextile filter cloth.

Filter cloth should only be utilized when the bank material is non-cohesive such as sand or gravel.

MATERIAL SPECIFICATIONS
• Filters: Material and design specifications for granular filters are found in Table 3.1a.

Table 3.1a: Granular Filter Material Grading Specifications

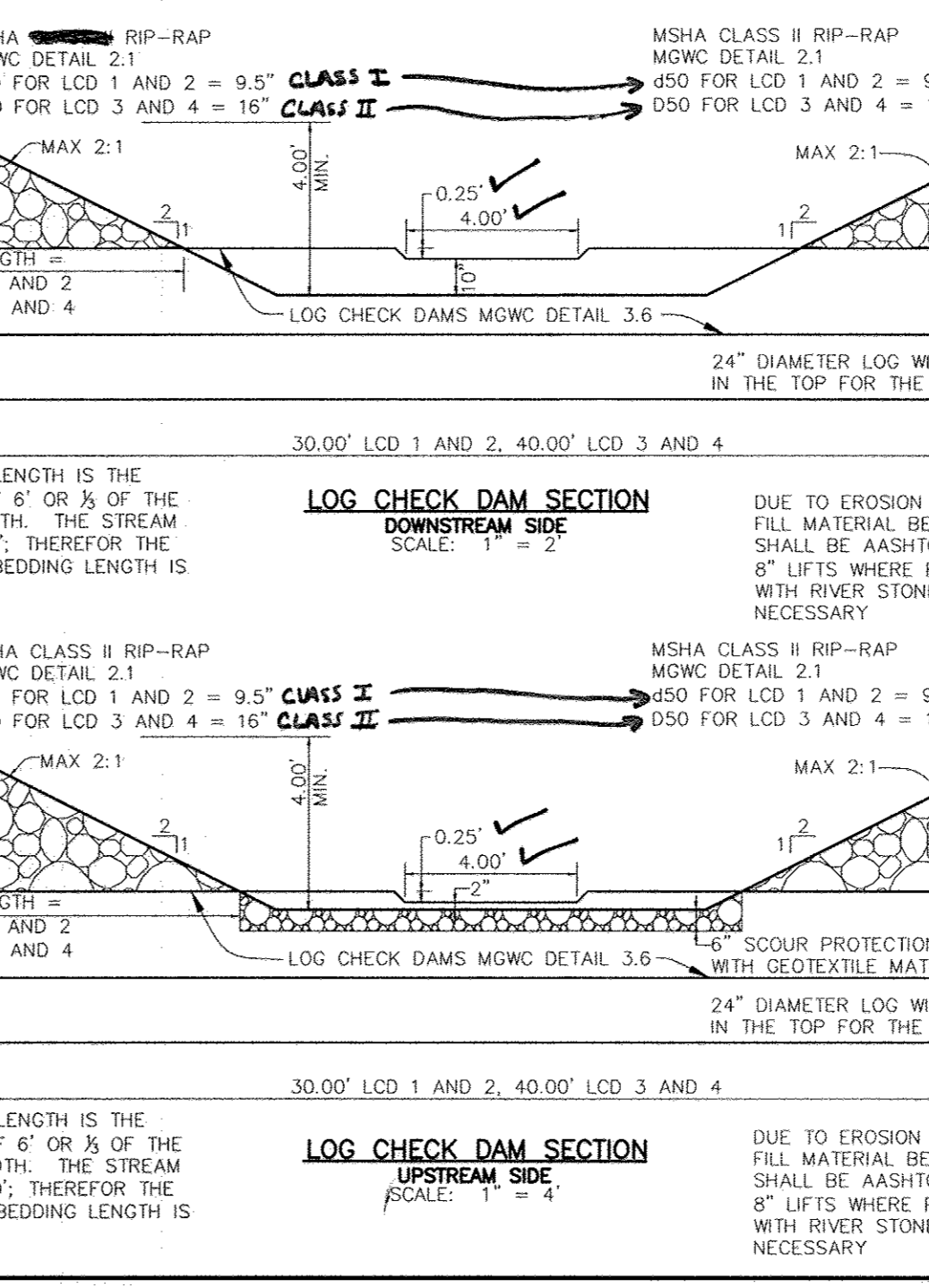
% less than	U.S. Standard sieve size
100	2 1/2 in (64 mm)
85-100	1 in (25 mm)
60-100	3/4 in (19 mm)
35-70	No. 10
20-50	No. 20
5-20	No. 40

The thickness of the filter should not be less than 6 inches (15 cm). Generally, filters that are one-half the thickness of the riprap layer are satisfactory.

