SITE DATA

LOCATION: CORONEOS PROPERTY

2nd ELECTION DISTRICT EXISTING ZONING: R-SC

GROSS AREA OF PROJECT: 4.01 AC. AREA OF 100-YEAR FLOODPLAIN & UTILITY EASEMENT: 0.30 AC.

AREA OF STEEP SLOPES: 0.27 AC.

NET AREA OF PROJECT: 3.44 AC.

AREA OF CREDITED OPEN SPACE REQUIRED: 25% x 4.01 AC. = 1.00 AC.
AREA OF CREDITED OPEN SPACE PROVIDED: 1.78 AC. (40%)

AREA OF NON-CREDITED OPEN SPACE PROVIDED: 0.24 AC. (PIPESTEM AREA OF O.S. LOTS 10 AND 11)

NUMBER OF PROPOSED OPEN SPACE LOTS: 2 AREA OF PROPOSED RIGHT-OF-WAY: 0.13 AC.

AREA OF PROPOSED BUILDABLE LOTS: 1.59 AC NUMBER OF LOTS/PARCELS ALLOWED (4 PER NET ACRE): 13 BUILDABLE LOTS NUMBER OF LOTS/PARCELS PROPOSED: 9 BUILDABLE LOTS

TOTAL APPROXIMATE LIMIT OF DISTURBANCE: 1.83 AC.

GENERAL NOTES

. ALL ASPECTS OF THE PROJECT ARE IN CONFORMANCE WITH THE LATEST HOWARD COUNTY STANDARDS UNLESS

WAIVERS HAVE BEEN APPROVED. 2. DEED REFERENCE:

3. DENSITY TABULATION: GROSS AREA OF PROJECT: 4.01 AC.

AREA OF 100-YR FLOODPLAIN DRAINAGE & UTILITY EASEMENT: 0.27 AC. AREA OF STEEP SLOPES OUTSIDE OF 100-YR FLOODPLAIN: 0.27 AC.

DWELLING UNITS PER NET ACRE ALLOWED: 4 UNITS PER NETACRE (4 X 3.47 AC. = 13 BUILDABLE LOTS)

DATED JANUARY 2004. 5. THE TOPOGRAPHY SHOWN HEREON IS BASED ON FIELD RUN TOPOGRAPHY PREPARED BY ROBERT H. VOGEL

4. THE PROJECT BOUNDARY IS BASED ON A BOUNDARY SURVEY PREPARED BY FREDERICK WARD AND ASSOCIATES

S. THIS SUBDIVISION IS SUBJECT TO SECTION 18.122B OF THE HOWARD COUNTY. PUBLIC WATER AND/OR SEWER SERVICE HAS BEEN GRANTED UNDER THE TERMS AND PROVISIONS, THEREOF, EFFECTIVE

THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS/BUREAU OF ENGINEERING/ CONSTRUCTION INSPECTION DIVISION AT (410) 313-1880 AT LEAST FIVE (5) WORKING DAYS PRIOR

ANY DAMAGE TO PUBLIC RIGHTS-OF-WAY, PAVING, OR EXISTING UTILITIES WILL BE CORRECTED AT

THIS SITE WILL UTILIZE GRASS CHANNEL. RAINGARDENS AND NATURAL AREA CONSERVATION CREDITS TO PROVIDE THE REQUIRED WQV AND ReV FOR STORMWATER MANAGEMENT. THE CPV IS NOT REQUIRED FOR THIS SITE SINCE THE 1-YR STORM EVENT IS LESS THAN 2 CFS. STORMWATER MANAGEMENT IS REQUIRED AND PROVIDED FOR THE 1 YEAR, 10 YEAR AND 100 YEAR STORM EVENT. THE FACILITY IS A DETENTION FACILITY AND IS TO BE

11. THE 100—YR FLOODPLAIN DRAINAGE AND UTILITY EASEMENT SHOWN ON—SITE IS BASED ON THE TIBER/HUDSON FLOODPLAIN STUDY, HOWARD COUNTY CAPITAL PROJECT NO. C-4-0119.

12. FOREST STAND DELINEATION PLAN APPROVED UNDER \$-00-09

PAYMENT OF \$10,454.40 TO THE HOWARD COUNTY FOREST CONSERVATION FUND FOR THE REMAINING 0.48 AC. OF REQUIRED REFORESTATION. FOREST CONSERVATION SURETY IN THE AMOUNT OF \$8.102.16

AND WAS APPROVED ON JULY 6, 2000.

7. STREET LIGHT PPLACEMENT AND THE TYPE OF FIXTURE AND POLE SHALL BE IN ACCORDANCE WITH THE HOWARD COUNTY DESIGN MANUAL, VOLUME III (1993) AND AS MODIFIED BY "GUIDELINES FOR STREET LIGHTS IN RESIDENTIAL DEVELOPMENTS (JUNE 1993). A MINIMUM SPACING OF 20' SHALL BE MAINTAINED BETWEEN ANY STREETLIGHT

18. SEDIMENT AND EROSION CONTROL IS PROVIDED FOR THIS SITE.

20. TO THE BEST OF THE OWNERS KNOWLEDGE, THERE ARE NO BURIAL/CEMETARY LOCATIONS ON-SITE.

22. WP-00-110: A WAIVER PETITION HAS BEEN SUBMITTED AND APPROVĘD, DATED JULY 6, 2000, TO WAIVE SECTION 16.121(e)(1) TO PERMIT THE REQUIRED 40 FEET OF OPEN SPACE LOT FRONTAGE ON A PUBLIC ROAD APPROVAL IS SUBJECT TO THE FOLLOWING CONDITIONS:

1. PEDESTRIAN AND MAINTENANCE VEHICLE ACCESS TO OPEN SPACE LOTS 10 & 11 SHALL BE PROVIDED BY A 24' ACCESS EASEMENT WITH A 14' ROADWAY AND THE 20' SWMF AND OPEN SPACE ACCESS EASEMENT ON 2. COMPLIANCE WITH THE ENCLOSED DFRS COMMENTS OF 6/19/00.

23. NO CLEARING, GRADING, OR CONSTRUCTION IS PERMITTED WITHIN THE 100-YR FLOODPLAIN, STREAMS, OR BUFFERS AND THE FOREST CONSERVATION EASEMENTS.

24. REFUSE COLLECTION, SNOW REMOVAL, AND MAINTENANCE FOR LOTS 1-9 SHALL BE PROVIDED AT THE JUNCTION OF THE PROPOSED PRIVATE ACCESS PLACE AND SAINT JOHN'S LANE.

25. TREE PROTECTION FENCING WILL BE PROVIDED AT THE LIMITS OF DISTURBANCE WHERE GRADING AND/OR CLEARING IS ADJACENT TO ENVIRONMENTAL AREAS AND THE FOREST CONSERVATION EASEMENT RETENTION AREA. 26. THERE ARE NO EXISTING STRUCTURES ON-SITE.

27. THIS SUBDIVISION IS SUBJECT TO THE 4TH EDITION OF THE SUBDIVISION AND LAND DEVELOPMENT REGULATIONS.

28. OPEN SPACE LOT 10 TO BE OWNED AND MAINTAINED BY THE HOMEOWNERS ASSOCIATIONS AND INCLUDES A SWMF. OPEN SPACE LOT 11 TO BE OWNED AND MAINTAINED BY HOWARD COUNTY DEPARTMENT OF RECREATION AND PARKS.

29. IN ACCORDANCE WITH THE HSCD COMMENTS OF 7/27/05 AND 9/19/05, HOWARD SCD WILL REVIEW OUTFALL E-1 UPON SUBMISSION OF THE FINAL PLANS TO VERIFY THAT ITS CURRENT INFRINGEMENT INTO THE ENVIRONMENTAL AREA IS NECESSARY. IF NOT, THE HOWARD SOD WILL REQUIRE ITS REDESIGN OUT OF THE ENVIRONMENTAL AREA.

30. FINANCIAL LANDSCAPE SURETY IN THE AMOUNT OF \$ 18,566.56 SHALL BE POSTED AS A PART OF THE DEVELOPER'S AGREEMENT FOR THE FOREST CONSERVATION OBLIGATION.

31. THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF THE PUBLIC WORKS/BUREAU OF ENGINEERING/CONSTRUCTION

INSPECTION DIVISION AT (410) 313-1880 AT LEAST (5) WORKING DAYS PRIOR TO THE START OF WORK. 32. THE CONTRACTOR SHALL NOTIFY "MISS UTILITY" AT 1-800-257-7777 AT LEAST 48 HOURS PRIOR TO ANY EXCAVATION WORK BEING DONE.

33. TRAFFIC CONTROL DEVICES, MARKINGS AND SIGNING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE

MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). ALL STREET AND REGULATORY SIGNS SHALL BE IN PLACE PRIOR TO THE PLACEMENT OF ANY ASPHALT.

34. ALL SIGN POSTS USED FOR TRAFFIC CONTROL SIGNS INSTALLED IN THE COUNTY RIGHT-OF-WAY SHALL BE MOUNTED ON A 2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE POST (14 GAUGE) INSERTED INTO A 2-1/2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE SLEEVE (12 GAUGE) - 3' LONG. A GALVANIZED STEEL POLE CAP SHALL BE MOUNTED ON

35. THE COORDINATES SHOWN HEREON ARE BASED UPON THE HOWARD COUNTY GEODETIC CONTROL, WHICH IS BASED UPON THE MARYLAND STATE PLANE COORDINATE SYSTEM. HOWARD COUNTY MONUMENT NO.S 28A1 AND 28AC WERE USED FOR THIS PROJECT.

36. EXISTING UTILITIES ARE BASED ON HOWARD COUNTY AS-BUILT CONSTRUCTION DRAWINGS.

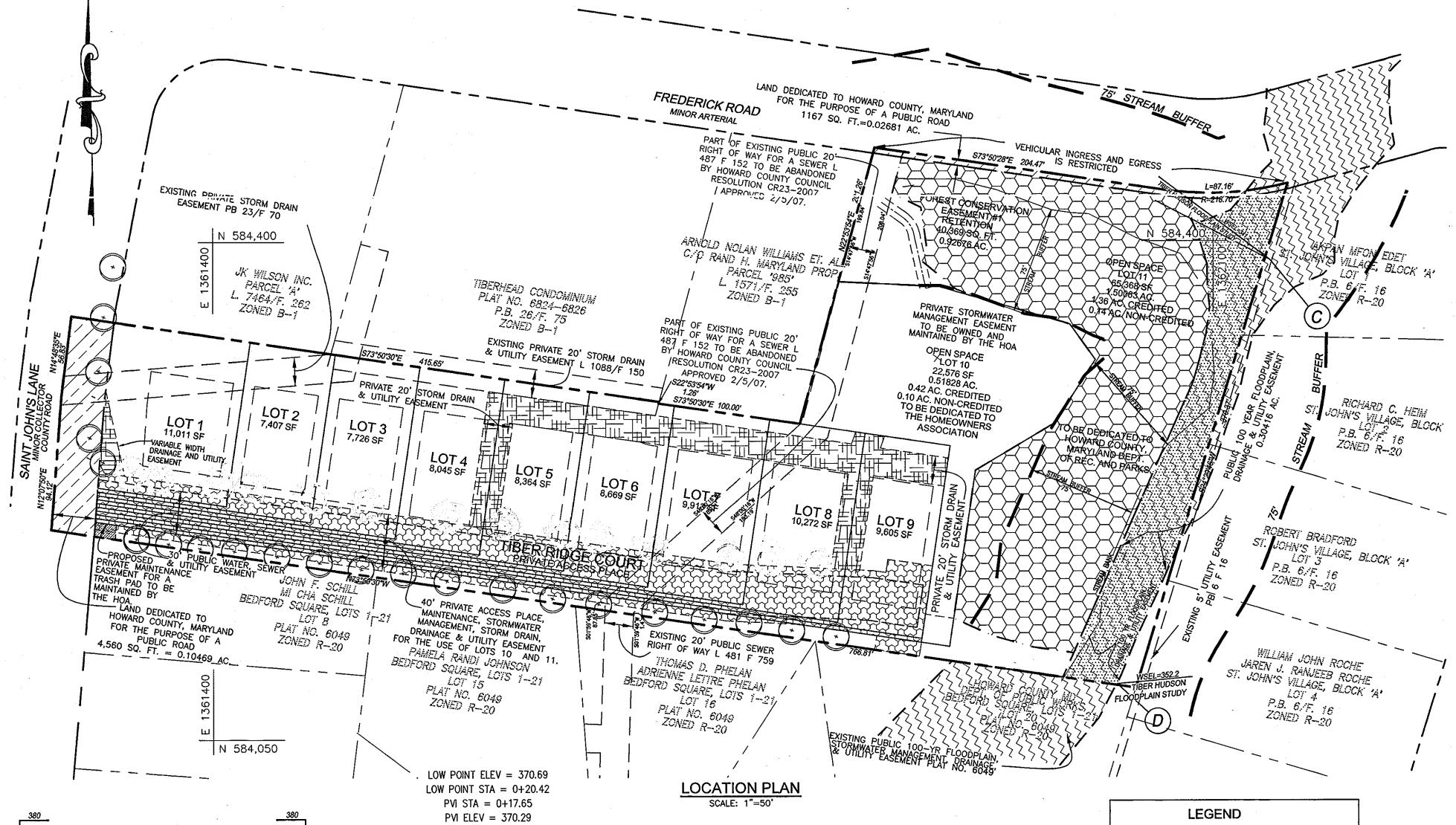
37. STREET TREES ARE REQUIRED FOR THIS PROJECT IN ACCORDANCE WITH SECTION 16.124(e)(1) OF THE SUBDIVISION REGULATIONS AND THE LANDSCAPE MANUAL. A FINANCIAL SURETY FOR FOUR PUBLIC ROAD STREET TREES IN THE AMOUNT OF \$1,200.00 WILL BE PAID AS A PART OF THE PUBLIC ROAD CONSTRUCTION DEVELOPER'S AGREEMENT AND FINANCIAL SURETY FOR 28 STREET TREES ALONG THE PRIVATE ACCESS PLACE, IN THE AMOUNT OF \$8,400.00 WILL BE PAID AS A PART OF THE LANDSCAPE SURETY, PART OF THE DEVELOPER'S AGREEMENT.

