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PROJECT:	KINGS WOODS, S	EC.5 FILE	STORM DRAIN	DATE:	10/03/07	JMC
DARCEL	INLET#	ZONING	SUBAREA	AREA (Ac)	"C" FACTOR	% IMPERVIOUS
PARCEL	IINLE I #	(Z)	(B)	(A)	(C)	(P) CALCULATE
	I-1	R-SC		0.62	0.59	79
	1-2	R-SC		0.03	0.52	80
	1-3	R-SC		0.28	0.40	72
	I-4	R-SC		0.45	0.35	69
	1-5	R-SC		0.34	0.47	77
	I-6	R-SC		0.67	0.27	60
	I-7	R-SC		0.26	0.24	50
	I-8	R-SC		0.46	0.28	65
	I-9	R-SC		0.51	0.31	63
	I-10	R-SC		0.33	0.53	80
	I <b>-</b> 11	R-SC		0.62	0.41	72
	I-12 & SW1	R-SC		0.43	0.29	65
	I-13	R-SC		0.16	0.57	58
	I-14	R-SC		0.11	0.70	69
	I-15	R-SC		0.22	0.48	72
	SW2	R-SC		0.16	0.23	34
ľ	SW3	R-SC		0.24	0.28	56
	Ex I-20	R-SC		0.10	0.86	100
	Ex-l-21	R-SC		1.24	0.58	16

													<u>LEGEN</u>
RUNOFF COMPUTATIONS													
			·					<u> </u>					SOILS CLASSIFICATION
D.A. #	ABEA (As )	ZONING	0/ 1140000 //01 10	"C"	"C"	tc (min)	l (in/hr)	l (in/hr)	l <sub>100</sub> (in/hr)	O (ofe)	(ofc)	Q <sub>100</sub> (cfs)	SOILS DELINEATION
D.A. #	AREA (Ac.)	ZOMING	% IMPERVIOUS			te (mm)	l <sub>2</sub> (in/hr)	l <sub>10</sub> (in/hr)	100 (117117)	Q <sub>2</sub> (cfs)	Q <sub>10</sub> (cfs)	4100 (0.0)	EVERTING CONTOURS
14	0.60	000	70	(< 25 Yr)	(> 25 Yr)	40.0	4.50	6,60	10.00	1.65	2.41	3.66	EXISTING CONTOURS
-1  -2	0.62 0.03	R-SC R-SC	79 80	0.59 0.52	0.68 0.62	10.0 10.0	4.50	6.60	10.00	0.07	0.10	0.16	LIMIT OF WETLANDS
							<del></del>						A second second
I-3	0.28	R-SC	72	0.40	0.49	10.0	4.50	6.60	10.00	0.50	0.74	1.12	EXISTING WOODS LINE
I-4	0.45	R-SC	69	0.35	0.44	10.0	4.50	6.60	10.00	0.71	1.04	1.58	PROPOSED WOODS LINE
I-5	0.34	R-SC	77	0.47	0.57	10.0	4.50	6.60	10.00	0.72	1.05	1.60	PROPOSED WOODS LINE
1-6	0.67	R-SC	60	0.27	0.35	10.0	4.50	6.60	10.00	0.81	1.19	1.81	EXISTING STRUCTURE
t-7	0.26	R-SC	50	0.24	0.32	10.0	4.50	6.60	10.00	0.28	0.41	0.62	PROPOSED STRUCTURE
1-8 -	0.46	R-SC	65	0.28	0.37	10.0	4.50	6.60	10.00	0.58	0.85	1.29	PROPOSED SINOCIONE
I <b>-</b> 9	0.51	R-SC	63	0.31	0.40	10.0	4.50	6.60	10.00	0.71	1.04	1.58	SPECIMEN TREE
I-10	0.33	R-SC	80	0.53	0.62	10.0	4.50	6.60	10.00	0.79	1.15	1.75	FOREST CONSERVATION EASEMENT
I-11	0.62	R-SC	72	0.41	0.50	10.0	4.50	6.60	10.00	1.14	1.68	2.54	·*
I-12 & SWALE 1	<b>" 0.43</b>	R-SC	65	0.29	0.38	10.0	4.50	6.60	10.00	0.56	0.82	1.25	EX. LANDSCAPE TREE F-96-139
1-13	0.16	R-SC	58	0.57	0.65	10.0	4.50	6.60	10.00	0.41	0.60	0.91	
l-14	0.11	R-SC	69	0.70	0.75	10.0	4.50	6.60	10.00	0.35	0.51	0.77	
I-15	0.22	R-SC	72	0.48	0.57	10.0	4.50	6.60	10.00	0.48	0.70	1.06	
SW2	0.16	R-SC	34	0.23	0.30	10.0	4.50	6.60	10.00	0.17	0.24	0.37	
SW3	0.24	R-SC	56	0.28	0.36	10.0	4.50	6.60	10.00	0.30	0.44	0.67	
Ex I-20	0.10	R-SC	100	0.86	0.96	10.0	4.50	6.60	10.00	0.39	0.57	0.86	
Ex I-21	1.24	R-SC	16	0.58	0.70	10.0	4.50	6.60	10.00	3.24	4.75	7.19	

