

		T	RAFFIC	CONTROL SIGN LEGEND
Symbol	Location -	€ Station	Offset	Type (
	STANSFIELD ROAD	23+87.81	30.59' Rt	ADDRESS SIGN & RI-1, "STOP" SIGN; 30" x 30" OCTAGON **
	USE-IN-COMMON	0+34	13.34¹ Rt	R4-7, "KEEP RIGHT" SIGN; 24" x 30"
	DRIVE			
	USE-IN-COMMON	0+33	9.00' Rt	"NO PARKING FIRE LANE-TOW AWAY ZONE" (LEFT ARROW) SIGN; 12" x 18"
	DRIVE			
	USE-IN-COMMON	0+93	9.00' Rt	"NO PARKING FIRE LANE-TOW AWAY ZONE" (DOUBLE ARROW) SIGN; 12" x 18"
	DRIVE			
	USE-IN-COMMON	1+53	9.00' Rt	"NO PARKING FIRE LANE-TOW AWAY ZONE" (RIGHT ARROW) SIGN; 12" x 18"
······································	DRIVE			
	USE-IN-COMMON	2+00	13.00' Rt	ADDRESS SIGN (LOTS ¢ 2)
	DRIVE	······································		

** Private range of address signs and/or private road street name signs (SNS) as well as public SNS shall be fabricated and installed by Howard County Bureau of Highways at the developer's / owner's expense. Contact Howard County Traffic Division at 410-313-5752 for details and cost estimates.



DATE

APPROVED: DEPARTMENT OF PUBLIC WORKS 1-9-09 CHIEF, BUREAU OF HIGHWAYS DATE APPROVED: DEPARTMENT OF PLANNING AND ZONING

CHIEF, DIVISION OF LAND DEVELOPMENT Charle Edmodson CHIEF, DEVELOPMENT ENGINEERING DIVISION

AND CRITICAL CONSTITUTION CONST	· \	
	-BACK OF CUES (DECUNIC TO 4 OF MONO) IS CONSOLOT	
\$===		
1/2" PREFURMED DOPARSON JUDIT	ROOM SAFACE WAS TO CONTINUE AS SACK OF CARS DESCRIPTED WAS TO SOMEWAY THAT THE SACK OF CARS DESCRIPTED WAS TO SACK DESCRIP	
And containing criss	PLAN (never)	
	TOP OF CURS IN TENSION CURS AND CUTES.	
}	ROW USE	
·	ELEVATION 7" C & G TO MODIFIED C & G DETAIL	
7" CURS AND CUTTER BACK OF CURS-		
\\	FLOW LINE	
1/z' PRETORNEO ESPHESIN JOHT	PLAN	
TOP OF CLESS	5-0°	
	ROTUE - S	
	ELEVATION * DESCRIPTION OF THE PROPERTY OF TH	EK A LX
Kirided Agen	oward County, Maryland curb AND GUTTER Detail portment of Public Works 7" Transition to Modified & R-3.02	

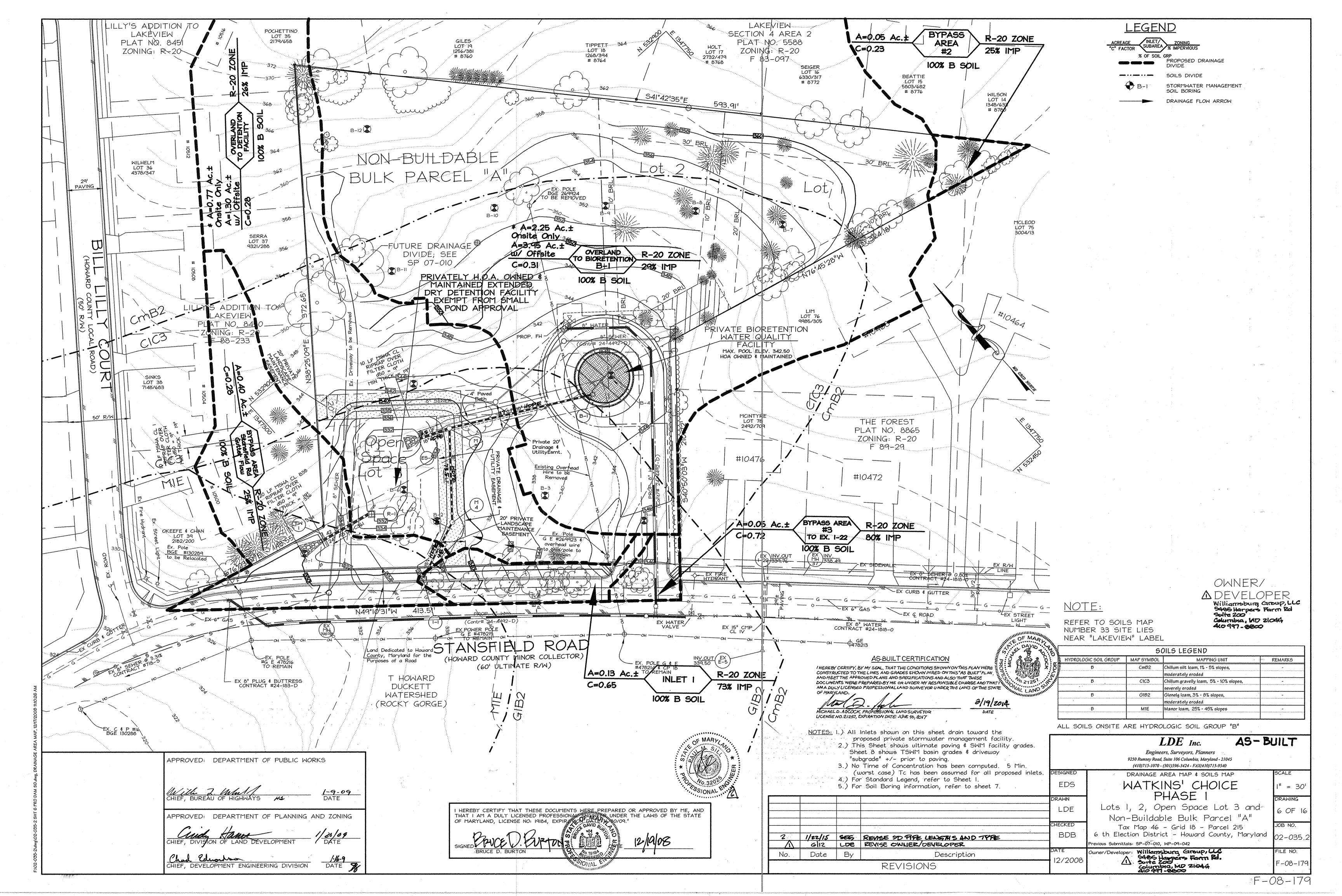
SECTION .	ROAD AND STREET	CALIFORNIA BEARING RATIO (CBR)	3 10 <	5 TO <7	≥7	3 ₹0 <	5 5 10 <7	≥ 7	
NUMBER	CLASSIFICATION	PAYEMENT MATERIAL (INCHES)	Min	MIN HWA WITH GAB			HAVA WITH CONSTANT GAB		
	PARIONS BAYS: RESIDENTIAL AND HON-RESIDENTIAL PARIONG DRIVE AISLES:	HIJA SUPERPAVE FRAL SURFACE 9.5 MM PG 6422, LEVEL 1 (ESAL)	1.5	15	1.0	1.5	1.5	1.5	
P-1	RESOUNTIAL AND NON-RESIDENTIAL WITH HO DONE-	HOMA SUPERPAVE INTERMEDIATE SURFACE (NA)	NA	NA .	NA	NA	NA.	NA.	
	THE PART INVIOLE FOR THE	19.0 MM, PG 64-22, LEVEL 1 (ESAL)	2.0	2.0	20	3.5	3.0	2.5	
		GRADED AGGREGATE BASE (GAB)	8.5	7.0	5:0	4.0	4.0	4.0	
	PARIONG DRIVE AISLES: RESIDENTIAL AND NON-RESIDENTIAL WITH NO MORE THAN 10 HEAVY TRUCKS PER DAY		1.5	1.5	1.5	1.5	1.5	1.5	
P2	LOCAL ROADS: ACESS PLACE, ACCESS STREET	HMA SUPERPAVE INTERMEDIATE SURFACE 9.5 MM, PO 64—22, LEVEL 1 (ESAL)	1.0	1.0	1.0	1.0	1.0	1.0	
	CUL-DE-SACS: RESIDENTIAL	HMA SUPERPAVE BASE 19.0 MM, PG 64-22, LEVEL 1 (ESAL)	2.0	2.0	2.0	3.5	2.0	2.0	
2) HMA SUPEI 12.5 IAM SURT 12.5 IAM SURT 4) THE INTERA PRIOR TO SUE 5) IN LIEU OF COUNTY RIGHT ADDED TO THE 6) THE CONST		GRADED AGGREGATE BASE (GAB)	8.0	4.0	3.0	4.0	4.0	4.0	
	PARKING DRIVE AISLES: RESIDENTIAL AND NON-RESIDENTIAL WITH NO MORE THAN 10 NEAVY TRUCKS PER DAY	Hua superpave final surface 9.5 kml, pg 84—22, level 1 (esal)	1.5	1.5	1.5	1,5	1.5	1.5	
P3	LOCAL ROADS: ACCESS PLACE, ACCESS STREET	HNA SUPERPAYE INTERMEDIATE SURFACE 9.5 NAI, PG 64-22, LEVEL 1 (ESAL)	1,0	1.0	1.0	1.0	1.0	1.0	
. •	CUL-DE-SACS: NON-RESIDENTIAL	HMA SUPERPAVE BASE 19.0 NBJ, PG 64-22, LEVEL 1 (ESAL)	2.0	3.0	3.0	4.5	3.0	2.0	
	MINOR COLLECTORS: RESIDENTIAL	GRADED AGGREGATE BASE (GAB)	10.0	6.0	3.0	6.0	6.0	6.0	
	MANOR COLLECTORS:	HMA SUPERPAVE FINAL SURFACE 12.5 MM, PG 64-22, LEVEL 2 (LOW ESAL)	2.0	20_		20	2.0	20	
D 4	MAJOR COLLECTORS	HIMA SUPERPAYE INTERMEDIATE SUBERICE 12.5 MU, PO. 64-68; LEVEL 2 (LOW ESAL)	2.0	20	2.0	2.0	- 20	20	
P-4		19.0 MM, PG 64-22, LEVEL 2 (LOW ESSE)	4.0	4.0	3.0	6.0	5.0	3.0	
		GRADED AGGREGATE BASE (GAB)	13.0	7.0		- 60	6.0	6.0	
1) HEAVY TRU 2) HIMA SUPEI 12.5 MM SURI 3) GRADED AC 4) THE INTERI PRIOR TO SUE 5 IN LIEU OF COUNTY RICHT ADDED TO THE	OKS ARE DEFINED AS THOSE WITH SIX (6) WHEELS OR MORE PRIVE LAYERS SHALL BE PLUCED IN APPROPRIATE COMPACTED FACE (1.5" IM 10" 3.0" MAX), AND 9.5 MM SURFACE (1.0" IM CORRECATE BASE (GAB) TO BE PLACED AND COMPACTED IN 6" I MEDIATE SURFACE COURSE LAYER MUST BE PLACED WITHIN 2 STANTIAL COMPLETION INSPECTION AND BOND REDUCTION. OF PULLUT HE INTERNIEDMENT SURFACE COURSE LAYER FOR COURSE LAYER MAY MAKER AUXILIARY LAYES ARE NOT REQUIRED, THE TE REQUIRED HACKHESS OF THE BASE ASPHALT LAYER. RICCITON DRAWINGS SHALL SHOW THE PAINING SECTION, ROAD	LIFT THOCHESS: 19.0 MM BASE (2.0" MIN TO 4.0" MAX), IN TO 2.0" MAX) MAX COMPACTED THOCHESS LAYERS. WEEKS OF PLACEMENT OF BASE COURSE, AND IS REQUIRED MIMERCUL/MOUSTRAL ENTRANCE APRONS WITHIN THE HICKORESS OF THE INTERMEDIATE PAVEMENT LAYER CAN BE			HMA SUPE	RPAVE INT	ul surfac Terwediate Se Base (Gab	SURFAC	
Howard County, Maryland		DAMMO SECTIONS	•				Deta	il	
	Department of Public Works	PAVING SECTIONS P-1 to P-4		٠.	,		R-2	01	
Valled		F-1 (0 F-4				•	Tr W	·UL	
Koléseš 5/7/2007 Approves	Approved State Square	,							

