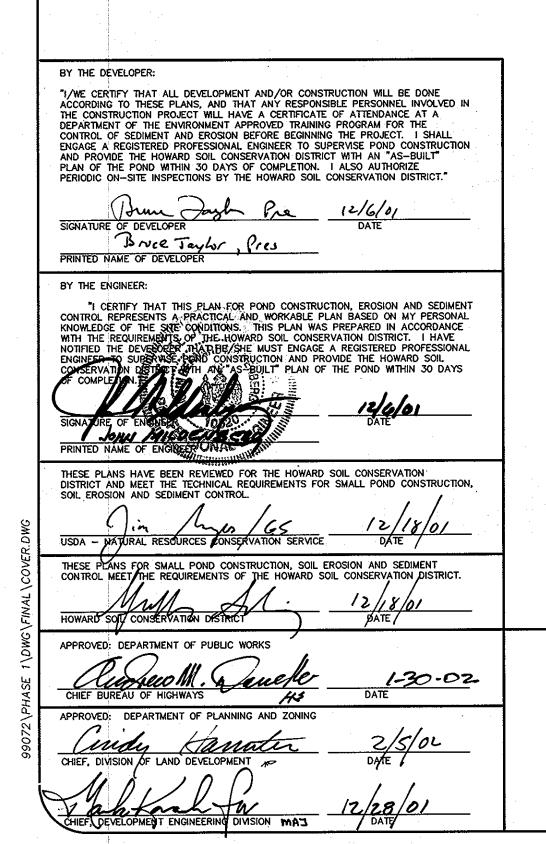
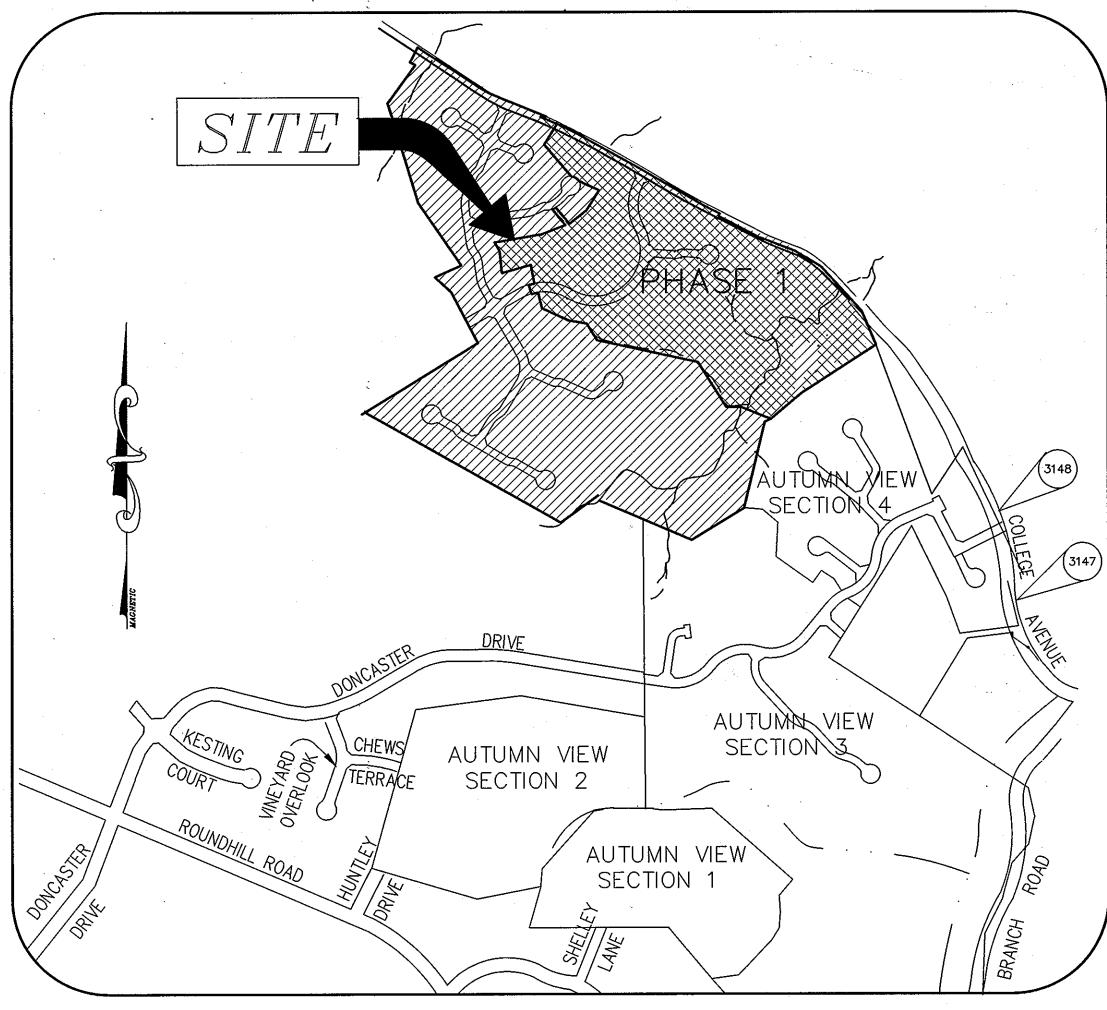
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OWNER/DEVELOPER

BONNIE BRANCH CORPORATION P.O.BOX 396 ELLICOTT CITY, MD 21043



ROAD CONSTRUCTION PLANS AUTUMN VIEW SECTION 5, PHASE 1 LOT 211 THRU 259 SECOND ELECTION DISTRICT HOWARD COUNTY, MARYLAND



VICINITY MAP

GENERAL NOTES

- 1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST STANDARDS AND SPECIFICATIONS OF HOWARD COUNTY PLUS MSHA STANDARDS AND SPECIFICATIONS IF APPLICABLE.
- THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS/CONSTRUCTION INSPECTIONS DIVISION AT (410) 313-1880 AT LEAST FIVE (5) WORKING DAYS PRIOR TO THE START OF WORK.
- 3. THE CONTRACTOR SHALL NOTIFY THE FOLLOWING UTILITIES OR AGENCIES AT LEAST FIVE (5) DAYS PRIOR TO ANY EXCAVATION WORK:

MISS UTILITY
C&P TELEPHONE COMPANY
HOWARD COUNTY BUREAU OF UTILITIES
AT&T CABLE LOCATION DIVISION
BALTIMORE GAS & ELECTRIC
STATE HIGHWAY ADMINISTRATION
HOWARD COUNTY DEPT. OF PUBLIC WORKS/
CONSTRUCTION INSPECTION DIVISION

1-800-257-7777
(410) 725-9976
(410) 313-4900
(410) 393-3533
(410) 531-5533

4. PROJECT BACKGROUND:
LOCATION: TAX MAP 25 &31, P/O PARCEL 75

ZONING: R-ED
ELECTION DISTRICT: 2ND
SECTION: 5
PHASE: 1
TOTAL AREA: 31.14 AC ±
DPZ FILES: S-99-01 APPROVED 7/1/99, PB 329

P-00-08 APPROVED 5/14/00

TOPOGRAPHIC INFORMATION ARE BASED ON AERIAL TOPOGRAPHIC SURVEY BY WINGS AERIAL MAPPING CO.,INC. FLOWN ON MARCH 25,1995. VERTICAL DATUM IS NAD 83.

6. COORDINATES BASED ON NAD'83 MARYLAND COORDINATE SYSTEM AS PROJECTED BY HOWARD COUNTY GEODETIC CONTROL STATIONS NO. 3147 AND 3148.

VSTA. 3147 N575598.0794, E137581.7684 EL.335.987

VSTA. 3148 N576015.4313 E1375770.4364 EL.379.248

7. BOUNDARY INFORMATION IS BASED ON MONUMENTED FIELD RUN SURVEY BY MILDENBERG, BOENDER AND ASSOC., INC. PERFORMED IN OR ABOUT MAY, 2000.

8. STORMWATER MANAGEMENT CONTROL WILL BE PROVIDED BY THE METHOD OF EXTENDED DETENTION STORMWATER MANAGEMENT FACILITIES WILL BE PRIVATELY OWNED AND MAINTAINED BY H.O.A.

 HOUSES NOT CONTROLLED BY THE SWM PONDS TO HAVE DRY WELLS AT SDP STAGE. SEE DETAIL ON SHEET 8 OF 21.

10. WETLANDS AND STREAM DELINEATION IS BY WILDMAN ENVIRONMENTAL SERVICES. DATED OCTOBER 1998.

11. FLOODPLAIN STUDY PERFORMED BY MILDENBERG, BOENDER & ASSOC., INC. IN OCTOBER 1999.

12 GEOTECHNICAL REPORT PREPARED BY HILLIS-CARNES ENGINEERING ASSOC. INC. IN OCTOBER 1999.
13. DEED REFERENCE: P/O PARCEL 4, L.380 , F. 426: P/P PARCEL 75, L. 530, F. 165.

14. NO CEMETERIES OR HISTORIC STRUCTURES EXIST ON SITE.

15. ALL EXISTING STRUCTURES AND DRIVEWAYS ARE TO BE REMOVED UNLESS OTHERWISE NOTED.16. TRAFFIC STUDY BY TRAFFIC GROUP, DATED JUNE 4, 1998.

17. PROPERTY IS LOCATED WITHIN THE METROPOLITAN DISTRICT. WATER AND SEWER ARE PUBLIC WATER CONTRACT NO 266 W SEWER PROVIDED MA PROPOSED BUMP STATION

WATER CONTRACT NO. 266-W. SEWER PROVIDED VIA PROPOSED PUMP STATION.

18. PROPOSED WATER AND SEWER ARE PUBLIC, CONTRACT # 14-3895-D.

19. LOTS 216-220 SHALL HAVE UNITS FACING COLLEGE AVE. IN ACCORDANCE WITH P.B. CASE 329, S-99-01 AND THE SCENIC ROAD GUIDELINES OF SUBDIVISION SECTION 16.125.

20. ON PROPOSED LOTS WHERE SLOPES EXCEED 10%, RETAINING WALLS, CUSTOM HOUSES AND/OR HOMES WITH WALK-OUT WILL BE PROPOSED AT THE SITE DEVELOPMENT PLAN STAGE.

21. P.B. CASE NO. 329, APPROVED ON JULY 1, 1999. PB 354 APPROVED 01/10/02

22. ALL LOTS WILL HAVE A MINIMUM OF 2 ON-SITE PARKING SPACES. NO STREET PARKING SPACES

23. THE FOREST CONSERVATION OBLIGATION IN ACCORDANCE WITH SECTION 16.1202 OF THE HOWARD COUNTY CODE AND THE FOREST CONSERVATION MANUAL HAVE BEEN SATISFIED UNDER AUTUMN VIEW SECTION 3 (F-99-45), BY THE PLACEMENT OF 41.22 ACRES IN FOREST CONSERVATION EASEMENT. AND UNDER AUTUMN VIEW SECTION 5 PHASE 1 (F-01-12), BY THE PLACEMENT OF 7.59 ACRES IN FOREST CONSERVATION EASEMENT, TOTAL FOREST CONSERVATION PROVIDED IS 48.81 ACRES, OF WHICH 43.84 IS THE REQUIRED BREAK EVEN POINT FOR THE ACREAGE OF AUTUMN VIEW, SECTION 3, 4 AND 5 PHASE 1) COMBINED. THE REMAINING 4.97 ACRES IS TO BE CREDITED

TOWARD THE REMAINING PHASES OF AUTUMN VIEW, SECTION 5.

24. TRAFFIC CONTROL DEVICES, MARKINGS, AND SIGNING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). ALL STREET AND REGULATORY SIGNS SHALL BE IN PLACE PRIOR TO THE PLACEMENT OF ANY ASPHALT.

25. COMPACTION IN FILL AREAS TO BE 95% AS DETERMINED PER AASHTO T-180.
26. CONTRACTOR TO VERIFY THE LOCATION OF ALL EXISTING UTILITIES ON SITE PRIOR TO COMMENCING CONSTRUCTION.

27. ALL STORM DRAIN PIPES TO BE HDPE PIPES UNLESS OTHERWISE NOTED.

28. ALL DRIVEWAYS ENTRANCES TO BE H.C.STD. R-6.03 UNLESS OTHERWISED NOTED.
29. STREET LIGHTS WILL BE REQUIRED IN THIS DEVELOPMENT IN ACCORDANCE WITH THE DESIGN MANUAL. STREET LIGHT PLACEMENT AND THE TYPE OF FIXTURE AND POLE SELECTED SHALL BE IN ACCORDANCE WITH THE LATEST HOWARD COUNTY DESIGN MANUAL, VOLUME III(1993) AND AS MODIFIED BY GUIDELINES FOR STREET LIGHTS IN RESIDENTIAL DEVELOPMENTS (JUNE 1993) THE JUNE 1993 POLICY INCLUDES GUIDELINES FOR LATERAL AND LONGITUDINAL PLACEMENT. A MINIMUM SPACING OF 20' SHALL BE MAINTAINED BETWEEN ANY STREET LIGHT AND ANY TREE.

30. PROVIDE 150-WATT HPS VAPOR PEDANT FIXTURE (CUTOFF) MOUNTED ON A 30' BRONZE FIBERGLASS POLE USING 12' ARM AT HIGH CASTLE ROAD, STA. 0+28, OFFSET 28' RIGHT. PROVIDE 100-WATT HPS VAPOR TRADITIONAIRE POST TOP FIXTURE MOUNTED ON A 14' BLACK FIBERGLASS POLE AT HIGH CASTLE ROAD, STA. 4+70 OFFSET 20' LEFT; STA. 7+91, OFFSET 16' LEFT; STA. 10+92, OFFSET 16' RIGHT AND AT BROADGATE CIRCLE L.P. STA. 1+92, OFFSET 3'.

AREA TABULATION:

	SECTION 3 (EXISTING)	SECTION 4	SECTION 5	SECTION 3, 4 & 5	SECTION 5 PHASE 1
GROSS AREA:	87.39 AC ±	19.08 AC ±	84.91 AC ±	191.38 AC ±	31.15 AC ±
AREA OF STEEP SLOPES:	13.20 AC. ±	2.05 AC. ±	11.44 AC ±	26.69 AC ±	6.28 AC ±
AREA OF FLOODPLAIN:	6.19 AC ±	0	2.03 AC ±	8.22 AC ±	0.90 AC ±
NET AREA:	68.00 AC ±	17.03 AC ±	71.44 AC ±	156.47 AC ±	23.97 AC ±
AREA OF PROPOSED BUILDABLE LOTS SFD:	16.46 AC ±	14.39 AC ±	32.64 AC ±	63.49 AC ±	9.93 AC ±
AREA OF PROPOSED BUILDABLE LOTS SFA:	0	0	1.12 AC ±	1.12 AC ±	0
TOTAL AREA OF PROPOSED BUILDABLE LOTS:	16.46 AC ±	14.39 AC ±	33.76 AC ±	64.61 AC ±	9.93 AC ±
AREA OF PROPOSED ROAD (R/W):	6.11 AC ±	2.12 AC ±	7.62 AC ±	15.85 AC ±	2.75 AC ±
REQUIRED OPEN SPACE (25% OF GROSS AREA):	21.85 AC ±	4.77 AC ±	21.23 AC ±	47.85 AC ±	7.88 AC ±
PROVIDED OPEN SPACE:	60.50 AC ±	6.89 AC ±	41.67 AC ±	109.06 AC ±	18.47 AC ±
NON CREDITED OPEN SPACE:	0.06 AC ±	0.08 AC ±	0.20 AC ±	0.34 AC ±	0.09 AC ±
NET OPEN SPACE:	60.44 AC ±	6.79 AC ±	41.47 AC ±	108.57 AC ±	18.38 AC ±
REQUIRED RECREATIONAL OPEN SPACE (250 S.F. PER LOT):	18,000 S.F.	14,250 S.F.	44,500 S.F.	76,750 S.F.	11,500 S.F.
PROVIDED RECREATIONAL OPEN SPACE:	18,000 S.F.	14,250 S.F.	44,500 S.F.	76,750 S.F.	15,000 S.F.
NUMBER OF BUILDABLE LOTS ALLOWED (NET AREA x 2):	136	34	142	312	48
NUMBER OF PROPOSED BUILDABLE LOTS (SFD):	72	56	156	284	46
NUMBER OF PROPOSED BUILDABLE LOTS (SFA):	0	0_	22	· 22	0
NUMBER OF PROPOSED BUILDABLE LOTS:	72	56	178_	306	46
NUMBER OF PROPOSED OPEN SPACE LOTS:	4	3	7	14	3
NUMBER OF BULK PARCELS:	1	0	0	1	0
TOTAL NUMBER OF PROPOSED LOTS:	76	59	185	320	49

AREA OF BULK PARCEL "A": 4.32 AC \pm *GROSS AREA OF PHASE 4 (INCLUDING PARCEL "A"): 23.40 AC \pm

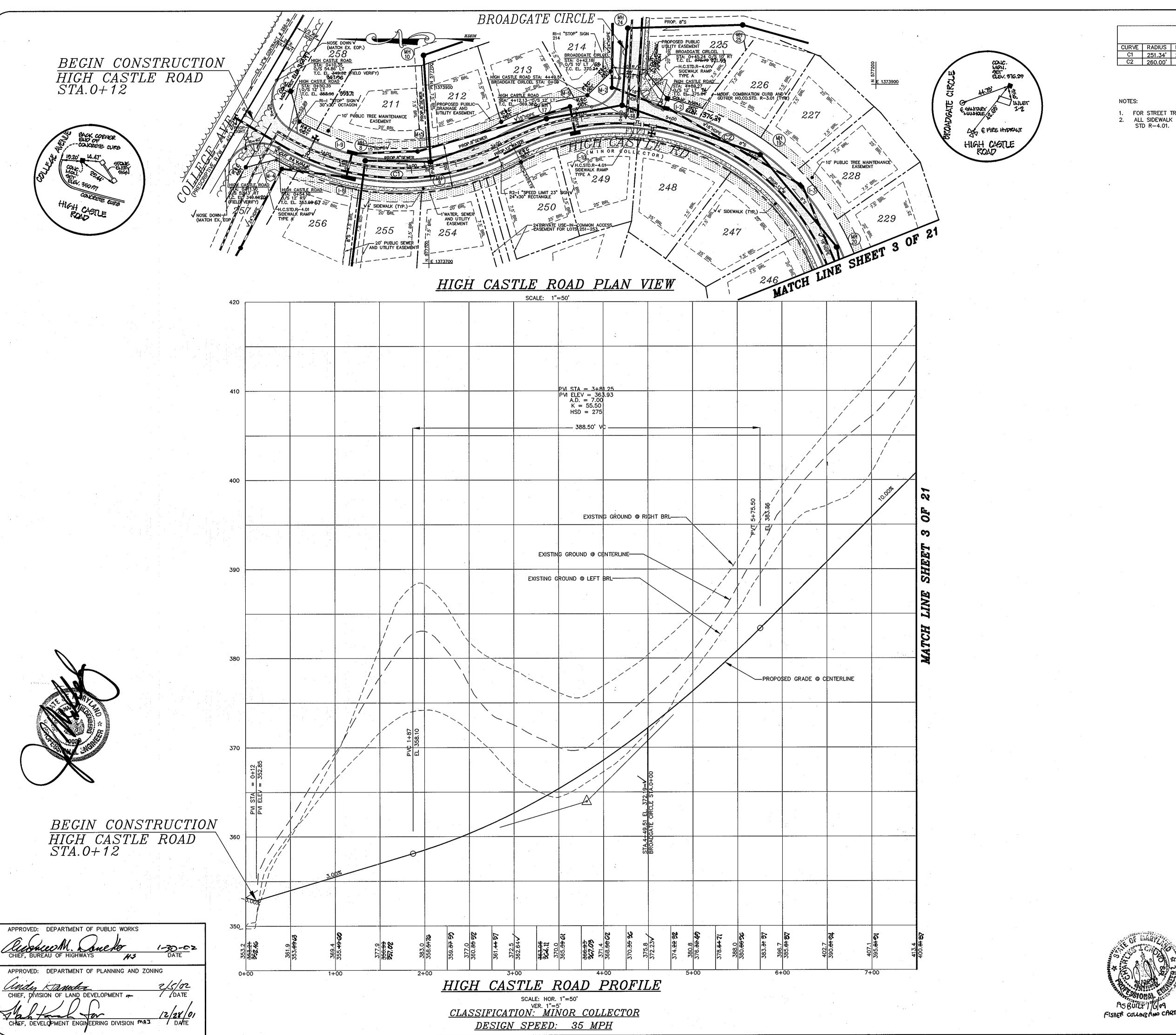
SECTION 5 SUMMARY

TOTAL NUMBER OF BUILDABEL LOTS/UNITS ALLOWED: 178
TOTAL NUMBER OF BUILDABLE LOTS/UNITS —PHASE 1—ALLOWED: 46
TOTAL NUMBER OF BUILDABLE LOTS/UNITS —PHASE 1—PROPOSED: 46
TOTAL NUMBER OF BUILDABLE LOTS/UNITS REMAIN: 132



Eng (410) 997-0296 H

CTION 211–25 P/0



CURVE TABLE
 CURVE
 RADIUS
 LENGTH
 TANGENT
 CHORD
 BEARING
 DELTA

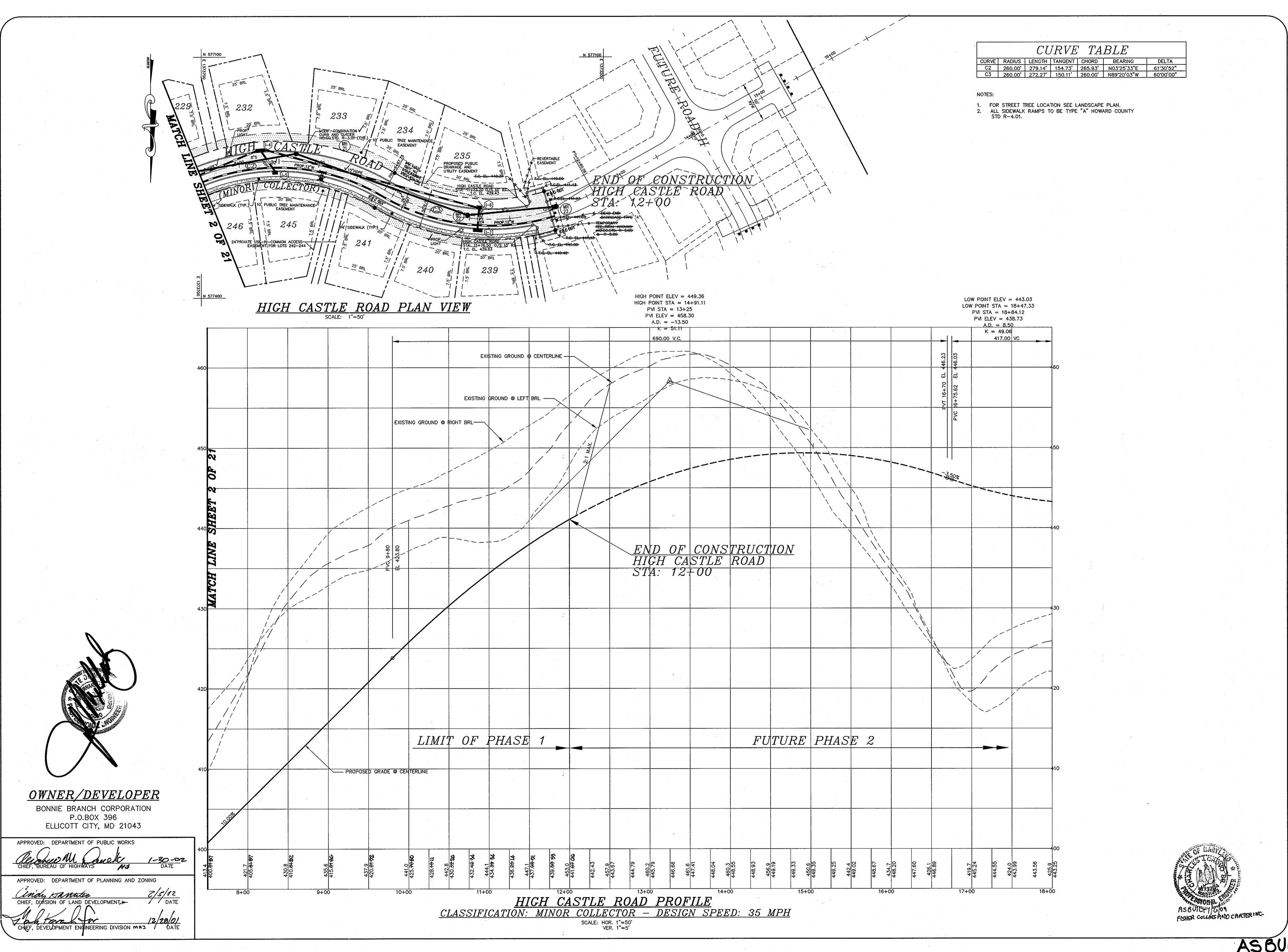
 C1
 251.34'
 263.20'
 145.11'
 251.34'
 S00'48'16"W
 60'00'00"