		SOILS LEGEND
MAP SYMBOL	SOIL TYPE	MAPPING UNIT
BeB2 *	С	BELTSVILLE SILT LOAM, 1 TO 5 PRCENT SLOPES, MODERATELY ERODED
BeC2 *	С	BELTSVILLE SILT LOAM, 5 TO 10 PERCENT SLOPES, MODERATELY ERODED
BeC3 *	С	BELTSVILLE SILT LOAM, 5 TO 10 PERCENT SLOPES, SEVERELY ERODED
BeD2 *	С	BELTSVILLE SILT LOAM, 10 TO 15 PERCENT SLOPES, MODERATELY ERODED
GP *	Α	GRAVEL PITS AND QUARRIES
L1 *	D	LEONARDTOWN SILT LOAM
SsE	В	SASSAFRAS SOILS, 15 TO 40 PERCENT SLOPES
SfC2	В	SASSAFRAS GRAVELY SANDY LOAM, 5 TO 10 PERCENT SLOPES, MODERATELY ERODED
ScD	С	SANDY AND CLAYEY LAND, MODERATELY SLOPING
	HYDRIC SOIL	S, ISSUED JULY 1968, MAP NO. 33



1	10-22-08	REJISIONS TO STORM DRAIN FROM INSTO EXISTIN	ig system as R	EQUIRED BY HOWARD COUNTY B
10.	DATE		VISION	of Highways
		BENCHMARK		
	ENGINE	ERS A LAND SURVEYORS A PLANNERS		

# ENGINEERING, INC.

8480 BALTIMORE NATIONAL PIKE A SUITE 418 ELLICOTT CITY, MARYLAND 21043 phone: 410-465-6105 ▲ fax: 410-465-6644 email: bei@bei-civilengineering.com

Wonald Mason 5-6-08 Professional Certification: I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 21443 Expiration Date: 12-21-2008. 2012.

**LEGEND** 

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PROJECT: OWNER: JAMES H. KING 9648 WHISKEY BOTTOM ROAD LAUREL, MD 20723 301-498-8922

DEVELOPER:

Design: JMC | Draft: AM

PROJECT: KINGS WOODS
SECTION 5
LOTS 362 - 386 AND OPEN SPACE LOTS 387 & 388

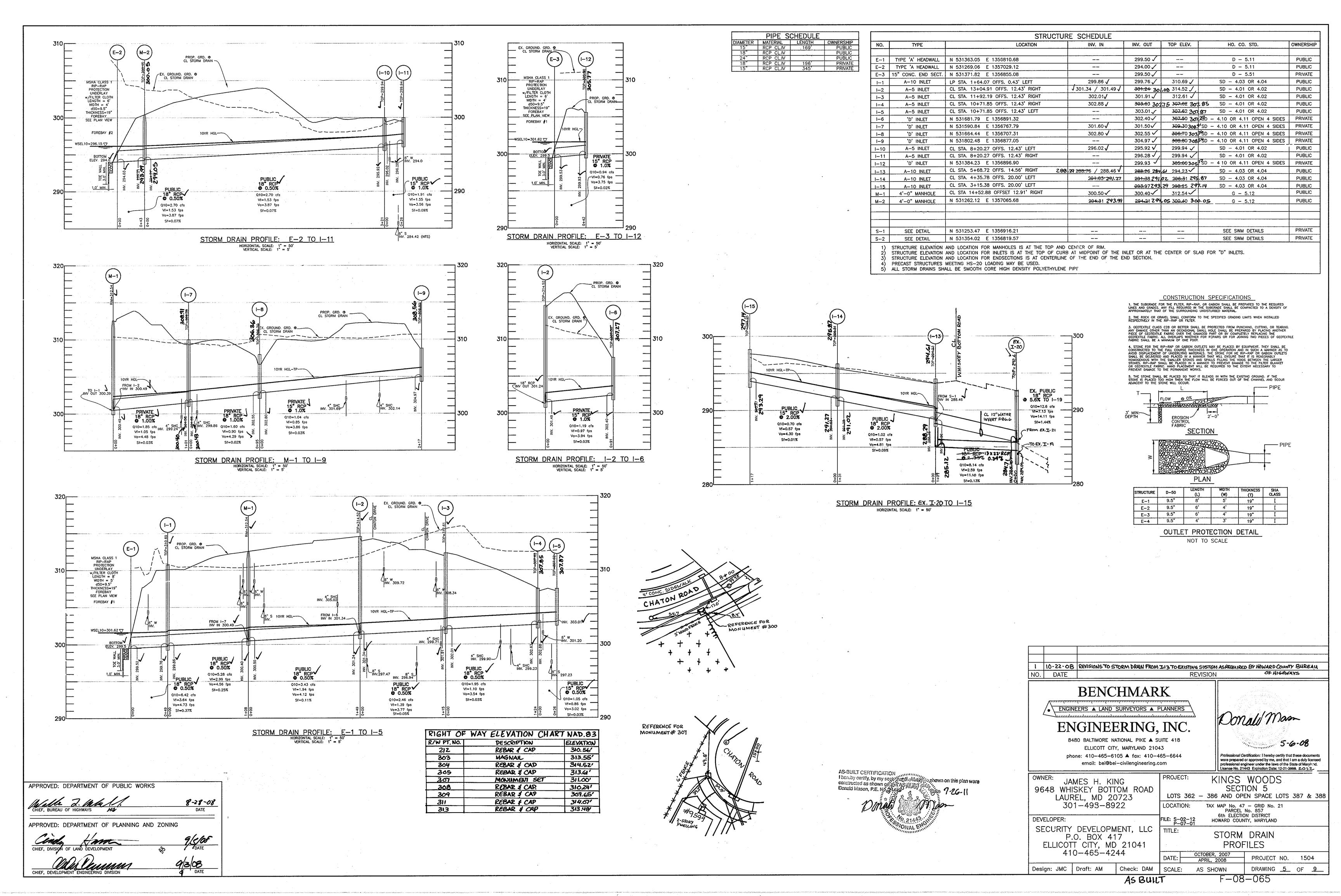
TAX MAP No. 47 - GRID No. 21
PARCEL No. 857
6th ELECTION DISTRICT
HOWARD COUNTY, MARYLAND STORM DRAIN