I HEREBY CERTIFY THAT THESE DOCUMENTS WERE OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSION LOFNEL BY ME LAWS OF THE STATE OF MARYLAND, LICENSE NO: 19184, EXPLANDING 15 6,30/09."
SIGNED PARTIES 12/19/08
BRUCE D. BURTON 1918 O. C. STERE
Will CONAL THE WAR THE

10NS -4			MIlliamsburg Group.LLC Sass Harpers Farm Rd. Suite 200 Columbia, MD 21044		LDE Inc. AS - B Engineers, Surveyors, Planners 9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045 (410)715-1070 - (301)596-3424 - FAX(410)715-9540	UILT
			410997-8800	DESIGNED EDS	MISCELLANEOUS DETAILS WATKINS CHOICE PHASE I	SCALE AS SHOWN
				DRAWN		DRAWING
		<u> </u>		LDE	Lots 1, 2, Open Space Lot 3 and	5 OF 16
			·		Non-Buildable Bulk Parcel "A"	
				CHECKED	Tax Map 46 - Grid 18 - Parcel 215	JOB NO.
	·			BDB	6 th Election District - Howard County, Maryland	02-035.2
Λ	6/12	LDE	REVISE OWNER/DEVELOPER		Previous Submittals: SP 07-010, WP 09-042	
٥.	Date	Ву	Description	DATE 12/2008	Owner/Developer: Williamsburg Group, LLC A 5485 Harpers Farm Ed.	FILE NO.
	REVISIONS			12/2000	(1) Suite 200 Columbia, MD 21044 410 997 - 8000	F-08-179

OWNER/

F - 08 - 179



HDPE SPECIFICATIONS

Trenches shall be excavated in such a manner as to ensure that the sides shall be stable under all working conditions. Trench walls shall be sloped or supported in conformance with all standards of safety. Only as much trench length as can be safely maintained shall be opened. All trenches shall be backfilled as soon as possible, but not later than the end of each working day.

Trench widths shall be sufficient to ensure working room to properly place and compact haunching and other backfill materials. Minimum trench width shall not be less than 1.5 times the pipe outside diameter plus 12 inches. If a trench box is used the bottom of the trench box shall be placed no lower than the top of the pipe. The minimum width of the trench, when a sub-trench is used in conjunction with a trench box, shall be three times the diameter of the pipe in order to prevent disruption of bedding material when moving the trench box.

Determination of trench width in unsupported, unstable soils shall include consideration of the size of the pipe, the stiffness of the backfill and in situ soil, and other site-specific conditions as applicable. The trench shall be excavated to the width, depth, and grade as indicated on the plans and/or given by the engineer.

Bedding Material and Structural Backfill: Bedding and structural backfill materials shall meet the requirements of ASTM D2321 as follows:

Manufactured aggregates; open graded clean Class 1A Manufactured processed aggregates; dense graded clean Class 1B Class 11 Coarse-grained soils, clean Coarse-grained soils with fines Class 111

Fine -grained soils - inorganic

Class 1VA

For all pipes greater than 24 inches in diameter only Class 1A, Class 1B, and Class 11 will

Bedding material shall have a maximum size of 1 1/2 inches. Backfill shall be free of organic material, stones larger than 1 1/2 inches in greatest dimensions, or frozen lumps. Moisture content shall be in the range of optimum content, typically minus 3 per cent to plus 2 percent, so as to permit thorough compaction. Consideration should be given to the potential for migration of fines from adjacent materials into open-graded backfill and bedding materials.

For pipe types that are not smooth on the outside, i.e. pipes with corrugated or profile walls, backfill gradations should be selected that will permit the filling of the corrugation of profile wall. Flowable fills, such as controlled low strength mortar, CLSM, or controlled density fill, CDF, may be used for backfill provided adequate flotation resistance can be achieved by restraints, weighting or placement technique. With CLSM backfill, trench width can be reduced provided an engineer for the contractor has performed a satisfactory analysis of the combined CLSM/in situ soil strength. When CLSM is used all joints must be

The following specifications detail material, installation and acceptance requirements for thermoplastic pipe used for storm sewer and drainage applications.

MATERIAL SPECIFICATIONS

Thermoplastic Pipe: All thermoplastic pipe materials shall conform to the workmanship and inspection requirements of:

- ASTM F679 Standard Specification for PolyVinyl Chloride (PVC) Large-Diameter Plastic Gravity Sewer Pipe and Fittings - 18" to 48"
- ASTM F 794 Standard Specification for PolyVinyl Chloride (PVC) Profile Gravity Sewer Pipe and Fittings Based on Controlled Inside Diameter - 4" to 48" ASTM F949 –Standard Specification for PolyVinyl Chloride (PVC) Corrugated Sewer Pipe With a Smooth Interior and Fittings - 4" to 36"
- AASHTO M294 Standard Specification for Corrugated Polyethylene Pipe 12"

except, that all thermoplastic pipe supplied under this contract will have a minimum pipe stiffness of 46psi at 5% deflection when tested in accordance with ASTM D 2412.

A Professional engineer registered in the State of Maryland shall provide a signed and sealed letter to the Engineer. This letter will certify that High Density Polyethylene Pipe -HDPE - supplied under this project was manufactured from raw materials in accordance with Section 6 - Materials - AASHTO M294, and that the material met or exceeded the Slow Crack Resistance Growth requirement of Section 9.5 of AASHTO M294.

INSTALLATION SPECIFICATIONS

General Installation Requirements: The installation of all thermoplastic pipe will be in accordance with ASTM D 2321 and as described in this specification. United States Department of Labor Occupational Safety & Health Administration Standard Number 1926.652 shall be observed at all times and shall supercede any requirement of this

A trench box or shoring system shall be used in excavations greater than 5 feet in depth. Excavations of earth material to a level not greater than two feet below the bottom of the trench box or shoring system shall be permitted, but only if the trench box is designed to resist the forces calculated for the full depth of the trench and there are no indications while the trench is open of a possible loss of soil from behind or below the bottom of the trench box.

Structural Backfill: Structural backfill shall:

Re-rounding will not be allowed.

· be placed in layers not exceeding 8 inches loose lift thickness

permitted without written permission from the Engineer.

- brought up evenly and simultaneously on both sides of the pipe · be to an elevation not less than twelve inches above the top of the pipe
- be worked into the haunch area and compacted by hand have a minimum compaction level of 90 percent standard proctor density • All compaction equipment used within 36 inches of the pipe shall be approved by
- Ponding or jetting the structural backfill to achieve compaction shall not be

Connection of Flexible Pipe to Manholes The installer shall use flexible water stops, resilient connectors, or other flexible systems approved by the project engineer to make watertight connections to manholes and other structures. Grouting between the thermoplastic pipe and the manhole shall not be permitted.

ACCEPTANCE SPECIFICATIONS

For all thermoplastic pipe materials short and long-term deflection shall be less than 5% of the pipe's actual internal diameter measured prior to installation. Short-term deflection shall be deemed to be any deflection measured not sooner than 30 days after backfilling and installation up to 1 year after backfilling and installation. Deflection tests shall not be carried out sooner than 30 days after installation and backfilling is complete to assess short term deflection and not sooner than 11 months to

assess long-term deflection. A total of two tests will be carried out. All tests shall be carried out in the presence of the Project Engineer or his representative Any section of pipe failing the test will be uncovered and reinstalled. Any damaged pipe will be replaced with new pipe at the contractor's expense. Reinstalled pipe will be subject to deflection testing as per this specification.

APPROVED: DEPARTMENT OF PUBLIC WORKS

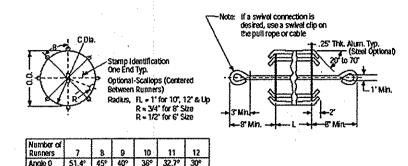
CHIEF, BUREAU OF WICH

BUREAU OF HIGHWAYS

CHIEF, DEVELOPMENT ENGINEERING DIVISION

All pipe up to and including 36 in. diameter shall be inspected with a go/no-go mandrel device as described herein. Pipe larger than 36 in. diameter shall be inspected with a suitable proving device to confirm that vertical deflection does not exceed either the maximum allowable short or long term deflection limits. The mandrel or proving device shall be pulled through the pipe in such a manner so as to ensure that excessive force is not used to advance the device through any deflected

The mandrel shall be cylindrical in shape, constructed with 9 evenly spaced arms and shall generally conform to Figure A1.

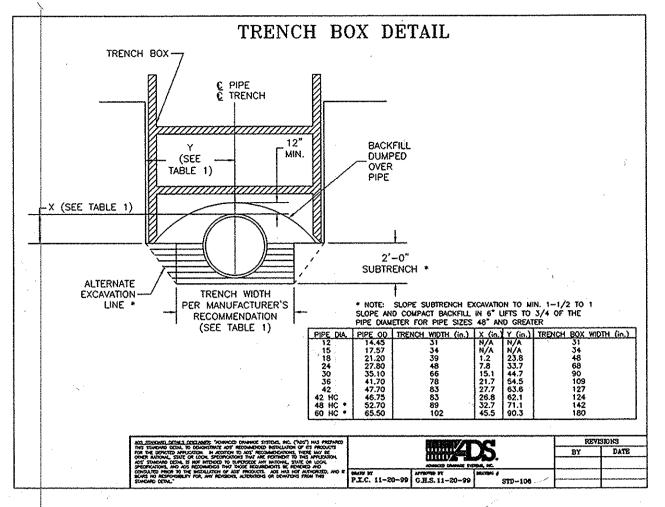


Mandrels larger than 18 in. in diameter shall be constructed of special breakdown devices to facilitate entry through access manholes.

Mandrel Dimensional Requirements The minimum diameter of the circle scribed around the outside of the mandrel arms shall be equal to the values for each specific pipe material, within a tolerance of +/- 1/100th inch. The contact length of the mandrel shall be measured between the points of contact, on the mandrel arm as indicated in Figure A1. The outside radius of the mandrel arms shall be checked for conformance with these specifications with a proving ring.

