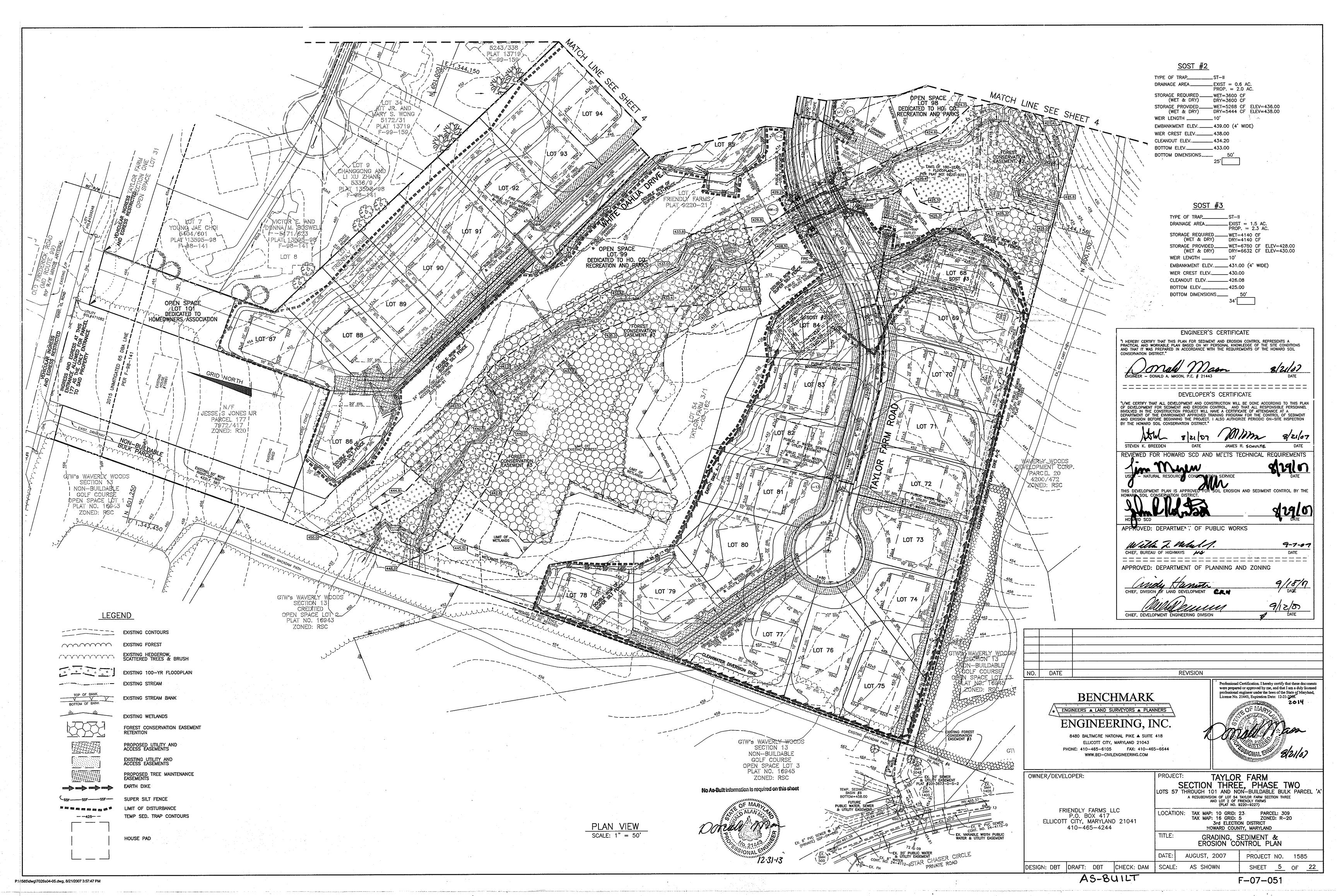


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F-07-051



SEDIMENT CONTROL NOTES

- 1. A MINIMUM OF 24 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT OF INSPECTION, LICENSES AND PERMITS, SEDIMENT CONTROL DIVISION PRIOR TO THE
- ALE VEGETATIVE AND STRUCTURAL PRACTICES ARE TO BE INSTALLED ACCORDING TO THE PROVISIONS OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE MOST CURRENT "MARYLAND STANDARDS AND SPECIFICATION FOR SOIL EROSION AND SEDIMENT CONTROL", REVISIONS THERETO.
- FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: A) 7 CALENDAR DAYS FOR ALL. PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, PERIMETER SLOPES AND ALL SLOPES GREATER THAN 3:1, B) 14 DAYS AS TO ALL OTHER DISTURBED OR GRADED
- ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 12, OF THE HOWARD COUNTY DESIGN MANUAL, STORM DRAINAGE.
- ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDINGS (SEC. 51) SOD (SEC. 54), TEMPORARY SEEDING (SEC. 50) AND MULCHING (SEC. 52), TEMPORARY STABILIZATION WITH MULCH ALONE CAN ONLY BE DONE WHEN RECOMMENDED SEEDING DATES DO NOT ALLOW FOR PROPER GERMINATION AND ESTABLISHMENT OF GRASSES.
- ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE MAINTAINED IN OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.
- 7. SITE ANALYSIS:

AREAS ON THE PROJECT SITE.

TOTAL AREA OF SITE	23.3	ACRES
AREA DISTURBED	13.9	_ ACRES
AREA TO BE ROOFED OR PAVED	4.4	_ ACRES
AREA TO BE VEGETATIVELY STABILIZED	9.5	_ ACRES
TOTAL CUT	21,073	_ CY
TOTAL FILL	32,159	CY
OFFSITE WASTE AREA LOCATION	N/A	_ • • •
		-

- ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF UTILITIES MUST BE REPAIRED ON THE SAME DAY OF DISTURBANCE.
- ADDITIONAL SEDIMENT CONTROL MUST BE PROVIDED, IF DEEMED NECESSARY BY THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.
- 10. ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING. OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL BY THE INSPECTION AGENCY IS MADE.
- TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGTHS OR THAT WHICH CAN BE BACK FILLED AND STABILIZED WITHIN ONE WORKING DAY,

TEMPORARY SEEDBED PREPARATIONS

APPLY TO GRADED OR CLEARED AREAS LIKELY TO BE REDISTURBED WHERE A SHORT-TERM

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED. SOIL AMENDMENTS: APPLY 600 LBS PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ FT). SEEDING: FOR PERIOD MARCH 1 THROUGH APRIL 30 AND FROM AUGUST 15 THROUGH NOVEMBER 15, SEED WITH 2-1/2 BUSHELS PER ACRE OF ANNUAL RYE (3.2 LBS/1000 SQ FT). FOR THE PERIOD MAY 1 THROUGH AUGUST 14, SEED WITH 3 LBS PER ACRE OF WEEPING LOVEGRASS (.07 LBS/1000 SQ FT). FOR THE PERIOD NOVEMBER 16 THROUGH FEBRUARY 28, PROTECT SITE BY APPLYING 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING, OR USE SOD.

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ FT) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ FT) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES, 8 FT. OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ FT) FOR ANCHORING.

REFER TO THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR RATE AND METHODS NOT COVEREI

PERMANENT SEEDBED PREPARATIONS

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED. SOIL AMENDMENTS: IN LIEU OF SOIL TEST RECOMMENDATIONS, USE ON OF THE FOLLOWING

- PREFERRED APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS/1000 SQ FT) AND 600 LBS PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ FT) BEFORE SEEDING. HARROW OR DISC INTO UPPER THREE INCHES OF SOIL. AT TIME OF SEEDING, APPLY 400 LBS PER ACRE 30-0-0- UREAFORM FERTILIZER (9 LBS/1000 SQ FT).
- ACCEPTABLE APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS/1000 SQ FT) AND 1000 LBS PER ACRE 10-10-10 FERTILIZER (23 LBS/1000 SQ FT)

SEEDING: FOR THE PERIODS MARCH 1 THROUGH APRIL 30 AND AUGUST 1 THROUGH OCTOBER 15, SEED WITH 60 LBS PER ACRE (1.4 LBS/1000 SQ FT) OF KENTUCKY 31 TALL FESCUE PER ACRE AND 2 LBS PER ACRE (.05 LBS/1000 SQ FT) OF WEEPING LOVEGRASS. DURING THE PERIOD OF OCTOBER 16 THROUGH FEBRUARY 28 PROTECT SITE BY: OPTION (1) 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING. OPTION (2) USE SOD. OPTION (3) SEED WITH 60 LBS PER ACRE OF KENTUCKY 31 TALL FESCUE AND MULCH WITH 2 TONS PER ACRE OF WELL ANCHORED STRAW.

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ FT) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING, ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ FT) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ FT) FOR ANCHORING MAINTENANCE: INSPECT ALL SEEDED AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND

TOPSOIL SPECIFICATIONS

Topsoil salvaged from the existing site may be used provided that it meets that standards as set forth in these specifications. Typically, the depth of topsoil to be salvaged for a given soil type can be found in the representative soil profile section in the Soil Survey published by USDA—SCS in cooperation with Maryland Agricultural Experimental Station.

II. Topsoil Specifications - Soil to be used as topsoil must meet the following: Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay loam, loamy sand. Other soils may be used if recommended by an agronomist or soil scientist and approved by the appropriate approval authority. Regardless, topsoil shall not be a mixture of contrasting texture subsoils and shall contain less than 5% by volume of cinders, stones, slag, coarse fragments, gravel, sticks, roots, trash, or other materials larger than 1-1/2" in diameter.

Topsoil must be free of plants or plant parts such as Bermuda grass, quack grass, Johnson grass, nutsedge, poison ivy, thistle, or others as specified.

iii. Where the subsoil is either highly acidic or composed of heavy clays, ground limestone shall be spread at the rate of 4-8 tons/acre (200-400 pounds per 1,000 square feet) prior to the placement of topsoil. Lime shall be distributed uniformly over designated areas and worked into the soil in conjunction with tillage operations as described in the following procedures.

III. For sites having disturbed areas under 5 acres: Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization — Section 1 — Vegetative Stabilization Methods and Materials.

IV. For sites having disturbed areas over 5 acres:

On soil meeting Topsoil specifications, obtain test results dictating fertilizer and lime amendments required to bring the soil into compliance with the following:

- pH for topsoil shall be between 6.0 and 7.5. If the tested soil demonstrates a pH of less than 6.0, sufficient lime shall be prescribed to raise the pH to 6.5 or higher.
- b. Organic content or topsoli shall be not less than 1.5 percent by weight.
- Topsoil having soluble salt content greater than 500 parts per million shall not be used.
- No sod or seed shall be placed on soil which has been treated with soil sterilants or chemicals used for weed control until sufficient time has Note: Topsoil substitutes or amendments, as recommended by a qualified agronomist or soil scientist and approved by the appropriate approval authority, may be used in lieu of

Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization — Section I — Vegetative Stabilization Methods and Materials.

- When topsoiling, maintain needed erosion and sediment control practices such as diversions, grade stabilization structures, earth dikes, slope silt fence and sediment
- Grades on the areas to be topsoiled, which have been previously established, shall be maintained, albeit 4" 8" higher in elevation. iii. Topscil shall be uniformly distributed in a 4" — 8" layer and lightly compacted to a minimum thickness of 4". Spreading shall be performed in such a manner that sodding or seeding can proceed with a minimum of additional soil preparation and tillage. Any irregularities in the surface resulting from topsciling or other operations shall be corrected in order to prevent the formation of depressions or
- iv. Topsoil shall not be placed while the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet or in a condition that may otherwise be detrimental to proper grading and seedbed preparation.
- VI. Alternative for Permanent Seeding instead of applying the full amounts of lime and commercial fertilizer, composted sludge and amendments may be applied as specified below:
- 1. Composted Sludge Material for use as a soil conditioner for sites having distributed areas over 5 acres shall be tested to prescribe amendments and for sites having disturbed areas under 5 acres shall conform to the following requirements:
- a. Composted sludge shall be supplied by, or originate from, a person or persons that are permitted (at the time of acquisition of the compost) by the Maryland Department of the Environment under COMAR 26.04.03. b. Composted sludge shall contain at least 1 percent nitrogen, 1.5 percent phosphorus, and 0.2 percent potassium and have a pH of 7.0 to 8.0. If compost does not meet these requirements, the appropriate constituents must be added to meet the requirements prior to use.
- c. Composted sludge shall be applied at a rate of 1 ton/1,000 square feet iv. Composted sludge shall be amended with a potassium fertilizer applied at the rate of 4 lb/1,000 square feet, and 1/3 the normal lime application rate. References: Guidelines Specifications, Soil Preparation and Sodding. MD-VA, Pub. #1, Cooperative Extension Service, University of Maryland and Virginia Polytechnic Institutes, Revised 1973.

30.0 DUST CONTROL

Controlling dust blowing and movement on construction sites and roads.

To prevent blowing and movement of dust from exposed soil surfaces, reduce on and off—site Conditions Where Practice Applies

This practice is applicable to areas subject to dust blowing and movement where on and off-site

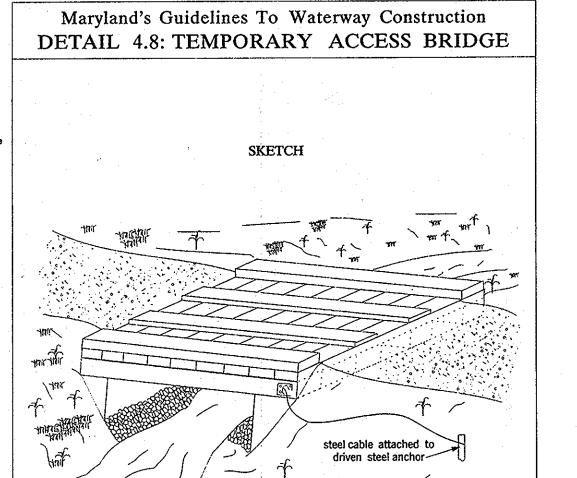
<u>Temporary Methods</u>

1. Mulches — See standards for vegetative stabilization with mulches only. Mulch should be crimped or tracked to prevent blowing.

2. Vegetative Cover - See standards for temporary vegetative cover.

- 3. Tillage To roughen surface and bring clods to the surface. This is an emergency measure which should be used before soil blowing starts. Begin plowing on windward side of site. Chisel—type plows spaced about 12" apart, spring—toothed harrows, and similiar plows are examples of equipment which may produce the desired effect.
- 4. Irrigation This is generally done as an emergency treatment. Site is sprinkled with water until the surface is moist. Repeat as needed. At no time should the site be irrigated to the point that runoff begins to flow.
- 5. Barriers Solid board fences, silt fences, snow fences, burlap fences, straw bales, and similiar material can be used to control air currents and soil blowing. Barriers placed at right angles to prevailing currents at intervals of about 10 times their height are effective in controlling soil blowing.
- 6. Calcium Chloride Apply at rates that will keep surface moist. May need retreatment.
- Permanent Methods

 1. Permanent Vegetation See standards for permanent vegetative cover, and permanent stabilization with sod. Existing trees or large shrubs may afford valuable protection if left in place.
- 2. Topsoiling Covering with less erosive soil materials. See standards for topsoiling
- 3. Stone Cover surface with crushed stone or coarse gravel. Agriculture Handbook 346. Wind Erosion Forces in the United States and Their Use in Predicting Soil Loss.
- 2. Agriculture Information Bulletin 354. How to Control Wind Erosion, USDA-ARS.