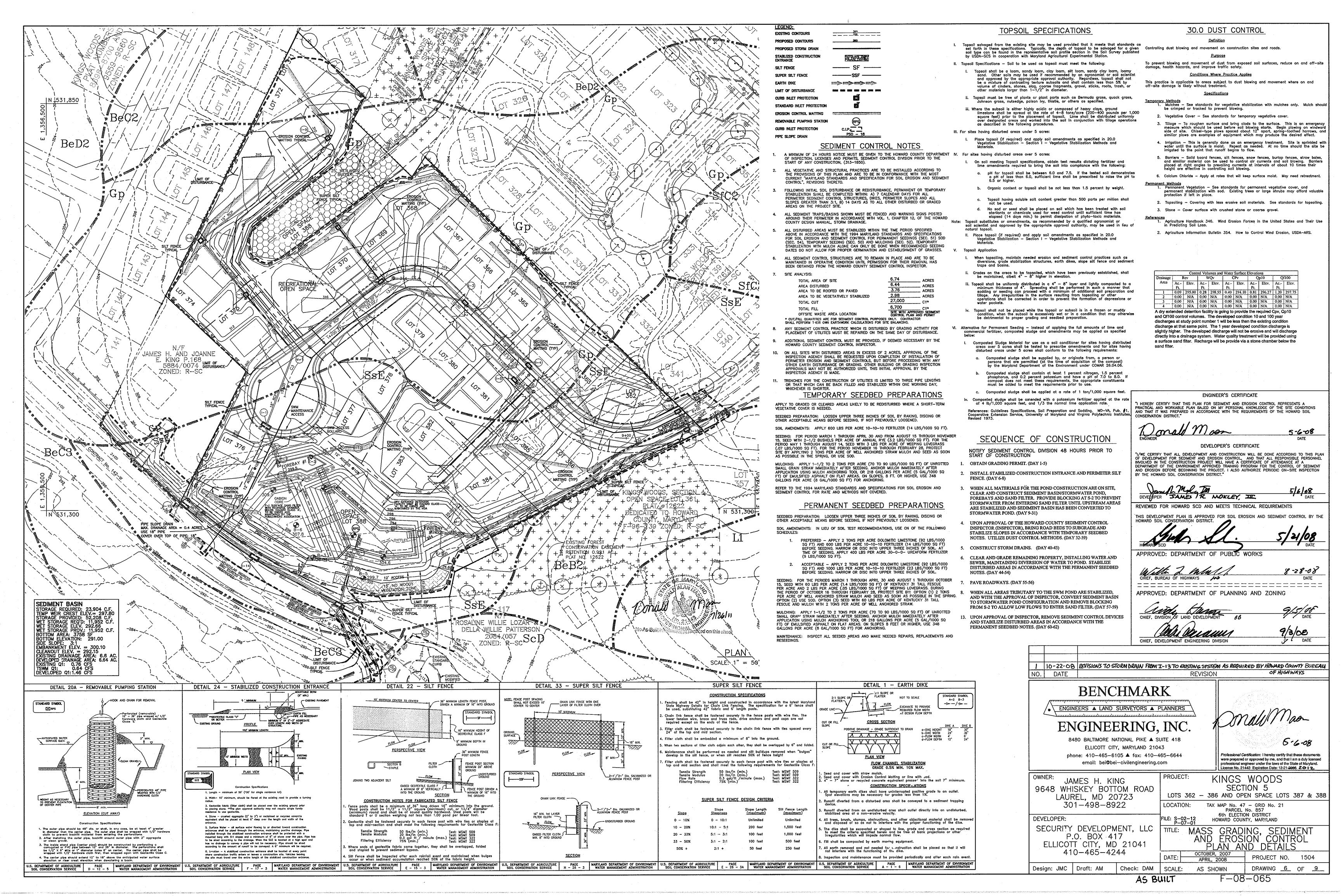
SECURITY DEVELOPMENT, LLC
P.O. BOX 417
ELLICOTT CITY, MD 21041
410-465-4244 DRAINAGE AREA MAP PROJECT NO. 1504