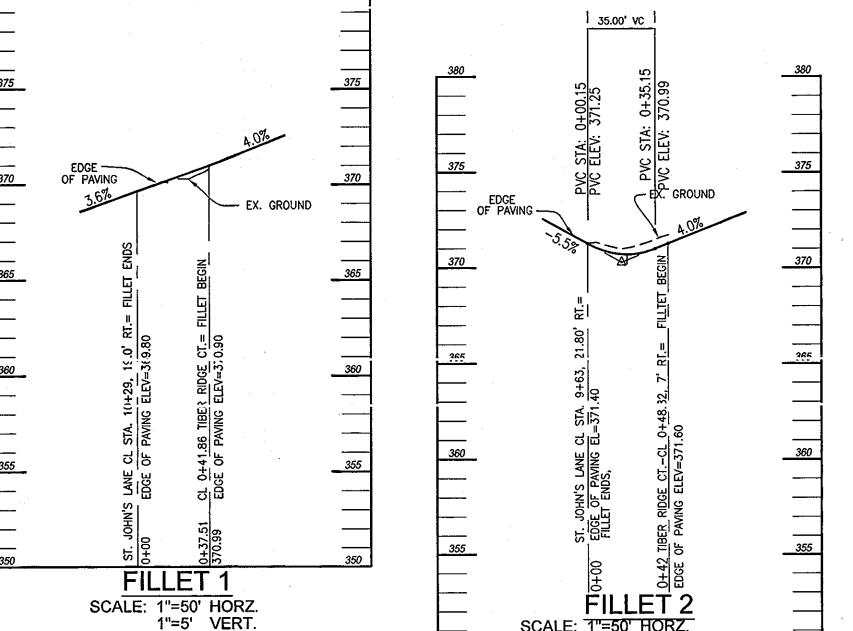
38. EXISTING PUBLIC 20' RIGHT OF WAY FOR A SEWER PREVIOUSLY RECORDED AS L. 487 F.152 ABANDONED BY HOWARD COUNTY COUNCIL RESOLUTION CR3-2007, APPROVED 02/05/2007

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS 5-14-07 CHIEF, DIVISION OF LAND DEVELOPMENT

FINAL ROAD CONSTRUCTION PLAN TIBER RIDGE

LOTS 1 - 9 AND OPEN SPACE LOTS 10 AND 11





AREA	REQUIREMENT	VOLUME REQUIREMENT	CREDITS	VOLUME REQUIREMENT	NOTES
1 1	WATER QUALITY VOLUME (WQv)	3,062 CF	3,062 CF	WITH CREDITS 583 CF	RAINGARDENS, GRASS CHANNEL AND NATURAL AREA CONSERVATION CREDIT
2	RECHARGE VOLUME (REV)	823 CF 0.22 AC	0.22 AC	0.00 AC	RAINGARDENS, GRASS CHANNEL AND NATURAL AREA CONSERVATION CREDIT
3	CHANNEL PROTECTION VOLUME (CPV)	NA	NA	NA	1 YR FLOW < 2.0 CFS 1 YR DETENTION PROVIDED BY POND
4	OVERHEAD FLOOD FROTECTION (Q10P)			<u></u> -	PROVIDED BY DETENTION POND
5	EXTREME FLOOD VOLUME (Q ₁₀₀ P)				PROVIDED BY DETENTION POND

OWNER / DEVELOPER WILLIAMSBURG GROUP 5485 HARPERS FARM ROAD, SUITE 200

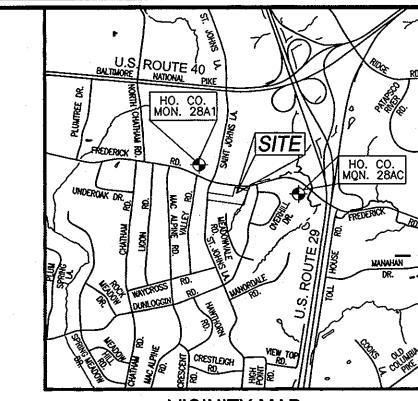
COLUMBIA, MARYLAND 21044

(410) 997-8800

PROP. PUBLIC WATER, SEWER, AND UTILITY EASEMENT

AREA DEDICATED TO PUBLIC R/W

FOREST CONSERVATION EASEMENT (RETENTION)



VICINITY MAP

BENCHMARKS							
NO.	NORTHING	EASTING					
28A1	576723.51	1314261.60					
28AC 578041.90 1314261.60							

MINIMUM LOT SIZE CHART							
LOT NO.	GROSS AREA	PIPESTEM AREA	MINIMUM LOT SIZE				
2	7,407 SF	242 SF	7,165 SF				
3	7,726 SF	401 SF	7,325 SF				
- 4	8,045 SF	560 SF	7,485 SF				
5	8,364 SF	720 SF	7,644 SF				
6	8,669 SF	879 SF	7,790 SF				
7	9,913 SF	1,038 SF	8,875 SF				
8	10,272 SF	1,217 SF	9,055 SF				
9	9,605 SF	1,397 SF	8,208 SF				
10	22,576 SF	1,539 SF	21,037 SF				
11	65,367 SF	6,038 SF	59,329 SF				

SHEET INDEX	
DESCRIPTION	SHEET NO.
COVER SHEET	1
SITE LAYOUT PLAN AND ROAD PROFILE	2
GRADING, SEDIMENT AND EROSION CONTROL PLAN	3
LANDSCAPING AND FOREST CONSERVATION PLAN	4
STORMDRAIN DRAINAGE AREA MAP	5
STORMDRAIN PROFILES	6
STORMWATER MANAGEMENT DETAILS	7
STORMWATER MANAGEMENT NOTES	8
STORMWATER MANAGEMENT NOTES	9
SEDIMENT AND EROSION CONTROL DETAILS	10

ELIMINATE STREET TREES ALONG N. SIDE OF TIBER RIGGE CT. 12.21.08

COVER SHEET TIBER RIDGE

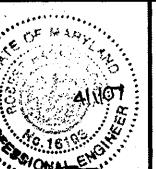
LOTS 1 - 9 AND OPEN SPACE LOTS 10 AND 11

TAX MAP 24 BLOCK 11 2ND ELECTION DISTRICT

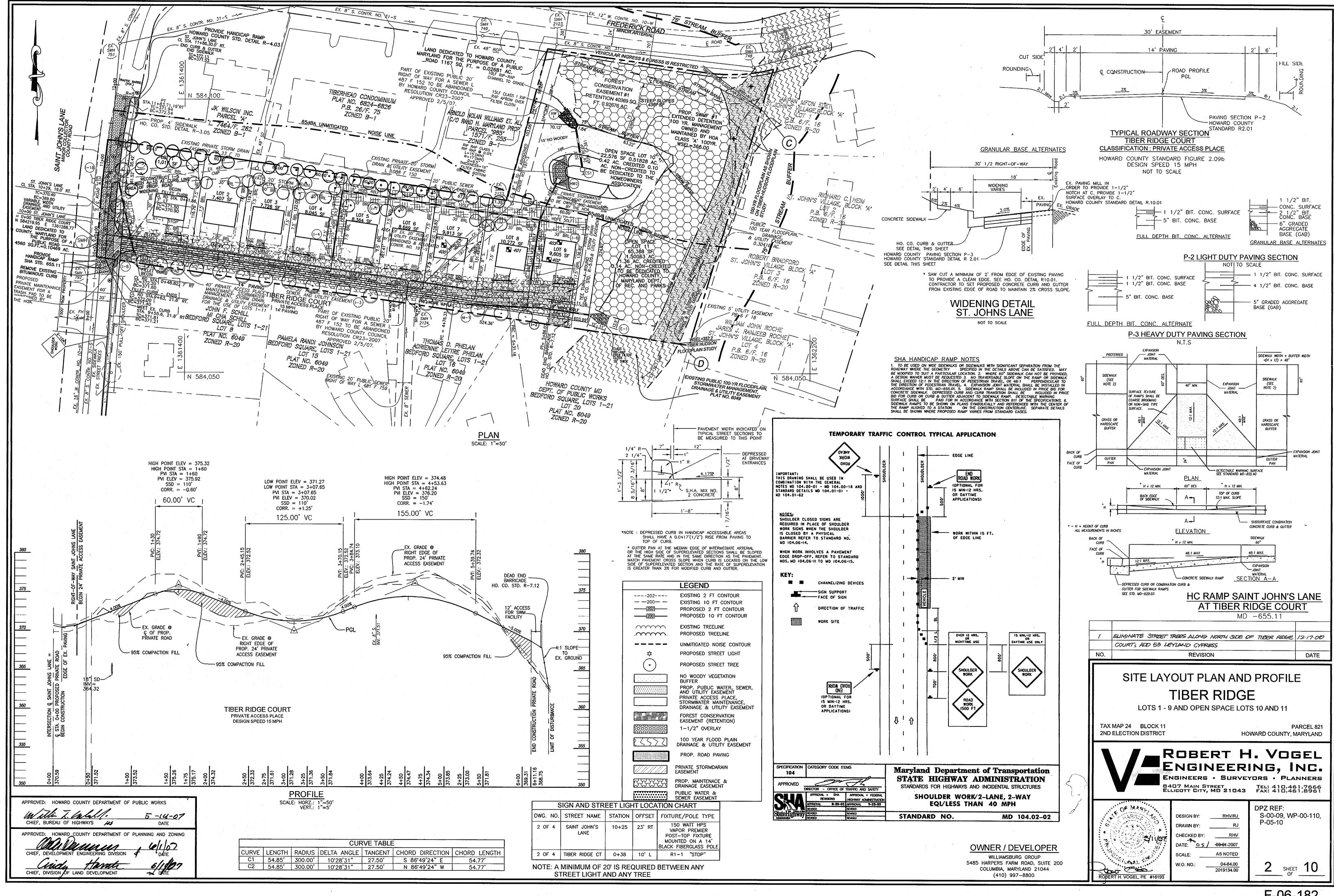
PARCEL 821 HOWARD COUNTY, MARYLAND

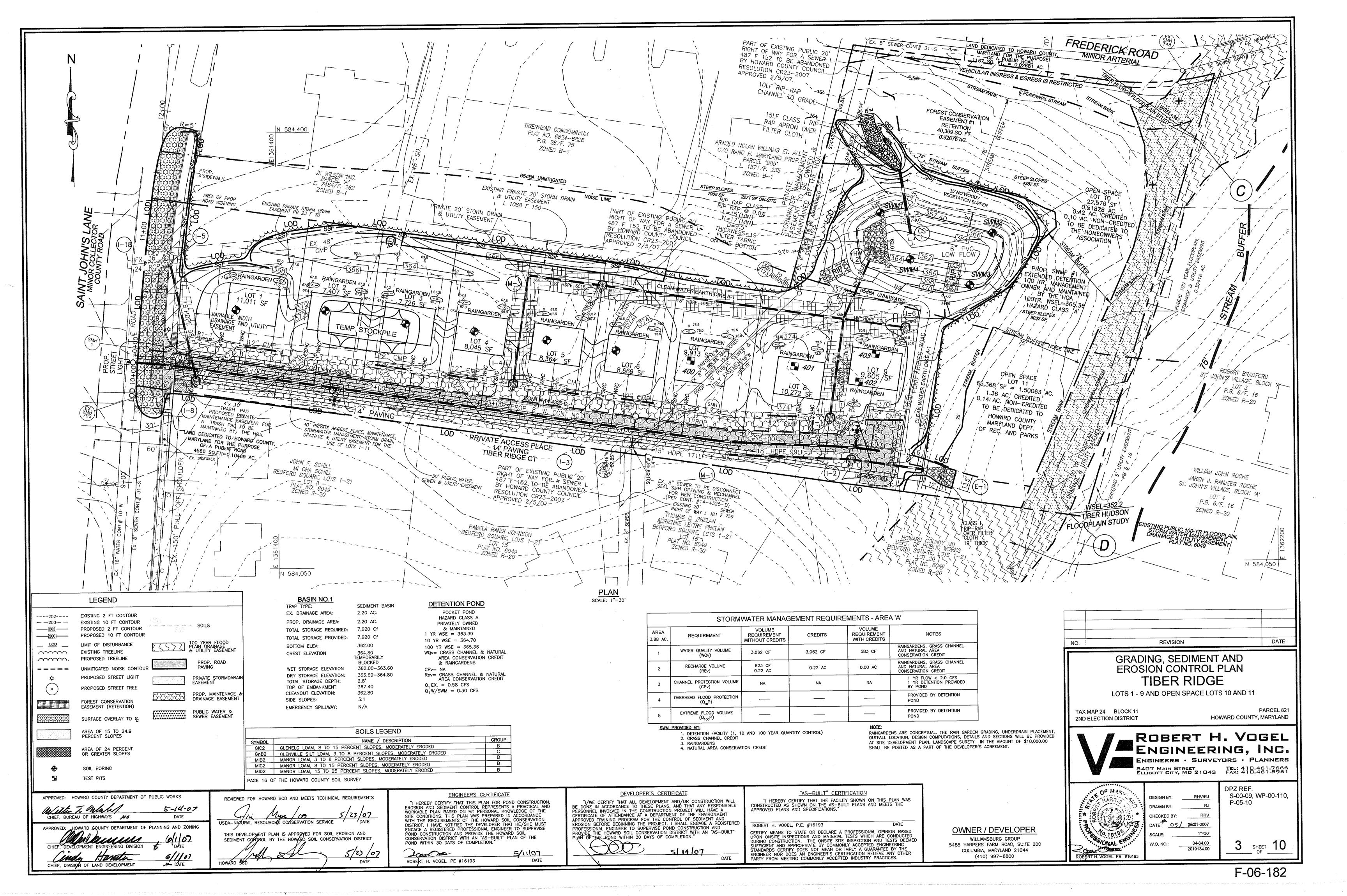


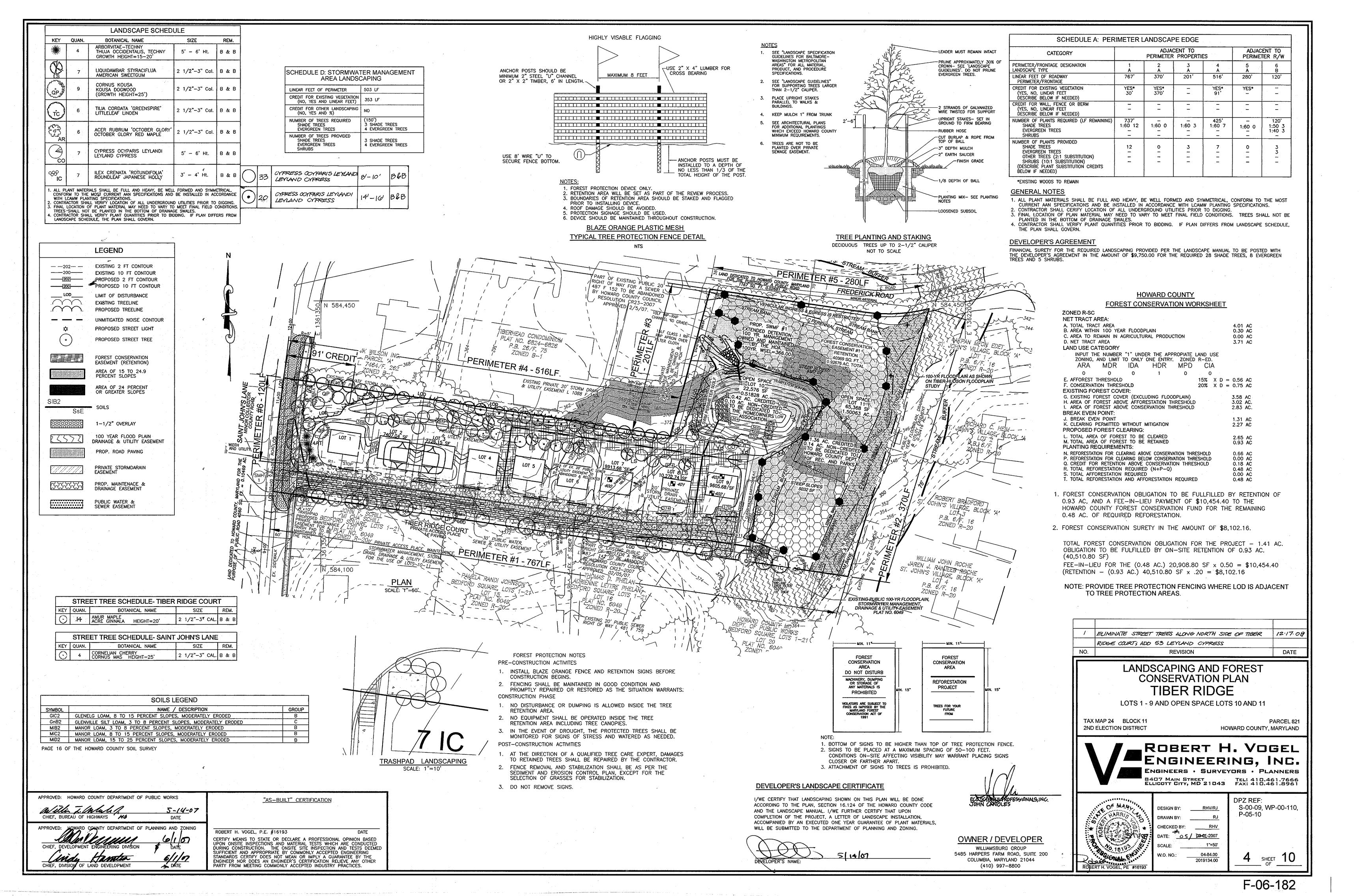
ROBERT H. VOGEL ENGINEERING, INC. ENGINEERS • SURVEYORS • PLANNERS 8407 MAIN STREET TEL: 410.461.7666 ELLICOTT CITY, MD 21043 FAX: 410.461.8961