STONE OUTLET SEDIMENT TRAP - ST II DETAIL 9 - STONE OUTLET SEDIMENT TRAP - ST II CONSTRUCTION SPECIFICATIONS TOP OF EMBANKMENT --WEIR LENGTH The structure shall be inspected periodically and after each rain and repairs made Construction of traps shall be carried out in such a manner that sediment pollution is abated. Once constructed, the top and outside face of the embankment shall be stabilized with seed and mutch. Points of concentration inflow shall be protected in accordance with grade stabilization structure criteria. The remainder of the interior slopes should be stabilized (one time) with seed and mutch upon trap SECTION B-B The Structure shall be dewatered by approved methods, removed and the area PROSPECTIVE VIEW Refer to Section D for specifications concerning trap dewatering. OUTLET ELEVATION The elevation of the top if any dike directing water into the trap must equal or exceed the elevation of the trap embankment. ∵~ GEOTEXTILE CLASS C NOTE: S' HINIMUM LENGTH UP TO S ACRES. OVER 5 ACRES USE STONE/RIPRAP SEDIMENT TRAP ST-TV. 2. Geotextile Class C shall be placed over the bottom and sides of the outlet chann prior to the placement of stone. Sections of filter cloth must overlap at least 1 with the section nearest the entrance placed on top. The filter cloth shall be L BOTTOM ELEVATION SECTION_A=A CONSTRUCTION SPECIFICATIONS embedded at least 6" Into existing ground at the entrance of the outlet channe . Area under embankment shall be cleared, grubbed and stripped of any vegetation Outlet — An outlet shall be provided, including a means of conveying the discharge in an erosion free manner to an existing stable channel. 2. The fill material for the embankment shall be free of roots and other woody vegetation as well as over—sized stones, rocks, organic material or other objectionable material. The embankment shall be compacted by traversing with equipment while it is being constructed. 3. All cut and fill slopes shall be 2:1 or flatter. 4. The stone used in the outlet shall be small rip—rap 4" to 7" in size with a 1' thick layer of 3/4" to 11/2" washed aggregate placed on the upstream face of the outlet. Stone facing shall be an necessary to prevent clagging. Geotextile Class C may be substituted for the stone facing by placing it on the inside face of the stone outlet. Sediment shall be removed and trap restored to its original dimensions when the sediment has accumulated to one half of the wet storage depth of the trap.

J.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT U.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT SOIL CONSERVATION SERVICE C - 9 - 10A WATER MANAGEMENT ADMINISTRATION SOIL CONSERVATION SERVICE C - 9 - 10A WATER MANAGEMENT ADMINISTRATION

Removed sediment shall be deposited in a suitable area and in such a manner tha

MGWC 4.8: TEMPORARY ACCESS BRIDGE

Temporary stream crossing intended for

minimum corridor disturbance

A temporary access bridge is a stream crossing made of wood, metal, or other materials designed to limit the amount

Temporary access bridges are the preferred method of waterway crossing since they typically cause the least

• Stringers: Stringers should either be logs, sawn timber, prestressed concrete beams, metal beams, or other

All erosion and sediment control devices, including stream diversions, should be implemented as the first order of

business according to a plan approved by the WMA or local authority. Dewatering basins should be built as needed and swales or ditches should be used to prevent surface drainage from entering the stream via the bridge crossing. (See the 1994 Maryland Standards and Specifications for Soil Erosion and Sediment Control.) The proposed

1. Abutments should be placed parallel to, and on, stable banks such that the structure is at or above bankfull depth

2. Temporary access bridges should be constructed to span the entire channel. If the bankfull channel width

bridge support will be permitted for each additional 8-foot width of the channel.

exceeds 8 feet (2.5 meters), then a footing, pier, or other bridge support may be constructed within the

3. All decking members should be placed perpendicularly to the stringers, butted tightly, and securely fastened to the stringers. Decking materials must be butted tightly to prevent any soil material tracked onto the bridge from

4. Although run planks are optional, they may be necessary to properly distribute loads. One run plank should be

6. Bridges should be securely anchored at one end using steel cable or chain to prevent the bridge from floating

5. Curbs or fenders may be installed along the outer sides of the deck to provide additional safety.

provided for each track of the equipment wheels and should be securely fastened to the length of the span.

downstream and possibly causing an obstruction to the flow. Anchoring at only one end will prevent channel

obstruction in the event that flood waters float the bridge. Acceptable anchors are large trees, boulders, or

PAGE 4.8 - 1

waterway. No support will be permitted within the channel for waterways less than 8 feet wide. One additiona

Deck Materials: Deck materials should be of sufficient strength to support the anticipated load

disturbance to the waterway bed and banks, pose the least chance for interference with fish migration, and can be

DESCRIPTION

of disturbance to the stream banks and bed.

EFFECTIVE USES & LIMITATIONS

ouickly removed and reused.

MATERIAL SPECIFICATIONS

CONSTRUCTION SEQUENC

STREAM CROSSINGS

construction, maintenance, and removal sequence is as follows:

to prevent the entrapment of floating materials and debris.

SEQUENCE OF CONSTRUCTION

NOTIFY SEDIMENT CONTROL DIVISION 48 HOURS PRIOR TO START OF CONSTRUCTION 1. OBTAIN GRADING PERMIT. (DAY 1)

2. INSTALL STABILIZED CONSTRUCTION ENTRANCE, TEMPORARY ACCESS BRIDGE, TREE PROTECTION FENCES, SUPER SILT FENCES, AND TEMPORARY CLEANWATER DIVERSION DIKES. (DAY 2-12)

3. INSTALL ALL 3 STONE OUTLET SEDIMENT TRAPS (DAY 13-60)

4. INSTALL EARTH DIKES AND ANY REMAINING SEDIMENT CONTROL DEVICES. (DAY 61-63)

5. REMOVE TEMPORARY ACCES BRIDGE. INSTALL CON/SPAN BRIDGE. (DAY 64-69)

5. UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, BRING ROAD BEDS TO SUBGRADE AND STABILIZE SLOPES IN ACCORDANCE WITH TEMPORARY SEEDBED NOTES. UTILIZE DUST CONTROL METHODS. NO MORE THAN 20 AC. MAY BE UNSTABILIZED AT ANY TIME. (DAY 70-81)

6. UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, INSTALL STORM DRAINS, WATER MAINS, SEWER MAINS AND SWALES. (DAY 82-112)

7. PAVE ROADWAYS. (DAY 113-128)

8. COMPLETE MASS GRADING OF SITE AND STABLIZE DISTURBED AREAS IN ACCORDANCE WITH THE PERMANENT SEEDBED NOTES. (DAY 129-144)

9. UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, REMOVE SEDIMENT CONTROL DEVICES, AND STABILIZED DISTURBED AREAS IN ACCORDANCE WITH THE PERMANENT SEEDBED NOTES. (DAY 154-161)

No As-Built information is required on this sheet

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

License No. 21443 Expiration Date: 12-21-14



MGWC 4.8: TEMPORARY ACCESS BRIDGE

7. All areas disturbed during installation should be stabilized within 14 calendar days in accordance with a revegetation plan approved by the WMA.

8. Periodic inspection should be performed by the user to ensure that the bridge, streambed, and stream banks are maintained and not damaged.

9. Maintenance should be performed as needed to ensure that the structure complies with all standards and specifications. This should include the removal of trapped sediment and debris which should then be disposed of and stabilized outside the floodplain

10. When the temporary bridge is no longer needed, all structures including abutments and other bridging materials should be removed within 14 calendar days. In all cases, the bridge materials should be removed within 1 year of installation. Removal of the bridge and clean-up of the area, including protection and stabilization of disturbed stream banks, should be accomplished without the use of construction equipment in the waterway

ENGINEER'S CERTIFICATE I HEREBY CERTIFY THAT THIS PLAN FOR SEDIMENT AND EROSION CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL

DEVELOPER'S CERTIFICATE

"I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN OF DEVELOPMENT FOR SEDIMENT AND EROSION CONTROL, AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ON—SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT."

4/29/07

7/17/07

1-20-07

REVIEWED FOR HOWARD SCD AND MEETS TECHNICAL REQUIREMENTS

USDA - NATURAL RESOURCES) POSSERVATION SERVICE 7/17/07

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE

APPROVED: DEPARTMENT OF PUBLIC WORKS

Willia T. Mala

CHIEF, BUREAU OF HIGHWAYS

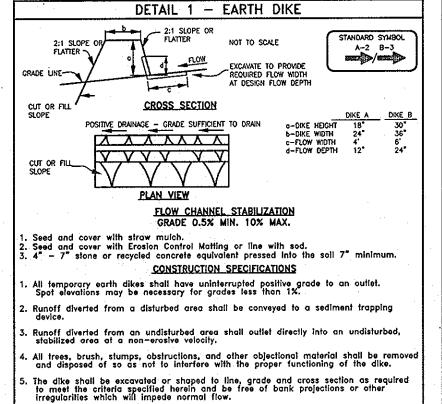
APPROVED: DEPARTMENT OF PLANNING AND ZONING

STREAM CROSSINGS PAGE 4.8 - 2

NO. I

MARYLAND DEPARTMENT OF THE ENVIRONMEN REVISED NOVEMBER 200

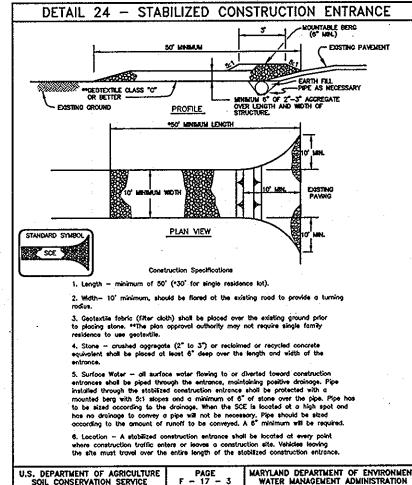
Tund CHIEF, DIVISION OF LAND DEVELOPMENT 7/24/07 CHIEF, DEVELOPMENT ENGINEERING DIVISION REVISION

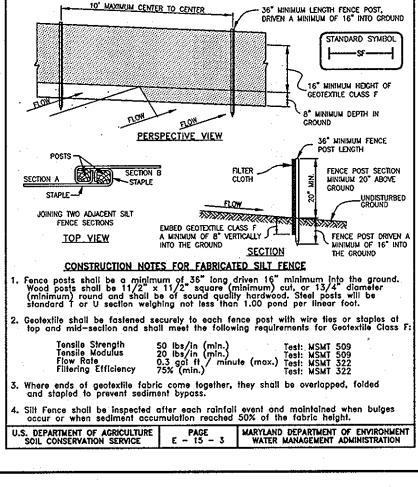


. All earth removed and not needed for construction shall be placed so that it will not interfere with the functioning of the dike.

inspection and maintenance must be provided periodically and after each rain event.

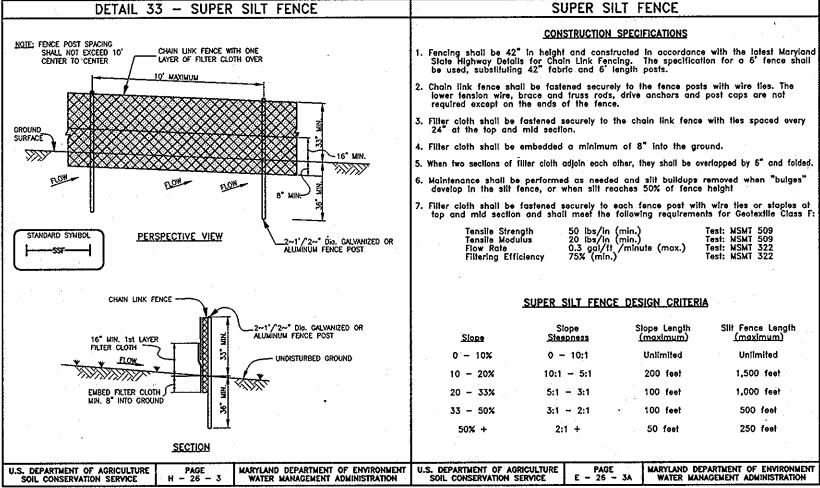
. Fill shall be compacted by earth moving equipment.



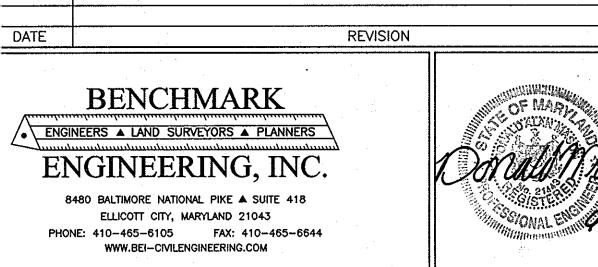


DETAIL 22 - SILT FENCE

MARYLAND DEPARTMENT OF THE ENVIRONMENT WATER MANAGEMENT ADMINISTRATION



MARYLAND DEPARTMENT OF THE ENVIRONME



PROJECT OWNER/DEVELOPER: TAYLOR FARM SECTION THREE, PHASE TWO THROUGH 101 AND NON-BUILDABLE BULK PARCEL 'A A RESUBDIVISION OF LOT 54 TAYLOR FARM SECTION THREE AND LOT 2 OF FRIENDLY FARMS (PLAT NO. 9220-9227) FRIENDLY FARMS LLC P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041 410-465-4244

LOCATION: TAX MAP: 10 GRID: 23 TAX MAP: 16 GRID: 5 ZONED: R-20 3rd ELECTION DISTRICT HOWARD COUNTY, MARYLAND TITLE: SEDIMENT AND EROSION CONTROL NOTES AND DETAILS

DATE: I JULY, 2007 PROJECT NO. 1585 CHECK: DAM SCALE: SHEET 6 OF 22 AS SHOWN

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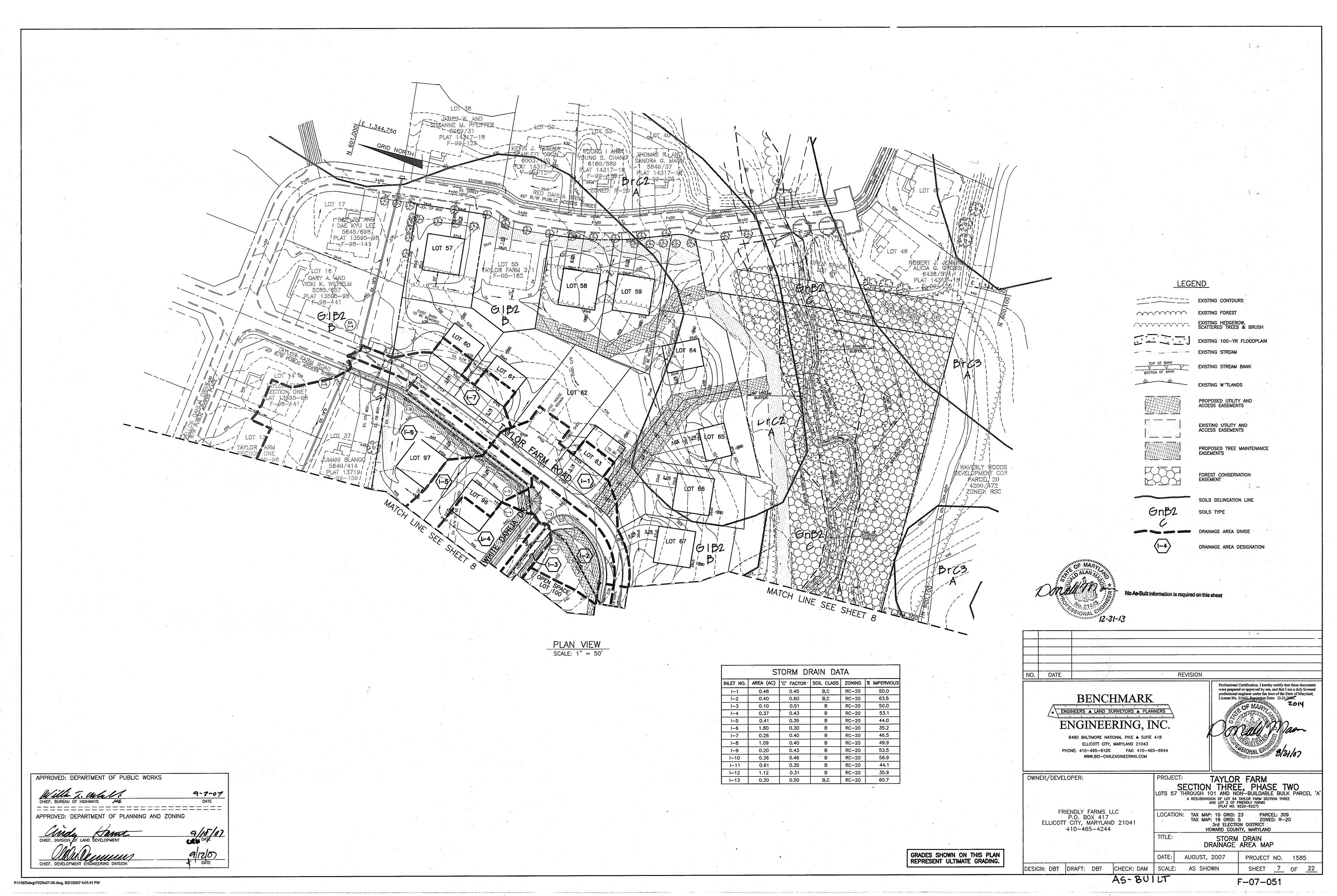
RESEEDINGS.