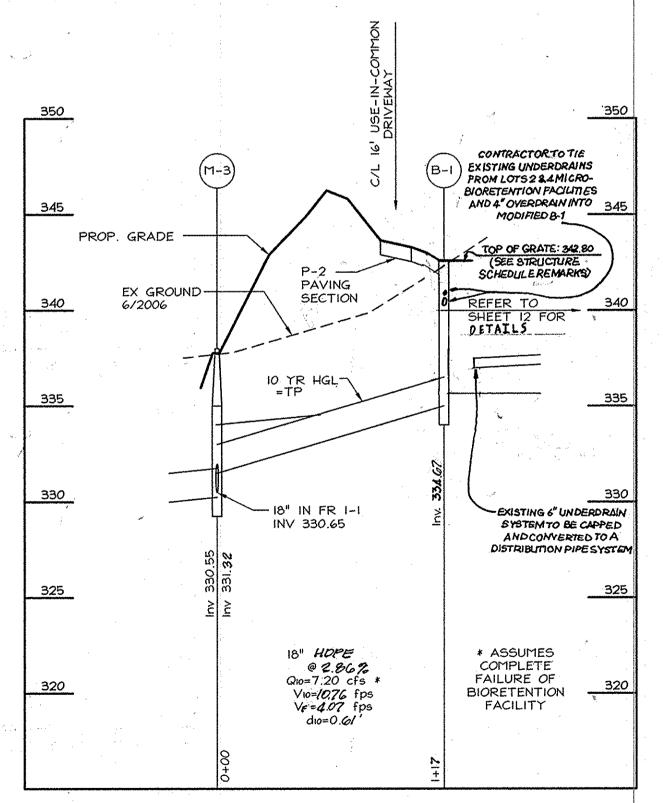
An oversized proving ring may be used, which shall be manufactured to a diameter equal to the outside diameter of the mandrel plus, to facilitate undertaking measurements to confirm that the size of the mandrel conforms the dimensions and dimensional tolerances specified herein. The proving ring shall be manufactured to within 1/100th inch of the specified size. The proving ring shall be fabricated from ¼ inch minimum thick steel.

As an alternative, a go/no-go proving ring device shall be permitted in which case the proving ring shall be sized up to 1/100th inch less than the circle that would be scribed by the specified mandrel size. If a go/no-go proving ring is utilized, an acceptable mandrel will not be able to pass through the proving ring. Go/no-go proving rings shall not be less than 1/100th inch of the specified dimension.

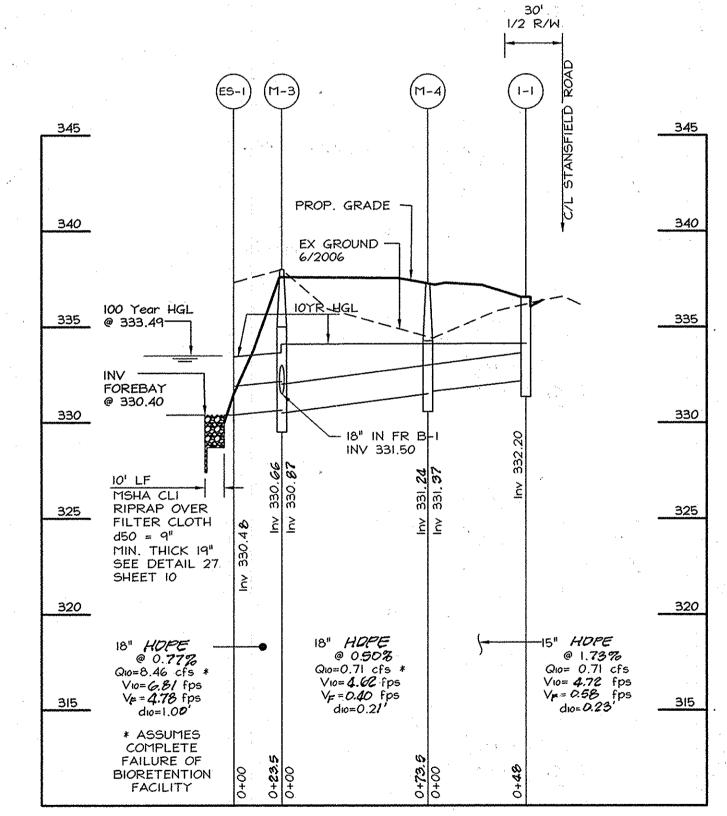


(2) Coordinate for End Section = Centerline of Pipe @ Downstream Face

water promorphisms		,	STR	UCTURE SCI	HEDULE		
Str. No.	Structure Type	Inv. In	Inv. Out	Top Elevation or Upper / Lower	Detail	Location	Remarks
I-2	A-5 (OR EQUIVALENT)	-	332.20 336.60	336.62/336.10 342.50	D-4.01	See Plan View	
M-3 M-4	48" Manhole 48" Manhole	330.65/331.50 331.65	330.55 331.40	337.87 336.24	G 5.12 G 5.12	See Plan View See Plan View	(1) N 532778.74 E 1347406.54 (1) N 532720.74 E 1347357.54
ES-1 EW-1	18" End Scetion "C" Endwall	330.40 330.00	330.40 330.00	301.90 332.75	D-5.51 D-5.21	See Plan View See Plan View	(2) 18" End Section (2) 24" Conc. Endwall
B-1	MODIFIEDD *		335.00	342.82.*	D-4.10	See Plan View	(1) N 532721.76 E 1347510.40 ** Contractor to remove *D'inle TOP AND REPLACE WITH GRATE INLE TOP AND BRICK TO GRADE AS



Storm Drain Profile: M-3 thru B-1 Scale: Hor. 1"=50' Ver. 1"=5"



Storm Drain Profile: ES-2 thru I-I Scale: Hor. 1"=50" Ver. 1"=5"

AS-BUILT CERTIFICATION I HERERY CERTIPY, BY MY SEAL, THAT THE CONDITIONS SHOWN ON THIS PLAN WERE CONSTRUCTED TO THE LINES AND GRADES SHOWN IN RED ON THIS "AS-BUILT" PLAN.

AND MEET THE APPROVED PLANS AND SPECIFICATIONS AND ALSO THAT THESE DOCUMENTS WERE PREPARED BY ME OR UNDER MY RESPONSIBLE CHARGE AND THAT I AMA DULY LICENSE DPROFESSIONALLAND SURVEYOR UNDER THE LAWS OF THE STATE MICHAEL D. ADCOCK, PROFESSIONAL LAND SURVEYOR LICENSE NO. 21257, EXPIRATION DATE: JUNE 16, 2017

PIPE SCHEDULE

HOPE

Total Length

214

Size

OWNER/ △ DEVELOPER Williamsburg Group, LC 5405 Harpers Farm Rd. Suite 200 Columbia, MD 21044 410 997-8800

AS - BUILT

ANTICIPATED

PERC RATE

ANTICIPATED

PERC RATE

IN. / HR.

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CLASS

CLASS

CLASS

CLASS

SW-SM

CLASS

CLASS

GM - MH

GM

CLASS

USDA

CLASS

N/A

CLASS

SP -- SM

USDA CLASS

DESCRIPTION

Rusty, tan and brown micaceous loamy sand with iron ore frags, damp to moist

Quartziferous sandstone and iron ore mixed with brown loamy micaceous sand

DESCRIPTION

Tan and rusty micaceous sandy loam, moist, trace clay and gravel

0 - 0.5'

2.5' - 4.0'

0 - 0.75'

0.75' - 1.5'

6,5' - 10.0' 5.0' - 6.5'

0 – 1.0'

0 - 1.0'

0 -- 0.3'

0 - 0.5

10.0' - 11.5' Variegated loamy sand

Loamy Topsoil

Hand auger refusal

Brown micaceous loamy clay - moist (likely fill)

Brown and rusty micaccous clayey sand - moist

Dark grey to tan micaceous wet silty clay (likely fill)

Tan silty micaceous clay loam, moist

Plowed topsoil - gravelly, clayey loam

3.0' - 5.0' Black, orange, tan and rust micaceous loamy sand (damp)

1.0' - 3.0' Tan and rusty micaceous loamy sand, trace clay (moist)

1.0' - 4.0' Brown and rusty micaceous clayey sand loam (moist)

Brown and rusty micaceous silty sand loam (moist)

Topsoil - clayey sand and gravel (moist)

4.0'-9.0' Reddish brown micaceous silty sand with some gravel (damp)
9.0'-12.0' Variegated micaceous loamy sand (dry)

Tan gravelly sandy silt, some mica (damp to dry)

7.0' - 12.0' Brown to variegated micaccous gravelly loamy sand (dry)

6.5' - 11.0' Rusty brown micaceous sandy silt with some gravel (dry)

0.3'-5.0' Tan micaceous loamy sand and gravel (damp to dry)

6.0' - 11.0' Quartziferous rocks mixed with tan to red micaceous loamy sand (dry)

Tan micaceous gravelly sandy clay loam (damp to dry) Tan, beige and gold micaceous silty sand loam (dry) 7.0' - 9.0' Variegated micaceous silty sand mixed with quartziferous rocks (dry)

Brown micaceous clayey silt mixed with quartziferous iron ore rocks, (damp)

(No Topsoil)

DESCRIPTION

Mixed quartziferous rock and tan and brown micaceous loamy silt (damp)

0.3'-3.0' Tan loarny gravelly silt - 1/2" thick iron ore layer underlain with soil, some mica dry)

5.0' - 11.0' Greyish micaceous loamy sand (dry)

0.0 – 1.0' Plowed loamy moist topsoil with gravel

4.0'-6.0' Brown and rusty micaceous silty sand loam (m
6.0'-6.5' Grey layer - micaceous loamy sand (damp)
6.5'-11.0' Brown and rusty micaceous loamy sand (dry)

1.0'-3.5' Light brown micaceous sandy clay (moist)

3.5'-4.0' Light brown micaceous clayey sand (damp)

Topsoil - gravelly silt loam (moist)

0 - 0.3' Topsoil - gravelly silt loam (moist)

3.0' - 6.5' Tan to reddish micaceous silty sand (dry)

Loamy topsoil (moist)

Quartziferous iron ore layer

Loamy topsoil (moist)

9.0' - 11.0' Tan to red micaceous clayey sand (dry)

Silty topsoil (moist)

8.0' - 11.0' Variegated micaceous clayey sand (moist

0 - 0.5' Tan and orange silty gravelly clay (wet)

3.5' - 5.0 Tan to brown micaceous loam sand (damps)

9.5'-11.0' Tan to orange micaceous loamy sand (dry)

0.5'-3.0' Tan micaceous gravely sand silt (moist)
3.0'-5.5' Tan to brown micaceous clayey silt (moist)

5.5' - 9.0' Variegated micaceous clayey sand (moist)
9.0' 1/2" thick quartziferous iron ore
9.0' - 12.0' Grey micaceous clayey sand (damp)

Topsoil (moist)

6.0' - 13.0' Brown to grey micaceous silty sand (dry)

5.0' - 9.0' Reddish micaceous loamy sand (dry)
9.0' - 9.5' Quartziferous oxidized iron ore with some loamy sand (dry)

Brown loamy silt, some micaceous (moist)

Reddish brown micaceous silty fine sand and gravel (damp

0.5' - 3.5' Tan gravelly silt, quartzite (moist

0.0 - 0.5' Gravelly topsoil (moist)

6.5' - 8.0' Tan micaceous clayey silt (moist)

0.5'-2.0' Tan micaceous gravelly silt with some clay (damp)

Brown micaceous toathy stir, toans

5.0'-6.0' Tan micaceous loamy sand (dry)

4.0' - 5.0' Yellowish tan micaceous silty clayey sand, moist

Brown micaceous silty / sandy loam, moist

	Engineers, Surveyors, Planners 9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045 (410)715-1070 - (301)596-3424 - FAX(410)715-9540	
DESIGNED	STORM DRAIN PROFILES & SOIL BORINGS	SCALE
EDS	WATKINS' CHOICE	l" = 5' \ l" = 50'
ORAWN	PHASE I	DRAWING
LDE	Lots 1, 2, Open Space Lot 3 and	7 OF 1
CHECKED	Non-Buildable Bulk Parcel "A" Tax Map 46 - Grid 18 - Parcel 215	JOB NO.
BDB	6 th Election District - Howard County, Maryland	02-035
	Previous Submittals: SP-07-010, WP-09-042	
DATE	Owner/Developer: Williamsburg Group, U.C.	FILE NO.