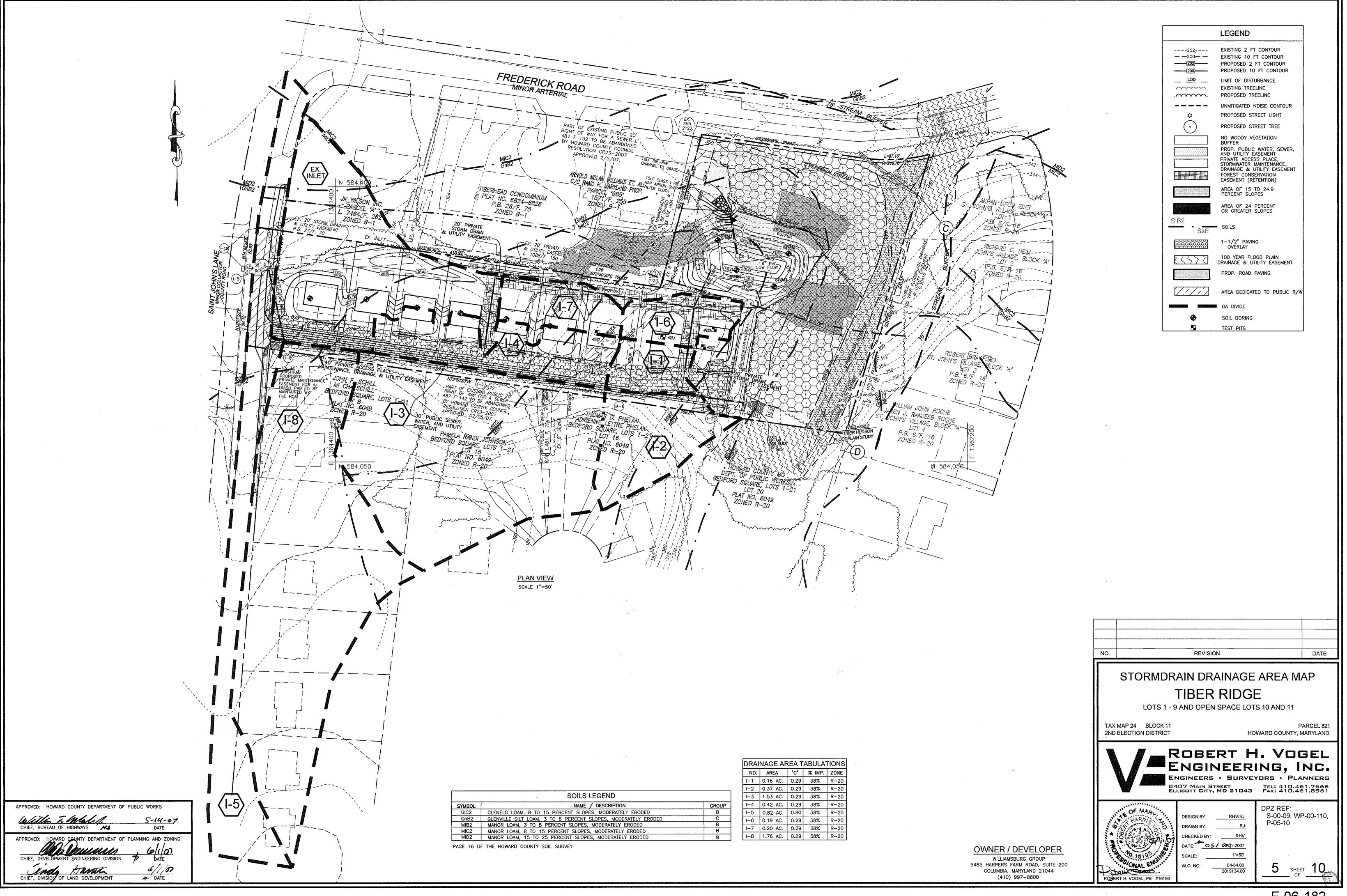


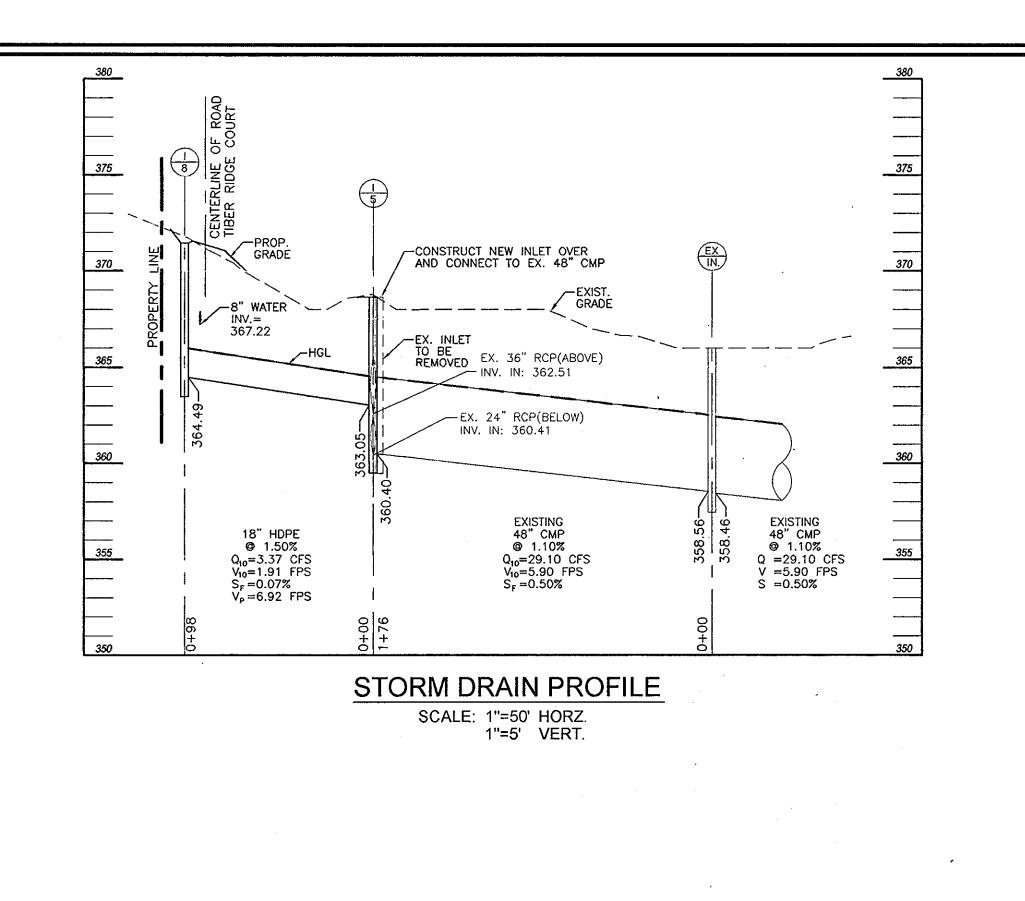
DESIGN BY: CHECKED BY: AS NOTED DPZ REF: S-00-09, WP-00-110,

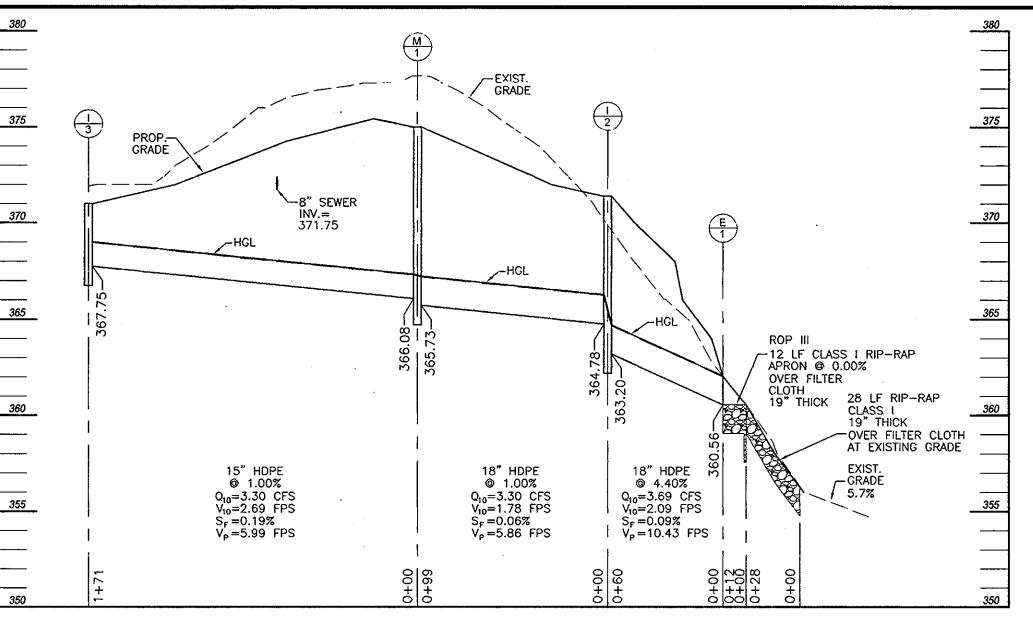




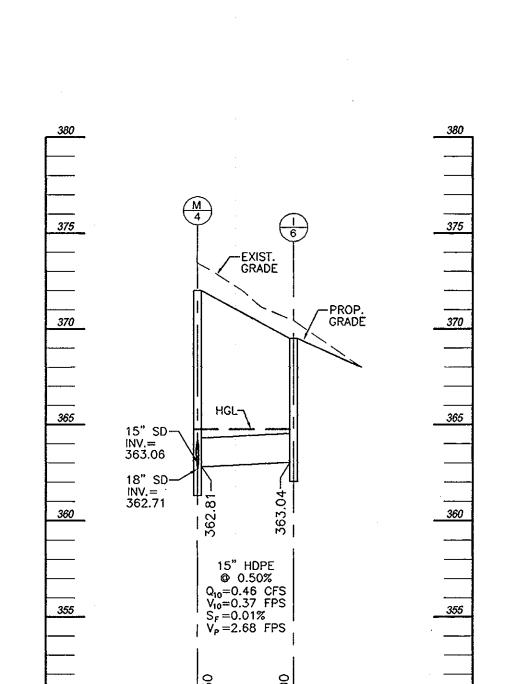






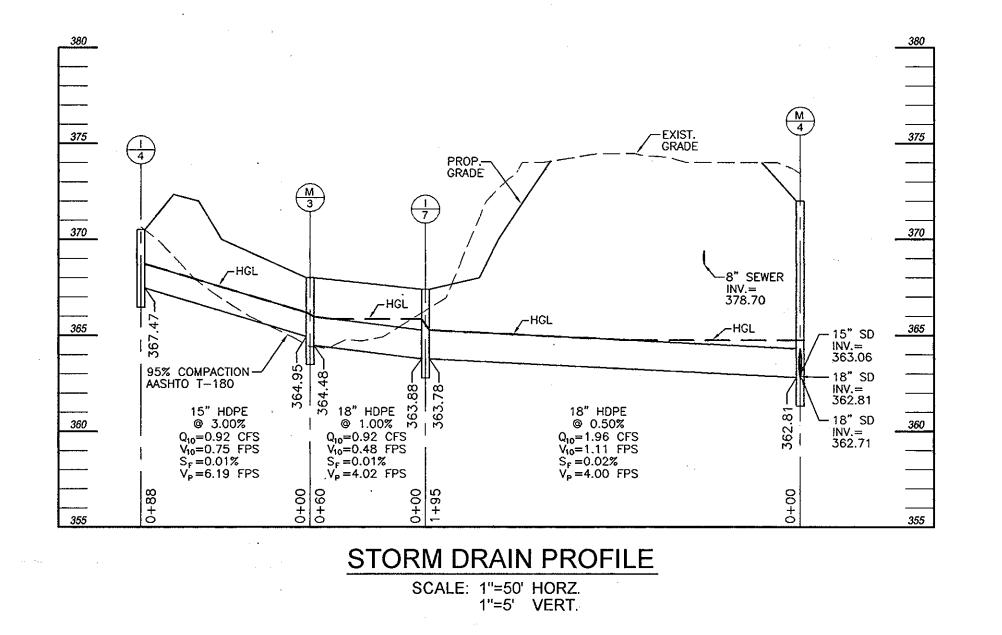


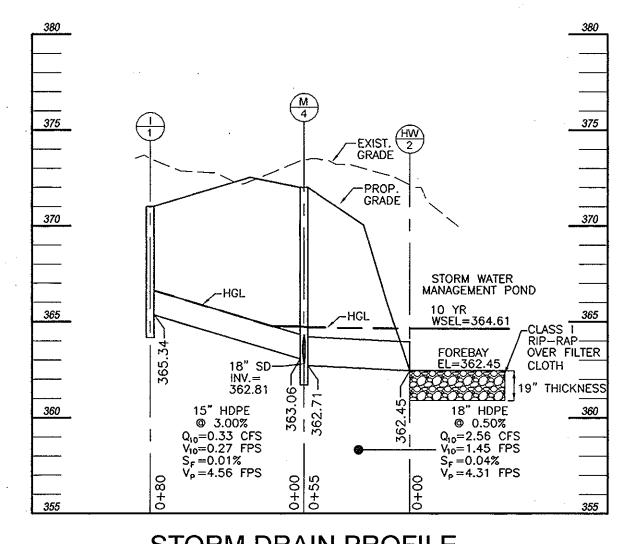
STORM DRAIN PROFILE SCALE: 1"=50' HORZ. 1"=5' VERT.



STORM DRAIN PROFILE

SCALE: 1"=50' HORZ. 1"=5' VERT.