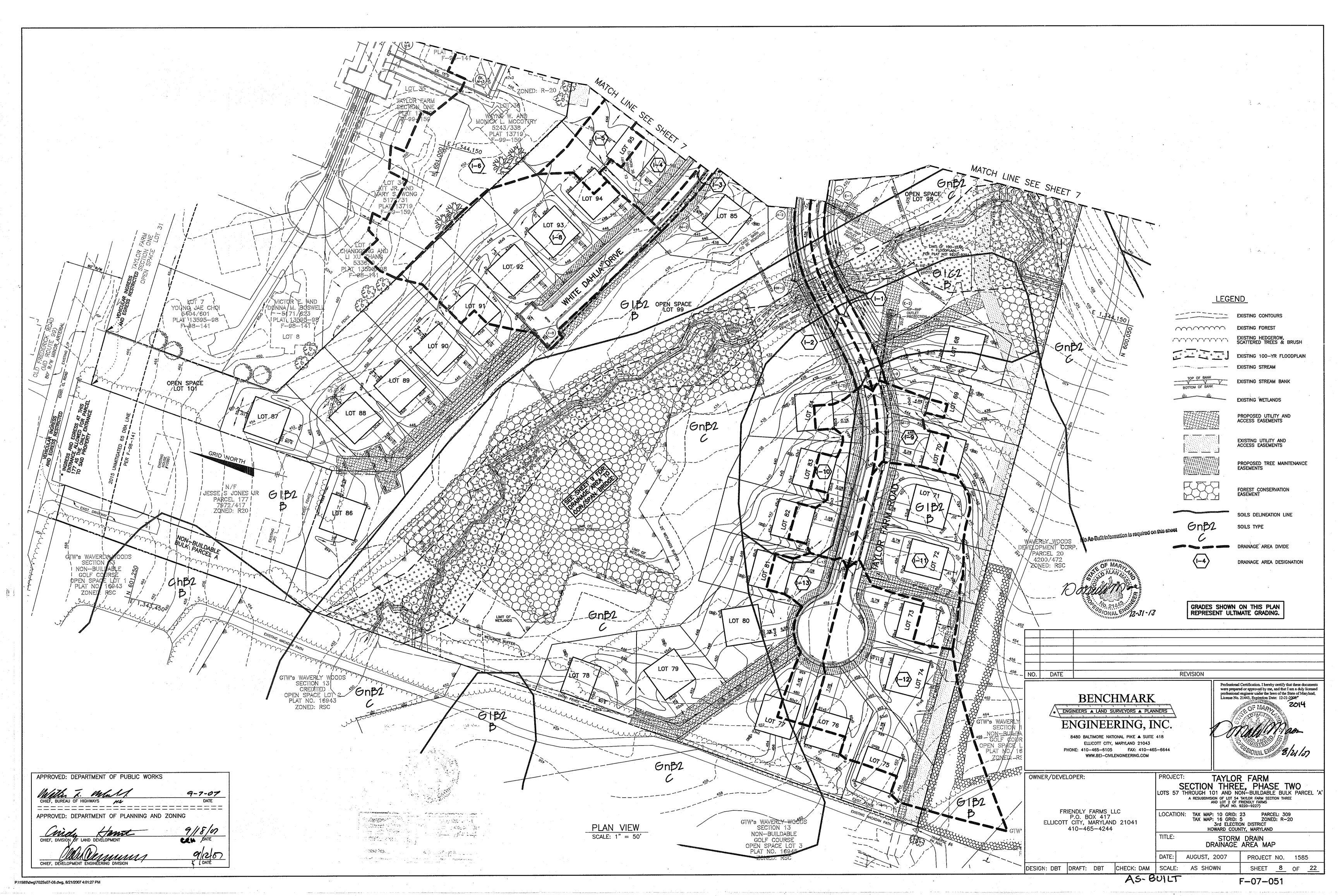
SUPER SILT FENCE

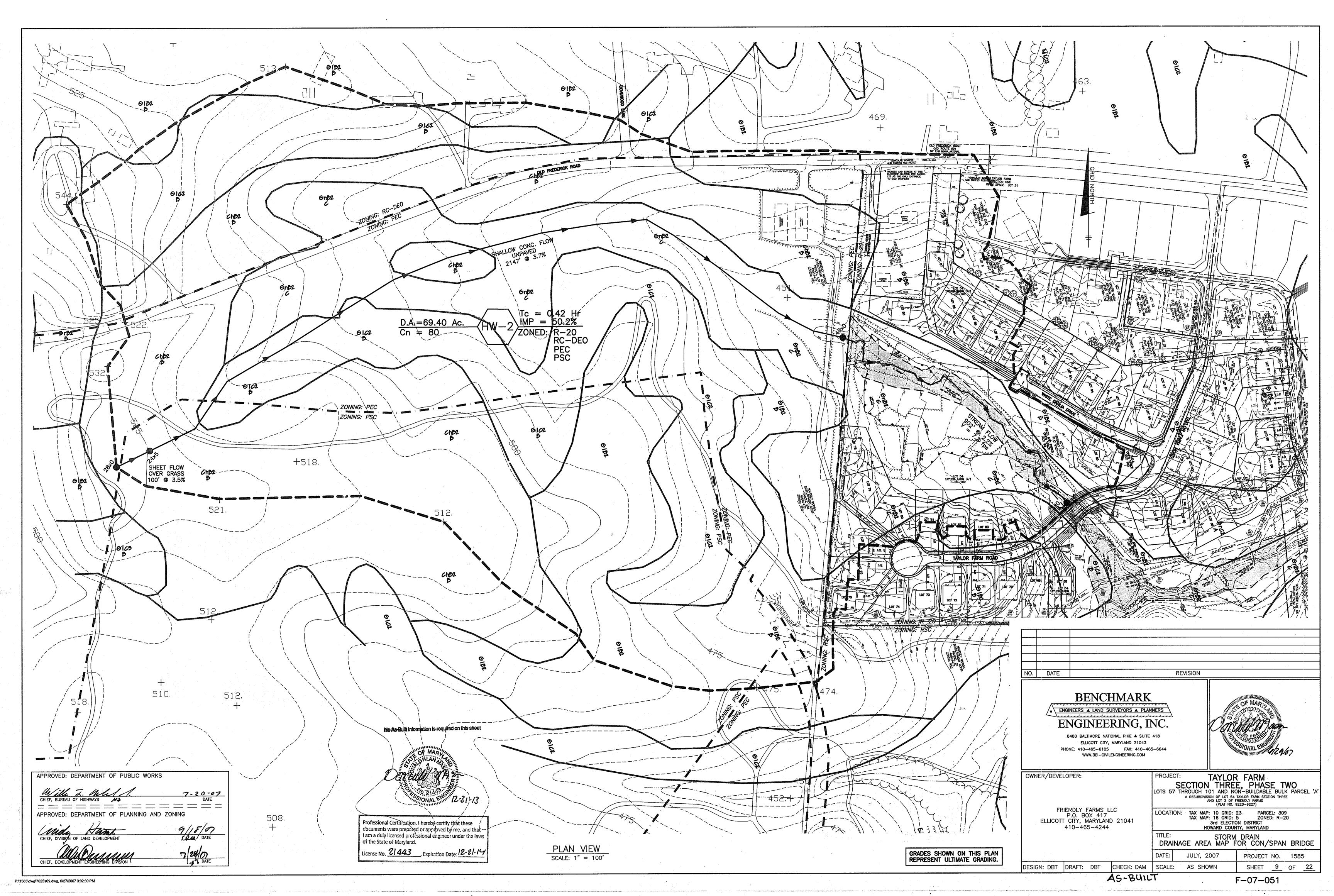
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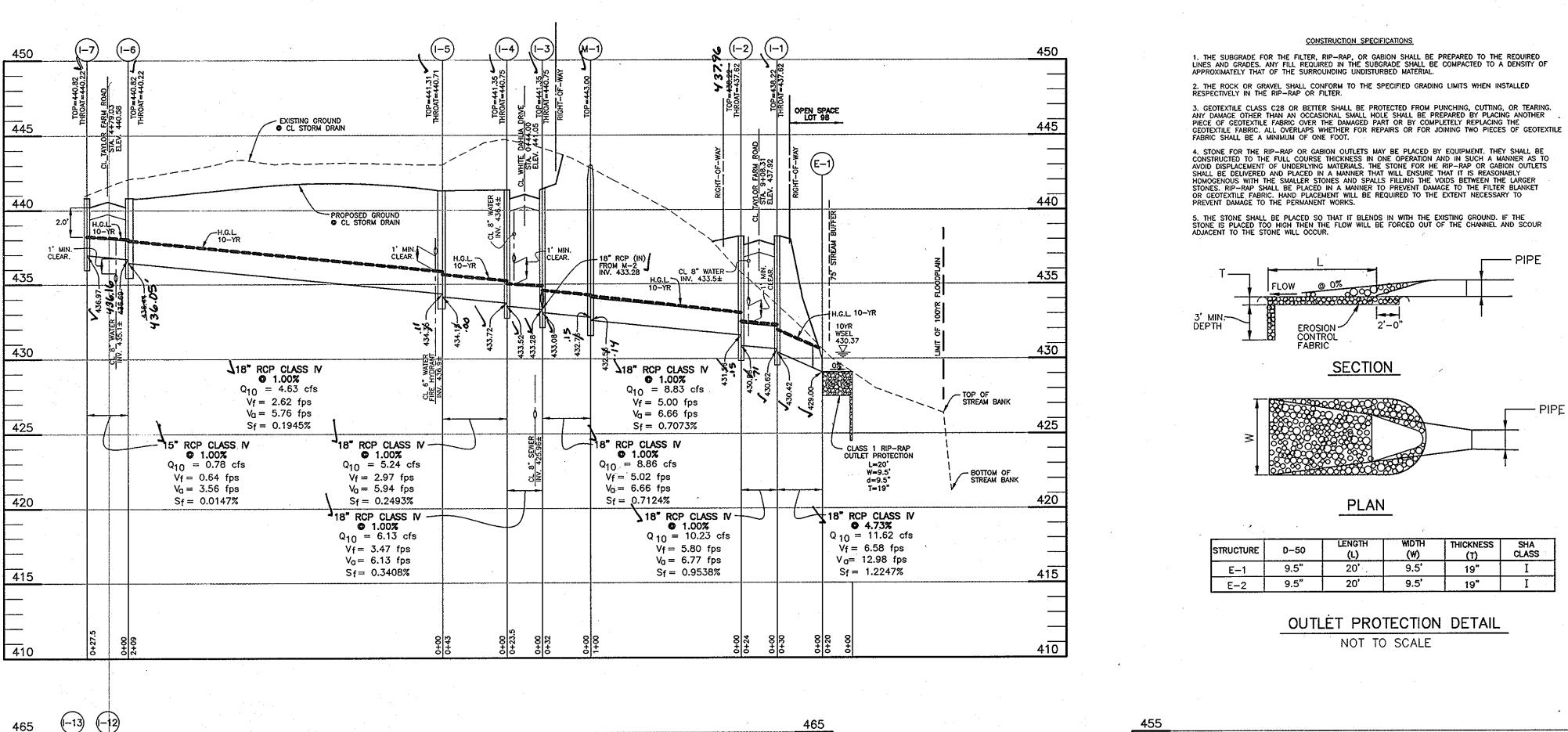
AS-BUILT

F-07-051









445

440

GRANITE BLOCK ---PROTECTION

435 RCP CLASS IV

-- Sf = 0.0110%

425

Q₁₀ = 1.10 cfs V_f = 0.62 fps V_q = 7.00 fps

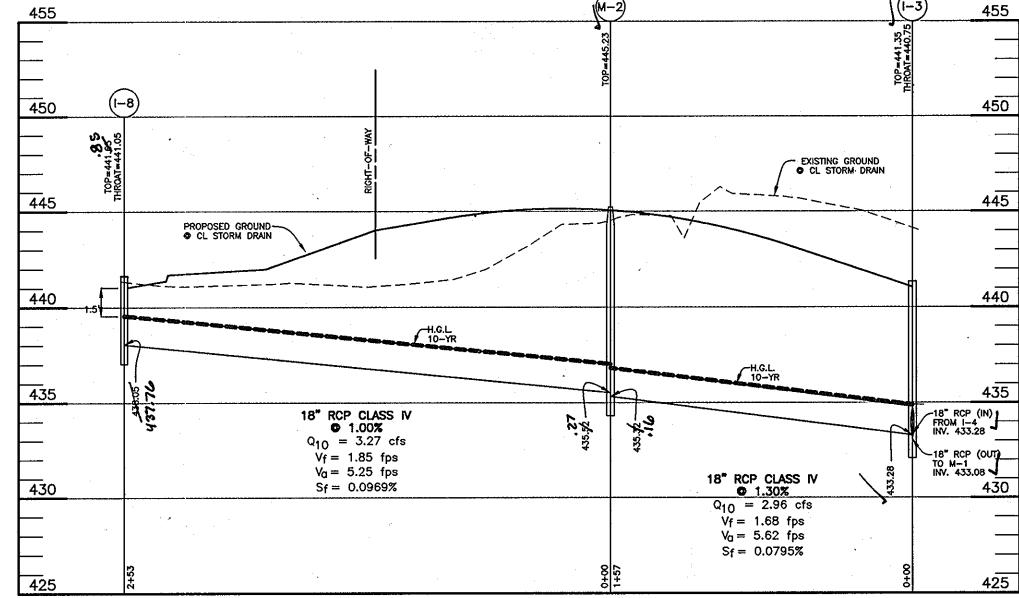
445

TOP OF STREAM BANK

CLASS 1 RIP-RAP
OUTLET PROTECTION
L=20'
W=9.5'
d=9.5"
T=19"