Synthetic filter cloth may be used cautiously based on the 1994 M/D Standards and Specifications for Soil Erosion and Sediment Control.

• Riprap: The minimum diameter or weight of stone for riprap should be based upon the design flow velocity using Figure 3.1. This chart is based on a maximum slope of 2H:1V. The stone gradation for Classes I - III are found in Table 3.1b.

SLOPE PROTECTION AND STABILIZATION TECHNIQUES MARYLAND DEPARTMENT OF THE ENVIRONMENT WATERWAY CONSTRUCTION GUIDELINES REVISED NOVEMBER 2000 PAGE 2.1-1



MGWC 3.6: LOG & ROCK CHECK DAMS

1. The stream should be diverted according to a WMA recommended method, and the construction area should be dewatered.

2. Check dams should be located in nonflood areas where the bank is stable and of adequate height. The structure should be embedded as far as possible into the streambed and should be anchored a minimum of 1/2 of the stream width or 6 feet (1.8 meters) into the stream bank, whichever is greater. Generally, a crest height of 1 foot (0.3 meters) above the bed is sufficient for scour pool formation. (For further design guidance, refer to MGWC 3.6: Step Pools.)

3. Once in place, the structure should be further secured against movement with rebar pins. Next, to prevent scour, geotextile fabric for scour prevention should be attached to the upstream portion of the log, buried at least 1 foot (0.30 meters) into the streambed, and backfilled with adequately sized rock. Once the excavated portion of the bank has been backfilled, it should be armored with apply sized riprap, soil mats, or willow transplants to prevent erosion and scour from compromising the integrity of the structure.

4. Adjacent weirs should be spaced sufficiently far apart to allow for proper riffle or pool development according to step-pool and riffle-pool sequences as provided in MGWC 3.6: Step Pools. (Refer to the 1994 M/D Standards and Specifications for Soil Erosion and Sediment Control for spacing guidelines of check dams in ditches and swales.) Additionally, it has been recommended that the overall drop controlled by a set of two consecutive check dams should be less than 2 feet (0.6 meters) for stability purposes.

5. All disturbed sections of the channel, including the banks and streambed, should be stabilized with methods approved by the WMA.

6. All check dams should be monitored to determine if:
• their orientation and geometry (e.g., the height of the drop) hinder fish migration;
• their performance is adversely affected by deposited sediment; and
• their placement creates bank instabilities and undesirable lateral stream movement, especially in the vicinity of the plunge pools.

CHANNEL STABILIZATION AND REHABILITATION TECHNIQUES MARYLAND DEPARTMENT OF THE ENVIRONMENT WATERWAY CONSTRUCTION GUIDELINES REVISED NOVEMBER 2000 PAGE 3.6-2

MGWC 2.1: RIPRAP

Table 3.1b: Stone Gradations for Riprap Stone Classes

Class	Size	% Total Weight < Given Size
I	150 lb (70 kg) 2 ft (1 kg)	10 max
II	700 lb (320 kg) 20 lb (10 kg)	100 10 max
III	2000 lb (910 kg) 40 lb (20 kg)	100 10 max

Uniform-grade riprap should incorporate angular rock to promote interlocking.

