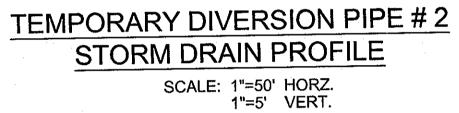
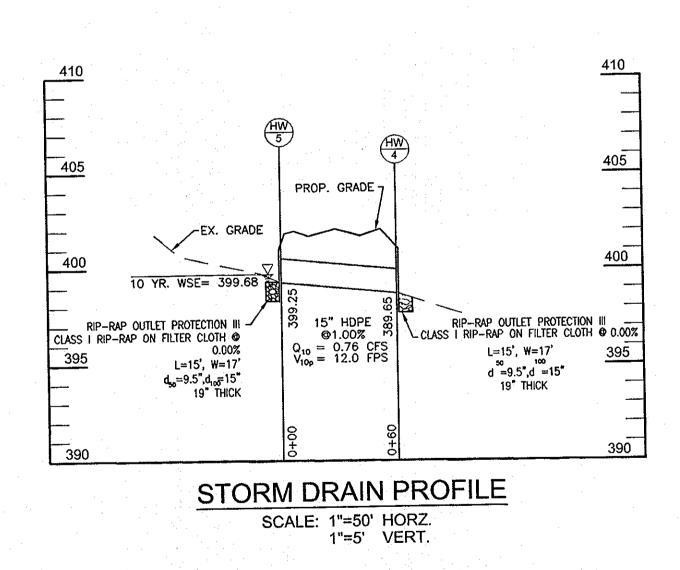
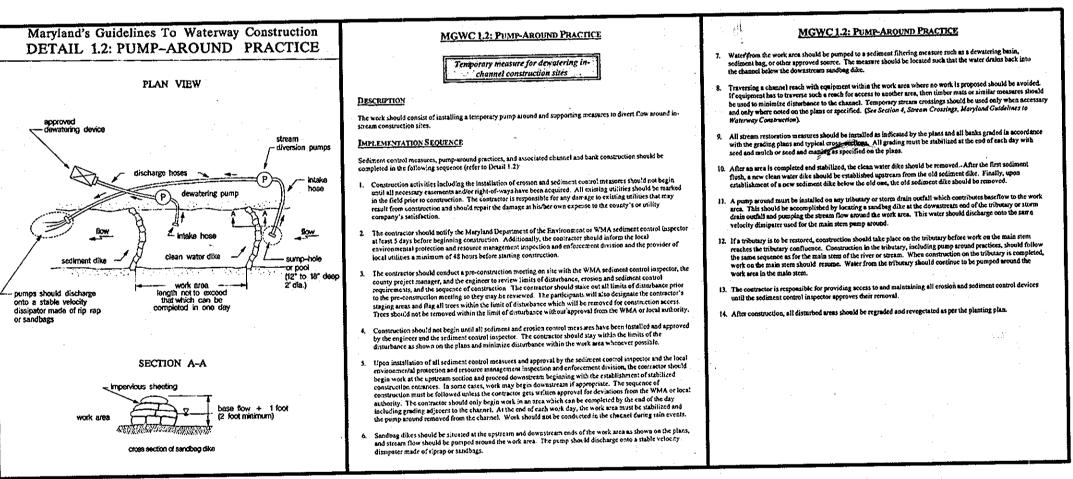


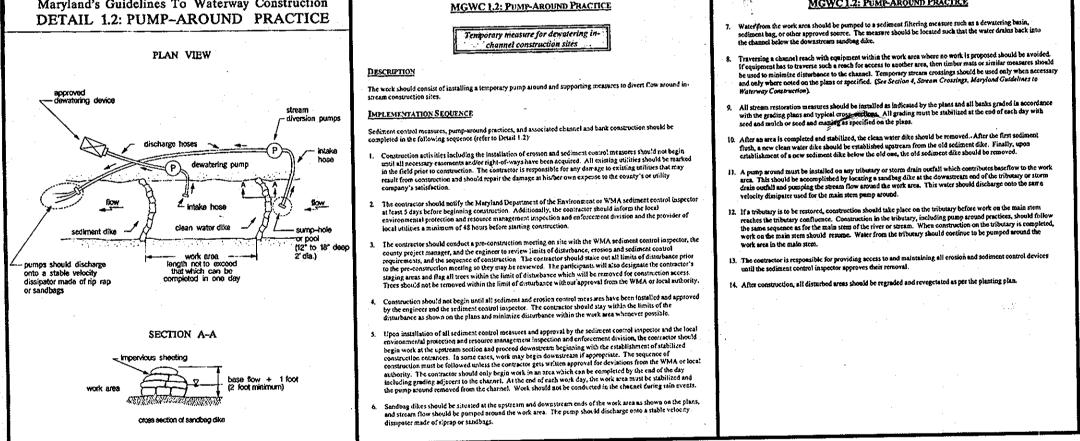
SCALE: 1"=50' HORZ. 1"=5' VERT.