		STRUCTURE SCHEDU	ILE			
NO.	TYPE	LOCATION	PIPE INV. IN	PIPE INV. OUT	TOP ELEV.	HO. CO. STD.
E-1	18" CONC. END SECT.	N 600463.6275 E 1344201.6810	NA	429.00	NA .	SD - 5.51
E-2	18" CONC. END SECT.	N 600360.2715 E 1344063.7619	NA	427.91	NA	SD - 5.51
1-1	A-5	CL STA. 9+08.31 OFFSET 10.43' LEFT TAYLOR FARM ROAD	430.62	430.42	438.22	SD - 4.01
I-2	A-5	CL STA. 9+08.31 OFFSET 10.43' RIGHT TAYLOR FARM ROAD	431.56 .1 5	430.86.71	438 .22 7.9	SD - 4.01
I-3	A-5	CL STA. 0+44.00 OFFSET 10.43' LEFT WHITE DAHLIA DRIVE	433.28 🗸 433.28 🗸	433.08	441.35 🗸	SD - 4.01
1-4	A-5	CL STA. 0+44.00 OFFSET 10.43' RIGHT WHITE DAHLIA DRIVE	433.72	433.52 🗸	441,35 🗸	SD - 4.01
I - 5	A-5	CL STA. 6+88.00 OFFSET 12.43' RIGHT TAYLOR FARM ROAD	43 4.35- 5.11	434. 16.00	441.31 🗸	SD - 4.01
I6	A5	CL STA. 4+79.03 OFFSET 12.47' RIGHT TAYLOR FARM ROAD	436. 89.16	436.14.05	440.82	SD - 4.01
I7	A5	CL STA. 4+79.03 OFFSET 12.40' LEFT TAYLOR FARM ROAD	NA	436.97	440.82 🗸	SD - 4.01
I – 8	A-5	CL STA. 4+49.17 OFFSET 26.83' LEFT WHITE DAHLIA DRIVE	NA _	43 8.05 7.74	441.85.85	SD - 4.01
1-9	A-5	CL STA. 11+80.29 OFFSET 10.41' LEFT TAYLOR FARM ROAD	435.90 (15") 435.65 (18")	431.66	443.80	SD - 4.01
1-10	A-5	CL STA. 11+80.29 OFFSET 10.45' RIGHT TAYLOR FARM ROAD	NA	43 7.10 6.89	443.80	SD - 4.01
!11	A10	CL STA. 12+88.49 OFFSET 10.37' LEFT TAYLOR FARM ROAD	441.X6 .29	44 1.56 0.92	448.30 🗸	SD - 4.02
I-12	A-10	CL STA. 14+25.00 OFFSET 10.43' LEFT TAYLOR FARM ROAD	44 8.86 7.7 (44 8.61 7.75	453.98 🗸	SD - 4.02
1-13	A-5	CL STA. 14+25.00 OFFSET 10.43' RIGHT TAYLOR FARM ROAD	NA	449.33	453.98	SD - 4.01
M-1	4'-0" MANHOLE	CL STA. 0+44.00 OFFSET 43.75' LEFT WHITE DAHLIA DRIVE	432.76.15	432.56.14	443.00 🗸	G - 5.12
M-2	4'-0" MANHOLE	CL STA. 1+99.46 OFFSET 15.00' LEFT WHITE DAHLIA DRIVE	435. 52. 27	435.32.16	445.23	G - 5.12
M-3	4'-0" MANHOLE	CL STA. 12+19.58 OFFSET 15.00' LEFT TAYLOR FARM ROAD	437. %3 .65	437.7 3.54	445.31	G - 5.12
		·				
HW-1	SEE SHEETS 14-20	CL STA. 10+29.25 OFFSET 30.00' LEFT TAYLOR FARM ROAD	NA	NA	432 .25 6.30	NA NA
HW-2	SEE SHEETS 14-20	CL STA. 10+29 25 OFFSET 30.00' RIGHT TAYLOR FARM ROAD	NA	NA	432 .25 6.3	Z NA
1) 2) 3) 4) 5)	STRUCTURE ELEVATION STRUCTURE ELEVATION	AND LOCATION FOR MANHOLES IS AT THE TOP / CENTER OF FAND LOCATION FOR INLETS IS AT THE FRONT MIDPOINT OF THE AND LOCATION FOR HEADWALLS IS AT THE MIDPOINT OF THE FAND LOCATION FOR ENDSECTIONS IS AT THE MIDPOINT OF THE MEETING HS-20 LOADING MAY BE USED.	INLET. RONT OF STRUC		- - - -	

PIPE SCHEDULE					
PIPE SIZE	LENGTH	TYPE			
15"	74.5'	RCCP CLASS IV			
18"	1197.5'	RCCP CLASS IV			
42' span x 10' height	60'	ASTM C150 TYPE 1, 2 OR 3			

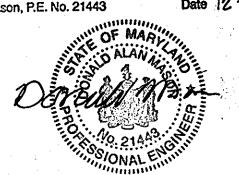


AS-BUILT CERTIFICATION I hereby certify, by my seal, that the facilities shown on this plan were constructed as shown on this AS-BUILT plan.

Donald Mason, P.E. No. 21443

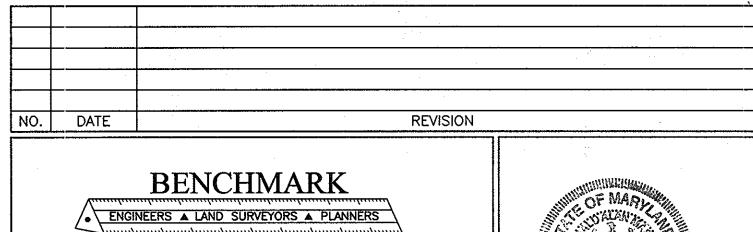
Date 12 -31 -/3

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Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 21443 Expiration Date: 12-21-14

4.17. 7 4111	7,3,4,4
CHIEF, BUREAU OF HIGHWAYS 43	7-20-07 DATE
APPROVED: DEPARTMENT OF PLANNING A	AND ZONING
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CHIEF, DIVISION OF LAND DEVELOPMENT CALL	/ DATE
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ENGINEERING, INC.

8480 BALTIMORE NATIONAL PIKE A SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644 WWW.BEI-CIVILENGINEERING.COM



OWNER/DEVELOPER: FRIENDLY FARMS LLC P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041 410-465-4244	PROJECT: TAYLOR FARM SECTION THREE, PHASE TWO LOTS 57 THROUGH 101 AND NON-BUILDABLE BULK PARCEL 'A' A RESUBDIVISION OF LOT 54 TAYLOR FARM SECTION THREE AND LOT 2 OF FRIENDLY FARMS (PLAT NO. 9220-9227)						
	LOCATION: TAX MAP: 10 GRID: 23 PARCEL: 309 TAX MAP: 16 GRID: 5 ZONED: R-20 3rd ELECTION DISTRICT HOWARD COUNTY, MARYLAND						
	TITLE: STORM DRAIN PROFILES						

JULY, 2007 PROJECT NO. 1585 DESIGN: DBT DRAFT: DBT CHECK: DAM SCALE: AS SHOWN SHEET 10 OF 22

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460

455

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EXISTING GROUND -
◆ CL STORM DRAIN

15" RCP CLASS IV © 2.00% Q₁₀ = 0.99 cfs

 $V_{f} = 0.81 \text{ fps}$ $V_0 = 4.87 \text{ fps}$

 $S_f = 0.0234\%$

18" RCP CLASS IV 5.00% Q₁₀ = 3.21 cfs

 $V_f = 1.82 \text{ fps}$ $V_0 = 9.31 \text{ fps}$

 $S_f = 0.0936\%$

H.G.L. 10-YR

Q₁₀ = 4.35 cfs V₀ = 2.46 fps V₀ = 10.15 fps S_f = 0.1714%

 $V_{Q} = 10.09 \text{ fps}$

 $S_f = 0.1642\%$

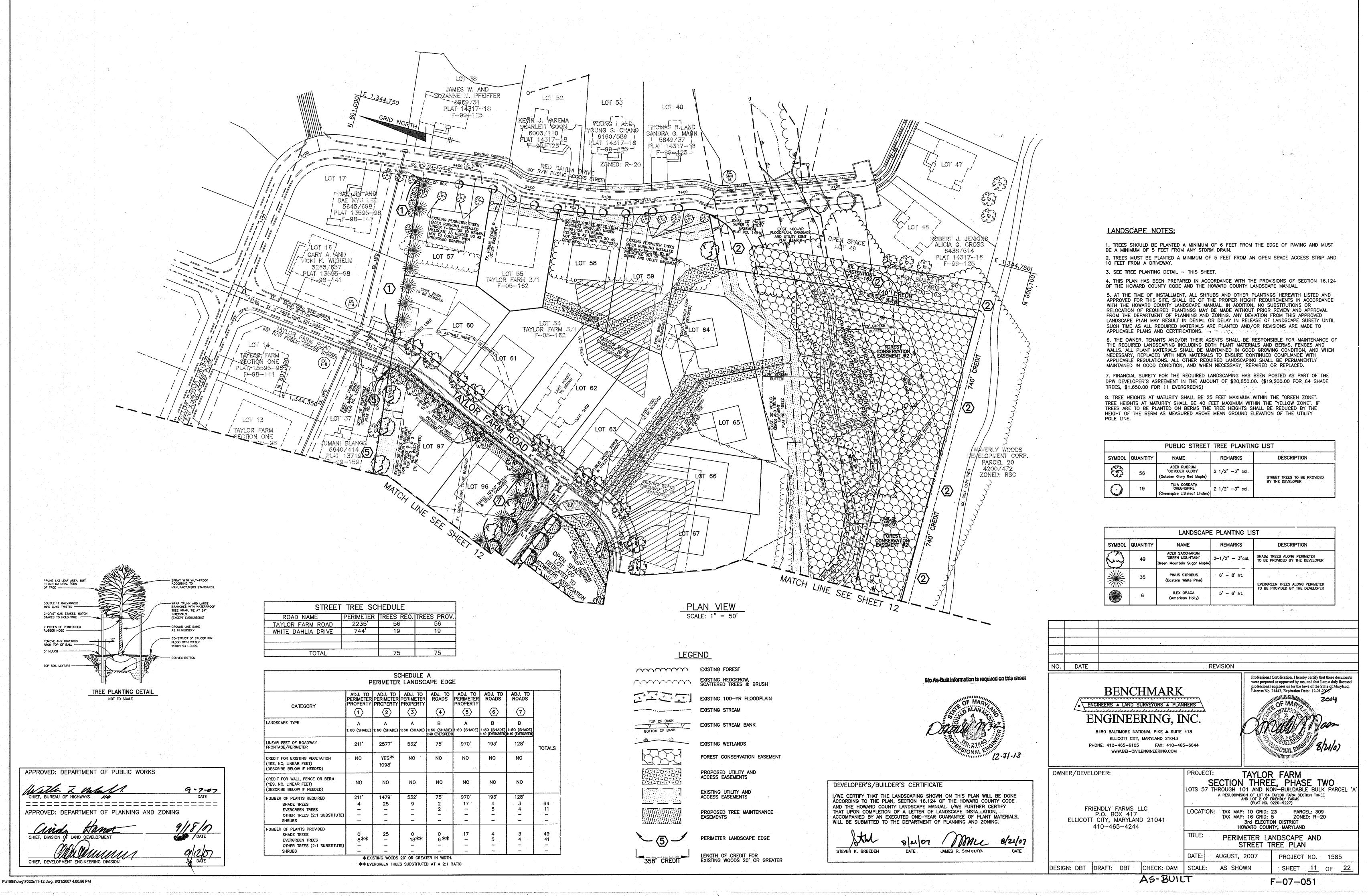
GRANITE BLOCK
PROTECTION

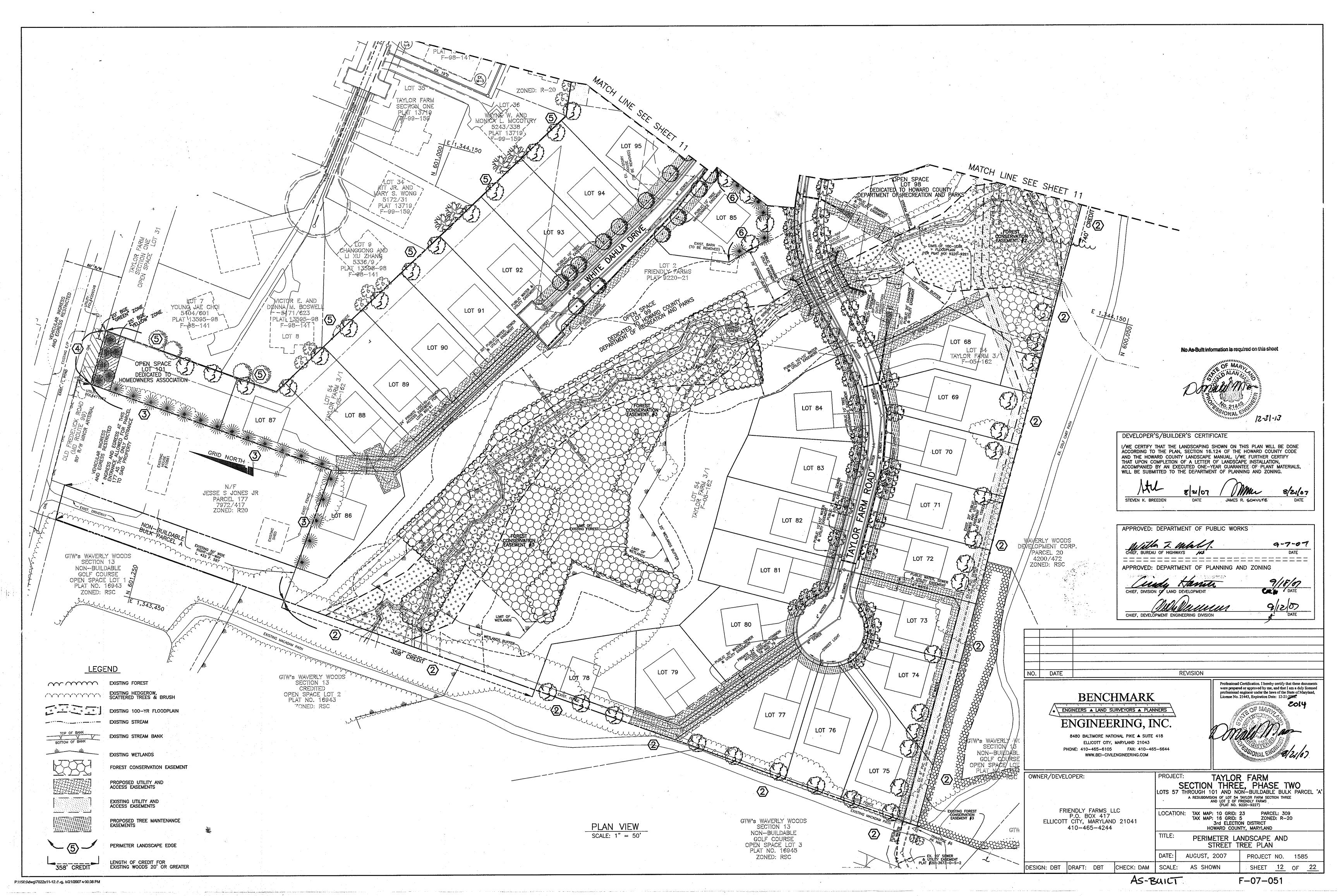
18" RCP CLASS IV 5.00% Q₁₀ = 5.71 cfs V_f = 3.23 fps