> Check: DAM DRAWING 4 OF 9 AS SHOWN

AS BUILT

F-08-065





POND CONSTRUCTION SPECIFICATIONS These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent version.

Areas designated for borrow areas, embankment, and structural works shall be cleared. grubbed and stripped to topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1.

Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry starmwater management ponds, a minimum of a 25-foot radius around the inlet structure shall be

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

<u>Material</u> — The fill material shall be taken from approved designated borrow areas. If shall be free of roots, stumps, wood, rubbish, stones greater than 6", frozen or other objectionable material. Fill material for the center of the embankment, and cut off trench passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer.

Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment.

Placement — Areas on which fill is to be placed shall be scarified prior to placement of fill.

Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall 6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings. be placed in the downstream portions of the embankment. The principal spillway must be

Compaction — the movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with teh contains track that the required degree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not be so wet that water can be squeezed out. When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within ± 2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-93 (Standard Proctor).

parallel to the centerline of the embankment as shown on the plans. The bottom width of parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be a least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density 5. Other details (anti-seep collars, valves, etc.) shall be shown on the drawings.

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the cores shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability in addition, the core shall be placed concurrently with the outer shell of the embankment.

Backfill adjacent to pipes of structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department o Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100—200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistively of 2,000 ohm—cm. Material shall be placed such that a minimum of 6 (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding over and, on the sided of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequa measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24 or greater over the structure or pipe. Backfill Care of Water during Construction conforming to that specified for the core of the embankment or other embankment

### Pipe Conduits All pipes shall be circular in cross section

Corrugated Metal Pipe - all of the following criteria shall apply for corrugated metal pipe a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 (

M-246 with watertight coupling bands or flanges.

INFLOW A:

A=5411 SF

We = 59'

Le = 130'

Le/We = 2.2

BAFFLE -- FLOW

COMPUTATIONS

LENGTH

D= 91

INFLOW B:

A=5411 SF

We = 67'

Le = 139'

Le/We = 2.1

D= 81'

Materials — (Aluminum Coated Steel Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M—274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M—190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound.

Aluminum surfaces that are to be in contact with concrete shall be painted with one coat

Materials — (Aluminum Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M—196 or M—211 with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M—190 Type A. Aluminum surfaces that are to be contact with concrete shall be a surface of the contact with concrete shall be a surface of the contact with concrete shall be a surface of the contact with sould with the contact of the contact with sould be surfaced with surfaces. be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be between 4 and 9.

2. Coupling bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at least 24 mils in thickness.

3. Connections - All connections with pipes must be completely watertight. The drain pipe of barrel connection to the riser shall be welded all around when the pipe and riser are metal Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.

All connection shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches in diameter: flanges on both ends of the pipe with a circular 3/8 inch closed cell neoprene gasket, prepunched to the flange bolt circle, sandwiched between adjacent flanges; a 12-inch wide standard lap type band with 12-inch wide by 3/8-inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2 inch. greater than the corrugation depth. Pipes 24 inches in minimum grameter or 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by a 24 inch long annular corrugated band using a minimum of 4 (four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8 inch closed cell gaskets the full width of the

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal coulking or a neoprene bead.

4. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete

 Moterials — Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C-361. 2. Bedding — Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used a described in the "Structure Backfill" section of this standard. Gravel

3. Laying pipe — Bell and spigot pipe shall be places with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation form the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

 Materials — PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4' — 10" inch pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" inch shall meet the requirements of AASHTO M294 Type S. 2. Joints and connections to anti-seep collars shall be completely watertight

3. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

4. Backfilling shall conform to "Structure Backfill" 5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings. <u>Drainage Diaphraams</u> — When a drainage diaphragm is used, a registered professional

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414,

Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 311. Geotextile shall be placed under all riprap and shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

All work on permanent structures shall be carried out in areas free from water. The contractor shall construct and maintain all temporary dikes, leves, cofferdams, drainage channels, and stream diversions necessary to protect the areas to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the evacuations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work.

After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the low of water to the spillway or outlet works and so as not to interfere in any way with the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the location being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall be pumped.

All borrow areas shall be graded to provide proper drainage and left in a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures.

