2. The contractor shall notify the Department of Public Works/Bureau of Engineering/Construction Inspection Division at (410) 313-1880 at least five (5) working days prior to the start of work.

3. The Contractor shall notify "Miss Utility" at 1-800-257-7777 at least forty-eight (48) hours prior to any excavation work.

4. Project Background: Location: Elkridge, Maryland Tax Map: 37 Election District: 1st

5. Traffic control devices, markings, and signing shall be in accordance with the latest edition of the Manual on Uniform Traffic Control Devices (MUTCD). All street and regulatory signs shall be in place prior to the placement of any asphalt.

6. Any damage caused by the Contractor to existing public right-of-way, existing paving, existing curb and gutter, existing utilities, etc. shall be corrected at the Contractor's expense.

7. The existing utilities shown hereon are located from construction drawings of record. The approximate locations of existing utilities are shown for the Contractor's information and convenience. The Contractor shall locate existing utilities to his own satisfaction and well in advance of any construction activities. Additionally, the Contractor shall take all necessary precautions to protect all existing utilities and maintain uninterrupted service.

8. Horizontal and vertical datums are related to the Maryland State Plane Coordinate System as projected from Howard County Control Station No. 2644006 and Howard County Control Station No. 2644005 (NAD 27). This allows for a smooth transition into the previously approved and constructed NAD 27 adjoining subdivisions; Lyndwood Manor & Marshalee Woods.

9. All hydraulic data is for the 10-year storm unless otherwise noted.

10. All fill areas shall be compacted to a minimum of 95% of the maximum dry density. as determined and verified in accordance with AASHTO T-180

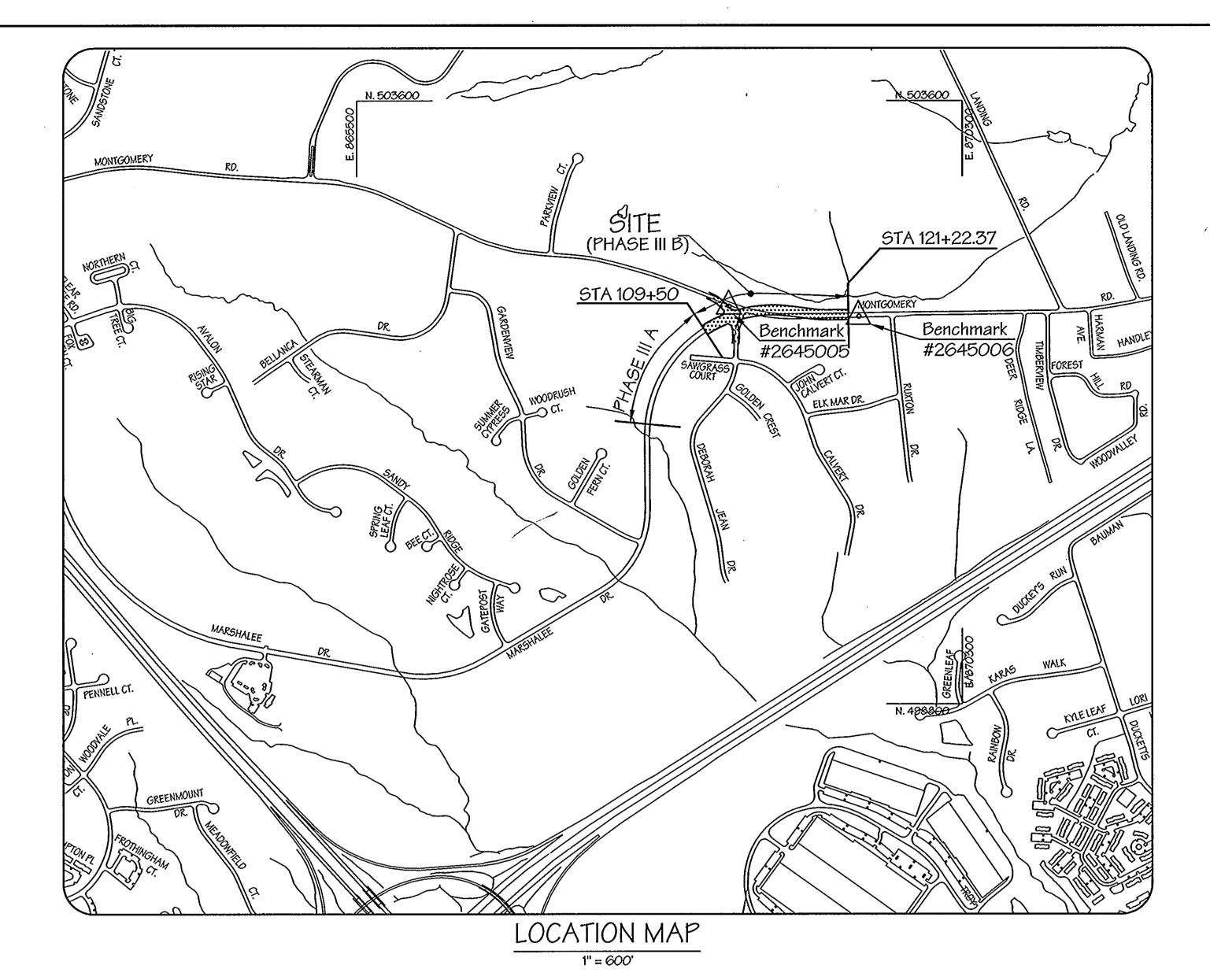
11. Water Quantity Stormwater Management has been waived for this project by the Howard County Department of Public Works. Water Quality management is met within the proposed "Vortechnics" Underground Systems.

12. All Limits of Disturbance must be approved on site by the following or their agents:

B.) Howard County Department of Public Works Construction Inspection Division.

13. This Plan assumes that construction work proposed under F 96-66 Capitol Project J-4136 Phase III A has been completed.

14. Where test pits have been made on existing utilities, they are noted by the symbol at the location of the test pit. A table containing the results of the test pit or pits is included on the drawings (See Sheet 8). Existing utilities in the vicinity of the proposed work for which test pits have not been dug shall be located by the Contractor two weeks in advance of construction operations at his own expense. Required test pits to be dug are represented by the symbol 🚡



HOWARD COUNTY, MARYLAND DEPARTMENT OF PUBLIC WORKS MARSHALEE DRIVE

STATION 109+50 TO 121+22.37 CAPITAL PROJECT J-4136 PHASE III B

BENCHMARKS

Ho. Co. Monument # 2645005 Elevation: 291.929 North 501966.231 East 868453.612 Description: Concrete Monument 0.3 ft. below surface at top of bank

Ho. Co. Monument # 2645006 Elevation: N/A

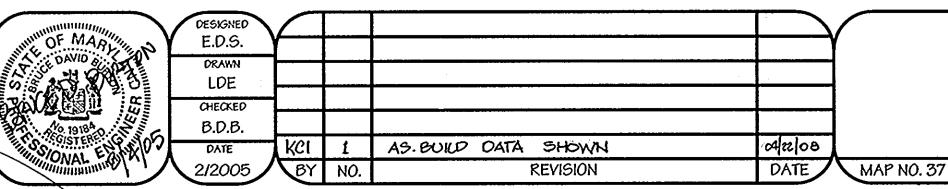
DEPARTMENT OF PUBLIC WORKS

North 501884.354 East 869482.835 Description: Concrete Monument 0.2 ft. below surface at top of bank

Engineers, Surveyors, Planners

LDE Inc.

9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045 (410)715-1070 - (301)596-3424 - FAX(410)715-9540



Cover Sheet

BLOCK 15

MARSHALEE DRIVE

INDEX OF SHEETS

Relocated Montgomery Road & Deborah Jean Drive Stations 10+00 thru 14+78.59
"Phase III B" Roadway Profile Stations 109+50 thru 121+22.68 & 10.00 thru 14+78.59

"Phase III B" Marshalee Drive Stations 109+50 thru 111+88.17

Montgomery Road Stations 111+88.17 thru 121+22.68

Grading, Soil Erosion & Sediment Control Plan Details

Gradina, Soil Erosion & Sediment Control Plan

Maintenance of Traffic Plan - PHASE 1

Maintenance of Traffic Plan - PHASE 2

Maintenance of Traffic Plan - PHASE 3

Maintenance of Traffic Plan - PHASE 4

Maintenance of Traffic Plan - PHASE 5

Street Light and Landscaping Plan

Paving and Striping Plan

2Btc Water Main Relocation

Vortechnics - Details

No.

Cover Sheet

Cross Sections

Cross Sections

Drainage Area Map

Storm Drain Profiles

Typical Road Sections & Road Details

CAPITAL PROJECT J-4136 PHASE III B

1st ELECTION DISTRICT, HOWARD COUNTY, MARYLAND

Owner: Howard County, Maryland

3430 Court House Drive

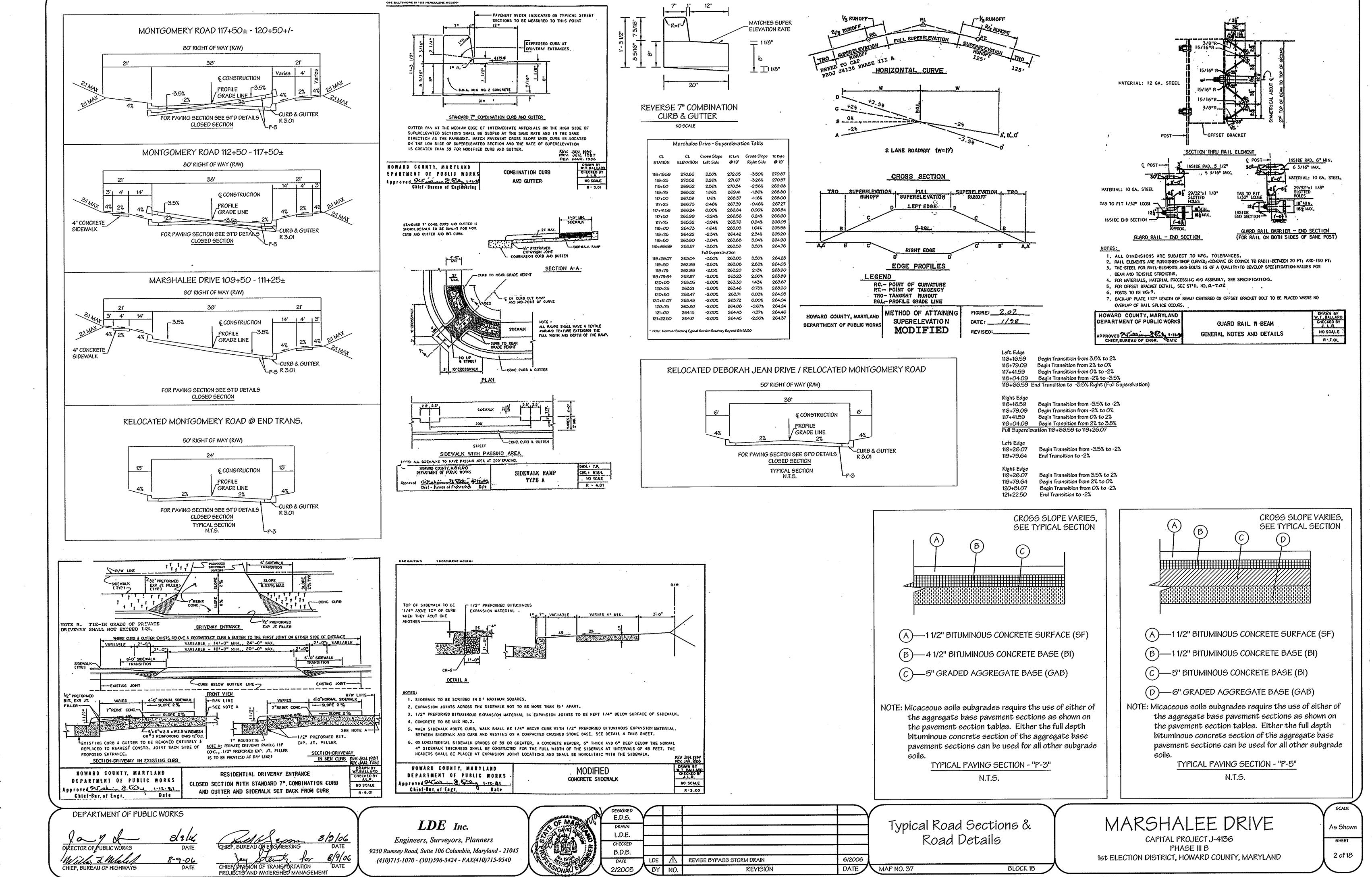
Ellicott City, MD. 21043

Developer: Marshalee Woods Limited Partnership

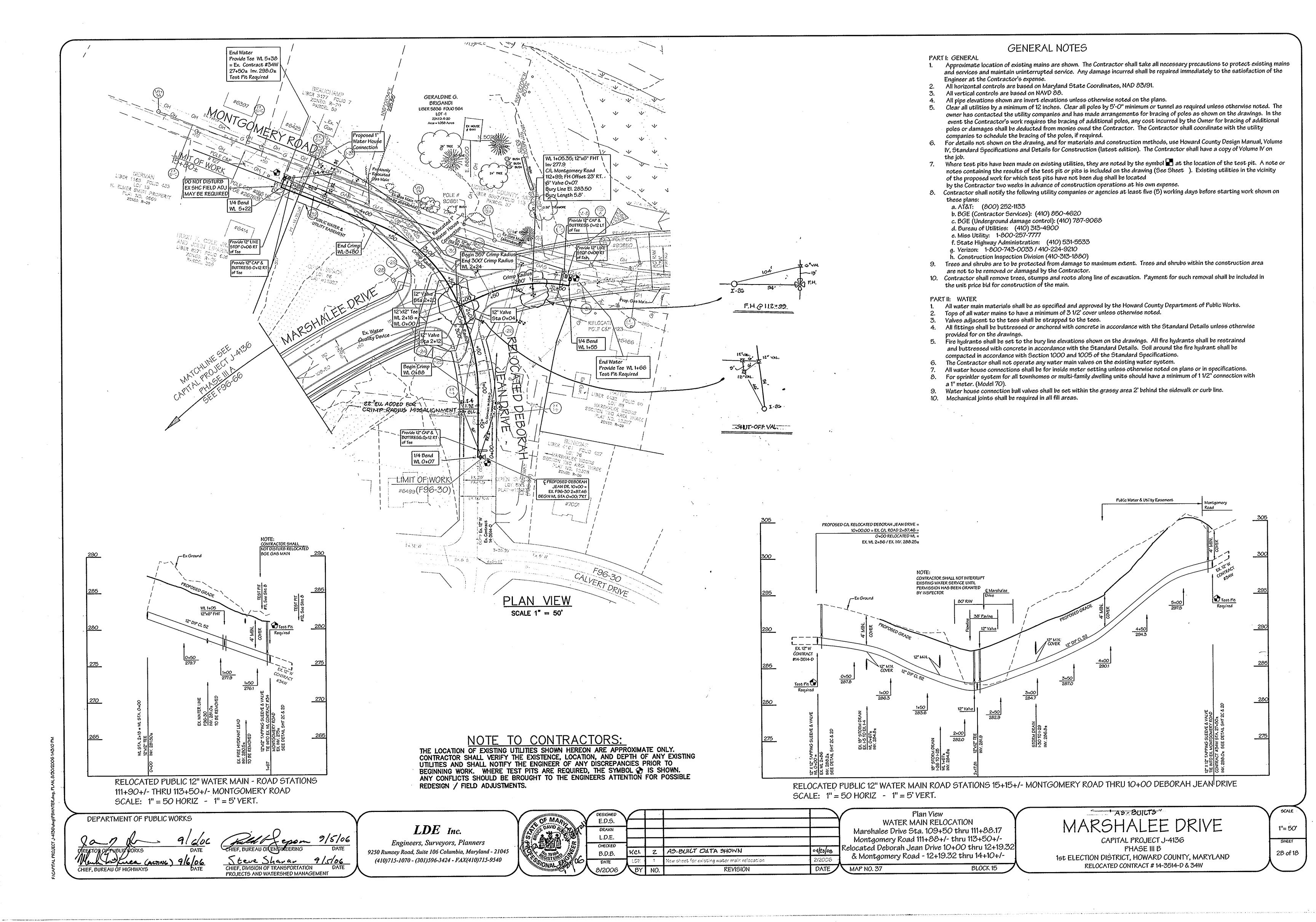
Columbia, MD. 21045

8835 Unit P Columbia 100 Parkway

As Shown 1 OF 18



FICAPITAL PROJECT J-4136/dwa/PBRSRD,dwa, RD DETAILS, 7/17/2006 14548 PM



- Contractor shall take all necessary precautions to protect the newly relocated (2006) Gas Main during 12" water main relocation.

TAPPING SLEEVE & VALVE

1. Install two proposed 12" x 12" Tapping Sleeve & Valve on the existing Water Main # 34 W at Water Main Stations 5+37.65 & 1+65.52 as shown in PLAN VIEW - Sheet 2B along existing Montgomery Road.

2. Install Tapping Sleeve & Valve on the existing Water Main # 14-3514-D at Water Main Station O+OO as shown in PLAN VIEW - Sheet 2B along Deborah Jean Drive.

3. All valves shall be closed upon completion of proposed sleeve installation.

4. Install concrete buttress' at all 12" x 12" Tapping Sleeves per Ho. Co. Standard Detail W-2.23. Contractor shall have a geotechnical engineer certify the bearing pressure of the existing soils to be 3000 psf minimum prior to installation of all buttress or adjust buttress design as specified by the contractors Geotechnical/Structural

5. Upon completion of above, attach tapping device to open 12" valve and tap into existing main. All existing water main "coupons" shall be recovered upon pipe tap and cutter retraction.

6. Close 12" Valve & remove tapping device.

7. Repeat steps 5 & 6 for the remaining two (2) 12" taps.

8. Complete installation of relocated 12" water main in accordance with the PLAN VIEW - Sheet 2B and Howard County specifications.

9. Upon completion of 12" relocated water main installation and with permission from Howard County inspector, test relocated 12" water mains.

10. Install short / workable piece of pipe to 12" valve at tapping sleeves. Connect end of tested 12" relocated water main to above workable piece via spacer & sleeve per Ho. Co. Standard Detail W-4.15.

11. Open 12" valves and place 12" relocated mains in service.

12. Provide new water service connections to Parcels 59 & 57.

EXISTING 12" WATER MAIN ABANDONMENT & REMOVAL AT DEBORAH JEAN DRIVE

NOTE: Deborah Jean Drive shutdown and water main capping per PLAN VIEW - Sheet 2B, shall occur overnight. All effected residences shall be notified at least 5 days prior to shutdown.

1. Prior to Deborah Jean Drive shutdown, contractor shall have a geotechnical engineer certify the bearing pressure of the existing soils to be 3000 psf minimum prior to installation of the buttress or adjust buttress design as specified by the contractors Geotechnical/Structural engineer.