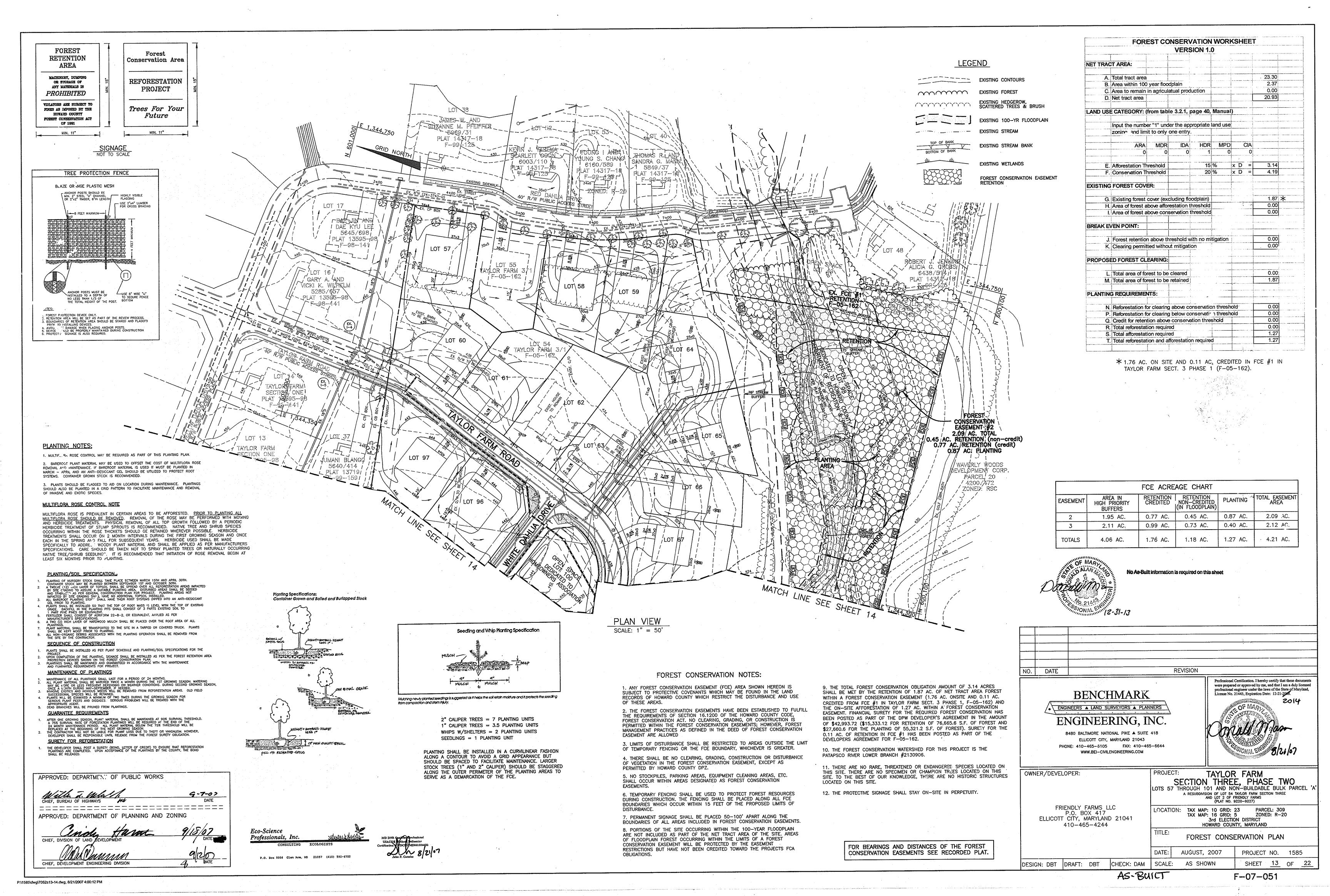
Va= 10.96 fps $S_f = 0.2956\%$

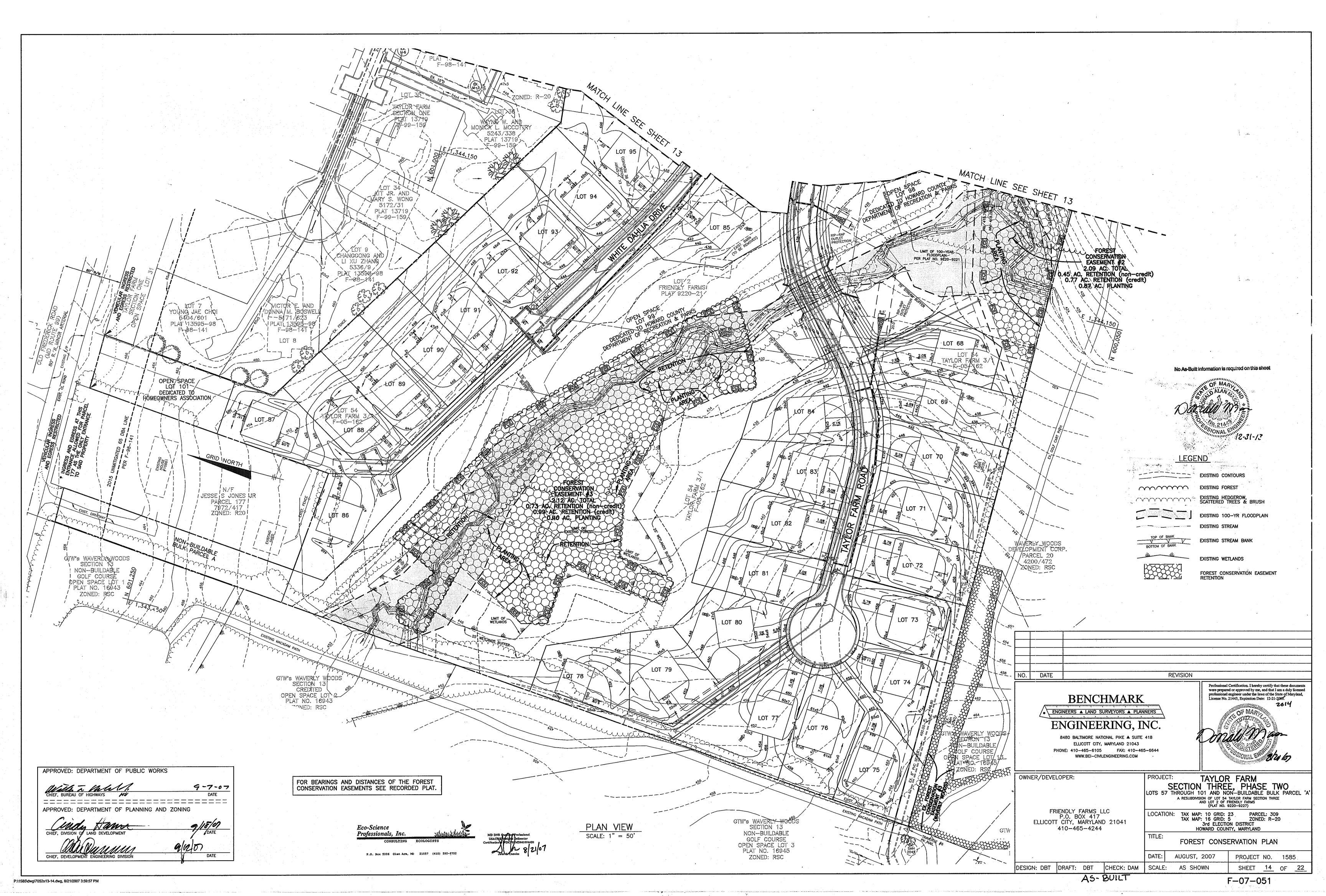
AS-BUILT

F-07-051









	Installation Drawings Sheet Index
CT1	TITLE SHEET & GENERAL NOTES
CT2	BRIDGE PLAN & DETAIL
СТЗ	FOUNDATION PLAN
СТ4	FOOTING SECTION & DETAILS
СТ5	ELEVATIONS
СТ6	SECTION & DETAILS
СТ7	SPECIFICATIONS
СТ8	SPECIFICATIONS

TAYLOR FARM SECTION THREE, PHASE TWO

NOTES

GENERAL NOTES:

- 1. This bridge has been designed for general site conditions. The project engineer shall be responsible for the structure's suitability to the existing site conditions and for the hydraulic evaluation -including scour and confirmation of soil conditions.
- 2. Prior to construction, contractor must verify all elevations shown through the engineer.
- 3. Only CONTECH Bridge Solutions Inc. the CON/SPAN® approved precaster in Maryland may provide the structure designed in accordance with these plans.
- 4. The use of another precast structure with the design assumptions used for the CON/SPAN® structure may lead to serious design errors. Use of any other precast structure with this design and drawings voids any certification of this design and warranty. CONTECH Bridge Solutions Inc. assumes no liability for design of any alternate or similar type structures.
- 5. Alternate structures may be considered, provided that signed and sealed design drawings (and calculations) are submitted to the engineer 2 weeks prior to the bid date for review and approval.
- 6. Proposed alternates to a CON/SPAN® Bridge System must submit at least two (2) independently verified full scale load tests that confirm the proposed design methodology of the three sided/arch structure(s). The proposed alternate, upon satisfactory confirmation of design methodology, may be considered an acceptable

DESIGN DATA

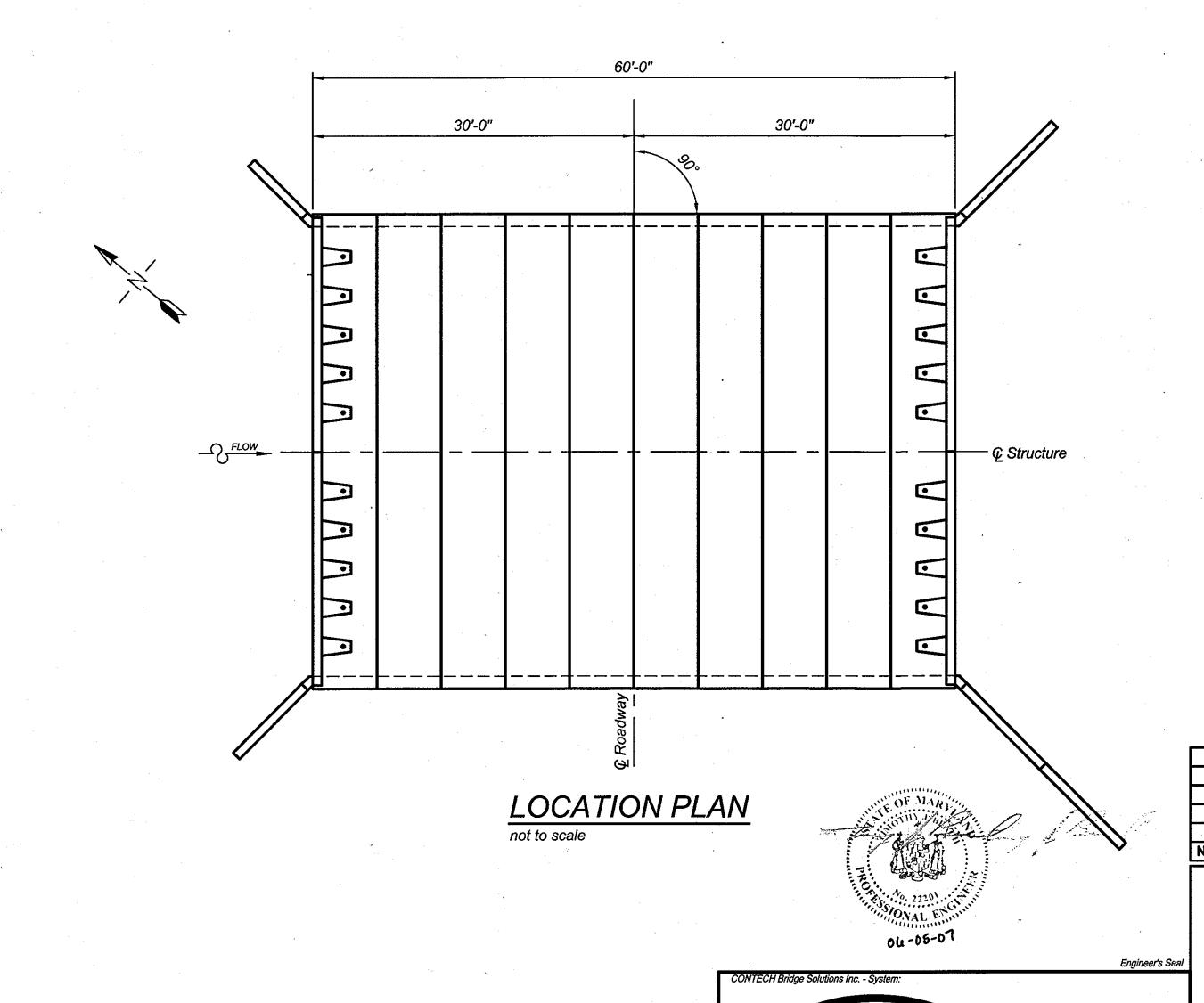
Design Loading:

Bridge Units: HS25-44 + Maryland Legal Loads Headwalls: Earth Pressure Only Wingwalls: Earth Pressure Only Design Fill Height: 2'-0" min. to 6'-0" max. from top of crown to top of pavement. Design Method: Load factor per AASHTO Specification Net allowable soil bearing pressure: 6000 PSF * Gross allowable soil bearing pressure: 6000 PSF *

*Foundation excavation and subgrade preparation shall be in accordance with the geotechnical report for this project prepared by Hills-Carnes Engineering Associates, Inc. dated 9/28/06.

MATERIALS

Precast units shall be constructed and installed in accordance with CON/SPAN® Specifications. Concrete for Footings shall have a minimum compressive strength of 4000 psi. Reinforcing steel for footings shall conform to ASTM A615 or A996-Grade 60.



documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 21443 Expiration Date: 12-21-14

APPROVED: DEPARTMENT OF PUBLIC WORKS CHIEF, BUREAU OF HIGHWAYS HS 7-20-07 APPROVED: DEPARTMENT OF PLANNING AND ZONING CHIEF, DIVISION OF LAND DEVELOPMENT CAN 9/18/07 VATE

NO. DATE REVISION **BENCHMARK** ENGINEERS A LAND SURVEYORS A PLANNERS ENGINEERING, INC

8480 BALTIMORE NATIONAL PIKE A SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644 WWW.BEI-CIVILENGINEERING.COM

OWNER/DEVELOPER:

CON SPAN®

Checked

4/12/07

APPROVED:

FOR CONSTRUCTION

BRIDGE SYSTEMS

MARYLAND

TAYLOR FARM SECTION THREE, PHASE TWO LOTS 57 THROUGH 101 AND NON-BUILDABLE BULK PARCEL 'A'

3rd ELECTION DISTRICT HOWARD COUNTY, MARYLAND 410-465-4244 CON/SPAN BRIDGE
TITLE SHEET & GENERAL NOTES

FRIENDLY FARMS LLC P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041

DATE: JULY, 2007 SCALE: AS SHOWN

DRAFT:

TAYLOR FARM

HOWARD COUNTY

© 2007, The design and information shown on this drawing is provided as a

service to the project owner, engineer and contractor by CONTECH Bridge Solutions Inc. No part of this drawing may be used, reproduced, or modified in any manner without the prior written authorization of CONTECH Bridge Solutions Inc. Any such use, reproduction, or modification of this drawing is done at the user's own risk and the user agrees to indemnify, defend and hold harmless CONTECH Bridge Solutions Inc. from and against any and all claims, liability, and expense, including, but not limited to, reasonable attorney's fees, arising from such use, reproduction or modification.

If discrepancies between the supplied information and actual field conditions are encountered as site work progresses, these discrepancies must be reported to CONTECH Bridge Solutions Inc., immediately for re-evaluation of the design. CONTECH Bridge Solutions Inc. accepts no liability for designs based on inaccurate information supplied by others.

BRIDGE SOLUTIONS INC.