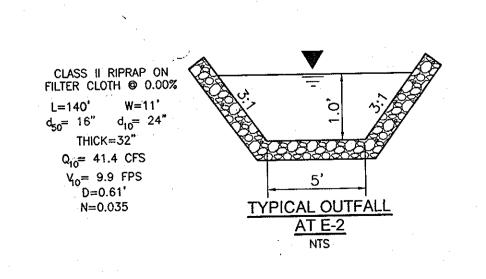


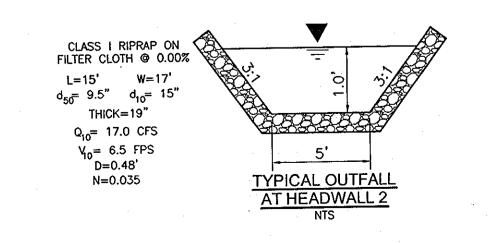


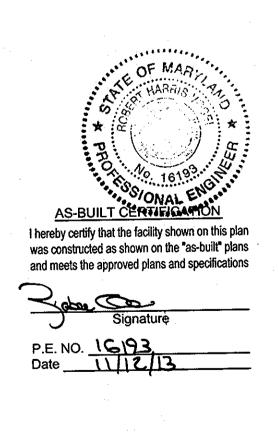


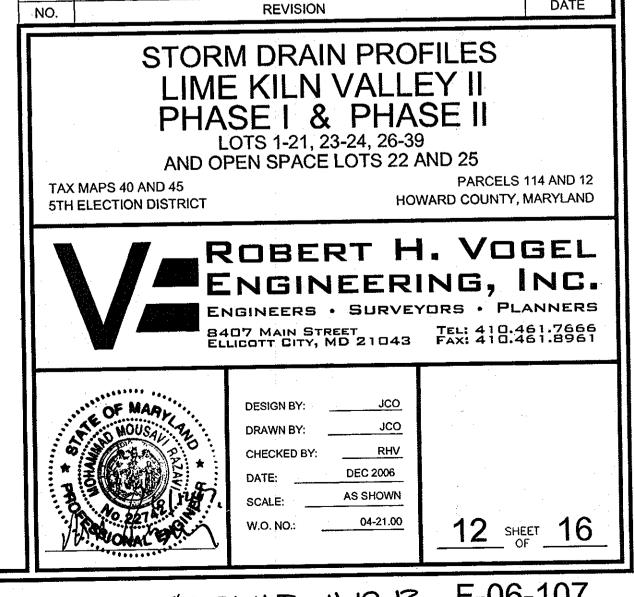






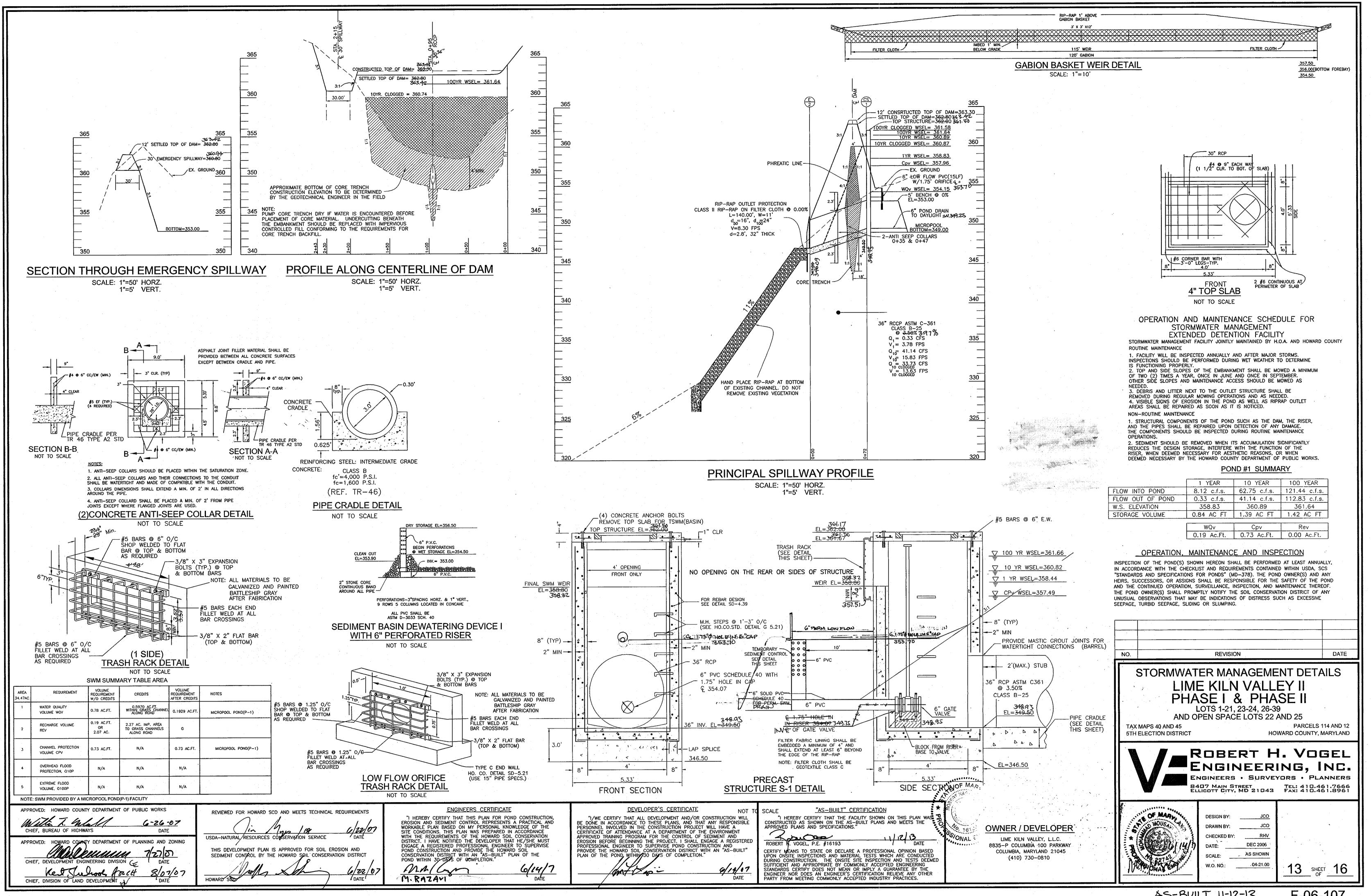






APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS 6-26-07

OWNER / DEVELOPER LIME KILN VALLEY, L.L.C. 8835-P COLUMBIA 100 PARKWAY COLUMBIA, MARYLAND 21045 (410) 730-0810



MARYLAND 378

STORMWATER MANAGEMENT POND CONSTRUCTION SPECIFICATIONS

These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent version.

Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the tow of the

Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

Material — The fill material shall be taken from approved designated borrow areas. It shall be free of roots, stumps, wood, rubbish, stones greater than 6°, frozen or other objectionable materials. Fill material for the center of the embankment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer. Materials used in the outer shell of the embankment must have the capability to support vegetation of the

Placement – Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment.

Compaction - The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not be so

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within $\pm \sqrt{-2\%}$ of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

Cut Off Trench — The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with

Embankment Core — The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed currently with the outer shell of the outer shell of the embankment.

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operated closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2,000 ohm-cm. Material shall be placed such that minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sides of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal avers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of the structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24" or greater over the structure or pipe. Backfill (flowable fill)zone shall be of the type and quality conforming to that specified for the core of the embankment or other embankment materials.