LDE Inc.

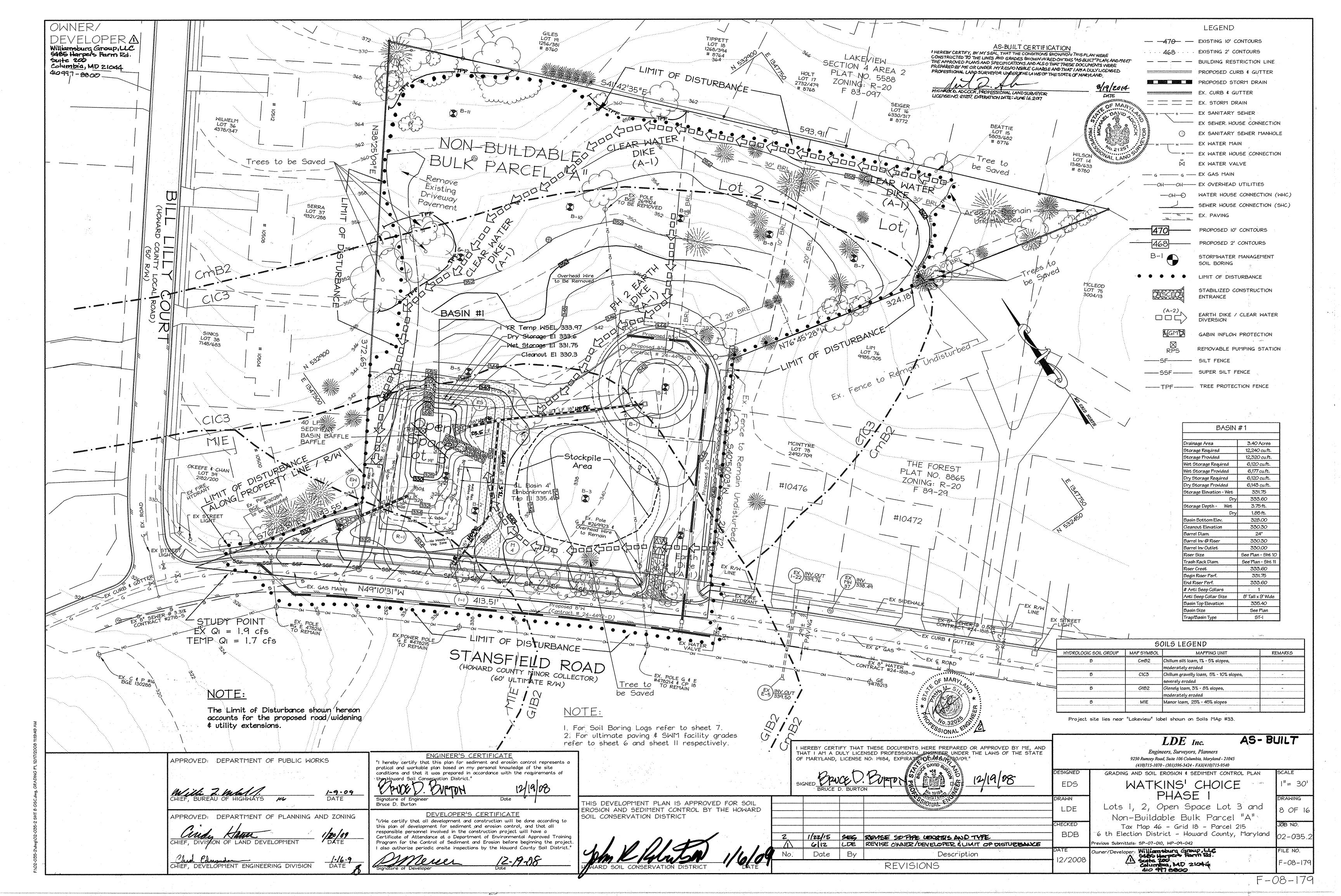
APPROVED: DEPARTMENT OF PLANNING AND ZONING

I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSION DENGINEER UNDER THE LAWS OF THE STATE OF MARY LAND, LICENSE NO: 19184, EXPLOYED TO THE STATE

REVISE SO PIPE COMPS, LENGTH & TYPE SAA REPLACE BIORETENTION FACILITY WITH STONE STORAGE-REVISE INLET B-1 & ADDA-2. LDE REVISE OWNER/DEVELOPERS Owner/Developer: Williamsburg Group, LLC 5485 Harpers Farm Ed. Suite 200 Columbia, MD 21044 410 997-8800 Date By No. Description REVISIONS

F-08-179

F-08-179



HOWARD SOIL CONSERVATION DISTRICT STANDARD SEDIMENT CONTROL NOTES

1. A minimum of 48 hours notice must be given to the Howard County Department of Inspections, Licenses and Permits, Sediment Control Division prior to the start of any construction, (313–1855)

2. All vegetative and structural practices are to be installed according to the provisions of this plan and are to be in conformance with the most current "MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL", and revisions thereto.

3. Following initial soil disturbance or redisturbance, permanent or temporary stabilization shall be completed within:

a) 7 calendar days for all perimeter sediment control structures, dikes, perimeter slopes and all slopes greater than 3:1,

b) 14 days as to all other disturbed or graded areas on the project site. 4. All sediment traps/basins shown must be fenced and warning signs posted around their perimeter in accordance with Vol. 1, Chapter 7, of the HOWARD COUNTY DESIGN MANUAL, Storm Drainage

5. All disturbed areas must be stabilized within the time period specified above in accordance with the 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL (Section G) for permanent seeding, sod, temporary seeding, and mulching. Temporary stabilization with mulch alone can only be done when recommended seeding dates do not allow for proper germination and establishment of grasses.

6. All sediment control structures are to remain in place and are to be maintained in operative condition until permission for their removal has been obtained from the Howard County Sediment Control Inspector. 7. Site Analysis:

lotal Area of Site	3.50	Acres	
⚠Area Disturbed	3.50	Acres	
Area to be roofed or paved	0.60	Acres	
Area to be vegetatively stabilized	2.90	Acres	
Total Cut	2,100	Cu. Yds.	*
Total Fill	6,100	Cu. Yds.	¥
Need	4,000	Cu. Yds.	*
	•		

* Contractor shall complete their own earthwork analysis - See Note 39 Sheet I

Offsite waste/borrow area location N/A

8. Any sediment control practice which is disturbed by grading activity for placement of utilities must be repaired on the same day of disturbance. 9. Additional sediment control must be provided, if deemed necessary by the Howard

County Sediment Control Inspector. 10. On all sites with disturbed areas in excess of 2 acres, approval of the inspection agency shall be requested upon completion of installation of perimeter erosion and sediment controls, but before proceeding with any other earth disturbance or grading Other building or grading inspection approvals may not be authorized until this initial approval by the inspection agency is made.

11. Trenches for the construction of utilities is limited to three pipe lengths or that which can be back filled and stabilized within one working day, whichever is shorter.

HOWARD SOIL CONSERVATION DISTRICT PERMANENT SEEDING NOTES

Apply to graded or cleared areas not subject to immediate further disturbance where a permanent long-lived vegetative cover is needed.

SEEDBED PREPARATION:

Loosen upper three inches of soil by raking, disking, or other acceptable means before seeding, if not previously loosened.

SOIL AMENDMENTS:

In lieu of soil test recommendations, use one of the following schedules: 1) PREFERRED Apply 2 tons per acres dolomitic limestone (92 lbs/1000sq, ft.) and 600 lbs per acre 10-10-10 fertilizer (14 lbs/1000 sq. ft.) before seeding. Harrow or disk into upper three inches of soil. At time of seeding, apply 400 lbs per acre 30-0-0 ureaform fertilizer (9 lbs/1000sq.

2) ACCEPTABLE Apply 2 tons per acres dolomitic limestone (92 lbs/1000sq, ft.) and 1000 lbs per acre 10-10-10 fertilizer (23 lbs/1000 sq. ft.) before seeding. Harrow or disk into upper three inches of soil,

SEEDING

For the periods March I thru April 30, and August I thru October 15, seed with 60 lbs per acre (1.4 lbs/1000sq, ft.) of Kentucky 31 Tall Fescue. For the period May I thru July 31, seed with 60 lbs per acre (1.4 lbs/1000sq, ft.) of Kentucky 31, Tall Fescue and 2 lbs. per acre (.05 lbs/1000sq, ft.) of weeping lovegrass.

During the period of October 16 thru February 28, protect site by: Option (1) - 2 tons per acre of well anchored straw mulch and seed as soon as possible in the spring.

Option (2) - Use sod. Option (3) - Seed with 60 lbs. per acre Kentucky 31 Tall Fescue and mulch 2 tons / acre well anchored straw.

MULCHING

Apply 1-1/2 to 2 tons per acre (70 to 90 lbs/1000sq, ft.) of unrotted small grain straw immediately after seeding. Anchor mulch immediately after application using mulch anchoring tool or 218 gallons per acre (5 gal/1000sq, ft.) of emulsified asphalt on flat areas. On slopes 8 feet or higher, use 348 gallons per acre (8 gal/1000sq, ft.) for anchoring.

MAINTENANCE

Inspect all seeding areas and make needed repairs, replacements and reseedings.

AS-BUILT CERTIFICATION

Willie 7. Melal

APPROVED: DEPARTMENT OF PUBLIC WORKS

DIVISION OF LAND DEVELOPMENT

CHIEF, DEVELOPMENT ENGINEERING DIVISION

APPROVED: DEPARTMENT OF PLANNING AND ZONING

THERE IS NO "AS BUILT" INFORMATION PROVIDED ON THIS SHEET.

MICHAEL D. ADCOCK, PROFESSIONAL LAND SURVEYOR LICENSENO. 21257, EXPIRATION DATE: JUNE 16,2017

HOWARD SOIL CONSERVATION DISTRICT TEMPORARY SEEDING NOTES

Apply to graded or cleared areas likely to be redisturbed where a short-term vegetative cover is needed.

SEEDBED PREPARATION:

Loosen upper three inches of soil by raking, disking, or other acceptable means before seeding, if not previously loosened.

SOIL AMENDMENTS:

Apply 600 lbs per acre 10-10-10 fertilizer (14 lbs/1000sa. ft.)

For periods March 1 thru April 30, and from August 15 thru October 15 seed with 2-12 bushels per acre of annual rye (3.2 lbs/1000sq, ft.). For the period May I thru August 14, seed with 3 lbs. per acre of weeping lovegrass (.07 1bs/1000sq, ft.), For the period November 16 thru February 28, protect site by applying 2 tons per acre of well anchored straw mulch and seed as soon as possible in the spring, or use sod.

MULCHING

Apply 1-1/2 to 2 tons per acre (70 to 90 lbs/1000sq, ft.) of unrotted weed free small grain straw immediately after seeding. Anchor mulch immediately after application using mulch anchoring tool or 218 gallons per acre (5 gal/1000sq, ft.) of emulsified asphalt on flat areas. On slopes 8 feet or higher, use 348 gallons per acre (8 gal/1000sq, ft.) for anchoring.

Refer to the 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL for additional rates and methods not covered.