8430 University Executive Park Drive Charlotte, North Carolina 28262

> 704-548-8420 704-548-8586 fax 800-526-3999

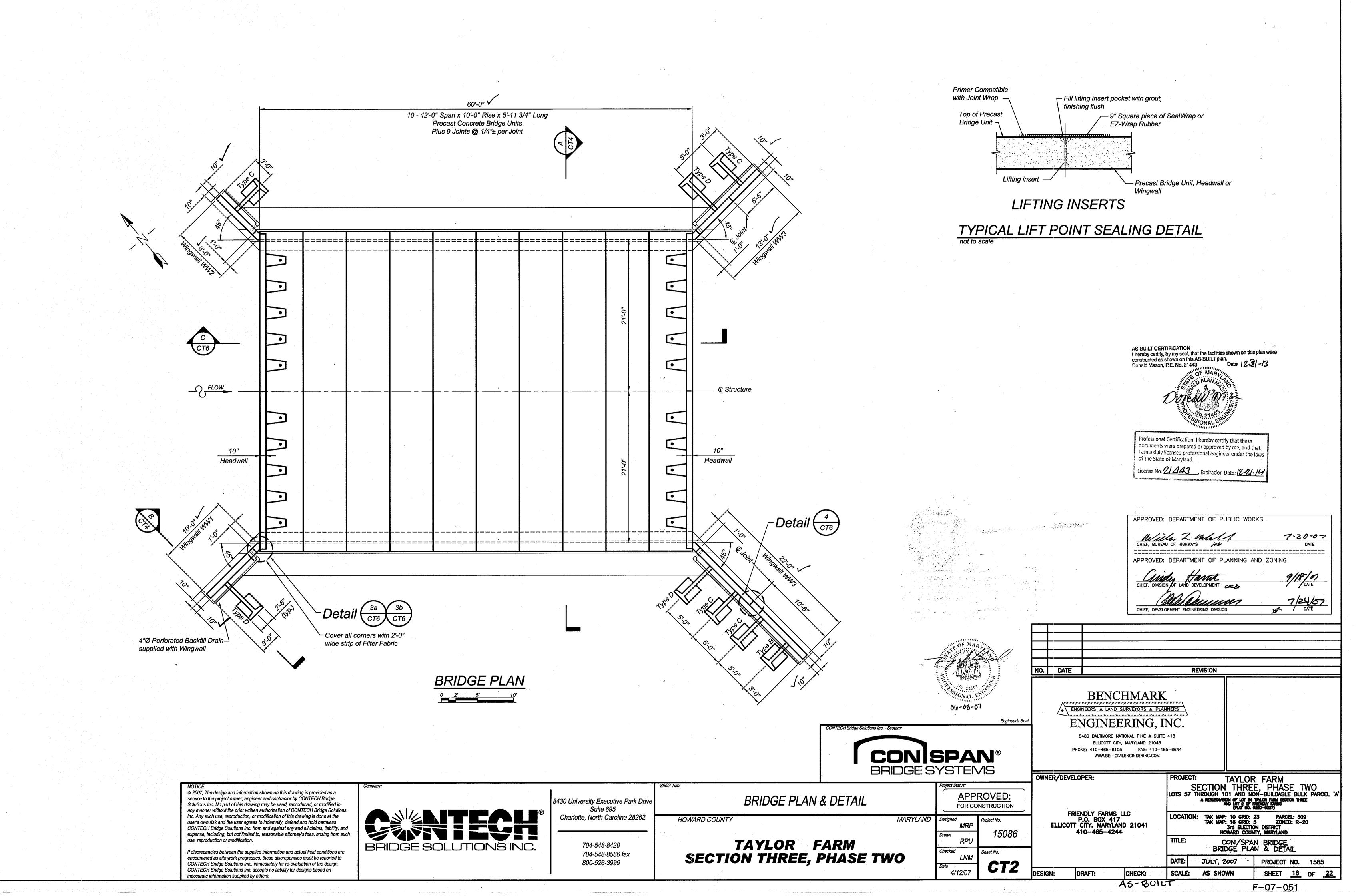
SECTION THREE, PHASE TWO

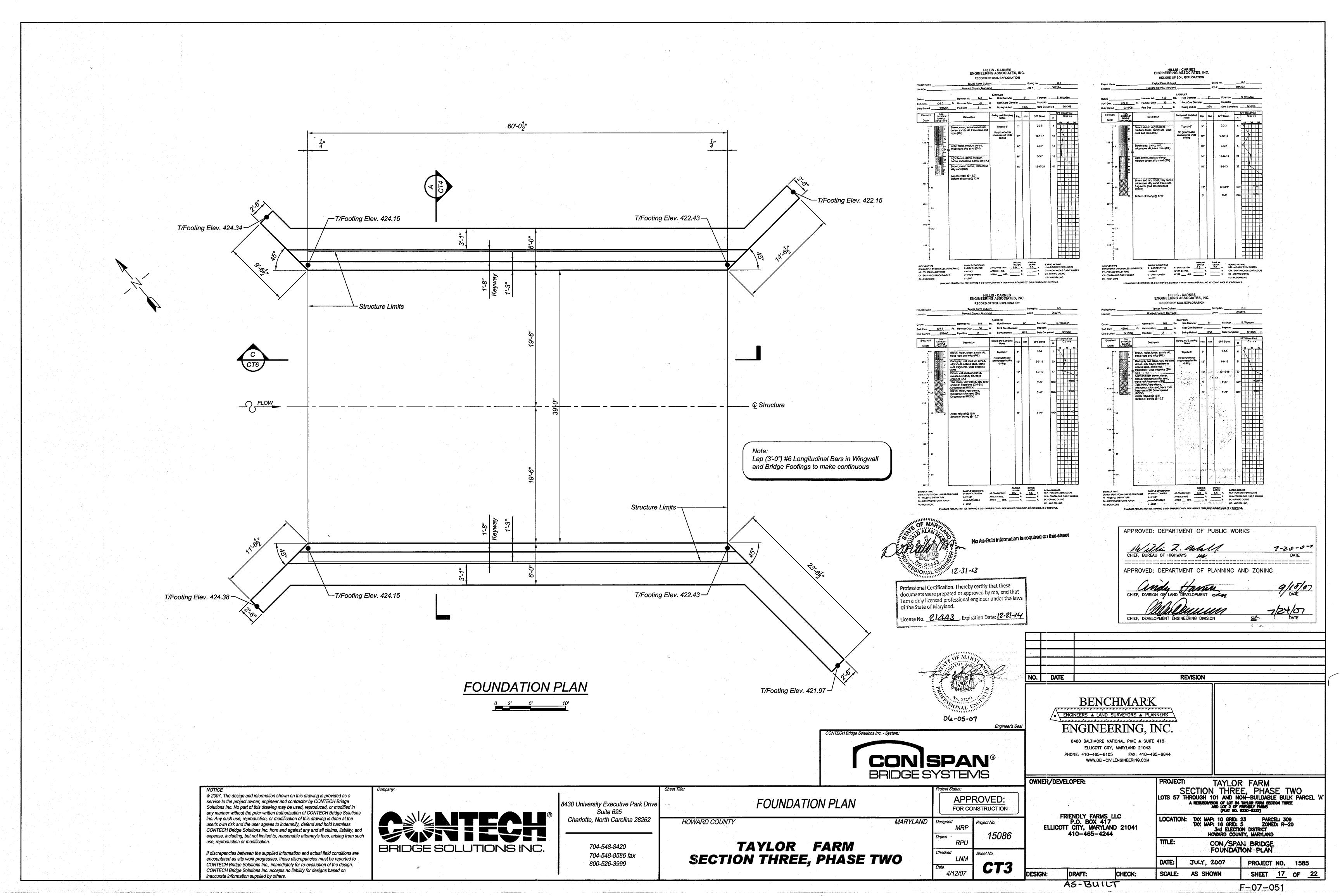
TITLE SHEET & GENERAL NOTES

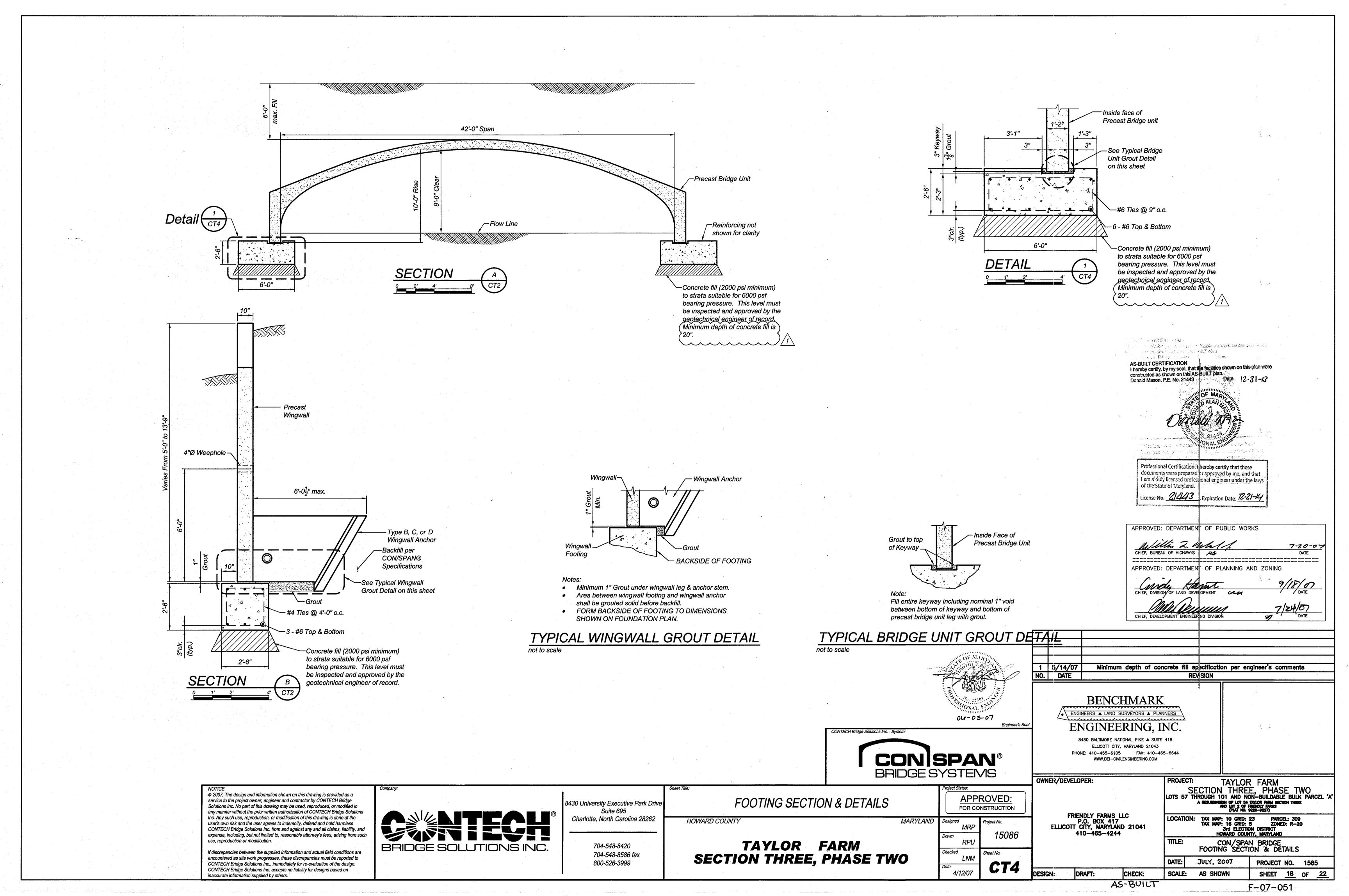
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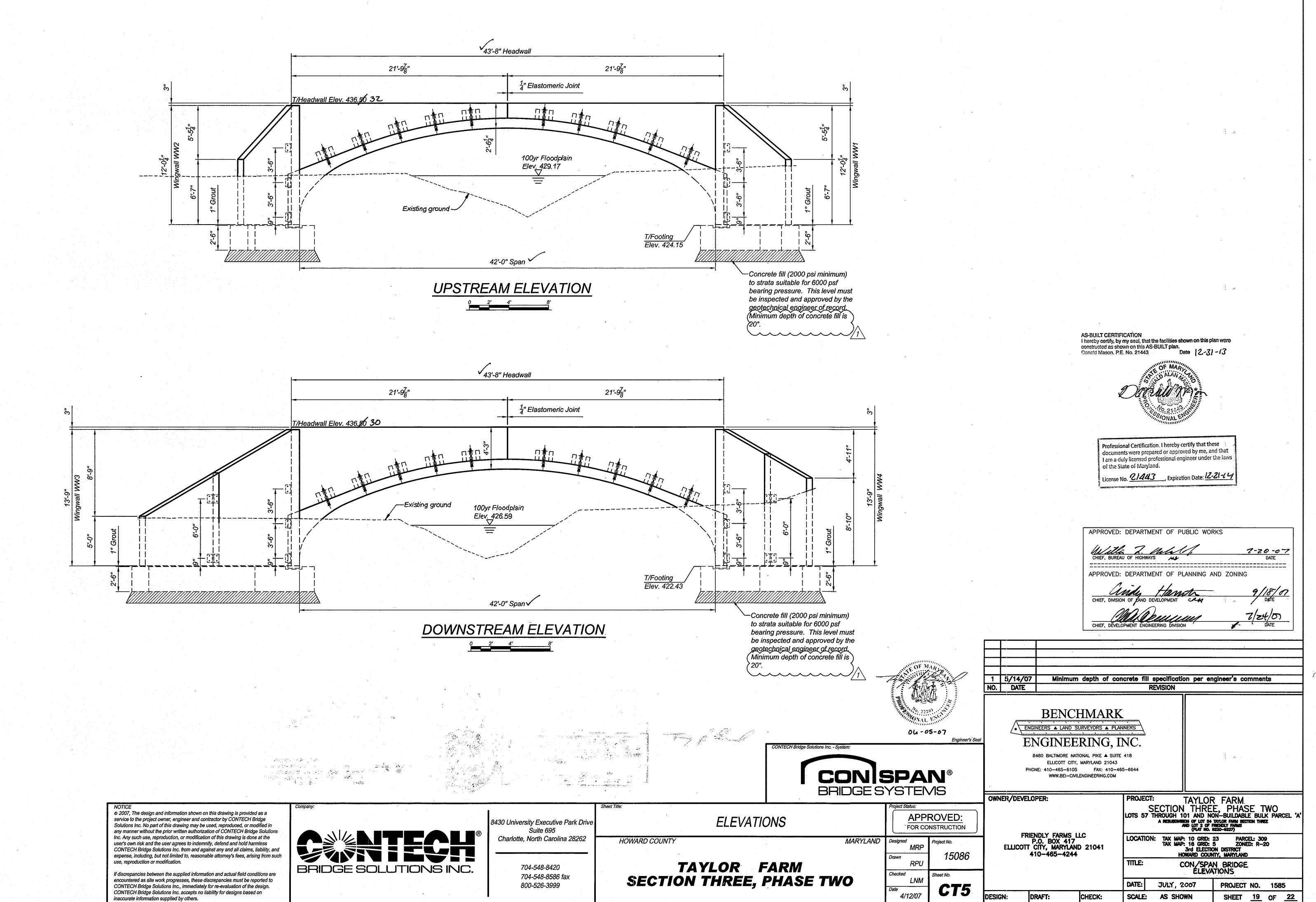
SHEET 15 OF 22 F-07-051

PROJECT NO. 1585



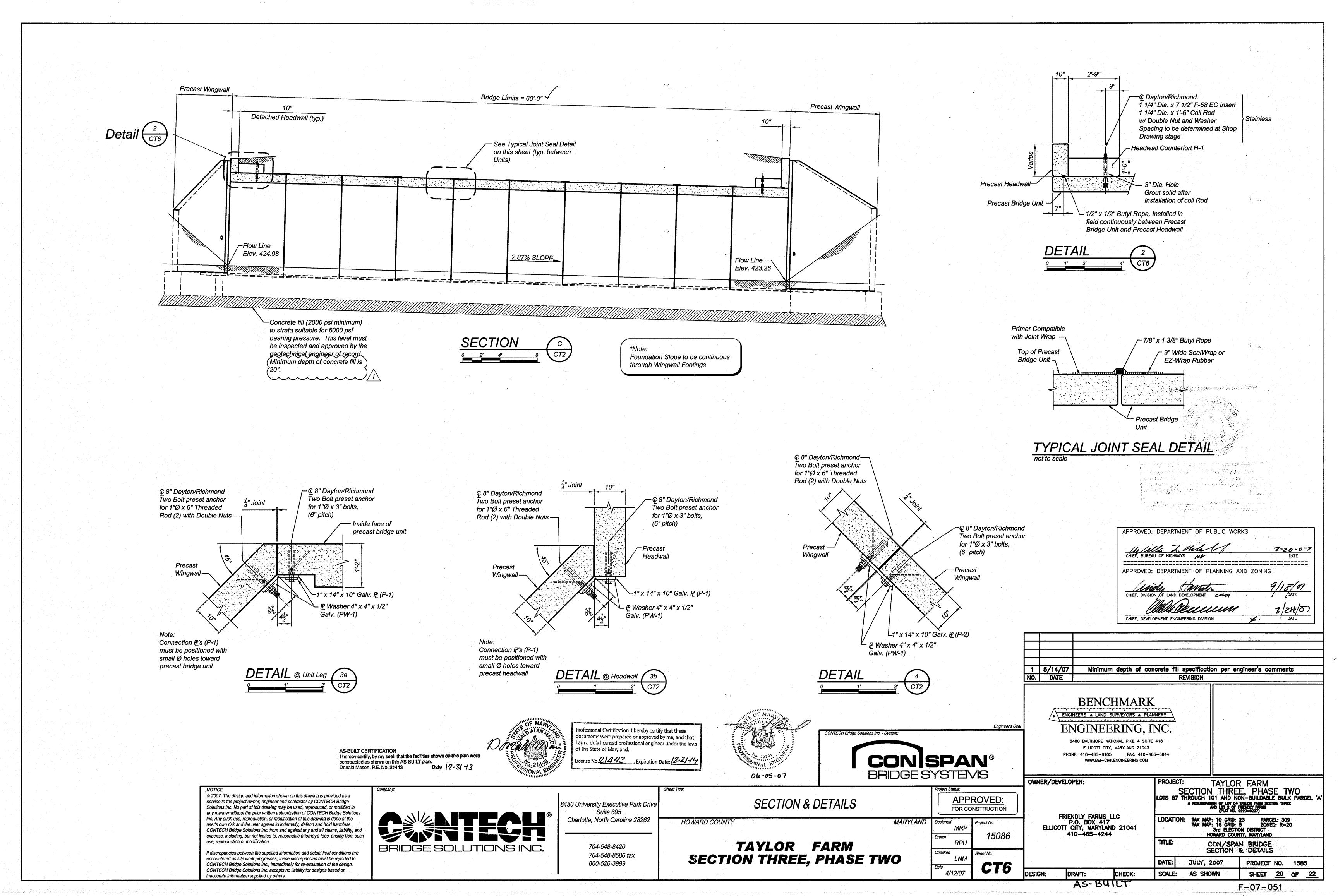






F-07-051

AS-BUILT



SPECIFICATIONS FOR MANUFACTURE AND INSTALLATION OF CON/SPAN® BRIDGE SYSTEMS

1. DESCRIPTION

This work shall consist of constructing a CON/SPAN ® bridge in accordance with these specifications and in reasonably close conformity with the lines, grades, design and dimensions shown on the plans or as established by the Engineer. In situations where two or more specifications apply to this work, the most stringent requirements shall govern.

