# TRIADELPHIA CROSSING

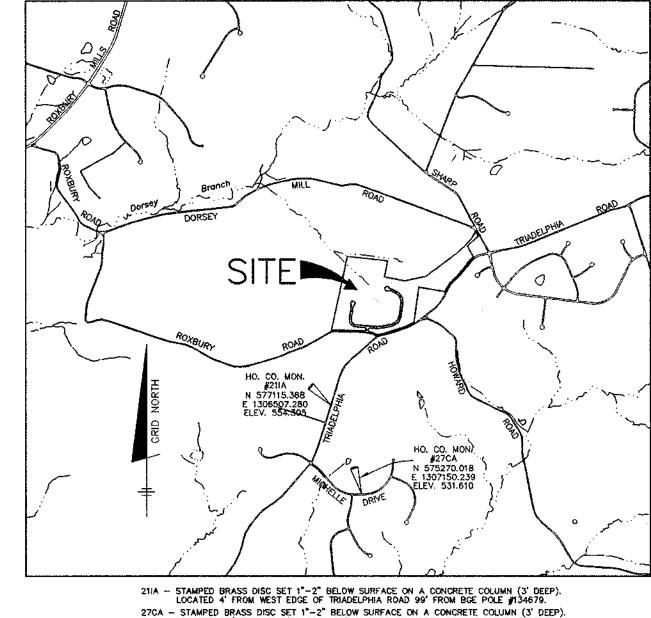
# LOTS 1 - 22 AND NON-BUILDABLE PRESERVATION PARCELS 'A' - 'H' AND NON-BUILDABLE BULK PARCEL 'I'

# ROADS, STORMWATER MANAGEMENT AND STORM DRAIN CONSTRUCTION PLANS



PLAN VIEW

SCALE: 1" = 200'



27CA - STAMPED BRASS DISC SET 1"-2" BELOW SURFACE ON A CONCRETE COLUMN (3' DEEP)
LOCATED 2.8' FROM PK NAIL IN MACADAM CURB ON NORTH SIDE OF MICHELLE DRIVE.
(APPROX. 158' FROM SAPLING DRIVE).

## SITE ANALYSIS DATA CHART

1.) PRESENT ZONING:	RC-DEO
2.) APPLICABLE DPZ FILE REFERENCES:	S-01-23, F-98-29, P-0
3.) PROPOSED USE OF SITE:	
4.) PROPOSED WATER AND SEWER SYSTEMS:	(SFD) PRIVATE
AREA TABULATION	
1.) GROSS TRACT AREA	62.31±
2.) AREA WITHIN 100-YEAR FLOODPLAIN	1.81 AC.±
3.) TOTAL AREA OF 25% OR GREATER STEEP SLO AREA NOT IN FLOODPLAIN (FOR NTA C	PES 0.00 AC.±
4.) NET TRACT AREA	
5.) TOTAL NUMBER OF LOTS ALLOWED PER ZONIN 1 UNIT PER 4.25 GROSS ACRES 1 UNIT PER 2 NET ACRES (MAX)	
6.) TOTAL NUMBER OF RESIDENTIAL UNITS/LOTS PROPOSED ON THIS SUBMISSION (POTENTIAL 5 ADDITIONAL LOTS IN PHASE 2)	
7.) AREA OF CLUSTER LOTS	
8.) AREA OF NON-BUILDABLE PRESERVATION PAR (PARCELS A - H)	CELS 30.02 AC.±
9.) AREA OF BUILDABLE PRESERVATION PARCELS	N/A
10.) AREA OF NON-BUILDABLE BULK PARCELS(PARCEL i)	5.23 AC.±
11.) AREA OF BUILDABLE BULK PARCELS	N/A
12.) AREA OF ROAD RIGHT-OF-WAY	4.49 AC.±
13.) OPEN SPACE ON-TOTAL SITE *	N/A
14.) AREA OF RECREATIONAL OPEN SPACE REQUIRE	ED N/A

**BENCHMARK** ENGINEERS A LAND SURVEYORS A PLANNERS

TOLL BROTHERS, INC.

SUITE 230

410-872-9105

DESIGN: DBT DRAFT: DBT CHECK: DAM

STORM DRAIN DRAINAGE AREA MAP

STORM DRAIN PROFILES AND DETAILS

STORM DRAIN PROFILES

GRADING, SEDIMENT AND EROSION CONTROL PLAN

GRADING, SEDIMENT AND EROSION CONTROL PLAN

GRADING, SEDIMENT AND EROSION CONTROL PLAN

SWM DETAILS - EXTENDED DETENTION FACILITY

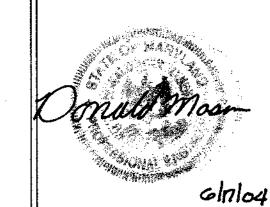
SWM DETAILS - DISCONNECTION CREDIT PLAN

FOREST CONSERVATION PLAN

FOREST CONSERVATION PLAN

WM NOTES AND SEDIMENT & EROSION CONTROL NOTES & DETAILS

ENGINEERING, INC 8480 BALTIMORE NATIONAL PIKE ▲ SÚITE 418 ELLICOTT CITY, MARYLAND 21043 phone: 410-465-6105 ▲ fax: 410-465-6644



email: Benchmrk@cais.com TRIADELPHIA CROSSING EDGEWOOD FARM, INC. 14919 ROXBURY ROAD LOTS 1-22 AND NON-BUILDABLE PRESERVATION PARCELS 'A' THRU 'H AND NON-BUILDABLE BULK PARCEL 'I' GLENELG, MARYLAND 21737

SCALE:

AP: 21 PARCEL: 97
17 & 23 ZONED: RC-DEO
FOURTH ELECTION DISTRICT HOWARD COUNTY, MARYLAND 7164 COLUMBIA GATEWAY DRIVE TITLE SHEET COLUMBIA, MARYLAND 21046

JUNE, 2004

SHEET <u>1</u> OF <u>17</u> F-04-118

PROJECT NO. 1634

Willin I. Malu /

CHIEF, DEVELOPMENT ENGINEERING DIVISION MAJ

GENERAL NOTES

3.) THE COORDINATES SHOWN HEREON ARE BASED UPON THE HOWARD COUNTY GEODETIC CONTROL WHICH IS BASED UPON THE MARYLAND STATE PLANE COORDINATE SYSTEM. HOWARD COUNTY MONUMENT NOS. 211A AND 27CA WERE USED FOR THIS PROJECT.

6.) STORMWATER MANAGEMENT SHALL BE PROVIDED BY AN EXTENDED DETENTION POND WITH MICRO-POOL, BIO-RETENTION FACILITY AND RAINGARDENS/DRYWELLS. SWM FACILITIES ARE TO BE PRIVATELY OWNED AND MAINTAINED.

IN MAY, 2001 FOR BORINGS 1, 2 AND 3. THE GEOTECHNICAL REPORT FOR BORINGS 4, 5, 6 AND 7 AND THE ADDITIONAL IN-SITU TESTING OF BORINGS 1, 2 AND 3 WAS PREPARED BY HILLIS CARNES ENGINEERING ASSOCIATES, INC. IN OCTOBER, 2003.

8.) THE FLOODPLAIN STUDY FOR THIS PROJECT WAS PREPARED BY BENCHMARK ENGINEERING, INC.,

THEIR BUFFERS OR STEEP SLOPES THAT ARE AT LEAST 20,000 S.F. UNLESS APPROVED BY THE DEPARTMENT OF PLANNING AND ZONING.

16.) BOUNDARY IS BASED ON A FIELD RUN MONUMENTED BOUNDARY SURVEY PERFORMED DURING SEPTEMBER, 2000 BY CLARK, FINEFROCK & SACKETT, INC.

18.) THIS PLAN IS SUBJECT TO COMPLIANCE WITH THE FOURTH EDITION OF THE SUBDIVISION REGULATIONS AS A CONSEQUENCE FOR ITS SUBMISSION PRIOR TO 11-15-2001. THIS PROJECT IS SUBJECT TO COMPLIANCE WITH COUNTY COUNCIL BILL 50-2001 WHICH AMENDS PORTIONS OF THE ZONING REGULATIONS AS A CONSEQUENCE FOR NOT HAVING PRELIMINARY PLAN APPROVAL PRIOR TO 11-1-2001.

OF A MODIFIED SEWERAGE EASEMENT PLAT SHALL NOT BE NECESSARY.

THIS AREA DESIGNATES A PRIVATE SEWERAGE EASEMENT OF 10,000 SQUARE FEET AS REQUIRED BY THE STATE DEPARTMENT OF THE ENVIRONMENT FOR INDIVIDUAL SEWERAGE DISPOSAL. IMPROVEMENTS OF ANY NATURE IN THIS AREA IS RESTRICTED UNTIL PUBLIC SEWER IS AVAILABLE. THIS EASEMENT SHALL BECOME NULL AND VOID UPON CONNECTION TO A PUBLIC SEWERAGE SYSTEM. THE COUNTY HEALTH OFFICER SHALL HAVE THE AUTHORITY

SENSITIVE AREAS INCLUDING STEEP SLOPES, STREAM BUFFERS, WETLANDS, FLOODPLAIN AND EXISTING FOREST, IT WILL BE DEDICATED TO THE HOWARD COUNTY DEPARTMENT OF RECREATION AND PARKS, IT IS ENCUMBERED BY AN EASEMENT AGREEMENT WITH THE HOMEOWNERS ASSOCIATION, THIS AGREEMENT PROHIBITS FURTHER SUBDIVISION OF THE PARCEL, OUTLINES THE MAINTENANCE RESPONSIBILITIES OF ITS OWNER AND ENUMERATES THE USES PERMITTED ON THE PROPERTY.

AND EXISTING FOREST, THEY WILL BE PRIVATELY OWNED. THEY ARE ENCUMBERED BY AN EASEMENT AGREEMENT WITH HOWARD COUNTY AND THE HOMEOWNERS ASSOCIATION. THIS AGREEMENT PROHIBITS FURTHER SUBDIVISION OF THE PARCEL, OUTLINES THE MAINTENANCE RESPONSIBILITIES OF ITS OWNER AND ENUMERATES THE USES PERMITTED ON THE PROPERTY.

PRESERVATION PARCELS 'F' AND 'G' ARE PROPOSED AS NON-BUILDABLE PARCELS FOR STORMWATER MANAGEMENT FACILITIES AS A REQUIREMENT TO CONTROL STORMWATER RUNOFF. THEY WILL BE OWNED BY THE HOMEOWNERS ASSOCIATION. THEY ARE ENCUMBERED BY AN EASEMENT AGREEMENT WITH HOWARD COUNTY. THIS AGREEMENT PROHIBITS FURTHER SUBDIVISION OF THE PARCEL, OUTLINES THE

BULK PARCEL 'I' IS PROPOSED AS A BULK PARCEL FOR A FUTURE RESUBDIMISION UNDER PHASE 2

21.) THE TOTAL FOREST CONSERVATION OBLIGATION AMOUNT OF 25.7 ACRES HAS BEEN MET BY THE ON-SITE RETENTION OF 6.6 AC. WITHIN A FOREST CONSERVATION EASEMENT AND THE ON-SITE REFORESTATION OF 15.4 AC. WITHIN A FOREST CONSERVATION EASEMENT AND BY THE OFFSITE REFORESTATION OF 3.7 ACRES WITHIN A FOREST CONSERVATION EASEMENT LOCATED ON PRESERVATION PARCEL 'A' OF THE CLARKS WOODS 1 (F-04-119) SUBDIVISION WITH A DPW, DEVELOPER'S AGREEMENT WITH SURETY IN THE AMOUNT OF \$473,497.00 (\$392,911.00 ON-SITE AND \$80,586.00 OFF-SITE).