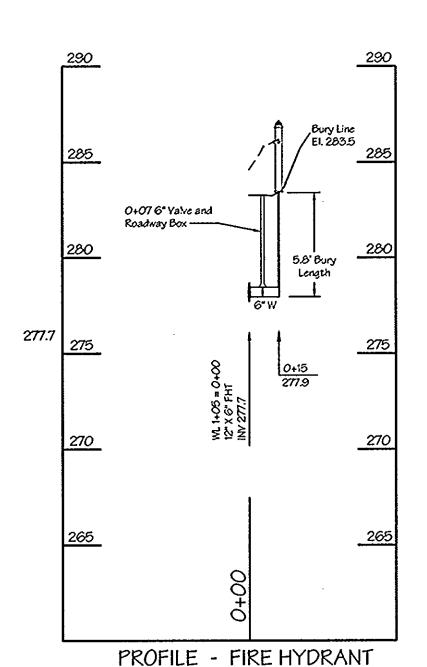
2. Contractor shall close existing valves at the Deborah Jean Drive / Calvert Drive & Sawgrass Court intersection. Water service to Calvert Drive residences is backfed through Contract #14-3120-D and Sawgrass Court is backfed through Contract #14-3284-D.

3. With permission from Howard County inspector, proceed by closing existing valves at the existing Deborah Jean Drive / Montgomery Road and Deborah Jean Drive / Calvert Drive & Sawgrass Court intersections as well as at the proposed tapping sleeve and valve at Station O+OO per PLAN VIEW - Sheet 2B. This will mark the beginning the shutdown for Deborah Jean Drive residences.

4. With water shutdown, cut ex 12" water main and install mechanical couplings (if required), end caps and buttress per Ho. Co. Standard Detail W-2.25. Again, contractor shall have a geotechnical engineer certify a bearing pressure of 3000 psf minimum prior to installation of all buttress or adjust buttress design as required.

5. Upon completion of cap & buttress installation, and with permission of Howard County inspector, restore water service to Deborah Jean Drive residences by opening valves at Station O+OO and at the Deborah Jean Drive / Calvert Drive & Sawgrass Court intersections, marking the end of the shutdown.

6. Contractor shall proceed with the removal of the existing 12" water main.



MONTGOMERY ROAD - CL ROAD STATION 113+00+/-SCALE: 1" = 50 HORIZ - 1" = 5' VERT.

SEQUENCE OF CONSTRUCTION (con't)

EXISTING 12" WATER MAIN ABANDONMENT AT MONTGOMERY ROAD

1. Contractor shall have a geotechnical / structural engineer, design a proper support block capable of supporting load of LineStop Machine per Line Stop manufacturer specifications, taking into account existing soil conditions & bearing pressures.

2. Install saddle tees and temporary gate valves on existing Contract #34 W as shown in PLAN VIEW - Sheet 2B. Temporary gate valve shall be closed.

3. Attach tapping machine to temporary gate valves at the two (2) LineStop locations.

4. Upon completion of above, open gate valve and tap into existing main. The existing water main "coupons" shall be recovered upon pipe tap and cutter retraction.

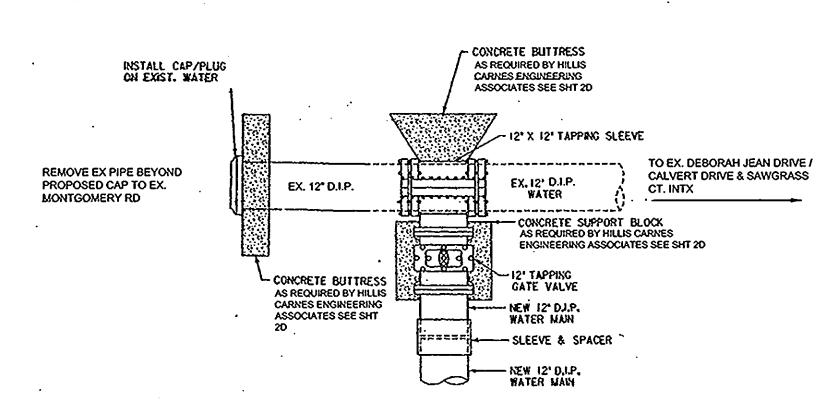
5. Close temporary gate valve & remove tapping device.

6. With properly designed support block in place per geotechnical / structural engineer design, install LineStop machine on each temporary gate valve.

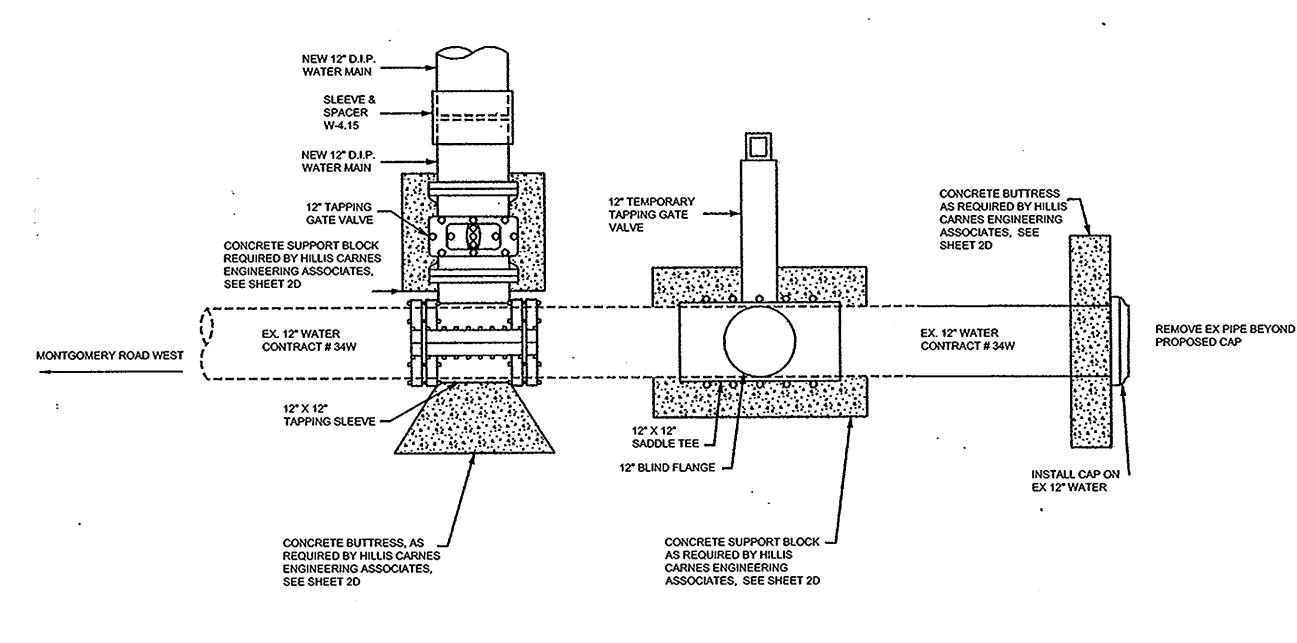
7. Open temporary gate valves and simultaneously insert Line Stopping plugging device into existing main to stop flow through existing 12" main (Contract #34W). Flow will continue through newly relocated main. 8. Upon successful flow interruption, cut existing water main and install mechanical couplings (if required), end caps and buttress per Ho. Co. Standard Detail W-2.25. Contractor shall have a geotechnical engineer certify a bearing pressure of 3000 psf minimum prior to installation of all buttress or adjust buttress design as required.

9. Upon completion of cap & buttress installation, and with permission of Howard County inspector, remove LineStop machine, attach completion machine and install completion plug in each of the two (2) tee outlets of the above saddle tees.

10. Remove completion machine and temporary gate valves. Install 12" blind flanges on each tee outlet. 11. Contractor shall proceed with the removal of the existing 12" water main.

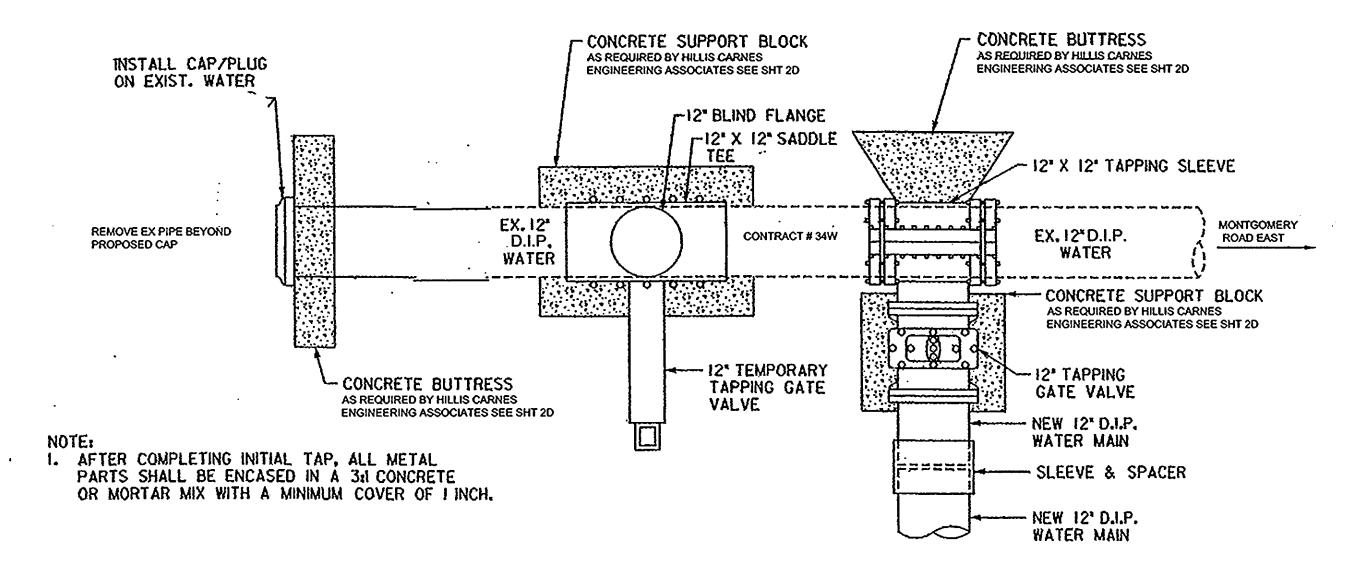


CONNECTION DETAIL ALONG DEBORAH JEAN DRIVE EX WL STATION 2+87+/- = PROPOSED RELOCATION WL STATION 0+00 N.T.S.



CONNECTION DETAIL ALONG MONTGOMERY ROAD EX WL STATION 27+50+/- = PROPOSED RELOCATION WL STATION 5+38

QUANTITIES AS BUILT QUANTITIES QUANTITIES TYPE MANUFACTURER/SUPPLIER **ESTIMATED** TYLER/FERGUSON WATER WORKS 12" TAPPING SLEEVE & YALVE (EA) ADS ENVIRONMENTAL GER. 12" LINE STOP (EA) TYLER TYLER FERGUSON WATER WORKS 12" CAP & BUTTRESS (EA) TYTON TYTON JOINT PIPE/U.S. PIPE 694 12" WATER (L.F.) MUELLER MUELLER/FERGUSON WATERWORKS 12" YALYE (Ea.) TYLER TYLER! " 12" x 12" TEE (Ea.) TYLER "/ 12" x 6" FIRE HYDRANT TEE (Ea.) 12" - 1/4 BEND (Ea.) TYTON TYTON JOINT PIPE U.S. PIPE 6" WATER (L.F.) MUCHER MULLER CO./FERGUSON WATER WORK 6" VALYE (Ea.) 6" FIRE HYDRANT (Ea.) MUELLER , o ' D P TYPE"K. 1" WHC (L.F.) 98 ඉප lı A NAME OF UTILITY CONTRACTOR: CHECK BOX: Survey & Drafting Division As Built Date:



CONNECTION DETAIL ALONG MONTGOMERY ROAD EX WL STATION 23+40+/- = PROPOSED RELOCATION WL STATION 1+65 N.T.S.

NOTE TO CONTRACTORS:

04 23 08

DATE

MAP NO. 37

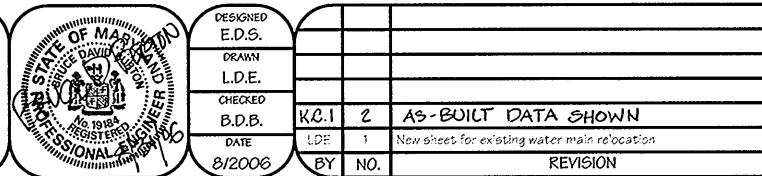
THE LOCATION OF EXISTING UTILITIES SHOWN HEREON ARE APPROXIMATE ONLY. CONTRACTOR SHALL VERIFY THE EXISTENCE, LOCATION, AND DEPTH OF ANY EXISTING UTILITIES AND SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES PRIOR TO BEGINNING WORK. WHERE TEST PITS ARE REQUIRED, THE SYMBOL (*) IS SHOWN. ANY CONFLICTS SHOULD BE BROUGHT TO THE ENGINEERS ATTENTION FOR POSSIBLE REDESIGN / FIELD ADJUSTMENTS.

DEPARTMENT OF PUBLIC WORKS

Sharan 9/5/06 PROJECTS AND WATERSHED MANAGEMENT

LDE Inc.

Engineers, Surveyors, Planners 9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045 (410)715-1070 - (301)596-3424 - FAX(410)715-9540



Profile & Details WATER MAIN RELOCATION Marshalee Drive Sta. 109+50 thru 111+88.17 Montgomery Road 111+88+/- thru 113+50+/-Relocated Deborah Jean Drive 10+00 thru 12+19.32 & Montgomery Road - 12+19.32 thru 14+10+/-

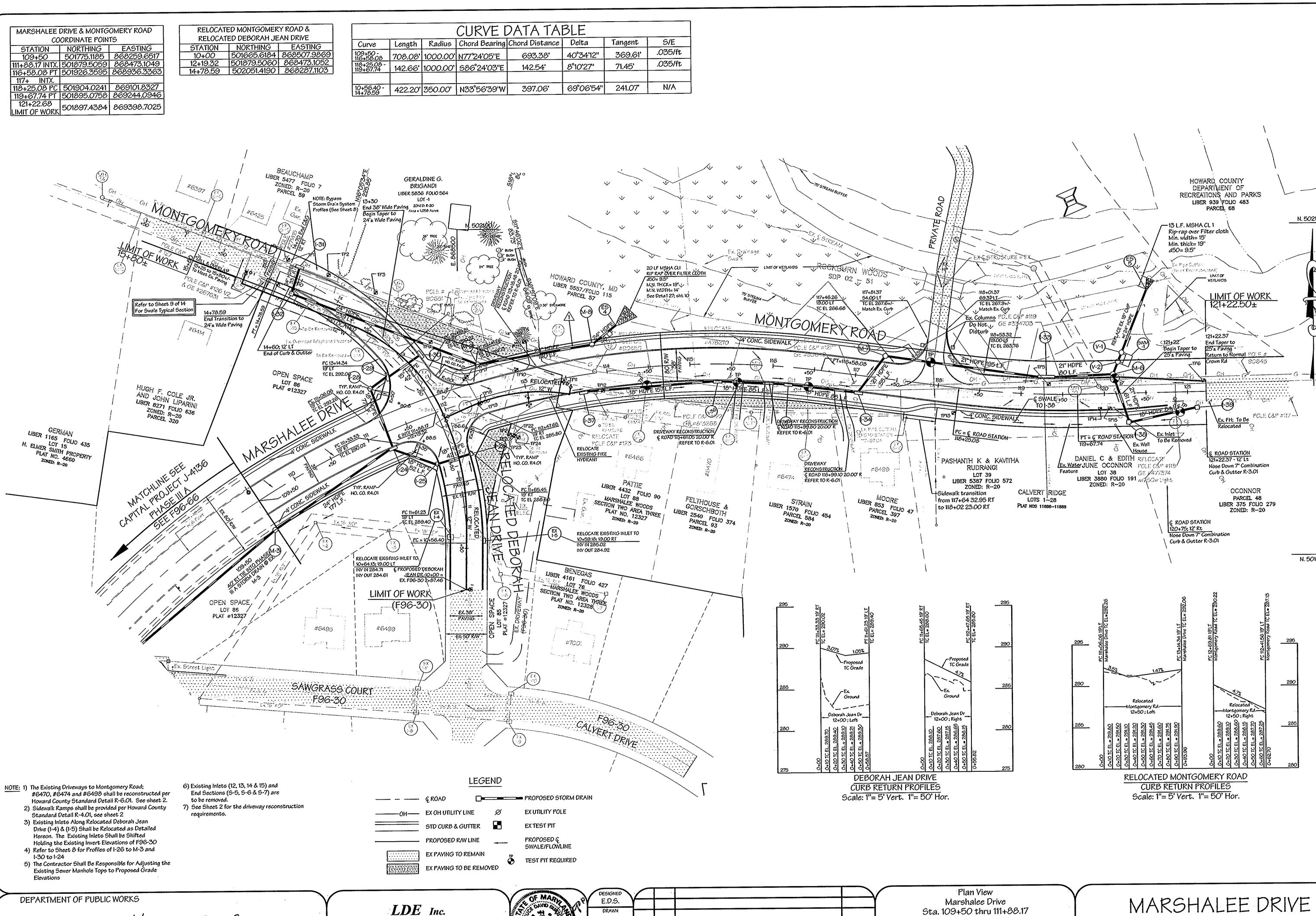
BLOCK 15

MARSHALEE DRIVE

CAPITAL PROJECT J-4136 PHASE III B - CONTRACT # 14-3514-D 1st ELECTION DISTRICT, HOWARD COUNTY, MARYLAND RELOCATED CONTRACT # 14-3514-D & 34W

2C of 18

1"= 50"



L.D.E.

CHECKED

B.D.B.