SCA	LE:	1"=50"	HORZ.	
		1"=5"	VERT.	

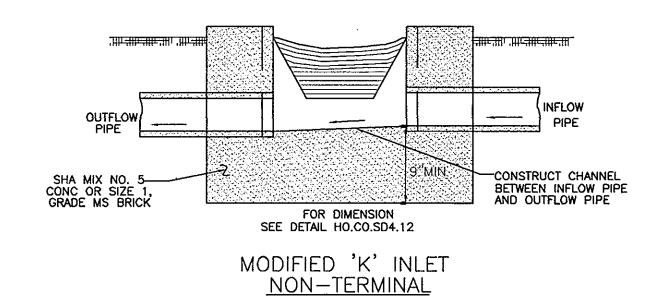
PUBLIC/ PRIVATE NO.	MO	TYPE	LOCA	LOCATION		INV.	INV.	OCHARICO
	NO.	TIPE	NORTHING	EASTING	ELEV.*	IN	OUT	REMARKS
PRIVATE	I-1	STANDARD PRECAST TYPE K	584167	1361842	371:00		365.34	SD-4.12
PRIVATE	1-2	STANDARD PRECAST TYPE K	584133	1361858	371.40	364.78	363.20	SD-4.12
PRIVATE	1-3	STANDARD PRECAST TYPE K	584163	1361590	371.00		367.75	SD-4.12
PRIVATE	I-4	STANDARD PRECAST TYPE K	584196	1361588	370.50		367.47	SD-4.12
PUBLIC	1-5	STANDARD PRECAST TYPE A-10	584299	1361314	368.64**	363.05	360.50	SD-4.02
PRIVATE	1-6	STANDARD PRECAST TYPE K	584238	1361903	369.50		363.04	SD-4.12
PRIVATE	1-7	STANDARD PRECAST TYPE K	584274	1361660	367.40	363.88	363.78	SD-4.12
PRIVATE	1–8	STANDARD PRECAST TYPE K	584201	1361334	371.60		364.49	SD-4.12

*TOP ELEV.=SLOT OPENING ELEVATION FOR TYPE K INLETS TO EXISTING ROAD AS REQUIRED

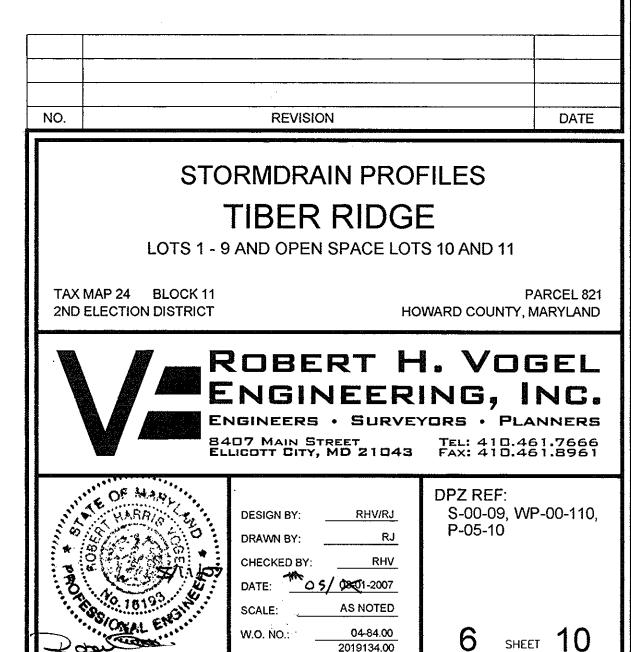
		STORM DR	AIN WA	ANHOL		コピレし	LE	
PUBLIC/	NO.	IO. TYPE LOCATION	ATION	TOP	INV.	INV.	DEMARKS	
PRIVATÉ	NO.	TIPE	NORTHING	EASTING	ELEV.	IN	OUT	REMARKS
PRIVATE	M-1	STANDARD 4' PRECAST MANHOLE	584138	1361759	375.00	366.08	365.73	G-5.12
PRIVATE	M-3	STANDARD 4' PRECAST MANHOLE	584283	1361601	368.00	364.95	364.48	G-5.12
PRIVATE	M-4	STANDARD 4' PRECAST MANHOLE	584246	1361853	372.00	363.06 362.81	362.71	G-5.12

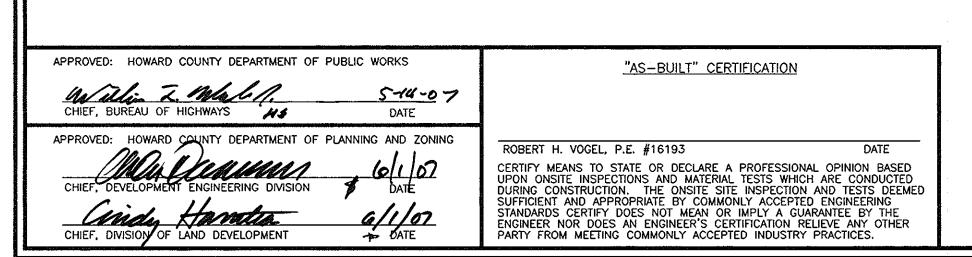
	STRUCTURE SCHEDULE								
PUBLIC/	NO.	TYPE	LOCATION		TOP	INV.	INV.	REMARKS	
PRIVATE	140.	1172	NORTHING	EASTING	ELEV.	IN.	OUT	NEMARNS	
PRIVATE	CS-1	CONTROL STRUCTURE	584311	1361918	366.33		355.60	SEE SWM DETAILS	
PRIVATE	HW-1	TYPE 'A' HEADWALL (FOR 30"ø)	584386	1361880	357.00		353.00	SD-5.11	
PRIVATE	E-1	24" ROUND CMP END SECTION	584124	1361919	363.54		360.56	SD-5.61 & SD-5.62	
PRIVATE	HW-2	TYPE 'A' HEADWALL	365.45	1361876	363.95		362.45	SD-5.11	

	PIPE SCHEDULE									
PUBLIC/ PRIVATE	PIPE SIZE	TYPE	TOTAL LENGTH							
PRIVATE	15"	HDPE	218							
PRIVATE	18"	HDPE	567							
PRIVATE	24"	RCP- ASTM C-361	50							
PUBLIC	48"	14 GAUGE CMP	5							



NTS





EXISTING INFORMATION

-EXTEND EX. 48" CMP TO 1-5

PROP. 18" HDPE - INV. IN: 363.05 EX. 36" RCP(ABOVE) INV. IN: 362.51

EX. 48" CMP @ 1.10%

CUT EX. 24" & 36"RCP AT I-5

AND CONNECT TO I-5

STORM DRAIN PROFILE

SCALE: 1"=50' HORZ. 1"=5' VERT.

EX. 24" RCP(BELOW)

AS NECESSARY.

REMOVED

EX. 36" RCP (ABOVE) INV. OUT: 363.54

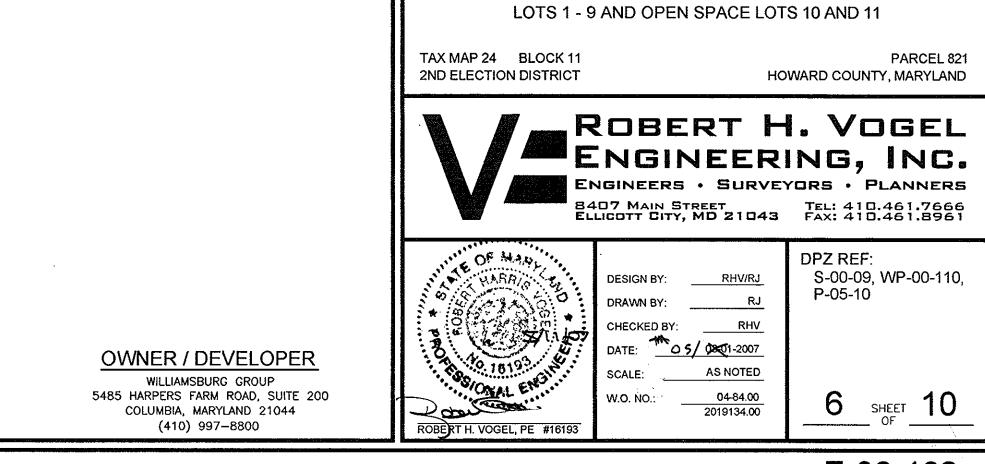
L-EX. 24" RCP (BELOW) INV. OUT: 360.79

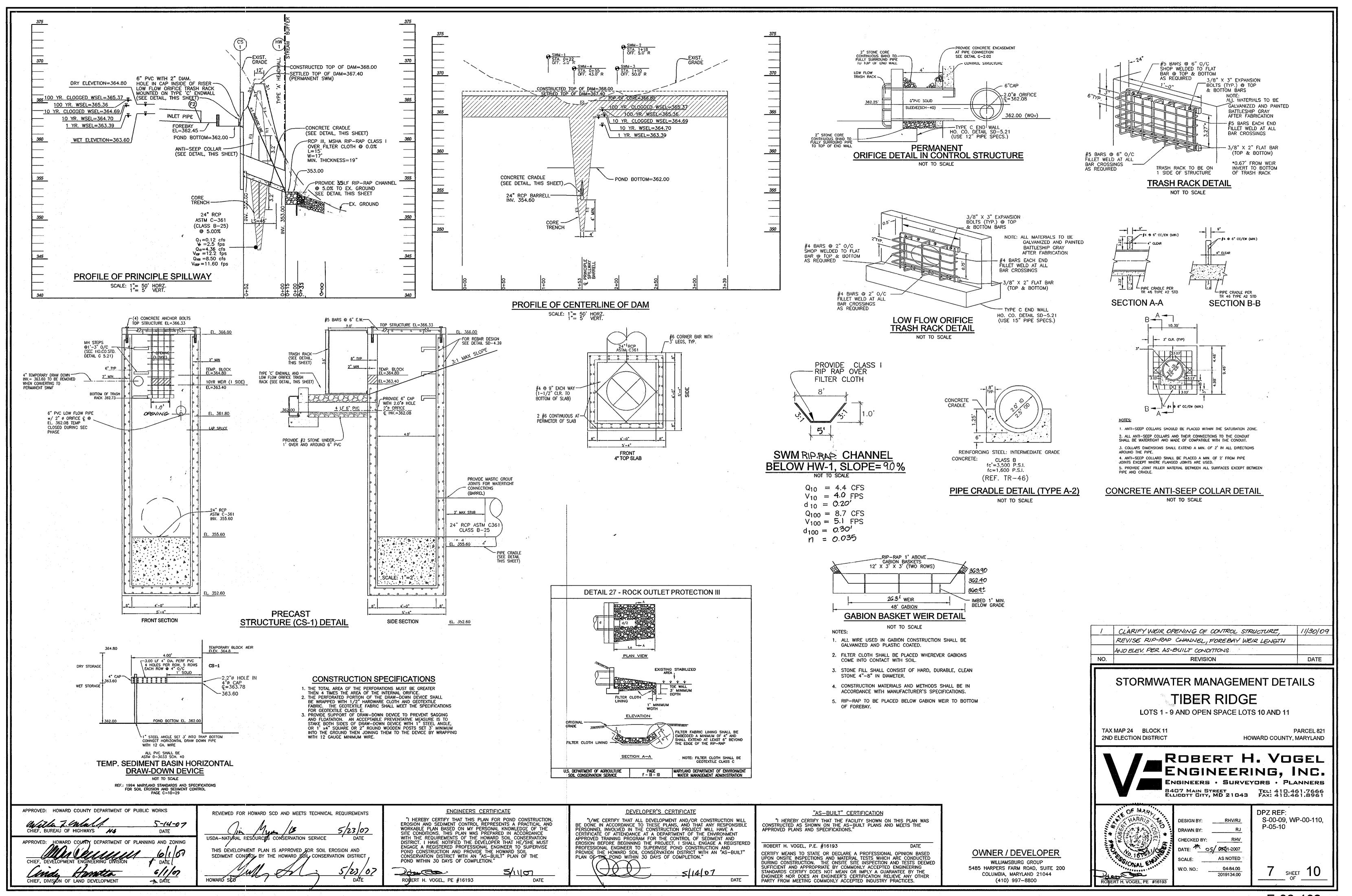
X Z X Z

* INFORMATION CONTAINED IN THIS PROFILE PROVIDED BY HOWARD COUNTY DPW

CONTRACTOR TO TEST PIT IN ADVANCE OF

CONSTRUCTION, CONFIRM ELEVATIONS AND ADJUST 375





Coroneos Property ECS Project No. 02-3464 October 4, 2004

RECOMMENDATIONS

Stormwater Management Pond and Drywells

At the time this subsurface investigation was performed, stormwater management pond details were not indicated on the site plan provided to us. However, based on the site topography it is anticipated that the pond embankments will consist of both cuts and fills.

Infiltration Rates for SWM Pond and Drywells

Field infiltration tests were originally planed to be performed at a total of three (3) locations on the site; one (1) at the proposed SWM pond location and two (2) within the area of the proposed Drywells. However, after evaluating the samples and reviewing the boring logs it was determined that generally the materials encountered at a depth of about 3.5 feet to the maximum depths explored, consisted of extremely dense residual materials identified as decomposed rock which should be considered to be a non-permeable material. Therefore, field infiltration tests were not performed and infiltration at the proposed SWM Pond and drywells was considered not

Pond Construction

Riser Structure The soil encountered within the Stormwater Management Pond at this site are generally suitable for the construction of conventional spread footing bearing on natural firm soils for a net allowable soil bearing pressure of 3,000 pounds per square foot (psf). The net allowable soil bearing pressure refers to that pressure which may be transmitted to the foundation bearing soils in excess of the final overburden pressure at the footing bearing level.

Total settlements of individual footings is anticipated to be on the order of 1 inch; and maximum differential settlements are expected to be on the order of 1/2 inch over 30 feet horizontally.

In order to reduce the possibility of foundation bearing failure and excessive settlement due to local shear or "punching" action, we recommend that footings have a minimum lateral dimension of 2.5 feet. In addition, footings should be placed at least 30 inches below final exterior grade to provide adequate protection against frost heave.

Cut-Off Trench and Impervious Core Construction

According to the site grading plan, cut-off trenches and impervious cores might be required at the north section of the proposed SWM pond.

Coroneos Property ECS Project No. 02-3464 October 4, 2004

In accordance with Maryland Code 378 requirements, the cutoff trench should extend at least 4 feet below the principal spillway pipe, have a minimum width of 4 feet, and have side slopes of 1H: 1V, or flatter. The impervious core should extend vertically upward from the cutoff trench to the 10-year stormwater surface elevation. Fill materials for the cutoff trench and impervious core construction should consist of GC, SC, CL, or CH soil types, having at least 30 percent by

Based upon the results of the soil borings, there appears to be an insufficient quantity of GC, SC, CL. and CH fills materials at the project site for use in cutoff trench and impervious core construction. These materials will need to be imported to the project site after prior approval for

Fill materials for the cut-off trench and impervious core should be placed in 8-inch loose lifts and compacted to at least 95 percent of the maximum dry density in accordance with the Standard Proctor test method, ASTM D 698. We recommend that moisture contents at the time of construction should generally be within the range of the optimum moisture content to 3 percentage points wet of the optimum moisture content. Placement and compaction of the cutoff trench and impervious core fill materials should be monitored by the Geotechnical Engineer on a full-time basis to ensure that fill materials are being placed and compacted in accordance with plans and specifications.