22.) THE CREATION OF CLARKS WOODS II, LOTS 1-3 NECESSITATED THE FUTURE CREATION OF A 9.38 ACRE PRESERVATION PARCEL (3 x 4.25 = 12.75 - 3.37 = 9.38) PER SECTION 104.E.1.g OF THE ZONING ORDINANCE UPON THE RESUBDIVISION OF THE RESIDUE OF PARCEL 97. THAT 9.38 ACRES IS INCLUDED

24.) THE DOWNSPOUTS FOR THE HOMES LOCATED ON LOTS 17 & 18 AND FUTURE LOTS 23 - 27 SHALL BE CONSTRUCTED AS SHOWN ON SHEET 15 OF THESE CONSTRUCTION PLANS. THE ROOFTOP DISCONNECTION DESIGN IS BASED ON A TOTAL ROOFTOP SQUARE FOOTAGE OF 3,960 S.F. ANY ADDITIONAL SQUARE FOOTAGE OF ROOFTOP (BASED ON WHAT TYPE OF HOUSE IS SOLD ON EACH LOT) OTHER THAN WHAT IS SHOWN ON

23.) ALL DRIVEWAY CULVERTS ARE TO BE 15" HOPEP, DESIGN CALCULATIONS ARE PROVIDED IN THE

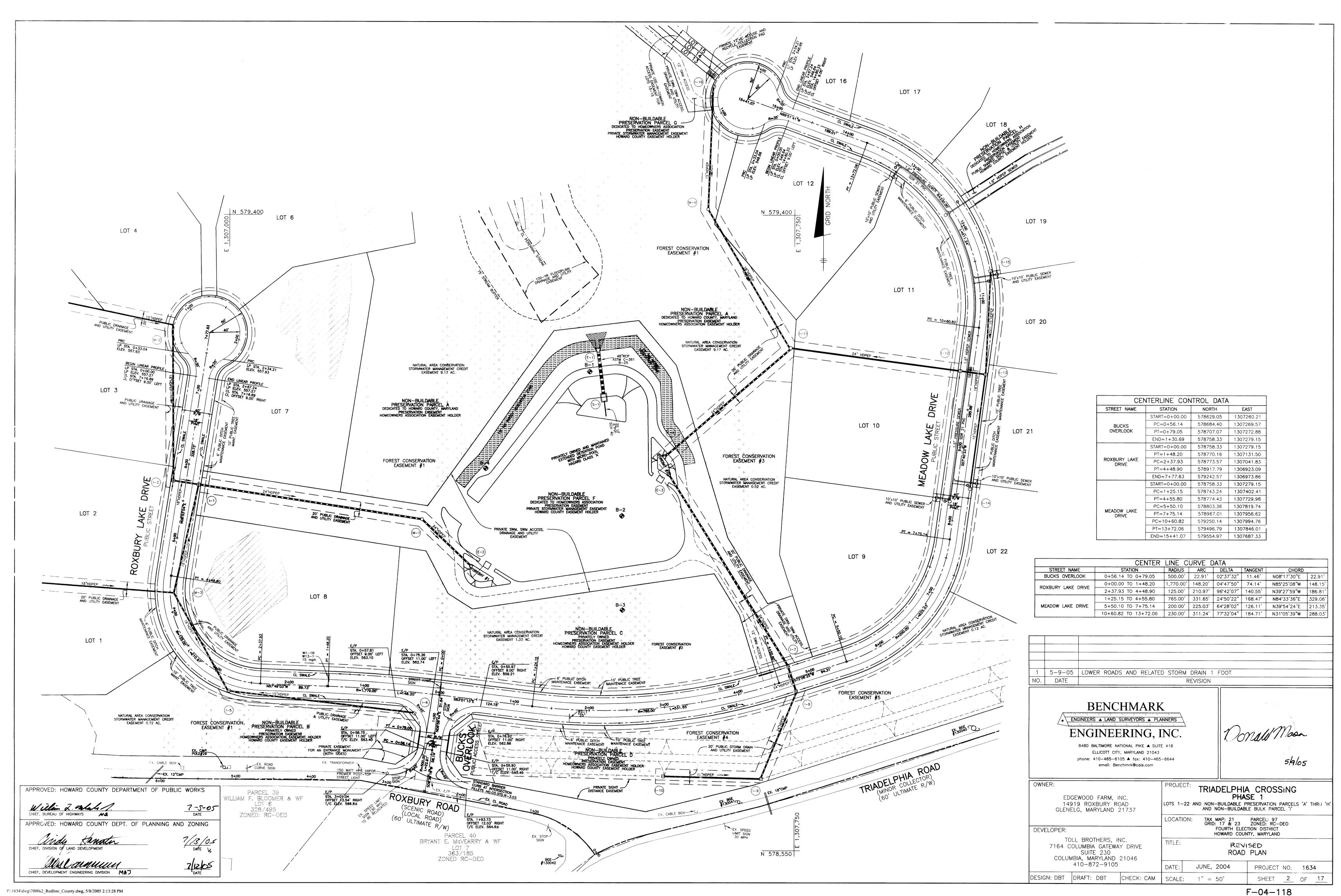
SHEET 15 SHALL REQUIRE ADDITIONAL DOWNSPOUTS (1 FOR EVERY 500 S.F.).

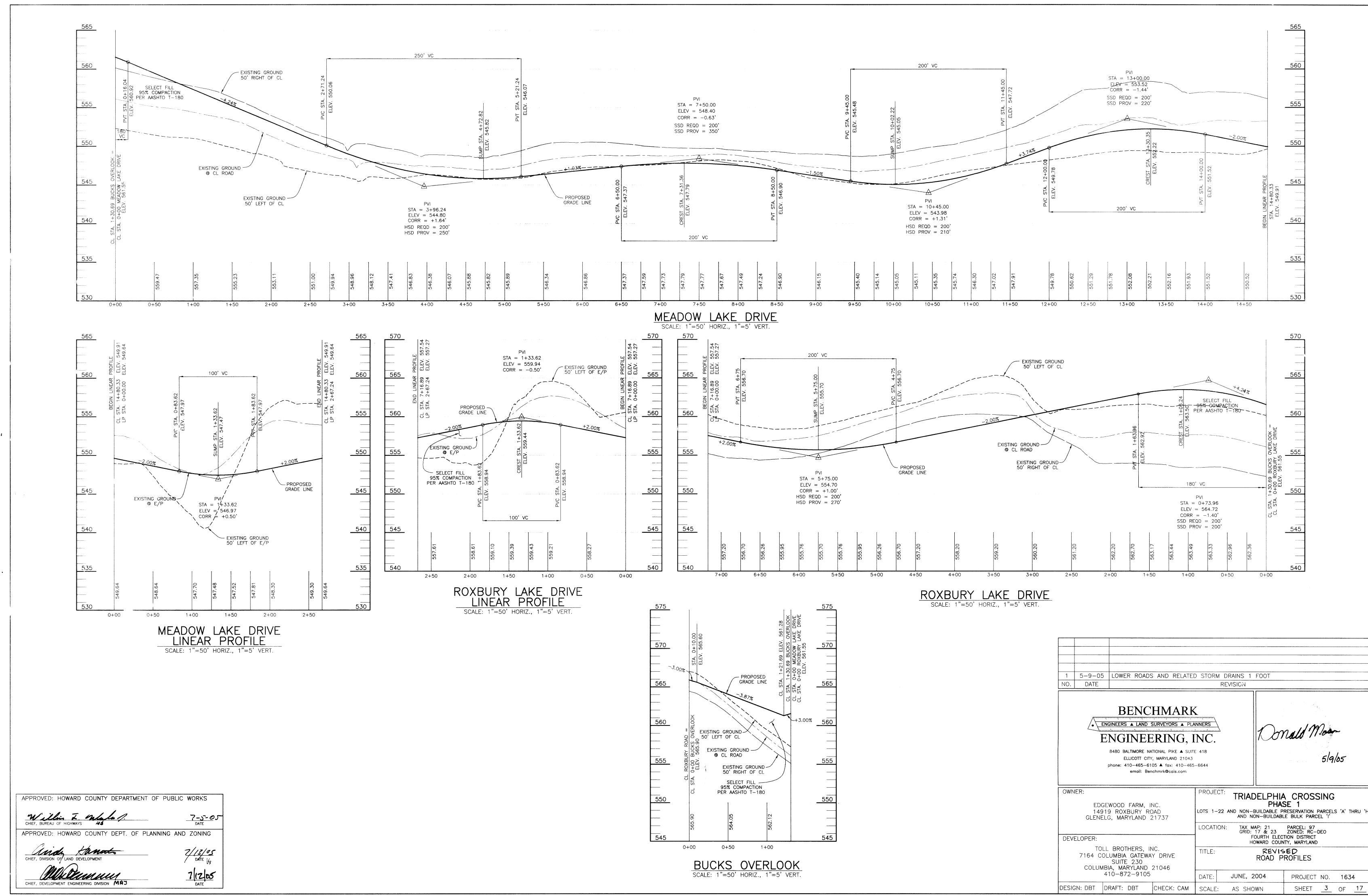
APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

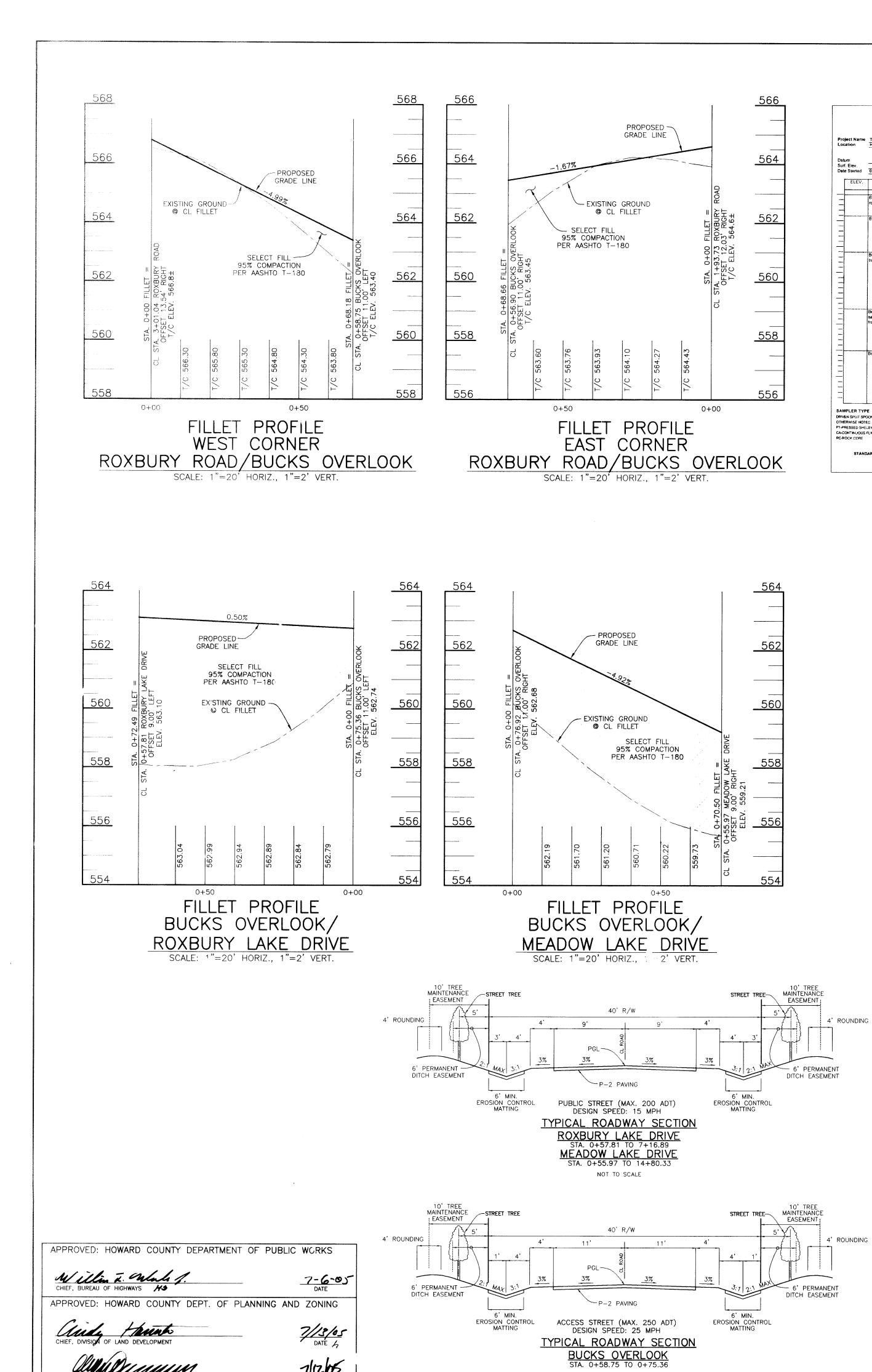
APPROVED: HOWARD COUNTY DEPT. OF PLANNING AND ZONING

8-2-04

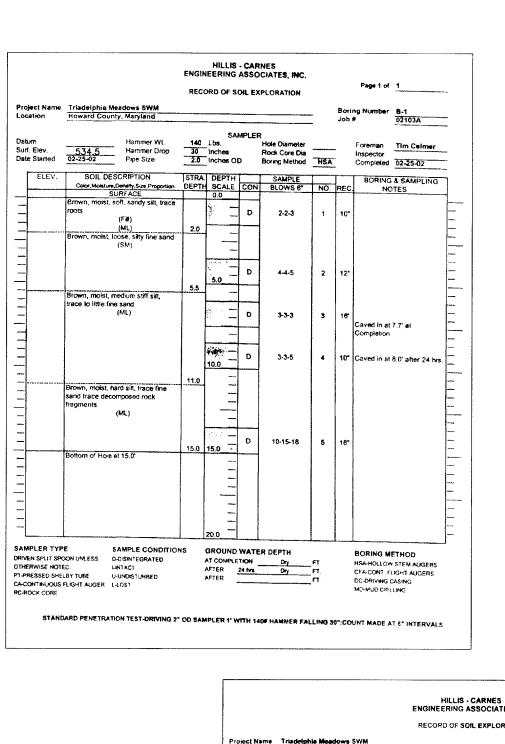
PRESERVATION PARCEL 'H' IS PROPOSED AS NON-BUILDABLE PARCEL FOR A SHARED SEPTIC FIELD IN ORDER TO PROVIDE SEWAGE DISPOSAL FOR LOTS 9-12 AND 19-22. IT WILL BE DEDICATED TO THE HOMEOWNERS ASSOCIATION. IT IS ENCUMBERED BY AN EASEMENT AGREEMENT WITH HOWARD COUNTY. THIS AGREEMENT PROHIBITS FURTHER SUBDIVISION OF THE PARCEL, OUTLINES THE MAINTENANCE RESPONSIBILITIES OF ITS OWNER AND ENUMERATES THE USES PERMITTED ON THE PROPERTY.