SEQUENCE OF CONSTRUCTION

NOTE:

Contractor shall contact the Construction Inspection Division 24 hours in advance of commencement of work at 410-313-1880

PHASE

1. Obtain Gradina Permit. - I day

2. Stakeout limits of disturbance. - I day 3. Install stabilized construction entrance with mountable berm, where shown hereon. - I day

4. Install silt fence \$ super silt fence along the north side of Stansfield Road. - 2 days 5. Install water main and sewer extensions in accordance with Contract # 24-4492-D. Water main shall loop / connect existing Contract 24-1830-D to the west of the site and existing Contract 24-1818-D to the east. This work shall include extension into Watkins' Choice across Stansfield Road and includes

service connections. - 3 weeks 6. Only that portion of water main which can be installed and backfilled on any given day shall be started. - Daily 7. During open cutting of Stansfield Road for water \$ sewer extensions, if work is to continue to the

next day, steel plates shall be used to allow traffic to flow uninterrupted. - Daily 8. When offsite work is completed; ie water main extension, immediately stabilize any and all disturbed areas with permanent seed mixture \$ straw mulch or sod. - I day

9. With sediment controls in place and with permission from sediment control inspector, install BASIN outfall from EW-1 to R-1 and rip rap outfall. See Profile Along CL of Embankment and Profile Along CL of Principal Spillway on sheet 11. - 5 days 10. Immediately protect disturbed areas around outfall, within Limits of Disturbance with sod. - 2 days 11. With all offsite work completed, provide any maintenance on silt fence \$ super silt fence along edge of Stansfield Road and complete construction of BASIN per specifications and design data on sheet II

(Profile Along CL of Embankment and Principal Spillway) and Riser Details on Sheet 10. Stabilize basin

with seed and straw mulch. - 3 days 12. With BASIN in place, install remainder of perimeter controls to include Clearwater Dike along western boundary of site. This control convey clean offsite water toward the Basin as well as dirty water. Install perimeter sediment controls as directed by sediment control inspector, to include Silt Fence and Super Silt Fence. Again, earth dike will direct sediment laden water toward the sediment basin. All Dikes shall be installed where shown hereon or as directed by the sediment control inspector. - 3 days 13. The sediment basin shall be dewatered by pumping. The accumulated sediment from the structure shall be placed up arade from the structure in such a manner as not to interfere with construction operations or cause erosion down grade from the structure. - 2 days

14. The sediment shall be removed from the structure when the cleanout elevation has been

15. With the sediment basin in place and with permission from the sediment control inspector, beain construction of the proposed widening for Stansfield Road. - 2 weeks 16. Install remainder of storm drain system. - I week 17. With Stansfield Road widening / grading complete, install curb \$ gutter and stone base, in accordance

with details hereon. - 2 days 18. With curb ¢ gutter in place, backfill curb ¢ gutter, install sidewalk and base course paving. - I week 19. Stabilize any disturbance associated with Stansfield Road widening with permanent seed mixture \$

20. Complete remainder of onsite grading operations to construct private use-in-common driveway and parking area and site mass grading as shown hereon. - 2 weeks 21. Construct Homes on Lots 1 \$ 2. - 3 months

22. Grading should occur in accordance with the requirements of the Dust Control Schedule shown on 23. Install the private use-in-common driveway \$ parking areas stone base and base coat paving. - 2

24. Backfill any areas along driveway work and stabilize all disturbed areas with permanent seed mixture 25. Mill existing 1/2 section of Stansfield Road and install final coat paving along Stansfield Road widening -

26. Upon completion of: a.) home construction on Lot 1 \$ 2, b.) with a 2" stand of grass on Lots ! \$ 2 \$ areas along eastern edge of site and around

use-in-common driveway c.) with a 5 day clear weather forecast

d.) with permission from sediment control inspector

Proceed to Phase 2.

PHASE 2

1-9-09

27. Install interior Phase 2 Earth Dike to direct "dirty" water away from flowing toward the "Bioretention Facility". - I day

28. Begin construction of the proposed Bioretention facility and conversion of sediment basin to Stormwater Facility in accordance with details hereon. - I week 29. Basin Conversion to Stormwater Facility includes re-grading facility, construction of retaining walls,

construction of foreboy and riser conversion to that shown on Sheet II. - 4 days 30. Upon completion of basin conversion to Stormwater Facility, stabilize any disturbances with permanent seeding mixture \$ straw mulch. 31. With permission from sediment control inspector, remove any remaining perimeter controls and

stabilize any disturbed areas with permanent seed mixture \$ straw mulch or sod. - I day

ENGINEER'S CERTIFICATE

conditions and that it was prepared in accordance with the requirements of

DEVELOPER'S CERTIFICATE

I/We certify that all development and construction will be done according to

Certificate of Attendance at a Department of Environmental Approved Training

Program for the Control of Sediment and Erosion before beginning the project

also authorize periodic onsite inspections by the Howard County Soil District."

this plan of development for sediment and erosion control, and that all

responsible personnel involved in the construction project will have a

pratical and workable plan based on my personal knowledge of the site

the Howard Soil Conservation District."

Signature of Engineer

Bruce D. Burton

I hereby certify that this plan for sediment and erosion control represents a

NOTE:

Quantities are provided for informational purposes only and are based upon comparison of existing ground to proposed arades shown hereon. Contractor to make his own analysis prior to placing a bid on grading work / earthwork.

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT

DETAIL I- EARTH DIKE

POSITIVE DRAINAGE Q-DIKE BEIGHT 24"

FLOW CHANNEL STABILIZATION GRADE 0.5% MIN. 10% MAX. STANDARD SYMBOL

Construction Specifications

1. All temporary earth dikes shall have uninterrupted positive grade

to an outlet. Spot elevations may be necessary for grades less

2. Runoff diverted from a disturbed area shall be conveyed to a

3. Runoff diverted from an undisturbed area shall outlet directly

4. All trees, brush, stumps, obstructions, and other objectional

5. The dike shall be excavated or shaped to line, grade and cross

7. All earth removed and not needed for construction shall be placed

so that it will not interfere with the functioning of the dike

8. Inspection and maintenance must be provided periodically and after

with the proper functioning of the dike.

6. Fill shall be compacted by earth moving equipment.

material shall be removed and disposed of so as not to interfere

section as required to meet the criteria specified herein and be

free of bank projections or other irregularities which will impede

into an undisturbed, stabilized area at a non-erosive velociti

PLAN VIEW

2. Seed and cover with Erosian Control Matting or line with sod.

3. 4" - 7" stone or recycled concrete equivalent pressed into

. Seed and cover with straw mulch.

sediment trappina device

-EXCAVATE TO PROVIDE

a-DIKE HEIGHTI8"

c-FLOW WIDTH4'

d-FLOW DEPTH2"

AT DESIGN FLOW DEPTH

DIKE A DIKE B

2:1 SLOPE OR FLATTER

CUT OR FILL SLOPE

GRADE LINE

NOTE: FENCE POST SPACING SHALL NOT EXCEED 10' CENTER TO CENTER 21/2" DIAMETER CHAIN LINK FENCE WITH I LAYER O FILTER CLOTH-EMBED FILTER CLOTH 3" MINIMUM INTO GROUND *IF MULTIPLE LAYERS ARE -- SSF --Construction Specifications I. Fencing shall be 42" in height and constructed in accordance with the latest Maryland State Highway Details for Chain Link Fencing. The specification for a 6' fence shall be used, substituting 42" fabric and 6' length posts. 2. Chain link fence shall be fastened securely to the fence posts with wire ties. not required except on the ends of the fence. 3. Filter cloth shall be fastened securely to the chain link fence with ties spaced 4. Filter cloth shall be embedded a minimum of 8" into the ground. 5. When two sections of filter cloth adjoin each other, they shall be overlapped 6. Maintenance shall be performed as needed and silt buildups removed when "bulges" 7. Filter cloth shall be fastened securely to each fence post with wire ties or staples at top and mid section and shall meet the following requirements for Geotextile Class F: Test: MSMT 509 Tensile Strength 50 lbs/in (min.)

20 lbs/in (min.)

0.3 gal/ft/minute(max

75 % (min.)