Approximate Cost (\$1999):
\$78 per linear ft

INSTALLATION GUIDELINES
All erosion and sediment control devices, including dewatering basins, should be implemented as the first order of business according to a plan approved by the WMA or local authority. Once a slope stabilization project is initiated, preparation and placement of the riprap should immediately follow the initial disturbance to minimize the chances for further slope degradation. The recommended construction procedure for riprap is as follows beginning with initial slope preparation (refer to Detail 2.1):

- The contractor should install all sediment and erosion control devices as the first order of business.
- Excavation should be made in reasonably close conformity with the existing stream slope and bed.
- All fill in the subgrade should be compacted to a density approximating that of the surrounding undisturbed material.
- Provisions must be made to anchor the riprap to the stream bed so as to provide protection against undermining. If this cannot be accomplished by creating a toe trench, an alternative method of protection must receive prior written approval from the WMA or local authority.
- The filter layer or blanket should be placed immediately after slope preparation.
 - The stone for granular filters should be spread in a uniform layer to the specified depth. Where more than one layer is employed, they should be spread such that there is minimal mixing.
 - When cloth filters are used, special care should be taken not to damage the fabric during riprap placement.
- Riprap placement should begin with the toe. The larger stones, as specified by the design gradation, should be placed in the toe and along the perimeter of the slope and channel protection. The riprap should be placed with suitable equipment in such a manner as to produce a reasonably graded mass of stones with even drop height. The placing of stones that cause extensive segregation is not allowed. Where appropriate, a low flow channel shall be constructed through the riprap.
- Any excavation voids existing along the edges of the completed slope and channel protection should be backfilled and compacted.
- All disturbed areas should be permanently stabilized in accordance with an approved sediment and erosion control plan.

Note: The use of rock sizes (MGWC 3.1: Rock Piles) should be considered to redirect high-velocity flows at the toe.

SLOPE PROTECTION AND STABILIZATION TECHNIQUES MARYLAND DEPARTMENT OF THE ENVIRONMENT WATERWAY CONSTRUCTION GUIDELINES REVISED NOVEMBER 2000 PAGE 2.1-3

BENCHMARK ENGINEERING, INC.
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APPROVED: DEPARTMENT OF PUBLIC WORKS
Will J. ...
DATE: 5-9-14

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING
...
DATE: 5-21-14

APPROVED: CHIEF, DIVISION OF LAND DEVELOPMENT
...
DATE: 5-19-14

OWNER/DEVELOPER:
MB HIGHLAND RESERVE, LLC
1686 EAST GUDE DRIVE
ROCKVILLE, MD 20850
301-762-9511

PROJECT: REGAN PROPERTY
LOTS 2 thru 23; BUILDABLE PRESERVATION PARCEL 'A', and NON-BUILDABLE PRESERVATION PARCELS 'B' thru 'I' AND NON-BUILDABLE BULK PARCEL 'A' PREVIOUSLY RECORDED AS PLAT NO. 22601-22604