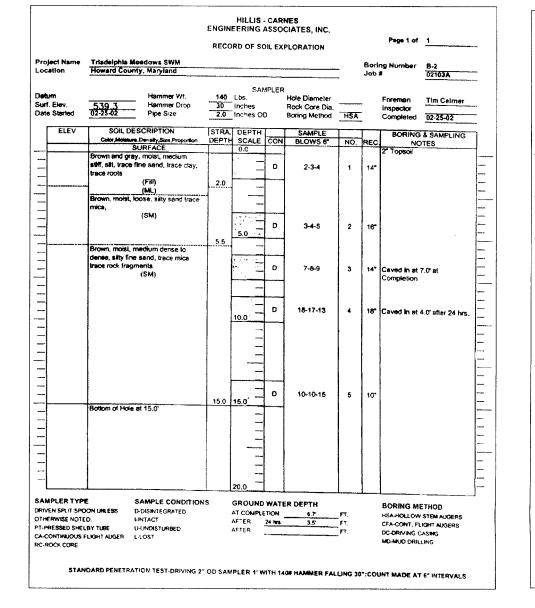


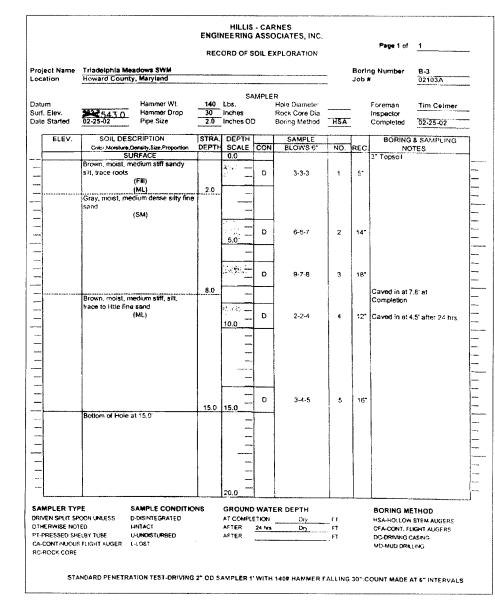


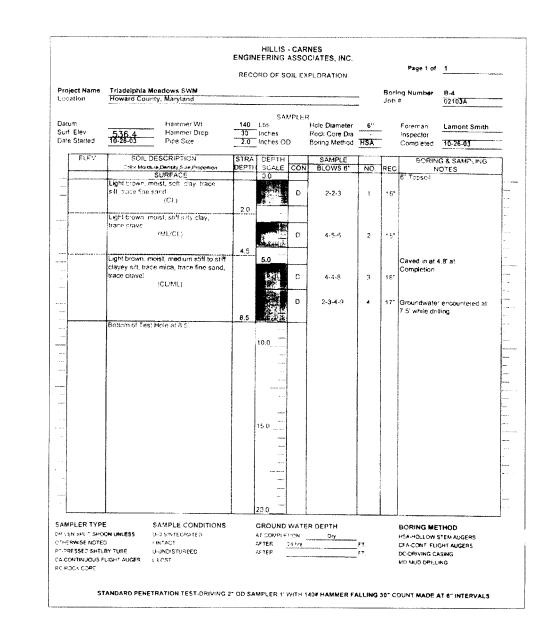


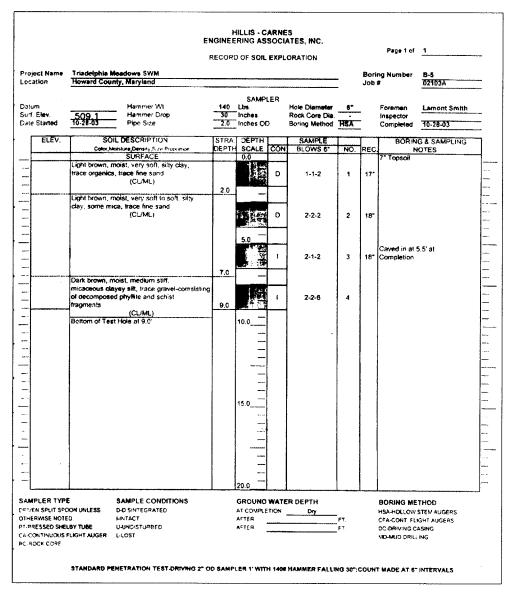
NOT TO SCALE

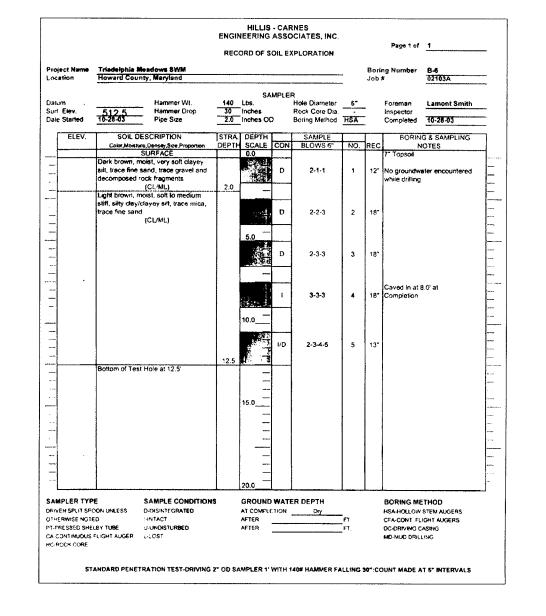


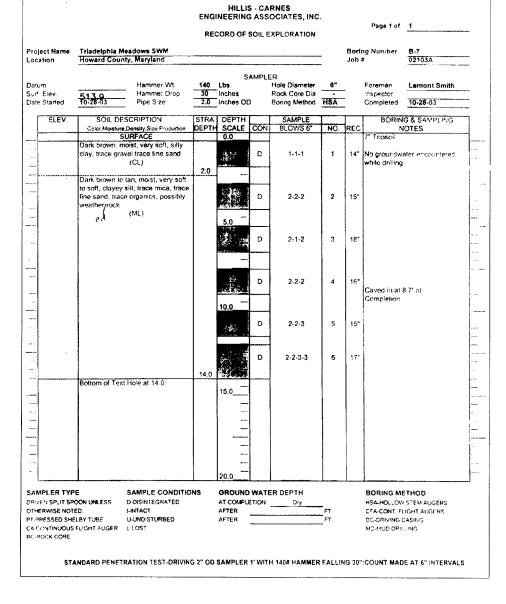


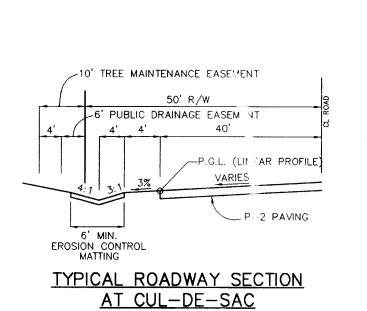






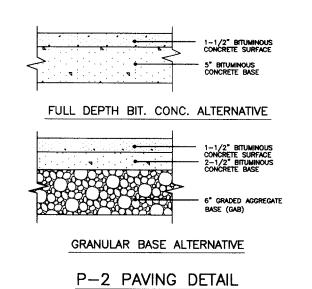




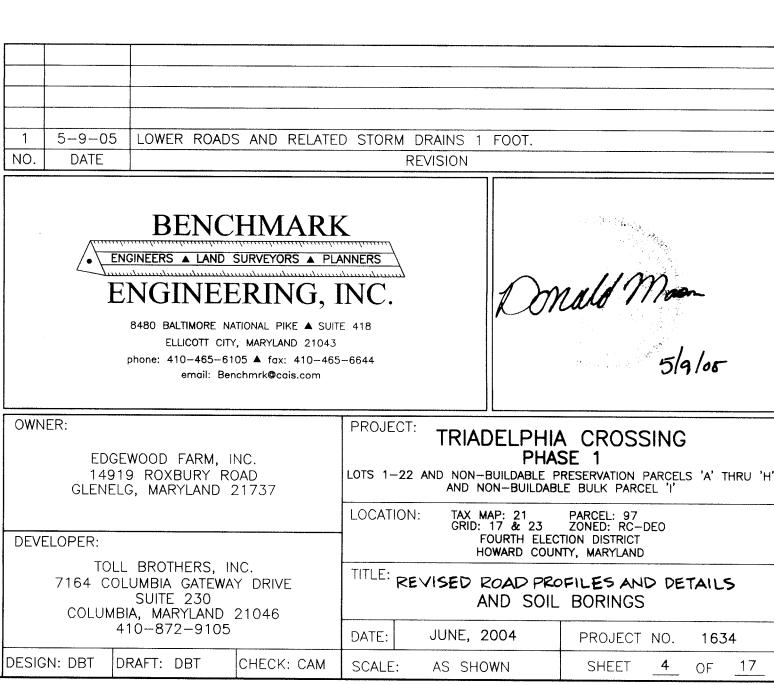


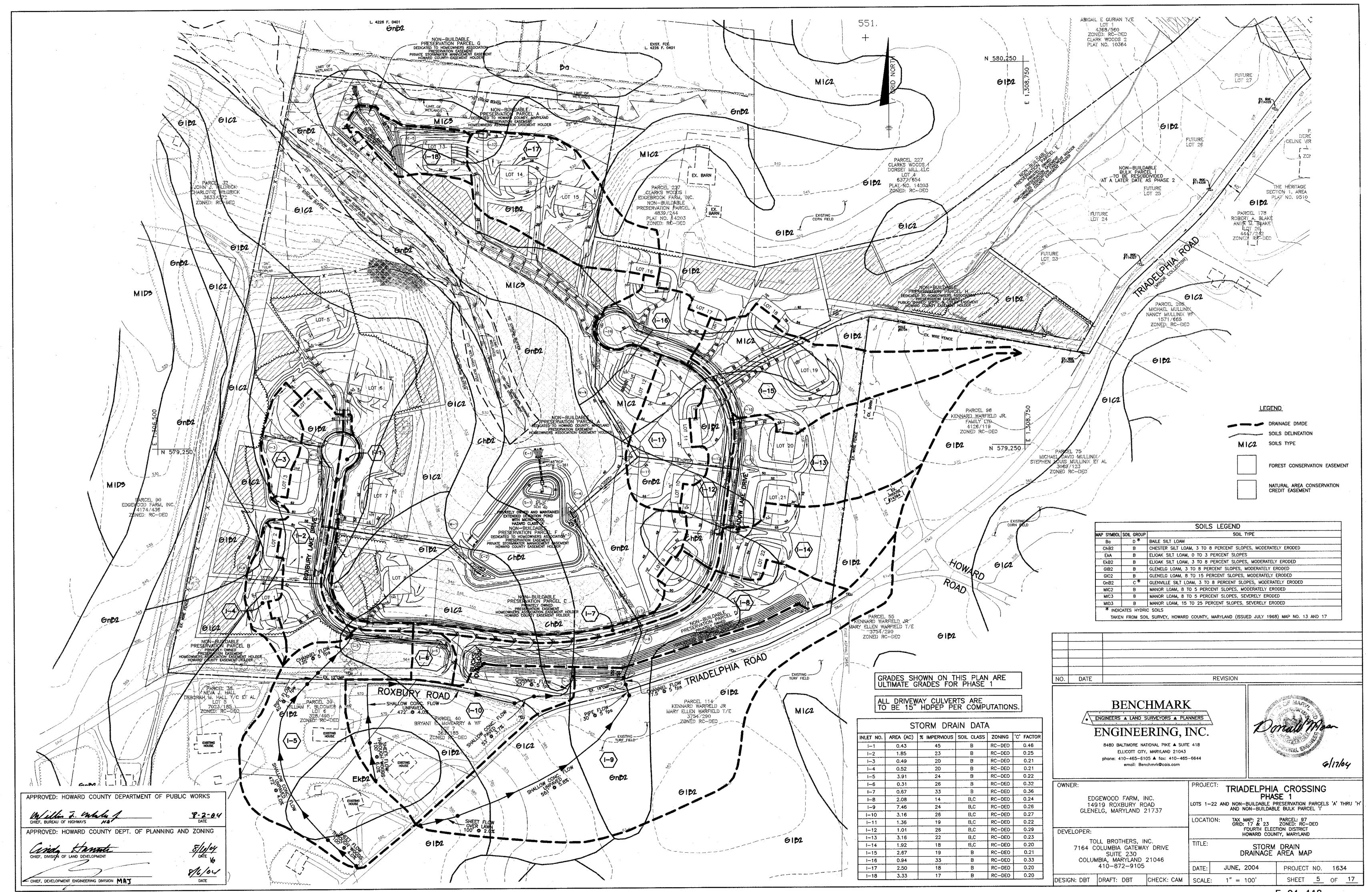
LINEAR PROFILE

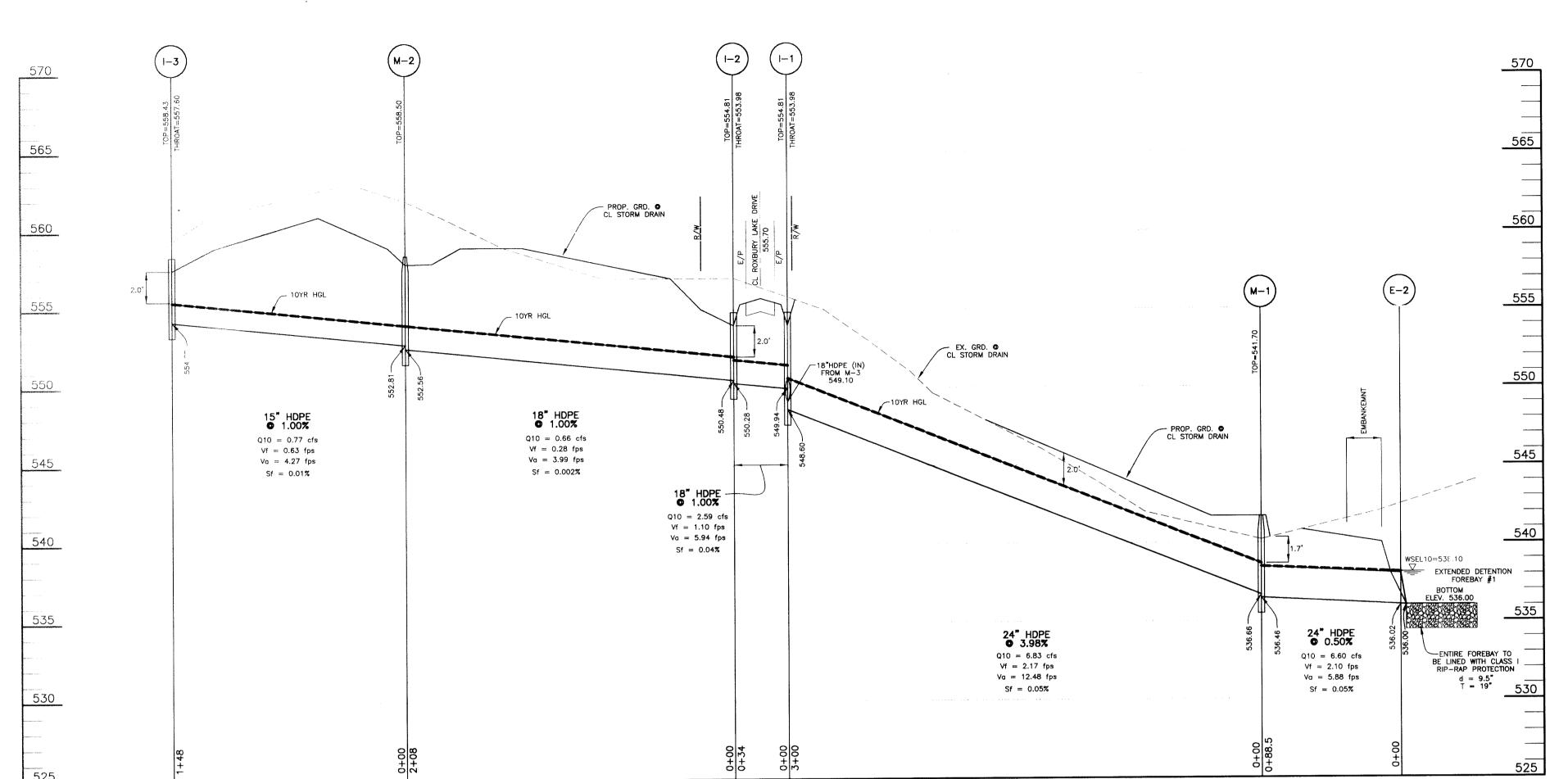
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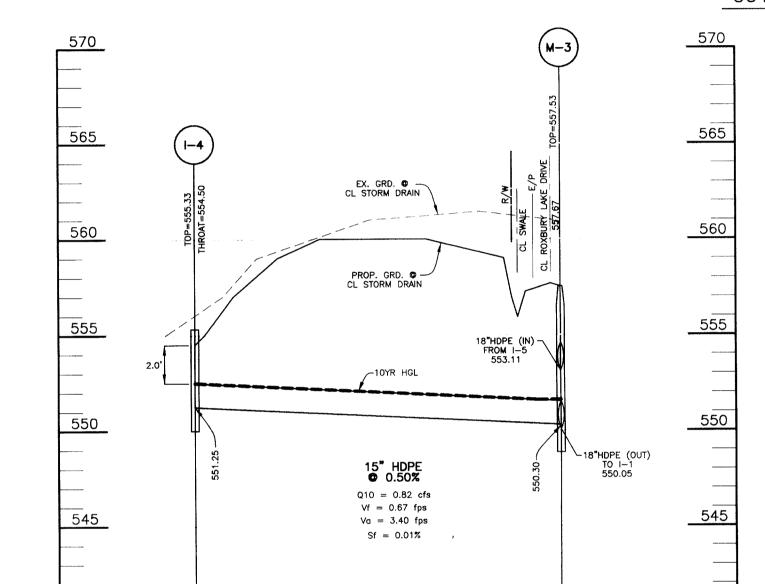
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CONSTRUCTION SPECIFICATIONS

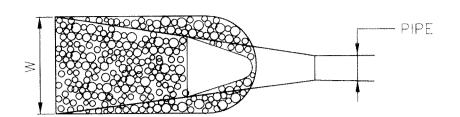
2. THE ROCK OR GRAVEL SHALL CONFORM TO THE SPECIFIED GRADING LIMITS WHEN INSTALLED RESPECTIVELY IN THE RIP—RAP OR FILTER.

4. STONE FOR THE RIP-RAP OR GABION OUTLETS MAY BE PLACED BY EQUIPMENT. THEY SHALL BE CONSTRUCTED TO THE FULL COURSE THICKNESS IN ONE OPERATION AND IN SUCH A MANNER AS TO AVOID DISPLACEMENT OF UNDERLYING MATERIALS. THE STONE FOR HE RIP-RAP OR GABION OUTLETS SHALL BE DELIVERED AND PLACED IN A MANNER THAT WILL ENSURE THAT IT IS REASONABLY HOMOGENOUS WITH THE SHALLER STONES AND SPALLS FILLING THE VOIDS BETWEEN THE LARGER STONES. RIP-RAP SHALL BE PLACED IN A MANNER TO PREVENT DAMAGE TO THE FILTER BLANKET OR GEOTEXTILE FABRIC. HAND PLACEMENT WILL BE REQUIRED TO THE EXTENT NECESSARY TO PREVENT DAMAGE TO THE PERMANENT WORKS.

