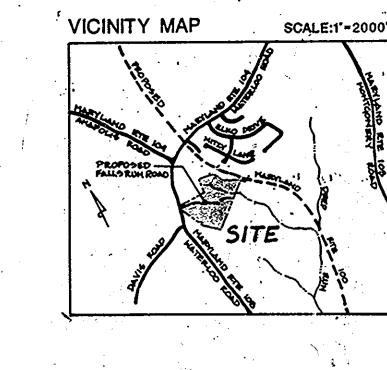
FOR





DEVELOPER/OWNER

SUMMIT PROPERTIES 203 KEY HIGHWAY BALTIMORE, MARYLAND 21230

SITE ANALYSIS

- K. ALLOWABLE UNIT DENSITY 8.0 UNITS/ACRE
- ACTUAL UNIT DENSITY: 7.7 UNITS /ACRE L. AREA OF PROPOSED ROADS - 5.39 ACRE
- CURRENTLY, NO UTILITY STRUCTURES EXIST ON SITE. WATER AND SEWER SERVICE WILL BE PUBLIC. ALL RETAINING WALLS SHALL BE LESS THAN 3 FEET IN HEIGHT, MEASURED FROM BOTTOM OF WALL TO TOP OF WALL, AT THE HIGHEST POINT, UNLESS OTHERWISE NOTED. SEE SHEETS 2 AND 3 OF 33

- REPER TO SHEETS 2 AND 3 OF 33 FOR EXISTING TREES TO

- S-89-34 SKETCH PLAN.

ENGINEER'S CERTIFICATE

I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND EDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABL LAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVA-

DEVELOPER'S CERTIFICATE

"I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN OF DEVELOPMENT AND PLAN FOR EROSION AND SEDIMENT CONTROL AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CEPTIFICATE OF ATTENDANCE AT A DEPARTMENT OF NATURAL RESOURCES APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE, PERIODIC ONSITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT OR THEIR AUTHORIZED AGENTS, AS ARE

- SITE LANDSCAPING PLANS

- PLOODPLAIN: LIMITS PROVIDED BY STV/LYON, 21 GOVERNOR'S COURT, BALTIMORE, MARYLAND 21207. MYLAR BASE SHEET WITH LIMITS DELINEATION RECEIVED BY LANDDESIGN, INCORPORATED ON
- TRASH COLLECTION SERVICE TO BE PROVIDED BY HOWARD COUNTY

1.0521 ACRES

1.1376 x 8 = 9 202 UNITS

- ALL SIDEWALKS ALONG PARKING TO BE 6"-0" WIDE, ALL, SIDEWALKS UP TO HANDICAP UNIT

- ALL CONSTRUCTION TO BE DONE IN ACCORDANCE WITH STANDARDS S
 FORTH BY CORPS OF ENGINEERS (SECTION 404) AND STA
 DEPARTMENT OF THE ENVIRONMENT (SECTION 401).

 CORPS OF ENGINEERS PERMIT 88-1967-4

 NARYLAND WATER OVALITY CERTIFICATION 89-MO-0577

SITE DATA SHEET

SHEET INDEX

- 2 50 SCALE GRADING PLAN
- 3 50 SCALE GRADING PLAN
- 4 50 SCALE DIMENSIONING PLAN
- 5 50 SCALE DIMENSIONING PLAN
- 6 50 SCALE LIGHTING AND REQUIRED PLANTING RLAN
- 7 50 SCALE LIGHTING AND REQUIRED PLANTING PLAN
- 8 PLANTING NOTES, DETAILS AND PLANT LIST
- 9 SITE DETAILS
- 10 SITE DETAILS
- 11 SITE DETAILS
- 12 SITE DETAILS
- 13 WATER & SANITARY SEWER PLAN
- 14 WATER & SANITARY SEWER PLAN
- 15 WATER PROFILES
- 16 WATER PROFILES

REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION

DISTRICT AND MEETS TECHNICAL REQUIREMENTS

THIS DEVELOPMENT IS APPROVED FOR

EROSION AND SEDIMENT CONTROL BY THE HOWARD

SOIL CONSERVATION DISTRICT

- 17 WATER PROFILES
- 18 SANITARY PROFILES
- 19 SANITARY PROFILES
- 20 SEDIMENT CONTROL PLAN
- 21 SEDIMENT CONTROL PLAN
- 22 SEDIMENT CONTROL DETAILS
- 23 ROAD PROFILE COTONEASTER DRIVE
- 24 ROAD PROFILE OAKTON LANE
- 25 STORM DRAIN PLAN & DRAINAGE AREA MAP
- 26 STORM DRAIN PLAN & DRAINAGE AREA MAP
- 27 STORM DRAIN PROFILES
- 28 STORM DRAIN PROFILES
- 29 CULVERT PIPE ELEVATIONS & DETAILS
- 30 STORM WATER MANAGEMENT FACILITIES NOS. 1 & 2