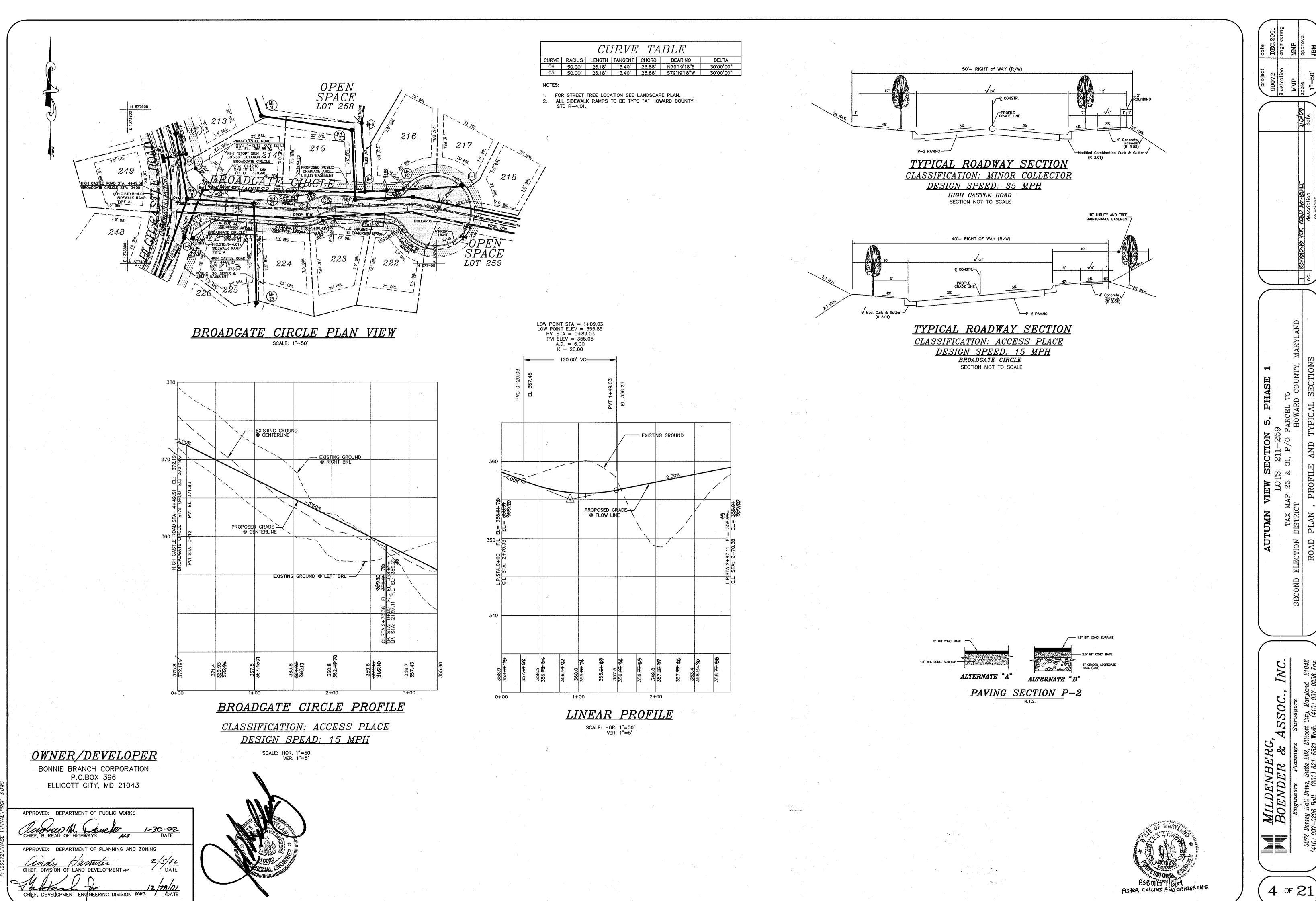
 C2
 260.00'
 279.14'
 154.73'
 265.93'
 N03'25'33"E
 61'30'52"

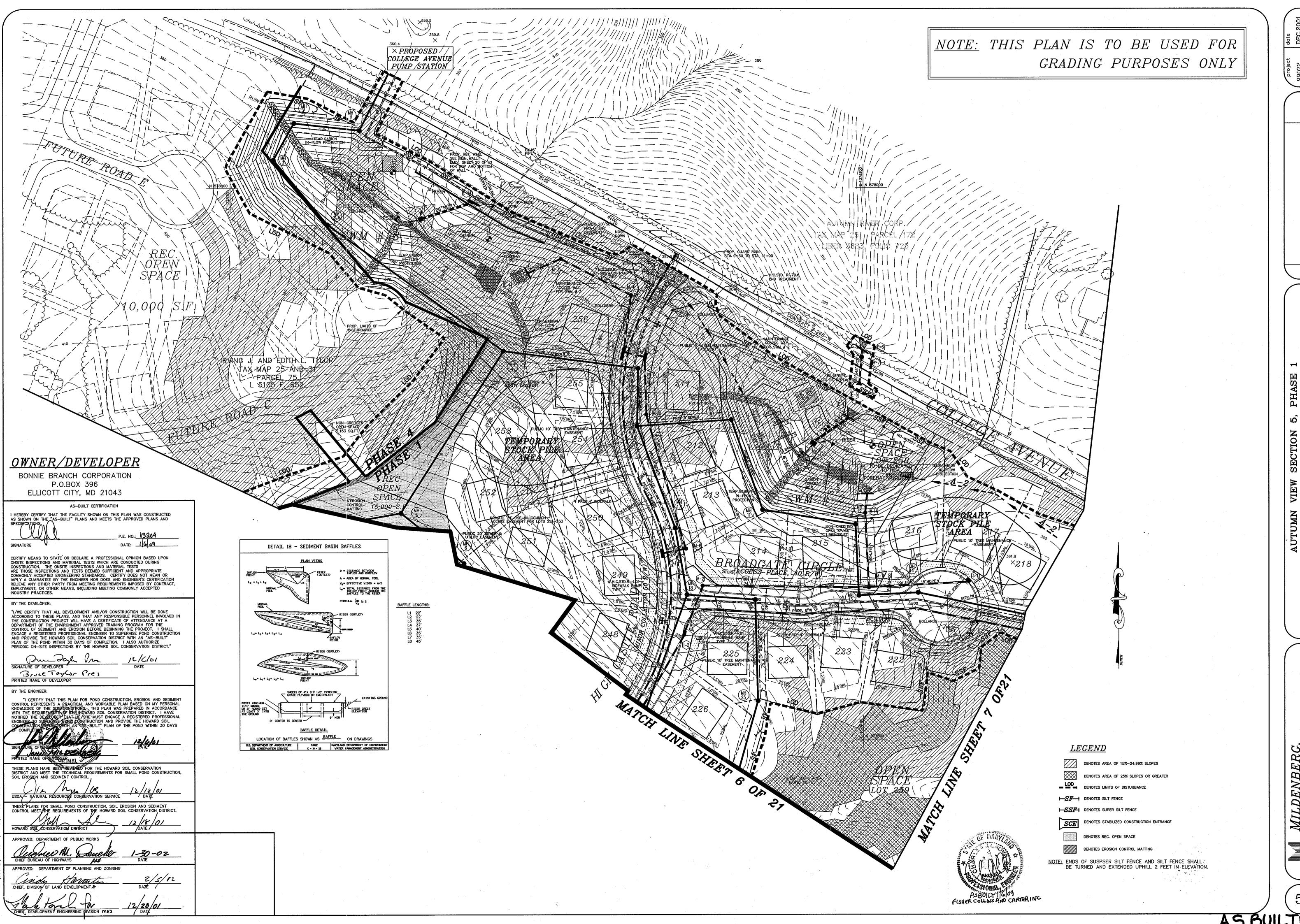
FOR STREET TREE LOCATION SEE LANDSCAPE PLAN.
 ALL SIDEWALK RAMPS TO BE TYPE "A" HOWARD COUNTY STD R-4.01.

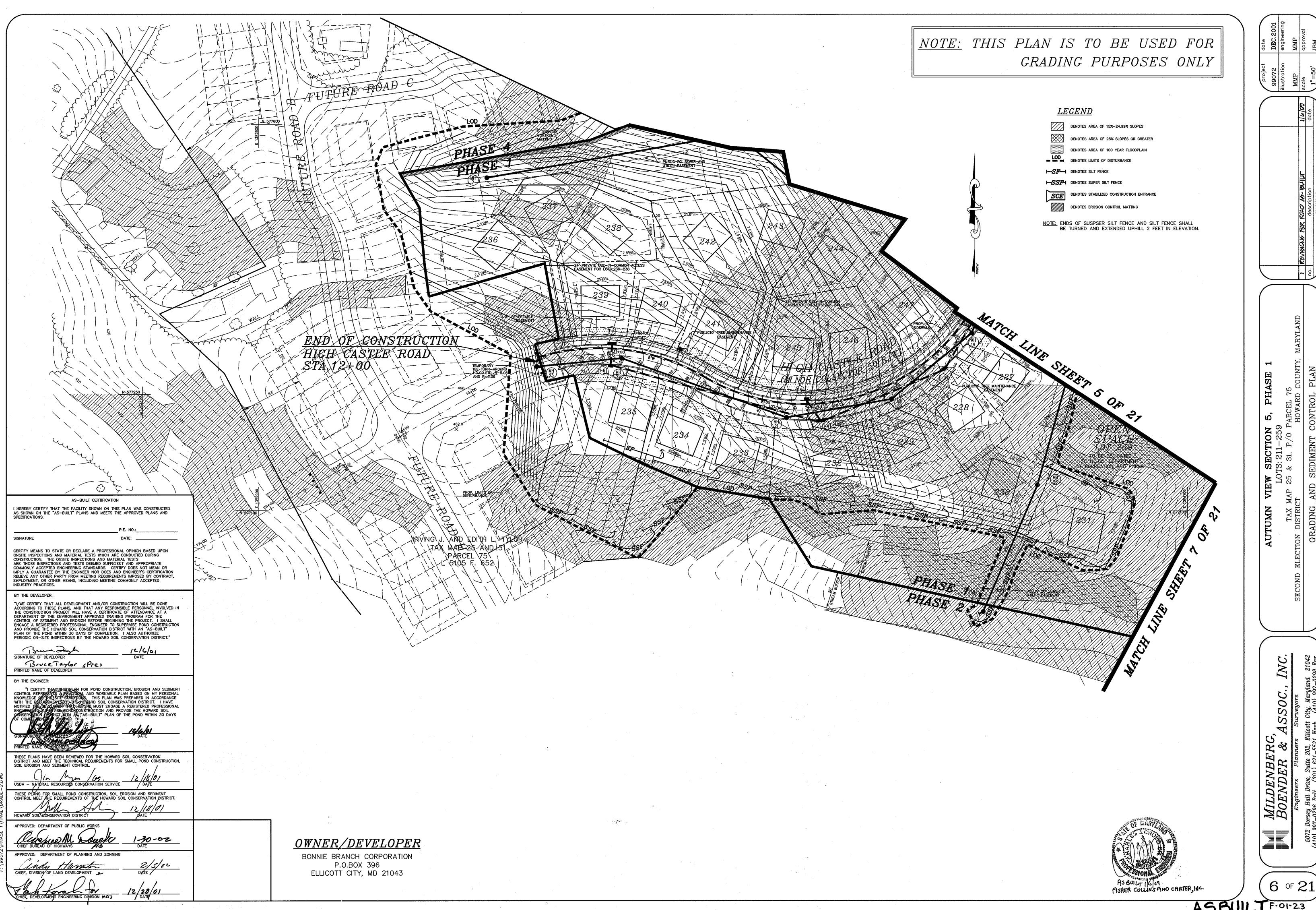
OWNER/DEVELOPER

BONNIE BRANCH CORPORATION P.O.BOX 396 ELLICOTT CITY, MD 21043











MILDENBERG, BOENDER & AS Engineers Planners St

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HOWARD SOIL CONSERVATION DISTRICT

PERMANENT SEEDING NOTES

APPLY TO GRADED OR CLEARED AREAS NOT SUBJECT TO IMMEDIATE FURTHER DISTURBANCE WHERE A PERMANENT LONG-LIVED VEGETATIVE COVER IS NEEDED. BED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISKING OR OTHER ACCEPTABLE MEANS

- BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED.
- SOIL AMENDMENTS: IN LIEU OF SOIL TEST RECOMMENDATIONS, USE ONE OF THE FOLLOWING SCHEDULES:

 1) PREFERRED APPLY 2 TONS PER ACRES DOLOMITIC LIMESTONE (92 LBS/1000 SQ.FT.)
 AND 600 LBS. PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ.FT.) BEFORE SEEDING.
 HARROW OR DISK INTO UPPER THREE INCHES OF SOIL. AT TIME OF SEEDING, APPLY
 400 LBS. PER ACRE 30-0-0 UREAFORM FERTILIZER (9 LBS./1000 SQ.FT.).

 2) ACCEPTABLE APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS./1000
 SQ.FT.) AND 1000 LBS. PER ACRE 10-10-10 FERTILIZER (23 LBS./1000 SQ.FT.) BEFORE
 SEEDING. HARROW OR DISK INTO UPPER THREE INCHES OF SOIL.

SEEDING - FOR THE PERIODS MARCH 1 THRU APRIL 30, AND AUGUST 1 THRU OCTOBER 15, SEED WITH 60 LBS. PER ACRE 1.4 LBS/1000 SQ.FT.) OF KENTUCKY 31 TALL FESCUE. FOR THE PERIOD MAY 1 THRU JULY 31, SEED WITH 60 LBS. KENTUCKY 31 TALL FESCUE PER ACRE AND 2 LOBS. PER ACRE (.05 LBS./1000 SQ.FT.) OF WEEPING LOVEGRASS. DURING THE PERIOD OF OCTOBER 16 THRU FEBRUARY 28, PROTECT SITE BY: OPTION (1) - 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING. OPTION (2) - USE SOD. OPTION (3) -SEED WITH 60 LBS./ACRE KENTUCKY 31 TALL FESCUE AND MULCH WITH 2 TONE/ACRE WELL ANCHORED STRAW.

MULCHING - APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS./1000 SQ.FT) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ.FT.) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ.FT.) FOR ANCHORING.

MAINTENANCE - INSPECT ALL SEEDING AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND RESEEDINGS.

<u>TEMPORARY SEEDING NOTES</u>

APPLY TO GRADED OR CLEARED AREAS LIKELY TO BE REDISTURBED WHERE A SHORT-TERM VEGETATIVE COVER IS NEEDED. SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISKING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, FOR NOT PREVIOUSLY LOOSENED.

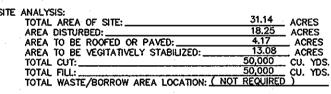
SOIL AMENDMENTS: APPLY 600 LBS. PER ACRE 10-10-10 FERTILIZER (14 LBS./1000 SQ.FT.) SEEDING: FOR PERIODS MARCH 1 THRU APRIL 30 AND FROM AUGUST 15 THRU OCTOBER 15, SEED WITH 2-1/2
BUSHEL PER ACRE OF ANNUAL RYE (3.2 LBS./1000 SQ.FT.) FOR THE PERIOD MAY 1 THRU AUGUST 14, SEED WITH 3
LBS. PER ACRE OF WEEPING LOVEGRASS (.07 LBS./1000 SQ.FT.). FOR THE PERIOD NOVEMBER 16 THRU NOVEMBER
28, PROTECT SITE BY APPLYING 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS./1000 SQ.FT.) OF UNROTTED WEED FREE SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GAL PER ACRE (5 GAL/1000 SQ.FT.) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GAL PER ACRE (8 GAL/1000 SQ.FT.) FOR ANCHORING.

REFER TO THE 1983 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR ADDITIONAL RATES AND METHODS NOT COVERED.

STANDARD SEDIMENT CONTROL NOTES

- 1) A MINIMUM OF 48 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT OF INSPECTIONS, LICENSES AND PERMITS, SEDIMENT CONTROL DIVISION PRIOR TO THE START OF NAY
- 2) ALL VEGETATIVE AND STRUCTURAL PRACTICES ARE TO BE INSTALLED ACCORDING TO. THE PROVISIONS OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE MOST CURRENT "MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT
- FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: A) 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, PERIMETER SLOPES AND ALL SLOPES GREATER THAN 3:1, B) 14 DAYS AS TO ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE.
- 4) ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 12, OF THE HOWARD COUNTY DESIGN MANUAL, STORM DRAINAGE.
- ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1991 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDING (SEC.51), SOD (SEC. 54), TEMPORARY SEEDING (SEC. 50) AND MULCHING (SEC.52). TEMPORARY STABILIZATION WITH AND SECOND WITH SECOND SECOND SECOND AND ALLOW FOR DESIGNATION OF THE PERIOD OF THE PROPERTY OF THE PERIOD OF THE MULCH ALONE CAN ONLY BE DONE WHEN RECOMMENDED SEEDING DATES DO NOT ALLOW FOR PROPER
- OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.



THESE QUANTITIES ARE FOR PERMIT PURPOSES ONLY. CONTRACTOR IS REQUIRED TO PROVIDE HIS OWN QUANTITY MEASUREMENTS.

AS-BUIL	T CERTIFICATION			iv.
	SHOWN ON THIS PLAN WAS CONSTRUCTED S AND MEETS THE APPROVED PLANS AND			VI. ALTI FER i.
	P.E. NO.:	_		-
SIGNATURE	DATE:	-		
ONSITE INSPECTIONS AND MATERIAL T CONSTRUCTION. THE ONSITE INSPECT ARE THOSE INSPECTIONS AND TESTS COMMONLY ACCEPTED ENGINEERING ST IMPLY A GUARANTEE BY THE ENGINEE	IONS AND MATERIAL TESTS DEEMED SUFFICIENT AND APPROPRIATE FANDARDS. CERTIFY DOES NOT MEAN OR R NOR DOES AND ENGINEER'S CERTIFICATION ETING REQUIREMENTS IMPOSED BY CONTRACT			iv. REFERENC EXTENSION
BY THE DEVELOPER:				CATCHOO
ACCORDING TO THESE PLANS, AND THE CONSTRUCTION PROJECT WILL HAD DEPARTMENT OF THE ENVIRONMENT A CONTROL OF SEDIMENT AND EROSION ENGAGE A REGISTERED PROFESSIONAL AND PROVIDE THE HOWARD SOIL CON PLAN OF THE POND WITHIN 30 DAYS	NT AND/OR CONSTRUCTION WILL BE DONE HAT ANY RESPONSIBLE PERSONNEL INVOLVED IVE A CERTIFICATE OF ATTENDANCE AT A PPROVED TRAINING PROGRAM FOR THE BEFORE BEGINNING THE PROJECT. I SHALL. ENGINEER TO SUPERVISE POND CONSTRUCT SERVATION DISTRICT WITH AN "AS-BUILT" OF COMPLETION. I ALSO AUTHORIZE HE HOWARD SOIL CONSERVATION DISTRICT."			
SIGNATURE OF DEVELOPER Brice Taylor P PRINTED NAME OF DEVELOPER	Pres 12/6/31 DATE	_		
BY THE ENGINEER:				
CONTROL REPRESENTS A BRAQUICAL KNOWLEDGE OF THE STIE CONGITIONS WITH THE REQUIREMENTS OF THIS HAM HE SENSINEER TO SOPERATE REAL ON CONSERVATIONS OF COMPLETIONS WITH AN ASSOCIATION OF COMPLETION OF COMPLETIONS WITH AN ASSOCIATION OF COMPLETION OF COMPLETI	R POND CONSTRUCTION, EROSION AND SEDIM AND WORKABLE PLAN BASED ON MY PERSON THIS PLAN WAS PREPARED IN ACCORDANG THIS PLAN WAS PREPARED IN ACCORDANG THE BUST ENGAGE A REGISTERED PROFESSION AND PROVIDE THE HOWARD SOIL BUILT PLAN OF THE POND WITHIN 30 DAY DATE	IAL E		
	FOR THE HOWARD SOIL CONSERVATION REQUIREMENTS FOR SMALL POND CONSTRUCT	ON,		
USDA - NATURAL RESOURCES CONSE	RVANON SERVICE /2/18/01		<i></i>	<i>1761 (\ \ 1761</i>
THESE PLANS FOR SMALL POND CONSCONTROL MEET THE REQUIREMENTS OF HOWARD SOIL CONSERVATION DISTRIC	STRUCTION, SOIL EROSION AND SEDIMENT F THE HOWARD SOIL CONSERVATION DISTRICT 2 8 0 DATE	- BO	NER/DE NNIE BRANCH P.O.BOX ELLICOTT CITY,	396
APPROVED: DEPARTMENT OF PUBLIC	WORKS		· · · · · · · · · · · · · · · · · · ·	
00 0				

8) ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF UTILITIES MUST BE REPAIRED ON THE SAME DAY OF DISTURBANCE.

- 9) ADDITIONAL SEDIMENT CONTROL MUST BE PROVIDED, IF DEEMED NECESSARY BY THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.
- 10) ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING. OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL
- 11) TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGTHS OR THAT WHICH CAN BE BACK FILLED AND STABILIZED WITHIN ONE WORKING DAY, WHICHEVER IS SHORTER.