General Embankment Construction

Detailed plan of the SWM pond was not available at the time this report was prepared. However, it is anticipated that embankments will be required at the northern section of the pond. The results of the borings indicate that the cut slopes should primarily contain medium dense to extremely dense Sand soils classified SM per ASTM D-2487. Based on the results of the test borings, the natural soils encountered in the cut slopes should be suitable for a 2.5H:1V or flatter slopes. The fill slopes should consist of embankment fill as recommended in the Fill Placement and Compaction Section.

Embankment soils placed outside the limits of the cut-off trench and impervious core should consist of soils classified as CL, ML, SC, SM, SC-SP, or SM-SP in accordance with ASTM D-2487. Soils of these types should be readily available from excavated materials associated with the pond construction, although care will need to be exercised to ensure that the materials do not contain excessive amounts of organics.

Coroneos Property ECS Project No. 02-3464 October 4, 2004 Page 7

Earthwork Operations

Williamsburg Group, LLC

St. Johns Lane at Frederick Road, Ellicott City, MD

E I ENGLESH UNITS

Topsoil Depth 4"

DESCRIPTION OF MATERIAL

Sitty SAND, Some Rock Fragments, Brown, Gray, Molst, Very Dense, (SM)

END OF BORING • 10.0'

Spoon Refusal at 8.7ft. Auger to 10.0ft.

370.50

THE STRATFFICATION LINES REPRESENT THE APPRICONATE BOUNDARY LINES BETVEON SIZE, TYPES DI-SITU THE TRANSFROM HAY BE GRADUM.

200EDNG COMPLETED 09-16-04 CAVE DE DEPTH ● 6.7/ 6.4 ● 24 HOURS

RIC CME 45 FORDERS D & S DELLERO METROD HSA

₩5 OR @ BORDES STARTED 09-16-04

Coroneos Property

Subgrade preparation generally should include the removal of the topsoil and the removal of soft or loose soils and other unsuitable materials from the house pad areas and any proposed pavement areas. The actual depths, quantities, and quality of surface materials must b a minimum of 10 feet beyond the building and pavement limits. Stripping limits should be

extended an additional 1 foot for each foot of fill required at the house's exterior edge.

For construction purposes, excavation difficulty may be correlated to SPT results. It should be possible to excavate materials displaying SPT results of 50 blows per 4 inches of penetration or less with conventional earthwork equipment which may include isolated ripping. Materials exhibiting SPT results greater than 50 blows per 4 inches or at auger refusal levels most likely will require hoe ramming, and possibly blasting, particularly in narrow trench excavations.

In addition to the excavation guidelines outlined above, it should be pointed out that the physical characteristics of the extremely dense residual soils and underlying rock, such as foliation, jointing, fracturing and lithology, along with the type of excavation equipment used, will greatly influence this aspect of the earthwork operations. We recommend that the applicable conti consider a test pit program, which can assist in correlating the subsurface data with anticipated excavation difficulties during construction with the type of equipment planned for use on th project site. It is also imperative that all contract documents related to the excavation of rock materials include specific criteria (i.e. equipment type and power ratings) to define rock excavation for mass grading and removal in narrow excavations. The applicable contractors should also be required to visit the site to observe the existing conditions and should be encouraged to perform their own explorations to determine the impact that rock may have on their working on the project site.

Prior to the placement of any fill materials or slab subbase materials, the exposed subgrade soils should be examined by a qualified representative of the Geotechnical Engineer. The exposed soils should be thoroughly proofrolled by a vehicle having an axle weight of at least 10 tons, such as a loaded tandem-axle dump truck. This procedure is intended to assist in identifying any localized yielding materials. In the event that any yielding areas are encountered during the proofrolling operations, the subgrade should be either thoroughly densified in-place, scarified/aerated and recompacted, or undercut to firm ground and replaced with controlled,

3464 DW-3 1 or 1

PLASTIC VALUE INCOME

INCOME X CONTENT X LIMIT X

B BOOK GOALSTY DESIGNATION & RECOVER

Coroneos Property ECS Project No. 02-3464 October 4, 2004 Page 8

Coroneos Property

St. Johns Lane at Frederick Road, Ellicott City, MD

SURFACE ELEVATION

Topsoil Depth 4"

Silty SAND, Some Rock

Fragments, Brownish Gray, Moist, Medium Dense, (SM)

Decomposed Rock, Brownish Gray Extremely Dense, (SM/Weathered

END OF BORING @ 10.0'

DESCRIPTION OF MATERIAL

Fill Placement and Compaction

Prior to placement of compacted fill, representative bulk samples (about 50 pounds) should be taken of the proposed fill soils and laboratory tests should be conducted to determine Atterberg limits, natural moisture content, grain-size distribution, and moisture-density relationships for compaction. In general, any materials to be used as structural fill should consist of those materials previously described in the previous sections pertaining to Cut-off Trench and Impervious Core Construction and General Embankment Construction. Materials acceptable for the interpretation of the chould be for of construction (for the 2 persons) and debris, containing no rocks greater than 4 inches in their largest dimension. Any off-site borrow soils, if required, should meet the same material requirements and should be approved by the

The on-site soils generally should be acceptable for re-use as embankment fill, with the restrictions previously addressed. As previously mentioned, there appears to be an insufficient quantity of the on-site soils suitable for use in the construction of the cutoff trenches and impervious cores. This material will need to be imported to the project site. Bulk comple should be taken of the proposed impervious soils and submitted to the Geotechnical Engineer for pproval prior to the importation of this material to the site.

Due to the textural variations of the on-site soils, variations in moisture-density relationships should be anticipated. Such changes must be determined in the field by the Geotechnical Engineer, or his authorized representative, during the earthwork operations and treated

All structural fill should be placed in loose lifts which do not exceed 8 inches in thickness, and should be compacted to at least 95 percent of the maximum dry density, as determined by the Standard Proctor compaction test method (ASTM D 698). Generally, the moisture content of the fill materials should be maintained within ±2 percentage points of the optimum moisture content (OMC) for the fill materials, as determined by ASTM D 698

The footprints of the proposed houses and SWM embankment should be well defined, including the limits of the fill zones, at the time of fill placement. Grade controls should be maintained throughout the filling operations. All filling operations should be observed on a full-time basis by a qualified representative of the Geotechnical Engineer to determine that the minimum compaction requirements are being achieved. A minimum of one compaction test per lift and every 2,500 square foot of lift area should be made. The elevations and locations of the tests should be clearly identified at the time of fill placements.

At the end of each work day, all fill areas should be graded to facilitate positive drainage of any precipitation and the surface should be sealed by use of a smooth-drum roller to limit infiltration of surface water. During placement and compaction of new fill at the beginning of each workday, the contractor should scarify existing subgrades so that a weak plane will not be formed between the new fill and the existing subgrade soils. We recommend that subgrade soils be scarified to depths of about 6 inches prior to placement of new fill.

373.00

PLASTIC WATER LEQUID LIBERT X CONTENT X LIBERT X

ROCK QUALITY DESIGNATION & RECOVERY

⊗ STANDARD PROTERATION
20 20 40 40 50+

Coroneos Property ECS Project No. 02-3464 October 4, 2004 Page 9

Fill materials should not be placed on frozen soils or frost-heaved soils and/or on soils that are excessively wet. Borrow fill materials should not contain frozen materials at the time of placement. All frozen or frost-heaved soils should be removed prior to placement of controlled and compacted fill, granular subbase materials, foundation or slab concrete, and/or asphalt pavement materials. Similarly, excessively wet soils should be scarified and acrated and

If any problems are encountered during the continued operations, or if the conditions decides from those encountered during our subsurface exploration, the Geotechnical Engineer should be notified immediately.

Construction Considerations

Williamsburg Group, LLC

St. Johns Lane at Frederick Road, Ellicott City, MD

DESCRIPTION OF MATERIAL

Fragments, Brown, Gray, Moist, Medium Dense, (SM)

END OF BORING @ 10.0'

Spoon Refusal at 8.8ft.
Auger to 10.0ft
Offset Boring. Set Infiltration Pipe at

371.00

THE STRATUPICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES RETYCEN SOIL TITPES IN-SITU THE TRANSITION HAT HE GRADUAL

¥#1(AC) DRY BORING COMPLETED 09-16-04 CAVE IN DEPTH ● 7.1/ 7.0 ● 24 HOURS

MICH AS FOREMAN D & S DRILLING METHOD HSA

W3 OR ௵ MORING STARTED 09-16-04

PLASTIC WATER LEQUI LIBERT X CONTENT X LIBERT

STANDARD PENETRATION
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Coroneos Property

Due to the presence of fines, some of the near surface on-site soils will be sensitive to moisture and disturbance. Construction activities in the presence of excess moisture can lead to softening of the subgrade soils and loss of hearing canacity. Therefore it will be nowent to schedu earthwork operations during the warmer and drier seasons that typically extend from late spring to early fall. Measures should also be taken to limit site disturbance, especially from rubber-tired heavy construction equipment, and to control and remove surface water from development areas including structural and pavement areas. It is advisable to designate haul roads and traffic areas to limit the areas of disturbance and to prevent construction traffic from excessively degrading the sensitive subgrade soils, especially the very moisture sensitive clayey soils.

A firm work surface should be established prior to construction of new fills. Also, the moisture contents of the fill soils at the time of placement should be carefully controlled. These measures are necessary to ensure that the required compaction effort can be achieved without excessive pumping or movement of the fill mass.

Groundwater was only encountered in the Boring DW-1 as noted in the Water Level Observations section of this report. However, this groundwater is believed to be either perched or restricted to the area in the vicinity of DW-1. Any groundwater encountered during the construction of the building will most likely be a result of surface water infiltration and perched water conditions, and should be readily managed by interceptor trenches and localized systems of sumps and pumps.

Surface drainage conditions should be properly maintained. Surface water should be directed away from the construction area, and the work area should be sloped at a grade of 1 to 2 percent to reduce the potential of ponding water and the subsequent saturation of the surface soils. At the end of each work day, the subgrade should be sealed by rolling the surface with a smooth drum roller to minimize infiltration of surface water.

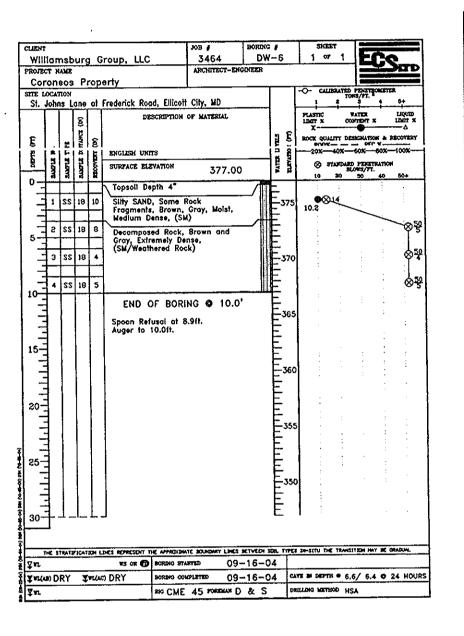
All foundation excavations must be protected to prevent the disturbance of the subgrade materials and to minimize any potential loss of support capacity. Foundation concrete should be placed on the same day that foundation excavations are excavated and approved. Should

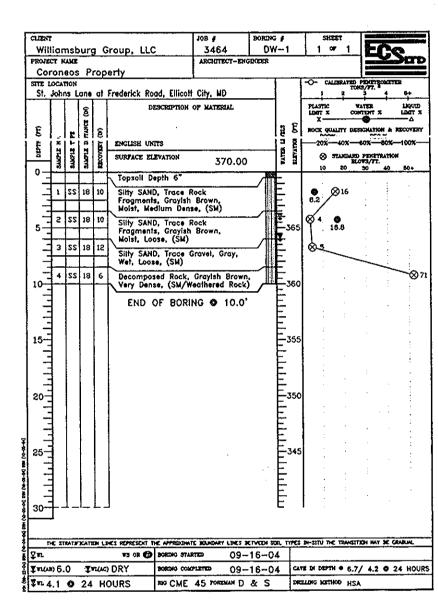
Coroneos Property ECS Project No. 02-3464 October 4, 2004

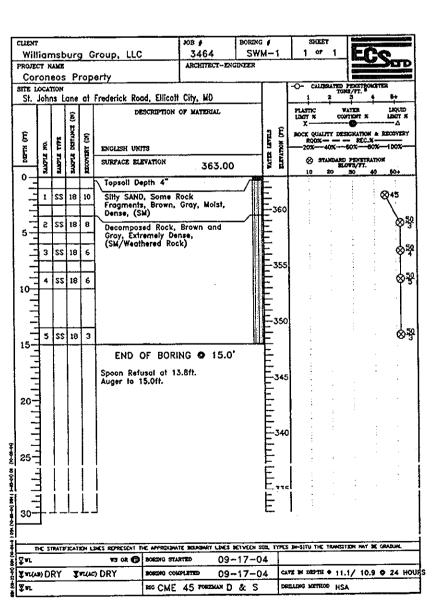
excavation and placement of foundation concrete the same day not be practical, we recommend that a concrete mud mat, 2 inches to 3 inches thick, be placed to protect the subgrade soils from moisture changes, and disturbance during construction activity. If protection of the soils is not provided, then undercutting of soft or loose soils and replacement with controlled fill may be necessary prior to the placement of reinforcing steel and foundation concrete. Prior to the placement of any foundation concrete or mud mat, the subgrade soils must be carefully examined and tested by the Geotechnical Engineer to confirm the availability of the design soil bearing capacity. To minimize disturbance to the subgrade soils during excavation, we recommend that a backet without scarifying teeth and hand excavation be utilized during the final phases of the excavation for the foundations.

Cuts or excavations associated with building and utility excavations may require forming or bracing, slope flattening or other physical measures to control sloughing and/or prevent slope failures. Contractors should be familiar with applicable OSHA and MOSHA codes to ensure that adequate protection of the excavations and trench walls is provided.