APPROVED DEPARTMENT OF PUBLIC WORKS
FOR PUBLIC WATER AND SEWER AND STORM

SECTION/AREA

1-8-90

PARCEL

DRAINAGE SYSTEMS AND ROADS

- WATER QUALITY INLETS
- 32 S.W.M. NOTES AND BORING LOGS

Wissim & Rely

SUBDIVISION NAME

CHIEF , BUREAU OF ENGINEERING

SITE DETAILS

PARKING TABULATION

LIMITS DED RITH DING

| UNITS PE | ER BUILDING | _ | • |
|--------------------|----------------------------|-----------------------|-----------------------|
| BUILDING NUMBER | NUMBER OF REGULAR UNITS | NUMBER OF HC UNITS | TYPE OF HC UNITS |
| 1 | 14 | 0 | |
| 2 | 16 | . 0 | |
| 3 . | 0 | 0 | |
| 4 | 16 | 0 | |
| 5 | 16 | 0 | |
| 6 | 14 | 2 | (2) 1 BR |
| 7 | 14 | 2 | (1) 2 BR; (1) 1 BR |
| 8 | 16 | 0 | (1) |
| 9 | 16 | 0 | • |
| 10 | 16 | 0 | , . . |
| - 11 | 16 | 0 | والمرابعة المستعدان |
| · 12 | 12 | 2 | (2) 2 BR - |
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| 14 | 14 | 2 | (1) 2 BR; |
| / . · | | | (1) 1 BR |