STANDARD AND SPECIFICATIONS FOR TOPSOIL

PLACEMENT OF TOPSOIL OVER A PREPARED SUBSOIL PRIOR TO ESTABLISHMENT OF PERMANENT VEGETATION.

TO PROVIDE A SUITABLE SOIL MEDIUM FOR VEGETATIVE GROWTH. SOILS OF CONCERN HAVE LOW MOISTURE CONTENT, LOW NUTRIENT LEVELS, LOW pH, MATERIALS TOXIC TO PLANTS, AND/OR UNACCEPTABLE SOIL GRADATION.

CONDITIONS WHERE PRACTICE APPLIES

- THIS PRACTICE IS LIMITED TO AREAS HAVING 2:1 OR FLATTER SLOPES WHERE: a. THE TEXTURE OF THE EXPOSED SUBSOIL/PARENT MATERIAL IS NOT ADEQUATE TO PRODUCE VEGETATIVE
- b. THE SOIL MATERIAL IS SO SHALLOW THAT THE ROOTING ZONE IS NOT DEEP ENOUGH TO SUPPORT PLANTS OR FURNISH CONTINUING SUPPLIES OF MOISTURE AND PLANT NUTRIENTS.
- c. THE ORIGINAL SOIL TO BE VEGETATED CONTAINS MATERIAL TOXIC TO PLANT GROWTH. d. THE SOIL IS SO ACIDIC THAT TREATMENT WITH LIMESTONE IS NOT FEASIBLE
- FOR THE PURPOSE OF THESE STANDARDS AND SPECIFICATIONS, AREAS HAVING SLOPES STEEPER THAN 2:1 REQUIRE SPECIAL CONSIDERATION AND DESIGN FOR ADEQUATE STABILIZATION. AREAS HAVING SLOPES STEEPER THAN 2:1 SHALL HAVE THE APPROPRIATE STABILIZATION SHOWN ON THE PLANS.

CONSTRUCTION AND MATERIAL SPECIFICATIONS

- TOPSOIL SALVAGED FROM THE EXISTING SITE MAY BE USED PROVIDED THAT IT MEETS THE STANDARDS AS SET FORTH IN THESE SPECIFICATION. TYPICALLY, THE DEPTH OF TOPSOIL TO BE SALVAGED FOR A GIVEN SOIL TYPE CAN BE FOUND IN THE REPRESENTATIVE SOIL PROFILE SECTION IN THE SOIL SURVEY PUBLISHED BY USDA-SCS IN COOPERATION WITH MARYLAND AGRICULTURAL EXPERIMENTAL STATION.
- TOPSOIL SPECIFICATIONS SOIL TO BE USED AS TOPSOIL MUST MEET THE FOLLOWING: TOPSOIL SHALL BE A LOAM, SANDY LOAM, CLAY LOAM, SILT LOAM, SANDY CLAY LOAM, LOAMY SAND. OTHER SOILS MAY BE USED IF RECOMMENDED BY AN AGRONOMIST OR SOIL SCIENTIST AND APPROVED BY THE APPROPRIATE APPROVAL AUTHORITY. REGARDLESS, TOPSOIL SHALL NOT BE A MIXTURE OF CONTRASTING TEXTURED SUBSOILS AND SHALL CONTAIN LESS THAN 5% BY VOLUME OF CINDERS, STONES, SLAG,
- COARSE FRAGMENTS, GRAVEL, STICKS, ROOTS, TRASH, OR OTHER MATERIALS LARGER THAN 1 1/2" IN DIAMETER. ii. TOPSOIL MUST BE FREE OF PLANTS OR PLANT PARTS SUCH AS BERMUDA GRASS, QUACKGRASS, JOHNSON—SON GRASS, NUTSEDGE, POISON IVY, THISTLE, OR OTHERS AS SPECIFIED.
- WHERE THE SUBSOIL IS EITHER HIGHLY ACIDIC OR COMPOSED OF HEAVY CLAYS, GROUND LIMESTONE SHALL BE SPREAD AT THE RATE OF 4—8 TONS/ACRE (200—400 POUNDS PER 1,000 SQUARE FEET) PRIOR TO THE PLACEMENT OF TOPSOIL. LIME SHALL BE DISTRIBUTED UNIFORMLY OVER DESIGNATED AREAS AND WORKED INTO THE SOIL IN CONJUNCTION WITH TILLAGE OPERATIONS AS DESCRIBED IN THE FOLLOWING
- III. FOR SITES HAVING DISTURBED AREAS UNDER 5 ACRES:

PLACE TOPSOIL (IF REQUIRED) AND APPLY SOIL AMENDMENTS AS SPECIFIED IN <u>20.0 VEGETATIVE</u> <u>STABILIZATION</u> — SECTION I — VEGETATIVE STABILIZATION METHODS AND MATERIALS.

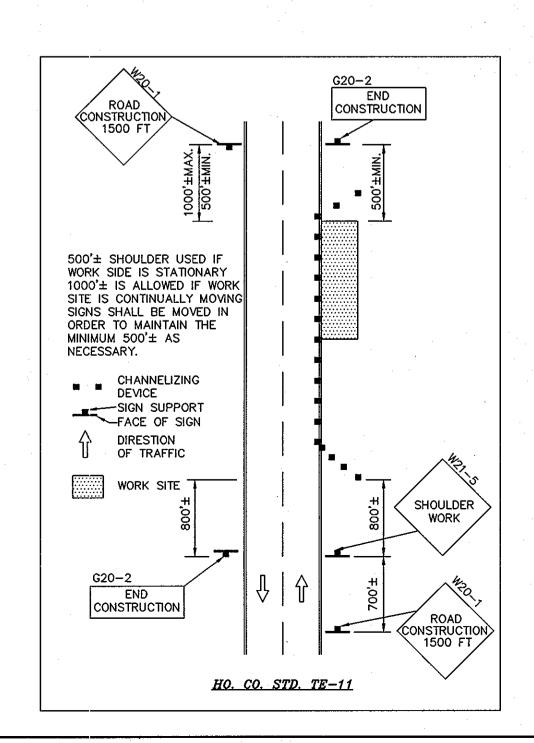
ON SOIL MEETING TOPSOIL SPI:CIFICATIONS, OBTAIN TEST RESULTS DICTATING FERTILIZER AND LIME AMENDMENTS REQUIRED TO BRING THE SOIL INTO COMPLIANCE WITH THE FOLLOWING:

- a. ph for topsoils shall be between 6.0 and 7.5. If the tested soil demonstrates a ph of less than 6.0, sufficient lime shall be perscribed to raise the ph to 6.5 or higher.
- b. ORGANIC CONTENT OF TOPSOIL SHALL BE NOT LESS THAN 1.5 PERCENT BY WEIGHT.
- c. TOPSOIL HAVING SOLUBLE SALT CONTENT GREATER THAN 500 PARTS PER MILLION SHALL NOT BE USED. d. NO SOD OR SEED SHALL BE PLACED ON SOIL WHICH HAS BEEN TREATED WITH SOIL STERILANTS OR CHEMICALS USED FOR WEED CONTROL UNTIL SUFFICIENT TIME HAS ELAPSED (14 DAYS MIN.) TO PERMIT DISSIPATION OF PHYTO-TOXIC MATERIALS.

NOTE: TOPSOIL SUBSTITUTES OR AMENDMENTS, AS RECOMMENDED BY A QUALIFIED AGRONOMIST OR SOIL SCIENTIST AND APPROVED BY THE APPROPRIATE APPROVAL AUTHORITY, MAY BE USED IN LIEU OF NATURAL

- ii. PLACE TOPSOIL (IF REQUIRED) AND APPLY SOIL AMENDMENTS AS SPECIFIED IN <u>20.0 VEGETATIVE</u> STABILIZATION SECTION I VEGETATIVE STABILIZATION METHODS AND MATERIALS.
- SURFACE RESULTING FROM TOPSOILING OR OTHER OPERATIONS SHALL BE CORRECTED IN ORDER TO PREVENT THE V. TOPSOIL APPLLICATION
- WHEN TOPSOILING, MAINTAIN NIEDED EROSION AND SEDIMENT CONTROL PRACTICES SUCH AS DIVERSIONS, GRADE STABILIZATION STRUCTURES, EARTH DIKES, SLOPE SILT FENCE AND SEDIMENT TRAPS AND BASINS.
- ii. GRADES ON THE AREAS TO BE TOPSOILED, WHICH HAVE BEEN PREVIOUSLY ESTABLISHED, SHALL BE
- III. TOPSOIL SHALL BE UNIFORMLY DISTRIBUTED IN A 4" TO 8" LAYER AND LIGHTLY COMPACTED TO A MINIMUM THICKNESS OF 4". SPREADING SHALL BE PERFORMED IN SUCH A MANNER THAT SODDING OR SEEDING CAN PROCEED WITH A MINIMUM OF ADDITIONAL SOIL PREPARATION AND TILLAGE. ANY IRREGULARITIES IN THE FORMATION OF DEPRESSIONS OR WATER POCKETS.
- IV. TOPSOIL SHALL NOT BE PLACED WHILE THE TOPSOIL OR SUBSOIL IS IN A FROZEN OR MUDDY CONDITION, WHEN THE SUBSOIL IS EXCESSIVELY WET OR IN A CONDITION THAT MAY OTHERWISE BE DETRIMENTAL TO PROPER OF THE SUBSOIL IS EXCESSIVELY WET OR IN A CONDITION THAT MAY OTHERWISE BE DETRIMENTAL TO PROPER
- VI. ALTERNATIVE FOR PERMANENT SEEDING INSTEAD OF APPLYING THE FULL AMOUNTS OF LIME AND COMMERCIAL FERTILIZER, COMPOSTED SLUDGE AND AMENDMENTS MAY BE APPLIED AS SPECIFIED BELOW: COMPOSTED SLUDGE MATERIAL FOR USE AS A SOIL CONDITIONER FOR SITES HAVING DISTURBED AREAS OVER 5 ACRES SHALL BE TESTED TO PRESCRIBE AMENDMENTS AND FOR SITES HAVING AREAS UNDER 5 ACRES SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
 - d. COMPOSTED SLUDGE SHALL BE SUPPLIED BY, OR ORIGINATE FROM, A PERSON OR PERSONS WHO ARE PERMITTED (AT THE TIME OF ACQUISITION OF THE COMPOST) BY THE MARYLAND DEPARTMENT OF THE
 - b. COMPOSTED SLUDGE SHALL CONTAIN AT LEASE 1 PERCENT NITROGEN, 1.5 PERCENT PHOSPHOURUS, AND 0.2 PERCENT POTASSIUM AND HAVE A Ph OF 7.0 TO 8.0. IF COMPOST DOES NOT MEET THESE REQUIREMENTS, THE APPROPRIATE CONSTITUENTS MUST BE ADDED TO MEET THE REQUIREMENTS PRIOR TO USE.
- c. COMPOSTED SLUDGE SHALL BE APPLIED AT A RATE OF 1 TON/1,000 SQUARE FEET. iv. COMPOSTED SLUDGE SHALL BE AMENDED WITH A POTASSIUM FERTILLIZER APPLIED AT THE RATE OF 4 LB/1,000 SQUARE FEET, AND 1/3 THE NORMAL LIME APPLICATION RATE.

REFERENCES: GUIDELINE SPECIFICATIONS, SOIL PREPARATION AND SODDING. MD-VA, PUB. #1, COOPERATIVE EXTENSION SERVICE, UNIVERSITY OF MARYLAND AND VIRGINIA POLYTECHNIC INSTITUTES. REVISED 1973.



TEMPORARY DUST CONTROL MEASURES

1. MULCHES — SEE STANDARDS FOR VEGETATIVE STABILIZATION WITH MULCHES ONLY. MULCH SHOULD BE CRIMPED OR TACKED TO PREVENT BLOWING.

2. VEGETATIVE COVER - SEE STANDARDS FOR TEMPORARY VEGETATIVE COVER. TILLAGE - TO ROUGHTN SURFACE AND BRING CLODS TO THE SURFACE. THIS IS AL EMERGENCY MEASURE WHICH SHOULD BE USED BEFORE SOIL BLOWING STARTS. BEGIN PLOWING ON WINDWARD SIDE OF SITE. CHISEL-TYPE PLOWS APCED ABOUT 12" APART, SPRING-TOOTHED HARROWS, AND SIMILAR PLOWS ARE EXAMPLES OF EQUIPMENT WHICH

4. IRRIGATION — THIS IS GENERALLY DONE AS AN EMERGENCY TREATMENT. SITE IS SPRINKLED WITH WATER UNTIL THE SURFACE IS MOIST. REPEAT AS NEEDED. AT NO TIME SHOULD THE SITE BE IRRIGATED TO THE POINT THAT RUNOFF BEGINS TO FLOW. 5. BARRIERS — SOLID BOARD FENCES, SILT FENCES, SNOW FENCES, BURLAP FENCES, STRAW BALES, AND SIMILAR MATERIAL CAN BE USED TO CONTROL AIR CURRENTS AND SOIL BLOWING. BARRIERS PLACED AT RIGHT ANGLES TO PREVAILING CURRENTS AT INTERVALT OF ABOUT 10 TIMES THEIR HEIGHT ARE EFFECTIVE IN CONTROLLING SOIL

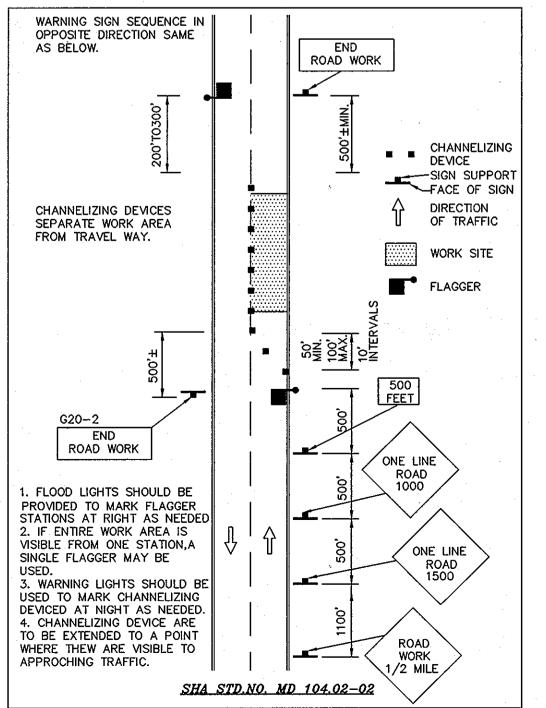
6. CALCIUM CHLORIDE — APPLY AT RATES THAT WILL KEEP SURFACE MOIST. MAY NEED RETREATMENT.

<u>SEQUENCE OF CONSTRUCTION</u>

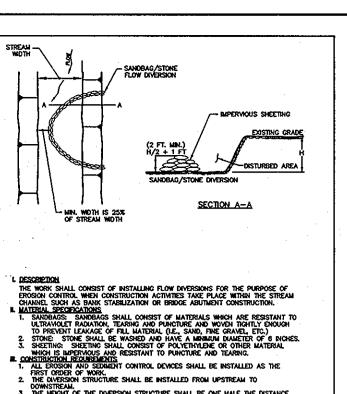
- 1. OBTAIN GRADING PERMIT.
- 2. CONSTRUCT STABILIZED CONSTRUCTION ENTRANCE WITH MOUNTABLE BERM AT LOCATION INDICATED. (1 DAY)
- 3. CONSTRUCT SILT FENCES, SUPER SILT FENCES AND TREE PROTECTION FENCES (4 DAYS)
- 4. CONSTRUCT AND STABILIZE STORMWATER MANAGEMENT PONDS 2 & 3. INCLUDING TEMPORARY LOW FLOW PLATES. CREST OF EMERGENCY SPILLWAY
- FOR SED. BASIN NO.2=339.5 FOR SED. CONTROL ONLY. PROVIDE BAFFLES FOR SEDIMENT BASIN NO. 2 PER DETAILS. (10 DAYS)
- IMMEDISTELLY STABILIZE DISTURBED AREA BELOW E-1 & E-3 WITH EROSION CONTROL MATTING, SEED AND MULCH. 6. BLOCK SWM POND RISERS AS SHOWN IN WEIR BLOCKING DETAILS ON SHEETS NO 20 OR 21. INSTALL TEMPORARY STANDPIPES.
- CONTRUCT EARTH DIKE AS INDICATED. (2 DAYS) 7. AFTER RECEIVING PERMISSION FROM THE SEDIMENT CONTROL INSPECTOR,
- CLEAR SITE PER LIMITS INDICATED. (30 DAYS)
- BRING SITE TO GRADE INDICATED ON THE PLANS, CONSTRUCT STORM DRAIN SYSTEM & UTILITIES. (PROVIDE DUST CONTROL MEASURES AS REQUIRED.(30 DAYS)
- DURING CONSTRUCTION, SEDIMENT SHALL BE REMOVED FROM THE STORMWATER MANAGEMENT PONDS WHEN THEIR CLEANOUT ELEVA-TIONS HAVE BEEN REACHED.
- 10. CONSTRUCT PAVEMENT AND CURB AND GUTTER AS INDICATED. (10 DAYS)
- 11. STABILIZE REMAINING DISTURBED AREAS. (10 DAYS) 12. WHEN ALL CONTRIBUTING DRAINAGE AREAS TO SEDIMENT CONTROL DEVICES HAVE BEEN STABILIZED AND WITH THE APPROVAL OF THE SEDIMENT CONTROL INSPECTOR, REMOVE SEDIMENT CONTROL DEVICES AND STABILZE REMAINING DISTURBED
- 13. CONTRACTOR SHALL FLUSH ALL STORM DRAIN SYSTEMS PRIOR TO CONVERSION OF SEDIMENT BASIN TO STORM WATER MANAGEMENT POND. (2 DAYS)
- 14. AFTER RECEIVING PERMISSION FROM THE SEDIMENT CONTROL INSPECTOR CONVERT SEDIMENT BASIN TO PERMANENT STORM WATER MANAGEMENT FACILITIES. REMOVE AND REPLACE PERFORATED STANDPIPES AND LOW FLOW PLATES. CONSTRUCT CREST OF EMERGENCY SPILLWAY TO 339.0 FOR SWMF # 2 AND STABILIZE.
- 15. FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN:
- A. 7 CALENDAR DAYS FOR ALL PERIMETER SLOPES AND ALL SLOPES GREATER B. 14 CALENDER DAYS FOR ALL OTHER DISTURBED AREAS ON THE SITE.

SEQUENCE OF CONSTRUCTION FOR CULVERT REPLACEMENT UNDER COLLEGE AVE.

- 1. DIVERT ALL TRAFFIC TO EXISTING WESTBOUND LANE, FLAGMEN ARE REQUIRED DURING ENTIRE DIVERSION ACTIVITY.
- 2. CONSTRUCT CULVERT TO CULVERT STATION 0+51±
- 3. BACKFILL CULVERT TO DESIGN GRADES. 4. DIVERT ALL TRAFFIC TO NEWLY CONSTRUCTED LANE.
- 6. BACKFILL REMAINING CULVERT TRENCH. 7. REFER TO MUTCD STANDARD 6B-7 FOR DETAILS NOT SHOWN.



TRAFFIC CONTROL PLAN



DOWNSTREAM.

THE HEIGHT OF THE DIVERSION STRUCTURE SHALL BE ONE HALF THE DISTANCE FROM STREAM BED TO STREAM BANK PLUS ONE FOOT, AS INDICATED ON THE

CROSS SECTION NEW.

CROSS SECTION NEW.

CROSS SECTION NEW.

CROSS SECTION NEW AS CONTINUED THE TOO THE PROPOSED OF IN A SCO APPROVED ON THE WIRE AS CONTINUED THE TOO THE PROPOSED OF THE SECTION AREA SHALL BE PUMPED TO A DEWATERING BASH PROPE TO RE-ENTERING THE STREAM.

S. ALL DEWATERING OF THE CONSTRUCTION AREA SHALL BE PUMPED TO A DEWATERING BASH PROPE TO RE-ENTERING THE STREAM.

S. SHEETING SHALL BE OVERLAPPED SHORT THE STREAM PORTION COVERS THE DOWNSTREAM PORTION WITH AT LEST AN 18-BOND OVERLAP.

7. SEDILENT CONTROL DEVICES ARE TO REMAIN IN PLACE UNITE, ALL DISTURBED AREAS ARE STABLEZED IN ACCORDANCE WITH AN APPROVED SEDILENT AND EROSION CONTROL PLAN AND THE INSPECTING AUTHORITY APPROVES THEIR REMOVED.

SANDBAG/STONE DIVERSION CHEP, WATERWAY PERMIS 2.3

DETAIL 30 - EROSION CONTROL MATTING

DETAIL 6 - GABION INFLOW PROTECTION

-ROOF LEADER

— BUILDING FOUNDATION

AWAY FROM THE ROAD.

- LEAF SCREEN

-SURCHARGE PIPE

---SPLASH BLOCK

4'x4'x4' DRY WELL

LOTS: 219 THRU 221 AND 229, 230 ,231

DRY WELLS ARE TO BE PROVIDED AT EACH DOWNSPOUT DRAINING

_CAP W/LOCK

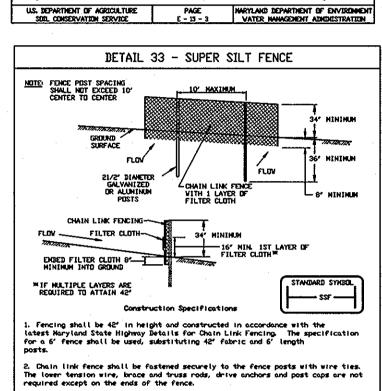
PERFORATED PVC PIPE W/CAP PERFORATION AREA

TOP & SIDES (NON-WOVEN MSHA CLASS TO

TYPICAL STAPLES NO. 11 GAUGE WIRE

СH

CROSS-SECTION



1. Fence posts shall be a minimum of 36" long driven 16" minimum into the ground. Wood posts shall be 11/2" x 11/2" square (minimum) cut, or 13/4" diamete. (minimum) round and shall be of sound quality handmood. Steet posts will be standard T or U section weighting not less than 1.00 pond per linear foot.

2. Geotextile shall be fastened securely to each fence post with wire ties or staples at top and mid-section and shall neet the following requirements

| Tensile Strength | 50 lbs/in (min.) | Testi MSMT 309 | Tensile Modulus | 20 lbs/in (min.) | Testi MSMT 309 | Flow Rote | 0.3 gol ft*/ minute (max.) | Testi MSMT 322 | Filtering Efficiency | 75% (min.) | Testi MSMT 322 | Testi MSMT 323 | Testi

3. Where ends of geotextile fabric cone together, they shall be overlapped folded and stapled to prevent sediment bypass.