2. TYPES

Precast reinforced concrete CON/SPAN® bridge units manufactured in accordance with this specification shall be designated by span and rise. Precast reinforced concrete CON/SPAN® wingwalls and headwalls manufactured in accordance with this specification shall be designated by length, height, and deflection angle.

3. MATERIALS - CONCRETE

The concrete for the culverts shall be air-entrained when installed in areas subject to freeze-thaw conditions, composed of portland cement, fine and coarse aggregates, admixtures and water. Air-entrained concrete shall contain 6 ± 2 percent air. The air entraining admixture shall conform to AASHTO M154.

- 3.1 Portland Cement Shall conform to the requirements of ASTM Specifications C150-Type I, Type II, or Type III cement.
- 3.2 Coarse Aggregate Shall consist of stone having a maximum size of 1 inch. Aggregate shall meet requirements for ASTM C33.
- Water Reducing Admixture The manufacturer may submit for approval by the Engineer, a water-reducing admixture for the purpose of increasing workability and reducing the water requirement for the concrete.
- 3.4 Calcium Chloride The addition to the mix of calcium chloride or admixtures containing calcium chloride will not be permitted.

4. MATERIALS - STEEL REINFORCEMENT AND **HARDWARE**

All reinforcing steel for the culverts shall be fabricated and placed in accordance with the detailed shop drawings submitted by the manufacturer.

- 4.1 Steel Reinforcement Reinforcement shall consist of welded wire fabric conforming to ASTM Specification A 185 or A 497, or deformed billet steel bars conforming to ASTM Specification A 615, Grade 60. Longitudinal distribution reinforcement may consists of welded wire fabric or deformed billet-steel bars.
- 4.2 Hardware:

Bolts and threaded rods for wingwall connections shall conform to ASTM A 307. Nuts shall conform to AASHTO M 292 (ASTM A 194) Grade 2H. All bolts, threaded rods and nuts used in wingwall connections shall be mechanically zinc coated in accordance with ASTM B 695 Class 50.

Structural Steel for wingwall connection plates and plate washers shall conform to AASHTO M 270 (ASTM A 709) Grade 36 and shall be hot dip galvanized as per AASHTO M 111 (ASTM A 123).

Inserts for wingwalls shall be 1" diameter Two-Bolt Preset Wingwall Anchors as manufactured by Dayton/Richmond Concrete Accessories, Miamisburg, Ohio, (800) 745-3700.

Ferrule Loop Inserts shall be F-64 Ferrule Loop Inserts as manufactured by Dayton/Richmond Concrete Accessories, Miamisburg, Ohio, (800) 745-3700.

Hook Bolts used in attached headwall connections shall be ASTM A 307.

Inserts for detached headwall connections shall be AISI Type 304 stainless steel, F-58 Expanded Coil inserts as manufactured by Dayton/Richmond Concrete Accessories, Miamisburg, Ohio, (800) 745-3700. Coil rods and nuts used in headwall connections shall be AISI Type 304 stainless steel. Washers used in headwall connections shall be either AISI Type 304 stainless steel plate washers or AASHTO M 270 (ASTM A 709) Grade 36 plate washers hot dip galvanized as per AASHTO M 111 (ASTM A 153).

Reinforcing bar splices shall be made using the Dowel Bar Splicer System as manufactured by Dayton/Richmond Concrete Accessories, Miamisburg, Ohio, (800) 745-3700, and shall consist of the Dowel Bar Splicer (DB-SAE) and Dowel-In (DI).

5. MANUFACTURE

- 5.1 Mixture The aggregates, cement and water shall be proportioned and mixed in a batch mixer to produce a homogeneous concrete meeting the strength requirements of this specification. The proportion of portland cement in the mixture shall not be less than 564 pounds (6 sacks) per cubic yard of concrete.
- 5.2 Curing The precast concrete culvert units shall be cured for a sufficient length of time so that the concrete will develop the specified compressive strength in 28 days or less. Any one of the following methods of curing or combinations thereof shall
 - 5.2.1 Steam Curing The culverts may be low pressure, steam cured by a system that will maintain a moist atmosphere.
 - 5.2.2 Water Curing The culverts may be water cured by any method that will
 - 5.2.3 Membrane Curing A sealing membrane conforming to the requirements of ASTM Specification C 309 may be applied and shall be left intact until the required concrete compressive strength is attained. The concrete temperature at the time of application shall be within ± 10 degrees F of the atmospheric temperature. All surfaces shall be kept moist prior to the application of the compounds and shall be damp when the compound is applied.
- 5.3 Forms the forms used in manufacture shall be sufficiently rigid and accurate to maintain the culvert dimensions within the permissible variations given in Section 7 of these specifications. All casting surfaces shall be of a smooth material.
- 5.4 Handling Handling devices or holes shall be permitted in each culvert for the purpose of handling and setting.
- 5.5 Storage The precast elements shall be stored in such a manner to prevent cracking or damage. The units shall not be moved until the concrete compressive strength has reached a minimum of 2500 psi, and they shall not be stored in an upright position until the concrete compressive strength is a minimum of 4,000 psi.

6. DESIGN

- 6.1 The precast element dimension and reinforcement details shall be as prescribed in the plan and the shop drawings provided by the manufacturer, subject to the provisions of Section 7, below. The minimum concrete compressive strength shall be as shown on the shop drawings. The minimum steel yield strength shall be 60,000 psi, unless otherwise noted on the shop drawings.
- 6.2 The precast elements are designed in accordance with the "Standard" Specifications for Highway Bridges" 17th Edition, adopted by the American Association of State Highway and Transportation Officials, 2002. A minimum of one foot of cover above the crown of the bridge units is required in the installed condition. (Unless noted otherwise on the shop drawings and designed
- 6.3 Placement of Reinforcement in Precast Bridge Units The cover of concrete over the outside circumferential reinforcement shall be 2 inches minimum. The cover of concrete over the inside circumferential reinforcement shall be 1 1/2 inches minimum, unless otherwise noted on the shop drawings. The clear distance of the end circumferential wires shall not be less than one inch nor more than two inches from the ends of each section. Reinforcement shall be assembled utilizing single or multiple layers of welded wire fabric (not to exceed 3 layers), supplemented with a single layer of deformed billet-steel bars, when necessary. Welded wire fabric shall be composed of circumferential and longitudinal wires meeting the spacing requirements of 6.6, below, and shall contain sufficient longitudinal wires extending through the bridge unit to maintain the shape and position of the reinforcement. Longitudinal distribution reinforcement may be welded wire fabric or deformed billet-steel bars and shall meet the spacing requirements of 6.6, below. The ends of the longitudinal distribution reinforcement shall be not more than 3 inches and not less than 1 1/2 inches from the ends of
- 6.4 Placement of Reinforcement for Precast Wingwalls and Headwalls The cover of concrete over the longitudinal and transverse reinforcement shall be 2 inches minimum. The clear distance from the end of each precast element to the end transverse reinforcing steel shall not be less than one inch nor more than two inches. Reinforcement shall be assembled utilizing a single layer of welded wire fabric, or a single layer of deformed billet-steel bars. Welded wire fabric shall be composed of transverse and longitudinal wires meeting the spacing requirements of 6.7, below, and shall contain sufficient longitudinal wires extending through the element to maintain the shape and position of the reinforcement. Longitudinal reinforcement may be welded wire fabric or deformed billet-steel bars and shall meet the spacing requirements of 6.7, below. The ends of the longitudinal reinforcement shall be not more than 3 inches and not less than 1 1/2 inches from the ends of the walls.
- 6.5 Bending of Reinforcement for Precast Bridge Units The outside and inside circumferential reinforcing steel for the corners of the bridge shall be bent to such an angle that is approximately equal to the configuration of the bridge's outside

6.6 Laps, Welds, and Spacing for Precast Bridge Units - Tension splices in the circumferential reinforcement shall be made by lapping. Laps may be tack welded together for assembly purposes. For smooth welded wire fabric, the overlap shall meet the requirements of AASHTO 8.30.2 and 8.32.6. For deformed welded wire fabric, the overlap shall meet the requirements of AASHTO 8.30.1 and 8.32.5. The overlap of welded wire fabric shall be measured between the outer most longitudinal wires of each fabric sheet. For deformed billet-steel bars, the overlap shall meet the requirements of AASHTO 8.25. For splices other than tension splices, the overlap shall be a minimum of 12" for welded wire fabric or deformed billet-steel bars. The spacing center to center of the circumferential wires in a wire fabric sheet

shall be not less than 2 inches nor more than 4 inches. The spacing center to center of the longitudinal wires shall not be more than 8 inches. The

spacing center to center of the longitudinal distribution steel for either line of

Laps, Welds, and Spacing for Precast Wingwalls and Headwalls - Splices in the reinforcement shall be made by lapping. Laps may be tack welded together for assembly purposes. For smooth welded wire fabric, the overlap shall meet the requirements of AASHTO 8.30.2 and 8.32.6. For deformed welded wire fabric, the overlap shall meet the requirements of AASHTO 8.30.1 and 8.32.5. For deformed billet-steel bars, the overlap shall meet the requirements of AASHTO 8.25. The spacing center-to-center of the wires in a wire fabric sheet shall be not less than 2 inches nor more than 8 inches.

reinforcing in the top slab shall be not more than 16 inches.

7. PERMISSIBLE VARIATIONS

- 7.1 Bridge Units
 - 7.1.1 Internal Dimensions The internal dimension shall vary not more than 1% from the design dimensions nor more than 1-1/2 inches whichever is less. The haunch dimensions shall vary not more than 3/4 inch from the design dimension.
 - 7.1.2 Slab and Wall Thickness The slab and wall thickness shall not be less than that shown in the design by more than 1/4 inch. A thickness more than that required in the design shall not be cause for rejection.
 - 7.1.3 Length of Opposite Surfaces Variations in laying lengths of two opposite surfaces of the bridge unit shall not be more than 1/2 inch in any section, except where beveled ends for laying of curves are specified by the purchaser.
 - 7.1.4 Length of Section The underrun in length of a section shall not be more than 1/2 inch in any bridge unit.
 - 7.1.5 Position of Reinforcement The maximum variation in position of the reinforcement shall be ± 1/2 inch. In no case shall the cover over the reinforcement be less than 1 1/2 inches for the outside circumferential steel or be less than 1 inch for the inside circumferential steel as measured to the external or internal surface of the bridge. These tolerances or cover requirements do not apply to mating surfaces of the joints.
 - 7.1.6 Area of Reinforcement The areas of steel reinforcement shall be the design steel areas as shown in the manufacturer's shop drawings. Steel areas greater than those required shall not be cause for rejection. The permissible variation in diameter of any reinforcement shall conform to the tolerances prescribed in the ASTM Specification for that type of reinforcement.

7.2 Wingwalls and Headwalls

8430 University Executive Park Drive

Suite 695

Charlotte, North Carolina 28262

704-548-8420

800-526-3999

704-548-8586 fax

- 7.2.1 Wall Thickness The wall thickness shall not vary from that shown in the design by more than 1/2 inch.
- 7.2.2 Length/ Height of Wall sections The length and height of the wall shall not vary from that shown in the design by more than
- 7.2.3 Position of Reinforcement The maximum variation in the position of the reinforcement shall be \pm 1/2 inch. In no case shall the cover over the reinforcement be less than 1 1/2
- 7.2.4 Size of Reinforcement The permissible variation in diameter of any reinforcing shall conform to the tolerances prescribed in the ASTM Specification for that type of reinforcing. Steel area greater than that required shall not be cause for rejection.

HOWARD COUNTY

8. TESTING AND INSPECTION

8.1 Type of Test Specimen - Concrete compressive strength shall be determined from compression tests made on cylinders or cores. For cylinder testing, a minimum of 4 cylinders shall be taken during each production run. For core testing, one core shall be cut from each of 3 precast elements selected at random from each production group. A production group shall be defined as 15 or fewer bridge units (of a particular size), wingwalls or headwalls in a continuous production run. For each continuous production run, each production group or fraction thereof shall be considered separately for the purpose of testing and acceptance. A production run shall be considered continuous if not interrupted for more than 3 consecutive days.

11. REPAIRS

12. INSPECTION

13. REJECTION

14. MARKING

to inspection by the purchaser.

Precast elements may be repaired, if necessary, because of imperfections in

manufacture or handling damage and will be acceptable if, in the opinion of the

purchaser, the repairs are sound, properly finished and cured, and the repaired

The quality of materials, the process of manufacture, and the finished culverts shall be subject

The precast elements shall be subject to rejection on account of any of the specification

does not exceed one half the thickness of the wall.

Section 5 of these specifications.

13.3 Honeycombed or open texture.

the inside of the vertical leg of the bridge section:

Name or trademark of the manufacturer

Bridge Span X Bridge Rise

Date of Manufacture

requirements. Individual precast elements may be rejected because of any of the following:

13.1 Fractures or cracks passing through the wall, except for a single end crack that

13.2 Defects that indicate proportioning, mixing, and molding not in compliance with

13.4 Damaged ends, where such damage would prevent making a satisfactory joint.

No As-Built information is required on this shoot

Professional Certification. I hereby certify that these

of the State of Maryland.

APPROVED: DEPARTMENT OF PUBLIC WORKS

documents were prepared or approved by me, and that

I am a duly licensed professional engineer under the laws

License No. 21443 Expiration Date: 12-21-14

APPROVED: DEPARTMENT OF PLANNING AND ZONING

7-2007

Each bridge unit shall be clearly marked by waterproof paint. The following shall be shown on

section conforms to the requirements of this specification.

- 8.2 Compression Testing Cylinders shall be made and tested as prescribed by the ASTM C 39 Specification. Cores shall be obtained and tested for compressive strength in accordance with the provisions of the ASTM C497 Specification.
- 8.3 Acceptability of Cylinder Tests When the average compressive strength of all cylinders tested is equal to or greater than the design compressive strength, and not more than 10% of the cylinders tested have a compressive strength less than the design concrete strength, and no cylinder tested has a compressive strength less than 80% of the design compressive strength, then the lot shall be accepted. When the compressive strength of the cylinders tested does not conform to this acceptance criteria, the acceptability of the lot may be determined as described in section 8.4. below.
- 8.4 Acceptability of Core Tests The compressive strength of the concrete in each production group as defined in 8.1 is acceptable when the average core test strength is equal to or greater than the design concrete strength. When the compressive strength of the core tested is less than the design concrete strength, the precast element from which that core was taken may be re-cored. When the compressive strength of the re-core is equal to or greater than the design concrete strength, the compressive strength of the concrete in that production group is acceptable.
 - 8.4.1 When the compressive strength of any recore is less than the design concrete strength, the precast element from which that core was taken shall be rejected. Two precast elements from the remainder of the group shall be selected at random and one core shall be taken from each. If the compressive strength of both cores is equal to or greater than the design concrete strength, the compressive strength of the remainder of that group is acceptable. If the compressive strength of either of the two cores tested is less than the design concrete strength, the remainder of the group shall be rejected or, at the option of the manufacturer, each precast element of the remainder of the group shall be cored and accepted individually, and any of these elements that have cores with less than the design concrete strength shall be rejected.
 - 8.4.2 Plugging Core Holes The core holes shall be plugged and sealed by the manufacturer in a manner such that the elements will meet all of the test requirements of this specification. Precast elements so sealed shall be considered satisfactory for use.
 - Test Equipment Every manufacturer furnishing culverts under this specification shall furnish all facilities and personnel necessary to carryout the test required.