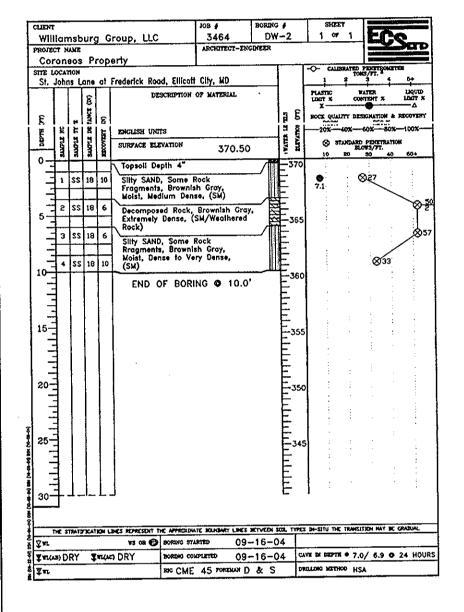
The surface soils will be erodible. Therefore, the contractor should provide and maintain good site drainage during earthwork operations to maintain the integrity of the surface soils. All erosion and sedimentation shall be controlled in accordance with sound engineering practice and

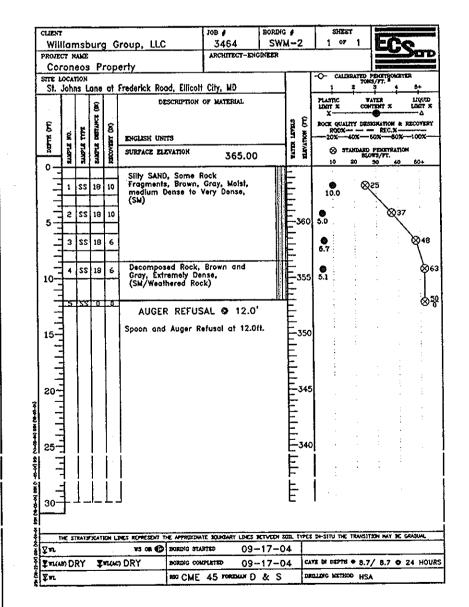


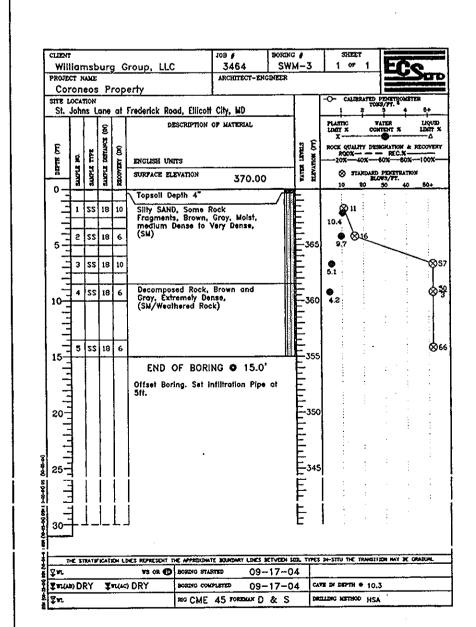


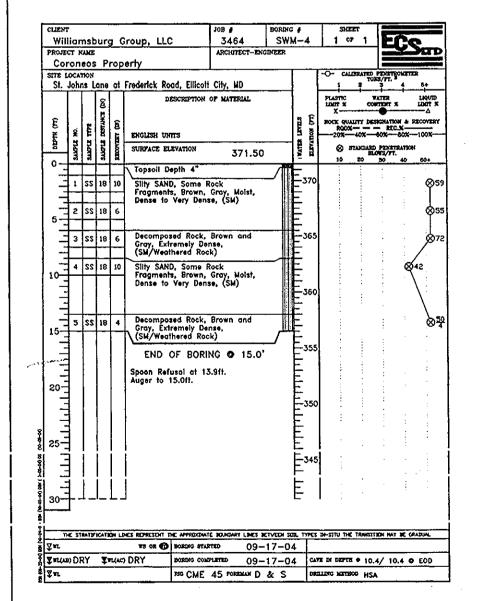


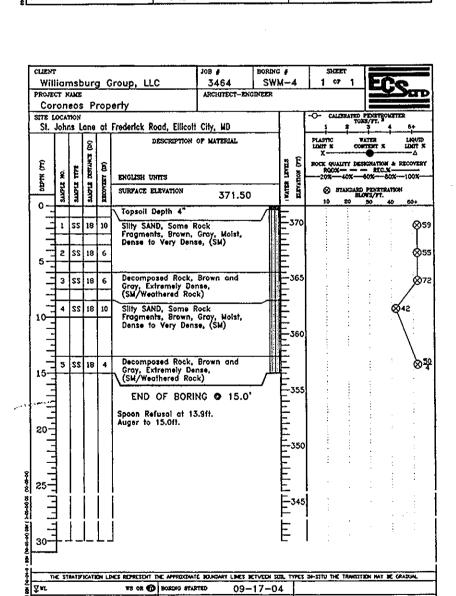
والمستقدمة لأراجيا سيروح بالمراسين المامل فيسترا فيجرانيهم والمستقدين فيقطين والمنازي المراجع والمراجع والمنازي والمستودي والما







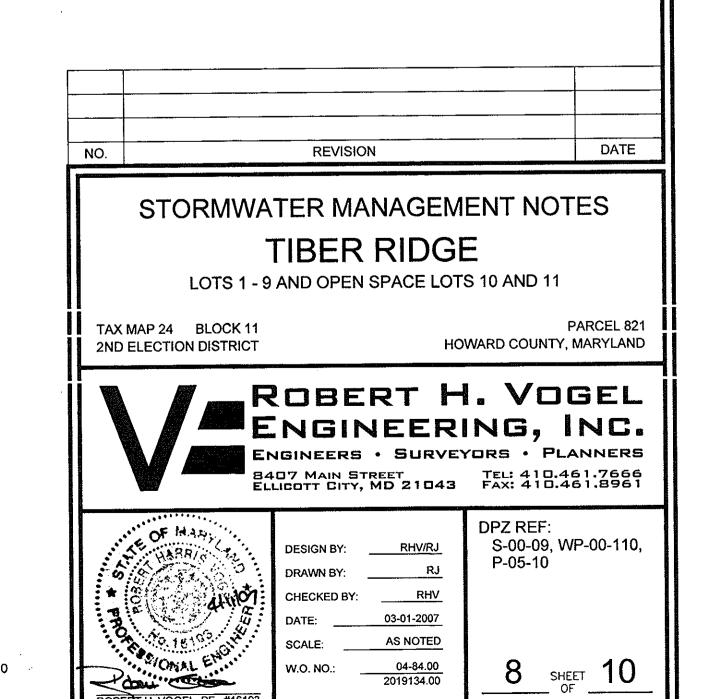




THE STRATUPICATION LINES REPRESENT THE APPROXIMATE RELIGIANY LINES DETVECH SOIL TYPES IN-SITU THE TRANSITION MAY BE GRADUAL

¥ WILLIAD) DRY \$ WILLIAD) DRY \$ \$000000 COMPLETED 09-16-04 CAPE DE DEPTE ♦ 6.8/ 6.6 € 24 HOURS

WS OR €D BOKENS STARTED 09-16-04



04-84.00 2019134.00

W.O. NO.:

Jan Com

ROBERT H. VOGEL, PE #16193

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS 5-14-07 CHIEF, BUREAU OF HIGHWAYS 45 CHIEF, DIVISION OF LAND DEVELOPMENT

OWNER / DEVELOPER WILLIAMSBURG GROUP 5485 HARPERS FARM ROAD, SUITE 200

COLUMBIA, MARYLAND 21044

(410) 997-8800

8

MARYLAND 378 STORMWATER MANAGEMENT POND CONSTRUCTION SPECIFICATIONS

These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent version.

Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the toe of the

Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

Material — The fill material shall be taken from approved designated borrow areas. It shall be free of roots, stumps, wood, rubbish, stones greater than 6°, frozen or other objectionable materials. Fill material for the center of the embankment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the \$200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer. Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment.

Placement - Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment

Compaction — The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with the equipment The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not be so wet that water can be squeezed out.

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within $+\-2\%$ of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

Cut Off Trench - The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with 3. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability.

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be design and construction inspection. placed concurrently with the outer shell of the embankment.

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the hackfilling operation shall driven equipment be allowed to operated closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum of 4.0 and a minimum resistivity of 2.000 ohm-cm. Material shall be placed such that minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sides of the pipe. it only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of the structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24° or greater over the structure or pipe. Backfill (flowable fill)zone shall be of the type and quality conforming to that specified for the core of the embankment or other embankment materials. Pipe Conduits

All pipes shall be circular in cross section.

Corrugated Metal Pipe — All of the following criteria shall apply for corrugated metal pipe: 1. Materials - (Polymer Coated steel pipe)- Steel pipes with polymeric coating shall have a minimum coating thickness of 0.01° inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the

Materials - (Aluminum Coated Steel Pipe) - This pipe and its appurtenances shall conform to the requirements of ASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.

POND BOTTOM SOIL CONDITIONS

If broken rock fragments are encountered at finished pond bottom, under cut a minimum of 12" below basin grade and to a horizontal distance of at least 18" beyond each edge of the broken rock and backfill with fine-grained ML or CL soils compacted to a firm condition. This procedure should be performed under the supervision of the project Geotechnical Engineer.

In order to lower the infiltration rate into the sands with gravel, it is recommmended that the sands with gravel be undercut and replaced with a minimum of 12 inches of soils classified as SM per ASTM D-2487 or Sandy Loam per USDA classification. The fill soil should be compacted to at least 95 percent of its maximum dry density per ASTM D-698.

OPERATION, MAINTENANCE AND INSPECTION INSPECTION OF THE POND(S) SHOWN HEREON SHALL BE PERFORMED AT LEAST ANNUALLY, IN ACCORDANCE WITH THE CHECKLIST AND REQUIREMENTS CONTAINED WITHIN USDA, NRCS "STANDARDS AND SPECIFICATIONS FOR PONDS" (MD-378). THE POND OWNER(S) AND ANY HEIRS, SUCCESSORS, OR ASSIGNS SHALL BE RESPONSIBLE FOR THE SAFETY OF THE POND AND THE CONTINUED OPERATION, SURVEILLANCE, INSPECTION, AND MAINTENANCE THEREOF. THE POND OWNER(S) SHALL PROMPTLY NOTIFY THE SOIL CONSERVATION DISTRICT OF ANY UNUSUAL OBSERVATIONS THAT MAY BE INDICATIONS OF DISTRESS SUCH AS EXCESSIVE SEEPAGE, TURBID SEEPAGE, SLIDING OR SLUMPING.

OPERATION AND MAINTENANCE SCHEDULE FOR STORMWATER MANAGEMENT DETENTION FACILITY

STORMWATER MANAGEMENT FACILITY ROUTINE MAINTENANCE 1. FACILITY WILL BE INSPECTED ANNUALLY AND AFTER MAJOR STORMS. INSPECTIONS SHOULD BE PERFORMED DURING WET WEATHER TO DETERMINE IS FUNCTIONING PROPERLY. . TOP AND SIDE SLOPES OF THE EMBANKMENT SHALL BE MOWED A MINIMUM OF TWO (2) TIMES A YEAR, ONCE IN JUNE AND ONCE IN SEPTEMBER. OTHER SIDÉ SLOPES AND MAINTENANCE ACCESS SHOULD BE MOWED AS

3. DEBRIS AND LITTER NEXT TO THE OUTLET STRUCTURE SHALL BE REMOVED DURING REGULAR MOWING OPERATIONS AND AS NEEDED. 4. VISIBLE SIGNS OF EROSION IN THE POND AS WELL AS RIPRAP OUTLET AREAS SHALL BE REPAIRED AS SOON AS IT IS NOTICED. NON-ROUTINE MAINTENANCE

1. STRUCTURAL COMPONENTS OF THE POND SUCH AS THE DAM, THE RISER, AND THE DIDES SHALL BE REPAIRED LIPON DETECTION OF ANY DAMAGE THE COMPONENTS SHOULD BE INSPECTED DURING ROUTINE MAINTENANCE **OPERATIONS**

2. SEDIMENT SHOULD BE REMOVED WHEN ITS ACCUMULATION SIGNIFICANTLY REDUCES THE DESIGN STORAGE, INTERFERE WITH THE FUNCTION OF THE RISER, WHEN DEEMED NECESSARY FOR AESTHETIC REASONS, OR WHEN DEEMED NECESSARY BY THE HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS.

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS Villia T. Wales la / 5-14-07 CHIEF, BUREAU OF HIGHWAYS /43 DATE

Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-196 or M-211 with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be between 4 and 9. 2. Coupling, bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at lease

24 mils in thickness. 3. Connections - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight. All connections shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be rerolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches diameter. flanges on both ends of the pipe with a circular 3/8 inch thick

closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by a 24 inch long annular corrugated band using a minimum of 4(four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8 inch closed cell gaskets the full width of the flange is also acceptable. Helically consigned nine shall have either continuously welded seams or have lock seams with internal coulding or

4. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth

compacted to provide adequate support. 5. Backfilling shall conform to "Structure Backfill."

6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete pipe: 1. Materials - Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C-361. 2. Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length.

This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in the "Structure Backfill" section of this standard. Gravel bedding is not permitted. 3. Laying pipe - Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

Backfilling shall conform to "Structure Backfill." 5. Other details (anti-seep collars, valves, etc.) shall be shown on the drawings.

Plastic Pipe - The following criteria shall apply for plastic pipe: 1. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" -10" inch pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" inch shall meet the requirements of AASHTO M294 Type S.

2. Joints and connections to anti-seep collars shall be completely watertight. spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

Backfilling shall conform to "Structure Backfill." 5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Drainage Diaphragms - When a drainage diaphragm is used, a registered professional engineer will supervise the

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3.

Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction Materials, Section 311. Geotexile shall be placed under all riprop and shall meet requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

Care of Water during Construction All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the excavations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall be pumped.

All borrow areas shall be graded to provide proper drainage and left 1 a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

Erosion and Sediment Control Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans

shall detail erosion and sediment control measures.

OPERATION AND MAINTENANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED

STORMWATER INFILTRATION TRENCHES

. THE MONITORING WELLS AND STRUCTURES SHALL BE INSPECTED ON A QUARTERLY BASIS AND AFTER EVERY LARGE STORM EVENT.

2. WATER LEVELS AND SEDIMENT BUILD UP IN THE MONITORING WELLS SHALL BE RECORDED OVER A PERIOD OF SEVERAL DAYS TO INSURE TRENCH DRAINAGE. 3. A LOGBOOK SHALL BE MAINTAINED TO DETERMINE THE RATE AT WHICH THE FACILITY DRAINS. 4. WHEN THE FACILITY BECOMES CLOGGED SO THAT IT DOES NOT DRAIN DOWN WITHIN THE INDICATED HOUR TIME PERIOD, CORRECTIVE ACTION SHALL BE TAKEN. 5. THE MAINTENANCE LOGBOOK SHALL BE AVAILABLE TO HOWARD COUNTY FOR INSPECTION TO INSURE COMPLIANCE WITH OPERATION AND MAINTENANCE

6. ONCE THE PERFORMANCE CHARACTERISTICS OF THE INFILTRATION FACILITY HAVE BEEN VERIFIED, THE MONITORING SCHEDULE CAN BE REDUCED TO AN ANNIHAL BASIS HALLESS THE DEDECOMANICE DATA INDICATES THAT A MORE FREQUENT SCHEDULE IS REQUIRED.

OPERATION AND MAINTENANCE SCHEDULE FOR STORMWATER MANAGEMENT EXTENDED DETENTION FACILITY

STORMWATER MANAGEMENT FACILITY ROUTINE MAINTENANCE 1. FACILITY WILL BE INSPECTED ANNUALLY AND AFTER MAJOR STORMS.