DETAIL 22 - SILT FENCE

PERSPECTIVE VIEW

TOP VIEW

JOINING TWO ADJACENT SILT FENCE SECTIONS

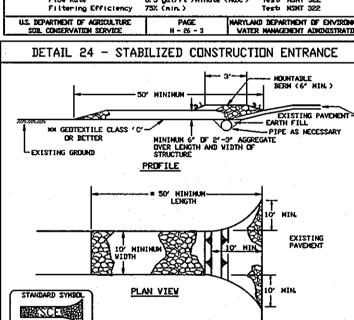
SECTION A

16' MINIMUM HEIGHT DE GEDTEXTILE CLASS F

FENCE POST DRIVEN

Chain link fence shall be fastened securely to the fence posts with wire ties.The lower tension wire, brace and truss rods, drive anchors and post caps are not required except on the ends of the fence. 3. Filter cloth shall be fastened securely to the chain link fence with ties spaced every 24° at the top and mid section. Filter cloth shall be embedded a minimum of $\boldsymbol{\theta^{\star}}$ into the ground.

5. When two sections of filter cloth adjoin each other, they shall be overlapped by 6' and folded. Maintenance shall be performed as needed and silt buildups removed when "bulges' develop in the silt fence, or when silt reaches 50% of fence height 7. Filter cloth shall be fastened securely to each fence post with wire ties or staples at top and nid section and shall neet the following requirements for Geotextile Class ${\sf Fi}$ | Tensile Strength | 50 lbs/in (nin.) | Test | MSMT 509 | Tensile Hodulus | 20 lbs/in (nin.) | Test | MSMT 509 | Flow Rate | 0.3 gal/ft*/ninute (no.) | Test | MSMT 322 | Filtering Efficiency | 75% (nin.) | Test | MSMT 322 | Test



2. Vidth - 10' minimum, should be flared at the existing road to provide a turning Geotextile fabric (filter cloth) shall be placed over the existing ground prior placing stone. SNITHE plan approval authority may not require single family isidences to use geotextile. 4. Stone – crushed aggregate (2' to 3') or reclained or recycled concrete equivalent shall be placed at least 6' deep over the length and width of the

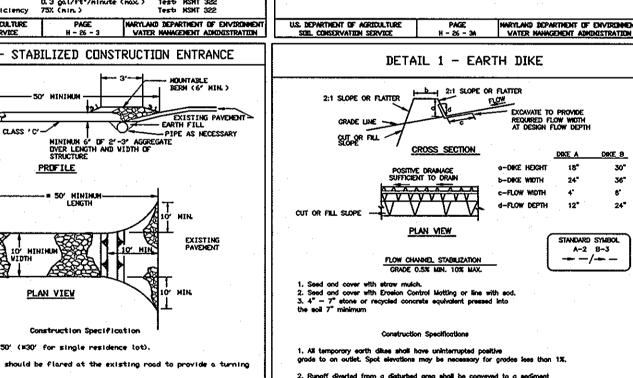
Gabion inflow protection shall be constructed of 9' \times 3' \times 9' gabion baskets forning a trapezoidal cross section 1' deep, with 2:1 side stopes, and a 3' botton width. 5. Surface Vater — all surface water flowing to or diverted toward construction entrances shall be piped through the entrance, naintaining positive drainage. Pipe installed through the stabilized construction entrance shall be protected with a 2. Geotextile Class C shall be installed under all gabion baskets. 3. The stone used to fill the gabion baskets shall be 4' = 7', 4. Gabiens shall be installed in accordance with nanufacturers reconnendations

nountable bern with 5:1 slopes and a ninimum of 6' of stone over the pips. Pipe he to be sized according to the drainage. When the SCE is located at a high spot and has no drainage to convey a pipe will not be necessary. Pipe should be sized according to the anount of runoff to be conveyed. A 6' ninimum will be required. 6. Location - A stabilized construction entrance shall be located at every point where construction traffic enters or leaves a construction site. Vehicles leaving the site must travel over the entire length of the stabilized construction entrance

Stit Fence Design Criteria Slope Length Silt Fence Length Slope Steepness Flatter than 50:1 untinited 125 feet 1,000 feet 50-1 to 10-1 10 1 to 5 1 100 feet 750 feet 60 feet 500 feet 5-1 to 3-1 40 feet 3+1 to 2-1 20 1 and steepe

SILT FENCE

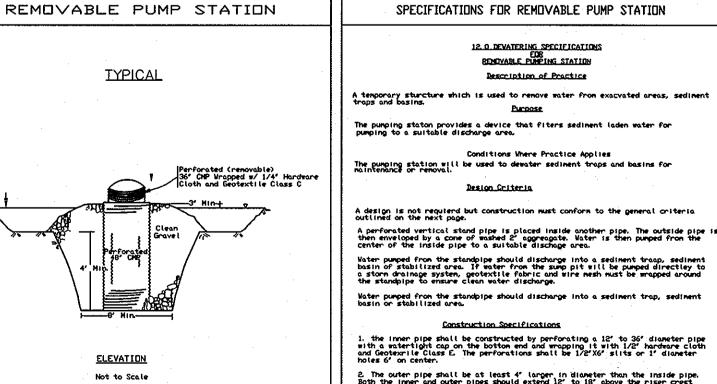
SUPER SILT FENCE Design Criteria Silt Fence Length \$ tope 0 - 10-1 L 8' HINIMU 10:1 - 5:1 200 feet 20 - 33% 51-31 100 feet 1,000 feet 31-21 100 feet



1. All temporary earth dikes shall have uninterrupted positive grade to an outlet. Spot elevations may be necessary for grades less than 1%. 2. Runoff diverted from a disturbed area shall be conveyed to a sediment 3. Runoff diverted from an undisturbed area shall outlet directly into an undisturbed, stabilized area at a non-erosive velocity. 4. All trees, brush, stumps, obstructions, and other objectional material shall be removed and disposed of so as not to interfere with the proper functioning of the dike.

5. The dike shall be excavated or shaped to line, grade and cross section as required to meet the criteria specified herein and be free of bank projections or other irregularities which will impede normal flow.

7. All earth removed and not needed for construction shall be placed so that it will not interfere with the functioning of the cities. 8. Inspection and mointenance must be provided periodically and after U.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENTS SUIL, CONSERVATION SERVICE A --1 - 6 VATER MANAGEMENT ADMINISTRATION



Mater pumped from the standpipe should discharge into a sediment trap, sediment basin or stabilized area. 1. the inner pipe shall be constructed by perforating a 12° to 36° diameter pipe with a matertight cap on the botton end and wrapping it with 1/2° hardware cloth and Geoteonile Class E. The perforations shall be 1/2°X6° slits or 1° diameter holes 6° on center.

2. The outer pipe shall be at least 4' larger in diameter than the inside pipe Both the inner and outer pipes should extend 12^{\prime} to 18^{\prime} above the riser crest elevation, or anticipated high mater elevation. Filter naterial ranging from clean gravet (ninimum fines) to 857 stone
 1/2" maximum diameter) should be backfilled around the outer pipe. 4. The suction hose from the pump shall be placed inside the inner pipe to begin denotering, the discharge hose shall be placed in a stabilized areas domistope of unstabilized areas to prevent erosion. Meadow or wooded areas are preferred discharge locations but storm drain and paved areas are acceptable. 5. <u>Maintenance</u>—The inner pipe can easily be removed to facilitate changing the geotextile when it clogs. Maintenance must be performed when the pump runs dry abd backed up water remains.

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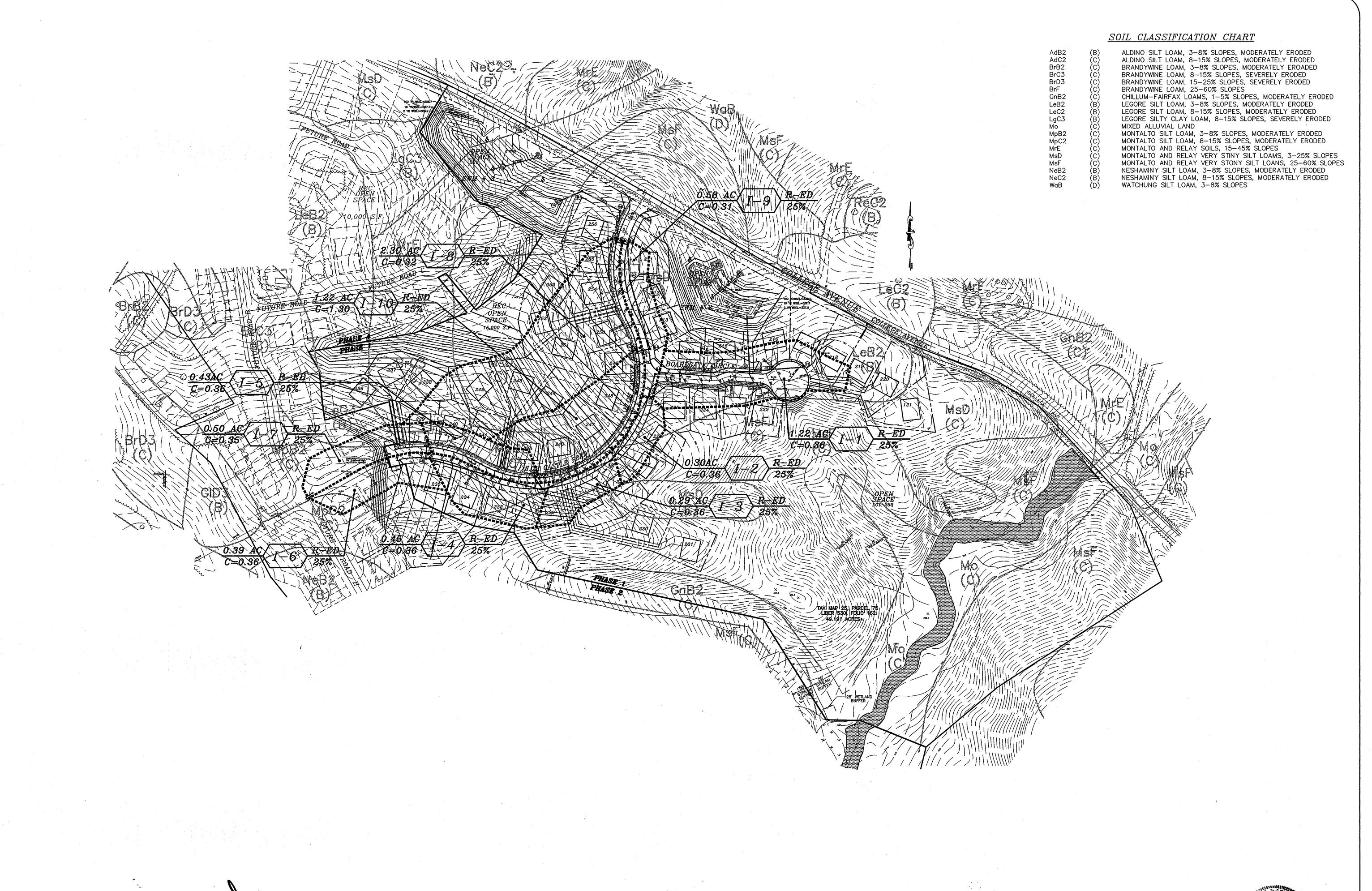
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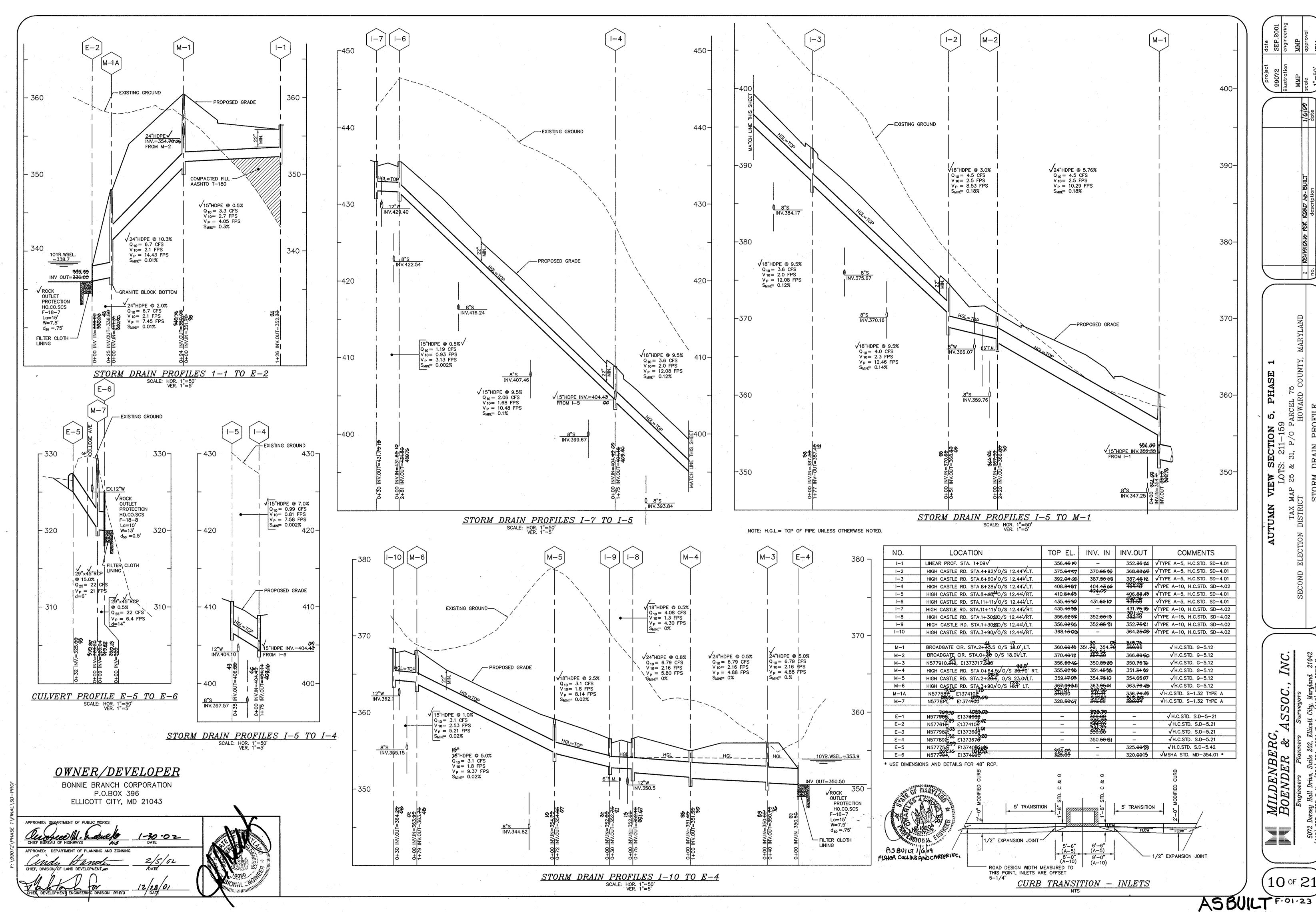
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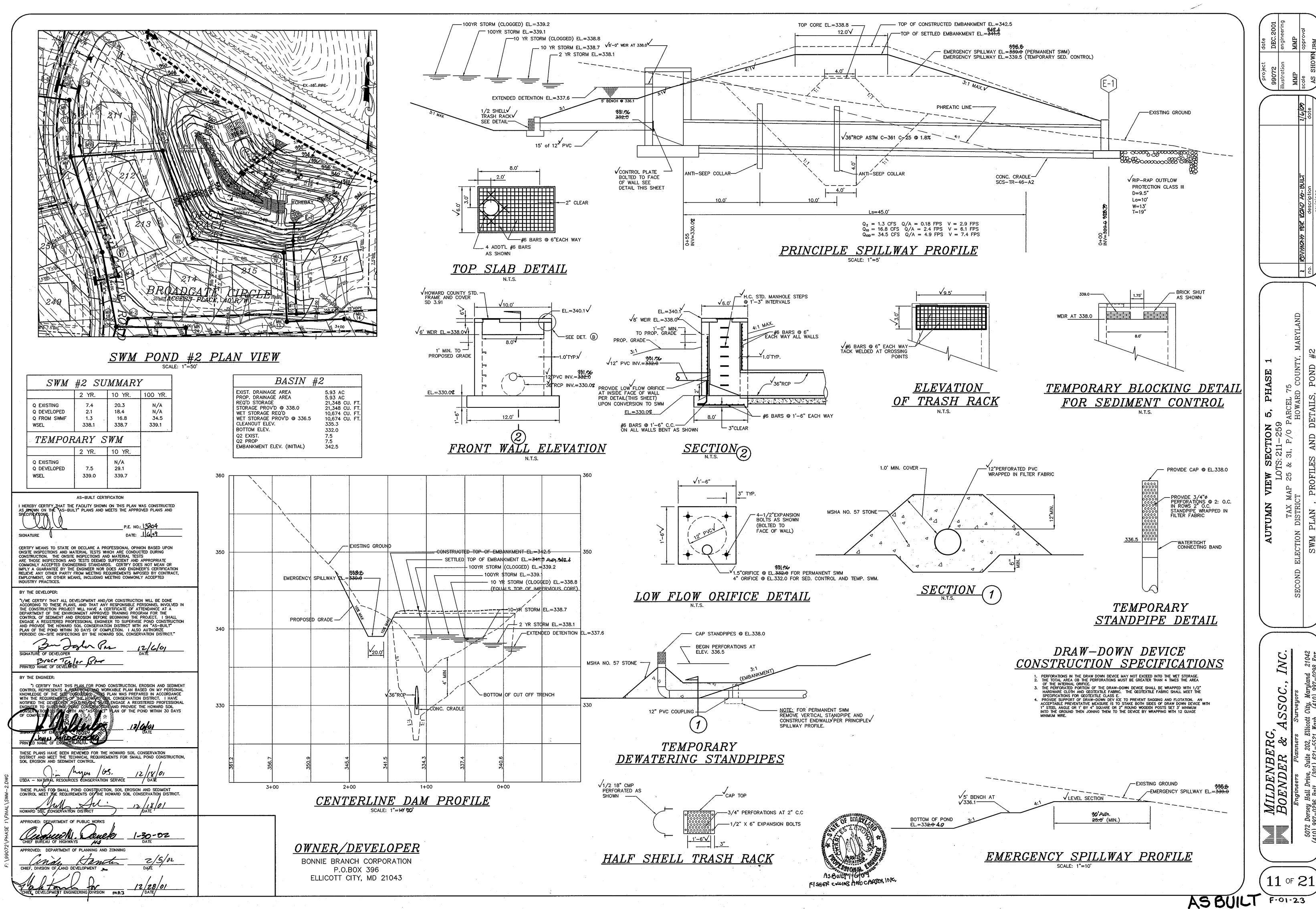


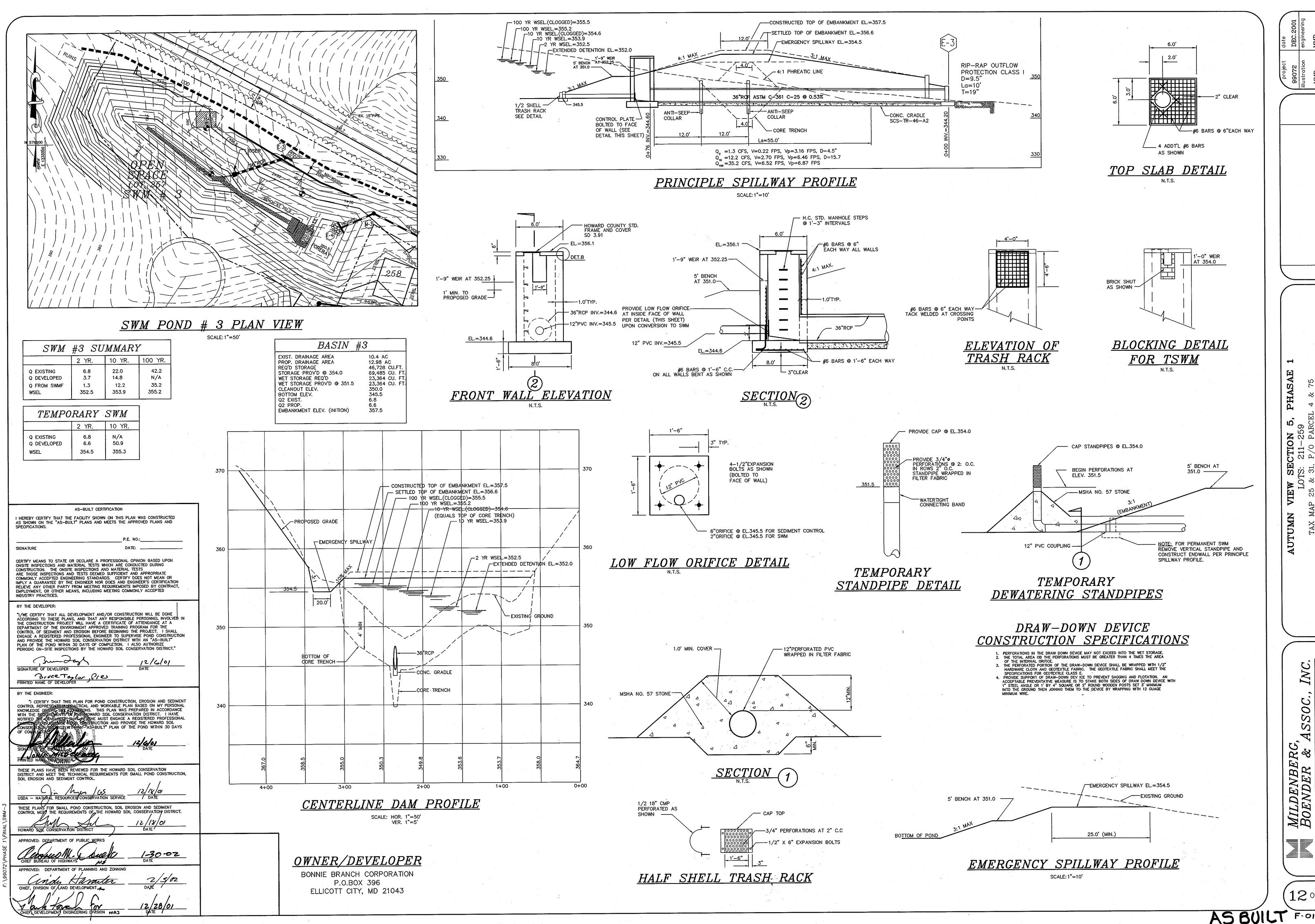
OWNER/DEVELOPER

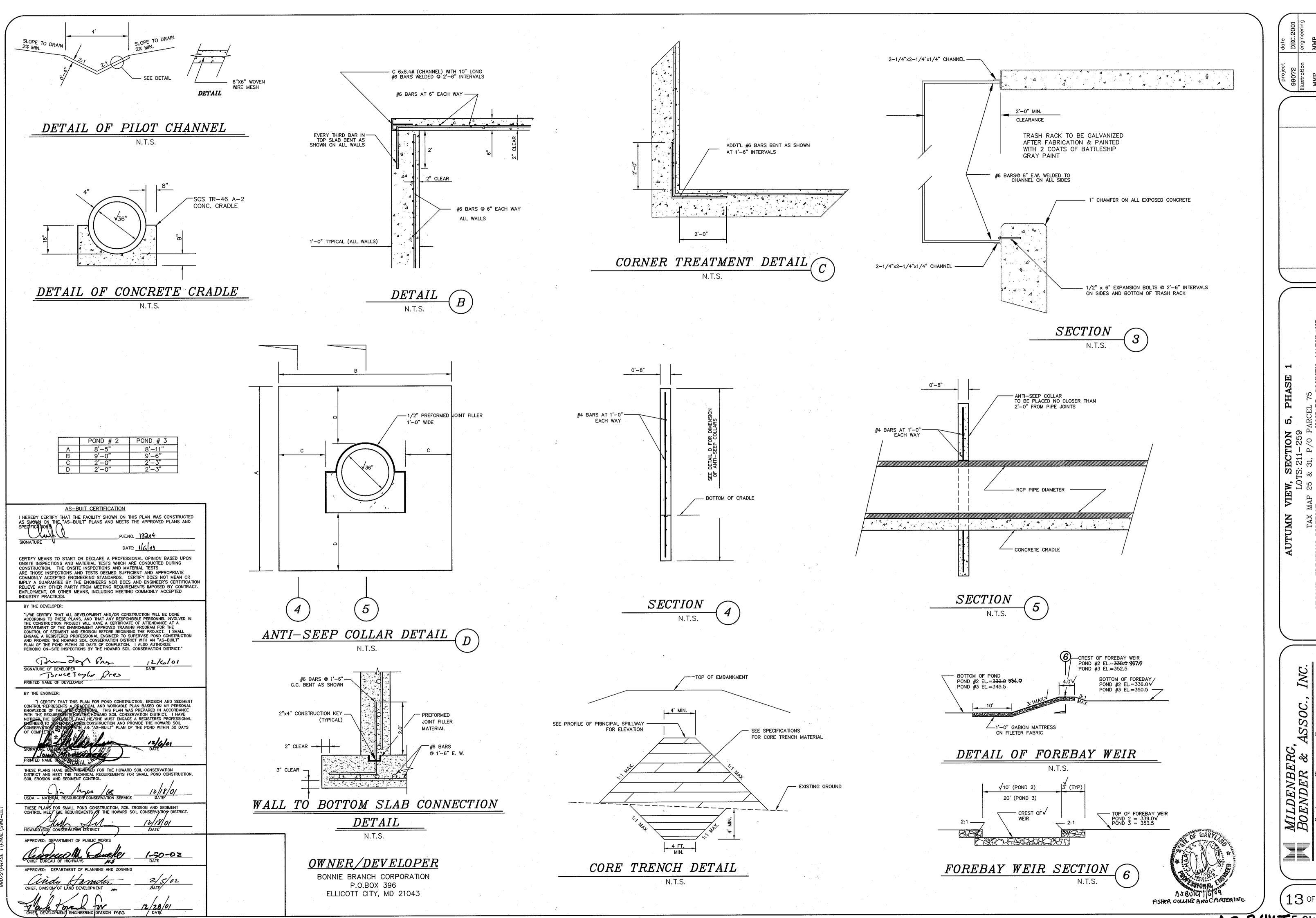
BONNIE BRANCH CORPORATION P.O.BOX 396 ELLICOTT CITY, MD 21043 ASBUILTI GOT FISHER COLLING AND CARTER INC