2/2005

LDE 🚹

REVISE BYPASS STORM DRAIN

REVISION

Engineers, Surveyors, Planners

9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045

(410)715-1070 - (301)596-3424 - FAX(410)715-9540

DIRÉCTOR OF PUBLIC WORKS

CHIEF, BUREAU OF HIGHWAYS

Willin I Males

CHIEF, CIVISION OF TRANSPORTATION PROJECTS AND WATERSHED MANAGEMENT

SCALE 1"= 50'

3 of 18

N. 502100

N. 501700

OCONNOR

Relocated

PARCEL 48

CAPITAL PROJECT J-4136

Montgomery Road 111+88.17 thru 121+22.68 Relocated Deborah Jean Drive 10+00 thru 12+19.32

& Montgomery Road - 12+19.32 thru 14+78.59

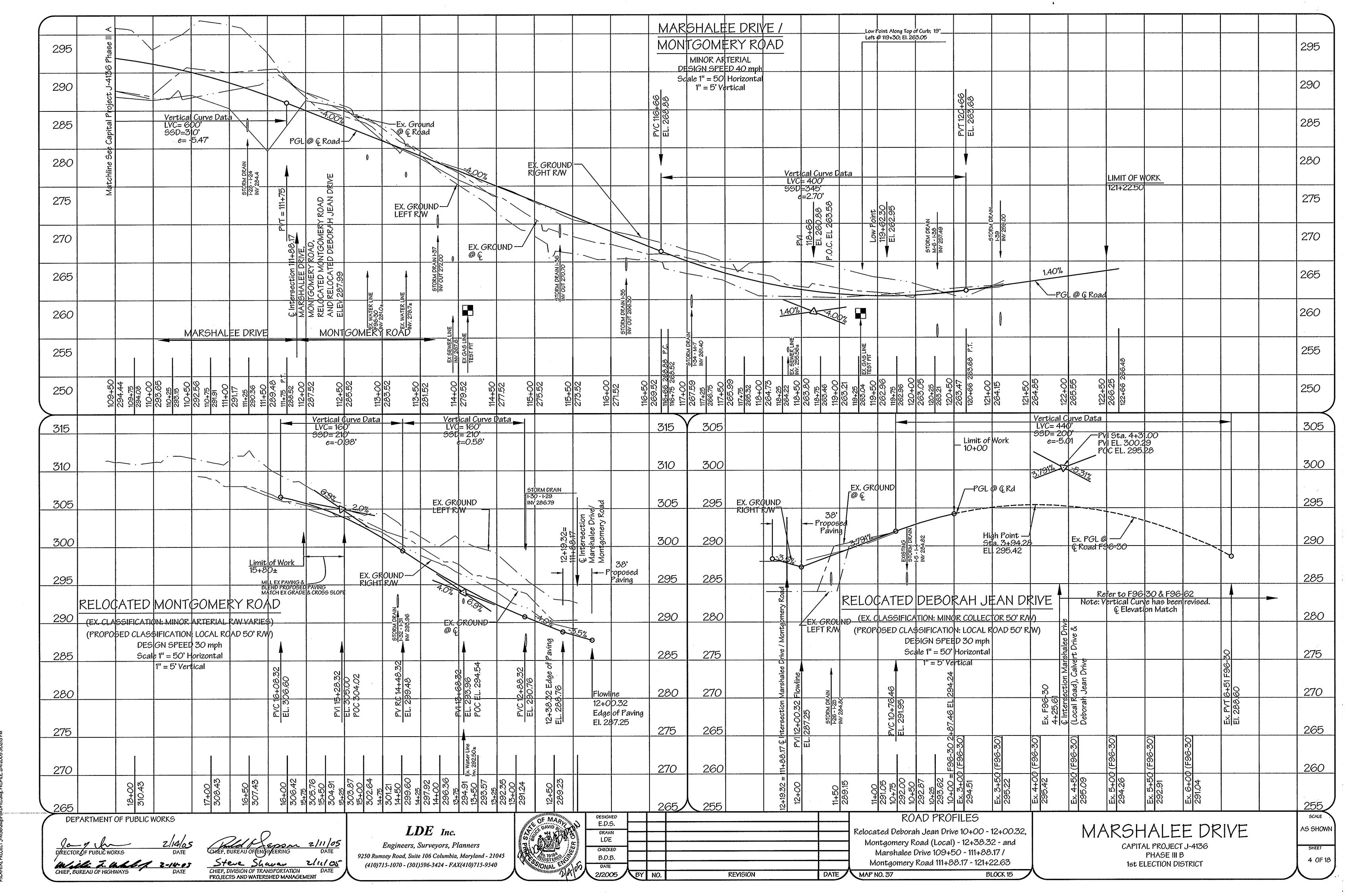
BLOCK 15

6/2006

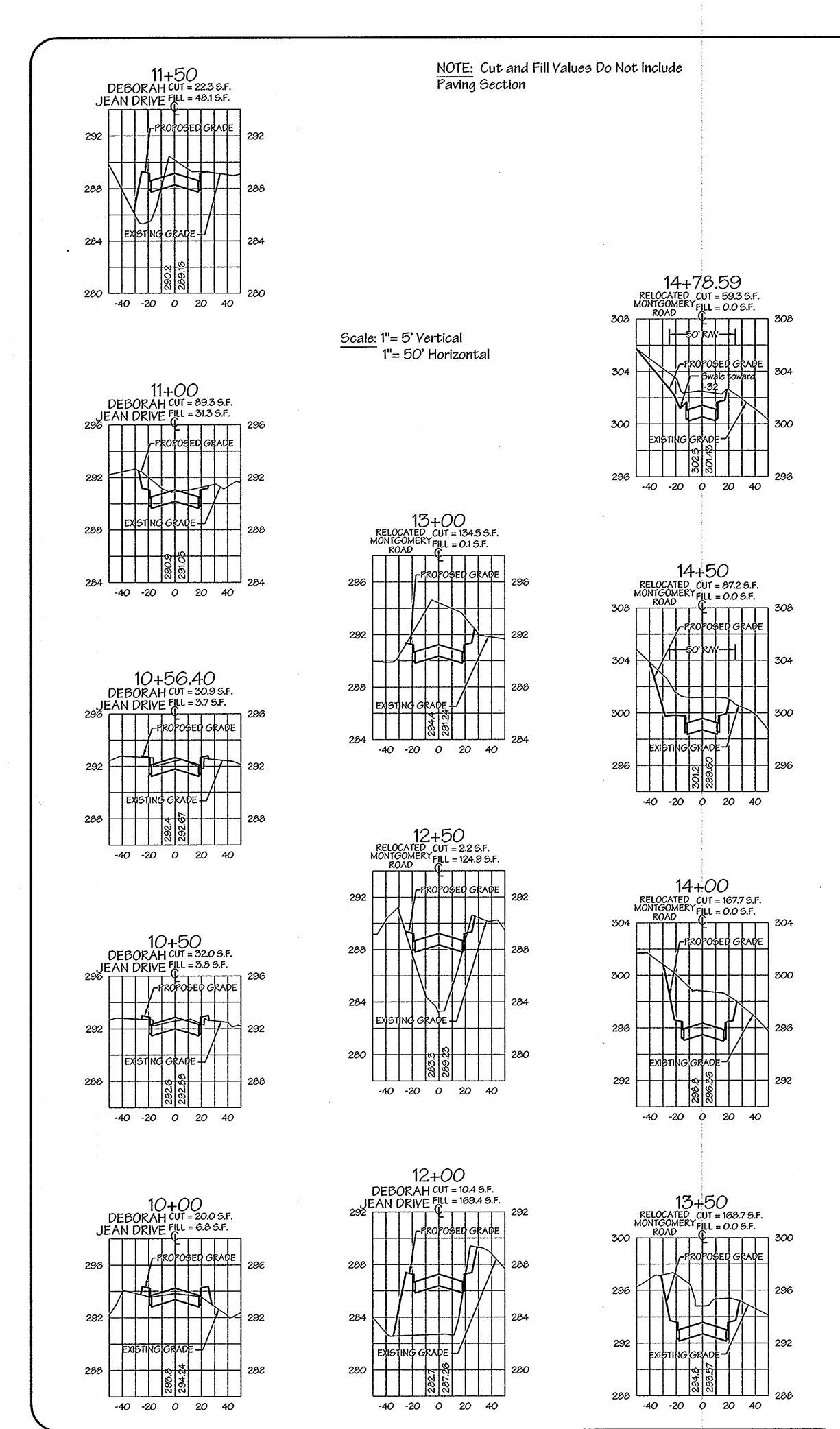
DATE

MAP NO. 37

PHASE III B 1st ELECTION DISTRICT, HOWARD COUNTY, MARYLAND



FICAPITAL PROJECT J-4136/dwg/PBCR055.dwg, CR0551, 2/4/2005 3x08x01 PM



| | | Marshalee | Drive / Mont | gomery Roa | id - EARTHW | ORK | |
|-----------------|-------------|--------------|---------------------|----------------------|-----------------------|-------------------|-----------------------|
| Road Station | Cut (SF) | Fill (SF) | Average Cut (SF) | Average Fill (SF) | Diff. in Dist (Ft) | Cut Cubic Feet | Fill Cubic Feet |
| 11000 | 0.0 | 388.9 | | and ar | 0 | 0.0 | 0.0 |
| 11050 | 0.0 | 264.6 | 0.00 | 326.75 | 50 | 0.0 | 16337.5 |
| 11100 | 0.0 | 172.3 | 0.00 5.80 | 218.45 244.15 | 50 | 0.0 | 10922.5 |
| 11150 | 11.6 | 316.0 | 6.90 | 271.65 | 50 | 290.0 | 12207.5 |
| 11200 | 2.2 | 227.3 | 81.20 | 113.65 | 50 | 345.0 | 13582.5 |
| 11250 | 160.2 | 0.0 | 161.40 | 0.85 | 50 | 4060.0 | 5682.5 |
| 11300 | 162.6 | 1.7 | 123.40 | 21.70 | 50 | 8070.0 | 42.5 |
| 11350 | 84.2 | 41.7 | 57.95 | 88.30 | 50 | 6170.0 | 1085.0 |
| 11400 | 31.7 | 134.9 | 21.55 | 162.70 | 50 | 2897.5 | 4415.0 |
| 11450 | 11.4 | 190.5 | 7.80 | 212.20 | 50 | 1077.5 | 8135.0 |
| 11500 | 4.2 | 233.9 | 2.90 | 270.20 | 50 | 390.0 | 10610.0 |
| 11550 | 1.6 | 306.5 | 2.65 | 283.65 | 50 | 145.0 | 13510.0 |
| 11600 | 3.7 | 260.8 | 10.75 | 230.20 | 50 | 132.5 | 14182.5 |
| 11650 | 17.8 | 199.6 | 33.30 | 169.35 | 50 | 537.5 | 11510.0 |
| 11700 | 48.8 | 139.1 | 54.45 | 120.35 | 50 | 1665.0 | 8467.5 |
| 11750 | 60.1 | 101.6 | 61.55 | 85.95 | 50 | 2722.5 | 6017.5 |
| 11800 | 63.0 | 70.3 | 57.95 | 61.30 | 50 | 3077.5 | 4297.5 |
| 11850 | 52.9 | 52.3 | 51.85 | 53.55 | 50 | 2897.5 | 3065.0 |
| 11900 | 50.8 | 54.8 | 50.90 | 64.80 | 50 | 2592.5 | 2677.5 |
| 11950 | 51.0 | 74.8 | 46.70 | 79.45 | 50 | 2545.0 | 3240.0 |
| 12000 | 42.4 | 84.1 | 54.80 | 87.45 | 50 | 2335.0 | 3972.5 |
| 12050 | 67.2 | 90.8 | 68.35 | 115.80 | 50 | 2740.0 | 4372.5 |
| 12100 | 69.5 | 140.8 | 66.95 | 138.10 | 50 | 3417.5 | 5790.0 |
| 12122.4 | 64.4 | 135.4 | 32.20 | 67.70 | 22.37 | 1497.7 | 3089.3 |
| 12125 | 0.0 | 0.0 | | | 2.63 | 84.7 | 178.1 |
| | | | | | | Cut 49689.9 | Fill CF 167389.8 (|
| | | | | | TOTAL | Cut 1841 | Fill CY 6200 C |

| Road Station | Cut (SF) | Fill (9F) | Average Cut (SF) | Average Fill (SF) | Diff. in Dist (Ft) | Cut Cubic Feet | | Fill Cubic Feet | _ |
|-----------------|----------------------|--------------|---------------------|----------------------|-----------------------|-------------------|----|--------------------|----|
| 1000 | 20.0 | 6.8 | | | | 0.0 | | 0.0 | |
| | | | 26.00 | 5.30 | | | | | |
| 1050 | 32.0 | 3.8 | | | 50 | 1300.0 | | 265.0 | |
| 1050 10 | 700 | a a | 31.45 | <i>3.7</i> 5 | C 4 | 0017 | | 04.0 | |
| 1056.40 | 30.9 | 3.7 | 60.10 | 17.50 | 6.4 | 201.3 | | 24.0 | |
| 1100 | 89.3 | 31.3 | 80.10 | 17.50 | 43.6 | 2620.4 | | 763.0 | |
| 1100 | 00.0 | 01.0 | 55.80 | 39.70 | 1010 | 202011 | | | |
| 1150 | 22.3 | 48.1 | | | 50 | 2790.0 | | 1985.0 | |
| | | | 16.35 | 108.75 | | | | | |
| 1200 | 10.4 | 169.4 | | | 50 | 817.5 | | 5437.5 | |
| | | | 6.30 | 147.15 | | 9.5 0 | | era e e e | |
| 1250 | 2.2 | 124.9 | 60.75 | 60.50 | 50 | 315.0 | | 7357.5 | |
| 1300 | 134.5 | <i>O</i> .1 | 68.35 | 62.50 | 50 | 3417.5 | | 3125.0 | |
| 1000 | 10 1.0 | 0.11 | 151.60 | 0.05 | 00 | o milo | | 0.20.0 | |
| 1350 | 168.7 | 0.0 | | | 50 | 7580.0 | | 2.5 | |
| | | | 168.20 | 0.00 | | | | | |
| 1400 | 167.7 | 0.0 | | | 50 | 8410.0 | | 0.0 | |
| | a | | 127.45 | 0.00 | | | | | |
| 1450 | 87.2 | 0.0 | 72.05 | 0.00 | 50 | 6372.5 | | 0.0 | |
| 1478.59 | 59.3 | 0.0 | 73.25 | 0.00 | 28.59 | 2094.2 | | 0.0 | |
| -170.00 | <i>∞.</i> . <i>o</i> | 0.0 | 29.65 | 0.00 | 20.00 | 2003.2 | | 0.0 | |
| 1480 | 0.0 | 0.0 | | - · · · · | 1.41 | 41.8 | | 0.0 | |
| | | | | | | Cut | | Fill | |
| | | | | | | 35960.2 | CF | 18959.5 | CF |
| | | | | | | Cut | | Fill | |
| | | | | | TOTAL | 1332 | CY | 702 | CY |

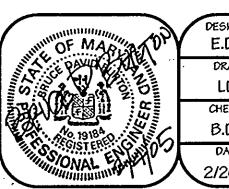
DEPARTMENT OF PUBLIC WORKS

CHIEF, BUREAU OF HIGHWAYS DATE

Steve Sharan 2/11/05
CHIEF, DIVISION OF TRANSPORTATION DATE
PROJECTS AND WATERSHED MANAGEMENT

LDE Inc.

Engineers, Surveyors, Planners 9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045 (410)715-1070 - (301)596-3424 - FAX(410)715-9540



| 9 | DESIGNED | | | | | |
|-------|-------------------|----|-----|----------|------|------------|
| י דע) | E.D.S. | Y | | | | DEBORAH. |
| | DRAIWN LDE | | | | | CF |
| | СНЕСКЕО В.D.В. | | | | | 10- |
| (NO) | DATE | 1 | | | | |
| ソ | 2/2005 | BY | NO. | REVISION | DATE | MAP NO. 37 |