Tensile Modulus

I.S. DEPARTMENT OF AGRICULTUR SOIL CONSERVATION SERVICE

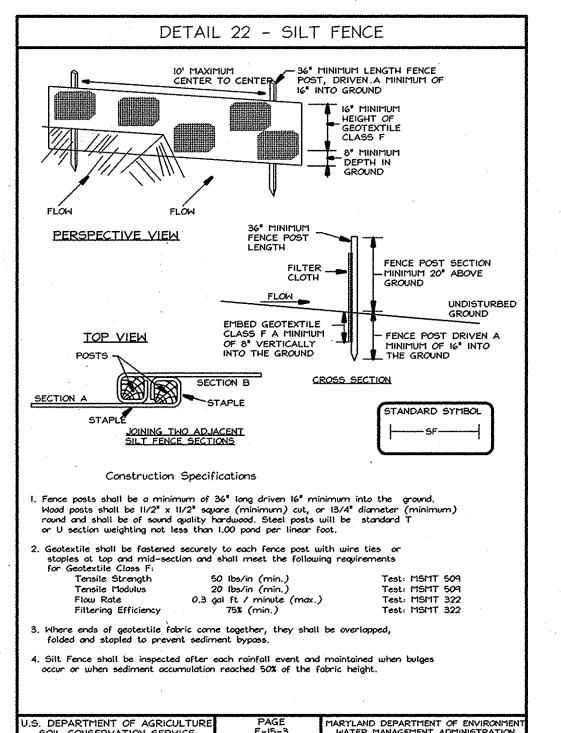
Filtering Efficiency

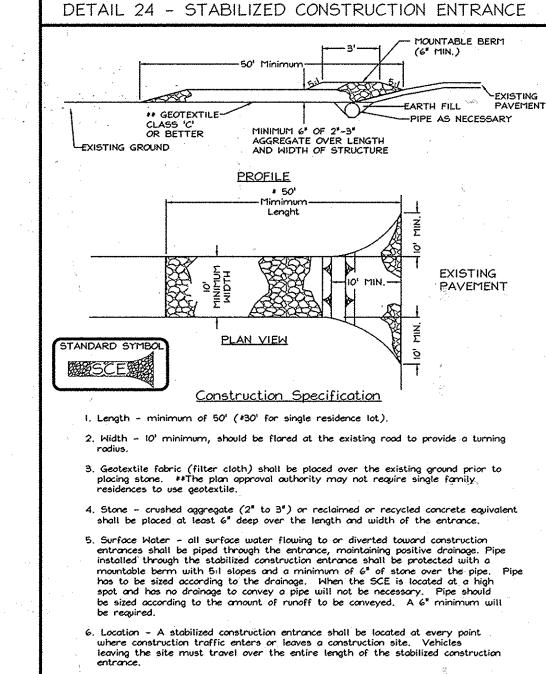
Test: MSMT 509

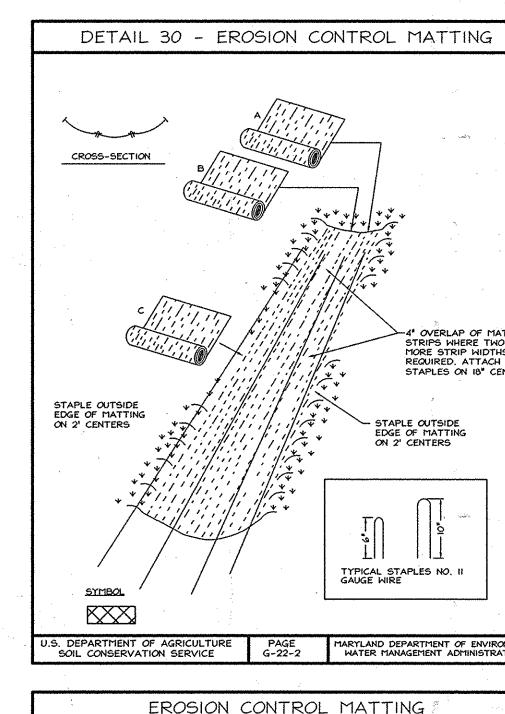
Test: MSMT 322

Test: MSMT 322

DETAIL 33 - SUPER SILT FENCE

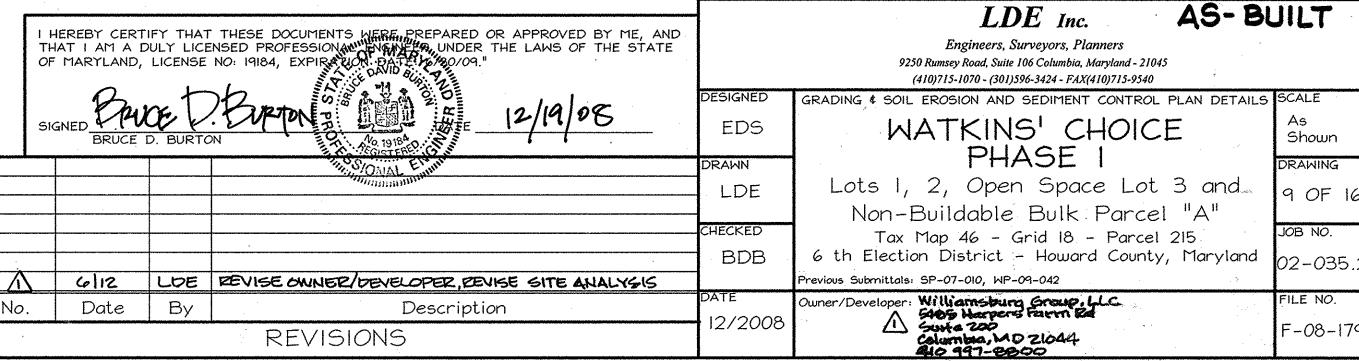






Construction Specifications 1. Key-in the matting by placing the top ends of the matting in a narrow trench, 6" in depth. Backfill the trench and tamp firmly to conform to the channel cross-section. Secure with a row of staples about 4" down slope from the trench. Spacing between staples is 6". 2. Stople the 4" overlap in the channel center using an 18" spacing between staples. 3. Before stapling the outer edges of the matting, make sure the matting is smooth and in firm contact with the soil. 4. Staples shall be placed 2' apart with 4 rows for each strip, 2. outer rows, and 2 alternating rows down the center. 5. Where one roll of matting ends and another begins, the end of the top strip shall overlap the upper end of the lower strip by 4", shiplap fashion. Reinforce the overlap with a double row of staples spaced 6" apart in a staggered pattern on either side. 6. The discharge end of the motting liner should be similarly

secured with 2 double rows of stoples. Note: If flow will enter from the edge of the matting then the area effected by the flow must be keyed-in. MARYLAND DEPARTMENT OF ENVIRONMEN
WATER MANAGEMENT ADMINISTRATION AS-BUILT LDE Inc. Engineers, Surveyors, Planners



OWNER/

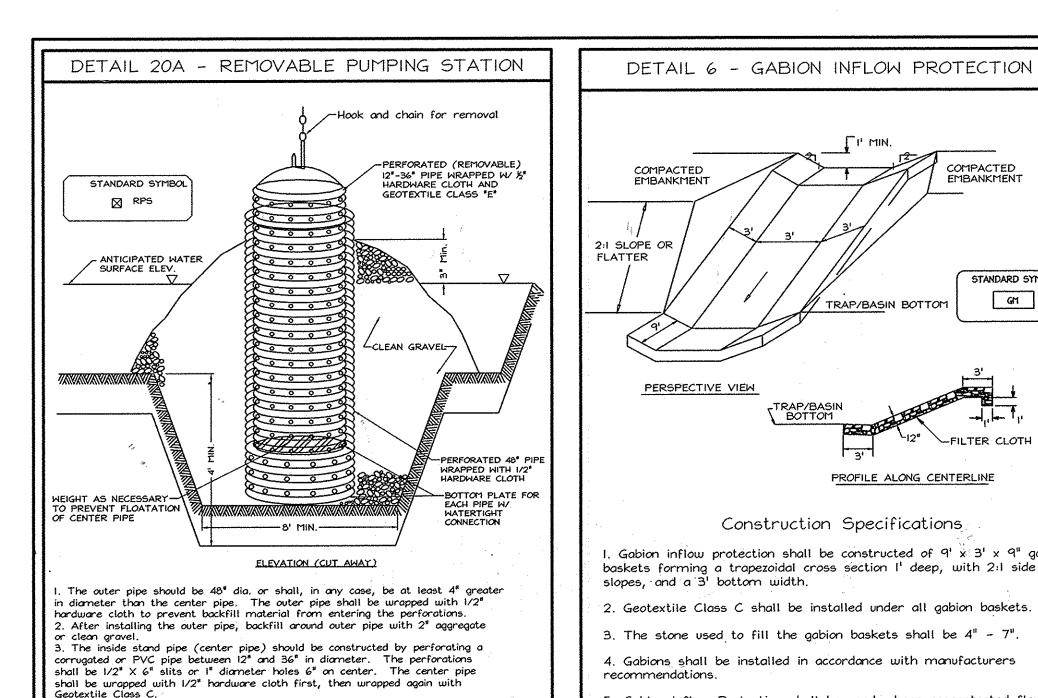
Suite 200

40 997-8800

△DEVELOPER

Columbia, MD 21044

Williamsburg Group, LLC 5485 Harpers Farm Ed.



. Gabion Inflow Protection shall be used where concentrated flow is present on slopes steeper than 4:1. U.S. DEPARTMENT OF AGRICULTURI SOIL CONSERVATION SERVICE MARYLAND DEPARTMENT OF ENVIRONMEN WATER MANAGEMENT ADMINISTRATIO SECTION 30.0 - DUST CONTROL

Placement of topsoil over a prepared subsoil prior to establishment of

4. The center pipe should extend 12" to 18" above the anticipated water

surface elevation or riser crest elevation when dewatering a basin.

To provide a suitable soil medium for vegetative growth. Soils of concern have low moisture content, low nutrient levels, low pH, materials toxic to plants, and/or unacceptable soil gradation. Conditions Where Practice Applies

a. The texture of the exposed subsoil/parent material is not adequate to produce vegetative growth.

b. The soil material is so shallow that the rooting zone is not deep enough to support plants or furnish continuing supplies of moisture and plant nutrients.

c. The original soil to be vegetated contains material toxic to plant

This practice is limited to areas having 2: I or flatter slopes where

d. The soil is so acidic that treatment with limestone is not feasible. . For the purpose of these Standards and Specifications, areas having slopes steeper than 2: I require special consideration and design for adequate stabilization. Areas having slopes steeper than 2: I shall have the appropriate stabilization shown on the plans.

Construction and Material Specifications

Topsoil salvaged from the existing site may be used provided that it meets the standards as set forth in these specifications. Typically, the depth of topsoil to be salvaged for a given soil type can be found in th representative soil profile section in the Soil Survey published by USDA-SCS in cooperation with Maryland Agricultural Experimental Station. Topsoil Specifications - Soil to be used as topsoil must meet the following:

Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay loam, loamy sand. Other soils may be used if recommended by an agronomist or soil scientist and approved by the appropriate approval authority. Regardless, topsoil shall not be a mixture of contrasting textured subsoils and shall contain less than 5% by volume of cinders stones, sloa, coarse fragments, grayel, sticks, roots, trash, or other

Topsoil must be free of plants or plant parts such as bermuda grass, auackgrass, Johnsongrass, nutsedge, poison ivy, thistle, or others as specified.

 Where the subsoil is either highly acidic or composed of heavy clays, ground limestone shall be spread at the rate of 4-8 tons/acre (200-400) pounds per 1,000 square feet) prior to the placement of topsoil. Lime shall be distributed uniformly over designated areas and worked into the soil in conjunction with tillage operations as described in the

following procedures.

111. For sites having disturbed areas under 5 acres: Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization - Section 1 - Vegetative Stabilization Methods and Materials.

On soil meeting Topsoil specifications, obtain test results dictating

fertilizer and lime amendments required to bring the soil into a. pH for topsoil shall be between 6.0 and 7.5. If the tested soil demonstrates a pH of less than 6.0, sufficient lime shall be

b. Organic content of topsoil shall be not less than 1.5 percent by weight. c. Topsoil having soluble salt content greater than 500 parts per million

d. No sod or seed shall be placed on soil which has been treated with soil sterilants or chemicals used for weed control until sufficient time has elapsed (14 days min.) to permit dissipation

Note: Topsoil substitutes or amendments, as recommended by a qualified agronomist or soilscientist and approved by the appropriate approval authority, may be used in lieu of natural topsoil.

Place topsoil (if required) and apply soil amendments as specified in 20.0 <u>Vegetative Stabilization</u> - Section I - Vegetative Stabilization Methods and Materials.

When topsoiling, maintain needed erosion and sediment control practices such as diversions, Grade Stabilization Structures, Earth Dikes, Slope Silt Fence and Sediment Traps and Basins.

Grades on the areas to be topsoiled, which have been previously established, shall be maintained, albeit 4" - 8" higher in elevation.

Topsoil shall be uniformly distributed in a 4" - 8" laver and lightly compact to a minimum thickness of 4° . Spreading shall be perform in such a manner that sadding or seeding can proceed with a minimum of additional soil preparation and tillage. "Any irregularitie in the surface resulting from topsoiling or other operations shall be corrected in order to prevent the formation of depressions or

30.0 DUST CONTROL

Construction Specifications:

Gabion inflow protection shall be constructed of 9' x 3' x 9" gabion

baskets forming a trapezoidal cross section 1' deep, with 2:1 side

3. The stone used to fill the gabion baskets shall be 4" - 7".

4. Gabions shall be installed in accordance with manufacturers

DETAIL 6 - GABION INFLOW PROTECTION

TRAP/BASIN BOTTO

PROFILE ALONG CENTERLINE

COMPACTED

PERSPECTIVE VIEW

slopes, and a 3' bottom width.

recommendations

2:I SLOPE OR

FLATTER

COMPACTED

STANDARD SYMBOL

GM

<u>Definition</u> Controlling dust blowing and movement on construction sites and roads.

To prevent blowing and movement of dust from exposed soil surfaces, reduce on and off-site damage, health hazards, and improve traffic safety. Conditions Where Practice Applies

This practice is applicable to areas subject to dust blowing and movement where on and off-site damage is likely without treatment.

<u>Specifications</u>

Temporary Methods

1. Mulches—See standards for vegetative stabilization with mulches only. Mulch should be crimped or tacked to prevent blowing. 2. Vegetative Cover- See standards for temporary vegetative cover. 3. Tilliage- To roughen surface and bring clods to the surface. This is and emergency measure which should be used before soil blowing starts. Begin plowing on windward side of site. Chisel-type plows spaced about 12" apart, spring-toothed harrows, and similar plows are examples of equipment which

may produce the desired effect. 4. Irrigation- This is generally done as an emergency treatment. Site is sprinkled with water until the surface is moist. Repeat as needed. At no time should the site be irrigated to the point that runoff begins to flow. 5. Barriers - Solid board fences, silt fences, snow fences, burlap fences, straw bales, and similar material can be used to control air currents and soil blowing. Barriers placed at right angles to prevailing currents at intervals of about 10 times their height are effective in controlling soil blowing. 6. Calcium Chloride- Apply at rates that will keep surface moist. May need

ermanent Methods

1. Permanent Vegetation- See standards for permanent vegetative cover, and permanent stabilization with sod. Existing trees or large shrubs mat afford valuable protection if left in place. 2. Topsoiling- Covering with less erosive materials. See standards for 3. Stone - Cover surface with crushed stone or coarse gravel.