5. THE STONE SHALL BE PLACED SO THAT IT BLENDS IN WITH THE EXISTING GROUND. IF THE STONE IS PLACED TOO HIGH THEN THE FLOW WILL BE FORCED OUT OF THE CHANNEL AND SCOUR ADJACENT TO THE STONE WILL OCCUR.

1. THE SUBGRADE FOR THE FILTER, RIP-RAP, OR GABION SHALL BE PREPARED TO THE REQUIRED LINES AND GRADES. ANY FILL REQUIRED IN THE SUBGRADE SHALL BE COMPACTED TO A DENSITY OF APPROXIMATELY THAT OF THE SURROUNDING UNDISTURBED MATERIAL. 3. GEOTEXTILE CLASS C28 OR BETTER SHALL BE PROTECTED FROM PUNCHING, CUTTING, OR TEARING. ANY DAMAGE OTHER THAN AN OCCASIONAL SMALL HOLE SHALL BE PREPARED BY PLACING ANOTHER PIECE OF GEOTEXTILE FABRIC OVER THE DAMAGED PART OR BY COMPLETELY REPLACING THE GEOTEXTILE FABRIC. ALL OVERLAPS WHETHER FOR REPAIRS OR FOR JOINING TWO PIECES OF GEOTEXTILE FABRIC SHALL BE A MINIMUM OF ONE FOOT.

SECTION

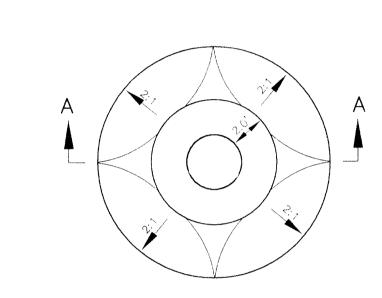


PLAN

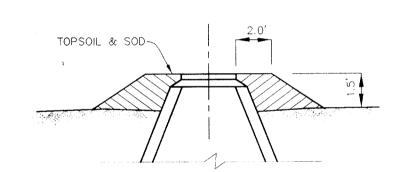
TRUCTURE	D-50	LENGTH (L)	WIDTH (W)	THICKNESS (T)	SHA CLASS
E-1	9.5"	20'	28'	19"	I
E-2	9.5"	FOREBAY	FOREBAY	19"	I
E-3	9.5"	FOREBAY	FOREBAY	19"	I
E-4	9.5"	FOREBAY	FOREBAY	19"	I
E-5	9.5"	FOREBAY	FOREBAY	19"	I
Гб	9.5"	20'	21 25'	19"	Ţ

OUTLET PROTECTION DETAIL

NOT TO SCALE



PLAN VIEW



SECTION A-A

NOTES:

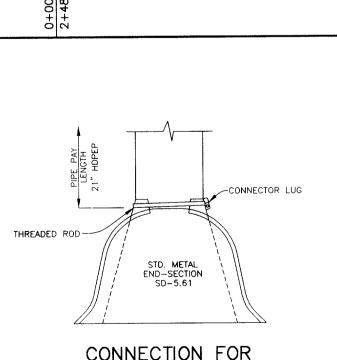
5-9-05 LOWER ROADS AND RELATED STORM DRAINS 1 FOOT.

1.) ALL MANHOLE FRAME AND COVERS SHALL BE FLUSH WITH GRADE EXCEPT WHEN SHOWN ON PROFILE TO BE SET HIGHER THAN GRADE.

2.) MANHOLE FRAME AND COVERS TO BE SET HIGHER THAN GRADE SHALL BE SET IN ACCORDANCE WITH THIS DETAIL.