9. JOINTS

The bridge units shall be produced with flat butt ends. The ends of the bridge units shall be such that when the sections are laid together they will make a continuous line of with a smooth interior free of appreciable irregularities, all compatible with the permissible variations in Section 7, above. The joint width shall not exceed 3/4 inches.

10. WORKMANSHIP AND FINISH

The bridge units, wingwalls, and headwalls shall be substantially free of fractures. The ends of the bridge units shall be normal to the walls and centerline of the bridge section, within the limits of the variations given in section 7, above, except where beveled ends are specified. The faces of the wingwalls and headwalls shall be parallel to each other, within the limits of variations given in section 7, above. The surface of the precast elements shall be a smooth steel form or troweled surface. Trapped air pockets causing surface defects shall be considered as part of a smooth, steel form finish.

Engineer's Seal

APPROVED:

FOR CONSTRUCTION

RPU

4/12/07

Checked

Project No.

Sheet No.

DESIGN:

CONTECH Bridge Solutions Inc. - System CONISPAN®

BRIDGE SYSTEMS

MARYLAND

DATE REVISION **BENCHMARK** ENGINEERS A LAND SURVEYORS A PLANNERS ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE A SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644

OWNER/DEVELOPER: TAYLOR FARM SECTION THREE, PHASE TWO LOTS 57 THROUGH 101 AND NON-BUILDABLE BULK PARCEL 'A FRIENDLY FARMS LLC P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041 TAX MAP: 10 GRID: 23 TAX MAP: 16 GRID: 5 3rd ELECTION DISTRICT HOWARD COUNTY, MARYLAND 410-465-4244 CON/SPAN BRIDGE SPECIFICATIONS

DATE: JULY, 2007 SCALE: AS SHOWN CHECK:

TAYLOR FARM

06-05-07

SECTION THREE, PHASE TWO

SPECIFICATIONS

DRAFT: AS-BUILT

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SHEET 21 OF 22 F-07-051

PROJECT NO.

1585

encountered as site work progresses, these discrepancies must be reported to CONTECH Bridge Solutions Inc., immediately for re-evaluation of the design. CONTECH Bridge Solutions Inc. accepts no liability for designs based on inaccurate information supplied by others.

If discrepancies between the supplied information and actual field conditions are

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BRIDGE SOLUTIONS INC.

SPECIFICATIONS FOR MANUFACTURE AND INSTALLATION OF CON/SPAN® BRIDGE SYSTEMS (CONT'D)

15. CONSTRUCTION REQUIREMENTS

- 15.1 Footings The bridge units and wingwalls shall be installed on either precast or cast-in-place concrete footings. The design size and elevation of the footings shall be as determined by the Engineer. A three inch deep keyway shall be formed in the top surface of the bridge footing three inches clear of the inside and outside faces of the bridge units, unless specified otherwise on the plans. No keyway is required in the wingwall footings, unless otherwise specified on the plans. The footings shall be given a smooth float finish and shall reach a compressive strength of 2,000 psi before placement of the bridge and wingwall elements. The completed footing surface shall be constructed in accordance with grades shown on the plans. When tested with a 10 foot straight edge, the surface shall not vary more than 1/4 inch in 10 feet. If a precast concrete footing is used, the contractor shall prepare a 4 inch thick base layer of compacted granular material the full width of the footing prior to placing the precast
- 15.2 Placement of the Bridge Units, Wingwalls and Headwalls The bridge units, wingwalls and headwalls shall be placed as shown on the Engineer's plan drawings. Special care shall be taken in setting the elements to the true line and grade. The bridge units and wingwalls shall be set on 6" x 6" masonite or steel shims. A minimum gap of 1/2 inch shall be provided between the footing and the bottom of the bridge's vertical legs or the wingwall. The gap shall be filled with cement grout (Portland cement and water or cement mortar composed of Portland cement, sand and water) with a minimum 28-day compressive strength of 3000 psi. If units have been set with temporary ties (cables, bars, etc.) grout must attain a minimum compressive strength of 1500 psi before ties may be removed.
- 15.3 External Protection of Joints The butt joint made by two adjoining bridge units shall be covered with a 7/8" x 1 3/8" preformed bituminous joint sealant and a minimum of a 9 inch wide joint wrap. The surface shall be free of dirt before applying the joint material. A primer compatible with the joint wrap to be used shall be applied for a minimum width of nine inches on each side of the joint. The external wrap shall be either EZ-WRAP RUBBER by PRESS-SEAL GASKET CORPORATION, SEAL WRAP by MAR MAC MANUFACTURING CO. INC. or approved equal. The joint shall be covered continuously from the bottom of one bridge section leg, across the top of the arch and to the opposite bridge section leg. Any laps that result in the joint wrap shall be a minimum of six inches long with the overlap running downhill.

In addition to the joints between bridge units, the joint between the end bridge unit and the headwall shall also be sealed as described above. If precast wingwalls are used, the joint between the end bridge unit and the wingwall shall be sealed with a 2'-0" strip of filter fabric. Also, if lift holes are formed in the arch units, they shall be primed and covered with a 9" x 9" square of joint wrap.

During the backfilling operation, care shall be taken to keep the joint wrap in its proper

15.4 Backfill - Backfill shall be considered as all replaced excavation and new embankment adjacent to the CON/SPAN® bridge units, wingwalls, and headwalls. The project construction and material specifications which include the specifications for excavation for structures and roadway excavation and embankment construction, shall apply except as modified in this section.

No backfill shall be placed against any structural elements until they have been approved by

Backfill against a waterproofed surface shall be placed carefully to avoid damage to the waterproofing material.

Mechanical tampers or approved compacting equipment shall be used to compact all backfill and embankment immediately adjacent to each side and over the top of each bridge unit until it is covered to a minimum depth of one foot, unless the design fill height is less than 1'-0". The backfill within the Critical Backfill Zone (shown in the diagrams below) shall be placed in lifts of eight inches or less (loose depth). Heavy compaction equipment shall not be operated in this area or over the bridge until it is covered to a depth of one foot, unless the design fill height is less than 1'-0".

Lightweight dozers and graders may be operated over bridge units having one foot of compacted cover, but heavy earth moving equipment (larger than a D-4 Dozer weighing in excess of 12 tons and having track pressures of eight psi or greater) shall require two feet of cover unless the design cover is less than two feet. In no case shall equipment operating in excess of the design load (HS20 or HS25) be permitted over the bridge units unless approved by CON/SPAN®.

Any additional fill and subsequent excavation required to provide this minimum cover shall be made at no additional cost to the project.

As a precaution against introducing unbalanced stresses in the bridge, when placing backfill at no time shall the difference between the heights of fill on opposite sides of the bridge exceed

Backfill in front of wingwalls shall be carried to ground lines shown in the plans.

For fill heights over 12 feet, no backfilling may begin until a backfill compaction testing plan has been coordinated with and approved by CON/SPAN® Bridge Systems. Cost of the backfill compaction testing shall be included in the cost of the precast units. This included cost applies only to projects with fill heights over 12 feet (as measured from top crown of arch to

16. QUALITY ASSURANCE

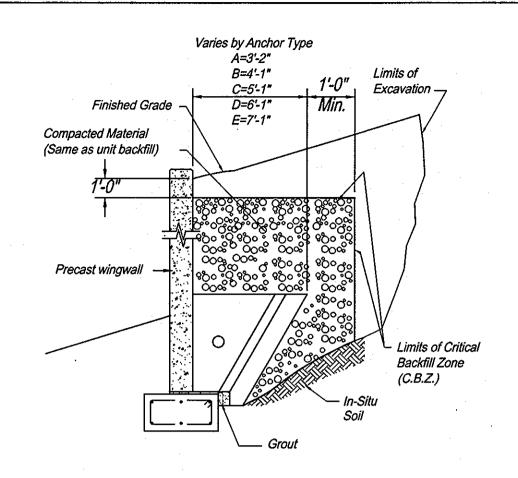
The Precaster shall demonstrate adherence to the standards set forth in the NPCA Quality Control Manual. The Precaster shall meet either Section 16.1 or 16.2.

- 16.1 Certification: The Precaster shall be certified by the Precast/Prestressed Concrete Institute Plant Certification Program or the National Precast Concrete Association's Plant Certification Program prior to and during production of the products covered by this specification.
- 16.2 Qualifications, Testing and Inspection
 - 16.2.1 The Precaster shall have been in the business of producing precast concrete products similar to those specified for a minimum of three years. He shall maintain a permanent quality control department or retain an independent testing agency on a continuing basis. The agency shall issue a report, certified by a licensed engineer, detailing the ability of the Precaster to produce quality products consistent with industry standards.
 - 16.2.2 The Precaster shall show that the following tests are performed in accordance with the ASTM standards indicated. Tests shall be performed for each 150 cubic yards of concrete placed, but not less frequently than once per production run, as defined in §8 of these specifications.

Air Content: C231 or C173

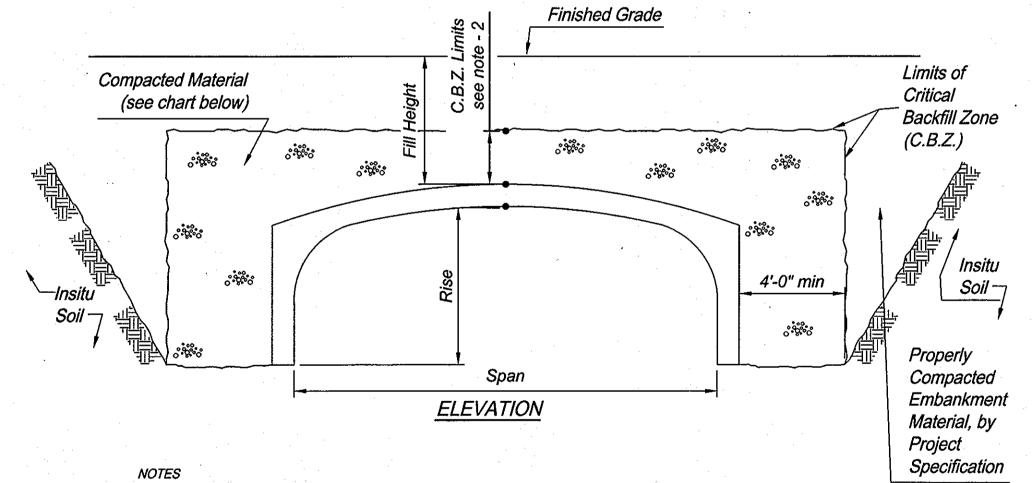
Compressive Strength: C39, C497

- 16.2.3 The Precaster shall provide documentation demonstrating compliance with this section to CON/SPAN® Bridge Systems at regular intervals or upon request.
- 16.2.4 The Owner may place an inspector in the plant when the products covered by this specification are being manufactured.



WINGWALL BACKFILL REQUIREMENTS

	BAC	KFILL DESC	CRIPTION (AAS	SHTO M 145	5- <i>91)</i>			•
Group Classification	A-1		A-3	A-2			A-4	
Group Classification	A-1-a	· A-1-b	- v	A-2-4	A-2-5	A-2-6	A-2-7	
Sieve Analysis, Percent Passing (100%	Passing 3" Si	eve)						•
No. 10	50 max.	•						
No. 40	30 max.	50 max.	51 min.					
No. 200	15 max.	25 max.	10 max.	35 max.	35 max.	35 max.	35 max.	36 min.
Characteristics of Fraction Passing								
No. 40								
Liquid Limit				40 max.	41 min.	40 max.	41 min.	40 max.
Plasticity Index	6 max.		N.P.	10 max.	10 max.	11 min.	11 min.	10 max.
Usual Types of Significant	Stone Fragments,		Fine	Silty or Clayey Gravel and Sand			Silty Soils	
Constituent Materials	Gravel &	•	Sand					
General Rating as Subgrade			Excellent to C	Good				Fair to Poor



1. SEE CON/SPAN® SPECIFICATIONS SECTION 15.4 FOR BACKFILL SPECIFICATIONS. 2. FOR FILL HEIGHTS GREATER THAN 2'-0", C.B.Z. LIMIT SHALL BE 2'-0" ABOVE ARCH CROWN. FOR FILL HEIGHTS

LESS THAN 2'-0", THE FINISHED GRADE SHALL BE THE BOUNDARY LINE FOR THE C.B.Z. 3. BACKFILLING OPERATIONS WITHIN THE C.B.Z. SHALL BE PERFORMED IN LIFTS OF 8" OR LESS (LOOSE DEPTH) 4. MAXIMUM DRY DENSITY SHALL BE DETERMINED BY AASHTO T-99 OR OTHER APPROVED METHODS. 5. BACKFILL SHALL BE COMPACTED IN LAYERS UNTIL THE DENSITY IS NOT LESS THAN 95% OF THE MAXIMUM

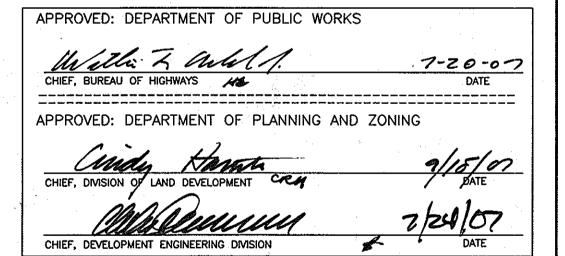
> ACCEPTABLE MATERIAL ACCEPTABLE MATERIAL FILL HEIGHT INSIDE C.B.Z. OUTSIDE C.B.Z. ≤24'-0" ≥ 12'-0" A1, A3 *≤24'-0"* < 12'-0" A1, A2, A3, A4 > 24'-0" ALL A1, A3 ** EMBANKMENT MATERIAL PER PROJECT SPECIFICATIONS

> > BACKFILL REQUIREMENTS

No As-Bullt information is required on this sheet

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

License No. 21403 Expiration Date: 12-21-14





APPROVED:

RPU

4/12/07

Checked

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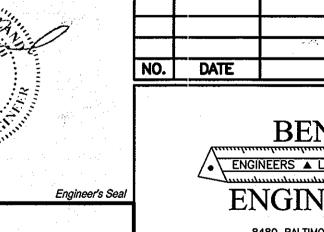
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BRIDGE SYSTEMS

MARYLAND



BENCHMARK ENGINEERS A LAND SURVEYORS A PLANNERS ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE ▲ SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644 WWW.BEI-CIVILENGINEERING.COM

OWNER/DEVELOPER:

DESIGN:

TAYLOR FARM SECTION THREE, PHASE TWO LOTS 57 THROUGH 101 AND NON-BUILDABLE BULK PARCEL "A" A RESUBONISION OF LOT 54 TAYLOR FARM SECTION THREE AND LOT 2 OF FRIENDLY FARMS (PLAT NO. 9220-9227) TAX MAP: 10 GRID: 23 TAX MAP: 16 GRID: 5

REVISION

410-465-4244

DRAFT:

FRIENDLY FARMS LLC P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041

3rd ELECTION DISTRICT HOWARD COUNTY, MARYLAND CON/SPAN BRIDGE SPECIFICATIONS JULY 2007 PROJECT NO. 1585

TAYLOR FARM SECTION THREE, PHASE TWO

REVISED 9/3/03 SPEC4.DWG

REVISED 1/15/02 SPEC4.DWG

REVISED 6/12/01 SPEC3.DWG

REVISED 7/21/00 SPEC3.DWG

REVISED 7/14/97 SPEC1.DWG

REVISED 4/4/96 SPEC1.DWG

REVISED 3/5/96 SPEC1.DWG REVISED 11/14/94 SPEC1.DWG

REVISED 5/25/94 SPEC1.DWG REVISED 9/10/93 SPEC1.DWG

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HOWARD COUNTY

SPECIFICATIONS

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DATE:

SHEET <u>22</u> OF <u>22</u> F-07-051

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