References 1. Agriculture Handbook. Wind erosion Forces in the United States and Their Use in Predicting Soil Loss. 2. Agriculture Information Bulletin 354, How to Control Wind Erosian,

WATER MANAGEMENT ADMINISTRATION

iv. Topsoil shall not be placed while the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet or in a condition that may otherwise be detrimental to proper grading and

VI. Alternative for Permanent Seeding - Instead of applying the full amounts of time and commercial fertilizer, composted sludge and amendments may be applied as specified below:

Composted Studge Material for use as a soil conditioner for sites having disturbed areas over 5 acres shall be tested to prescribe amendments and for sites having disturbed areas under 5 acres shall conform to the following requirements:

a. Composted studge shall be supplied by, or originate from, a person or persons that are permitted (at the time of acquisition of the compost) by the Maryland Department of the Environment under COMAR 26.04.06. b. Composted sludge shall contain at least 1 percent nitrogen, 1.5 percent phosphorus, and 0.2 percent potassium and have a PH of 7.0 to 8.0. If compost does not meet these requirements, the appropriate constituents must be added to meet the requirements prior to use.

c. Composted sludge shall be applied a rate of 1 ton / 1,000 square feet. iv. Composted sludge shall be amended with a potassium fertilizer applied at the rate of 4 lb/1,000 sayare feet, and 1/3 the normal lime

References: Guideline Specifications, Soil Preparation and Sodding. MD-VA, Pub. #1, Cooperative Extension Service, University of Maryland and Virginia

DEVELOPMENT ENGINEERING DIVISION

DETAIL 27- ROCK OUTLET PROTECTION III PLAN VIEW EXISTING STABILIZED ELEVATION SECTION A-A FILTER FABRIC LINING SHALL BE EMBEDDED A MINIMUM OF 4" AND SHALL EXTEND AT LEAST 6" BEYON THE EDGE OF THE RIP-RAP NOTE: FILTER CLOTH SHALL BE GEOTEXTILE CLASS C MARYLAND DEPARTMENT OF ENVIRONMEN WATER MANAGEMENT ADMINISTRATION

ROCK OUTLET PROTECTION III

Construction Specifications

1. The subgrade for the filter, rip-rap, or gabion shall be prepared to the required lines and grades. Any fill required in the subgrade shall be compacted to a density of

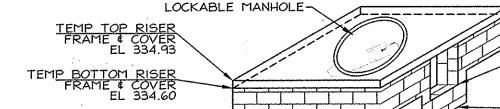
approximately that of the surrounding undisturbed material. 2. The rock or gravet shall conform to the specified grading limits when installed respectively in the rip-rap or filter.

3. Geotextile shall be protected from punching, cutting, or tearing. Any domage other than an occasional small hole shall be repaired by placing another piece of geotextile over the damaged part or by completely replacing the geotextile. All

overlaps whether for repairs or for joining two pieces of

geotextile shall be a minimum of one foot 4. Stone for the rip-rap or gabion outlets may be placed by equipment. They shall be constructed to the full course thickness in one operation and in such a manner as to avoid displacement of underlying materials. The stone for rip-rap or gabion outlets shall be delivered and placed in a manner that will ensure that it is reasonably homogeneous with the smaller stones and spalls filling the voids between the larger stones. Rip-rap shall be placed in a monner to prevent damage to the filter blanket or geotextile. Hand placement will be required to the extent necessary to prevent damage to the

5. The stone shall be placed so that it blends in with the existing ground. If the stone is placed too high then the flow will be forced out of the channel and scour adjacent to the stone will occur.



TYP. DEWATERING

DEVICE

ISOMETRIC VIEW #1

NOT TO SCALE

LOCKABLE MANHOLE-

---5'-4"----

Top slob Elev.= 334.931

Bot. slob Elev.= 334.601

TEMPORARY

ULTIMATE

BLOCK/BRICK-

STAGE 2 WEIRS

WEIR (I' LF) INV. 333.60 ULTIMATE STAGE 2 WEIR INV. 333.00 TEMPORARY BLOCK/BRICK-TO BE REMOVED, SEE SHT. II

14" RISER FRAME \$

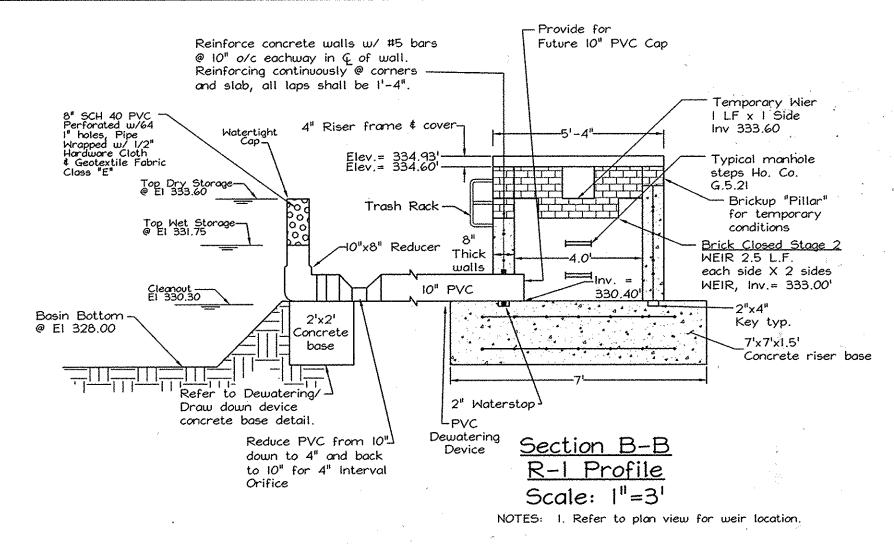
TCOVER (SEE DETAIL)

TYP. 24" ASTM

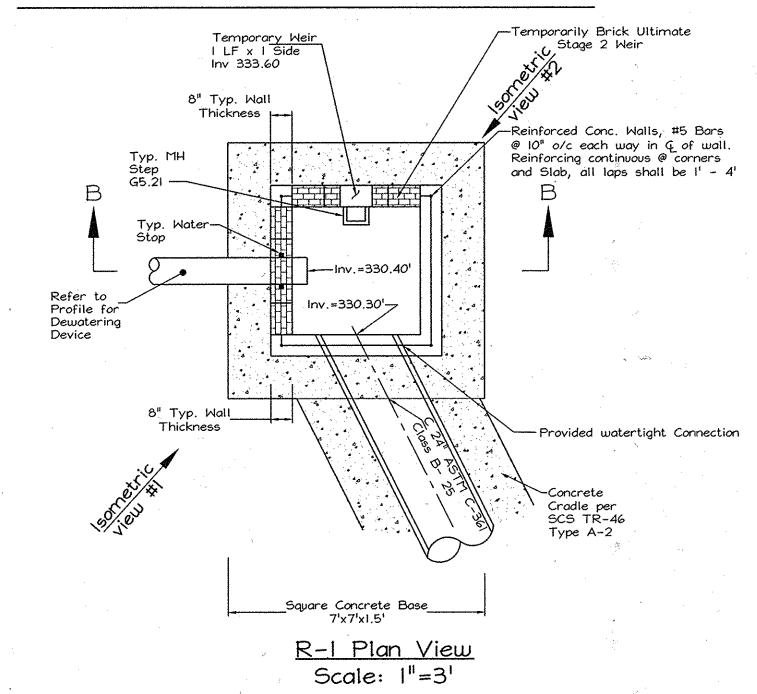
BARREL

TEMPORARY

ISOMETRIC VIEW #2 NOT TO SCALE



SEDIMENT BASIN RISER DETAILS



OWNER/ Williamsburg Group, LLC 5485 Harpers Farm Rd. Suite 200 Columbia, MD 21044 410 997-8800

LDE Inc.



AS-BUILT CERTIFICATIONS THERE IS NO "AS-BUILT" INFORMATION PROVIDED ON THIS SHEET. MICHAEL D. ADCOCK, PROFESSIONAL LAND SURVEYOR LICENSE NO. 21257, EXPIRATION DATE: JUNE 16, 2017

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT

Engineers, Surveyors, Planners 9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045 (410)715-1070 - (301)596-3424 - FAX(410)715-9540 GRADING & SOIL EROSION AND SEDIMENT CONTROL PLAN DETAILS SCALE WATKINS' CHOICE EDS Shown PHASE DRAWING Lots 1, 2, Open Space Lot 3 and LDE 10 OF 16 Non-Buildable Bulk Parcel "A" HECKED Tax Map 46 - Grid 18 - Parcel 215 6 th Election District - Howard County, Maryland 02-035.2 revious Submittals: SP-07-010, WP-09-042 LDE REVISE OWNER/DEVELOPER loper: Williamsburg Group, LLC
5485 Harpers Farm Rd.
Soite 200
Columbia, MD 21044
410 997-2000 FILE NO. Date By Description 12/2008 -08-179 REVISIONS

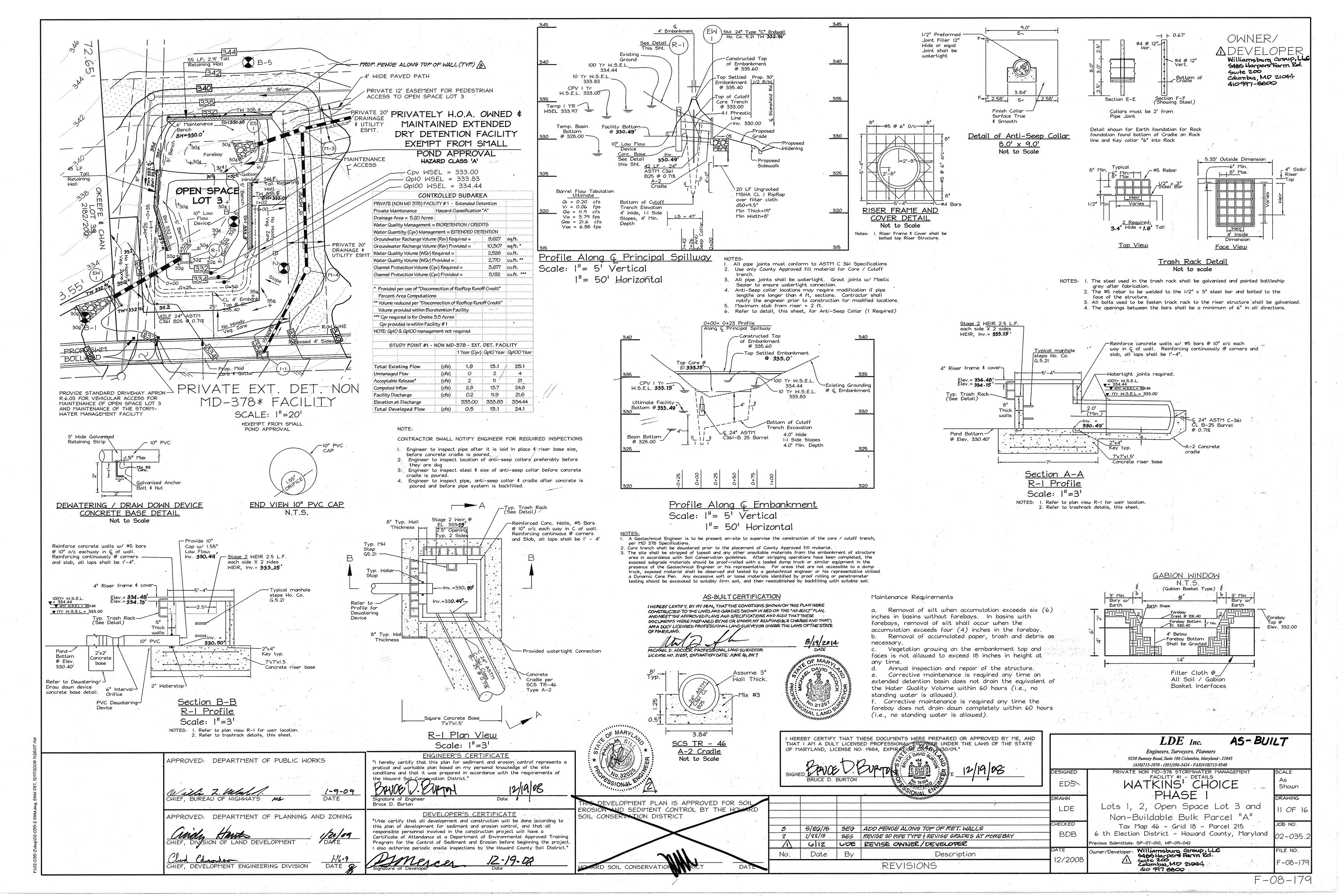
ENGINEER'S CERTIFICATE APPROVED: DEPARTMENT OF PUBLIC WORKS 'I hereby certify that this plan for sediment and erosion control represents a pratical and workable plan based on my personal knowledge of the site conditions and that it was prepared in accordance with the requirements of the Howard Soil Conservation District."