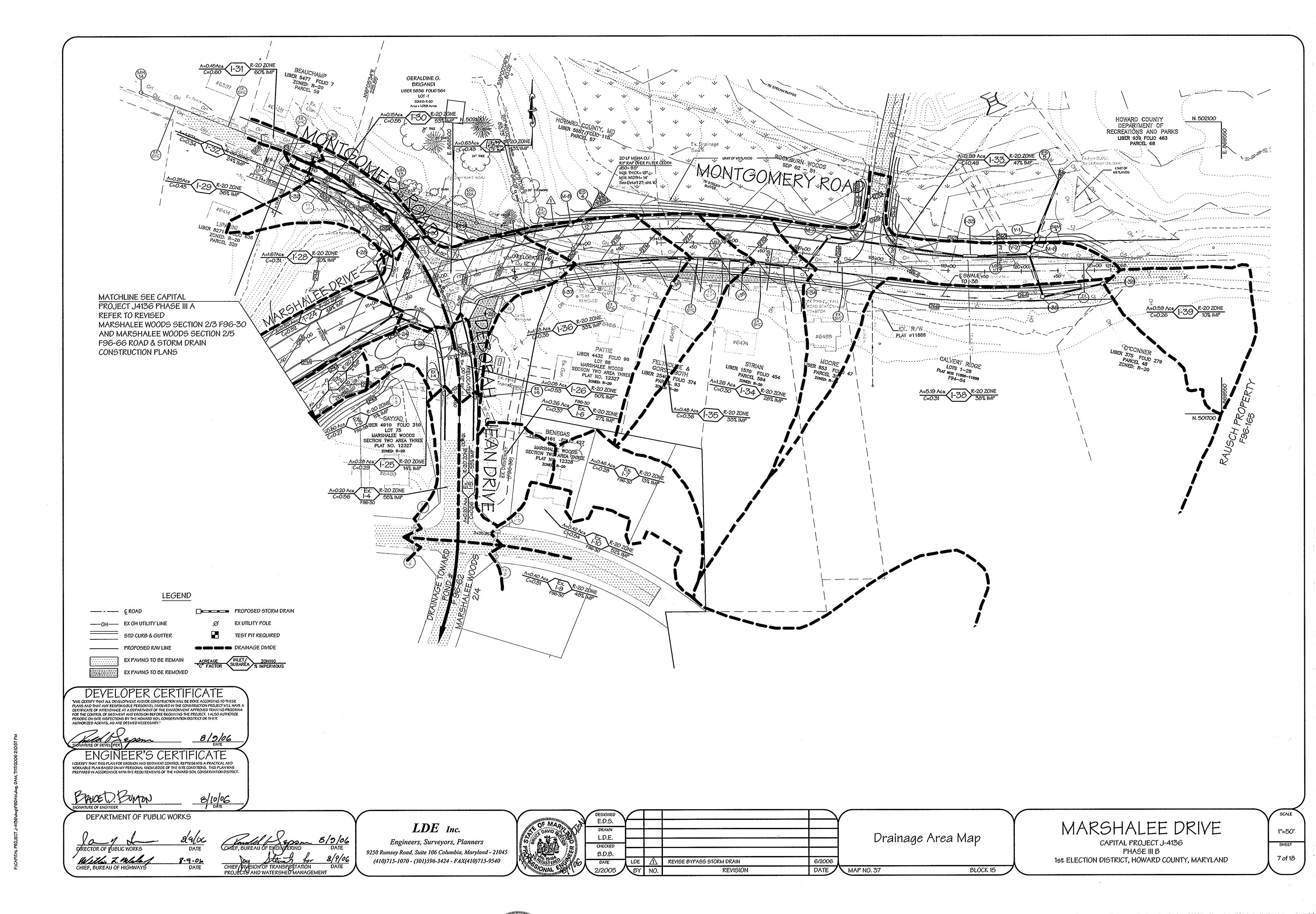
DEBORAH JEAN DRIVE / MONTGOMERY POND CROSS SECTIONS 10+00 TO 14+78.59

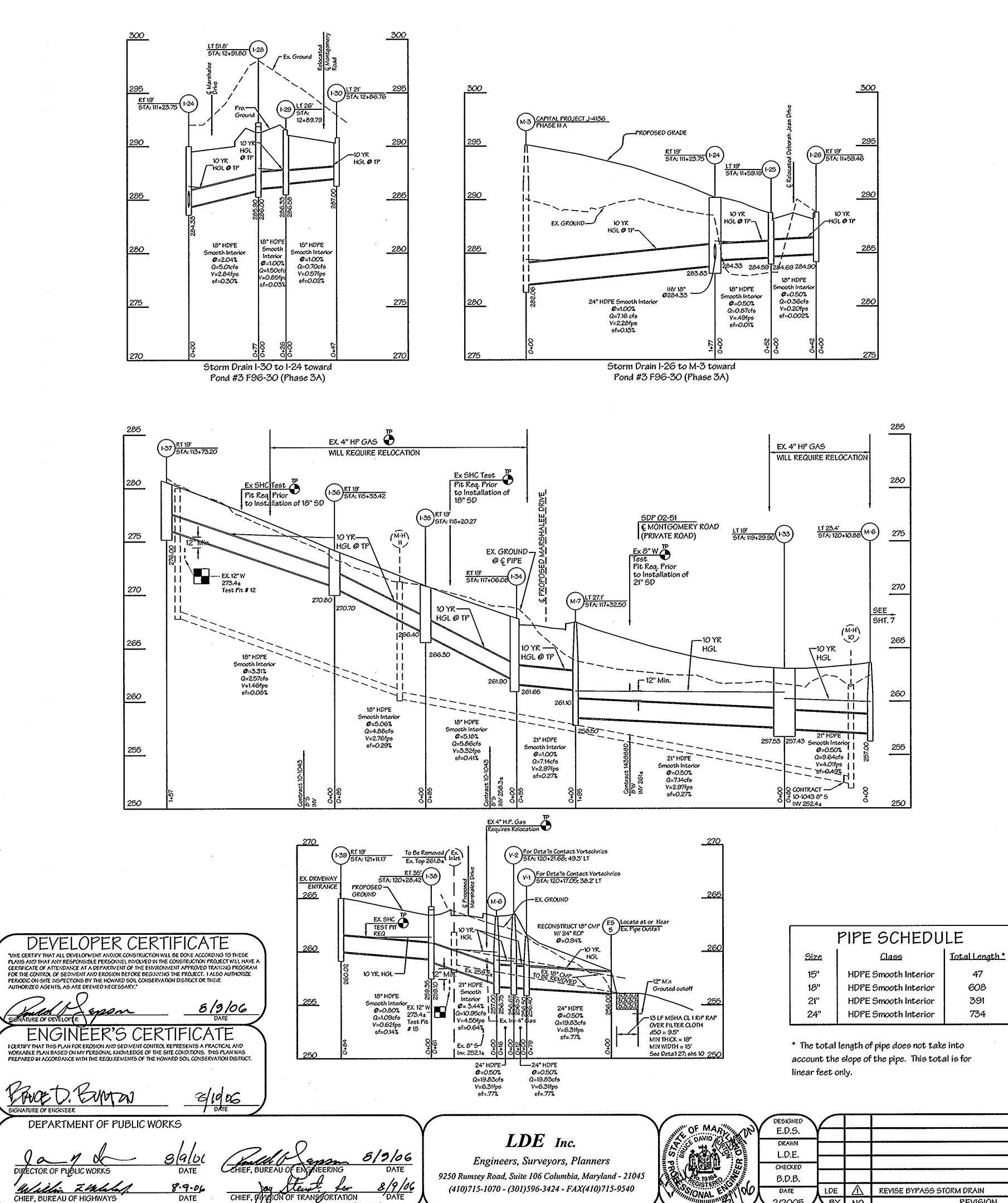
BLOCK 15

MARSHALEE DRIVE CAPITAL PROJECT J-4136 PHASE III B

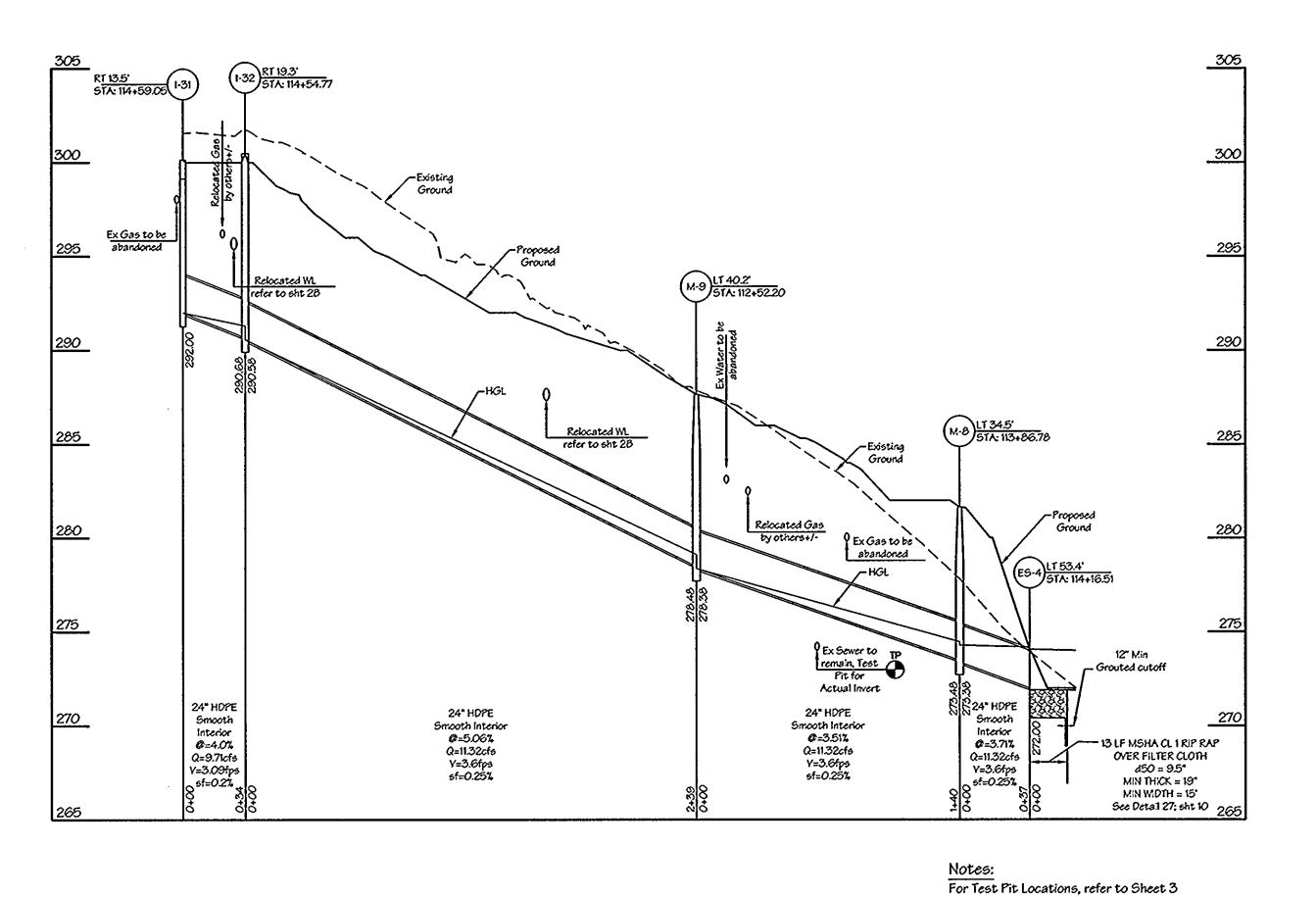
1st ELECTION DISTRICT, HOWARD COUNTY, MARYLAND

1"=5' Yert. 1"=50' Hor. 6 OF 18





CHIEF, CIVISION OF TRANSPORTATION PROJECTS AND WATERSHED MANAGEMENT



Notes:

- 1. Where test pits have been made on existing utilities, they are noted by the symbol at the location of the test pit (See Sheet 3).

 The table containing the results of the test pit or pits is included hereon.
- 2. Existing utilities in the vicinity of the proposed work for which test pits have not been dug shall be located by the Contractor two weeks in advance of construction operations at his own expense. Proposed Test Pits are represented by the symbol 🚡
- 3. LDE, Inc. shall be notified of all conflicts for possible redesign.

| | | | | | Str. No. | Structure Type | inv. In | Inv. Out | Top Elevation or Upper / Lower | Detail | Location | Remarke |
|------------|---------------------------------------|---------------------|-------------------|-------------------|------------|----------------------|-------------------|-----------------|---------------------------------------------------|--------------|----------------------------------------------|------------------------------------|
| | | | | | 1-24 | COG-10 | 284.33 / 284.33 | 283.83 | 290.52 / 290.15 | MD-374.51 | 111+23.75; 19'RT | |
| | Test | t Hole Tab | 2 P | | 1-25 | COG-5 | 284.69 | 284.59 | 289.50 / 289.40 | MD-374.51 | 11+59.19; 19°LT | |
| | · · · · · · · · · · · · · · · · · · · | _ | | Ta . 45 | 1-26 | COG-5 | • | 284.90 | 288.75 / 288.60 | MD-374.51 | 11+59.46; 19' RT | |
| Test Hole# | Survey Marker | Ground Elevation | Field Measured | Top of Utility | 1-28 | Yard Inlet | 286.00 | 285.90 | 290.00 | MD-381.02 | 12+91.80; 51.8' LT | |
| 1 | Peg | 301.41 | 3.21 | 298.20 | 1-29 | COG-10 | 286.58 | 286.33 | 291.65 / 291.50 | MD-374.51 | 12+89.79; 24.09' LT | |
| 2 | Peg | 298.86 | 2.57 | 296.29 | 1-30 | COG-10 | • | 287.00 | 291.12 / 290.68 | MD-374.51 | 12+86.76; 19' RT | |
| 3 | Peg | 295.39 | 3.65 | 291.74 | 1-31 | COG-10 | - | 292.00 | 300.90 / 300.18 | MD-374.51 | 14+59.05; 13.5' RT | |
| 4 | Peg | 283.69 | 3.67 | 280.02 | 1-32 | Type "D" | 290.68 | 290.58 | 300.33 | MD-378.05 | 14+54.77; 19.3' LT | See Note 1-Throat Uphill Side Only |
| 5 | P.K. Nail | 262.09 | 3.28 | 258.81 | 1-33 | COG-20 | 257.53 | 257.43 | 262.98 | MD-374.61 | 119+29.90; 19'LT | Sump Inlet |
| 6 | P.K. Nail | 263.02 | 3.36 | 259.66 | 1-34 | COG-10 | 261.90 | 261.65 | 267.96 / 267.64 | MD-374.51 | 117+06.08; 19' LT | |
| 7 | 8" Nail | 301.12 | 4.20 | 296.92 | 1-35 | COG-10 | 266.40 | 266.30 | 270.91 / 270.47 | MD-374.51 | 116+20.27; 19° RT | |
| 8 | 8" Nail | 300.41 | 4.32 | 296.09 | 1-36 | COG-10 | 270.80 | 270.70 | 274.33 / 273.89 | MD-374.51 | 115+33.42; 19' RT | |
| 9 | P.K. Nail | 289.64 | 4.02 | 285.62 | 1-37 | COG-10 | | 276.00 | 280.74 / 280.30 | MD-374.51 | 113+68.09; 21.0° RT | |
| 10 | P.K. Nail | 284.12 | 4.58 | 279.54 | 1-38 | Yard Inlet | 259.35 | 259.10 | 263.50 | MD-381.02 | 120+28.42; 35' RT | |
| 11 | P.K. Nail | 281.36 | 4.41 | 276.95 | 1-39 | Type "5" | - | 260.02 | 265.00 | MD-374.73 | 121+11.17; 19' RT | See Note 2 |
| 12 | P.K. Nail | 278.68 | 4.25 | 274.43 | | | | | | | | |
| 13 | Peg | 264.52 | 3.31 | 261.21 | M-6 | Manhole | 257.00 | 256.75 | 263.30 | MD-383.01 | 120+10.88; 23.4° LT | |
| 14 | P.K. Nail | 261.38 | 3.81 | 257.57 | M-7 | Manhole | 261.10 | 258.50 | 267.50 | MD-383.01 | 117+32.50; 27.1' LT | |
| 15 | P.K. Nail | 261.30 | 3.82 | 257.48 | м-8 | Manhole | 273.48 | 273.38 | 281.80 | MD-383.01 | 113+86.78;34.5°LT | |
| 16 | Peg | 288.29 | 3.76 | 284.53 | М-9 | Manhole | 278.48 | 278.38 | 287.70 | MD-383.01 | 112+52.20; 40.2 LT | |
| 17 | Peg | 288.76 | 0.91 | 287.85 | V-1 | Junction Manhole | 256.50 | 256.40 | 261.00 | MD-383.01 | 120+21.68; 49.26' LT | See Sheet 18 / Vortechnics |
| 18 | Peg | 286.93 | 6.09 | 280.84 | V-2 | Bypass Manhole | 256.67 | 256.57 | 264.00 | MD-383.01 | 120+17.05; 38.18' LT | See Sheet 18 / Vortechnics |
| 19 | Peg | 286.98 | 1.19 | 285.79 | ES-4 | HDPE End Section | 272.0 | 00 | 274.00 | - | 114+16.51; 53.42 LT | Typical HDPE Flare End Section |
| 20 | Peg | 287.50 | 3.51 | 283.99 | E9-5 | HDPE End Section | 256. | | 258.00 | _ | 11710.00,0071227 | Typical HDPE Flare End Section |
| 21 | Peg | 287.50 | 1.59 | 285.91 | <u> </u> | | | | d proposed curb & gutte | .I1 r | <u>. </u> | 1 - 13F |
| 22 | Peg | 287.66 | 3.53 | 284.13 | | • | | | a proposed corba gove at existing ground eleva | | | |
| 23 | Peg | 287.74 | 4.66 | 283.08 | 2. 1101210 | any washing our last | Liviation iniorgi | are sop onem so | V.3021119 9100101 01012 | | | |
| 24 | Peg | 287.79 | 1.12 | 286.67 | | | | | | , | | |

REVISION

BY NO.

6/2006

DATE

MAP NO. 37

Storm Drain Profiles

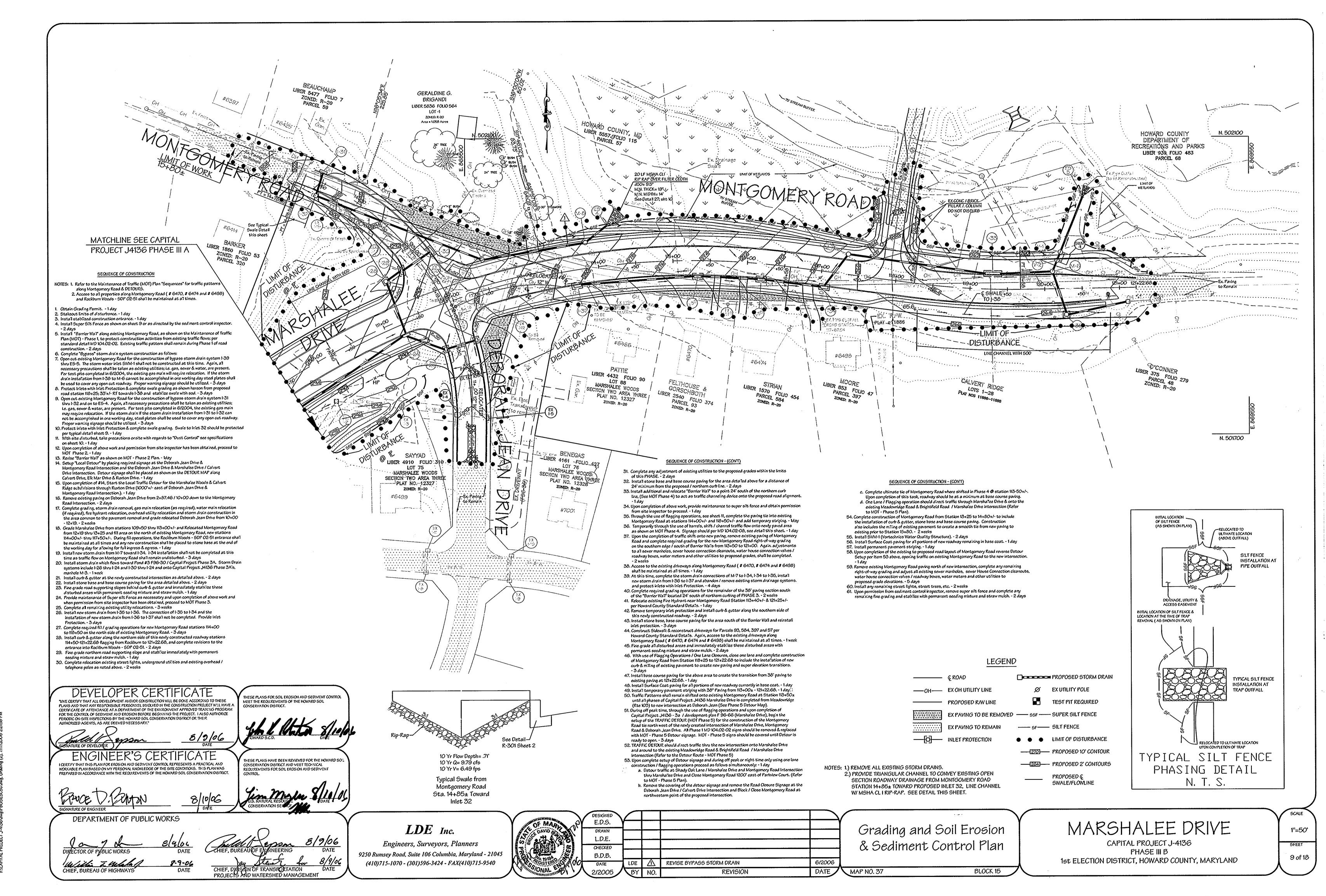
BLOCK 15

MARSHALEE DRIVE

STRUCTURE SCHEDULE

CAPITAL PROJECT J-4136 PHASE III B 1st ELECTION DISTRICT, HOWARD COUNTY, MARYLAND

1"=50" 8 of 18



13. The total amount of super sat ferce = 1,230 L.F. 4. The total amount of earth dike = 0 15. The total amount of super diversion fence = 0 HOWARD SOIL CONSERVATION DISTRICT PERMANENT SEEDING NOTES

can be back filed and stabilized within one working day, whichever is shorter.

approval by the inspection agency is made.