FILL AROUND MANHOLE

REVISION



Q10 = 2.48 cfs

Vf = 1.05 fps

Va = 5.72 fps

Sf = 0.03%

Q10 = 0.75 cfs

Vf = 0.61 fps

Va = 3.96 fps

Sf = 0.01%

PIPE SCHEDULE			
PIPE SIZE	LENGTH	TYPE	
15"	1751.0'	HDPEP	
18"	920.5'	HDPEP	
24"	1296.5'	HDPEP	
48"	54.0'	RCP	

CONNECTION FOR HDPEP TO METAL END-SECTION NOT TO SCALE

		LOCATION	1813 / 181	INV. OUT	TOP ELEV.	HO. CO. STD.
NO.	TYPE	LOCATION	INV. IN			SD - 5.52
E-1	48" CONC. END SECT.	N 579214.31 E 1307489.87	533.00	532.92		SD - 5.61
E-2	24" METAL END SECT.	N 578932.14 E 1307319.60	536.02	536.00		SD = 5.61
E-3	24" METAL END SECT.	N 579051.46 E 1307584.54	536.04	536.00		SD = 5.61
E-4	24" METAL END SECT.	N 579077.94 E 1307578.84	536.02	536.00		
E-5	15" METAL END SECT.	N 580086.65 E 1307120.69	508.04	508.00		SD - 5.61
E-6	15" HDPE END SECT.	N 580170.56 E 1307019.61	504.54	504.51		HANCOR (SEE MAN. SPECS
I—1	'D' INLET - 6"	CL STA. 5+75.00 OFFS. 17.00' RIGHT (ROXBURY LAKE)	549.94 18" 549.10 18"	548.60	554.81	SD - 4.11 OR 4.39
1-2	'D' INLET - 6"	CL STA. 5+75.00 OFFS. 17.00' LEFT (ROXBURY LAKE)	550.48	550.28	554.81	SD - 4.11 OR 4.39
1-3	'D' INLET - 6"	LP STA. 0+72.86 OFFS. 158.00' LEFT (ROXBURY LAKE)		554.29	558.43	SD - 4.11 OR 4.39
1-4	'D' INLET - 6"	CL STA. 4+33.84 OFFS. 184.61' LEFT (ROXBURY LAKE)		554.25	555.33	SD - 4.11 OR 4.39
i-5	'D' INLET - 6"	CL STA. 2+75.00 OFFS. 17.00' LEFT (ROXBURY LAKE)	554.76	554.51	559.81	SD - 4.11 OR 4.39
1-6	'D' INLET - 6"	CL STA. 0+30.70 OFFS. 28.16' LEFT (ROXBURY LAKE)		556.82	562.46	SD - 4.11 OR 4.39
1-7	'D' INLET - 6"	CL STA. 4+73.86 OFFS. 17.00' LEFT (MEADOW LAKE)	538.99	538.79	544.93	SD - 4.11 OR 4.39
1-8	'D' INLET - 6"	CL STA. 4+73.86 OFFS. 17.00' RIGHT (MEADOW LAKE)	540.30	540.10	544.93	SD - 4.11 OR 4.39
1-9	'D' INLET - 8"	CL STA. 4+01.84 OFFS. 117.19' RIGHT (MEADOW LAKE)	545.80	545.30	550.30	SD - 4.11 OR 4.39
1-10	'D' INLET - 6"	CL STA. 2+89.52 OFFS. 92.11' RIGHT (MEADOW LAKE)		547.70	552.53	SD - 4.11 OR 4.39
I-11	'D' INLET - 6"	CL STA. 9+99.90 OFFS. 244.00' LEFT (MEADOW LAKE)	537.32 24" 537.62 18"	537.12	541.95	SD - 4.11 OR 4.39
1-12	'D' INLET - 6"	CL STA. 9+99.90 OFFS. 17.00' LEFT (MEADOW LAKE)	539.29	538.79	544.16	SD - 4.11 OR 4.39
I-13	'D' INLET - 6"	CL STA. 9+99.90 OFFS. 17.00' RIGHT (MEADOW LAKE)	539.83 15" 539.83 15"	539.63	544.16	SD - 4.11 OR 4.39
1-14	'D' INLET - 6"	CL STA. 8+45.90 OFFS. 17.00' RIGHT (MEADOW LAKE)		541.99	546.07	SD - 4.11 OR 4.39
I15	'D' INLET - 6"	CL STA. 11+33.46 OFFS. 17.00' RIGHT (MEADOW LAKE)		542.33	546.41	SD - 4.11 OR 4.39
I-16	'D' IN ET - 6"	LP STA. 1+33.62 OFFS. 8.00' LEFT (MEADOW LAKE)		542.77	546.85	SD - 4.11 OR 4.39
I <b>-</b> 17	'D' INLET - 6"	N 580069.38 E 1307348.03		512.25	516.33	SD - 4.11 OR 4.39
I-18	'D' INLET - 6"	N 580127.80 E 1307029.95	505.08	504.98	510.01	SD - 4.11 OR 4.39
M-1	4'-0" MANHOLE	N 578993.42 E 1307255.76	536.66	536.46	541.70	G - 5.12
M-2	4'-0" MANHOLE	LP STA. 0+72.86 OFFS. 10.00' LEFT (ROXBURY LAKE)	552.81	552.56	558.50	G - 5.12
M-3	4'-0" MANHOLE	CL STA. 4+26.00 OFFS. 5.00' RIGHT (MEADOW LAKE)	553.11 18" 550.30 18"	550.05	557.53	G - 5.12
M-4	4'-0" MANHOLE	N 579407.80 E 1307635.02	538.95	538.70	543.70	G - 5.12
S-1	· SEE DETAIL	N 579155.96 E 1307496.92	533.50	533.50	539.67	

540

STRUCTURE ELEVATION AND LOCATION FOR MANHOLES IS AT THE TOP AND CENTER OF RIM. STRUCTURE ELEVATION AND LOCATION FOR INLETS IS AT THE TOP OF CURB AT MIDPOINT OF THE INLET OR AT THE CENTER OF SLAB FOR "D" INLETS. 3) STRUCTURE ELEVATION AND LOCATION FOR ENDSECTIONS IS AT THE CONNECTION OF PIPE AND END SECTION. 4) PRECAST STRUCTURES MEETING HS-20 LOADING MAY BE USED. 5) ALL STORM DRAINS SHALL BE SMOOTH CORE HIGH DENSITY POLYETHYLENE PIPE.

**BENCHMARK** ● ENGINEERS ▲ LAND SURVEYORS ▲ PLANNERS ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE ▲ SUITE 418 ELLICOTT CITY, MARYLAND 21043 phone: 410-465-6105 ▲ fax: 410-465-6644 email: Benchmrk@cais.com OWNER: EDGEWOOD FARM, INC. 14919 ROXBURY ROAD GLENELG, MARYLAND 21737 DEVELOPER: TOLL BROTHERS, INC. 7164 COLUMBIA GATEWAY DRIVE SUITE 230 COLUMBIA, MARYLAND 21046 410-872-9105

DESIGN: DBT | DRAFT: DBT

DATE

NO.

5/9/05

TRIADELPHIA CROSSING PHASE 1 LOTS 1-22 AND NON-BUILDABLE PRESERVATION PARCELS 'A' THRU 'H'
AND NON-BUILDABLE BULK PARCEL 'I'

TAX MAP: 21 PARCEL: 37
GRID: 17 & 23 ZONED: RC-DEO
FOURTH ELECTION DISTRICT HOWARD COUNTY, MARYLAND revised

STORM DRAIN PROFILES AND DETAILS JUNE, 2004 DATE: PROJECT NO. 1634 CHECK: CAM | SCALE: SHEET <u>6</u> OF <u>17</u>

P:\1634\dwg\7044s6-7\_Redline\_County.dwg, 5/8/2005 2:08:53 PM

Milling T. Muly J. CHIEF, BUREAU OF HIGHWAYS MS

565

560

555

18"HDPE (IN) -FROM 1-2 550 549.94

CL STORM DRAIN

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

APPROVED: HOWARD COUNTY DEPT. OF PLANNING AND ZONING

7-6-05 DATE

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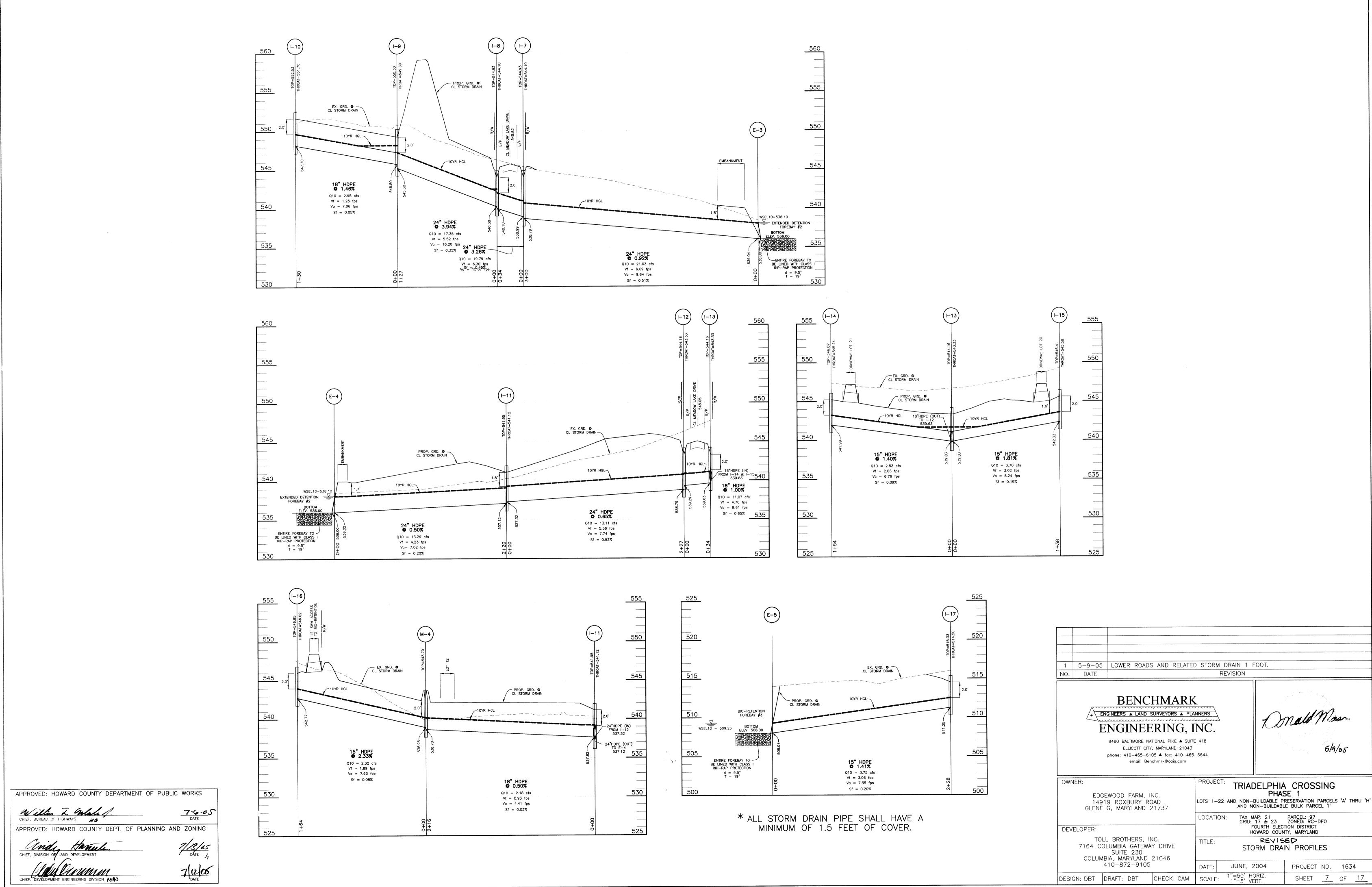
18" HDPE 9 0.64%