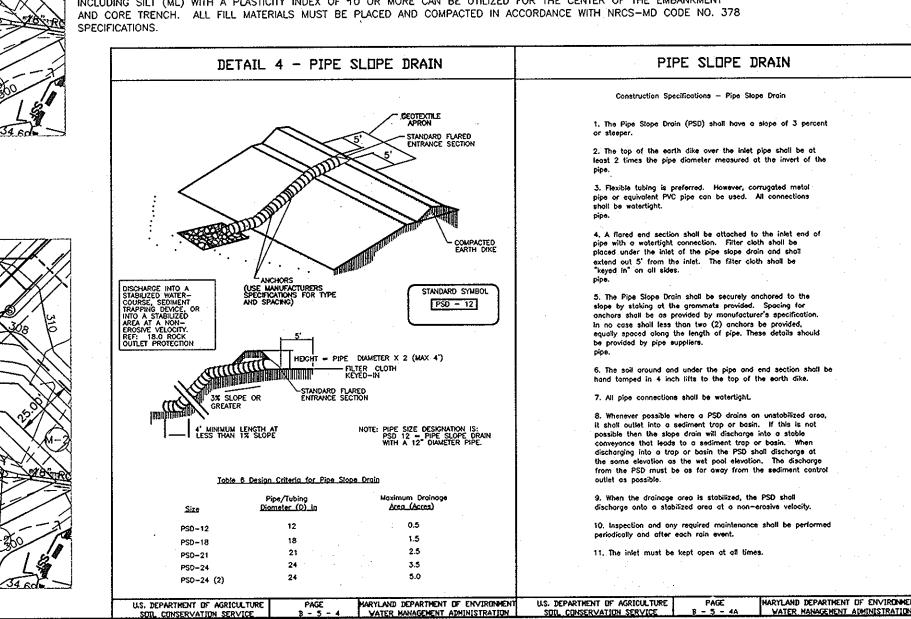
RF-ESTABLISHED BY BACKFILLING WITH SUITABLE SOIL.

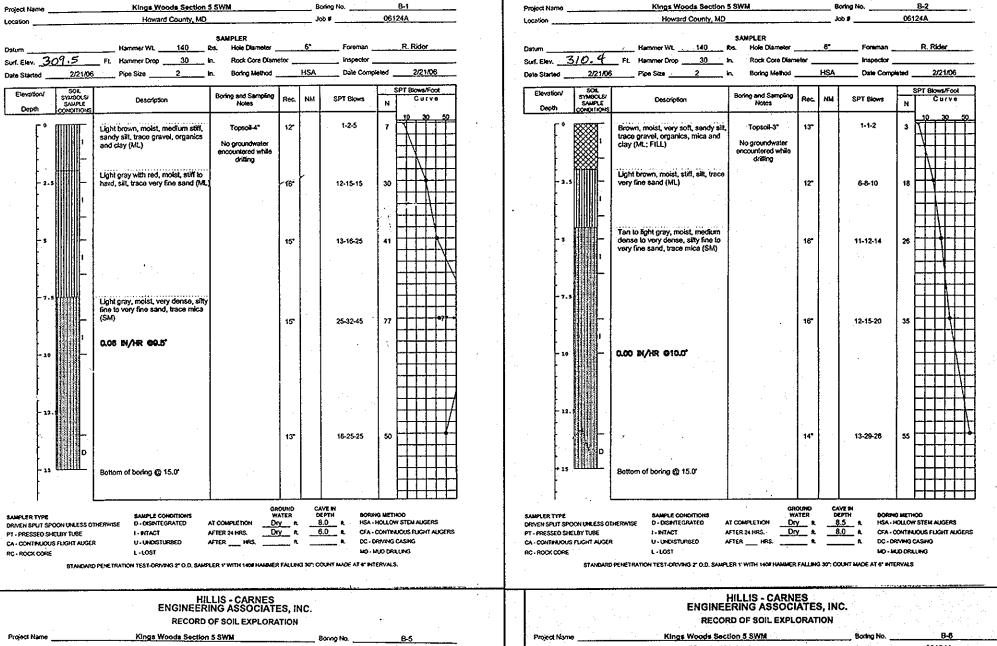
## GEOTECHNICAL ENGINEER RECOMMENDATIONS:

EMBANKMENT AND CUT-OFF TRENCH CONSTRUCTION AREAS OF THE PROPOSED SWM FACILITIES SHOULD BE STRIPPED OF TOPSOIL AND ANY OTHER UNSUITABLE MATERIALS FROM THE EMBANKMENT OR STRUCTURE AREAS IN ACCORDANCE WITH SOIL CONSERVATION GUIDELINES. AFTER STRIPPING OPERATIONS HAVE BEEN COMPLETED, THE EXPOSED SUBGRADE MATERIALS SHOULD BE PROOFROLLED WITH A LOADED DUMP TRUCK OR SIMILAR EQUIPMENT IN THE PRESENCE OF A GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE. FOR AREAS THAT ARE NOT ACCESSIBLE TO A DUMP TRUCK, THE EXPOSED MATERIALS SHOULD BE OBSERVED AND TESTED BY A GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE UTILIZING A DYNAMIC CONE PENETROMETER. ANY EXCESSIVELY SOFT OR LOOSE MATERIALS IDENTIFIED BY PROOFROLLING OR PENETROMETER TESTING SHOULD BE EXCAVATED TO SUITABLE FIRM SOIL, AND THEN GRADES

REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER SHOULD BE PRESENT TO MONITOR PLACEMENT AND COMPACTION OF FILL FOR THE EMBANKMENT AND CUT-OFF TRENCH. IN ACCORDANCE WITH NRCS-MD CODE NO. 378 POND STANDARDS/SPECIFICATIONS, SOILS CONSIDERED SUITABLE FOR THE CENTER OF EMBANKMENT AND CUTOFF TRENCH SHALL CONFORM TO UNIFIED SOIL CLASSIFICATION GC, SC, CH, OR CL AND MUST HAVE AT LEAST 30% PASSING THE #200 SIEVE.

IT IS OUR PROFESSIONAL OPINION THAT IN ADDITION TO THE SOIL MATERIALS DESCRIBED ABOVE, A FINE-GRAINED SOIL, INCLUDING SILT (ML) WITH A PLASTICITY INDEX OF 10 OR MORE CAN BE UTILIZED FOR THE CENTER OF THE EMBANKMENT





Location Howard County, MD Job # 06124A

Date Started 2/21/06 Pipe Size 2 in. Boring Method HSA Date Completed 2/21/06

STANDARD PENETRATION TEST-DRIVING 2" O.D. SAMPLER 1" WITH 1408 HANGER FALLING 30"; COUNT MADE AT 6" INTERVALS.

Hammer Wt. 140 &s. Hole Diameter 6° Foreman R. Rider

2-2-3

4-7-13

CFA - CONTINUOUS FLIGHT AUGERS

MO-MUD DRILLING

 Datum
 Hammer Wt.
 1su
 soa.
 Inspector
 Inspector
 Inspector

 Surf. Elov.
 Z99.7
 FL
 Hammer Drop
 30
 in.
 Rodo Core Diameter
 Inspector
 Inspector

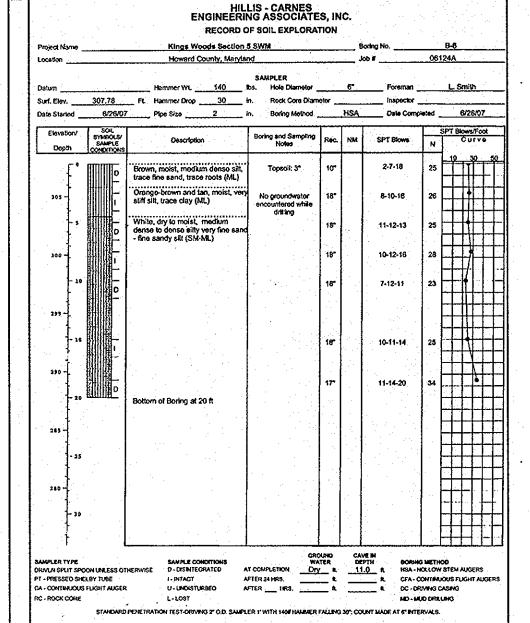
Brown, moist, soft, sandy silt,

Ught brown, moist, loose, slit, sand to sandy slit, trace grave organics and clay (SM-ML)

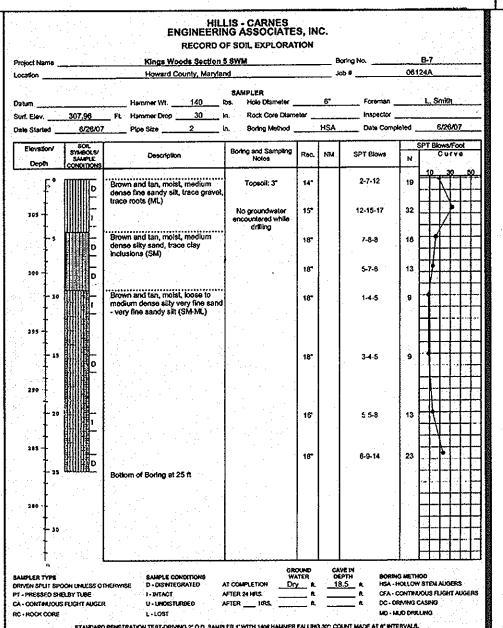
Light brown, moist, stiff, silt, tra-

Tan, moist, stiff, fine sandy silt, trace mica (ML)

RC - ROCK CORE



RECORD OF SOIL EXPLORATION



MO - MUO DRILLING

RECORD OF SOIL EXPLORATION

Date Started 2/21/06 Pipe Size 2 In. Boring Method HSA Date Completed 2/21/06

Toosoil-3"

Boring No.