12. The total amount of sat fence = 0

sediment controls, but before proceeding with any other earth disturbance or grading. Other building or grading inspection approvals may not be authorized until this irritial

Trenches for the construction of utilities is limited to three pipe lengths or that which

Apply to graded or cleared areas not subject to immediate further disturbance where a permanent long-lived vegetative cover is needed

SEEDBED PREPARATION: Loosen upper three inches of soil by raking, disking, or other acceptable means before seeding, if not previously loosered.

SOIL AMENDMENTS: In lieu of soil test recommendations, use one of the following

1) PREFERRED -- Apply 2 tons per acres dolomitic limestone (92 lbs/1000sq. ft.) and 600 lbs per acre 10-10-10 fertilizer (14 lbs/1000 sq. ft.) before seeding. Harrow or disk into upper three inches of soil. At time of seeding, apply 400 los per acre 30-0-0 ureaform

fertilizer (9 lbs/1000sq. ft.) 2) ACCEPTABLE -- Apply 2 tons per acres dolomitic limestone (92 lbs/1000sq. ft.) and 1000 lbs per acre 10-10-10 fertaizer (23 lbs/1000 sq. ft.) before seeding. Harrow or disk into upper three inches of soil.

SEEDING -- For the periods March 1 thru April 30, and August 1 thru October 15, seed with 60 lbs per acre (1.4 lbs/1000sq. ft.) of Kentucky 31 Tall Fescue. For the period May 1 thru July 31, seed with 60 lbs per acre (1.4 lbs/1000so. ft.) of Kentucky 31 Tall Fescus and 2 lbs. per acre (05 lbs/1000sq. ft.) of weeping lovegrass. During the period of October 16 thru February 28, protect site by Option (1) - 2 tons per acre of well archored straw mulch and seed as soon as possible in the spring. Option (2) - Use sod. Option (3) - Seed with 60 lbs. per acre Kentucky 31 Tall Fescue and mulch 2 tons / acre well archored straw.

MULCHING -- Apply 1-1/2 to 2 tons per acre (70 to 90 lbs/1000sq. ft.) of unrotted small grain straw immediately after seeding. Anchor mulch immediately after application using mulch archoring tool or 218 gallons per acre (5 gal/1000sq. ft.) of emulsified asphalt on flat areas. On slopes 8 feet or higher, use 348 gallons per acre (8 gal/1000sq.ft.) for anchoring.

MAINTENANCE -- Inspect all seeding areas and make reeded repairs, replacements and

HOWARD SOIL CONSERVATION DISTRICT TEMPORARY SEEDING NOTES Apply to graded or cleared areas likely to be redisturbed where a short-term/egetative cover

SEEDBED PREPARATION: -- Loosen upper three inches of soil by raking, disking, or other acceptable means before seeding, if not previously loosered.

SOIL AMENDMENTS: -- Apply 600 lbs per zore 10-10-10 fertifizer (14 lbs/1000sq. ft.).

SEEDING -- For periods March 1 thru April 30, and from August 15 thru October 15 seed with 2-12 bushels per acre of annual rye (3.2 lbs/1000so, ft.). For the period May 1 thru August 14, seed with 3 lbs. per acre of weeping lovegrass (.07 lbs/1000so, ft.). For the period November 16 thru February 28, protect site by applying 2 tons per acre of well anchored straw mulch and seed as soon as possible in the spring, or use sod.

MULCHING -- Apply 1-1/2 to 2 tons per scre (70 to 90 Fbs/1000eq. ft.) of unrotted weed free small grain straw immediately after seeding. Archor mulch immediately after application using mulch anchoring tool or 218 gallons per acre (5 gal/1000so, ft.) of emulsified asphalt on flat areas. On slopes 8 feet or higher, use 348 gallons per acre (8 gal/1000sq. ft.) for archoring

Refer to the 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL for additional rates and methods not covered.

TWE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE ACCORDING TO THESE PLANS AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SED WENT AND EROSION BEFORE BEGINNING THE PROJECT. LAUSO AUTHORIZE PERIODIC ON-SITE INSPECTIONS BY THE HOWARD SOIL CONSERVATION DISTRICT OR THEIR authorized agents, as are deeved neces

I CERTIFY THAT THIS PLAN FOR EROSION AND SED WENT CONTROL REPORTED A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDS FOR THE STEEDING SO. THIS PLAN WAS ESS. THIS PLANWAS PREPARED IN ACCORDANCE WITH THE REQUIREMENT

PROJECTS AND WATERSHED MANAGEMENT

30.0 DUST CONTROL Definition

Controlling dust blowing and movement on construction sites and roads.

Purpose

To prevent blowing and movement of dust from exposed soil surfaces, reduce on and off-site damage, health hazards, and improve traffic safety.

Conditions Where Practice Applies

Specifications 5 4 1

This practice is applicable to areas subject to dust blowing and movement where on and off-site damage is likely without treatment

Temporary Methods

1. Mulches- See standards for vegetative stabilization with mulches only. Mulch Should be crimped or tacked to prevent blowing.

2. Vegetative Cover- See standards for temporary vegetative cover.

3. Tillage- To roughen surface and bring clods to the surface. This is and emergency measure which should be used before soil blowing starts. Begin plowing on windward side of site. Chisel-type plows spaced about 12" apart, spring- toothed harrows, and similar plows are examples of equipment

4. Irrigation-This is generally done as an emergency treatment. Site is sprinkled with water until the surface is moist. Repeat as reeded. At no time should the site be imigated to the point that

5. Barriers- Solid board fences, sat fences, snow fences, burlap fences, straw bales, and similar material can be used to control air currents and soil blowing. Barriers placed at right angles to prevailing currents at intervals of about 10 times their height are effective in controlling soil blowing.

6. Calcium Chloride- Apply at rates that will keep surface moist. May need retreatment.

1. Permanent Vegetation-See standards for permanent vegetative cover, and permanent stabilization with sod. Existing trees or large shrubs mat afford valuable protection if left in place.

2. Topsoiling. Covering with less crosive materials. See standards for topsolling.

3. Store - Cover surface with crushed store or coarse gravel.

Permanent Methods

1. Agriculture Handbook. Wind erosion Forces in the United States and Their Use in Predicting Soil Loss.

2. Agriculture Information Bulletin 354, How to Control Wind Erosion, USDA-ARS.

H-30-1

21.0 STANDARD AND SPECIFICATIONS FOR TOPSOIL

Placement of topsoil over a prepared subsoil prior to establishment of permanent vegetation.

To provide a suitable soil medium for vegetative growth. Soils of concern have low moisture content, low nutrient levels, low pH, materials toxic to plants, and/or unacceptable soil gradation.

Conditions Where Practice Applies

1. This practice is limited to areas having 2:1 or flatter slopes where:

a. The texture of the exposed subsoll/parent material is not adequate to produce vegetative growth. b. The soil material is so shallow that the rooting zone is not deep enough to support plants or furnish continuing supplies of moisture and plant nutrients.

c. The original soil to be vegetated contains material toxic to plant growth d. The soil is so acidic that treatment with Emestone is not feasible.

II. For the purpose of these Standards and Specifications, areas having slopes steeper than 2:1 require special consideration and design for adequate stabilization. Areas having slopes steeper than 21 shall have the appropriate stabilization shown on the plans.

Construction and Material Specifications

1. Topsoil salvaged from the existing site may be used provided that it meets the standards as set forth in these specifications. Twoically, the depth of topsoil to be salvaged for a given soil type can be found in the representative soil profile section in the Soil Survey published by USDA-SCS in cooperation with

Maryland Agricultural Experimental Station.

II. Topsoil Specifications - Soil to be used as topsoil must meet the following:

i. Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay Loam, loamy sand. Other soils may be used if recommended by an agronomist or soil scientist and approved by the appropriate approval authority. Regardless, topsoil shall not be a mixture of contrasting textured subsoils and shall contain less than 5% by volume of cinders, stores, slag, coarse fragments, gravel, sticks, roots,

trash, or other materials larger than 1-1/2" in diameter. ii. Topsoil must be free of plants or plant parts such as bermuda grass, quackgrass, Johnsongrass, nutsedge, poison by, thistle, or others as specified.

Ei. Where the subsoil is either highly acidic or composed of heavy clays, ground limestone shall be spread at the rate of 4-8 tons/acre (200-400 pounds per 1,000 square feet) prior to the placement of topsoil. Lime shall be distributed uniformly over designated areas and worked into the soil in conjunction with tillage operations as described in the following procedures.

III. For sites having disturbed areas under 5 acres: i. Place topsoil (if required) and apply soil amendments as specified in 20.0 Yegetative Stabilization -

Section I - Vegetative Stabilization Methods and Materials. IV. For sites having disturbed areas over 5 acres: i. On soil meeting Topsoil specifications, obtain test results dictating fertilizer and lime amendments

required to bring the soil into compliance with the following: a. pH for topsoil shall be between 6.0 and 7.5. If the tested soil demonstrates a pH of less than 6.0, sufficient lime shall be prescribed to raise the pH to 6.5 or higher. b. Organic content of topsoil shall be not less than 1.5 percent by weight c. Topsoil having soluble salt content greater than 500 parts per mation shall not be used.

d. No sod or seed shall be placed on soil which has been treated with soil sterilants or chemicals used for weed control until sufficient time has elapsed (14 days min.) to permit dissipation of phyto-toxic materials. Note: Topsoil substitutes or amendments, as recommended by a qualified agronomist or soil scientist and approved by the appropriate approval authority, may be used in lieu of natural toosoil.

ii. Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization -- Section I - Vegetative Stabilization Methods and Materials.

When topsoiling, maintain needed erosion and sediment control practices such as diversions, Grade Stabilization Structures, Earth Dikes, Slope Silt Fence and Sediment Traps and Basins. ii. Grades on the areas to be topsoiled, which have been previously established, shall be maintained. a:beit 4" - 8" higher in devation.

ii. Topsoil shall be uniformly distributed in a 4" - 8" layer and lightly compacted to a minimum thickness of 4". Spreading shall be performed in such a marner that sodding or seeding can proceed with a minimum of additional soil preparation and tillage. Any irregularities in the surface resulting from topsolling or other operations shall be corrected in order to prevent the formation of decressions or

iv. Topsoil shall not be placed while the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet or in a condition that may otherwise be detrimental to proper grading and VI. Alternative for Permanent Seeding - Instead of applying the full amounts of lime and commercial fertilizer,

composted studge and amendments may be applied as specified below: i. Composted Studge Material for use as a soil conditioner for sites having disturbed areas over 5 acres shall be tested to prescribe amendments and for sites having disturbed areas under 5 acres shall conform to the following requirements:

a. Composted sludge shall be supplied by, or originate from, a person or persons that are permitted (at the time of acquisition of the compost) by the Maryland Department of the Environment under COMAR 26.04.06. b. Composted sludge shall contain at least 1 percent ritrogen, 1.5 percent phosphorus, and 0.2 percent potassium and have a Ph of 7.0 to 8.0. If compost does not meet these

requirements, the appropriate constituents must be added to meet the requirements c. Composted sludge shall be applied at a rate of 1 ton/1,000 square feet,

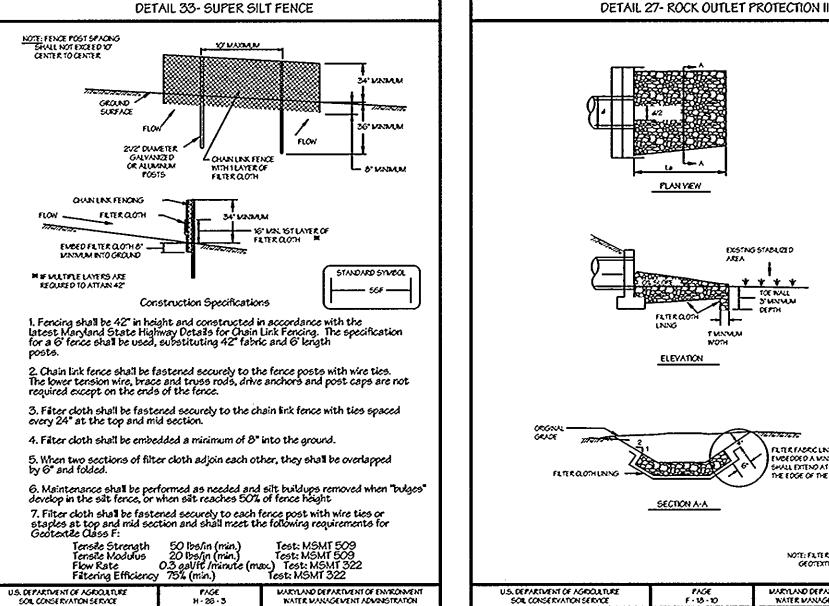
ii. Composted sludge shall be amended with a potassium fertilizer applied at the rate of 4 lb./1,000 square feet, and 1/3 the normal lime application rate.

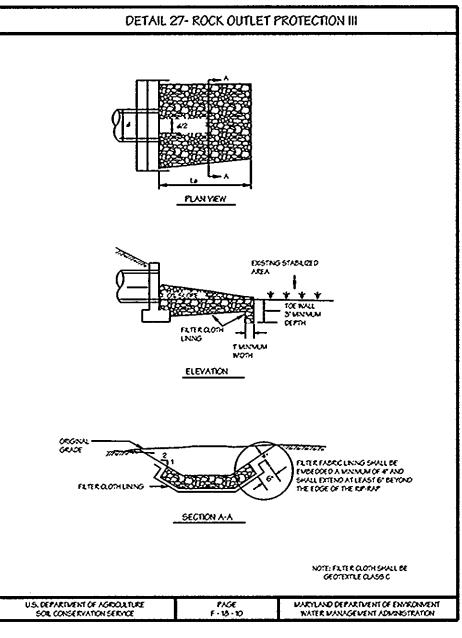
THESE PLANS HAVE BEEN REVIEWED FOR THE HOWARD SOIL THESE PLANS FOR SOIL EROSION AND SEDIMENT CONTROL REET THE REQUIREMENTS OF THE HOWARD SOL CONSERVATION DISTRICT AND MEET TECHNICAL

LDE Inc.

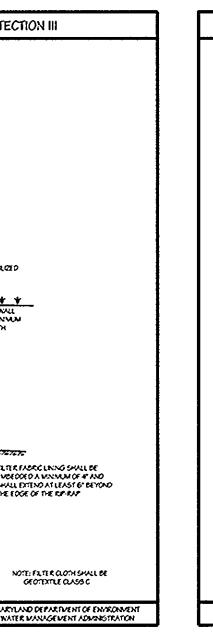
Engineers, Surveyors, Planners

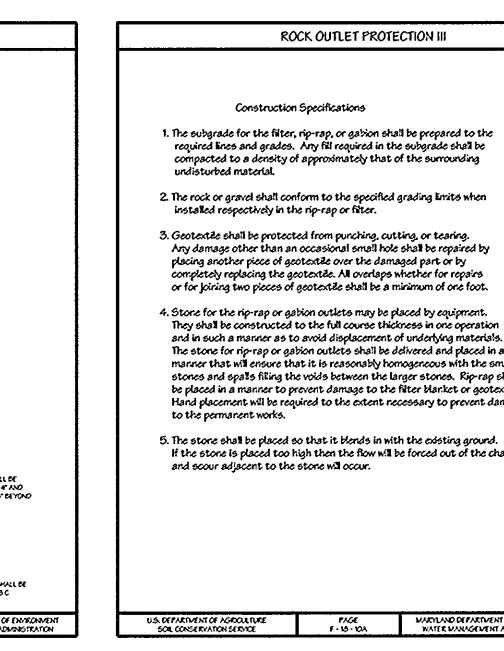
9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045 (410)715-1070 - (301)596-3424 - FAX(410)715-9540





DETAIL 26 - ROCK OUTLET PROTECTION II





ROCK OUTLET PROTECTION III

Construction Specifications

installed respectively in the rip-rap or filter.

undisturbed material.

to the permanent works.

and scour adjacent to the stone will occur.

required lines and grades. Any fill required in the subgrade shall be

compacted to a density of approximately that of the surrounding

Any damage other than an occasional small hole shall be repaired by

completely replacing the geotextee. All overlaps whether for repairs

or for joining two pieces of geotextile shall be a minimum of one foot.

They shall be constructed to the full course thickness in one operation

and in such a manner as to avoid displacement of underlying materials.

The stone for rip-rap or gabion outlets shall be delivered and placed in a

manner that will ensure that it is reasonably homogeneous with the smaller

stones and spalls filling the voids between the larger stones. Rip-rap shall

be placed in a manner to prevent damage to the filter blanket or geotextile.

Hand placement will be required to the extent necessary to prevent damage

If the stone is placed too high then the flow will be forced out of the channel

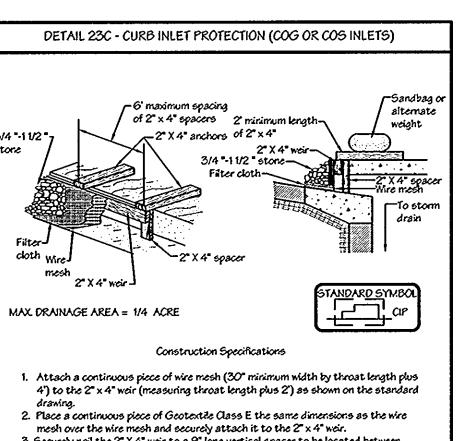
placing another piece of geotextile over the damaged part or by

DETAIL 24 - STABILIZED CONSTRUCTION ENTRANCE EMENEU ---- PATE AS NECKSOARS MNWAM 6" OF 2"-3" AGGREGATE PROFILE SE SCENE Construction Specification 1. Length - minimum of 50' (*30' for single residence lot). 2. Width - 10' minimum, should be flared at the costing road to provide a turning Geotextile fabric (filter cloth) shall be placed over the existing ground prior to placing stone. **The plan approval authority may not require single family residences to use geotextile.