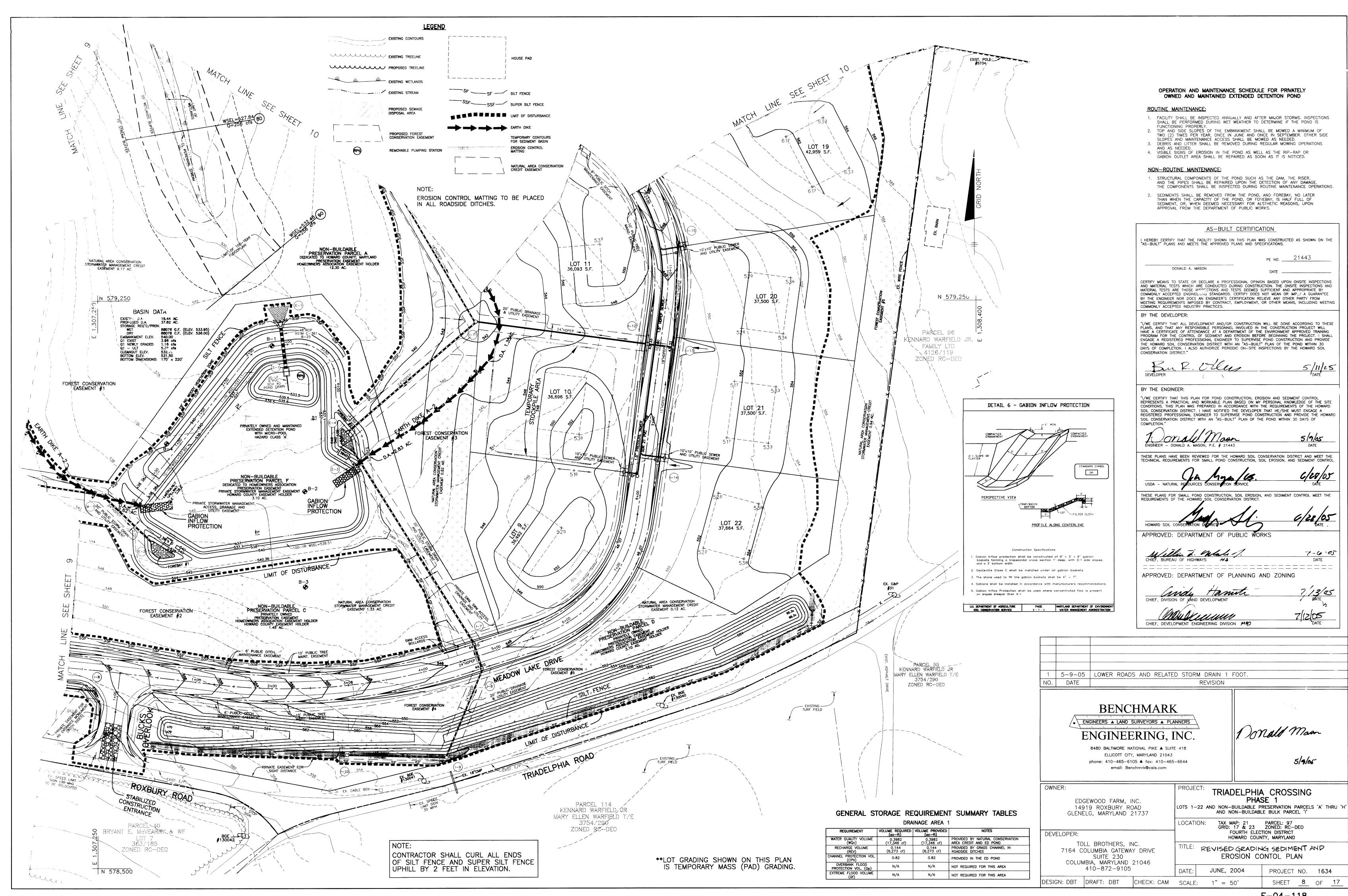
Q10 = 3.89 cfs

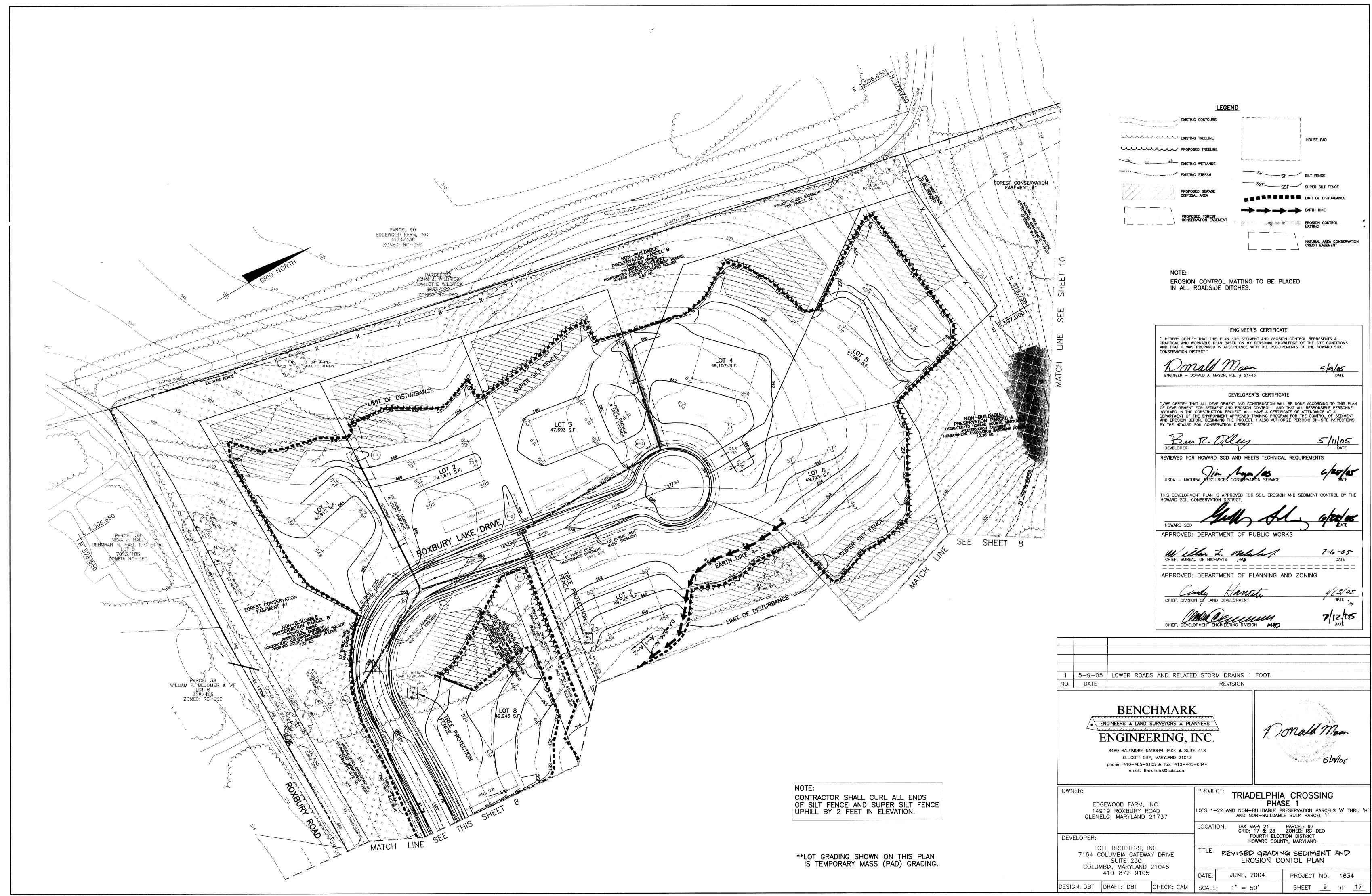
Vf = 1.65 fps

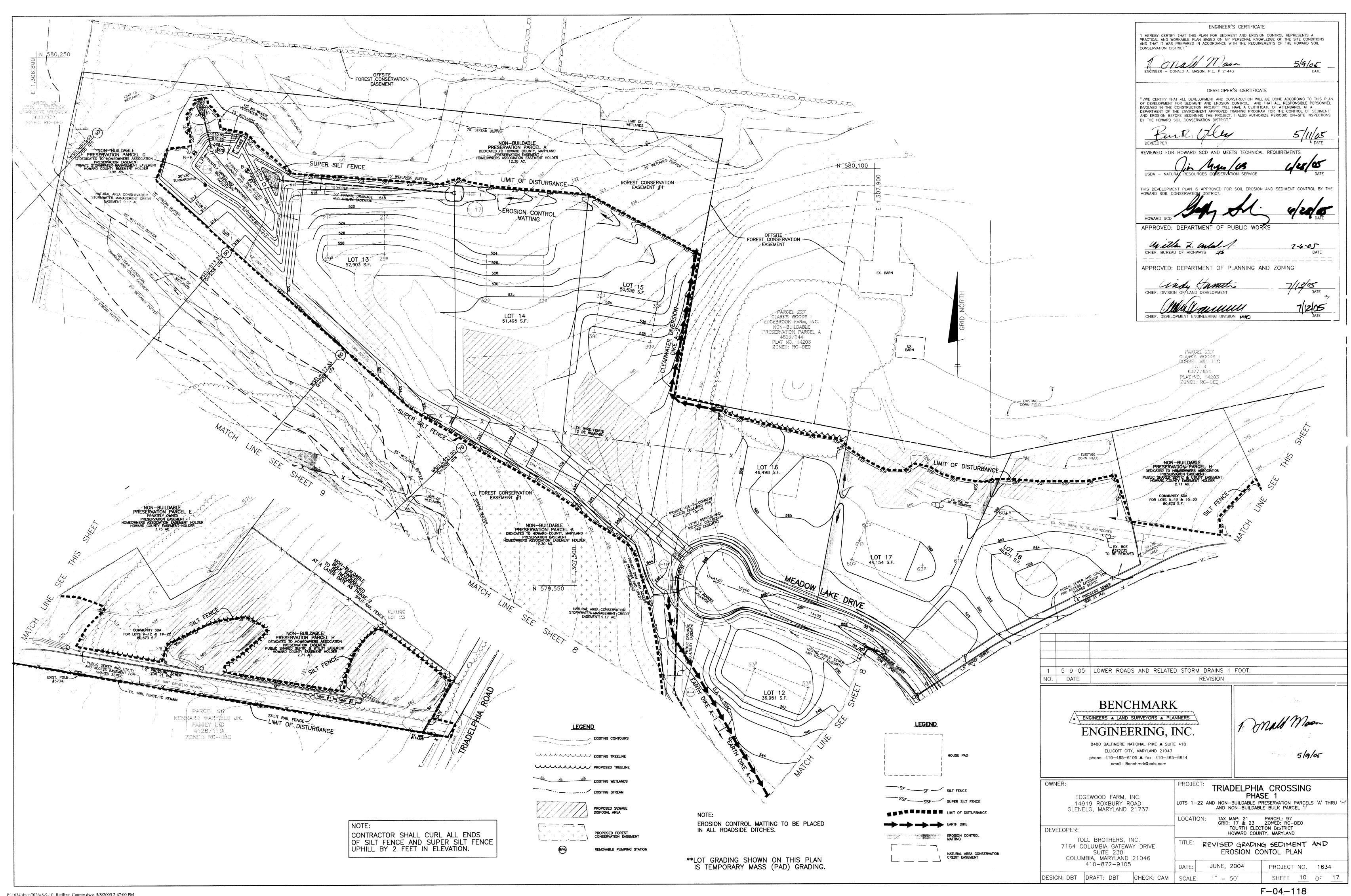
Va = 5.66 fps

Sf = 0.08%









### Site Preparation

Areas designated for borrow areas, embankment, and structural works shall be cleared. grubbed and stripped to topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1: All trees shall be cleared and grubbed within 15 feet of the toe of the embankment.

Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet structure shall be

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

Material - The fill material shall be taken from approved designated borrow areas. If shall be free of roots, stumps, wood, rubbish, stones greater than 6", frozen or other objectionable material. Fill material for the center of the embankment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer.

Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment.

Placement — Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be

installed concurrently with fill placement and not excavated into the embankment.

Compaction — the movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory railer. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with teh equipment used. The fill material shall contain sufficient maisture so that if formed into a ball it will not crumble, yet not be so wet that water can be squeezed out.

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within  $\pm$  2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor)

<u>Cut Off Trench</u> — The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be a least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability.

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the cores shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slapes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed concurrently with the outer shell of the embankment.

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistively of 2,000 ohm-cm. Material shall be placed such that a minimum of 6 (measured perpendicular to the outside of the pipe) of flowable fill shall be under (beddin over and, on the sided of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers of other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part a structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24' or greater over the structure or pipe. Backfill material outside the structural backfill (flowable fill) zone shall be of the type and qualit conforming to that specified for the core of the embankment or other embankment

## Pipe Conduits

between 4 and 9.

All pipes shall be circular in cross section

Corrugated Metal Pipe — all of the following criteria shall apply for corrugated metal pipe: 1. Materials — (Polymer Coated steel pipe) — Steel pipes with polymeric coatings shall bave a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 & M-246 with watertight coupling bands or flanges.

Maerials — (Aluminum Coated Steel Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M—274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or atherwise removed shalf be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.

Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-196 or M-211 with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be

2. Coupling bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at least 24 mils in thickness.

3. Connections - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.

All connection shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches in diameter: flanges on both ends of the pipe with a circular 3/8 inch closed cell neoprene gasket, prepunched to the flange bolt circle, sandwiched between adjacent flanges; a 12-inch wide standard lop type band with 12-inch wide by 3/8-inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by a 24 inch long annular corrugated band using minimum of 4 (four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8 inch closed cell gaskets the full width of the

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or a neoprene bead

4. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

5. Backfilling shall conform to "Structure Backfill".

6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings. Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete

1. Materials — Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C-361.

Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle or their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used a described in the "Structure Backfill" section of this standard. Gravel bedding is not permitted.

3. Laying pipe — Bell and spigot pipe shall be places with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation form the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

4. Backfilling shall conform to "Structure Backfill".

5. Other details (anti-seep collors, valves, etc.) shall be shown on the drawings.

Plastic Pipe - The following criteria shall apply for plastic pipe: 1. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4' - 10" inch pipe shall meet the requirements of AASHTO M252

2. Joints and connections to anti-seep collars shall be completely watertight. 3. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be

Type S, and 12" through 24" inch shall meet the requirements of AASHTO M294 Type S.