All pipes shall be circular in cross section.

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

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APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

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CHIEF, DIVISION OF LAND DEVELOPMENT N

CHIEF. BUREAU OF HIGHWAYS

Corrugated Metal Pipe - All of the following criteria shall apply for corrugated metal pipe:

1. Materials — (Polymer Coated steel pipe)— Steel pipes with polymeric coating shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 & M-246 with watertight coupling bands or flanges.

Materials — (Aluminum Coated Steel Pipe) — This pipe and its appurtenances shall conform to the requirements of ASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged o otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.

Materials — (Aluminum Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M—196 or M—211 with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be pointed with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be between 4 and 9.

2. Coupling, bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at lease

Connections - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.

All connections shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be rerolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches diameter: flanges on both ends of the pipe with a circular 3/8 inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected to a 24 inch long annular corrugated band using a minimum of 4(four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8'inch closed cell gaskets the full width of the flange is also acceptable.

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or

4. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

Backfilling shall conform totructure Backfill . Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete pipe:

Materials — Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or

Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in the "Structure Backfill" section of this standard. Gravel

3. Laying pipe — Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

Other details (anti-seep collars, valves, etc.) shall be shown on the drawings.

Plastic Pipe — The following criteria shall apply for plastic pipe:

1. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241.

Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" -10" inch pipe shall meet the MASHTO M252 Type S, and 12" through 24" inch shall meet the requirements of AASHTO M294 Type S.

2. Joints and connections to anti-seep collars shall be completely watertight.

3. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

4. Backfilling shall conform totructure Backfill "

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5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings

Drainage Diaphragms — When a drainage diaphragm is used, a registered professional engineer will supervise the

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3.

Rock riprop shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction Materials, Section 311.

Geotexile shall be placed under all riprap and shall meet requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the excavations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall be pumped.

All borrow areas shall be graded to provide proper drainage and left I a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