9 of 21









POND SPECIFICATIONS

THESE SPECIFICATIONS ARE APPROPRIATE TO ALL PONDS WITHIN THE SCOPE OF THE STANDARD FOR PRACTICE MD-378. ALL REFERENCES TO ASTM AND AASHTO SPECIFICATIONS APPLY TO THE MOST RECENT

SITE PREPARATION

AREAS DESIGNATED FOR BORROW AREAS, EMBANKMENT, AND STRUCTURAL WORKS SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL. ALL TREES, VEGETATION, ROOTS AND OTHER OBJECTIONABLE MATERIAL SHALL BE REMOVED. CHANNEL BANKS AND SHARP BREAKS SHALL BE SLOPED TO NO STEEPER THAN 1:1 AREAS TO BE COVERED BY THE RESERVOIR WILL BE CLEARED OF ALL TREES, BRUSH, LOGS, RUBBISH AND OTHER OBJECTIONABLE MATERIAL UNLESS OTHERWISE DESIGNATED TO THE PLANS. TREES, BRUSH AND STUMPS SHALL BE CUT APPROXIMATELY LEVEL WITH THE GROUND SURFACE. FOR DRY STORMWATER MANAGEMENT PONDS, A MINIMUM OF A 50 FOOT RADIUS AROUND THE INLET STRUCTURE SHALL BE CLEARED.

ALL CLEARED AND GRUBBED MATERIAL SHALL BE DISPOSED OF OUTSIDE AND BELOW THE LIMITS OF THE DAM AND RESERVOIR AS DIRECTED BY THE OWNER OR HIS REPRESENTATIVE. WHEN SPECIFIED, A SUFFICIENT QUALITY OF TOPSOIL WILL BE STOCKPILED IN A SUITABLE LOCATION FOR USE ON THE EMBANKMENT AND OTHER DESIGNATED AREAS.

EARTH FILL

MATERIAL- THE FILL MATERIAL SHALL BE TAKEN FROM APPROVED DESIGNATED BORROW AREAS. IT SHALL BE FREE OF ROOTS, STUMPS, WOOD, RUBBISH, STONES GREATER THAN 6", FROZEN OR OTHER OBJECTIONABLE MATERIALS. FILL MATERIAL FOR THE CENTER OF THE EMBANKMENT AND CUT OFF TRENCH SHALL CONFORM TO UNIFIED SOIL CLASSIFICATION GC, SC, CH, OR CL. CONSIDERATION MAY BE GIVEN TO THE USE OF OTHER MATERIALS IN THE EMBANKMENT IF DESIGN AND CONSTRUCTION ARE SUPERVISED BY A GEOTECHNICAL

PLACEMENT- AREAS ON WHICH FILL IS TO BE SHALL BE SCARIFIED PRIOR TO PLACEMENT OF FILL. FILL MATERIALS SHALL BE PLACED IN MAXIMUM 8 INCH THICK (BEFORE COMPACTION) LAYERS WHICH ARE TO BE CONTINUOUS OVER THE ENTIRE LENGTH OF THE FILL. THE MOST PERMEABLE BORROW MATERIAL SHALL BE PLACED IN THE DOWNSTREAM PORTIONS OF THE EMBANKMENT. THE PRINCIPAL SPILLWAY MUST BE INSTALLED CONCURRENTLY WITH FILL PLACEMENT AND NOT EXCAVATED INTO THE EMBANKMENT.

COMPACTION - THE MOVEMENT OF AND SPREADING EQUIPMENT OVER THE FILL SHALL BE CONTROLLED SO THAT THE ENTIRE SURFACE OF EACH LIFT SHALL BE TRAVERSE BY NOT LESS THAN ONE TREAD TRACK OF THE EQUIPMENT OR COMPACTION SHALL BE ACHIEVED BY A MINIMUM OF FOUR COMPLETE PASSES OF A SHEEPSFOOT, RUBBER TIRED OR VIBRATORY ROLLER. FILL MATERIAL SHALL CONTAIN SUFFICIENT MOISTURE SUCH THAT THE REQUIRED DEGREE OF COMPACTION WILL BE OBTAINED WITH THE EQUIPMENT USED. THE FILL MATERIAL SHALL CONTAIN SUFFICIENT MOISTURE SO THAT IF FORMED INTO A BALL IT WILL NOT CRUMBLE YET NOT BE SO WET THAT WATER CAN BE SQUEEZED OUT.

WHERE A MINIMUM REQUIRED DENSITY IS SPECIFIED, IT SHALL NOT BE LESS 95% OF MAXIMUM DRY DENSITY WITH A MOISTURE CONTENT WITHIN+- 2% OF THE OPTIMUM. EACH LAYER OF FILL SHALL BE COMPACTED AS NECESSARY TO OBTAIN THAT DENSITY, AND IS TO BE CERTIFIED BY THE ENGINEER AT THE TIME OF CONSTRUCTION. ALL COMPACTION IS TO BE DETERMINED BY AASHTO METHOD T-99

CUT OFF TRENCH- THE CUFF OFF TRENCH SHALL BE EXCAVATED INTO IMPERVIOUS MATERIAL ALONG OR PARALLEL TO THE CENTERLINE OF THE EMBANKMENT AS SHOWN ON THE PLANS. THE BOTTOM WIDTH OF THE TRENCH SHALL BE GOVERNED BY THE EQUIPMENT USED FOR EXCAVATION, WITH THE MINIMUM WIDTH BEING FOUR FEET. THE DEPTH SHALL BE AT LEAST FOUR FEET BELOW EXISTING GRADE OR AS SHOWN ON THE PLANS. THE SIDE SLOPES OF THE TRENCH SHALL BE 1 TO 1 OR FLATTER. THE BACKFILL SHALL BE COMPACTED WITH CONSTRUCTION EQUIPMENT, ROLLERS, OR HAND TAMPERS TO ASSURE MAXIMUM DENSITY AND MINIMUM PERMEABILITY.

STRUCTURE BACKFILL

BACKFILL ADJACENT TO PIPES OR STRUCTURES SHALL BE OF THE TYPE AND QUALITY CONFORMING TO THAT SPECIFIED FOR THE ADJOINING FILL MATERIAL. THE FILL MATERIAL SHALL BE PLACED IN HORIZONTAL LAYERS NOT TO EXCEED FOUR INCHES IN THICKNESS AND COMPACTED BY HAND TAMPERS OR OTHER MANUALLY DIRECTED COMPACTION EQUIPMENT. THE MATERIAL NEEDS TO FILL COMPLETELY ALL SPACES UNDER AND ADJACENT TO THE PIPE. AT NO TIME DURING THE BACKFILLING OPERATION SHALL DRIVEN EQUIPMENT BI ALLOWED TO OPERATE CLOSER THAN FOUR FEET, MEASURED HORIZONTALLY, TO ANY PART OF A STRUCTURE. UNDER NO CIRCUMSTANCES SHALL EQUIPMENT BE DRIVEN OVER ANY PART OF A CONCRETE FILL OF 24" OR GREATER OVER THE STRUCTURE OR PIPE.

PIPE CONDUITS

ALL PIPES SHALL BE CIRCULAR IN CROSS SECTION.

CORRUGATED METAL PIPE- ALL OF THE FOLLOWING CRITERIA SHALL APPLY FOR CORRUGATED METAL PIPE: 1. MATERIALS— (STEEL PIPE)— THIS PIPE AND ITS APPURTENANCE SHALL BE GALVANIZED AND FULLY BITUMINOUS COATED AND SHALL CONFORM TO THE REQUIREMENTS OF AASHTO SPECIFICATION M-190 TYPE A WITH WATERTIGHT COUPLING BANDS. ANY BITUMINOUS COATING DAMAGED OR OTHERWISE REMOVED SHALL BE REPLACED WITH COLD APPLIED BITUMINOUS COATING COMPOUND. STEEL PIPES WITH POLYMERIC COATINGS SHALL HAVE A MINIMUM COATING THICKNESS OF 0.01 INCH (10 MIL) ON BOTH SIDES OF THE PIPE. THE FOLLOWING COATINGS OR AN APPROVED EQUAL MAY BE USED: NEXON, PLASTI-COTE, BLAC-KLAD, AND BETH-CU-LOY. COATED CORRUGATED STEEL PIPE SHALL MEET THE REQUIREMENTS OF AASHTO M-245 AND M-246.

MATERIALS- (ALUMINUM COATED STEEL PIPE)- THIS PIPE AND ITS APPURTENANCES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO SPECIFICATION M-274 WITH WATERTIGHT COUPLING BANDS OR FLANGES. ANY ALUMINUM COATING DAMAGED OF OTHERWISE REMOVED SHALL BE REPLACED WITH COLD APPLIED BITUMINOUS COATING COMPOUND.?

MATERIALS-(ALUMINUM PIPE)- THIS PIPE AND ITS APPURTENANCES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO SPECIFICATION M-196 OR M-211 WITH WATERTIGHT COUPLINGS BANDS OR FLANGES. ALUMINUM SURFACES THAT ARE TO BE IN CONTACT WITH CONCRETE SHALL BE PAINTED WITH ONE COAT OF ZINC CHROMATE PRIMER. HOT DIP GALVANIZED BOLTS MAY BE USED FOR CONNECTIONS. THE PH OF THE SURROUNDING SOILS SHALL BE BETWEEN 4 AND 9.

2. COUPLING BANDS, ANTI-SEEP COLLARS, END SECTIONS, ETC., MUST BE COMPOSED OF THE SAME TERIALS WITH USE RUBBER OR

AS-8UILT	CERTIFICATION	
	SHOWN ON THIS PLAN WAS CONSTRUCTED AND MEETS THE APPROVED PLANS AND	
	P.E. NO.:	
SIGNATURE	DATE:	
ONSITE INSPECTIONS AND MATERIAL TES CONSTRUCTION. THE ONSITE INSPECTION ARE THOSE INSPECTIONS AND TESTS DE COMMONLY ACCEPTED ENGINEERING STA IMPLY A GUARANTEE BY THE ENGINEER	NS AND MATERIAL TESTS EMED SUFFICIENT AND APPROPRIATE NDARDS. CERTIFY DOES NOT MEAN OR NOR DOES AND ENGINEER'S CERTIFICATION ING REQUIREMENTS IMPOSED BY CONTRACT,	
BY THE DEVELOPER:		
ACCORDING TO THESE PLANS, AND THA THE CONSTRUCTION PROJECT WILL HAV DEPARTMENT OF THE ENVIRONMENT AND CONTROL OF SEDIMENT AND EROSION BENGAGE A REGISTERED PROFESSIONAL I AND PROVIDE THE HOWARD SOIL CONSEPLAN OF THE POND WITHIN 30 DAYS O	EFORE BEGINNING THE PROJECT. I SHALL ENGINEER TO SUPERVISE POND CONSTRUCTION ERVATION DISTRICT WITH AN "AS-BUILT"	
Brue Dayler Pr SIGNATURE OF DEVELOPER	Lee 12/6/01 DATE	
Bruce Taylor , Pr	'es	_
CONTROL REPRESENTS A PRACTICAL AN KNOWLEDGE OF THE SITE CONDITIONS. WITH THE REQUIREMENTS OF THE HOWANDTIFFED THE DEVELOPER THAT HE (SHE	POND CONSTRUCTION, EROSION AND SEDIMENT NO WORKABLE PLAN BASED ON MY PERSONAL THIS PLAN WAS PREPARED IN ACCORDANCE IN SOIL CONSERVATION DISTRICT. I HAVE E MUST ENGAGE A REGISTERED PROFESSIONAL WORLD AND PROVIDE THE HOWARD SOIL BULT PLAN OF THE POND WITHIN 30 DAYS	
SIGNATURE OF ENGINEER JOHN MILDENBER PRINTED NAME OF ENGINEER	12/4/01 DATE	
THESE PLANS HAVE BEEN REVIEWED FO DISTRICT AND MEET THE TECHNICAL RE SOIL EROSION AND SEDIMENT CONTROL	QUIREMENTS FOR SMALL POND CONSTRUCTION,	
USDA - WATURAL RESOURCES CONSER	VATION SERVICE / DATE	
THESE PLANS FOR SMALL POND CONST	RUCTION, SOIL EROSION AND SEDIMENT THE HOWARD SOIL CONSERVATION DISTRICT.	- <u>OWNER/DEVELOPER</u>
HOWARD SOIL CONSERVATION DISTRICT	1/17/02	BONNIE BRANCH CORPORATION P.O.BOX 396 ELLICOT CITY, MD 21043
APPROVED: DEPARIMENT OF PUBLIC W	OBKS /	222.55.1, 115.2.5.15
AFFICUED: DEPARTMENT OF PUBLIC W		Name of the last o

Mute

CHIEF, DIVISION OF LAND DEVELOPMENT W.

3. CONNECTIONS- ALL CONNECTIONS WITH PIPES MUST BE COMPLETELY WATERTIGHT. THE DRAIN PIPE OR BARREL CONNECTION TO THE RISER SHALL BE WELDED ALL AROUND WHEN THE PIPE AND RISER ARE METAL. ANTI-STEEP COLLARS SHALL BE CONNECTED TO THE PIPE IN SUCH A MANNER AS TO BE COMPLETELY WATERTIGHT. DIMPLE BANDS ARE NOT CONSIDERED TO BE WATERTIGHT.

ALL CONNECTIONS SHALL USE A RUBBER OF NEOPRENE GASKET WHEN JOINING PIPE SECTIONS. THE END OF EACH PIPE SHALL BE-ROLLED AND ADEQUATE NUMBER OF CORRUGATIONS TO ACCOMMODATE THE BAND WIDTH. THE FOLLOWING TYPE CONNECTIONS ARE ACCEPTABLE FOR PIPE LESS THAN 24" IN DIAMETER: FLANGES ON BOTH ENDS OF THE PIPE, A 12" WIDE STANDARD LAP TYPE BAND WITH 12" WIDE BY 3/8" THICK CLOSED CELL CIRCULAR NEOPRENE GASKET; AND A 12" WIDE HUGGER TYPE BAND WITH 0-RING GASKETS HAVING MINIMUM DIAMETER OF 1/2" GREATER THAN THE CORRUGATION DEPTH. PIPES 24: IN DIAMETER AND LARGER SHALL BE CONNECTED BY A 24" LONG ANNULAR CORRUGATED BAND USING RODS AND LUGS. A 12" WIDE BY 3/8" THICK CLOSED CELL CIRCULAR NEOPRENE GASKET WILL BE INSTALLED ON THE END OF EACH PIPE FOR A TOTAL OF 24"

HELICALLY CORRUGATED PIPE SHALL HAVE EITHER CONTINUOUSLY WELDED SEAMS OR HAVE LOCK SEAMS WITH INTERNAL CAULKING OR A NEOPRENE BEAD.

4. BEDDING- THE PIPE SHALL BE FIRMLY AND UNIFORMLY BEDDED THROUGHOUT ITS ENTIRE LENGTH. WHERE ROCK OR SOFT, SPONGY OR OTHER UNSTABLE SOIL IS ENCOUNTERED ALL SUCH MATERIAL SHALL BE REMOVED AND REPLACED WITH SUITABLE EARTH COMPACTED TO PROVIDE ADEQUATE SUPPORT.

5. BACKFILLING SHALL CONFORM TO "STRUCTURE BACKFILL." 6. OTHER DETAILS (ANTI-SEEP COLLARS, VALVES, ETC.) SHALL BE AS SHOWN ON THE DRAWINGS.

REINFORCED CONCRETE PIPE- ALL OF THE FOLLOWING CRITERIA SHALL APPLY FOR REINFORCED CONCRETE

1. MATERIALS-REINFORCED CONCRETE PIPE SHALL HAVE BELL AND SPIGOT JOINTS WITH RUBBER GASKETS AND SHALL EQUAL OR EXCEED ASTM DESIGNATION C-361.

2. BEDDING- ALL REINFORCED CONCRETE PIPE CONDUITS SHALL BE LAID IN A CONCRETE BEDDING FOR THEIR ENTIRE LENGTH. THIS BEDDING SHALL CONSIST OF HIGH SLUMP CONCRETE PLACED UNDER THE PIPE AND UP THE SIDES OF THE PIPE AT LEAST 10% OF ITS OUTSIDE DIAMETER WITH A MINIMUM THICKNESS OF 3 INCHES, OR AS SHOWN ON THE DRAWINGS.

3. LAYING PIPE- BELL AND SPIGOT PIPE SHALL BE PLACED WITH THE BELL END UPSTREAM. JOINTS SHALL BE MADE IN ACCORDANCE WITH RECOMMENDATIONS OF THE MANUFACTURER OF THE MATERIAL. AFTER THE JOINTS ARE SEALED FOR THE ENTIRE LINE, THE BEDDING SHALL BE PLACED SO THAT ALL SPACES UNDER THE PIPE ARE FILLED. CARE SHALL BE EXERCISED TO PREVENT ANY DEVIATION FROM THE ORIGINAL LINE AND GRADE OF THE PIPE. THE FIRST JOINT MUST BE LOCATED WITHIN 2 FEET FROM THE RISER.

4. BACKFILLING SHALL CONFORM TO "STRUCTURE BACKFILL."

5. OTHER DETAILS (ANTI-SEEP COLLARS, VALVES, ETC.) SHALL BE AS SHOWN ON THE DRAWINGS. POLYVINYL CHLORIDE (PVC) PIPE- ALL OF THE FOLLOWING CRITERIA SHALL APPLY FOR POLYVINYL CHLORIDE (PVC) PIPE:

1. MATERIALS-PVC PIPE SHALL BE PVC-1120 OR PVC-1220 CONFORMING TO ASTM D-1785 OR

2. JOINTS AND CONNECTIONS TO ANTI-SEEP COLLARS SHALL BE COMPLETELY WATERTIGHT.

3. BEDDING- THE PIPE SHALL BE FIRMLY AND UNIFORMLY BEDDED THROUGHOUT ITS ENTIRE LENGTH. WHERE ROCK OR SOFT, SPONGY OR OTHER UNSTABLE SOIL IS ENCOUNTERED, ALL SUCH MATERIAL SHALL BE REMOVED AND REPLACED WITH SUITABLE EARTH COMPACTED TO PROVIDE ADEQUATE SUPPORT.

4. BACKFILLING SHALL CONFORM TO "STRUCTURE BACKFILL."

5. OTHER DETAILS (ANTI-SEEP COLLARS, VALVES, ETC.) SHALL BE AS SHOWN ON THE DRAWINGS.

CONCRETE SHALL MEET THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION, STATE HIGHWAY ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION 905.

THE RIPRAP SHALL BE PLACED TO THE REQUIRED THICKNESS IN ONE OPERATION. THE ROCK SHALL BE DELIVERED AND PLACED IN A MANNER THAT WILL INSURE THE RIPRAP IN PLACE SHALL BE REASONABLY HOMOGENOUS WITH THE LARGER ROCKS UNIFORMLY DISTRIBUTED AND FIRMLY IN CONTACT ONE TO ANOTHER WITH THE SMALLER ROCKS FILLING THE VOIDS BETWEEN THE LARGER ROCKS. FILTER CLOTH SHALL BE REPLACED UNDER ALL RIPRAP AND SHALL MEET THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION, STATE HIGHWAY ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION

CARE OF WATER DURING CONSTRUCTION

ALL WORK ON THE PERMANENT STRUCTURES SHALL BE CARRIED OUT IN AREAS FREE FROM WATER. THE CONTRACTOR SHALL CONSTRUCT AND MAINTAIN ALL TEMPORARY DIKES, LEVEES, COFFERDAMS, DRAINAGE CHANNELS, AND STREAM DIVERSIONS NECESSARY TO PROTECT THE AREAS TO BE OCCUPIED BY THE PERMANENT WORKS. THE CONTRACTOR SHALL ALSO FURNISH, INSTALL, OPERATE AND MAINTAIN ALL NECESSARY PUMPING AND OTHER EQUIPMENT REQUIRED FOR REMOVAL OF WATER FROM THE VARIOUS PARTS OF THE WORK AND FOR MAINTAINING THE EXCAVATIONS, FOUNDATION AND OTHER PARTS OF THE WORK FREE FROM WATER AS REQUIRED OR DIRECTED BY THE ENGINEER FOR CONSTRUCTING EACH PART OF THE WORK. AFTER HAVING SERVED THEIR PURPOSE, ALL TEMPORARY PROTECTIVE WORKS SHALL BE REMOVED OR LEVELED AND GRADED TO THE EXTENT REQUIRED TO PREVENT OBSTRUCTION IN ANY DEGREE WHATSOEVER OF THE FLOW OF WATER TO THE SPILLWAY OR OUTLET WORKS AND SO AS NOT TO INTERFERE IN ANY WAY WITH THE OPERATION OR MAINTENANCE OF THE STRUCTURE. STREAM DIVERSIONS SHALL BE MAINTAINED UNTIL THE FULL FLOW CAN BE PASSED THROUGH THE PERMANENT WORKS. THE REMOVAL OF WATER FROM THE REQUIRED EXCAVATION AND THE FOUNDATION SHALL BE ACCOMPLISHED IN A MANNER AND TO THE EXTENT THAT WILL MAINTAIN STABILITY OF THE EXCAVATED SLOPES AND BOTTOM OF THE REQUIRED EXCAVATIONS AND WILL ALLOW SATISFACTORY PERFORMANCE OF ALL AND CONSTRUCTION OPERATIONS. DURING THE PLACING AND COMPACTING OF MATERIAL IN REQUIRED EXCAVATIONS, THE WATER LEVEL AT THE LOCATIONS BEING REFILLED SHALL BE MAINTAINED BELOW THE BOTTOM OF THE EXCAVATION AT SUCH LOCATIONS WHICH MAY REQUIRE DRAINING THE WATER TO SUMPS FROM WHICH THE WATER SHALL BE PUMPED.