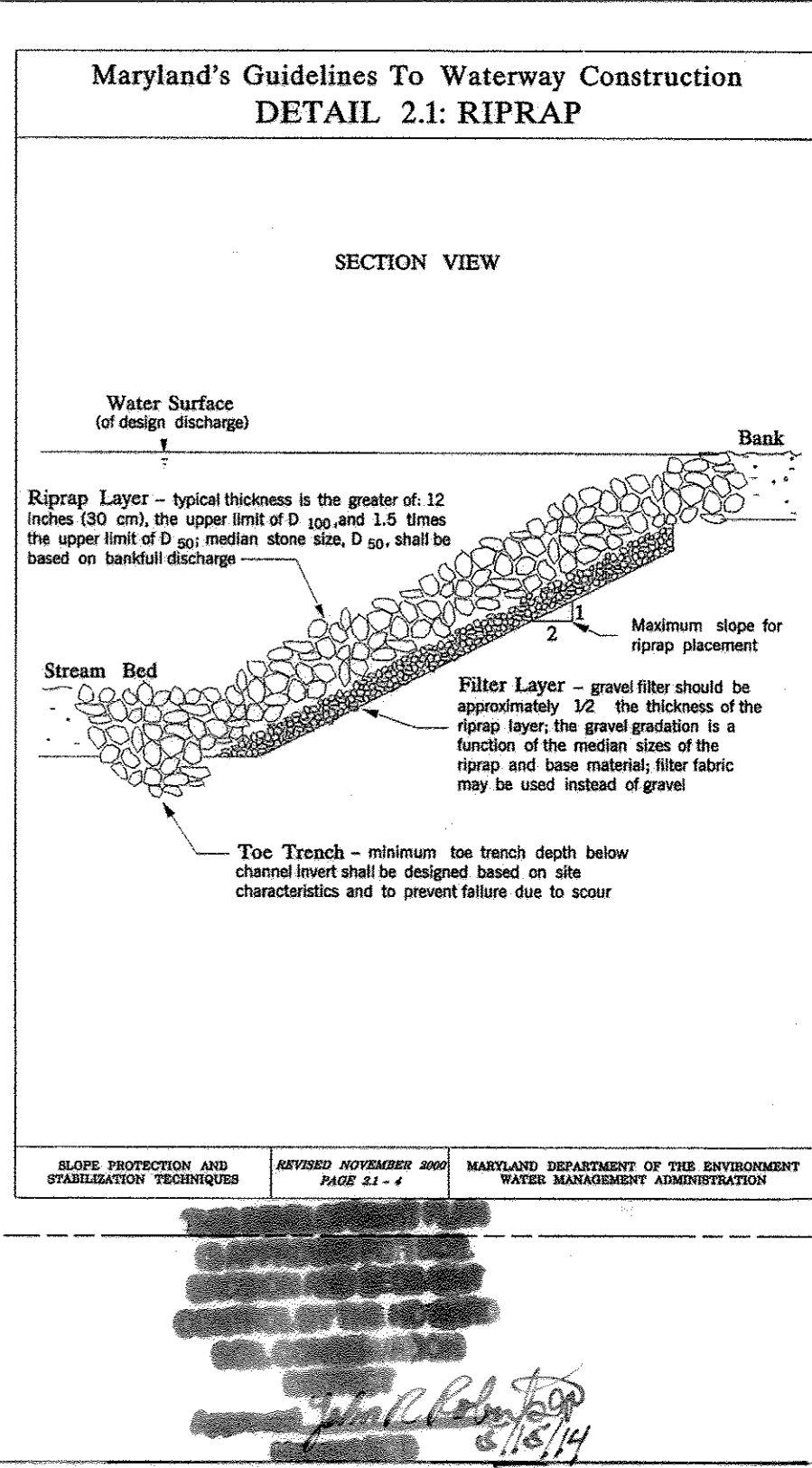
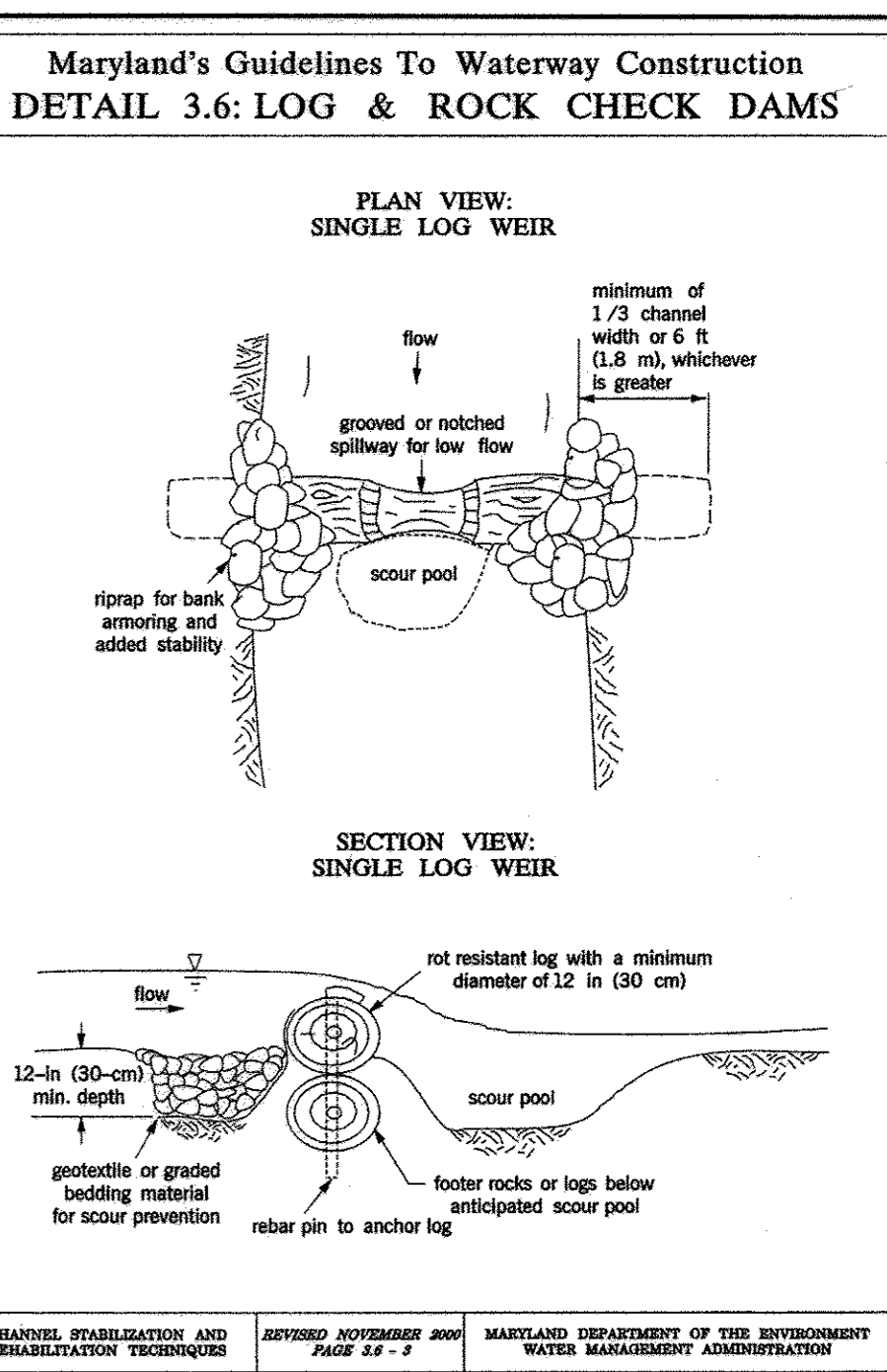
LOCATION:
TAX MAP No. 34, GRID No. 24, PARCEL No. 200
HOWARD COUNTY, MARYLAND
DPZ NO.: SP-12-004, ECP-12-045, WP-13-025