Erosion and Sediment Control

REVIEWED FOR HOWARD SCD AND MEETS TECHNICAL REQUIREMENTS

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND

SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT

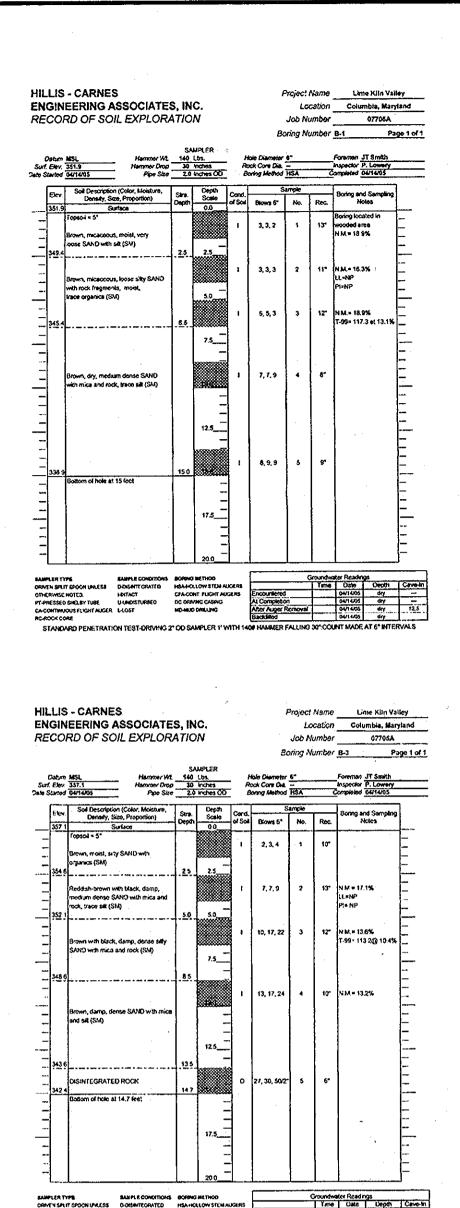
USDA-NATURAL RESOURCES CONSERVATION SERVICE

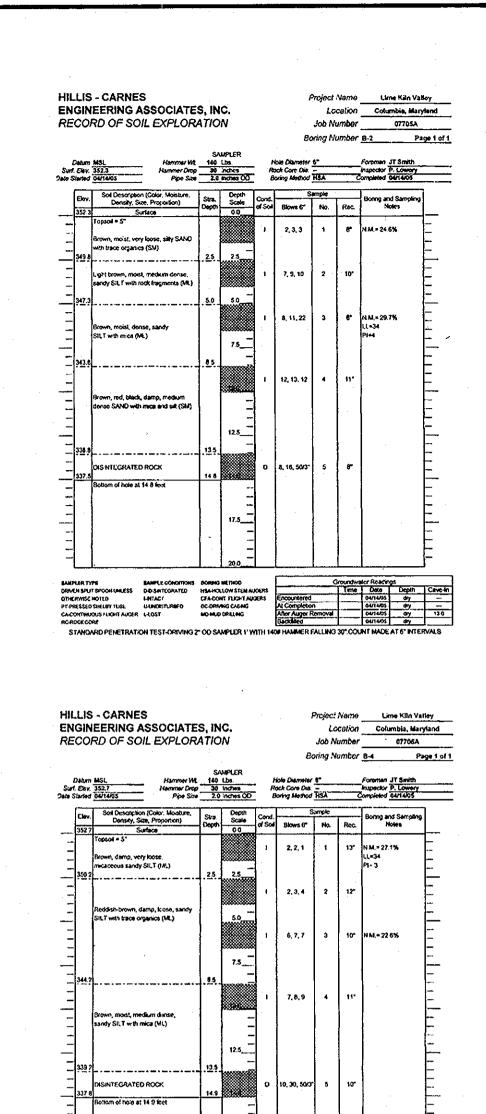
Construction operations will be carried out in such a manner that erosion will be controlled and water and oir pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures.

POND BOTTOM SOIL CONDITIONS

If broken rock fragments are encountered at finished pond bottom, under cut a minimum of 12" below basin grade and to a horizontal distance of at least 18," beyond each edge of the broken rock and backfill with fine—grained ML or CL soils compacted to a firm condition. This procedure should be performed under the supervision of the project Geotechnical Engineer.

1mm





GEOTECHNICAL RECOMMENDATIONS

Lime Kiln Valley II SWM April 25, 2005

EVALUATION AND RECOMMENDATIONS

Hillis-Carnes Job No. 07705A

We have considered the general infiltration potential of the project soils with respect to groundwater recharge at the site. We estimate from the laboratory test results that the USDA textural classifications of the soils are Loam and Sandy Loam indicating a reasonable potential for infiltration. The Maryland Stormwater Design Manual (2000) assigns infiltration values of 1.02 inches per hour and 0.52 inches per hour to Sandy Loam and Loam, respectively. These values are applicable for the Recharge Volume (Rev) in stormwater capacity calculations. The use of infiltration as a technique to accommodate the Water Quality Volume (WQv) requires field infiltration testing per

SLOPE STABILITY

Pond cut slopes should be set no steeper than 3(Horizontal):1(Vertical). The silty project soils are susceptible to erosion, therefore we recommend the use of surface stabilization techniques such as erosion control matting while vegetation is established. Cut and fill slopes not subject to a pond elevation should be set no steeper than

EVALUATION OF ON-SITE SOILS AS FILL MATERIAL

The on-site soils excavated in the SWM facility area should be suitable for use as general embankment material or structural fill. The soils are generally not suitable for use in seepage control or cutoff applications. All compacted fill should be placed in horizontal layers, maximum 8-inch loose lift thickness and compacted to at least 95 percent of the maximum dry density per ASTM D 698 at a moisture content within two percentage points of the D 698 optimum value.

The high mica content of the soils generally indicates low California Bearing Ratio (CBR) values and poor subgrade support in pavement applications. Subgrade stabilization may be necessary to achieve a stable subgrade for pavements.

Excavations for the SWM facility will not likely encounter disintegrated rock or intact rock to reach proposed bottom elevations. Excavations that extend into disintegrated rock may encounter zones of hard and/or intact rock. We note that geotechnical drilling equipment used in our exploration is sometimes capable of penetrating material that would not be rippable using conventional excavating equipment.