STABILIZATION

ALL BORROW AREAS SHALL BE GRADED TO PROVIDE PROPER DRAINAGE AND LEFT IN A SLIGHTLY CONDITION.
ALL EXPOSED SURFACES OF THE EMBANKMENT, SPILLWAY, SPOIL AND BORROW AREAS, AND BERMS SHALL BE STABILIZED BY SEEDING, LIMING, FERTILIZING AND MULCHING IN ACCORDANCE WITH THE MARYLAND SOIL CONSERVATION SERVICE STANDARDS AND SPECIFICATIONS FOR CRITICAL AREA PLANTING (MD-342) OR AS SHOWN ON THE ACCOMPANYING DRAWINGS.

EROSION AND SEDIMENT CONTROL

CONSTRUCTION OPERATIONS WILL BE CARRIED OUT IN SUCH A MANNER THAT EROSION WILL BE CONTROLLED AND WATER AND AIR POLLUTION MINIMIZED. STATE AND LOCAL LAWS CONCERNING POLLUTION ABATEMENT WILL BE FOLLOWED. CONSTRUCTION PLANS SHALL DETAIL EROSION AND SEDIMENT CONTROL MEASURES TO BE EMPLOYED DURING THE CONSTRUCTION PROCESS.

OPERATION, MAINTENANCE AND INSPECTION

INSPECTION OF THE POND(S) SHOWN HEREON SHALL BY PERFORMED AT LEAST ANNUALLY, IN ACCORDANCE WITH THE CHECKLIST AND REQUIREMENTS CONTAINED WITHIN USDA. SCS "STAND-ARDS AND SPECIFICATIONS FOR PONDS" (MD-378), THE POND OWNER(S) AND THE HEIRS SUCCESSORS OR ASSIGNS SHALL BE RESPONSIBLE FOR THE SAFETY OF THE POND AND THE CONTINUED OPERATION, SURVEILLANCE, INSPECTION AND MAINTENANCE THEREOF. THE POND OWNER(S) SHALL PROMPTLY NOTIFY THE SOIL CONSERVATION DISTRICT OF ANY UNUSUAL OBSERVATIONS THAT MAY BE INDICATIONS OF DISTRESS SUCH AS EXCESSIVE SEEPAGE, TURBID SEEPAGE, SLIDING OR SLUMPING.

OPERATION. MAINTENANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED EXTENDED DETENTION POND

- ROUTINE MAINTENANCE:

 1. FACILITY SHALL BE INSPECTED ANNUALLY AND AFTER MAJOR STORMS. INSPECTIONS SHALL BE PERFORMED DURING WET WEATHER TO DETERMINE IF THE POND IS FUNCTIONING PROPERLY. 2. TOP AND SIDE SLOPES OF EMBANKMENT SHALL ME MOWED A MINIMUM TWO (2) TIMES PER YEAR, ONCE IN JUNE AND ONCE IN SEPTEMBER. OTHER SIDE SLOPES AND MAINTENANCE ACCESS
- SHALL BE MOWED AS NEEDED. DEBRIS AND LITTER SHALL BE REMOVED DURING REGULAR MOWING OPERATIONS AND AS NEEDED. 4. VISIBLE SIGNS OF EROSION IN THE POND AS WELL AS THE RIP-RAP OR GABION OUTLET AREA SHALL BE REPAIRED AS SOON AS IT IS NOTICED.
- 1. STRUCTURAL COMPONENTS OF THE POND SUCH AS THE DAM, THE RISER, AND THE PIPES SHALL BE REPAIRED UPON THE DETECTION OF ANY DAMAGE. THE COMPONENTS SHALL BE INSPECTED DURING
- ROUTINE MAINTENANCE OPERATIONS. 2. SEDIMENT SHALL BE REMOVED FROM THE POND, AND FOREBAY, NO LATER THAN WHEN THE CAPACITY OF THE POND, OR FOREBAY, IS HALF FULL OF SEDIMENT, OR, WHEN DEEMED NECESSARY FOR AESTHETIC REASONS, UPON APPROVAL FROM THE DEPARTMENT OF PUBLIC WORKS.

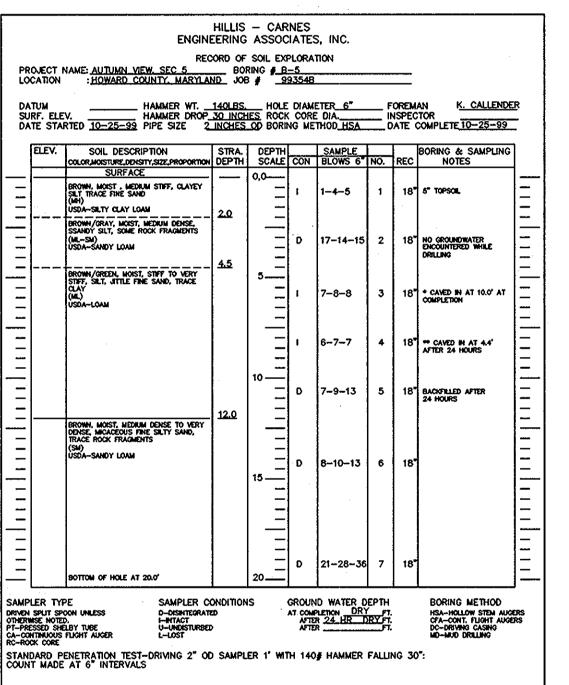
GEOTECHNICAL RECOMENDATIONS:

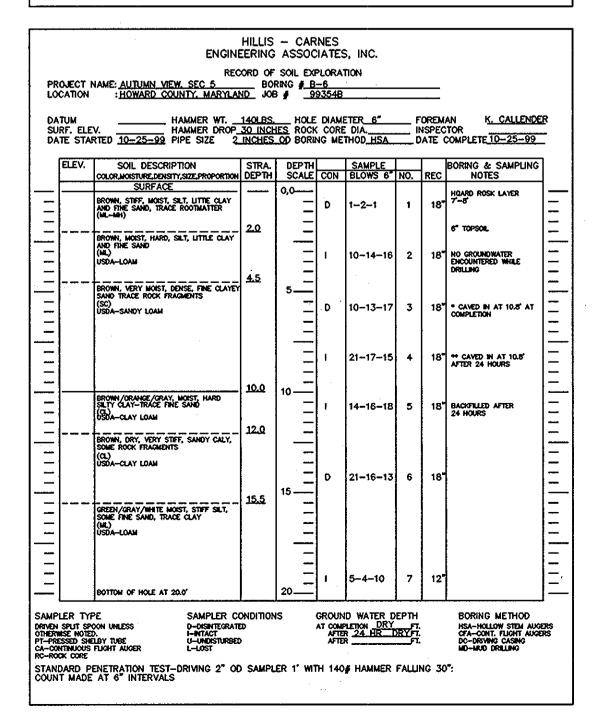
EMBANKMENT AND CUT-OFF TRENCH CONSTRUCTION

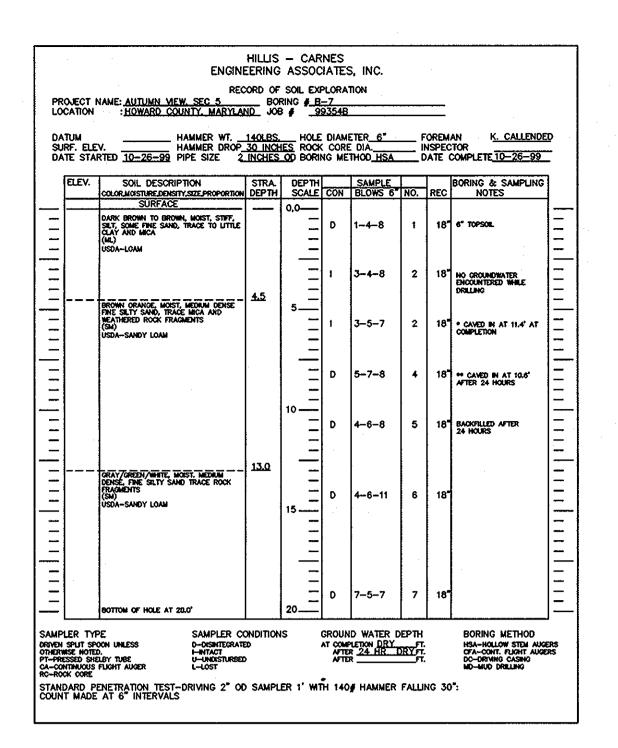
THE SITE SHOULD BE STRIPPED OF TOPSOIL AND ANY OTHER UNSUITABLE MATERIALS FROM THE EMBANKMENT OT STRUCTURE AREA IN ACCORDANCE WITH SOIL CONSERVATION GUIDELINES. AFTER STRIPPING OPERATIONS HAVE BEEN COMPLETED, THE EXPOSED SUBGRADE MATERIALS SHOULD BE PROOFROLLED WITH A LOADED DUMP TRUCK OR SIMILAR EQUIPMENT IN THE PRESENCE OF A GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE. FOR AREAS THAT ARE NOT ACCESSIBLE TO A DUMP TRUCK, THE EXPOSED MATERIALS SHOULD BE OBSERVED AND TESTED BY A GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE UTILIZING A DYNAMIC CONE PENETROMETER. ANY EXCESSIVELY SOFT OR LOOSE MATERIALS IDENTIFIED BY PROOFROLLING OR PENETROMETER TESTING SHOULD BE EXCAVATED TO SUITABLE FIRM SOIL, AND THEN GRADES REESTABLISHED BY BACKFILLING WITH SUITABLE SOIL. A REPRESENTATIVE OF GEOTECHNICAL ENGINEER SHOULD BE PRESENT TO MONITOR PLACEMENT

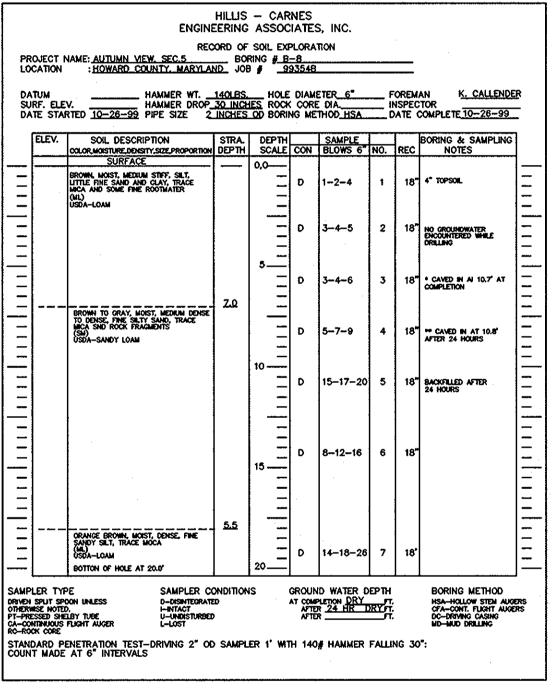
AND COMPACTION OF FILL FOR EMBANKMENT AND CUT-OFF TRENCH. IN ACCORDANCE WITH MARYLAND SOIL CONSERVATION SPECIFICATION 378 SOILS CONSIDERED SUITABLE FOR THE CENTER OF EMBANKMENT AND CUT-OFF TRENCH SHALL CONFORM TO UNIFIED SOIL CLASSIFICATION GC, SC, SH OR CL. ALL FILL MATERIALS MUST BE PLACED AND COMPACTED IN ACCORDANCE WITH MD SSC

ROJECT	NAME: AUTUMN VIEW SEC.5	во	SOIL EXI						
TUM JRF. E	LEV. HAMMER WT.	140LBS.	HOLE ROCK	DIAM	ETER 6"		OREM.	TOR	<u>R</u>
	FARTED 10-25-99 PIPE SIZE	2 INCHES	OD BORI	NG ME	THOD <u>HSA</u>	^c	DATE (COMPLETE 10-25-99	-
ELEV	COLOR MOISTURE DENSITY, SIZE PROPORTIO	STRA. N DEPTH	DEPTH SCALE		SAMPLE BLOWS 6"	NO.	REC	BORING & SAMPLING NOTES	
	SURFACE BROWN/GRAY, MOIST, MEDIUM STIFF TO VERY STIFF, SANDY SILT WITH SOME ROSEFRAGMENTS	*	0.0	1	2-5-4	1	18*	4° TOPSOIL	=
	USDA-LOAM		=	D	35-11-15	2	18*	NO GROUNDWATER	=
	BROWN/GREEN, MOIST, STIFF TO VERY STIFF, SILT, SIME FINE SAND, TRACE	4.5	5					ENCOUNTERED WHILE DRILLING	
	WEATHERED ROCK FRAGMENTS (ML) USDA-SANDY LOAM		=	D	6-6-9	3	18"	* CAVED IN AT 10.7° AT COMPLETION	=
				1	5-6-8	4	18"	♥ CAVED IN AT 10.7' AFTER 24 HOURS	= =
			10 —	,	6-10-14	5	18*	GROUNDWATER ENCOUNTERED AT 3.0'	<u> </u>
	BROWN, MIOST, MEDIUM DENSE TO DENS FINE SILTY SAND, LITTLE MICA AND ROC	12.5		:				ENCOGNIEDED XI 3.0	= = =
	FRAGMENTS (SM) USDA-SANDY LOAM		Ξ		9-11-17	6	18"		=
			15 —						=
			=						=
	BOTTOM HOLE AT 20.0°	20.0	20		28-24-22	7	18"		=
MISE NO	SPOON UNLESS D-DISINTEGRA TED. I-HTACT SHELBY TUBE U-UNOISTURB US FLIGHT AUGER L-LOST	TED			ID WATER D PLETION DRY R 24r 10 R			BORING METHOD HSA-HOLLOW STEM AUG CFA-CONT, FUIGHT AUGE DC-DRIVING CASING MO-MUD DRILLING	





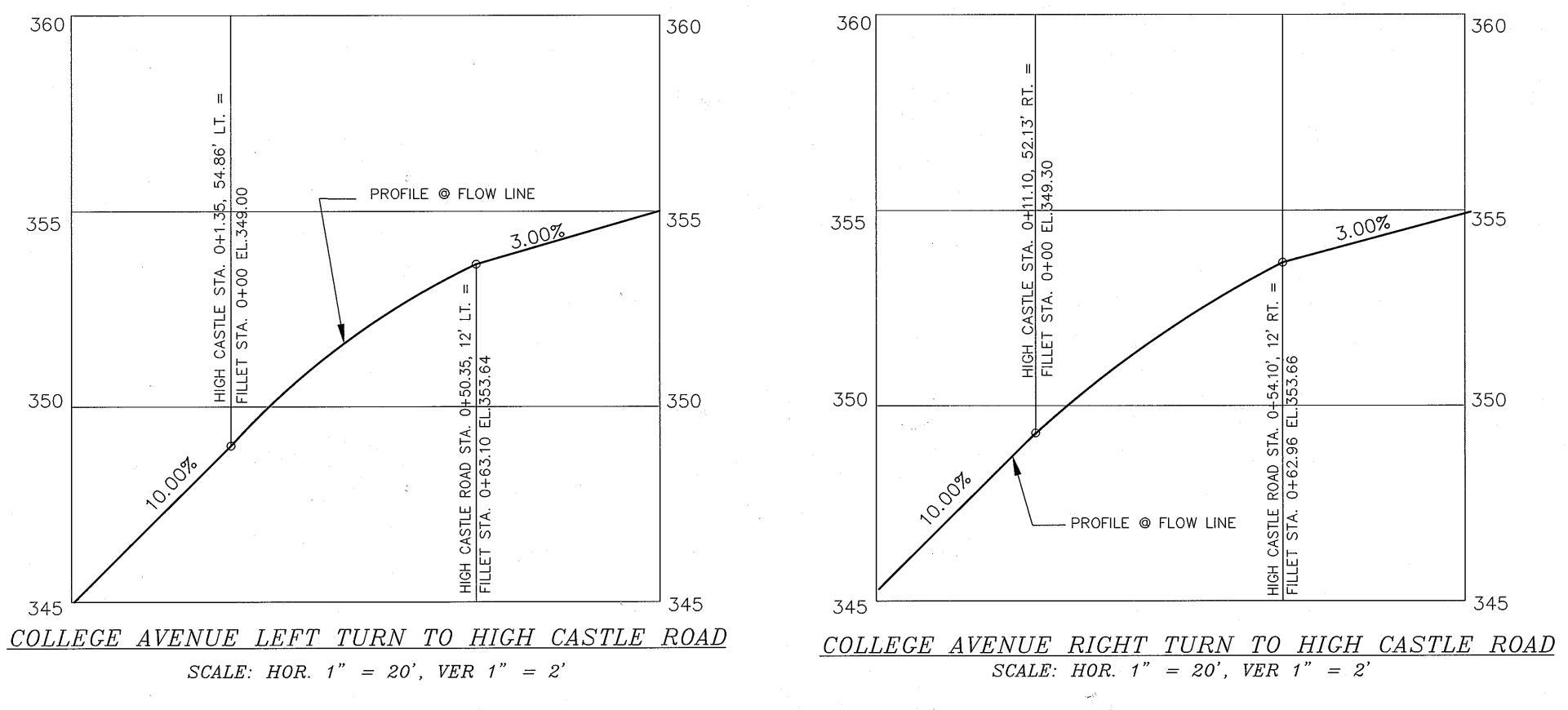


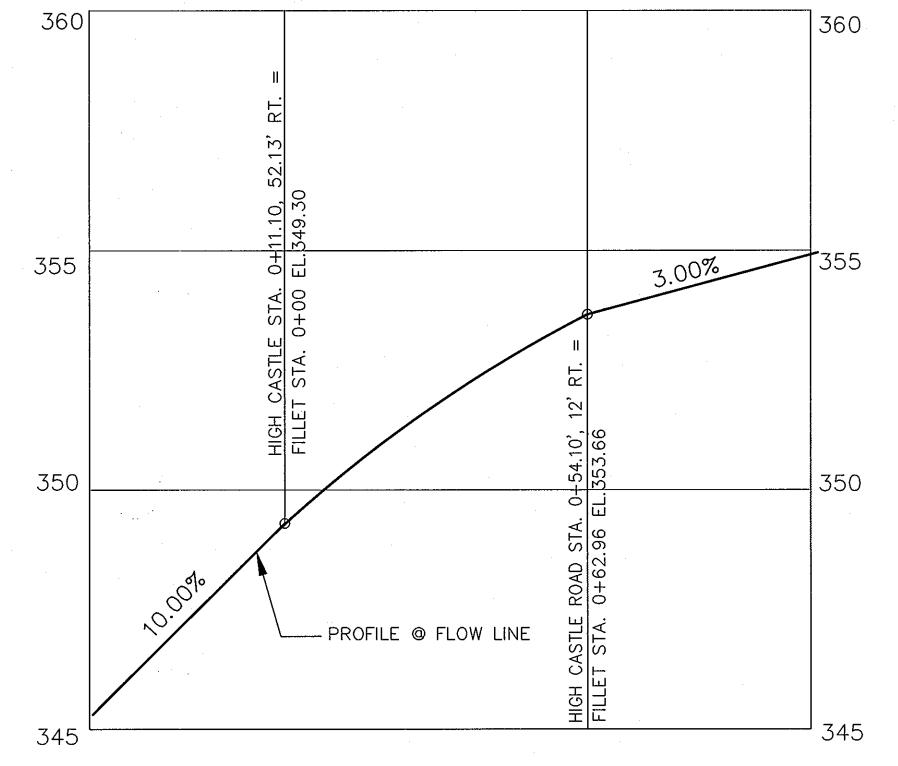


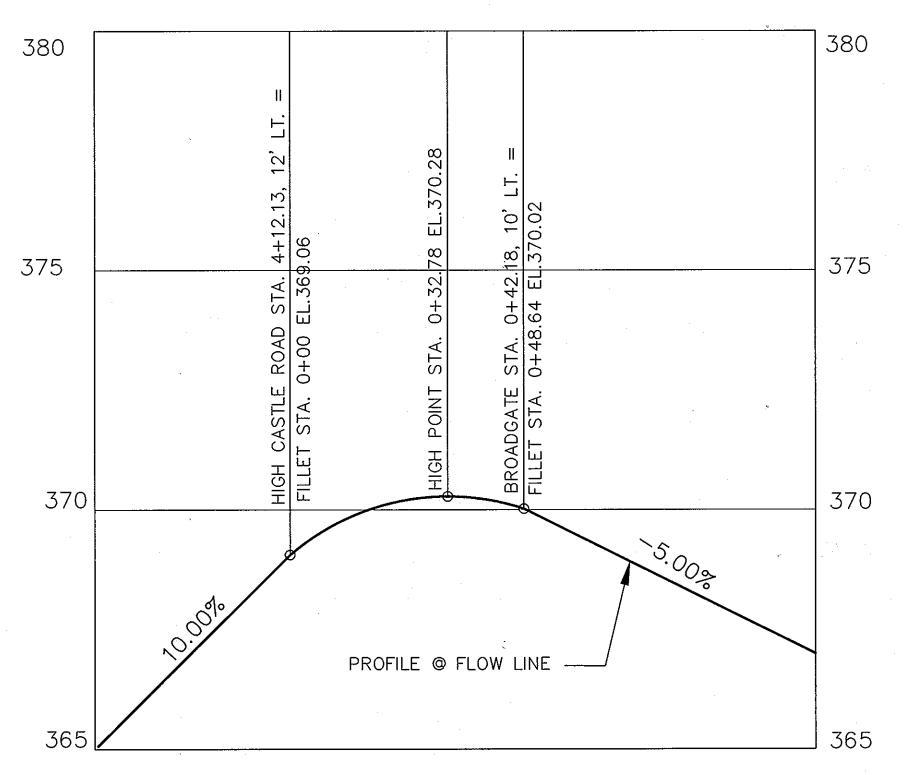
OCATION OATUM SURF. ELI	NAME: AUTUMN VIEW, SWC.5 : HOWARD COUNTY, MARYLA HAMMER WT. EV. HAMMER DROP	ND JOS 140LBS 30 INC	HOLE	9354B	ETER_6"		OREM ISPEC	TOR	Ŗ -
ELEV.	RTED 10-25-99 PIPE SIZE 2 SOIL DESCRIPTION COLORMOISTURE, DENSITY, SIZE, PROPORTION	STRA.	OD BORI DEPTH SCALE		SAMPLE BLOWS 6"	D		BORING & SAMPLING NOTES	
	SURFACE BROWN, MOIST, SOFT TO VERY STIFF, SILT, LITTLE TO SOME CLAY, TRACE FINE SAND AND MICA (MI-MH)		0,0	D	1-2-2	1	18"		
	USDA-LOAM/SILTY CLAY LOAM		<u>-</u>	1.	4-5-8	2	18"	NO GROUNDWATER ENCOUNTERED WHILE ORILLING	=======================================
			5 <u>-</u>	ı	9-10-12	3	18"	CAVED IN AT 11.0" AT COMPLETION	<u>-</u> <u>-</u>
:	BROWN, MOIST, VERY STIFF, CLAYEY SILT WITH QUARTZ GRAVEL (MH) USDA-SILTY CLAY LOAM	7.0		D	6-9-11	4	18"	AFTER 24 HOURS REFUSED AT 5.0"	
	TAN TO GRAY/WHITE, MOIST, MEDIUM DONSE TO DENSE, SILTY SAND WITH SOME ROCK FRAGMENTS (SM) USDA-SANDY LOAM	9.5	10 —	D	6–10–13	5	18"	BACKFILLED AFTER 24 JOURS	=
			15	D	6-9-13	6	18	:	
			=						= =
	BOTTOM OF HOLE AT 20.0'		20	D	9-14-17	7	18*		=
rwise note Ressed sh	OON UNLESS D-DISINTEGRAT	ED	ıs	GROUN AT COMM AFTE AFTE	D WATER D	EPTH 11.9FT. 11.9FT.		BORING METHOD HSA-HOLLOW STEM AUG CFA-CONT, FLICHT AUGE DC-DRIVING CASING MD-MUD DRILLING	irs Rs