TITLE:
FINAL ROAD CONSTRUCTION PLAN
STREAM REALIGNMENT PLAN

DATE: MARCH, 2014 **PROJECT NO.:** 2171

SCALE: AS SHOWN **DRAWING:** 33 OF 33

DESIGN: JC **DRAFT:** JC



Maryland's Guidelines To Waterway Construction
DETAIL 3.6: LOG & ROCK CHECK DAMS

PLAN VIEW: SINGLE LOG WEIR

minimum of 1/3 channel width or 6 ft (1.8 m), whichever is greater

grooved or notched spillway for low flow

scour pool

riprap for bank armoring and added stability

SECTION VIEW: SINGLE LOG WEIR

rot resistant log with a minimum diameter of 12 in (30 cm)

12-in (30-cm) min. depth

scour pool

geotextile or graded bedding material for scour prevention

riprap pin to anchor log

footer rocks or logs below anticipated scour pool

Maryland's Guidelines To Waterway Construction
DETAIL 2.1: RIPRAP

SECTION VIEW

Water Surface (of design discharge)

Bank

Stream Bed

Filter Layer - gravel filter should be approximately 1/2 the thickness of the riprap layer; the gravel gradation is a function of the median size of the riprap and base material; filter fabric may be used instead of gravel

Toe Trench - minimum toe trench depth below channel invert shall be designed based on site characteristics and to prevent failure due to scour

SLOPE PROTECTION AND STABILIZATION TECHNIQUES MARYLAND DEPARTMENT OF THE ENVIRONMENT WATERWAY CONSTRUCTION GUIDELINES REVISED NOVEMBER 2000 PAGE 2.1-4