OWNER / DEVELOPER: SUMMIT PROPERTIES 203 KEY HIGHWAY BALTIMORE, MARYLAND 21230 (301) 234-0070

ENGINEER: STV/LYON: ASSOCIATES 21 GOVERNOR'S COURT BALTIMORE, MARYLAND 21207 (301) 944-9112

Fand Donien

PROJECT NO: 88074 REVISIONS: REVISED 9-5-89 REVISED 5-1-80

■ 1216 Prince Street, Alexandria, VA 22314

and Design

Landscape Architecture Land Planning Urban Désign

SITE DATA SHEET

5DP 87-254

APPROVED HOWARD COUNTY HEALTH DEPARTMENT

FOR PUBLIC WATER AND SEWERAGE

PPROVED HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

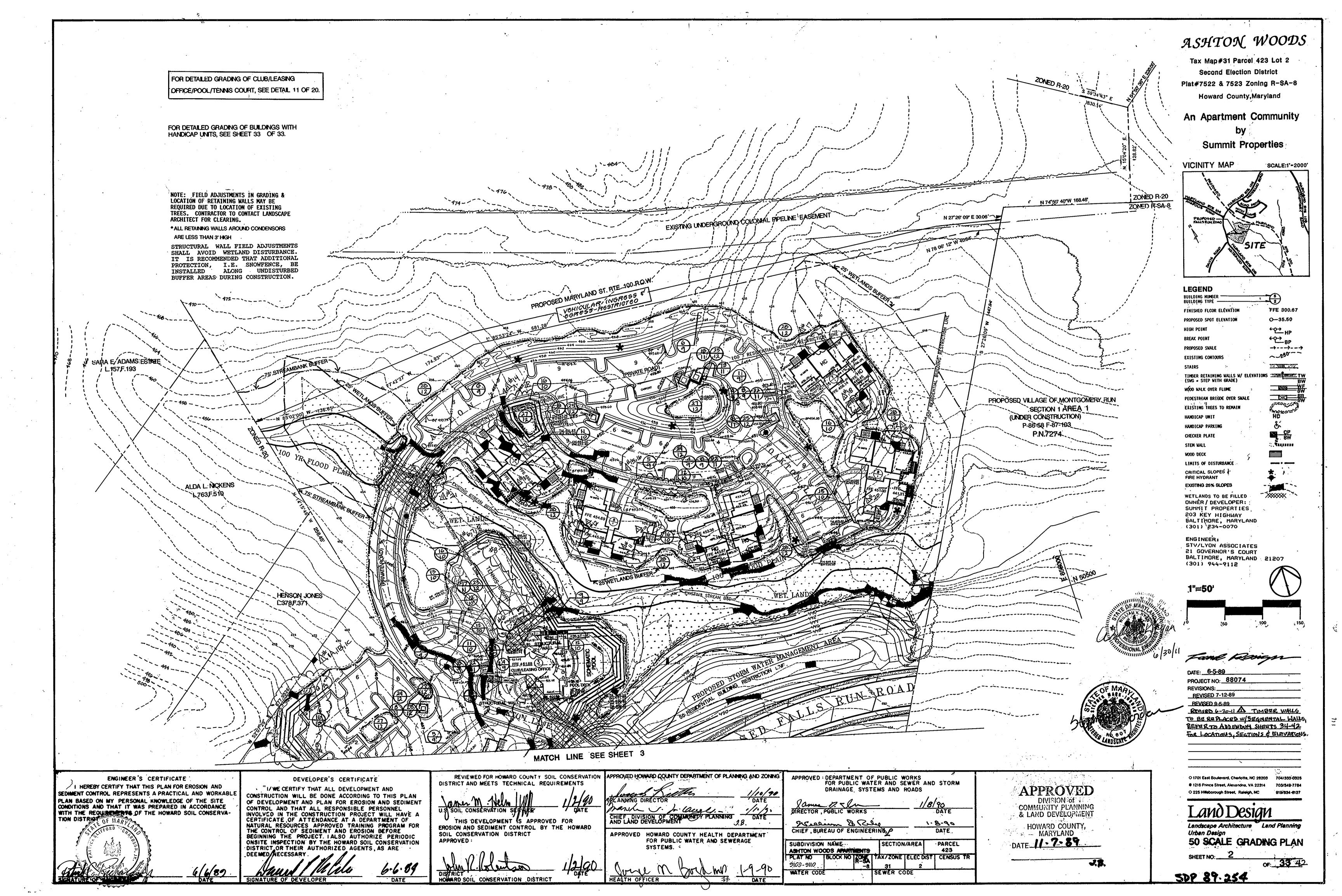
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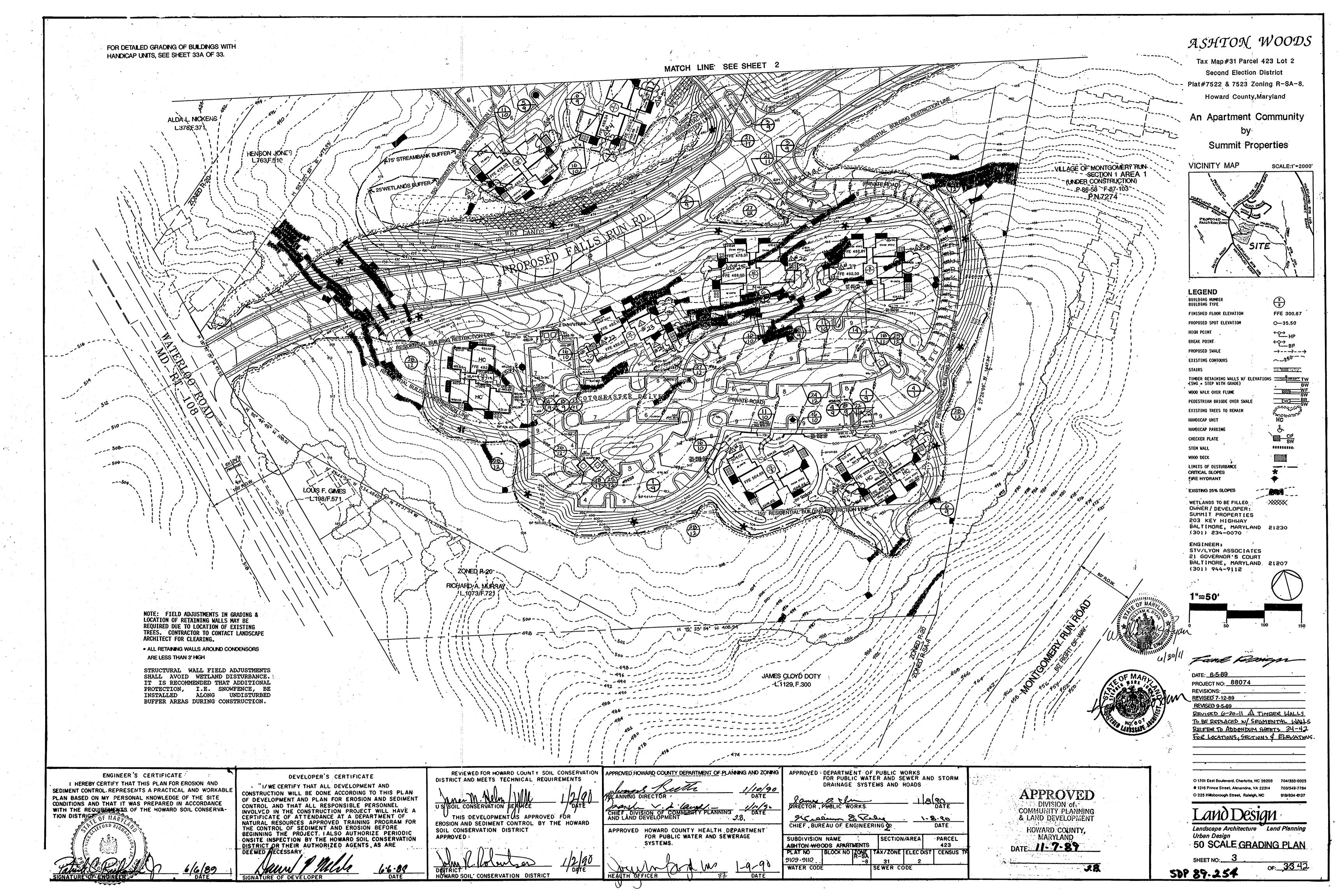
ASHTON WOODS APARTMENTS EWER CODE HOWARD COUNTY, MARYLAND