4. Backfilling shall conform to "Structure Backfill".

5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings. <u>Drainage Diophragms</u> — When a drainage diaphragm is used, a registered professional engineer will supervise the design and construction inspection

removed and replaced with suitable earth compacted to provide adequate support.

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414,

Rock riprop shall meet the requirements of Maryland Deportment of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 311. Geotextile shall be placed under all riprap and shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for

### Care of Water during Construction

Construction and Materials, Section 921.09, Class C.

All work on permanent structures shall be carried out in areas free from water. The contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect the areas to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the evacuations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the eration or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the location being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall be pumped.

All borrow areas shall be graded to provide proper drainage and left in a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

## Erosion and Sediment Control

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures.

### SEDIMENT CONTROL NOTES

- A MINIMUM OF 24 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT OF INSPECTION, LICENSES AND PERMITS, SEDIMENT CONTROL DIVISION PRIOR TO THE START OF ANY CONSTRUCTION, (313-1850).
- ALL VEGETATIVE AND STRUCTURAL PRACTICES ARE TO BE INSTALLED ACCORDING TO THE PROVISIONS OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE MOST CURRENT "MARYLAND STANDARDS AND SPECIFICATION FOR SOIL EROSION AND SEDIMENT CONTROL", REVISIONS THERETO.
- FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: A) 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, PERIMETER SLOPES AND ALL SLOPES GREATER THAN 3:1, B) 14 DAYS AS TO ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE
- ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 12, OF THE HOWARD
- ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDINGS (SEC. 51) SOD (SEC. 54), TEMPORARY SEEDING (SEC. 50) AND MULCHING (SEC. 52), TEMPORARY TABILIZATION WITH MULCH ALONE CAN ONLY BE DONE WHEN RECOMMENDED SEEDING DATES DO NOT ALLOW FOR PROPER GERMINATION AND ESTABLISHMENT OF GRASSES.
- ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE MAINTAINED IN OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.

### 7. SITE ANALYSIS:

TOTAL AREA OF SITE	0Z.3 ACRES
AREA DISTURBED	30.6 ACRES
AREA TO BE ROOFED OR PAVED	1.3 ACRES
AREA TO BE VEGETATIVELY STABILIZED	29.3 ACRES
TOTAL CUT	33,915c <sub>Y</sub>
TOTAL FILL	36,308 cy
OFFSITE WASTE AREA LOCATION	SITE WITH APPROVED

62.3

\_ ACRES

- ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF LIBITIES MUST BE REPAIRED ON THE SAME DAY OF DISTURBANCE.
- ADDITIONAL SEDIMENT CONTROL MUST BE PROVIDED, IF DEEMED NECESSARY BY THE IOWARD COUNTY SEDIMENT CONTROL INSPECTOR.
- ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER FROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING, OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL BY THE
- TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGTHS OR THAT WHICH CAN BE BACK FILLED AND STABILIZED WITHIN ONE WORKING DAY, WHICHEVER IS SHORTER.

## TEMPORARY SEEDBED PREPARATIONS

APPLY TO GRADED OR CLEARED AREAS LIKELY TO BE REDISTURBED WHERE A SHORT-TERM VEGETATIVE COVER IS NEEDED.

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING. IF NOT PREVIOUSLY LOOSENED SOIL AMENDMENTS: APPLY 600 LBS PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ FT) SEEDING: FOR PERIOD MARCH 1 THROUGH APRIL 30 AND FROM AUGUST 15 THROUGH NOVEMBER 15. SEED WITH 2-1/2 BUSHELS PER ACRE OF ANNUAL RYE (3.2 LBS/1000 SQ FT). FOR THE PERIOD MAY 1 THROUGH AUGUST 14, SEED WITH 3 LBS PER ACRE OF WEEPING LOVEGRASS (.07 LBS/1000 SQ FT), FOR THE PERIOD NOVEMBER 16 THROUGH FEBRUARY 28. PROTECT SITE BY APPLYING 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING, OR USE SOD.

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ FT) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ FT) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES, 8 FT. OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ FT) FOR ANCHORING REFER TO THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND

## PERMANENT SEEDBED PREPARATIONS

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED. IN LIEU OF SOIL AMENDMENTS: IN LIEU OF SOIL TEST RECOMMENDATIONS, USE ON OF THE FOLLOWING

PREFERRED - APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS/1000 SQ FT) AND 600 LBS PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ FT) BEFORE SEEDING, HARROW OR DISC INTO UPPER THREE INCHES OF SOIL. AT TIME OF SEEDING, APPLY 400 LBS PER ACRE 30-0-0- UREAFORM FERTILIZER

ACCEPTABLE - APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS/1000 SQ FT) AND 1000 LBS PER ACRE 10-10-10 FERTILIZER (23 LBS/1000 SQ FT) BEFORE SEEDING. HARROW OR DISC INTO UPPER THREE INCHES OF SOIL.

SEEDING: FOR THE PERIODS MARCH 1 THROUGH APRIL 30 AND AUGUST 1 THROUGH OCTOBER 15, SEED WITH 60 LBS PER ACRE (1.4 LBS/1000 SQ FT) OF KENTUCKY 31 TALL FESCUE PER ACRE AND 2 LBS PER ACRE (.05 LBS/1000 SQ FT) OF WEEPING LOVEGRASS. DURING HE PERIOD OF OCTOBER 16 THROUGH FEBRUARY 28, PROTECT SITE BY: OPTION (1) 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING. OPTION (2) USE SOD. OPTION (3) SEED WITH 60 LBS PER ACRE OF KENTUCKY 31 TALL FESCUE AND MULCH WITH 2 TONS PER ACRE OF WELL ANCHORED STRAW.

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ FT) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ FT) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ FT) FOR ANCHORING.

MAINTENANCE: INSPECT ALL SEEDED AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND

## TOPSOIL SPECIFICATIONS

- Topsoil salvaged from the existing site may be used provided that it meets that standards as set forth in these specifications. Typically, the depth of topsoil to be solvaged for a given soil type can be found in the representative soil profile section in the Soil Survey published by USDA—SCS in cooperation with Maryland Agricultural Experimental Station.
- Topsoil Specifications Soil to be used as topsoil must meet the following:
- Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay loam, loamy sand. Other soils may be used if recommended by an agronomist or soil scientist and approved by the appropriate approval authority. Regardless, topsoil shall not be a mixture of contrasting texture subsoils and shall contain less than 5% by volume of cinders, stones, stag, coarse fragments, gravet, sticks, roots, trash, or other materials larger than 1-1/2" in diameter.
- ii. Topsoii must be free of plants or plant parts such as Bermuda grass, quack grass, Johnson grass, nutsedge, poison ivy, thistie, or others as specified.
- iii. Where the subsoil is either highly acidic or composed of heavy cloys, ground limestone shall be spread at the rate of 4-8 tons/acre (200-400 pounds per 1,000 square feet) prior to the placement of topsoil. Lime shall be distributed uniformly over designated areas and worked into the soil in conjunction with tillage operations as described in the following procedures.
- III. For sites having disturbed areas under 5 acres: Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization - Section 1 - Vegetative Stabilization Methods and
- IV. For sites having disturbed areas over 5 acres
  - On soil meeting Topsoil specifications, obtain test results dictating fertilizer and lime amendments required to bring the soil into compliance with the following:
  - pH for topsoli shall be between 6.0 and 7.5. If the tested soil demonstrates a pH of less than 6.0, sufficient time shall be prescribed to raise the pH to 6.5 or higher.
  - b. Organic content or topsoil shall be not less than 1.5 percent by weight.
  - Topsoil having soluble salt content greater than 500 parts per million shall
  - d. No sad or seed shall be placed on soil which has been treated with sai elapsed (14 days min.) to permit dissipation of phyto-toxic materials

Note: Topsoil substitutes or amendments, as recommended by a qualified agronomist soil scientist and approved by the appropriate approval authority, may be used in lieu of

- Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization Section I Vegetative Stabilization Methods and Materials. V. Topsoil Application
- When topsoiling, maintain needed erosion and sediment control practices such as diversions, grade stabilization structures, earth dikes, slope sitt fence and sedimen
- Grades on the areas to be topsoiled, which have been previously established, shall be maintained, albeit 4" 8" higher in elevation.
- iii. Topsoil shall be uniformly distributed in a 4" 8" layer and lightly compacted to a minimum thickness of 4". Spreading shall be performed in such a manner that sodding or seeding can proceed with a minimum of additional soil preparation and tillage. Any irregularities in the surface resulting from topsoiling or other operations shall be corrected in order to prevent the formation of depressions or
- iv. Topsoil shall not be placed while the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet or in a condition that may otherwise be detrimental to proper grading and seedbed preparation.
- VI. Alternative for Permanent Seeding Instead of applying the full amounts of lime and commercial fertilizer, composted sludge and amendments may be applied as specified

must be added to meet the requirements prior to use.

STANDARD SYMBOL

[∑] RP5

- Composited Studge Material for use as a soil conditioner for sites having distributed areas over 5 acres shall be tested to prescribe amendments and for sites having disturbed areas under 5 acres shall conform to the following requirements:
- a. Composted sludge shall be supplied by, or originate from, a person or persons that are permitted (at the time of acquisition of the compost) by the Maryland Department of the Environment under COMAR 26.04.06. b. Composted siudge shall contain at least 1 percent nitrogen, 1.5 percent phosphorus, and 0.2 percent potassium and have a pH of 7.0 to 8.0. I compost does not meet these requirements, the appropriate constituents
- c. Composted studge shall be applied at a rate of 1 ton/1,000 square feet. iv. Composted sludge shall be amended with a potassium fertilizer applied at the rate References: Guidelines Specifications, Sail Preparation and Sodding. MD—VA, Pub. #1, Cooperative Extension Service, University of Maryland and Virginia Polytechnic Institutes, Revised 1973.

DETAIL 20A - REMOVABLE PUMPING STATION

STATE THE

0000

ELEVATION (CUT AWAY)

DETAIL 33 - SUPER SILT FENCE

HOOK AND CHAIN FOR REMOVAL

Perforated (removable) 12" – 36" pipe wrapped w/ nardwore cloth and Gest

## 30.0 DUST CONTROL

Controlling dust blowing and movement on construction sites and roads

To prevent blowing and movement of dust from exposed soil surfaces, reduce on and off-site azards, and improve troffic safety.

Conditions Where Practice Applies This practice is applicable to areas subject to dust blowing and movement where on and off-site

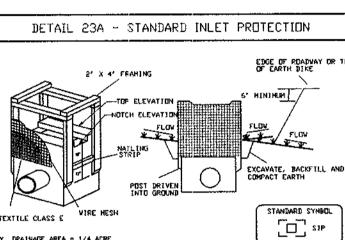
### <u>Iemporary Methods</u> 1. Mulches - See standards for vegetative stabilization with mulches only. Mulch should be crimped or tracked to prevent blowing.

- 2. Vegetative Cover See standards for temporary vegetative cover
- 3. Tillage To roughen surface and bring clods to the surface. This is an emergency measure which should be used before soil blowing starts. Begin plowing on windword side of site. Chisel-type plows spaced about 12" apart, spring-toothed harrows, and
- Irrigation This is generally done as an emergency treatment. Site is sprinkled with water until the surface is moist. Repeat as needed. At no time should the site be irrigated to the point that runoff begins to flow.
- Barriers Solid board fences, silt fences, snow fences, burlap fences, straw bales, and similiar material can be used to control air currents and soil blowing. Barriers placed at right angles to prevailing currents at intervals of about 10 times their height are effective in controlling soil blowing.
- 6. Calcium Chloride Apply at rates that will keep surface moist. May need retreatment Permanent Methods

  1. Permanent Vegetation — See standards for permanent vegetative cover, and permanent stabilization with sod. Existing trees or large shrubs may afford valuable protection if
- 2. Topsoiling Covering with tess erosive soil materials. See standards for topsoiling 3. Stone - Cover surface with crushed stone or coarse gravel

1. Agriculture Handbook 346. Wind Erosion Forces in the United States and Their Use

2. Agriculture Information Bulletin 354. How to Control Wind Erosion, USDA-ARS.



EDTEXTILE CLASS E MAX. DRAINAGE AREA = 1/4 ACR Construction Specifications

 Excavate completely around the inlet to a depth of 18° below the 2. Brive the 2' x 4' construction grade lumber posts I' into the ground at each corner of the inlet. Place noil strips between the posts on the ends of the inlet. Assemble the top portion of the

op of the frame (welr) rust be 6' below adjacent roadways where coding and safety issues may arise 3. Stretch the 1/2"  $\times$  1/2" wire mesh tightly around the frame and fasten securely. The ends must meet and overlap at a

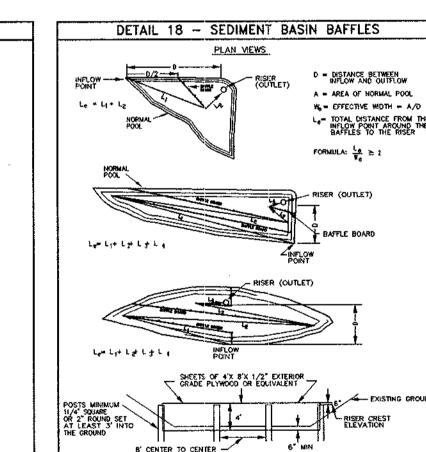
2' x 4' frame using the overtop joint shown on Detail 23A. The

4. Stretch the Geotextile Class E tightly over the wire mesh with the geotextile extending from the top of the frame to 18° below the inlet notch elevation. Fasten the geotextile firmly to the frame. The ends of the geotextile nust neet at a post, be overlapped and folded, then fostened down.