DIFTON Sianature of Engineer Date / Bruce D. Burton DEVELOPER'S CERTIFICATE APPROVED: DEPARTMENT OF PLANNING AND ZONING "I/We certify that all development and construction will be done according to this plan of development for sediment and erosion control, and that all responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of Environmental Approved Training Program for the Control of Sediment and Erosion before beginning the project. also authorize periodic onsite inspections by the Howard County Soil District."

HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO: 19184, EXPIRATION PATA 1/30/09."

AS-BUILT

F-08-179



CONTRACTOR TO THE EXISTING UNDERDRAINS FROM LOTS 2 & 4 MICRO-EXISTING UNDERDRAIN SYSTEM FROM BIORETENTION FACILITIES ONLOT BIORETENTION FACILITY INTO MODIFIED B-1, CIRCUMVENTING THE STONE STORAGE AREA. Non-Buildable Bulk Parcel "A" OUTSIDE PAVING EDGE EL. 343.8 PROP 8" 5 INSIDE PAVING EDGE EL. 343.0 EDGE PARKING OUTSIDE PAVING EDGE EL. 343.8 ____ INSIDE PAVING EDGE EL. 343.0 Catch basin " STONE STORAGE AREA EDGE PARKING EL. 343.0 T -4" OVERDRAIN H.O.A. OWNED & MAINTAINED-PRIVATE STONE STORAGE FACILITY MAX. POOL ELEV. 342.50 SCALE 1"=20" FILTER FABRIC ON THE SIDES AND TOP OF I TOP ELEV.; 342.50 FACILITY ONLY FROM 1-2 TO. MODIFIED B-1 INV. 340.69 #4. #5 OR #6 STONE CONNECT INLET TO (OR EQUAL) **EXISTING UNDER-**REMOVE PLANTING SOIL AND REPLACE WITH STONE DISTRIBUTION PIPE SYSTEM EX. RECHARGE CHAMBER ----

AS-BUILT CERTIFICATION

I HEREBY CERTIFY, BY MY SEAL, THAT THE CONDITIONS SHOWN ON THIS PLAN WERE

CONSTRUCTED TO THE LINES AND GRADES SHOWN IN RED ON THIS "AS-BUILT" PLAN,

DOCUMENTS WERE PREPARED BY ME OR UNDER MY RESPONSIBLE CHARGE AND THAT

AMA DULY LICENSED PROFESSIONAL LAND SURVEYOR UNDER THE LAWS OF THE STATE

AND MEET THE APPROVED PLANS AND SPECIFICATIONS AND ALSO THAT THESE

HAEL D. ADCOCK. PROFESSIONALLAND SURVEYOR LICENSE NO, 21257, EXPIRATION DATE: JUNE 16, 2017

OFMARYLAND.

STABILIZATION:

All borrow areas shall be graded to provide proper drainage and left in a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resource Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

EROSION AND SEDIMENT CONTROL:

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures. These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent version.

SITE PREPARATION

Areas designated for borrow areas, embankment and structural works shall be cleared grubbed, and stripped of topsoil. All trees, vegetation, roots, and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than I.I. All trees shall be cleared and grubbed within 15 feet of the toe of the embankment

Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish, and other objectionable material unless otherwise designated on the plans. Trees, brush and stumps shall be cut approximately level to the ground surface. For dry stormwater management ponds, a minimum of a 25 foot radius around the inlet structure shall be cleared.

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

Material - The fill material shall be taken from approved designated borrow areas. It shall be free of roots, stumps wood, rubbish, stones greater than 6", frozen or other objectionable materials. Fill material for the center of the embankment and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer.

Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment.

<u>Placement</u> - Areas on which fill is to be placed shall be scarified prior to the placement of fill. Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment.

Compaction - The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired, or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble yet not be wet that water can be squeezed out.

The minimum required density shall not be less than 95% of maximum dry density with a moisture content within +/- 2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

<u>Cut off Trench</u> - The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be I to I of flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability.

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be I to I or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In additional, the core shall be placed concurrently with the outer shell of the embankment.

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is compacted fill of 24" or greater over the structure or pipe. Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2,000 ohm-cm. Material shall be placed such that a minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sides of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is compacted fill of 24" or greater over the structure pipe. Backfill material outside the structural backfill (flowable fill) zone shall be of the type and quality conforming to that specified for the core of the embankment or other embankment materials.

PIPE CONDUITS

All pipes shall be circular in cross section.

Corrugated Metal Pipe - All of the following criteria shall apply for corrugated metal pipe:

1. Materials - (Polymer Coated steel pipe) - Steel pipes with polymeric coatings shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 & M-246 with watertight coupling bonds or

Materials - (Aluminum Coated Steel Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.

Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-196 or M2II with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specifications M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer. Hot dip galvonized bolts may be used for connections. The pH of the surrounding soils shall be between 4 and 9.

2. Coupling bands, anti seep-collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at least 24 mils in thickness.

3. Connections - All connections with pipes must be completely watertight. The drain or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be

All connections shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the band width. The following type connections are acceptable for pipes less than 24" in diameter: flanges on both ends of the pipe, with a circular 3/8" closed cell neoprene gasket, pre-punched to the flange bolt circle, sandwiched between adjacent floriges; a 12 inch wide standard lap type band with 12" wide by 3/8" thick closed cell circular neoprene gasket; and a 12 inch wide hugger type band with O-ring gaskets having a minimum diameter of 1/2 inch greater than the corrugated depth. Pipes 24" in diameter and larger shall be connected by a 24" long annular corrugated band using a minimum of 4 (four) rods and lugs, 2 on each connecting pipe end. A 24" wide by 3/8" thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8" closed cell gaskets the full width of the flange is also acceptable.

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or a neoprene bead.

4. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spanay or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

- 5. Backfilling shall conform to "Structure Backfill."
- 6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.
- Reinforced Concrete Pipe All of the following criteria shall apply for reinforced concrete pipe:
- 1. Materials Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM Designation C-361.

2. Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50 % of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in "Structure Backfill" section of this standard. Gravel bedding is not permitted.

3. Laying pipe - Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire length, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

- 4. Backfilling shall conform to "Structure Backfill."
- 5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

<u>Plastic Pipe</u> - The following criteria shall apply for plastic pipe.

- I. Materials PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" - 10" pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" shall meet the requirements of AASHTO M294 Type S.
- 2. Joints and connections to anti-seep collars shall be completely watertight.
- 3. Bedding The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, sponay or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.
- 4. Backfilling shall conform to "Structure Backfill."
- 5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

DRAINAGE DIAPHRAGMS - When a drainage diaphragm is used, a registered professional engineer will supervise the design and construction inspection. CONCRETE:

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3.

ROCK RIPRAP:

Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 311.

Geotexile shall be placed under all riprap and shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section

CARE OF WATER DURING CONSTRUCTION

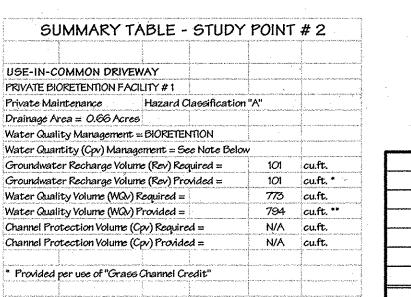
All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect the areas to be occupied by the permanent works. The contractor shall also furnish, install, operate and maintain all necessary pumping and other equipment required for removal of water from the various parts of the work and for maintaining the excavations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extend required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which may required draining the water to sumps from which the water shall be

OPERATION AND MAINTENANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED STONE STORAGE FACILITY

- TO BEINSPECTED AND CLEANED ANNUALLY INCLUDING THE INLET AND OVERDRAIN TO AVOID THE POTENTIAL FOR CLOGGING.
- PONDING, STANDING WATER OR ALGAEGROWTH ON THE TOP MAY INDICATE FAILURE DUETO SEDIMENTATION INTHE GRAVEL MEDIA
- IFWATER PONDS FOR MORE THAN 48 HOURS AFTER A MAJORS TORM OF MORE THAN 6" OF SEDIMENT HAS ACCUMULATED, THE GRAVEL MEDIASHOULD BE EXCAVATED AND REPLACED
- THE WATKING CHOICE HOMEOWNERS ASSOCIATION IS RESPONSIBLE FOR MAINTENANCE OF THE STORMWATER MANAGEMENT FACILITY AS RECORDED UNDER PLAT NO 22153.

OWNER/ △ DEVELOPER Williamsburg Group, LC 5485 Harpers Farm Rd. Suite 200 Columbia, MD 21044 410 997-8800





* Volume provided within Bioretention Facility

AS -BUILT LDE Inc. HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND Engineers, Surveyors, Planners THAT I AM A DULY LICENSED PROFESSIONAL PROFESSIONAL ENGINEER UNDER-THE LAWS OF THE STATE OF MARYLAND, LICENSE NO: 19184, EXPIRATION DATE (1)30/09." 9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045 (410)715-1070 - (301)596-3424 - FAX(410)715-9540 PRIVATE BIORETENTION FACILITY - NOTES & DETAILS AND MD-378 SPECIFICATIONS WATKINS' CHOICE As Showr RAWING Lots 1, 2, Open Space Lot 3 and 12 OF 10 Non-Buildable Bulk Parcel "A" HECKED JOB NO. Tax Map 46 - Grid 18 - Parcel 215 6 th Election District - Howard County, Maryland SAA REPLACE BIORETENTION FACILITY WITH STONE STORAGE-REVISE INLET BA & ADD A-2. 2-035. 4/12 LDE REVISE OWNER / DEVELOPER Previous Submittals: SP-07-010, WP-09-042 Williamsburg Group, LLC ILE NO. Owner/Developer: Date Description 5489 Harpers Farm Rd. Suite 200 Columbia, IAD 21044 410 999-8000 12/2008 -08-179 REVISIONS

acility No.

Circle

Bruce D. Burton, P.E. No. 19184

STONE STORAGE FACILITY DIMENSION TABLE

Stone

Stone.

Top

APPROVED: DEPARTMENT OF PLANNING AND ZONING

342.50

Overdrain Pavement

APPROVED: DEPARTMENT OF PUBLIC WORKS

341.85

HIEF, DEVELOPMENT ENGINEERING DIVISION

Location

Driveway