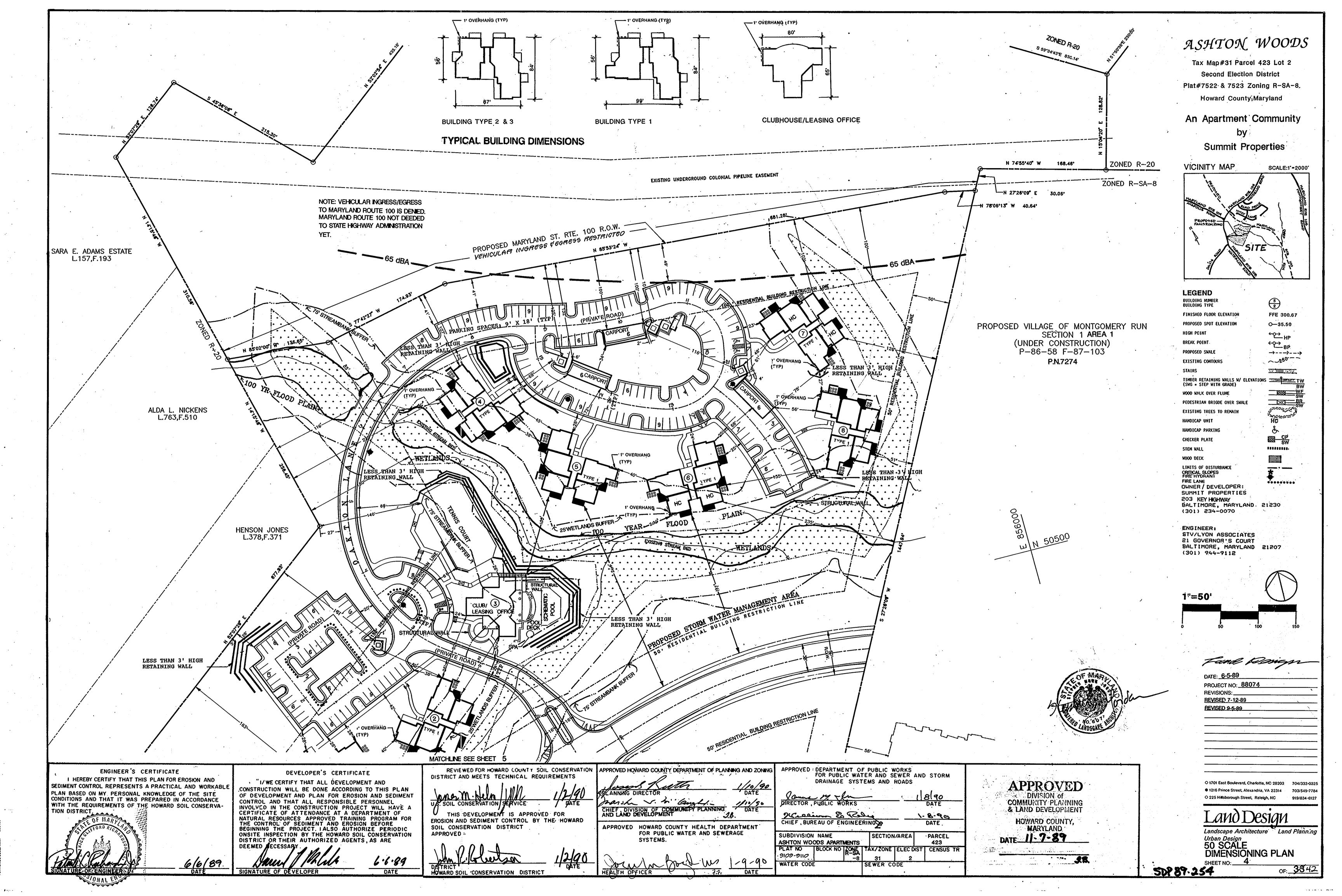
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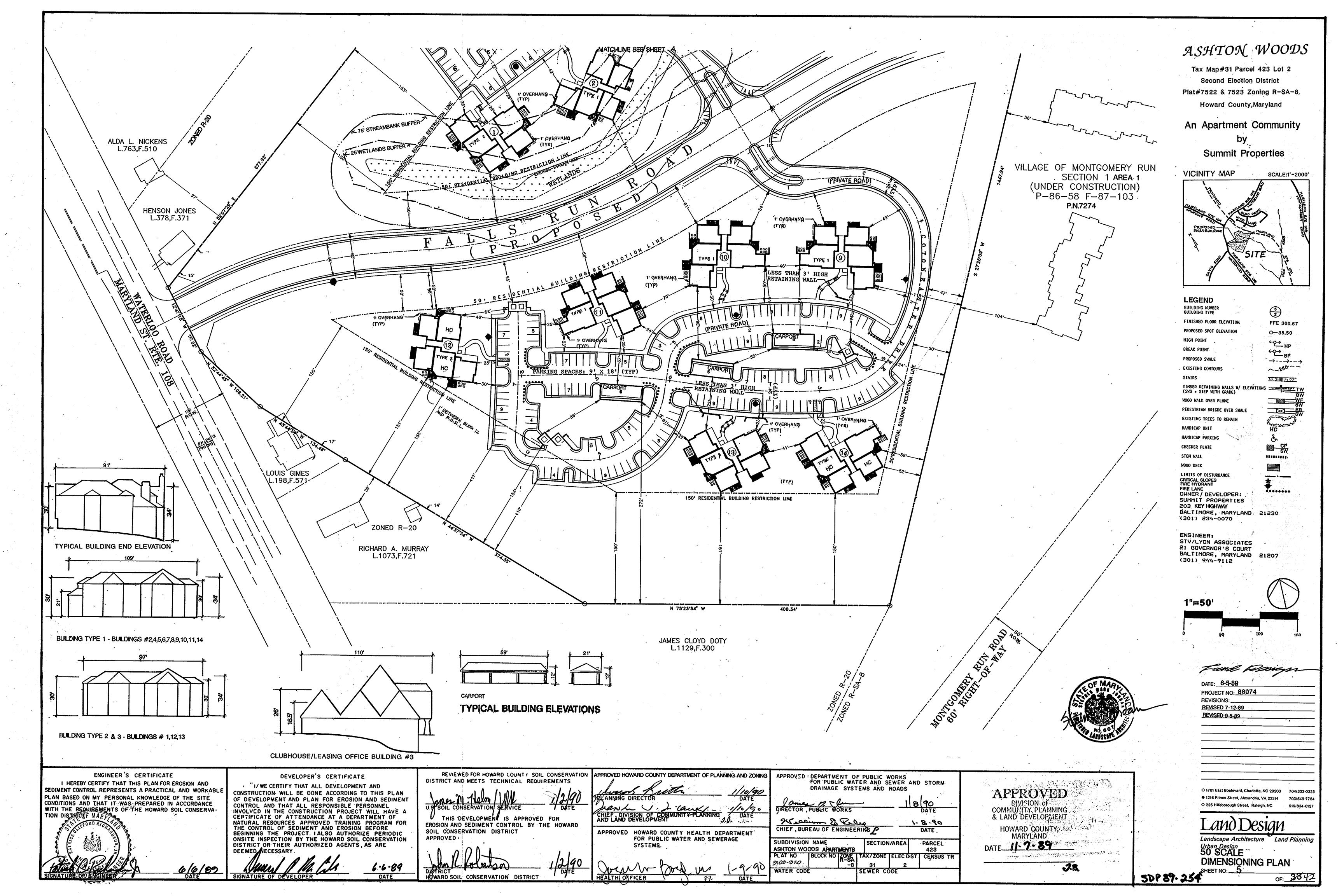
COMMUNITY PLANNING