INSPECTIONS SHOULD BE PERFORMED DURING WET WEATHER TO DETERMINE IS FUNCTIONING PROPERLY. TOP AND SIDE SLOPES OF THE EMBANKMENT SHALL BE MOWED A MINIMUM OF TWO (2) TIMES A YEAR, ONCE IN JUNE AND ONCE IN SEPTEMBER. OTHER SIDE SLOPES AND MAINTENANCE ACCESS SHOULD BE MOWED AS

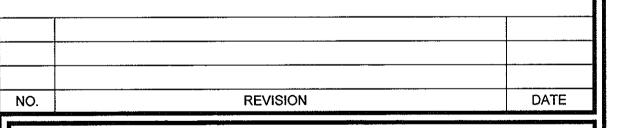
3. DEBRIS AND LITTER NEXT TO THE OUTLET STRUCTURE SHALL BE REMOVED DURING REGULAR MOWING OPERATIONS AND AS NEEDED. 4. VISIBLE SIGNS OF EROSION IN THE POND AS WELL AS RIPRAP OUTLET AREAS SHALL BE REPAIRED AS SOON AS IT IS NOTICED.

1. STRUCTURAL COMPONENTS OF THE POND SUCH AS THE DAM, THE RISER, AND THE PIPES SHALL BE REPAIRED UPON DETECTION OF ANY DAMAGE. THE COMPONENTS SHOULD BE INSPECTED DURING ROUTINE MAINTENANCE

2. SEDIMENT SHOULD BE REMOVED WHEN ITS ACCUMULATION SIGNIFICANTLY REDUCES THE DESIGN STORAGE, INTERFERE WITH THE FUNCTION OF THE RISER, WHEN DEEMED NECESSARY FOR AESTHETIC REASONS, OR WHEN DEEMED NECESSARY BY THE HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS.

OPERATION, MAINTENANCE AND INSPECTION

INSPECTION OF THE POND(S) SHOWN HEREON SHALL BE PERFORMED AT LEAST ANNUALLY, IN ACCORDANCE WITH THE CHECKLIST AND REQUIREMENTS CONTAINED WITHIN USDA, SCS "STANDARDS AND SPECIFICATIONS FOR PONDS" (MD-378). THE POND OWNER(S) AND ANY HEIRS, SUCCESSORS, OR ASSIGNS SHALL BE RESPONSIBLE FOR THE SAFETY OF THE POND AND THE CONTINUED OPERATION, SURVEILLANCE, INSPECTION, AND MAINTENANCE THEREOF. THE POND OWNER(S) SHALL PROMPTLY NOTIFY THE SOIL CONSERVATION DISTRICT OF ANY UNUSUAL OBSERVATIONS THAT MAY BE INDICATIONS OF DISTRESS SUCH AS EXCESSIVE SEEPAGE, TURBID SEEPAGE, SLIDING OR SLUMPING.



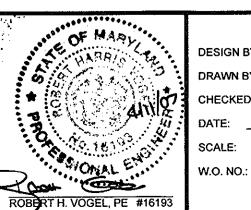
STORMWATER MANAGEMENT NOTES TIBER RIDGE

LOTS 1 - 9 AND OPEN SPACE LOTS 10 AND 11

TAX MAP 24 BLOCK 11 2ND ELECTION DISTRICT

PARCEL 821 HOWARD COUNTY, MARYLAND





OWNER / DEVELOPER

WILLIAMSBURG GROUP 5485 HARPERS FARM ROAD, SUITE 200

COLUMBIA, MARYLAND 21044

(410) 997-8800

DPZ REF: DESIGN BY: DRAWN BY: CHECKED BY: 03-01-2007 04-84.00

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S-00-09, WP-00-110, P-05-10

SHEET 10

F-06-182

21.0 STANDARDS AND SPECIFICATIONS FOR TOPSOIL

DEFINITION

PLACEMENT OF TOPSOIL OVER A PREPARED SUBSOIL PRIOR TO ESTABLISHMENT OF PERMANENT VEGETATION.

TO PROVIDE A SUITABLE SOIL MEDIUM FOR VEGETABLE GROWTH. SOILS OF CONCERN HAVE LOW MOISTURE CONTENT, LOW NUTRIENT LEVELS, LOW PH, MATERIALS TOXIC TO PLANTS, AND/OR UNACCEPTABLE SOIL GRADATION.

CONDITIONS WHERE PRACTICE APPLIES

SLOPES WHERE: THE TEXTURE OF THE EXPOSED SUBSOIL/PARENT MATERIAL

THIS PRACTICE IS LIMITED TO AREAS HAVING 2:1 OR FLATTER

IS NOT ADEQUATE TO PRODUCE VEGETATIVE GROWTH. THE SOIL MATERIAL IS SO SHALLOW THAT THE ROOTING

ZONE IS NOT DEEP ENOUGH TO SUPPORT PLANTS OR FURNISH CONTINUING SUPPLIES OF MOISTURE AND PLANT NUTRIENTS.

C. THE ORIGINAL SOIL TO BE VEGETATED CONTAINS MATERIAL TOXIC TO PLANT GROWTH.

D. THE SOIL IS SO ACIDIC THAT TREATMENT WITH LIMESTONE IS NOT FEASIBLE.

II. FOR THE PURPOSE OF THESE STANDARDS AND SPECIFICATIONS. AREAS HAVING SLOPES STEEPER THAN 2:1 REQUIRE SPECIAL CONSIDERATION AND DESIGN FOR ADEQUATE STABILIZATION. AREAS HAVING SLOPES STEEPER THAN 2:1 SHALL HAVE THE APPROPRIATE STABILIZATION SHOWN ON THE PLANS.

CONSTRUCTION AND MATERIAL SPECIFICATIONS TOPSOIL SALVAGED FROM THE EXISTING SITE MAY BE USED PROVIDED THAT IT MEETS THE STANDARDS AS SET FORTH IN THESE SPECIFICATIONS. TYPICALLY, THE DEPTH OF TOPSOIL TO BE SALVAGED FOR A GIVEN SOIL TYPE CAN BE FOUND IN THE REPRESENTATIVE SOIL PROFILE SECTION IN THE SOIL SURVEY

II. TOPSOIL SPECIFICATIONS - SOIL TO BE USED AS TOPSOIL MUST MEET THE FOLLOWING:

PUBLISHED BY USDA-SCS IN COOPERATION WITH MARYLAND

AGRICULTURAL EXPERIMENTAL STATION.

TOPSOIL SHALL BE A LOAM, SANDY LOAM, CLAY LOAM, SILT LOAM, SANDY CLAY LOAM, LOAMY SAND. OTHER SOILS MAY BE USED IF RECOMMENDED BY AN AGRONOMIST OR A SOIL SCIENTIST AND APPROVED BY THE APPROPRIATE APPROVAL AUTHORITY. REGARDLESS, TOPSOIL SHALL NOT BE A MIXTURE OF CONTRASTING TEXTURED SUBSOILS AND SHALL CONTAIN LESS THAN 5% BY VOLUME OF CINDERS, STONES, SLAG, COARSE FRAGMENTS, GRAVEL, STICKS, ROOTS, TRASH, OR OTHER MATERIALS LARGER THAT 1 AND 1/2" IN

II. TOPSOIL MUST BE FREE OF PLANTS OR PLANT PARTS SUCH AS BERMUDA GRASS. QUACKGRASS, JOHNSONGRASS, NUTSEDGE, POISON MY, THISTLE, OR OTHERS AS SPECIFIED.

III. WHERE THE SUBSOIL IS EITHER HIGHLY ACIDIC OR COMPOSED OF HEAVY CLAYS, GROUND LIMESTONE SHALL BE SPREAD AT THE RATE OF 4-8 TONS/ACRE (200-400 POUNDS PER 1,000 SQUARE FFFT) PRIOR TO THE PLACEMENT OF TOPSOIL. LIME SHALL BE DISTRIBUTED UNIFORMLY OVER DESIGNATED AREAS AND WORKED INTO THE SOIL IN CONJUNCTION WITH TILLAGE OPERATIONS AS DESCRIBED IN THE FOLLOWING PROCEDURES.

II. FOR SITES HAVING DISTURBED AREAS UNDER 5 ACRES: PLACE TOPSOIL (IF REQUIRED) AND APPLY SOIL AMENDMENTS AS SPECIFIED IN 20.0 VEGETATIVE STABILIZATION . SECTION I - VEGETATIVE STABILIZATION METHODS AND MATERIALS.

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

5-14-07

DATE

L. Melan

CHIEF, BUREAU OF HIGHWAYS /

CHIEF, DIVISION OF LAND DEVELOPMENT

III. FOR SITES HAVING DISTURBED AREAS OVER 5 ACRES:
I. ON SOIL MEETING TOPSOIL SPECIFICATIONS, OBTAIN TEST
RESULTS DICTATING FERTILIZER AND LIME AMENDMENTS REQUIRED BRING THE SOIL INTO COMPLIANCE WITH THE FOLLOWING A. PH FOR TOPSOIL SHALL BE BETWEEN 6.0 AND 7.5. IF THE TESTED SOIL DEMONSTRATES A PH OF LESS THAN SUFFICIENT LIME SHALL BE PRESCRIBED TO RAISE

PH TO 6.5 OR HIGHER. B. ORGANIC CONTENT OF TOPSOIL SHALL BE NOT LESS THAN 1.5 PERCENT BY WEIGHT T.5 PERCENT BY WEIGHT.

C. TOPSOIL HAVING SOLUBLE SALT CONTENT GREATER THAN
500 PARTS PER MILLION SHALL NOT BE USED.

D. NO SOD OR SEED SHALL BE PLACED ON SOIL SOIL WHICH
HAS BEEN TREATED WITH SOIL STERILANTS OR CHEMICALS USED FOR WEED CONTROL UNTIL SUFFICIENT TIME HAS ELAPSED (14 DAYS MIN.) TO PERMIT DISSIPATION OF PHYTO-TOXIC MATERIALS.

NOTE: TOPSOIL SUBSTITUTES OR AMENDMENTS, AS RECOMMENDED BY A QUALIFIED AGRONOMIST OR SOIL SCIENTIST AND APPROVED BY THE APPROPRIATE APPROVAL AUTHORITY, MAY BE USED IN LIEU OF NATURAL TOPSOIL.

11. PLACE TOPSOIL (IF REQUIRED) AND APPLY SOIL AMMENDMENTS SPECIFIED IN 20.0 VEGETATIVE STABILIZATION—SECTION I—VEGETATIVE STABILIZATION METHODS AND MATERIALS.

V. TOPSOIL APPLICATION

WHEN TOPSOILING, MAINTAIN NEEDED EROSION AND SEDIMENT CONTROL PRACTICES SUCH AS DIVERSIONS, GRADE STABILIZATION STRUCTURES, EARTH DIKES, SLOPE SILT FENCE AND SEDIMENT TRAPS AND BASINS.

II. GRADES ON THE AREAS TO BE TOPSOILED, WHICH HAVE BEEN PREVIOUSLY ESTABLISHED, SHALL BE MAINTAINED, ALBEIT 4" - 8" HIGHER IN ELEVATION.

III. TOPSOIL SHALL BE UNIFORMLY DISTRIBUTED IN A 4" - 8" LAYER AND LIGHTLY COMPACTED TO A MINIMUM THICKNESS OF 4". SPREADING SHALL BE PERFORMED IN SUCH A MANNER THAT SODDING OR SEEDING CAN PROCEED WITH A MINIMUM OF ADDITIONAL SOIL PREPARATION AND TILLAGE. ANY IRREGULARITIES IN THE SURFACE RESULTING FROM TOPSOILING OR OTHER OPERATIONS SHALL BE CORRECTED IN ORDER TO PREVENT THE FORMATION OF DEPRESSIONS OR WATER POCKETS.

IV. TOPSOIL SHALL NOT BE PLACE WHILE THE TOPSOIL OR SUBSOIL IS IN A FROZEN OR MUDDY CONDITION, WHEN THE SUBSOIL IS EXCESSIVELY WET OR IN A CONDITION THAT MAY OTHERWISE BE DETRIMENTAL TO PROPER GRADING AND SEEDBED PREPARATION.

REVIEWED FOR HOWARD SCD AND MEETS TECHNICAL REQUIREMENTS

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND

SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT

USDA-NATURAL RESOURCES CONSERVATION SERVICE

HOWARD SCO

PERMANENT SEEDING NOTES

APPLY TO GRADED OR CLEARED AREAS NOT SUBJECT TO IMMEDIATE FURTHER DISTURBANCE WHERE A PERMANENT LONG-LIVED VEGETATIVE COVER IS NEEDED.

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED.

. AMENDMENTS: IN LIEU OF SOIL TEST RECOMMENDATIONS, USE ONE OF THE FOLLOWING SCHEDULES:

1) PREFERRED-APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS/100 SQ.FT.) AND 600 LBS PER ACRE 10-10-10 FERTILIZER (14 LBS./ 1000 SQ.FT.) BEFORE SEEDING. HARROW OR DISC INTO UPPER THREE INCHES OF SOIL. AT THE TIME OF SEEDING, APPLY 400 LBS. PER ACRE 30-0-0 UREAFORM FERTILIZER (9 LBS/1000 SQ.FT.) 2) ACCEPTABLE-APPLY 2 TONS PER ACRE DOLOMATIC LIMESTONE (92 LBS/ 1000 SQ.FT.) AND APPLY 1000 LBS. PER ACRE 10-10-10- FERTILIZER (23 LBS./1000 SQ.FT.) BEFORE SEEDING. HARROW OR DISC INTO UPPER

THREE INCHES OF SOIL. SEEDING: FOR THE PERIODS MARCH 1 THRU APRIL 30, AND AUGUST 1 THRU OCTOBER 15, SEED WITH 60 LBS. PER ACRE (1.4 LBS/1000 SQ.FT.) OF KENTUCKY 31 TALL FESCUE. FOR THE PERIOD MAY 1 THRU JULY 31, SEED WITH 60 LBS. KENTUCKY 31 TALL FESCUE PER ACRE AND 2 LBS. PER ACRE (.05 LBS./1000 SQ.FT.) OF WEEPING LOVEGRASS. DURING THE PERIOD OF OBER 16 THRU FEBRUARY 28, PROTECT SITE BY: OPTION (1) 2 TONS PER ACRE WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING. OPTION (2) USE SOD. OPTION (3) SEED WITH 60 LBS/ACRE KENTUCKY 31 TALL FESCUE AND MULCH WITH 2 TONS/ACRE WELL ANCHORED

MULCHING: APPLY 1 1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ. FT.) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ.FT.) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ.FT.) FOR ANCHORING.

MAINTENANCE: INSPECT ALL SEEDED AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND RESEEDINGS.

TEMPORARY SEEDING NOTES

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY

SOIL AMENDMENTS: APPLY 600 LBS. PER ACRE 10-10-10 FERTILIZER (14 LBS./1000 SQ.FT).

SEEDING: FOR PERIODS MARCH 1 THRU APRIL 30 AND FROM AUGUST 15 THRU NOVEMBER 15, SEED WITH 2 1/2 BUSHEL PER ACRE OF ANNUAL RYE (3.2 LBS./1000 SQ.FT.) FOR THE PERIOD MAY 1 THRU AUGUST 14, SEED WITH 3 LBS. PER ACRE OF WEEPING LOVEGRASS (.07 LBS./1000 SQ.FT.). FOR THE PERIOD NOVEMBER 1 THRU FEBRUARY 28, PROTECT SITE BY APPLYING 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE

MULCHING: APPLY 1 1/2 TO 2 TONS PER ACRE (70 TO 90 LBS./1000 SQ.FT.) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ.FT.) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ.FT.) FOR ANCHORING.