4. Stone - crushed aggregate (2" to 3") or reclaimed or recycled concrete equivalent shall be placed at least 6" deep over the length and width of the

5. Surface Water - all surface water flowing to or diverted toward construction entrances shall be piped through the entrance, maintaining positive drainage. Pipe installed through the stabilized construction entrance shall be protected with a mountable berm with 5:1 slopes and a minimum of 6" of stone over the pipe. Pipe has to be sized according to the drainage. When the SCE is located at a high spot and has no drainage to convey a pipe will not be necessary. Pipe should be sized according to the amount of runoff to be conveyed. A 6" minimum will be required. 6. Location - A stabilized construction entrance shall be located at every point where construction traffic enters or leaves a construction site. Vehicles leaving the site must travel over the entire length of the stabilized construction entrare

#AG€ F - 17 - 3



3. Securely nail the 2" X 4" weir to a 9" long vertical spacer to be located between

the weir and the inlet face (max. 4' apart). 4. Place the assembly against the inlet throat and na1 (minimum 2 lengths of 2" x 4" to the top of the weir at spacer locations). These 2" x 4" anchors shall extend across the inlet top and be held in place by sandbags or alternate weight. 5. The assembly shall be placed so that the end spacers are a minimum I beyond

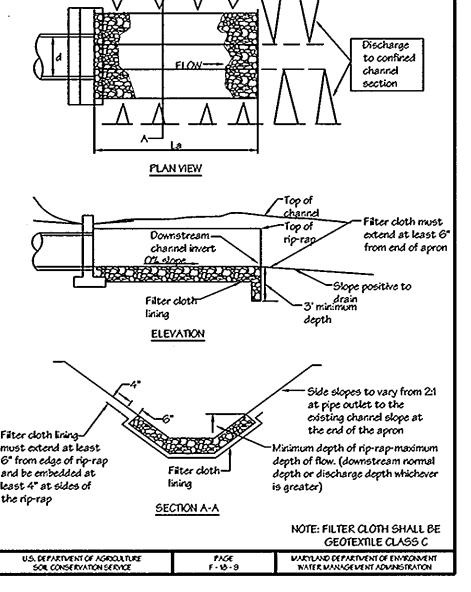
both ends of the throat opening.

6. Form the 1/2 *x 1/2 * wire mesh and the geotextile fabric to the concrete gutter and against the face of the curb on both sides of the inlet. Place clean 3/4"x11/2" stone over the wire mesh and geotextile in such a mariner to prevent water from entering the inlet under or around the geotextile. 7. This type of protection must be inspected frequently and the filter cloth

8. Assure that storm flow does not bypass the inlet by installing a temporary

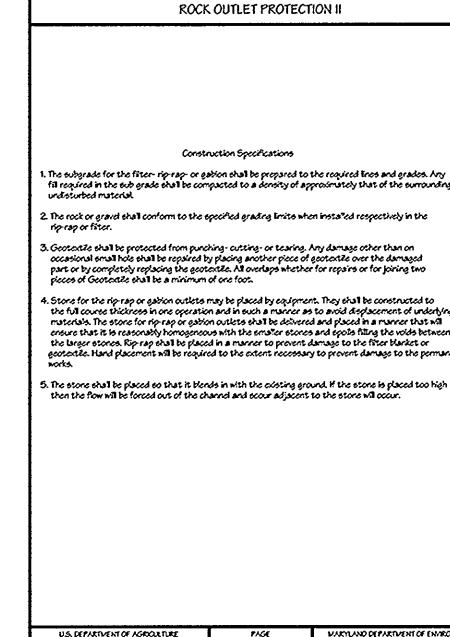
and stone replaced when clogged with sediment.

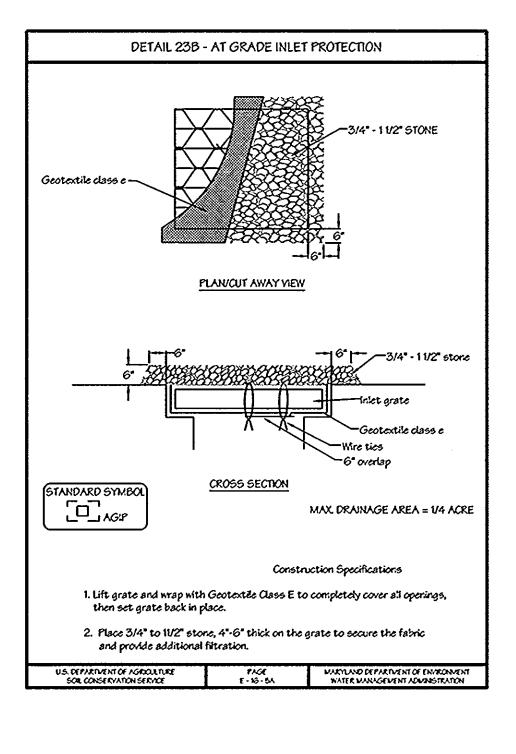
earth or asphalt dike to direct the flow to the inlet. U.S. DEPARTMENT OF AGRICUATURE SOIL CONSERVATION SERVICE



DATE

MAP NO. 37





LDE CHECKED

E.D.S. B.D.B. REVISION

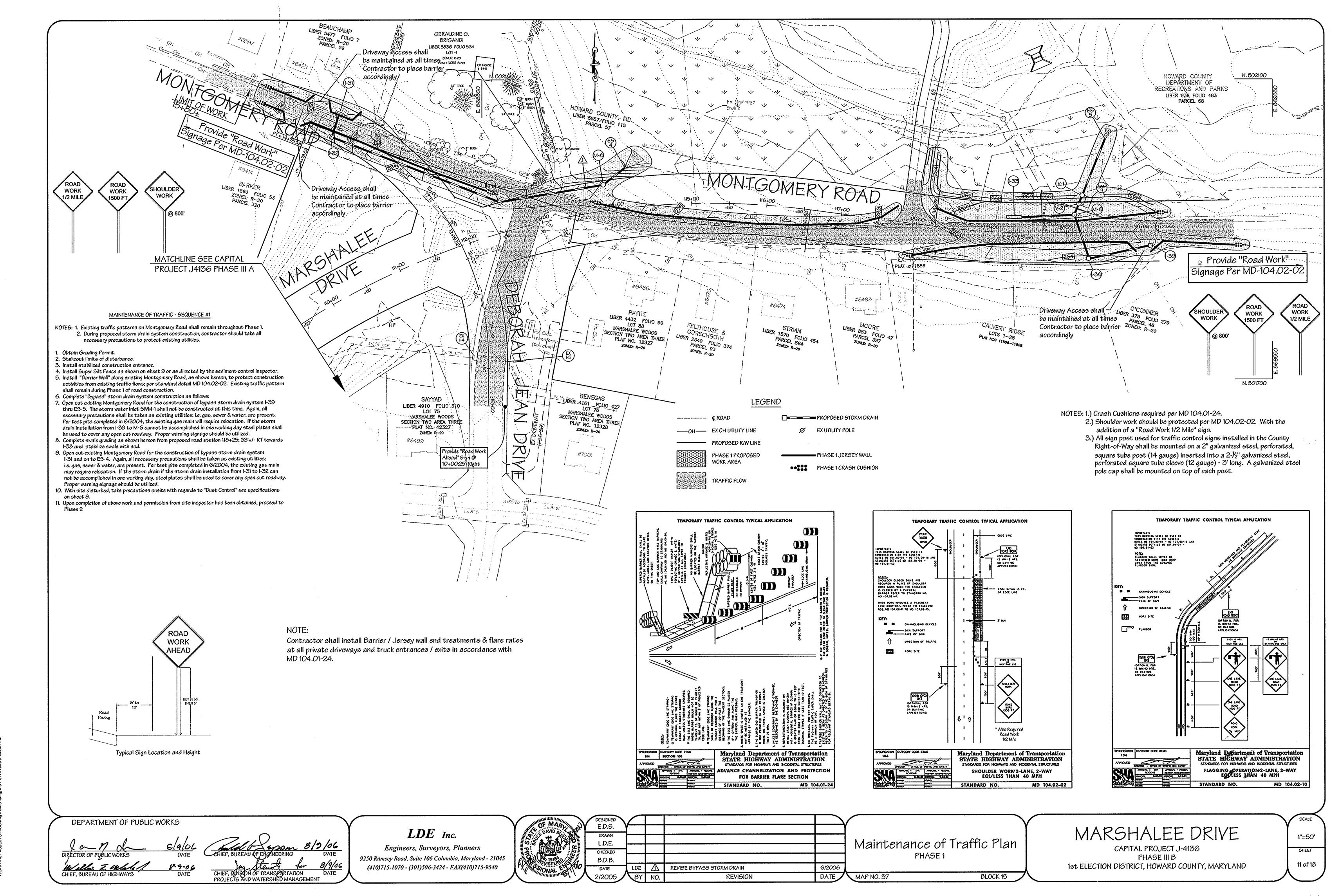
Grading and Soil Erosion & Sediment Control Plan -Details

BLOCK 15

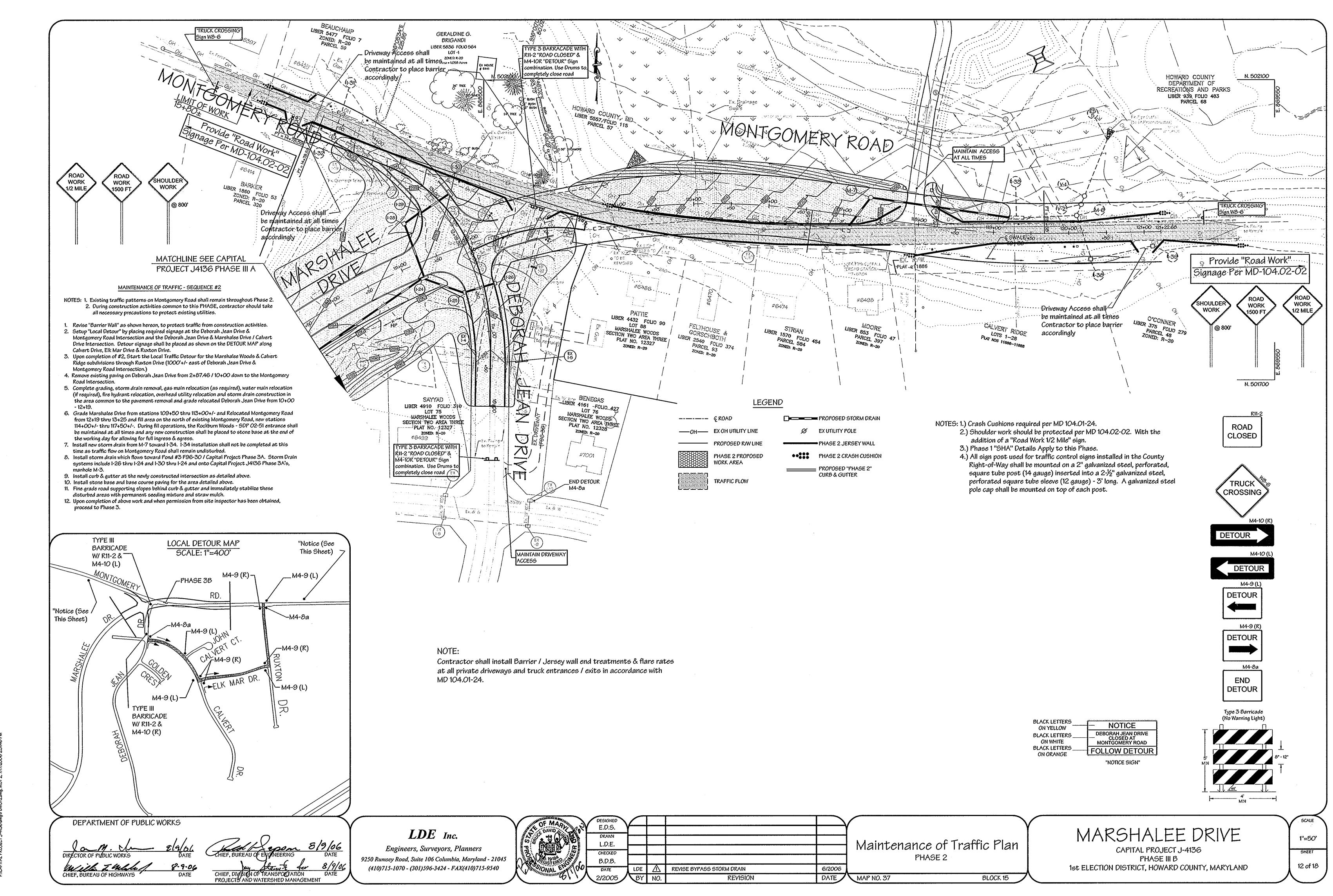
MARSHALEE DRIVE CAPITAL PROJECT J-4136 PHASE III B 1st ELECTION DISTRICT, HOWARD COUNTY, MARYLAND

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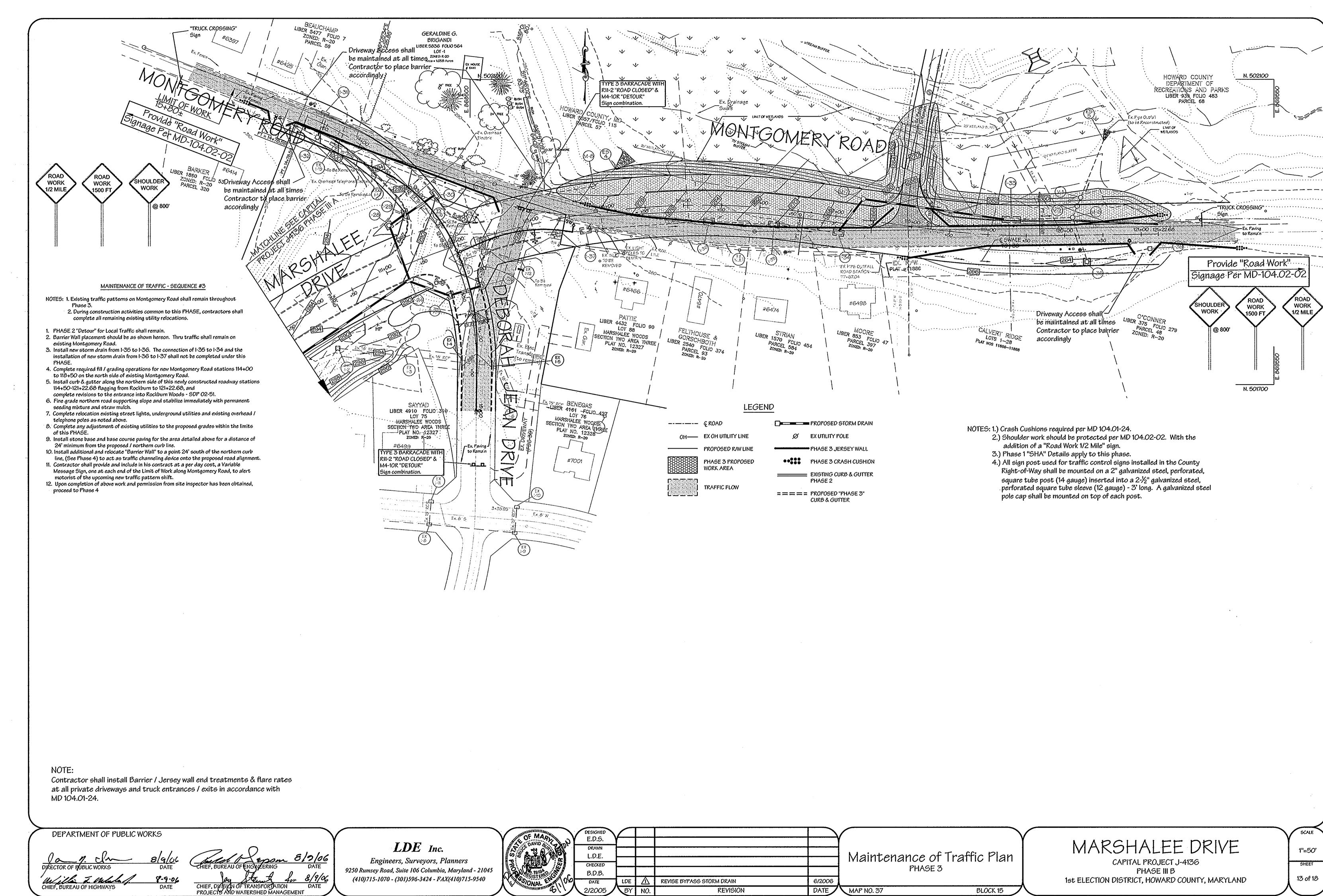
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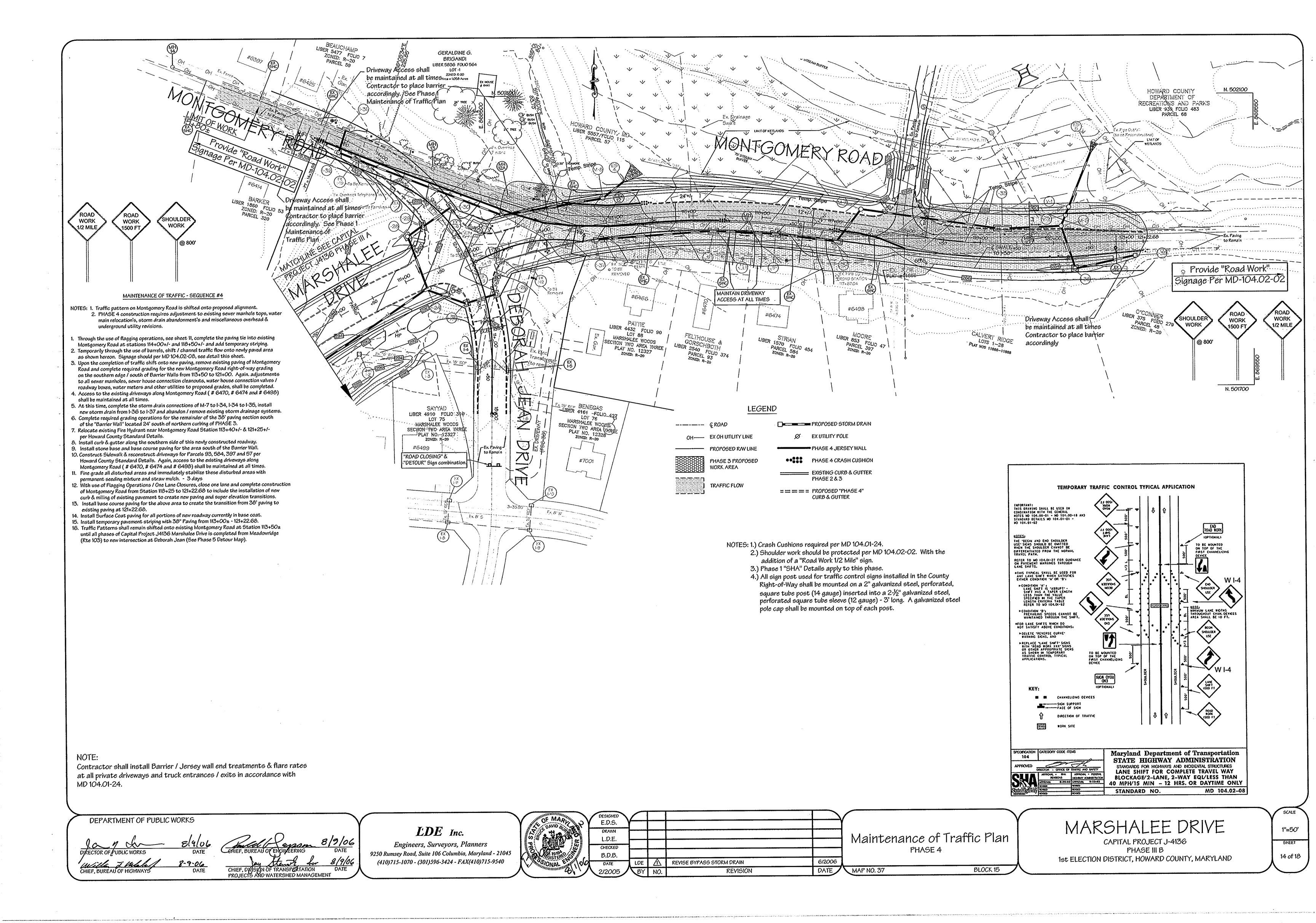