Rock excavation quantities are frequently an issue of contention, therefore the following definition of rock is provided for general use at this site. Rock is defined as any material

HILLIS-CARNES

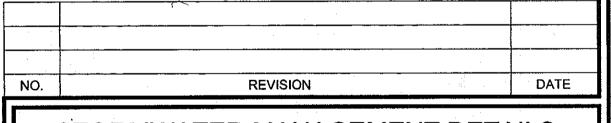
Lime Kiin Valley II SWM

Page 5 of 6 Hillis-Carnes Job No. 07705A

which cannot be dislodged by a Caterpillar D-8 tractor with a hydraulic ripper (or a Caterpillar 235 excavator with a rock bucket), or equivalent, without the use of blasting. Excavation of boulders or masses of rock exceeding one cubic yard in volume should also be considered rock excavation.

We recommend an unclassified earthwork specification. The unclassified excavation pays for any and all excavation on either a lump sum or single unit price basis. Minimal record keeping is required for the unclassified specification; however the contractor assumes much of the risk for variability in subsurface conditions and may result in an increased cost contingency in the bid. The unclassified classification is appropriate if an up front excavation cost is desired. Alternatively, the classified Rock excavation specification pays for Rock excavation on a unit rate basis.

We anticipate that control of groundwater will not be a significant issue during excavation, and surface runoff accumulating in the excavations may be controlled by sump pits and pumps. Any unsuitable or disturbed soils should be removed and replaced with compacted fill. Whenever possible, earthwork should be scheduled in the drier months (June to October) to reduce the amount of dewatering and additional construction costs.



STORMWATER MANAGEMENT DETAILS LIME KILN VALLEY II PHASE I & PHASE II

LOTS 1-21, 23-24, 26-39 AND OPEN SPACE LOTS 22 AND 25

TAX MAPS 40 AND 45 **5TH ELECTION DISTRICT**

PARCELS 114 AND 12 HOWARD COUNTY, MARYLAND



ENGINEERING, INC. ENGINEERS . SURVEYORS . PLANNERS 8407 MAIN STREET TEL: 410.461.7666 ELLICOTT CITY, MD 21043 FAX: 410.461.8961

DESIGN BY:

DRAWN BY: **DEC 2006** AS SHOWN SCALE: 04-21.00 W.O. NO.:

14 SHEET

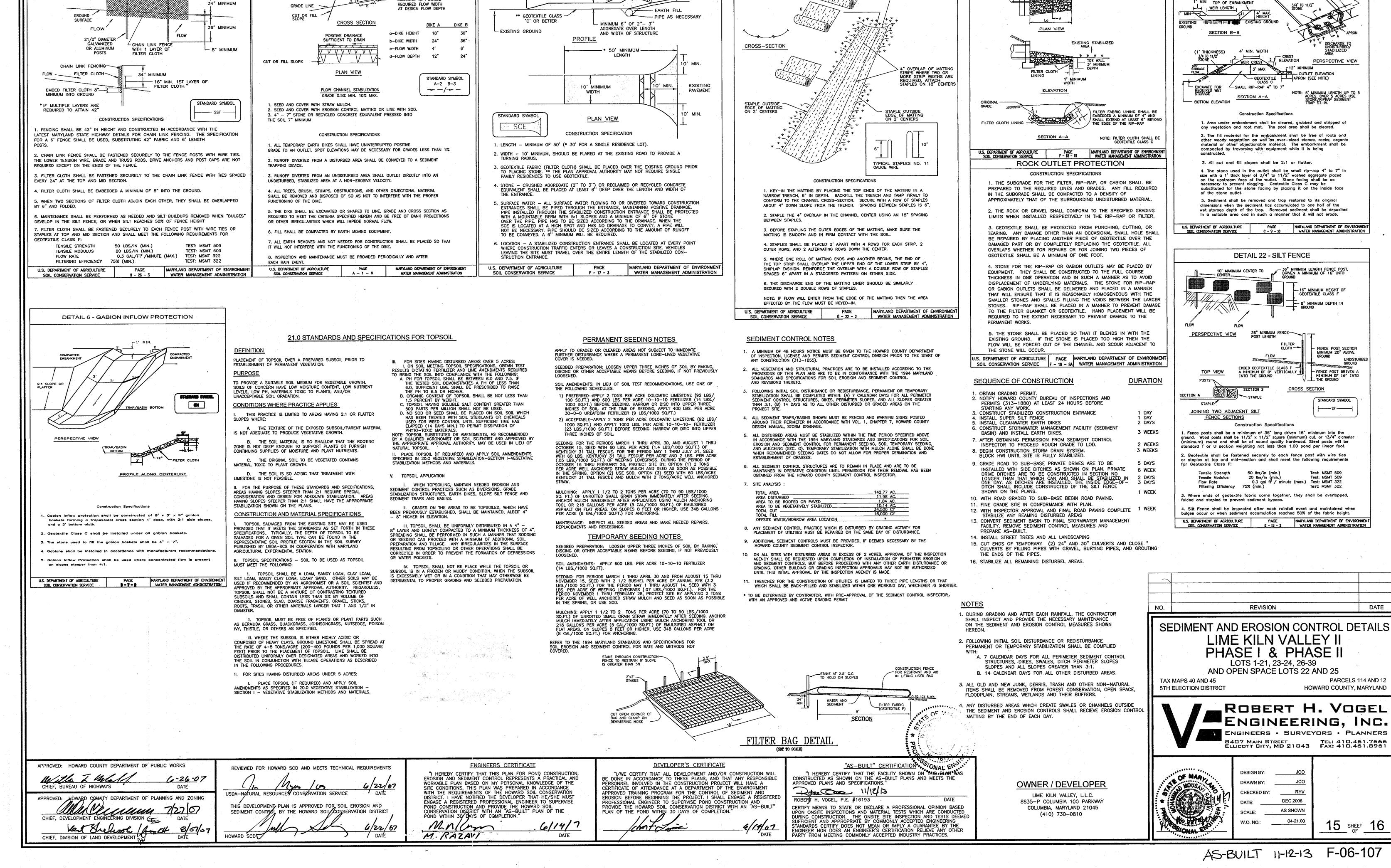
DEVELOPER'S CERTIFICATE ENGINEERS CERTIFICATE "AS-BUILT" CERTIFICATION "I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN WAS CONSTRUCTED AS SHOWN ON THE AS-BUILT PLANS AND MEETS THE APPROVED PLANS AND SPECIFICATIONS." "I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE IN ACCORDANCE TO THESE PLANS, AND THAT ANY RESPONSIBLE "I HEREBY CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, "I HEREBY CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION." INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT ROBERT H. VOGEL, P.E. #16193 APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION." CERTIFY MEANS TO STATE OR DECLARE A PROFESSIONAL OPINION BASED CERTIFY MEANS TO STATE OR DECLARE A PROFESSIONAL OPINION BASED UPON ONSITE INSPECTIONS AND MATERIAL TESTS WHICH ARE CONDUCTED DURING CONSTRUCTION. THE ONSITE SITE INSPECTION AND TESTS DEEMED SUFFICIENT AND APPROPRIATE BY COMMONLY ACCEPTED ENGINEERING STANDARDS CERTIFY DOES NOT MEAN OR IMPLY A GUARANTEE BY THE ENGINEER NOR DOES AN ENGINEER'S CERTIFICATION RELIEVE ANY OTHER PARTY FROM MEETING COMMONLY ACCEPTED INDUSTRY PRACTICES. an M. RAZAVI

LIME KILN VALLEY, L.L.C.

8835-P COLUMBIA 100 PARKWAY

COLUMBIA, MARYLAND 21045

(410) 730-0810



DETAIL 24 - STABILIZED CONSTRUCTION ENTRANCE

BERM (6" MIN.)

PAVEMEN[®]

DETAIL 33 - SUPER SILT FENCE

NOTE: FENCE POST SPACING

SHALL NOT EXCEED 10'

CENTER TO CENTER

DETAIL 1 - EARTH DIKE

2:1 SLOPE OR FLATTER

2:1 SLOPE OR FLATTER

EXCAVATE TO PROVIDE

DETAIL 9 - STONE OUTLET SEDIMENT TRAP - ST II

DETAIL 27 - ROCK OUTLET PROTECTION III

DETAIL 30 - EROSION CONTROL MATTING