Hammer Wt. 140 lbs. Hole Diemeter 6" Foreman R. Rider

Boring and Sampling | Rec. | NM | SPT Blows

Job# 06124A

Kings Woods Section 5 SWM

Light brown, moist, medium dense, sity sand to sandy si

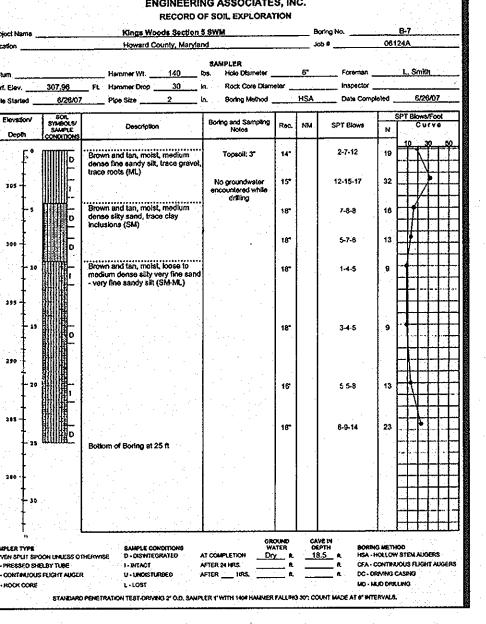
Light gray, moist, stiff, slit, tra-mica and very fine and diff.

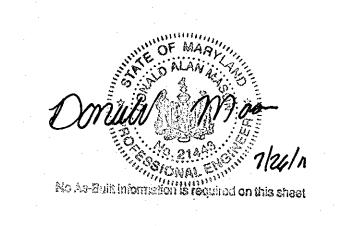
Bottom of boring @ 10.0\*

CA - CONTINUOUS FLIGHT AUGER

0.00 PI/NR @ 4.0"

Surl. Elev. 301.4 Ft. Hammer Drop 30 in. Rock Core Diameter \_\_\_\_\_\_ Inspector \_\_\_\_\_





DEVELOPER'S CERTIFICATE "I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN OF DEVELOPMENT FOR SEDIMENT AND EROSION CONTROL, AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT. me D. Mala M 5/6/08 DEVELOPER JAMES R. MOXLEY III REVIEWED FOR HOWARD SCD AND MEETS TECHNICAL REQUIREMENTS THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE APPROVED: DEPARTMENT OF PUBLIC WORKS 8-58-08

ENGINEER'S CERTIFICATE

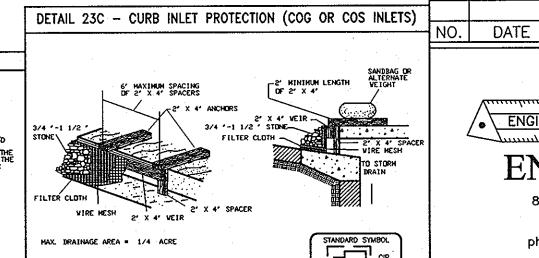
5-6-*0*8

I HEREBY CERTIFY THAT THIS PLAN FOR SEDIMENT AND EROSION CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL

APPROVED: DEPARTMENT OF PLANNING AND ZONING

LAND DEVELOPMENT

REVISION



). Attach a continuous piece of wire nesh (30° minimum width by throat length plus 4°) to the 2°  $\times$  4° weir (neasuring throat length plus 2°) as shown on the standard

ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE A SUITE 418 ELLICOTT CITY, MARYLAND 21043 phone: 410-465-6105 ▲ fax: 410-465-6644

**BENCHMARK** 

ENGINEERS & LAND SURVEYORS A PLANNERS

email: bei@bei-civilengineering.com

RECORD OF SOIL EXPLORATION

ocation Howard County, MD Job # 08124A

 Datum
 Kammer Wt.
 140
 IDB
 Date Cornolated

 Surf. Elev.
 304.6
 Ft.
 Hammer Drop
 30
 In.
 Rock Core Diameter
 Inspector

Date Started 2/21/06 Pipe Size 2 In. Boring Method HSA Date Completed 2/21/06

Topso#-3"

Boring No.