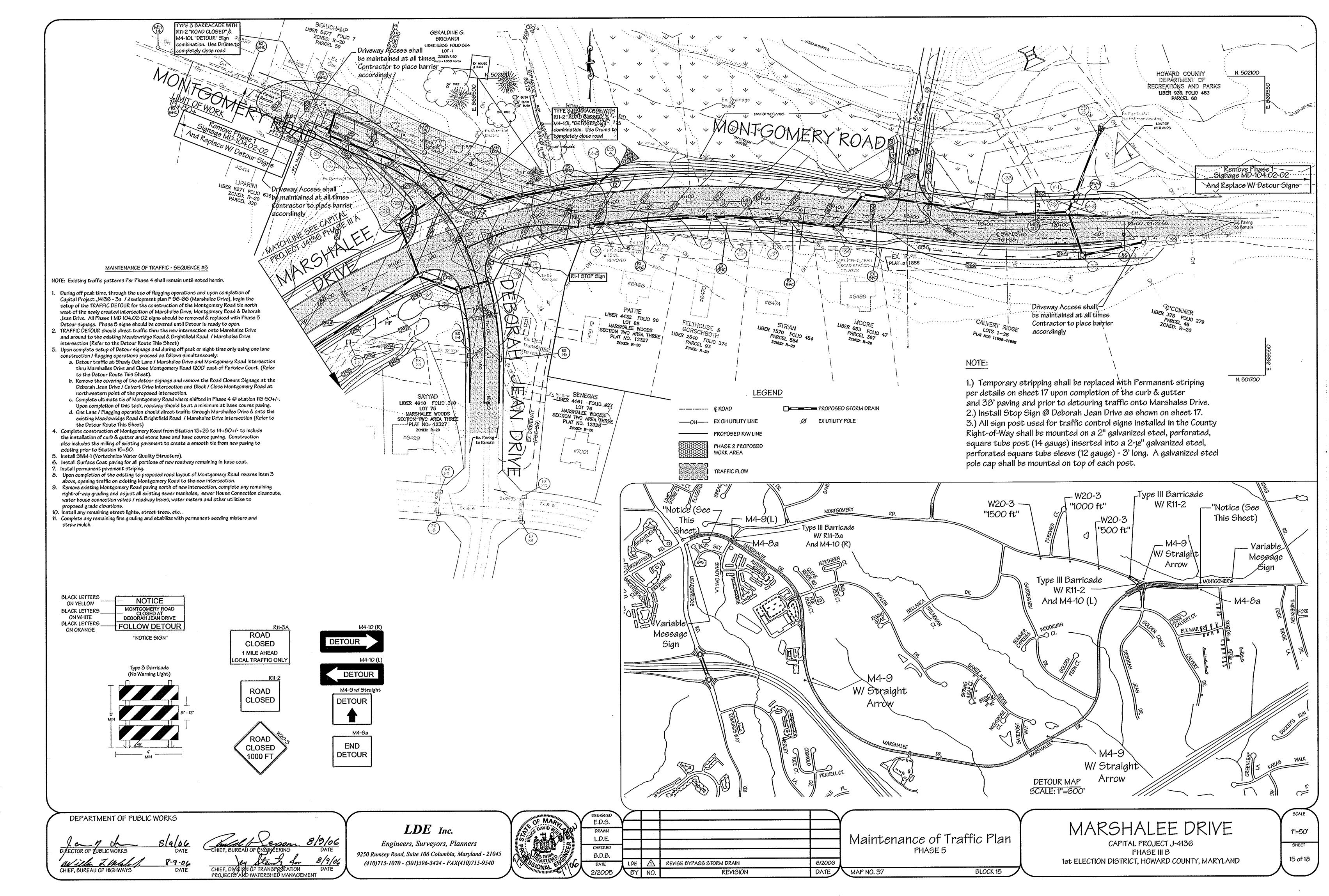
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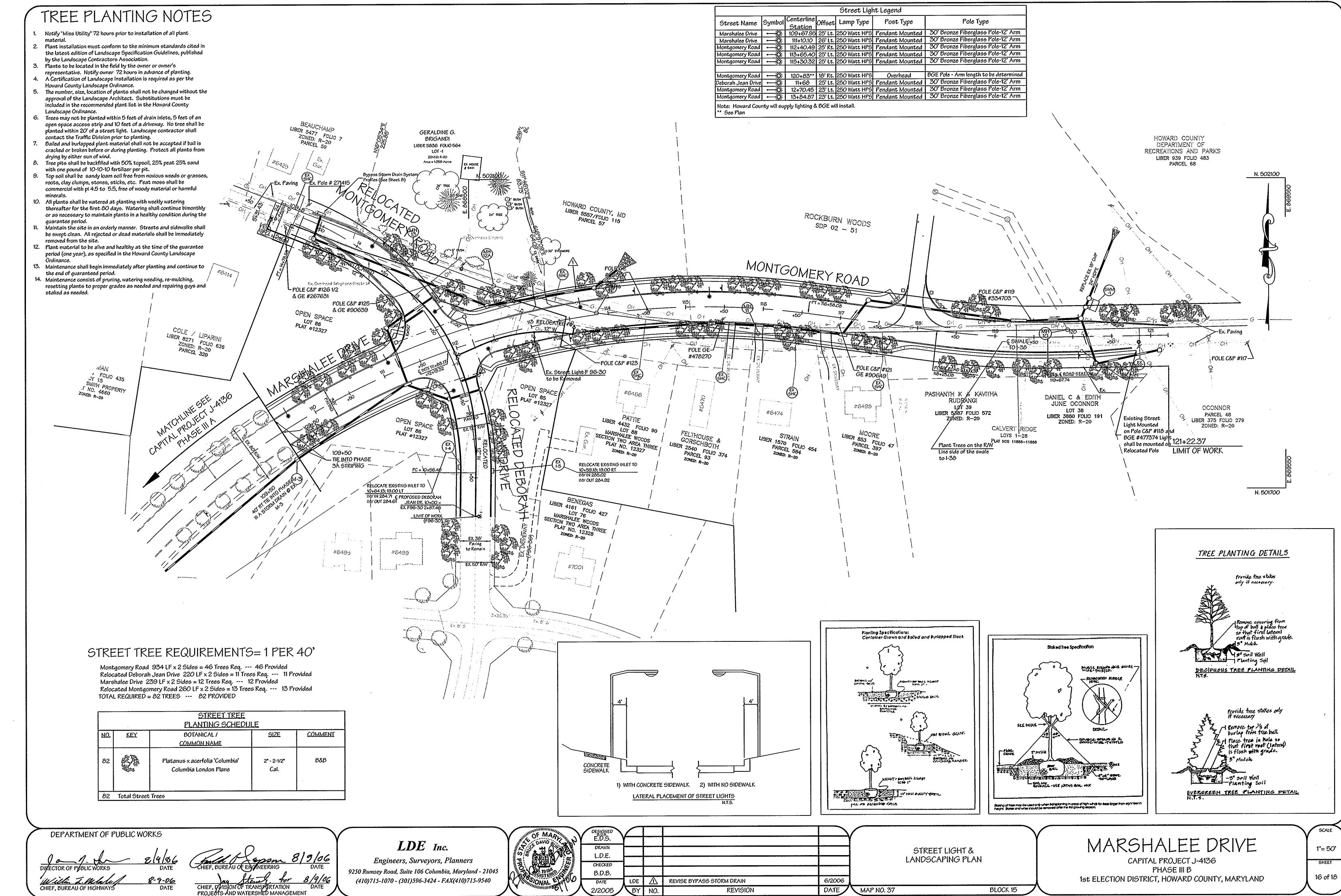
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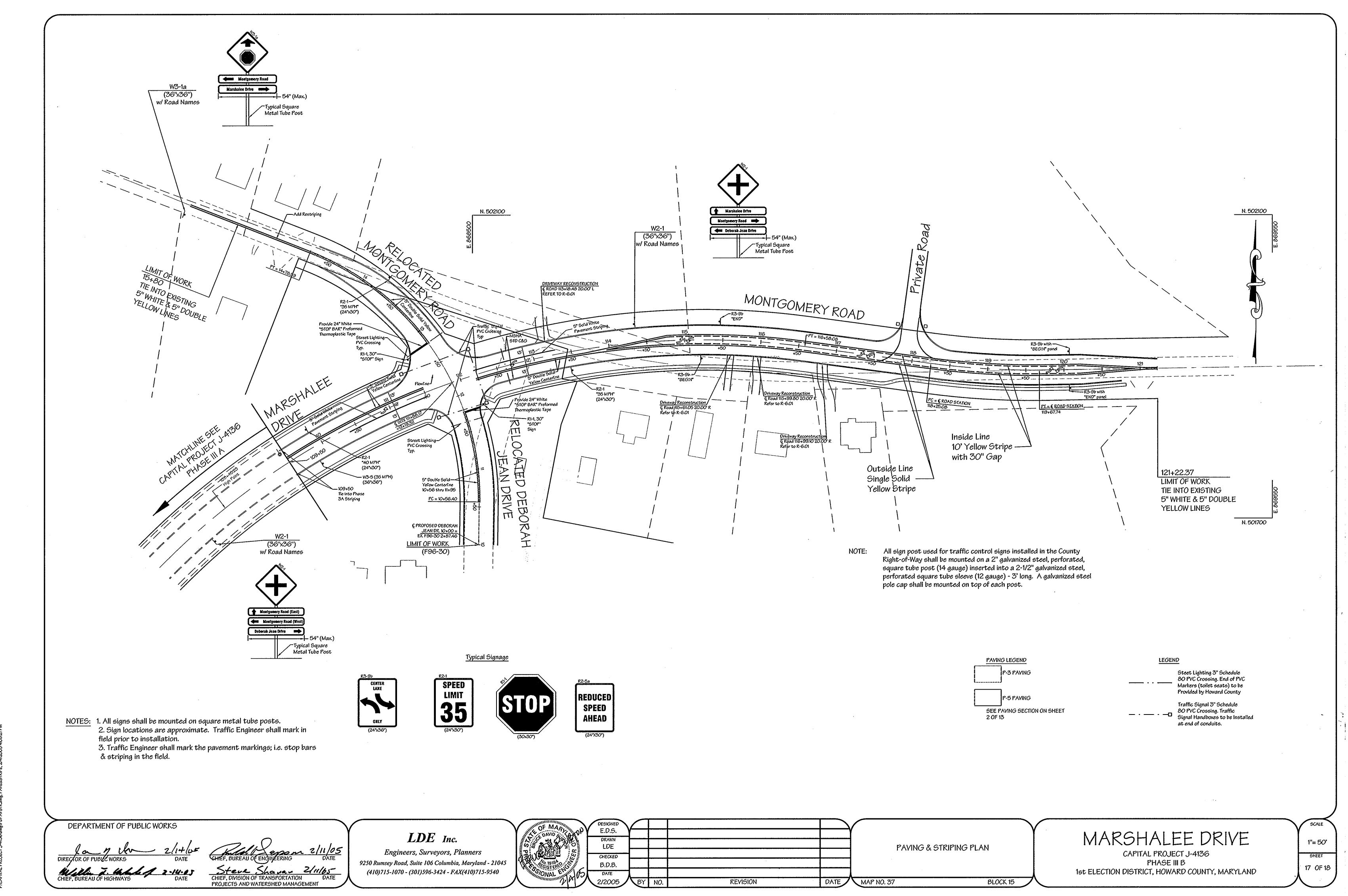
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CHIEF, BUREAU OF HIGHWAY

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| CHECKED B.D.B. | | · | | | |
| DATE | LDE | | REVISE BYPASS STORM DRAIN | 6/2006 | |
| 2/2005 | BY | NO. | REVISION | DATE | MAP NO. 37 |

BLOCK 15



KADAN BAAN DA HERE AND BANKA DEGINERALA SAN TERRITOR OF S

MAINTENANCE

The Vortechs System should be inspected at regular intervals and maintained when necessary to ensure optimum performance. The rate at which the System collects pollutants will depend more heavily on site activities than the size of the unit, e.g., unstable soils or heavy winter sanding will cause the grit chamber to fill more quickly but regular sweeping will slow accumulation.

Inspection is the key to effective maintenance and is easily performed. Vortechnics recommends ongoing quarterly inspections of the accumulated sediment. Pollutant deposition and transport may vary from year to year and quarterly inspections will help insure that Systems are cleaned out at the appropriate time. Inspections should be performed more often in the winter months in climates where sanding operations may lead to rapid accumulations, or in equipment wash down areas. It is very useful to keep a record of each inspection. A simple form for doing so is provided. (See Below)

The Vortechs System should be cleaned when inspection reveals that the sediment depth has accumulated to within six inches of the dry-weather water surface elevation. This determination can be made by taking 2 measurements with a stadia rod or similar measuring device; one measurement from the manhole opening to the top of the sediment pile and the other from the manhole opening to the water surface. The System should be cleaned out if the difference between the two measurements is six inches or less. Note: to avoid underestimating the volume of sediment in the chamber, the measuring device must be lowered to the top of the sediment pile carefully. Finer, silty particles at the top of the pile typically offer less resistance to the end of the rod than larger particles toward the bottom of the pile.

Maintaining the Vortechs System is easiest when there is no flow entering the System. For this reason, it is a good idea to schedule the cleanout during dry weather. Cleanout of the Vortechs System with a vacuum truck is generally the most effective and convenient method of excavating pollutants from the System. If such a truck is not available, a "clamshell" grab may be used, but it is difficult to remove all accumulated pollutants

In Yortechs installations where the risk of large petroleum spills is small, liquid contaminants may not accumulate as quickly as sediment. However, an oil or gasoline spill should be cleaned out immediately. Motor oil and other hydrocarbons that accumulate on a more routine basis should be removed when an appreciable layer has been captured. To remove these pollutants, it may be preferable to use absorbent pads since they are usually cheaper to dispose of than the oil water emulsion that may be created by vacuuming the oily layer. Trash can be netted out if you wish to separate it from the other pollutants.

Accumulated sediment is typically evacuated through the manhole over the grit chamber. Simply remove the cover and insert the vacuum hose into the grit chamber. As water is evacuated, the water level outside of the grit chamber will drop to the same level as the crest of the lower aperture of the grit chamber. It will not drop below this level due to the fact that the bottom and sides of the grit chamber are sealed to the tank floor and walls. This 'Water Lock' feature prevents water from migrating into the grit chamber, exposing the bottom of the baffle wall. Floating pollutants will decant into the grit chamber as the water level there is drawn down. This allows most floating material to be withdrawn from the same access point above the grit chamber.

If maintenance is not performed as recommended, sediment may accumulate outside the grit chamber. If this is the case, it may be necessary to pump out all chambers. It is a good idea to check for accumulation in all chambers during each maintenance event to prevent sediment build up there.

Manhole covers should be securely seated following cleaning activities, to ensure that surface runoff does not leak into the unit from above.

| Model: | | | AINTENANCE LOG Location: | | | | |
|--------|--------------------------------|----------------------------------|--------------------------|--------------------------|----------|--|--|
| Date | Water Depth to Sediment' | Floatable Layer Thickness² | Maintenance Performed | Maintenance Personnel | Comments | | |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |

- 1. The water depth to sediment is determined by taking two measurements with a stadia rod: one measurement from the manhole opening to the top of the sediment pile and the other from the manhole opening to the water surface. When the difference between the two measurements is six inches or less, the System should be cleaned out.
- 2. For optimum performance, the System should be cleaned out when the floating hydrocarbon layer accumulates to an appreciable thickness. In the event of a spill, the System should be cleaned immediately.

TAKE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE ACCORDING TO THESE

PLANS AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE

CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM

FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE

ENGINEER'S CERTIFICATE

ICERTIFY THAT THIS PLAN FOR EROSON AND SEDIMENT CARD. LEAST A PRACTICAL AND
WORKABLE PLAN BASED ON MY PERSONAL KNOWLES OF THIS BATTOCK STATES PLAN WAS
PREPARED IN ACCORDANCE WITH THE RECURRENCE OF THE HOUR POSSESSION SERVATION DISTRICT.

PERIODIC ON-SITE INSPECTIONS BY THE HOWARD SOIL CONSERVATION DISTRICT OR THEIR

AUTHORIZED AGENTS, AS ARE DEEMED NECESSARY."

Jan Toxpaunu

DEPARTMENT OF PUBLIC WORKS

SECTION 02721 STORMWATER TREATMENT SYSTEM

PART 1.00 GENERAL DESCRIPTION

A. Work included: The Contractor, and/or a manufacturer selected by the Contractor and approved by the Engineer, shall furnish all labor, materials, equipment and incidentals required and install ali precast concrete stormwater treatment systems and appurtenances in accordance with the Drawings and these specifications.

QUALITY CONTROL INSPECTION

A. The quality of materials, the process of manufacture, and the finished sections shall be subject to inspection by the Engineer. Such inspection may be made at the place of manufacture, or on the work site after delivery, or at both places, and the sections shall be subject to rejection at any time if material conditions fail to meet any of the specification requirements, even though sample sections may have been accepted as satisfactory at the place of manufacture. Sections rejected after delivery to the site shall be marked for identification and shall be removed from the site at once. All sections which have been damaged beyond repair during delivery will be rejected and, if already installed, shall be repaired to the Engineer's acceptance level, if permitted, or removed and replaced, entirely at the Contractor's expense.

- B. All sections shall be inspected for general appearance, dimensions, soundness, etc. The surface shall be dense, close textured and free of blisters, cracks, roughness and exposure of reinforcement.
- C. Imperfections may be repaired, subject to the acceptance of the Engineer, after demonstration by the manufacturer that strong and permanent repairs result. Repairs shall be carefully inspected before final acceptance. Cement mortar used for repairs shall have a minimum compressive strength of 4,000 psi (28 MPa) at the end of 7 days and 5,000 psi (34 MPa) at the end of 28 days when tested in 3 inch (76 mm) diameter by 6 inch (152 mm) long cylinders stored in the standard manner. Epoxy mortar may be utilized for repairs.