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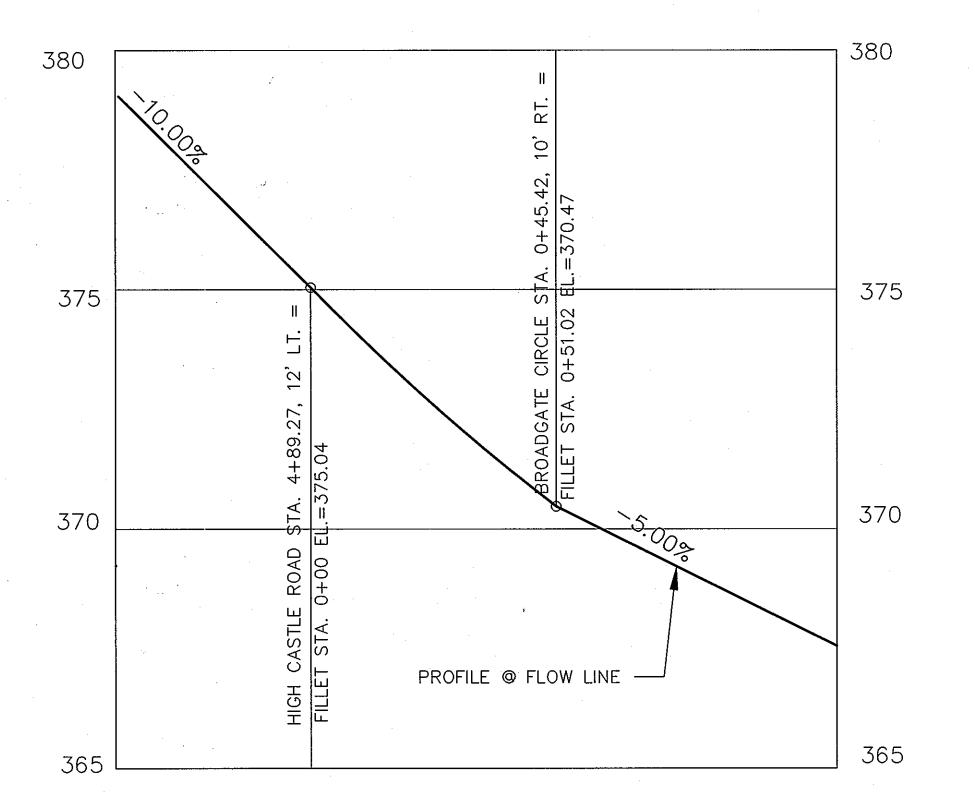
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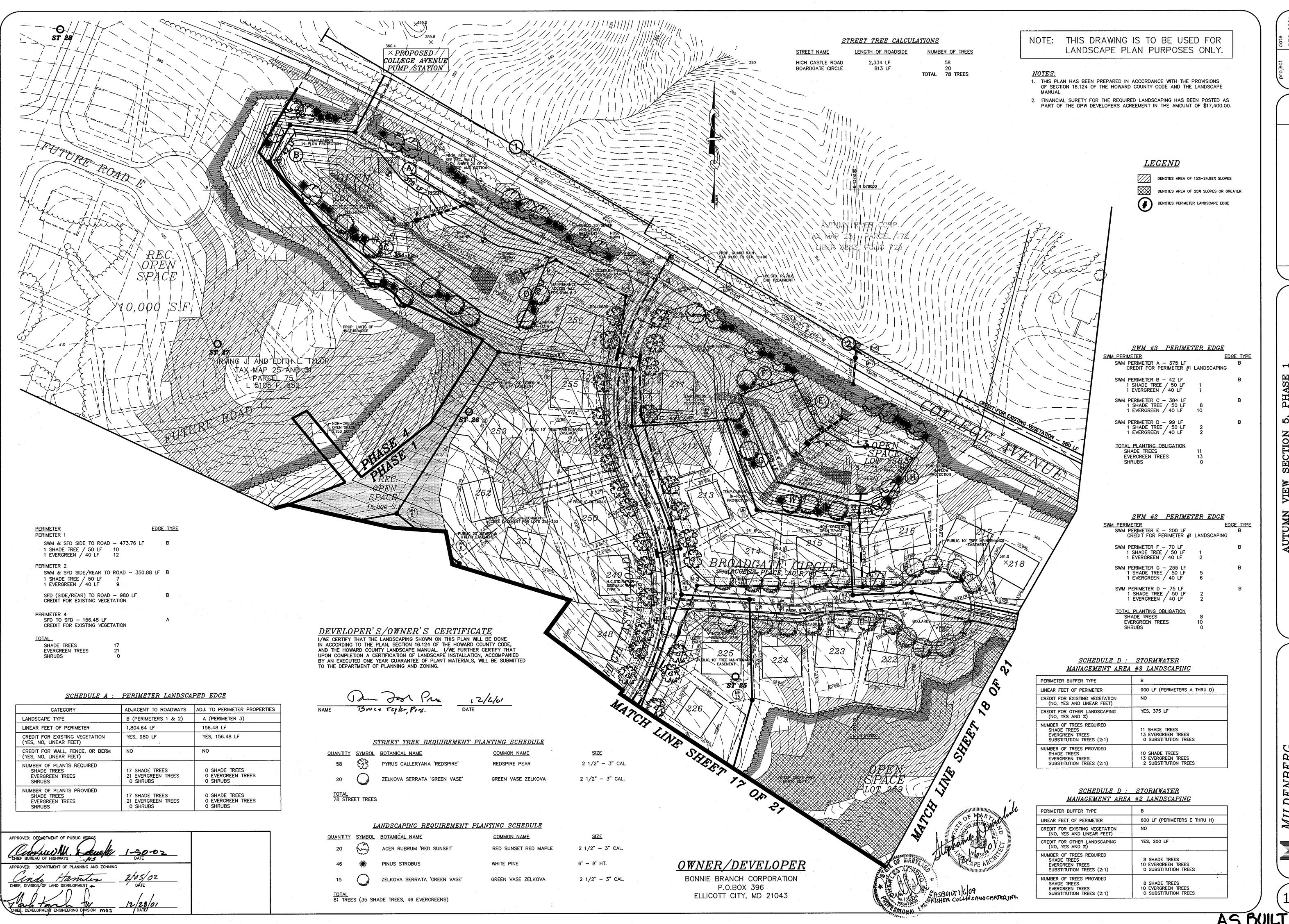


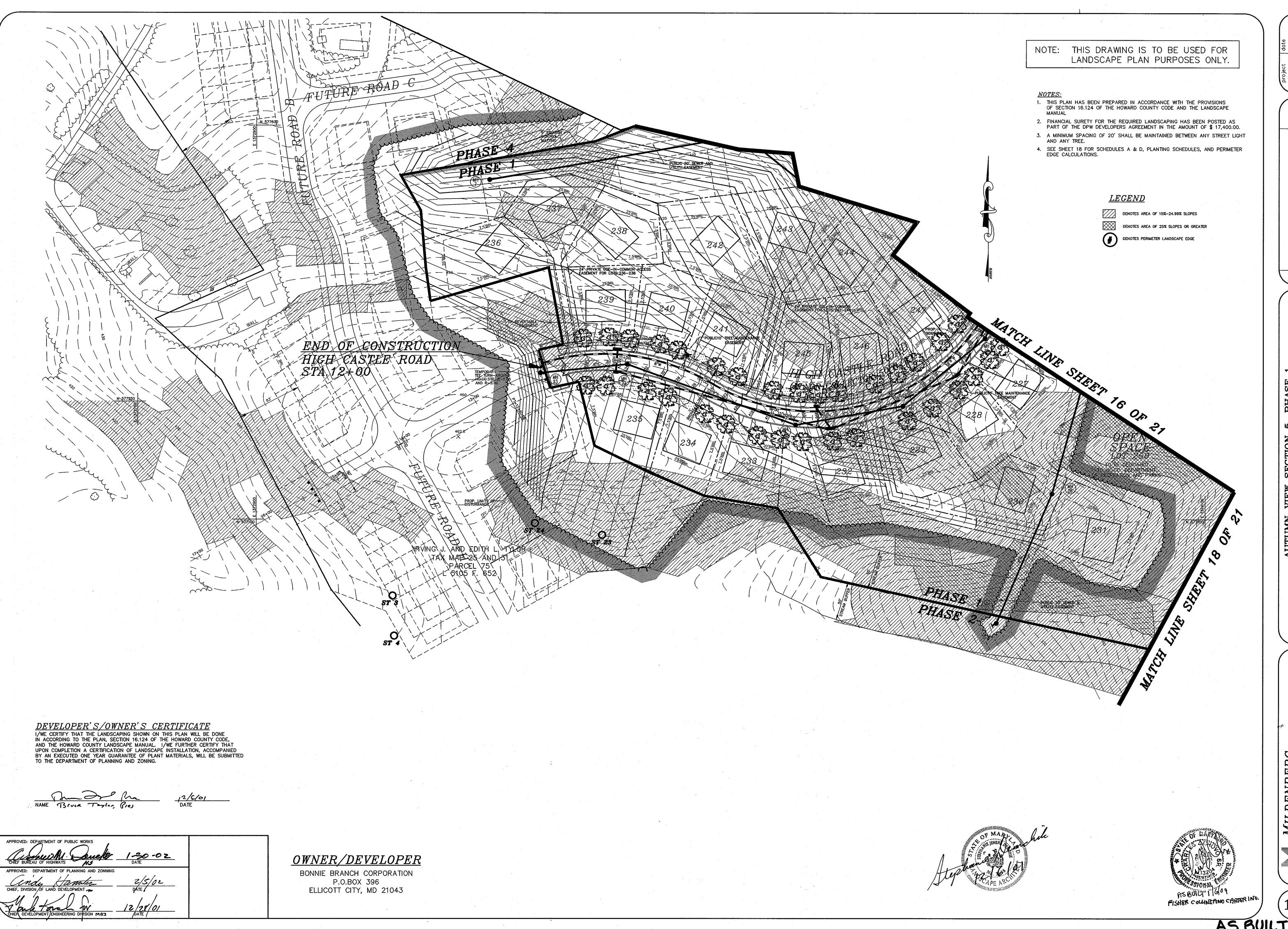
HIGH CASTLE ROAD LEFT TURN TO BROADGATE CIRCLE SCALE: HOR. 1" = 20', VER 1" = 2'



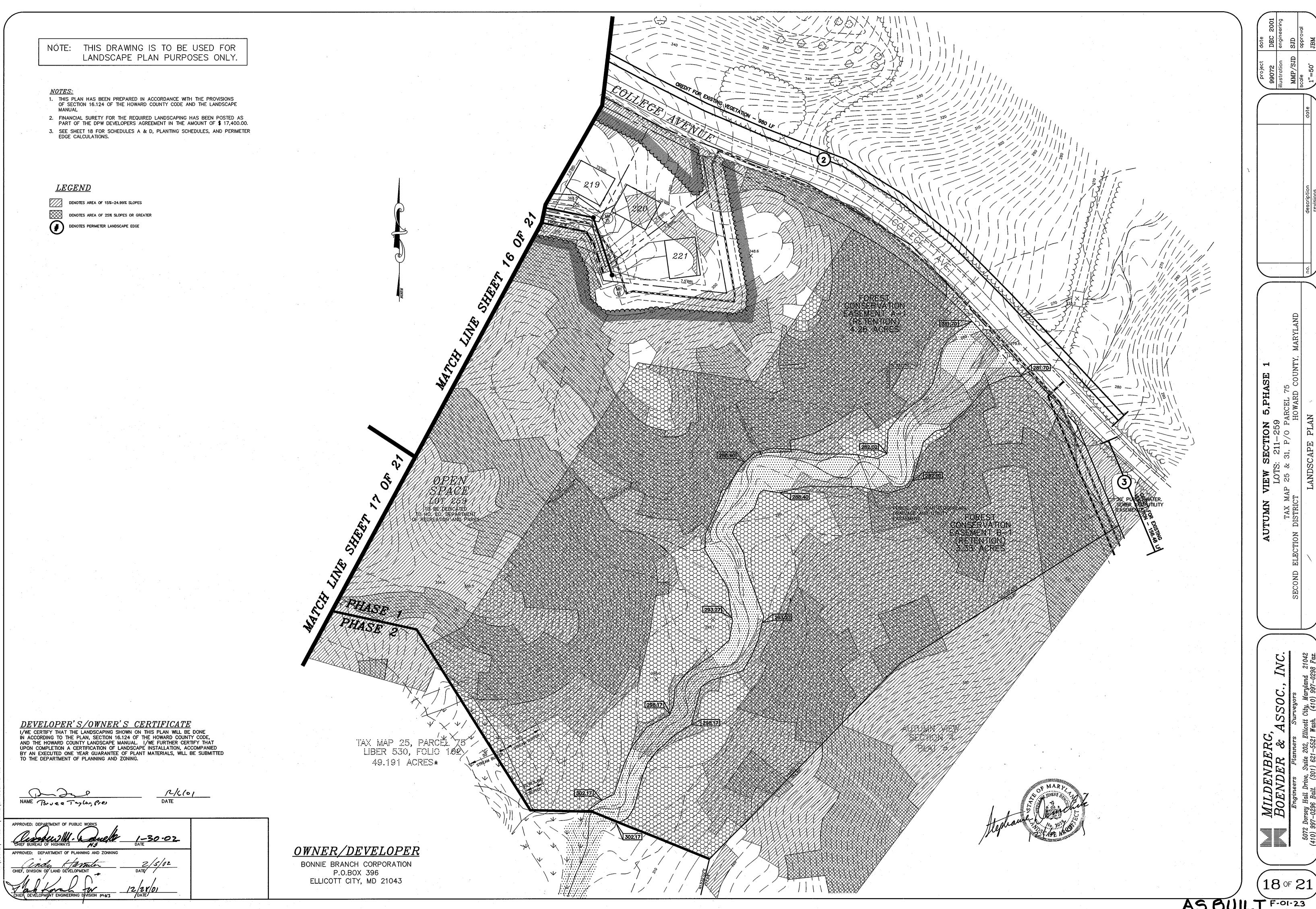
HIGH CASTLE ROAD RIGHT TURN TO BROADGATE CIRCLE SCALE: HOR. 1" = 20', VER 1" = 2'

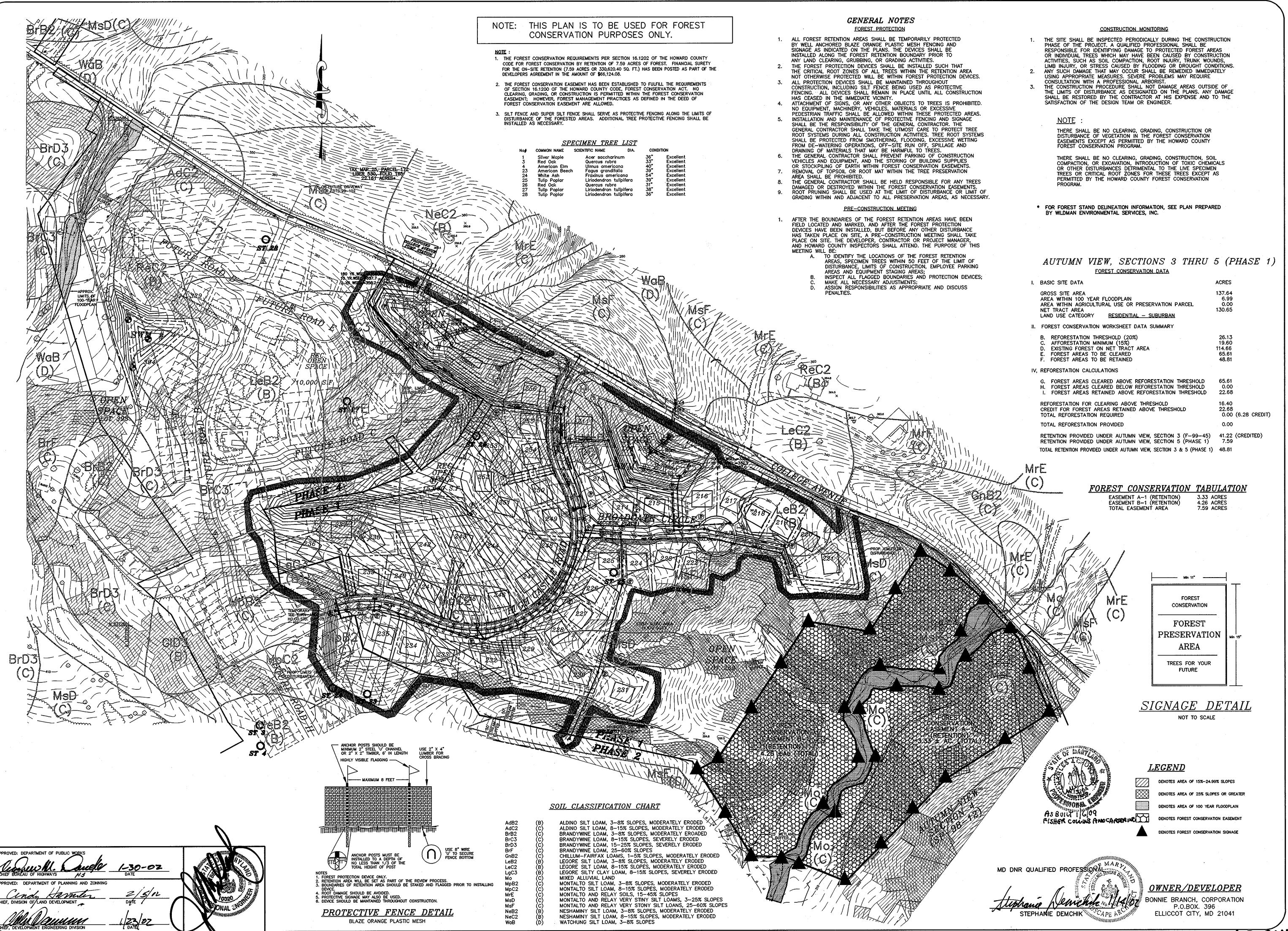
OWNER/DEVELOPER BONNIE BRANCH CORPORATION
P.O.BOX 396
ELLICOTT CITY, MD 21043

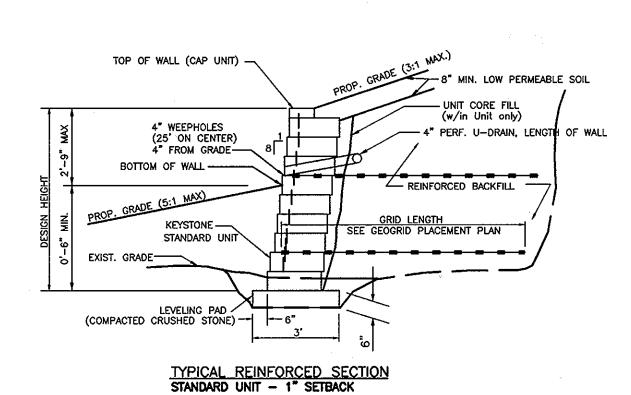




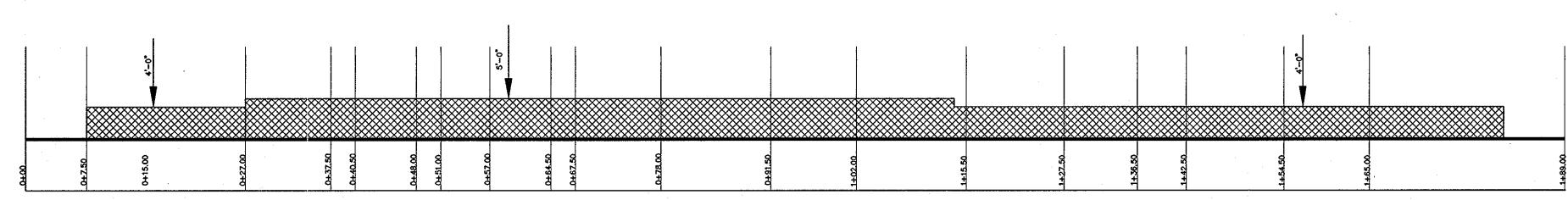
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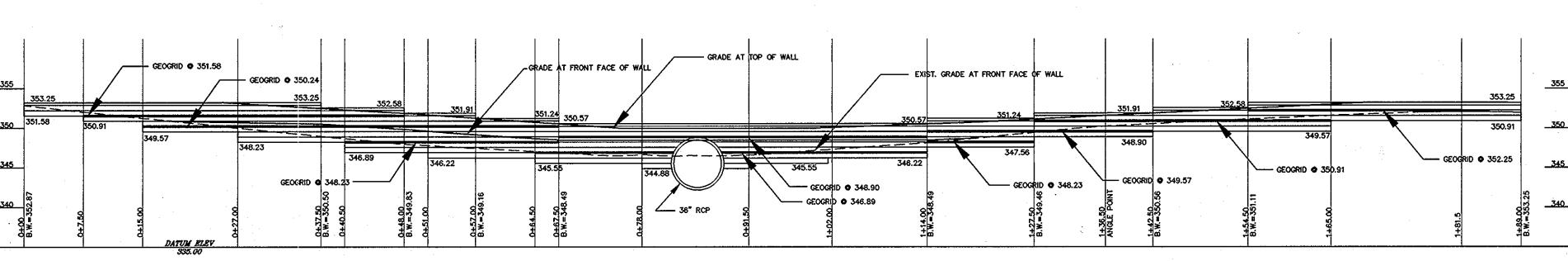




RETAINING WALL - SITE PLAN

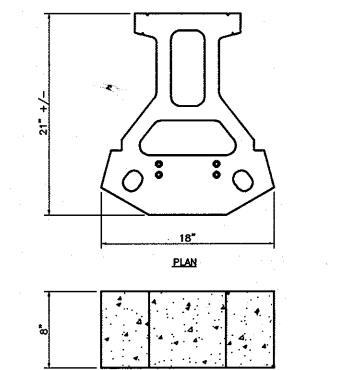


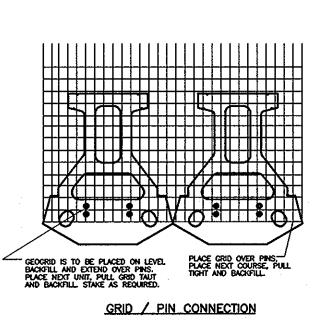
KEYSTONE PLAN - GEOGRID PLACEMENT

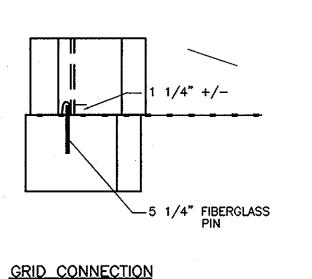


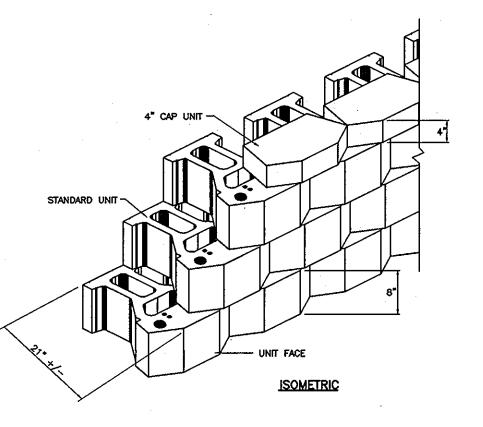
KEYSTONE RETAINING WALL - ELEVATION

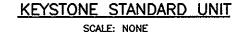
SCALE: 1"=10'





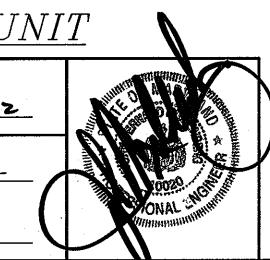






KEYSTONE STD. UNIT

APPROVED: DEPARTMENT OF PUBLIC WORKS 2/5/02-



OWNER/DEVELOPER

BONNIE BRANCH CORPORATION P.O. BOX 396

RETAINING WALL - SPECIFICATIONS

2.02 MODULAR CONCRETE RETAINING WALL UNITS.

A. MODULAR CONCRETE UNITS SHALL CONFORM TO THE FOLLOWING ARCHITECTURAL REQUIREMENTS:

FACE COLOR - STANDARD MANUFACTURERS' COLOR OR CUSTOM COLOR AS SPECIFIED BY THE OWNER.