5. Backfill around the inlet in compacted 6' layers until the layer of earth is level with the notch elevation on the ends and top elevation on the sides.

6. If the inlet is not in a sump, construct a compacted earth dike

across the ditch line directly below it. The top of the earth dike should be at least 6' higher than the top of the frame. The structure must be inspected periodically and after each rain and the geotextile replaced when it becomes clogged.



# CLEAN GRAVELT 1. The outer pipe should be 48" dia, or shall, in any case, be at least 4" greater in diameter than the center pipe. The outer pipe shall be wrapped with 1/2" hardware cloth to prevent backfill material from entering the perforations. 2. After installing the outer pipe, backfill around outer pipe with 2" aggregate or clean gravel. Construction Specifications 3. The inside stand pipe (center pipe) should be constructed by perforating a corrugated or PVC pipe between 12" and 36" in diameter. The perforations shall be 1/2" x 6" sits or 1" diameter holes 6" on center. The center pipe shall be wropped with 1/2" hardware cloth first, then wropped agen with Geotextile Class C. 4. The center pipe should extend 12° to 18° above the anticipated water surface ejevation or riser crest elevation when dewatering a basin. S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT SOIL CONSERVATION SERVICE D - 12 - 5 WATER MANAGEMENT ALIMINISTRATION

PAGE MARYLAND DEPARTMENT OF ENVIRONM
C - 10 - 28 WATER MANAGEMENT ADMINISTRATIO SUPER SILT FENCE CONSTRUCTION SPECIFICATIONS Fencing shall be 42" in height and constructed in accordance with the latest Maryland State Highway Details for Chain Link Fencing. The specification for a 6' fence shall be used, substituting 42" fabric and 6' length posts.

> When two sections of filter cloth adjain each other, they shall be overlapped by 6" and folder Maintenance shall be performed as needed and silt buildups removed when "builges" develop in the silt fence, or when silt reaches 50% of fence height Filler cloth shall be fastened securely to each fence post with wire ties or stoples a

BAFFLE DETAIL

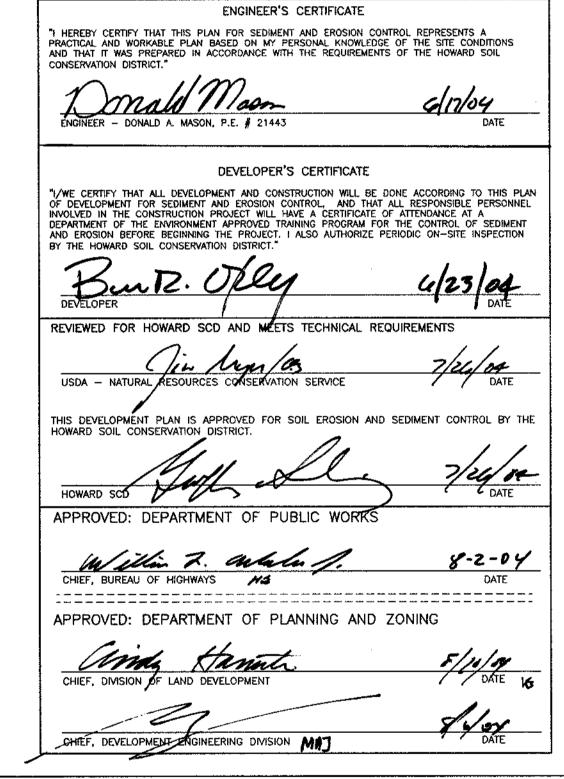
Slope	Slope Steepness	Stope Length (maximum)	Silt Fence Len (maximum)
0 - 10%	0 - 10:1	Unlimited	Unlimited
10 - 20%	10:1 - 5:1	200 feet	1,500 feet
20 - 33%	5:1 - 3:1	100 feet	1,000 feet
33 - 50%	3:1 - 2:1	100 feet	500 feet
50¥ ±	2.1 ±	SO (ant	250 feet

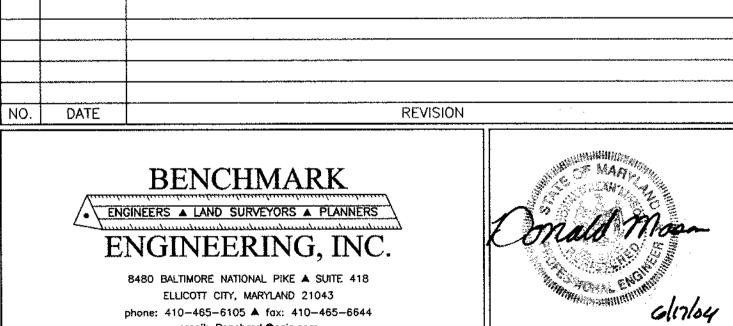
2:1 + 250 feet

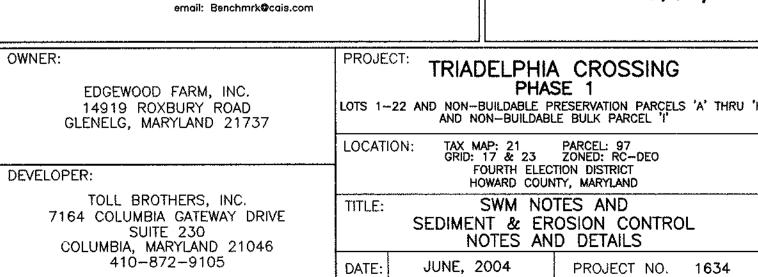
DESIGN: DBT | DRAFT: DBT

SEQUENCE OF CONSTRUCTION

- NOTIFY SEDIMENT CONTROL DIVISION 48 HOURS PRIOR TO START OF CONSTRUCTION
- 1. OBTAIN GRADING PERMIT. (DAY 1)
- 2. INSTALL STABILIZED CONSTRUCTION ENTRANCE, TREE PROTECTION FENCES, SUPER SILT FENCES, SILT FENCES AND TEMPORARY CLEANWATER DIVERSION DIKES. (DAY 2-12)
- 3. INSTALL SEDIMENT BASIN. (DAY 13-45)
- 4. INSTALL ANY REMAINING SEDIMENT CONTROL DEVICES. (DAY 46-50)
- 5. UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, BRING ROAD BEDS TO SUBGRADE AND STABILIZE SLOPES IN ACCORDANCE WITH TEMPORARY SEEDBED NOTES. UTILIZE DUST CONTROL METHODS. (DAY 51-81)
- 6. UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, INSTALL STORM DRAINS, I-18 SHALL BE PROTECTED DURING CONSTRUCTION. THE STORM DRAIN SYSTEMS FROM E-5 TO I-17 AND THE SWALE THAT DRAINS TO I-17 MUST NOT BE INSTALLED AT THIS TIME. (DAY 82-112)
- 7. PAVE ROADWAYS. (DAY 113-128)
- 8. COMPLETE GRADING OF SITE AND STABLIZE DISTURBED AREAS IN ACCORDANCE WITH THE PERMANENT SEEDBED NOTES. (DAY 129-144)
- 9. UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, INSTALL THE STORM DRAINS FROM E-5 TO 1-17. (DAY 145-147)
- 10. UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR. CONVERT SEDIMENT BASINS TO STORMWATER MANAGEMENT FACILITIES. SHAPE FACILITIES PER FINAL GRADES SHOWN ON THE PLANS AND STABILIZE DISTURBED AREAS IN ACCORDANCE WITH THE PERMANENT SEEDBED NOTES. CONTRACTOR SHALL REMOVE ALL OLD AND NEW TRASH, JUNK AND DEBRIS FROM ENTIRE SITE. (DAY 148-153)
- 11. UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, REMOVE REMAINING SEDIMENT CONTROL DEVICES, AND STABILIZED DISTURBED AREAS IN ACCORDANCE WITH THE PERMANENT SEEDBED NOTES(DAY 154-161)







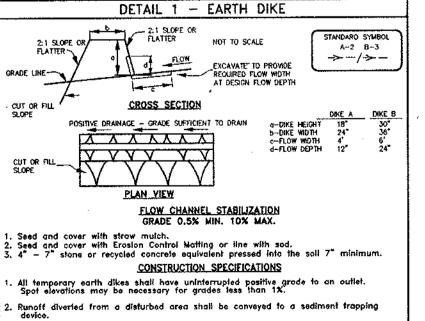
SCALE:

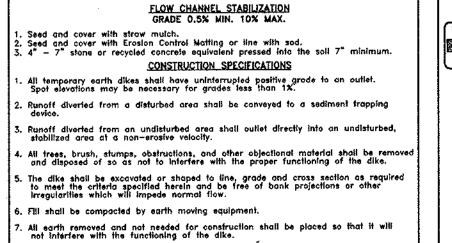
NOT TO SCALE

CHECK: CAM

## . Chain link fence shall be fastened securely to the fence posts with wire ties. The lower tension wire, brace and truss rods, drive anchors and post caps are not required except on the ends of the fence. . Filler cloth shall be fastened securely to the chain tink fence with ties spaced every 24" at the top and mid section. SURFACE Filter cloth shall be embedded a minimum of 8" into the ground 2222A STANDARD SYMBOL PERSPECTIVE VIEW \_\_2~1'/'2~" Dio. (\$ALVANIZED OF ALUMINUM FENCE POST ------SUPER SILT FENCE DESIGN CRITERIA Flow

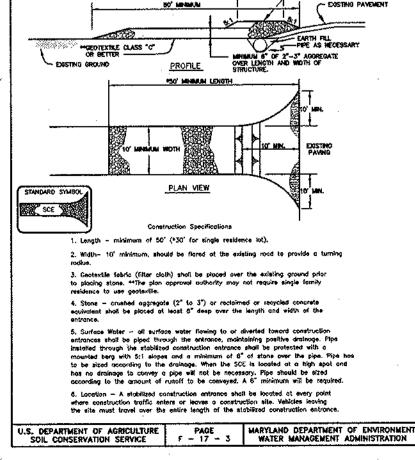
TREE PROTECTION FENCE BLAZE ORANGE PLASTIC MESH -8 FEET MAXIMUM ANCHOR POSTS MUST BE INSTALLED TO A DEPTH OF NO LESS THAN 1/3 OF THE TOTAL HEIGHT OF THE POST. NOTES: 1. FOREST PROTECTION DEVICE ONLY.
2. RETENTION AREA WILL BE SET AS PART OF THE REVIEW PROCESS.
3. BOUNDARIES OF RETENTION AREA SHOULD BE STAKED AND FLAGGED PRIOR TO INSTALLING DEVICES.
4. AVOID ROOT DAMAGE WHEN PLACING ANCHOR POSTS.
5. DEVICE SHOULD BE PROPERLY MAINTAINED DURING CONSTRUCTION 6. PROTECTIVE SIGNAGE IS ALSO REQUIRED.



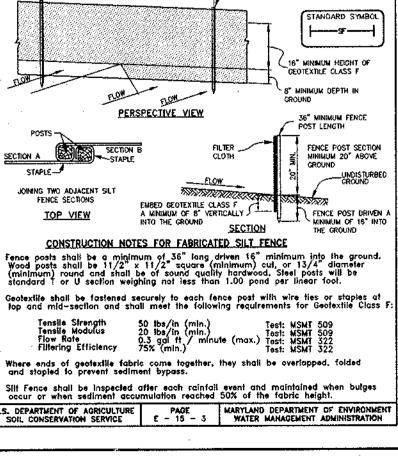


R. inspection and maintenance must be provided periodically and after each rain event.

U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE



DETAIL 24 - STABILIZED CONSTRUCTION ENTRANCE



DETAIL 22 - SILT FENCE

- 36" MINIMUM LENGTH FENCE POST

MINIMUM OF 16" INTO GROUN

10' MAXIMUM CENTER TO CENTER

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OF 17

SHEET