REFER TO THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR RATE AND METHODS NOT

ENGINEERS CERTIFICATE

"I HEREBY CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION

EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AN WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE

WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS—BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

ROBERT H. VOGEL, PE #16193

41110

DATE

SEDIMENT CONTROL NOTES

. A MINIMUM OF 48 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT INSPECTION, LICENSE AND PERMITS SEDIMENT CONTROL DIVISION PRIOR TO THE START OF ANY CONSTRUCTION (313-1855).

2. ALL VEGETATION AND STRUCTURAL PRACTICES ARE TO BE INSTALLED ACCORDING TO THE PROVISIONS OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL.

FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: (A) 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, PERIMETER SLOPES, AND ALL SLOPES GREATER HAN 3:1, (B) 14 DAYS AS TO ALL OTHER DISTURBED OR GRADED AREAS ON THE

4. ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 7, HOWARD COUNTY DESIGN MANUAL, STORM DRAINAGE

5. ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDING, SOD, TEMPORARY SEEDING AND MULCHING (SEC. G). TEMPORARY STABILIZATION WITH MULCH ALONE SHALL BE DONE WHEN RECOMMENDED SEEDING DATES DO NOT ALLOW FOR PROPER GERMINATION AND ESTABLISHMENT OF GRASSES.

6. ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE MAINTAINED IN OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.

7. SITE ANALYSIS AREA DISTURBED AREA TO BE ROOFED OR PAVED ARFA TO BE VEGETATIVELY STABILIZED OFFSITE WASTE/BORROW AREA LOCATION

8. ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF UTILITIES MUST BE REPAIRED ON THE SAME DAY OF DISTURBANCE

9. ADDITIONAL SEDIMENT CONTROLS MUST BE PROVIDED, IF DEEMED NECESSARY BY THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR

10. ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING, OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL BY THE INSPECTION AGENCY IS MADE.

11. TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGTHS OR THAT WHICH SHALL BE BACK-FILLED AND STABILIZED WITHIN ONE WORKING DAY, WHICHEVER IS SHORTER.

* TO BE DETERMINED BY CONTRACTOR, WITH PRE-APPROVAL OF THE SEDIMENT CONTROL INSPECTOR WITH AN APPROVED AND ACTIVE GRADING PERMIT

OUTER ROWS, AND 2 ALTERNATING ROWS DOWN THE CENTER.

SPACED 6" APART IN A STAGGERED PATTERN ON EITHER SIDE.

SECURED WITH 2 DOUBLE ROWS OF STAPLES.

EFFECTED BY THE FLOW MUST BE KEYED-IN.

4-10-07

U.S. DEPARTMENT OF AGRICULTURE

DEVELOPER'S CERTIFICATE

BE DONE IN ACCORDANCE TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A

PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED

I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL

6. THE DISCHARGE END OF THE MATTING LINER SHOULD BE SIMILARLY

. WHERE ONE ROLL OF MATTING ENDS AND ANOTHER BEGINS. THE END OF

THE TOP STRIP SHALL OVERLAP THE UPPER END OF THE LOWER STRIP BY 4".

SHIPLAP FASHION. REINFORCE THE OVERLAP WITH A DOUBLE ROW OF STAPLES

NOTE: IF FLOW WILL ENTER FROM THE EDGE OF THE MATTING THEN THE AREA

MARYLAND DEPARTMENT OF ENVIRONMEN WATER MANAGEMENT ADMINISTRATION

"AS-BUILT" CERTIFICATION

CONSTRUCTED AS SHOWN ON THE AS-BUILT PLANS AND MEETS THE APPROVED PLANS AND SPECIFICATIONS."

CERTIFY MEANS TO STATE OR DECLARE A PROFESSIONAL OPINION BASED UPON ONSITE INSPECTIONS AND MATERIAL TESTS WHICH ARE CONDUCTED DURING CONSTRUCTION. THE ONSITE SITE INSPECTION AND TESTS DEEMED SUFFICIENT AND APPROPRIATE BY COMMONLY ACCEPTED ENGINEERING

STANDARDS CERTIFY DOES NOT MEAN OR IMPLY A GUARANTEE BY THE ENGINEER NOR DOES AN ENGINEER'S CERTIFICATION RELIEVE ANY OTHER PARTY FROM MEETING COMMONLY ACCEPTED INDUSTRY PRACTICES.

ROBERT H. VOGEL, P.E. #16193

"I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN WAS

SEQUENCE OF CONSTRUCTION

 OBTAIN GRADING PERMIT. 2. NOTIFY HOWARD COUNTY BUREAU OF INSPECTIONS AND

PERMITS (313-1880) AT LEAST 24 HOURS BEFORE STARTING ANY WORK. 3. CONSTRUCT STABILIZED CONSTRUCTION ENTRANCE 1 DAY 4. INSTALL SUPER SILT FENCE 1 WEEK

5. INSTALL BYPASS STORMDRAIN SYSTEMS (I-8 TO EX. INLET AND 5 DAYS I-3 TO E-1) AND SWALES ON HIGH SIDE OF DRIVEWAY. 3 WEEKS

DURATION

1 WEEK

6. CONSTRUCT SEDIMENT BASIN/STORMWATER MANAGEMENT FACILITY 3 WEEKS 7. AFTER OBTAINING PERMISSION FROM SEDIMENT CONTROL INSPECTOR TO PROCEED ROUGH GRADE TO LOD. 2 WEEKS 8. CONSTRUCT BALANCE OF STORMDRAIN SYSTEM.

1 WEEK 9. GRADE PRIVATE ACCESS PLACE TO SUB-BASE AND INSTALL UTILITIES 10. WITH ROAD GRADED TO SUB-BASE BEGIN ROAD PAVING. 2 WEEKS 11. FINE GRADE SITE IN CONFORMANCE WITH PLAN. 1 WEEK 12. WITH INSPECTOR APPROVAL AND FINAL ROAD PAVING COMPLETE 3 DAYS STABILIZE ANY REMAINING DISTURBED AREAS

14. INSTALL STREET TREES AND ALL LANDSCAPING 15. WITH APPROVAL OF SEDIMENT CONTROL INSPECTOR REMOVE SEDIMENT CONTROL MEASURES AND CONVERT SEDIMENT BASIN

TO PERMANENT STORMWATER MANAGEMENT FACILITY.

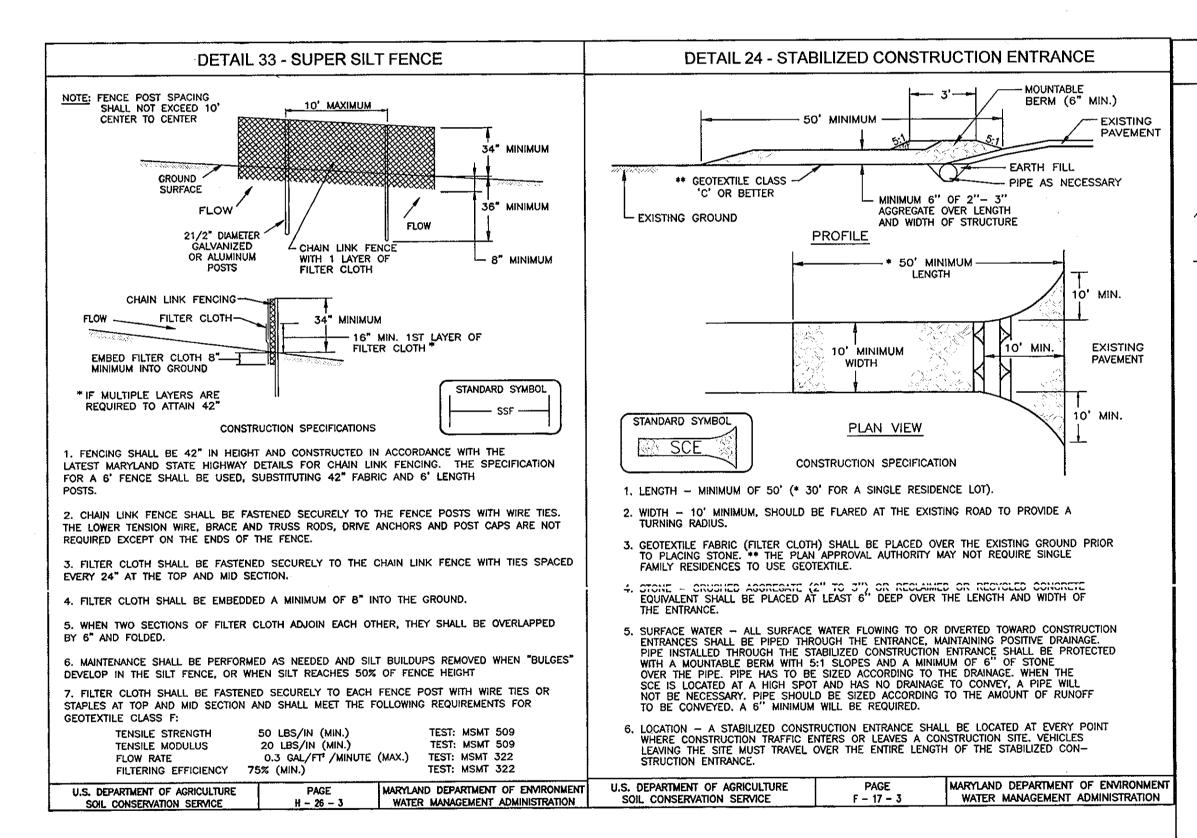
DURING GRADING AND AFTER EACH RAINFALL, THE CONTRACTOR

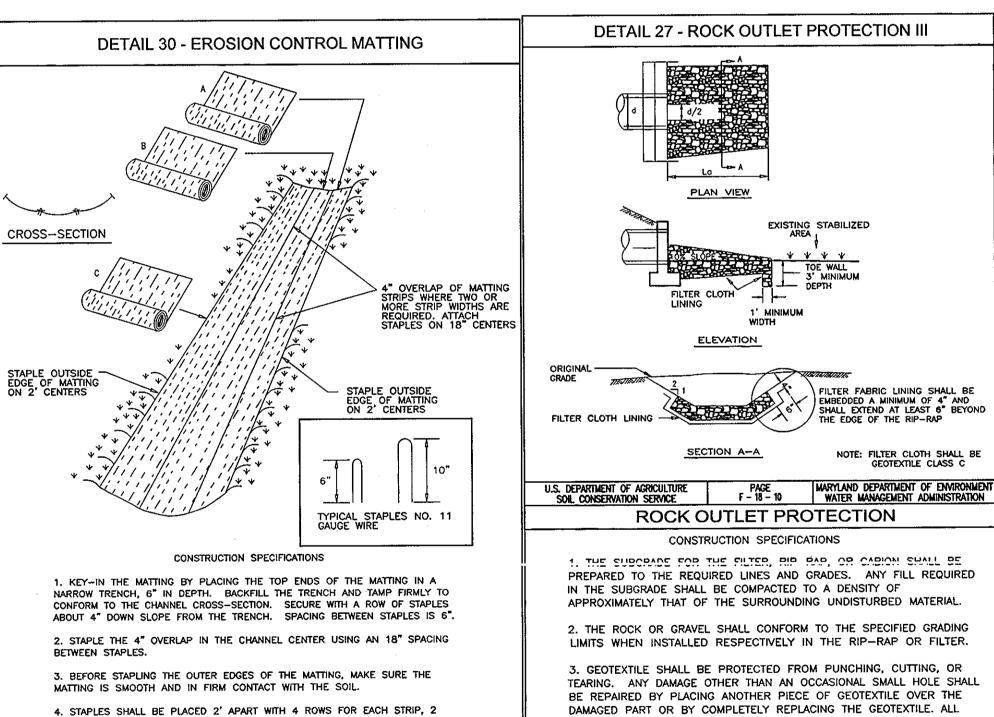
SHALL INSPECT AND PROVIDE THE NECESSARY MAINTENANCE ON THE SEDIMENT AND EROSION CONTROL MEASURES SHOWN

FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLIED

A. 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, SWALES, DITCH PERIMETER SLOPES SLOPES AND ALL SLOPES GREATER THAN 3:1.

B. 14 CALENDAR DAYS FOR ALL OTHER DISTURBED AREAS.





DAMAGED PART OR BY COMPLETELY REPLACING THE GEOTEXTILE. ALL OVERLAPS WHETHER FOR REPAIRS OR FOR JOINING TWO PIECES OF GEOTEXTILE SHALL BE A MINIMUM OF ONE FOOT.

4. STONE FOR THE RIP-RAP OR GABION OUTLETS MAY BE PLACED BY EQUIPMENT. THEY SHALL BE CONSTRUCTED TO THE FULL COURSE THICKNESS IN ONE OPERATION AND IN SUCH A MANNER AS TO AVOID DISPLACEMENT OF UNDERLYING MATERIALS. THE STONE FOR RIP-RAP OR GABION OUTLETS SHALL BE DELIVERED AND PLACED IN A MANNER THAT WILL ENSURE THAT IT IS REASONABLY HOMOGENEOUS WITH THE SMALLER STONES AND SPALLS FILLING THE VOIDS BETWEEN THE LARGER STONES. RIP-RAP SHALL BE PLACED IN A MANNER TO PREVENT DAMAGE TO THE FILTER BLANKET OR GEOTEXTILE. HAND PLACEMENT WILL BE REQUIRED TO THE EXTENT NECESSARY TO PREVENT DAMAGE TO THE PERMANENT WORKS.

5. THE STONE SHALL BE PLACED SO THAT IT BLENDS IN WITH THE EXISTING GROUND. IF THE STONE IS PLACED TOO HIGH THEN THE FLOW WILL BE FORCED OUT OF THE CHANNEL AND SCOUR ADJACENT TO THE STONE WILL OCCUR.

U.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT SOIL CONSERVATION SERVICE | F - 18 - 84 | WATER MANAGEMENT ADMINISTRATION

OWNER / DEVELOPER

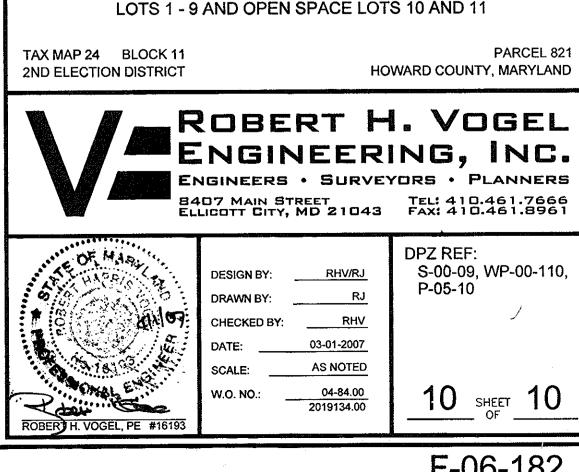
WILLIAMSBURG GROUP

COLUMBIA, MARYLAND 21044

(410) 997-8800

SEDIMENT AND EROSION **CONTROL DETAILS** TIBER RIDGE TAX MAP 24 BLOCK 11 2ND ELECTION DISTRICT 5485 HARPERS FARM ROAD, SUITE 200

NO.



REVISION

DATE