FACE FINISH - SCULPTURED ROCK FACE IN ANGULAR MULTIPLANER CONFIGURATION. OTHER FACE FINISHES WILL NOT BE ALLOWED WITHOUT

BOND CONFIGURATION - RUNNING WITH BONDS NOMINALLY LOCATED AT MIDPOINT VERTICALLY ADJACENT UNITS, IN BOTH STRAIGHT AND CURVED ALIGNMENTS.

EXPOSED SURFACES OF UNITS SHALL BE FREE OF CHIPS, CRACKS OR OTHER IMPERFECTIONS WHEN VIEWED FROM A DISTANCE OF 10 FEET UNDER DIFFUSED

B. MODULAR CONCRETE UNITS SHALL CONFORM TO THE FOLLOWING MATERIAL

1. CEMENT - MATERIALS SHALL CONFORM TO THE FOLLOWING APPLICABLE SPECIFICATIONS.

A. PORTLAND CEMENT - ASTM C 150

WRITTEN APPROVAL OF OWNER.

B. MODIFIED PORTLAND CEMENT - PORTLAND CEMENT CONFORMING TO ASTM C 150, MODIFIED AS FOLLOWS. LIMESTONE - CALCIUM CARBONATE, WITH A MINIMUM 85 % CONTENT, MAY BE ADDED TO THE CEMENT, PROVIDED THESE MINIMUM 85 % CONTENT, MAY BE ADDED TO REQUIREMENTS OF C 150 AS MODIFIED ARE MET; (1) LIMITATION ON INSOLUBLE RESIDUE 1.5 %; (2) LIMITATION ON AIR CONTENT OF MORTAR - VOLUME PERCENT, 22% MAXIMUM; AND (3) LIMITATIONS OF LOSS OF IGNITION - 7 %

C. BLENDED CEMENTS - ASTM C 618

D. POZZOLANS - ASTM C 618

E. BLAST FURNACE SLAG CEMENT - ASTM C 989

2. AGGREGATES - AGGREGATES SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS, AS APPLICABLE.

A. NORMAL WEIGHT AGGREGATES - ASTM C 33

B. LIGHTWEIGHT AGGREGATES - ASTM C 331

3. OTHER CONSTITUENTS - AIR ENTRAINING AGENTS, COLORING PIGMENTS, INTEGRAL WATER REPELLENTS, FINELY GROUND SILICA, AND OTHER CONSTITUENTS SHALL BE PREVIOUSLY ESTABLISHED AS SUITABLE FOR USE IN MODULAR CONCRETE RETAINING WALL UNITS AND SHALL CONFORM TO APPLICABLE ASTM STANDARDS OR, SHALL BE SHOWN BY TEST OR EXPERIENCE TO BE NOT DETRIMENTAL TO THE DURABILITY OF THE MODULAR CONCRETE UNITS OR ANY MATERIAL CUSTOMARILY USED IN RETAINING WALL CONSTRUCTION.

C. MODULAR CONCRETE UNITS SHALL CONFORM TO THE FOLLOWING STRUCTURAL AND GEOMETRIC REQUIREMENTS:

COMPRESSIVE STRENGTH = 3000 PSI MINIMUM;

ABSORPTION = 8 % MAXIMUM (6% IN NORTHERN STATES) FOR STANDARD WEIGHT AGGREGATES;

UNIT DEPTH - 20 INCHES MINIMUM;

UNIT WIDTH TO HEIGHT RATIO = 2.25: 1;

UNIT WEIGHT - 90 LBS/UNIT MINIMUM FOR STANDARD WEIGHT AGGREGATES INTER-UNIT SHEAR STRENGTH - 1500 PLF MINIMUM AT 2 PSI NORMAL

GEOGRID/UNIT PEAK CONNECTION STRENGTH -1000 PLF MINIMUM AT 2 PSI

MAXIMUM HORIZONTAL GAP BETWEEN ERECTED UNITS SHALL BE - 1/2 INCH. D. MODULAR CONCRETE UNITS SHALL CONFORM TO THE FOLLOWING

VERTICAL SETBACK = $1/8"\pm$ PER COURSE (NEAR VERTICAL) OR $1"\pm$ PER COURSE PER THE DESIGN DRAWINGS; ALIGNMENT AND GRID POSITIONING MECHANISM - FIBERGLASS PINS, TWO PER

2.03 SHEAR CONNECTORS

CONSTRUCTABILITY REQUIREMENTS:

A. STRENGTH OF SHEAR CONNECTORS BETWEEN VERTICAL ADJACENT UNITS SHALL BE APPLICABLE OVER A DESIGN TEMPERATURE OF 10 DEGREES F TO + 100 DEGREES F. SHEAR CONNECTORS SHALL BE 1/2 INCH DIAMETER THERMOSET ISOPTHALIC POLYESTER RESIN-PULTRUDED FIBERGLASS REINFORCEMENT RODS. CONNECTORS SHALL HAVE A MINIMUM FLEXURAL STRENGTH OF 128,000 PSI AND SHORT BEAM SHEAR OF 6,400 PSI. B. SHEAR CONNECTORS SHALL BE CAPABLE OF HOLDING THE GEOGRID IN THE PROPER DESIGN POSITION DURING GRID PRE-TENSIONING AND BACKFILLING.

2.04 BASE LEVELING PAD MATERIAL

A. MATERIAL SHALL CONSIST OF A COMPACTED CRUSHED STONE BASE OR NON-REINFORCED CONCRETE AS SHOWN ON THE CONSTRUCTION DRAWINGS. THE LEVELING PAD SHALL BE A MINIMUM OF 6 INCHES THICK. AS AN OPTION. CONCRETE MAY BE 3 INCHES THICK WITH A COMPACTED GRANULAR BASE FOR A TOTAL THICKNESS OF 6 INCHES.

2.05 UNIT FILL

A. UNIT FILL SHALL CONSIST OF CLEAN 1" MINUS CRUSHED STONE OR CRUSHED GRAVEL MEETING THE GRADATION LISTED BELOW.

SIEVE SIZE PERCENT PASSING 1 INCH 100

3/4 INCH 75-100

NO. 50 0-5

B. ONE CUBIC FOOT, MINIMUM, OF DRAIN FILL SHALL BE USED FOR EACH SQUARE FOOT OF WALL FACE. DRAIN FILL SHALL BE PLACED WITHIN CORES OF, BETWEEN, AND BEHIND UNITS TO MEET THIS REQUIREMENT.

2.06 REINFORCED BACKFILL

A. REINFORCED BACKFILL SHALL BE FREE OF DEBRIS AND MEET THE FOLLOWING GRADATION REQUIREMENTS:

SIEVE SIZE PERCENT PASSING 2 INCH 100-75

3/4 INCH 100-75 NO. 4 100-20 NO. 40 0-60 NO. 200 0-35

PHI ANGLE = 28°

UNIT WGT. = 120 LBS./CU.FT.

PLASTICITY INDEX (PI) <10 AND LIQUID LIMIT <40.

B. THE MAXIMUM AGGREGATE SIZE SHALL BE LIMITED TO 3/4 INCH UNLESS FIELD TESTS HAVE BEEN OR WILL BE PERFORMED TO EVALUATE POTENTIAL STRENGTH

REDUCTIONS TO THE GEOGRID DESIGN DUE TO DAMAGE DURING CONSTRUCTION. C. MATERIAL CAN BE SITE EXCAVATED SOILS WHERE THE ABOVE REQUIREMENTS

CAN BE MET. UNSUITABLE SOILS FOR BACKFILL (HIGH PLASTIC CLAYS OR ORGANIC SOILS) SHALL NOT BE USED IN THE BACKFILL OR IN THE REINFORCED SOIL MASS.

D. CONTRACTOR SHALL SUBMIT REINFORCED FILL SAMPLE AND LABORATORY TEST RESULTS TO THE ARCHITECT/ENGINEER FOR APPROVAL PRIOR TO THE USE OF ANY PROPOSED REINFORCED FILL

2.07 GEOGRID

- A. TA, ALLOWABLE TENSILE DESIGN LOAD, SHALL BE DETERMINED AS FOLLOWS: TA = TCR/(FD*FC*FS)TA SHALL BE EVALUATED BASED ON A 75 YEAR DESIGN LIFE.
- B. TCR, CREEP LIMITED TENSILE LOAD TCR SHALL BE DETERMINED FROM 10,000 HOUR CREEP TESTING PERFORMED
- IN ACCORDANCE WITH ASTM D5262. C. FD. FACTOR FOR DURABILITY/AGING

FD SHALL BE DETERMINED FROM POLYMER SPECIFIC DURABILITY TESTING

- COVERING THE RANGE OF EXPECTED SOIL ENVIRONMENTS. D. FC, FACTOR FOR CONSTRUCTION DAMAGE FC SHALL BE DETERMINED FROM PRODUCT SPECIFIC CONSTRUCTION DAMAGE TESTING PERFORMED IN ACCORDANCE WITH GRI-GG4. TEST RESULTS SHALL BE PROVIDED FOR EACH PRODUCT TO BE USED WITH PROJECT SPECIFIC OR MORE
- E. FS, OVERALL FACTOR OF SAFETY FS SHALL BE 1.5 UNLESS OTHERWISE NOTED.
- F. THE MAXIMUM DESIGN TENSILE LOAD OF THE GEOGRID SHALL NOT EXCEED THE LABORATORY TESTED ULTIMATE STRENGTH OF THE GEOGRID/FACING UNIT CONNECTION AS LIMITED BY THE "HINGE HEIGHT" DIVIDED BY A FACTOR OF SAFETY OF 1.5. THE CONNECTION STRENGTH TESTING AND COMPUTATION PROCEDURES SHALL BE IN ACCORDANCE WITH NCMA TEST METHODS.
- G. SOIL INTERACTION COEFFICIENT, CI CI VALUES SHALL BE DETERMINED PER GRI: GG5 AT A MAXIMUM 0.75 INCH
- H. MANUFACTURING QUALITY CONTROL
- THE GEOGRID MANUFACTURER SHALL HAVE A MANUFACTURING QUALITY CONTROL PROGRAM THAT INCLUDES QC TESTING FOR EACH 40,000 SF OF PRODUCTION, EACH LOT, OR EACH PRODUCTION DAY. THE QC TESTING SHALL INCLUDE: TENSILE MODULUS
- SPECIFIC GRAVITY MELT FLOW INDEX (PP&HDPE) MOLECULAR WEIGHT (PETP)
- G. GEOGRID SHALL CONFORM TO MIRAFI "MIRAGRID XT (8XT)" FABRIC.

PART 3 EXECUTION 3.01 EXCAVATION

A. CONTRACTOR SHALL EXCAVATE TO THE LINES AND GRADES SHOWN ON THE CONSTRUCTION DRAWINGS. ARCHITECT/ENGINEER WILL INSPECT THE EXCAVATION AND APPROVE PRIOR TO PLACEMENT OF LEVELING MATERIAL OR FILL SOILS.

B. OVER-EXCAVATION OF DELETERIOUS SOILS AND REPLACEMENT WITH SUITABLE FILL WILL BE PAID AT UNIT COST RATES.

3.02 BASE LEVELING PAD

A. LEVELING PAD MATERIAL(S) SHALL BE PLACED TO THE LINES AND GRADES SHOWN ON THE CONSTRUCTION DRAWINGS, TO A MINIMUM THICKNESS OF 6

B. SOIL LEVELING PAD MATERIALS SHALL BE COMPACTED TO A MINIMUM OF 95 % STANDARD OR 90 % MODIFIED PROCTOR.

C. LEVELING PAD SHALL BE PREPARED TO INSURE FULL CONTACT TO THE BASE SURFACE OF THE CONCRETE UNITS.

3.03 KEYSTONE UNIT INSTALLATION

A. FIRST COURSE OF UNITS SHALL BE PLACED ON THE LEVELING PAD, AND ALIGNMENT AND LEVEL CHECKED. PINS OR MOLDED SURFACES OF MODULAR CONCRETE UNITS SHALL BE USED FOR ALIGNMENT CONTROL.

B. POSITION VERTICALLY ADJACENT MODULAR CONCRETE UNITS AS RECOMMENDED BY THE MANUFACTURER.

C. MAXIMUM STACKED VERTICAL HEIGHT OF WALL UNITS, PRIOR TO WALL DRAIN FILL AND BACKFILL PLACEMENT AND COMPACTION, SHALL NOT EXCEED TWO

D. WHOLE, OR CUT, UNITS ON CURVES AND CORNERS TO SHALL BE ERECTED WITH RUNNING BOND APPROXIMATELY CENTERED ON UNITS ABOVE AND BELOW.

E. CAP UNITS SHALL BE GLUED TO UNDERLAYING UNITS WITH AN ADHESIVE RECOMMENDED BY THE MANUFACTURER.

3.04 STRUCTURAL GEOGRID INSTALLATION

A. GEOGRID SHALL BE ORIENTED WITH THE HIGHEST STRENGTH AXIS PERPENDICULAR TO THE WALL ALIGNMENT.

B. GEOGRID REINFORCEMENT SHALL BE PLACED AT THE ELEVATIONS AND TO THE EXTENT SHOWN ON THE CONSTRUCTION DRAWINGS OR AS DIRECTED BY THE

C. THE GEOGRID SHALL BE LAID HORIZONTALLY ON COMPACTED BACKFILL. PLACE THE NEXT COURSE OF MODULAR CONCRETE UNITS OVER GEOGRID. THE GEOGRID SHALL BE PULLED TAUT, AND ANCHORED PRIOR TO BACKFILL PLACEMENT ON THE GEOGRID.

D. GEOGRID REINFORCEMENTS SHALL BE CONTINUOUS THROUGHOUT THEIR EMBEDMENT LENGTHS. SPLICED CONNECTIONS BETWEEN SHORTER PIECES OF GEOGRID IS NOT ALLOWED UNLESS PRE-APPROVED BY THEARCHITECT/ENGINEER

3.05 REINFORCED BACKFILL PLACEMENT.

PRIOR TO CONSTRUCTION.

A. REINFORCED BACKFILL SHALL BE PLACED, SPREAD, AND COMPACTED IN SUCH A MANNER THAT MINIMIZES THE DEVELOPMENT OF SLACK IN THE

B. REINFORCED BACKFILL SHALL BE PLACED AND COMPACTED IN LIFTS NOT TO EXCEED 8 INCHES WHERE HAND COMPACTION IS USED, OR 12 INCHES WHERE HEAVY COMPACTION EQUIPMENT IS USED.

C. REINFORCED BACKFILL SHALL BE COMPACTED TO 95 % OF THE MAXIMUM DENSITY AS DETERMINED BY ASTM D695. THE MOISTURE CONTENT OF THE BACKFILL MATERIAL PRIOR TO AND DURING COMPACTION SHALL BE UNIFORMLY DISTRIBUTED THROUGHOUT EACH LAYER AND SHALL BE WITHIN 2 PERCENTAGE POINTS DRY OF OPTIMUM.

D. ONLY LIGHTWEIGHT HAND-OPERATED EQUIPMENT SHALL BE ALLOWED WITHIN 3 FEET FROM THE TAIL OFTHE MODULAR CONCRETE UNIT.

E. TRACKED CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY UPON THE GEOGRID REINFORCEMENT. A MINIMUM FILL THICKNESS OF 6 INCHES IS REQUIRED PRIOR TO OPERATION OF TRACKED VEHICLES OVER THE GEOGRID. TRACKED VEHICLE TURNING SHOULD BE KEPT TO A MINIMUM TO PREVENT TRACKS FROM DISPLACING THE FILL AND DAMAGING THE GEOGRID.

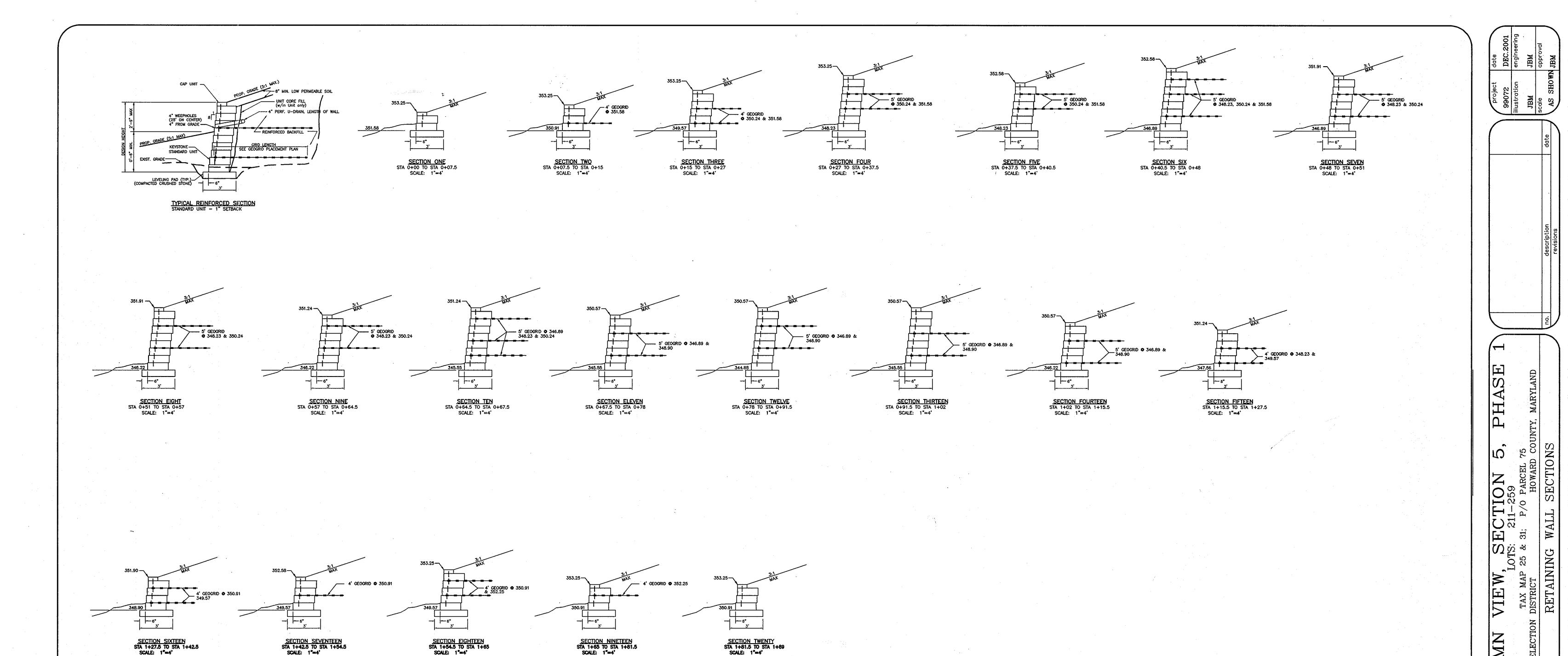
F. RUBBER TIRED EQUIPMENT MAY PASS OVER GEOGRID REINFORCEMENT AT SLOW SPEEDS, LESS THAN 10 MPH. SUDDEN BRAKING AND SHARP TURNING SHALL BE AVOIDED.

G. AT THE END OF EACH DAY'S OPERATION, THE CONTRACTOR SHALL SLOPE THE LAST LIFT OF REINFORCED BACKFILL AWAY FROM THE WALL UNITS TO DIRECT RUNOFF AWAY FROM WALL FACE. THE CONTRACTOR SHALL NOT ALLOW SURFACE RUNOFF FROM ADJACENT AREAS TO ENTER THE WALL CONSTRUCTION

ELLICOTT CITY, MD 21043

VIE.

ILDENBERG, SOENDER & 1



NOTE: GEOTECHNICAL ENGINEER WILL CERTIFY THE SUITABILITY OF FILL MATERIALS AND THE BEARING PRESSURES REQUIRED FOR EACH WALL SECTION.

OWNER/DEVELOPER BONNIE BRANCH CORPORATION P.O. BOX 396 ELLICOTT CITY, MD 21043

AS BUILT

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