11-12-13

MO - MUD DRILLING

Hammer WL 140 Ibs. Hole Diameter 6° Foreman R. Rider

Kings Woods Section 5 SWM

ace organics, gravel and rock agments (SM)

sand, trace six and iron st

0.125 N/AR @ 7.0°

Bottom of boring @ 12.0\*

SAMPLE CONDITIONS D-DISPITEGRATED 1-BITACT

5-608 were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 21443 Expiration Date: 12-21-2008. **2012.** 

OWNER: JAMES H. KING 9648 WHISKEY BOTTOM ROAD LAUREL, MD 20723 301-498-8922

DEVELOPER: SECURITY DEVELOPMENT, LLC P.O. BOX 417

**PROJECT** KINGS WOODS SECTION 5 LOTS 362 - 386 AND OPEN SPACE LOTS 387 & 388 LOCATION: TAX MAP No. 47 - GRID No. 21 PARCEL No. 857 6th ELECTION DISTRICT HOWARD COUNTY, MARYLAND

PROJECT NO. DRAWING <u>7</u> OF <u>9</u> AS SHOWN

AS BUILT

SCALE:

D - DISTANCE BETWEEN INFLOW AND OUTFLOW A = AREA OF NORMAL POO Wo = EFFECTIVE WIDTH - A/D ~4" OVERLAP OF MATTING STRIPS WHERE TWO OR MORE STRIP WIDTHS ARE REQUIRED, ATTACH STAPLES ON 18 INCH CENTERS. FORMULA: 😓 🗠 2 STAPLE OUTSIDE EDGE OF MATTING ON 2" CENTERS-- RISER (OUTLET) CONSTRUCTION SPECIFICATIONS KEY-IN THE MATTING BY PLACING THE TOP ENDS OF THE MATTING IN A NARROW TRENCH. 6° IN DEPTH. BACKFILL THE TRENCH AND TAMP FIRMLY TO CONFORM TO THE CHANNEL CROSS-SECTION. SECURE WITH A ROW OF STAPLES ABOUT 4° DOWN SLOPE FROM THE TRENCH. SPACING BETWEEN STAPLES IS 6°. 2. STAPLE THE 4" OVERLAP IN THE CHANNEL CENTER USING AN 18" SPACING BETWEEN STAPLES. 5. BEFORE STAPLING THE OUTER EDGES OF THE MATTING, MAKE SURE THE MATTING IS SMOOTH AND IN FIRM CONTACT WITH THE SOIL. 4. STAPLES SHALL BE PLACED 2' APART WITH 4 ROWS FOR EACH STRIP, 2 OUTER ROWS, AND 2 ALTERNATING ROWS DOWN THE CENTER.

TSWM PERFORATION DETAIL

6" TEMPORARY SWM

VERTICAL DRAW-DOWN DEVICE

5. WHERE ONE ROLL OF MATTING ENDS AND ANOTHER BEGINS, THE END OF THE TOP STRIP SHALL OVERLAP THE UPPER END OF THE LOWER STRIP BY 4", SHIPLAP FASHION STRIP STACE THE OVERLAP WITH A DOUBLE ROW OF STAPLES SPACED 6" APART IN A
STAGGERED PATTERN ON EITHER SIDE.

6. THE DISCHARGE END OF THE MATTING LINER SHOULD BE SIMILARLY SECURED WITH 2 DOUBLE ROWS OF STAPLES. NOTE: IF FLOW WILL ENTER FROM THE EDGE OF THE MATTING THEN THE AREA EFFECTED BY THE FLOW MUST 8E KEYED-IN. SOIL STABILIZATION MATTING - (ECM)

NOT TO SCALE

NOTE: FOR TEMP, SWM THE PERFORATED PORTION OF THE DRAW-DOWN DEVICE SHALL BE WRAPPED WITH  $1/2^\circ$  Hardware cloth and geotextile fabric. The geotextile fabric shall meet the specifications for geotextile class E.

WET STORAGE = 292

CONCRETE ANTI-FLOTATION COLLAR (3.0' SQUARE) ---

DETAIL 18 - SEDIMENT BASIN BAFFLES

PLAN MEWS

CAP DETAIL

2. Place a continuous piece of Geotextile Class & the same dimensions as the wire 3. Securely noti the 2' X 4' wein to a 9' long ventical spacer to be located between the wein and the inlet face (nox. 4' apart). 6. Place the assembly against the inlet throat and nail (minimum 2' lengths of  $2^{\prime} \times 4^{\prime}$  to the top of the weir at spacer locations). These  $2^{\prime} \times 4^{\prime}$  anchors shall extend across the inlet top and be held in place by sandbags or alternate weight. 5. The assembly shall be placed so that the end spacers are a minimum  $1^\prime$  beyond both ends of the throat opening. 6 Form the 1/2 \* v 1/2 \* wire mash and the peatextile fabric to the concrete outter against the face of the curb on both sides of the inlet. Place clean  $3/4^\circ \times 1$  1/2 stone over the wire nesh and geotextile in such a nanner to prevent water from

7. This typ. of protection must be inspected frequently and the filter cloth and stone reptaced when clogged with sediment. 8. Assure that storm flow does not bypass the inlet by installing a temporary

ELLICOTT CITY, MD 21041 410-465-4244 U.S. DEPARTMENT OF AGRICULTURE PAGE HARYLAND DEPARTMENT OF ENVIRONMENT Design: JMC Draft: AM STIL CONSERVATION SERVICE E - 16 - 58 VATER MANAGEMENT ADMINISTRATION Check: DAM