1.3 SUBMITTALS A. Shop Drawings

The Contractor, shall be provided with dimensional drawings and, when specified, utilize these drawings as the basis for preparation of shop drawings showing details for construction, reinforcing, joints and any cast-in-place appurtenances. Shop drawings shall be annotated to indicate all materials to be used and all applicable standards for materials, required tests of materials and design assumptions for structural analysis. Structural design calculations and shop drawings shall be certified by a Professional Engineer retained by the system manufacturer or Contractor and licensed in the state where the system is to be installed. Shop drawings shall be prepared at a scale of not less than 3/16-inches per foot (1:75). Six (6) hard copies of said shop drawings shall be submitted to the Engineer for review and approval.

B. Affidavit on patent infringement

The Contractor shall submit to the Engineer, prior to installation of the stormwater treatment system, an affidavit regarding patent infringement rights stating that any suit or claim against the Owner due to alleged infringement rights shall be defended by the Contractor who will bear all the costs, expenses and attorney's fees incurred thereof.

The following documentation must be submitted by the Contractor and approved by the Engineer prior to the manufacture and delivery of any materials. 1. Laboratory Data

The stormwater treatment system supplier shall provide documentation of Total Suspended Solids (TSS) removal efficiency from laboratory testing conducted on the supplier's full-scale system. The documentation shall include:

- a. TSS removal efficiency versus operating rate for the full operating range of the
- stormwater treatment system for a 50-micron particle size. b. TSS removal calculations for each system specified herein. The calculations must demonstrate that the system(s) is capable of achieving a net annual TSS removal efficiency as required by local regulations and as based upon a 50-micron particle size and the best available rainfall data for the project site location.

2. Field Test Data

The stormwater treatment system supplier shall provide documentation of TSS removal efficiency from field testing conducted on an installed system. The documentation shall be in accordance with the following:

the performance requirements for the system(s) specified herein.

- a. The testing and documentation shall have been conducted by an independent third
- b. The testing and documentation shall include at least 10 storms. c. The testing and documentation must show TSS removal results that meet or exceed
- 3. Manufacturing Experience

The stormwater treatment supplier shall provide evidence of at least 5 years of successful product design and use. The supplier shall provide an installation list of projects, model sizes installed and installation dates where the same type systems as specified herein have been designed and produced by the supplier.

THESE PLANS HAVE BEEN REVIEWED FOR THE HOWARD SO

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CONSERVATION DISTRICT AND MEET TECHNICAL

U.S. NATURAL RESOURCE

PROJECTS AND WATERSHED MANAGEMENT

REQUIREMENTS FOR SOIL EROSION AND SEDIMEN

2.1 MATERIALS AND DESIGN 2.1 A. Concrete for precast stormwater treatment systems shall conform to ASTM C 857 and C 858 and meet the following additional requirements:

1. The wall thickness shall not be less than 6 inches (152 mm) or as shown on the dimensional drawings. In all cases the wall thickness shall be no less than the minimum thickness necessary to sustain HS20-44 (MS18) loading

- requirements as determined by a Licensed Professional Engineer. 2. Sections shall have tongue and groove or ship-lap joints with a butyl mastic sealant conforming to ASTM C 990.
- 3. Cement shall be Type II Portland cement conforming to ASTM C 150. 4. All sections shall be cured by an approved method. Sections shall not be shipped until the concrete has attained a compressive strength of 4,000 psi (28 MPa) or until 5 days after fabrication and/or repair, whichever is the longer.
- 5. Pipe openings shall be sized to accept pipes of the specified size(s) and material(s), and shall be sealed by the Contractor with a hydraulic cement conforming to ASTM C 595M.
- B. Internal aluminum plate components shall be aluminum alloy 5052-H32 in accordance with ASTM B 209.
- C. Sealant to be utilized at the base of the swirl chamber shall be 60 durometer extruded nitrile butadiene rubber (Buna N) and shall be provided to the concrete precaster for installation.

THESE PLANS FOR SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL

HOWARD S.C.D.

E. Casting for manhole frames and covers shall be in accordance with ASTM A48, CL.30B and AASHTO MIO5. The manhole frame and cover shall be equivalent to Campbell Foundry Pattern #1009A or #10120 custom cast with the Vortechnics logo and words "YortechnicsTM Stormwater Treatment System".

D. Brick or masonry used to build the manhole frame to grade shall conform to

ASTM C 32 or ASTM C 139 and shall be installed in conformance with all local

F. A bitumen sealant in conformance with ASTM C 990 shall be utilized in the sealing of the joint between the swirl chamber and the vault at the long wall tangent points. The butyl material shall be 3/4 inch thick by 3/4 inch wide.

2.2 PERFORMANCE

Each storm water treatment system shall adhere to the following performance specifications at the design treatment capacities, as listed below:

Table 2.2

| Vortechs Model | Design Treatment Capacity (cfs)/(I\s) | Sediment Storage (yd³)/(m³) |
|-------------------|------------------------------------------------|-----------------------------------|
| 1000 | 0-1.6 (0-45) | 0.7 (0.54) |
| 2000 | 1.6-2.8 (45-80) | 1.2 (0.91) |
| 3000 | 2.8-4.5 (80-125) | 1.8 (1.38) |
| 4000 | 4.5-6.0 (125-175) | 2.4 (1.84) |
| Q= 5000 | 6.0-8.5 (175-240) | 3.2 (2.45) |
| 7000 | 8.5-11.0 (240-315) | 4.0 (3.06) |
| 9000 | 11.0-14.0 (315-400) | 4.8 (3.67) |
| 11000 | 14.0-17.5 (400-495) | 5.6 (4.28) |
| 16000 | 17.5-25.0 (495-710) | 7.1 (5.43) |

Each stormwater treatment system shall include a circular aluminum "swirl chamber" (or "grit chamber") with a tangential inlet to induce a swirling flow pattern that will accumulate and store settleable solids in a manner and a location that will prevent resuspension of previously captured particulates.

Each stormwater treatment system shall be of a hydraulic design that includes flow controls designed and certified by a professional engineer using accepted principles of fluid mechanics that raise the water surface inside the tank to a pre-determined level in order to prevent the re-entrainment of trapped floating contaminants.

Each stormwater treatment system shall be capable of removing 85% of the net annual Total Suspended Solids (TSS) load based on a 50-micron particle size. Annual TSS removal efficiency models shall be based on documented removal efficiency performance from full scale laboratory tests. Annual TSS removal efficiency models shall only be considered valid if they are corroborated by independent third party field testing. Said field testing shall include influent and effluent composite samples from a minimum of ten storms at one location. Individual stormwater treatment systems shall have the Design Treatment Capacity listed in Table 2.2, and shall not re-suspend trapped sediments or re-entrain floating contaminants at flow rates up to and including the specified Design Treatment Capacity.

Individual stormwater treatment systems shall have usable sediment storage capacity of not less than the corresponding volume listed in Table 2.2. The systems shall be designed such that the pump-out volume is less than 1/2 of the total system volume. The systems shall be designed to not allow surcharge of the upstream piping network during dry weather conditions.

A water-lock feature shall be incorporated into the design of the stormwater treatment system to prevent the introduction of trapped oil and floatable contaminants to the downstream piping during routine maintenance and to ensure that no oil escapes the system during the ensuing rain event. Direct access shall be provided to the sediment and floatable contaminant storage chambers to facilitate ement shaßmaintenance. There shall be no appurtenances or restrictions within these chambers.

The stormwater treatment system manufacturer shall furnish documentation which supports all product performance claims and features, storage capacities and maintenance requirements.

Stormwater treatment systems shall be completely housed within one rectangular

2.3 MANUFACTURER

Each stormwater treatment system shall be of a type that has been installed and used successfully for a minimum of 5 years. The manufacturer of said system shall have been regularly engaged in the engineering design and production of systems for the physical treatment of stormwater runoff during the aforementioned period.

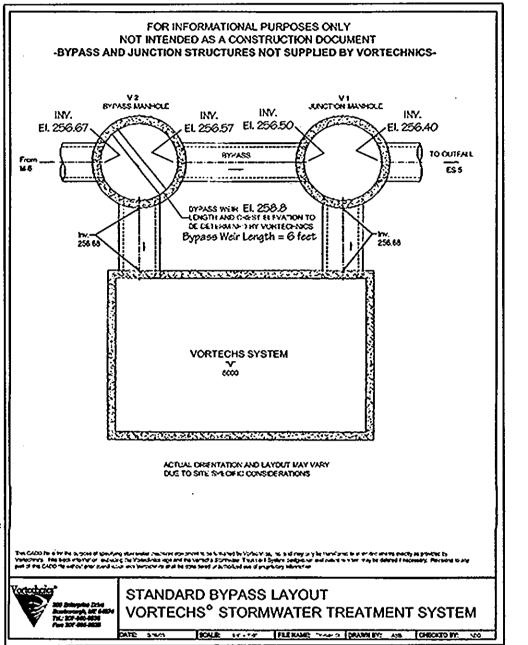
Each stormwater treatment system shall be a Vortechs System as manufactured by Vortechnics, Inc., 200 Enterprise Drive, Scarborough, Maine 04074, phone: 207-885-9830, fax: 207-885-9825; and as protected under U.S. Patent #5,759,415 or approved

PART 3.00 EXECUTION

- INSTALLATION A. Each Stormwater Treatment System shall be constructed according to the sizes shown on the Drawings and as specified herein. Install at elevations and locations shown on the Drawings or as otherwise directed by the Engineer.
- B. Place the precast base unit on a granular subbase of minimum thickness of six inches (152 mm) after compaction or of greater thickness and compaction if specified elsewhere. The granular subbase shall be checked for level prior to setting and the precast base section of the trap shall be checked for level at all four corners after it is set. If the slope from any corner to any other corner exceeds 0.5% the base section shall be removed and the granular subbase material re-leveled.
- C. Prior to setting subsequent sections place bitumen sealant in conformance with ASTM C 990-91 along the construction joint in the section that is already in place.
- D. After setting the base and wall or riser sections, prepare to install the swirl chamber. Place the 3/4-inch thick by 3/4-inch wide butyl mastic seal vertically on the outside of the swirl chamber starting one inch, (1") above the bottom of the swirl chamber and continuing to a height equal to the elevation of the bottom of the upper aperture of the swirl chamber. The butyl mastic seal should abut the downstream side of the pre-drilled mounting holes that attach the swirl chamber to the long walls of the concrete vault. Next, install the extruded Buna N seal on the bottom edge of the 180 degree downstream section of the swirl chamber by first applying a bead of Sikaflex-1a polyurethane elastomeric sealant into the extruded slot then slide the seal onto the swirl chamber. The extruded seal should extend 3-inches upstream of the mounting holes, toward the inlet end of the vault. Set the swirl chamber into position and keep the seal approximately 1/2inch above the floor of the concrete vault. Apply a continuous bead of Sikaflex-1a sealant under the cupped bottom of the seal. Set the circular swirl chamber on the floor of the vault and anchor it by bolting the swirl chamber to the side walls of the concrete vault at the three (3) tangent points and at the inlet tab

using HIL TI brand stainless steel drop-in wedge anchors or equivalent 3/8-inch diameter by 2-3/4 inch minimum length at heights of approximately three inches (3") off the floor and at fifteen inch (15") intervals to approximately the same height of the butyl mastic sealant (at locations of pre-drilled holes in aluminum components). Apply a continuous bead of Sikaflex-la sealant to the intersection of the inside bottom edge of the extruded seat and the vault floor.

- E. Prior to setting the precast roof section, bitumen sealant equal to ASTM C 990 shall be placed along the top of the oil baffle wall (Baffle A), using more than one layer of mastic if necessary, to a thickness at least 1-inch greater than the nominal gap between the top of the baffle and the roof section. The nominal gap shall be determined either by field measurement or the shop drawings. After placement of the roof section has compressed the butyl mastic sealant in the gap, finish sealing the gap with an approved non-shrink grout on both sides of the gap using the butyl mastic as a backing material to which to apply the grout. Apply non-shrink grout or Sikaflex-1a sealant to each end of the oil baffle wall and the flow control wall at the upstream intersection with the side walls of the concrete vault. Do not seal the top of the flow control wall (Baffle B) unless specified on the shop drawings to do so.
- F. After setting the precast roof section of the stormwater treatment system, set precast concrete manhole riser sections, to the height required to bring the cast iron manhole covers to grade. so that the sections are vertical and in true alignment with a 1/4 inch (6 mm) maximum tolerance allowed. Backfill in a careful manner, bringing the fill up in 6-inch (152 mm) lifts on all sides. If leaks appear, clean the inside joints and caulk with lead wool to the satisfaction of the Engineer. Precast sections shall be set in a manner that will result in a watertight joint. In all instances, installation of Stormwater Treatment Systems shall conform to ASTM specification C 891 "Standard Practice for Installation of Underground Precast Utility Structures".
- G. Holes made in the concrete sections for handling or other purposes shall be plugged with a nonshrink grout or by using grout in combination with concrete
- H. Where holes must be cut in the precast sections to accommodate pipes, do all cutting before setting the sections in place to prevent any subsequent jarring which may loosen the mortar joints. The Contractor shall make all pipe connections.



Vortechs™ Stormwater Treatment Systems

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MAP NO. 37

DATE

| | | chs Orifice Cd = 0.56 A (ħ²) = 0.33 Elevation (ħ) = 256 | Weir Cres | ortechs Welr Cd = 3.37 It Length (ft) = 1.17 Elevation (ft) = 258.18 | Bypass Weir Cd = 3.3 V/eir Crest Length (ft) = 6 Crest Elevation (ft) = 258.8 | | |
|---------------|----------------------------------------------------------------------|------------------------------------------------------------------|-------------------------------|-------------------------------------------------------------------------------|---------------------------------------------------------------------------------|------------|--|
| Head | | Elevation | Orifice Flow | Welr Flow | Bypass Flow | Total Flow | |
| (ft) | | (ft) | (cfs) | (cfs) | (cfs) | (cfs) | |
| 0.00 | | 256.68 | 0.00 | 0.00 | 0.00 | 0.00 | |
| 0.20 | | 256.88 | 0.20 | 0.00 | 0.00 | 0.20 | |
| 0.40 | +- | 257.08 | 0.57 | 0.00 | 0.00 | 0.57 | |
| 0.60 | \neg | 257.28 | 0.91 | 0.00 | 0.00 | 0.91 | |
| 0.80 | | 257.48 | 1.12 | 0.00 | 0.00 | 1.12 | |
| 1.00 | | 257.68 | 1.30 | 0.00 | 0.00 | 1.30 | |
| 1.20 | | 257.88 | 1.46 | 0.00 | 0.00 | 1.46 | |
| 1.40 | | 258.08 | 1.60 | 0.00 | 0.00 | 1.60 | |
| 1.60 | 1 | 258.28 | 1.72 | 0.13 | 0.00 | 1.85 | |
| 1.80 | 1 | 258.48 | 1.84 | 0.66 | 0.00 | 2.50 | |
| 2.00 | | 258.68 | 1.96 | 1.41 | 0.00 | 3.36 | |
| 2.12 | | 258.80 | 2.02 | 1.92 | 0.00 | 3.94 | |
| 2.20 | _ | 258.88 | 2.06 | 2.32 | 0.47 | 4.86 | |
| 2.40 | | 259.08 | 2.16 | 3.38 | 2.98 | 8.53 | |
| 2.60 | | 259.28 | 2.26 | 4.57 | 6.65 | 13.47 | |
| 2.81 | | 259.50 | 2.36 | 5.95 | 11.49 | 19.60 | |
| 1,01 | | 259.61 | 1.47 | 3.98 | 14.35 | 19.80 | |
| | nn show i by: D | s 10-yr flow with T | Weiev = 258.6 ft 2/24/2004 | (full outlet pipe) Checked by: | | | |
| Chevasion (R) | 260.0 259.5 259.0 258.5 258.0 257.5 257.0 258.5 | | | /ortechs™ System ge Discharge Curve | Dypass Crest Week Crest Onface Crest | | |
| | 0 | .0 6 | 0 1 | 0 0 15.0 Discharge (ds) | 20.0 | 25.0 | |

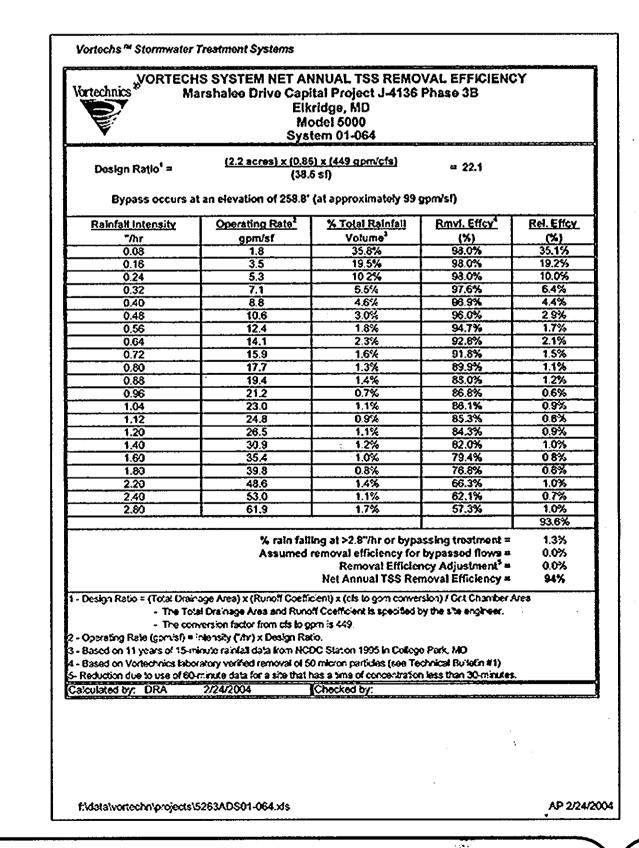
FLOW CALCULATIONS

STORMWATER TREATMENT SYSTEM VORTECHS*MODEL 5000 PROPRIETARY INFORMATION - NOT TO BE USED FOR CONSTRUCTION PURPOSES E: 09/09/02 SCALE: UF + FO' FRENAME: STOOK DRAWN BY: DMF CHECKED BY: NOS ACTUAL FINAL DESIGN / SHOP DRAWINGS SHALL BE PREPARED BY VORTECHNICS - Contractor shall contact VORTECHNICS to order SWM-1 (Model 5000) - Shop drawings shall be supplied to LDE, Inc. for review and approval prior to fabrication & delivery.

PLAN VIEW B - B

The state of the s

bypess configuration require an upstream diversion structure that shall be detailed by the Consulin Engineer with elevation and were width data provided by Vortechnic



LDE Inc.

Engineers, Surveyors, Planners

9250 Rumsey Road, Suite 106 Columbia, Maryland - 21045 (410)715-1070 - (301)596-3424 - FAX(410)715-9540



LDE CHECKED B.D.B. REVISION

VORTECHNICS - Details

AP 2/24/2004

BLOCK 15

MARSHALEE DRIVE

CAPITAL PROJECT J-4136 PHASE III B 1st ELECTION DISTRICT, HOWARD COUNTY, MARYLAND

As Shown

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