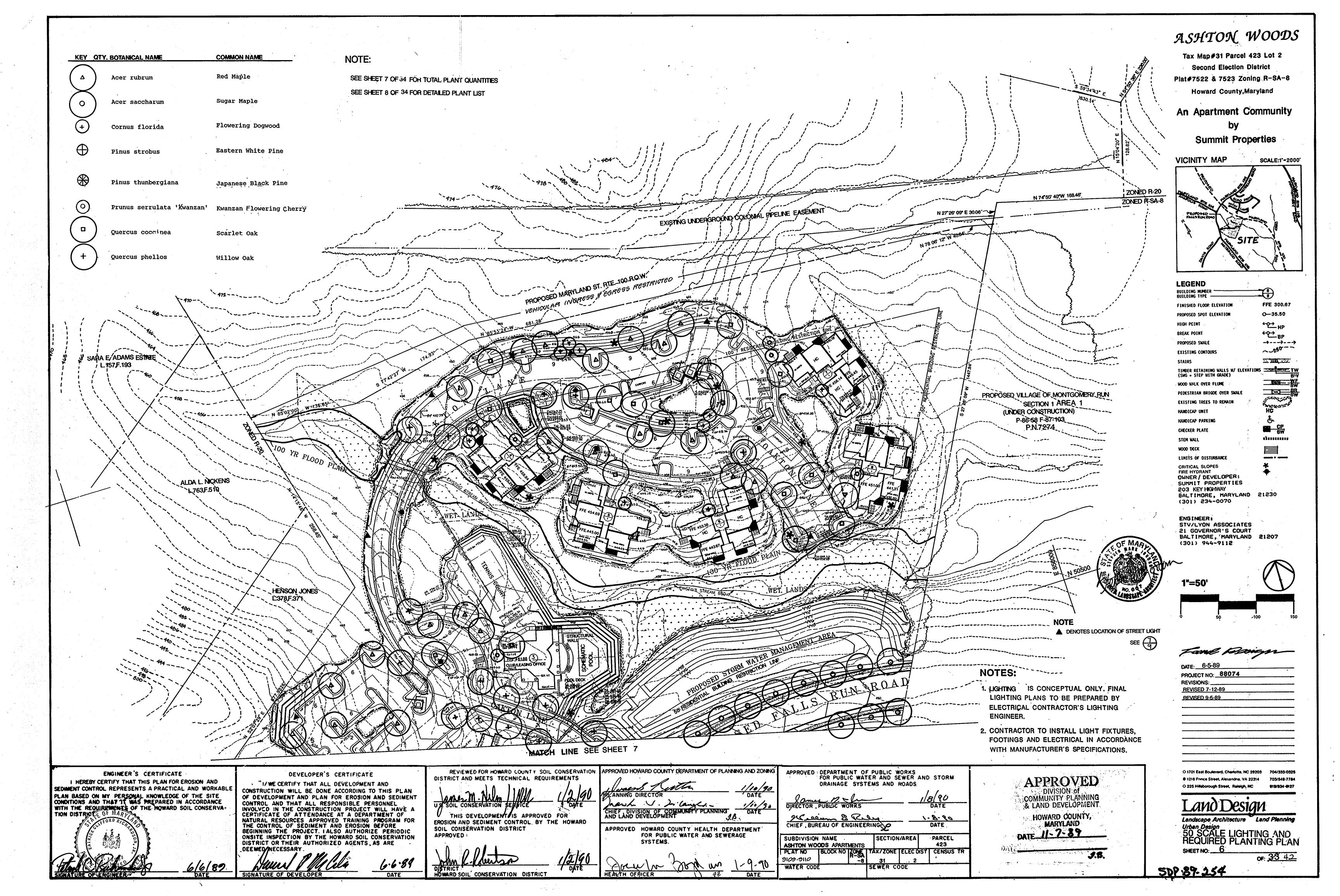
& LAND DEVELOPMENT

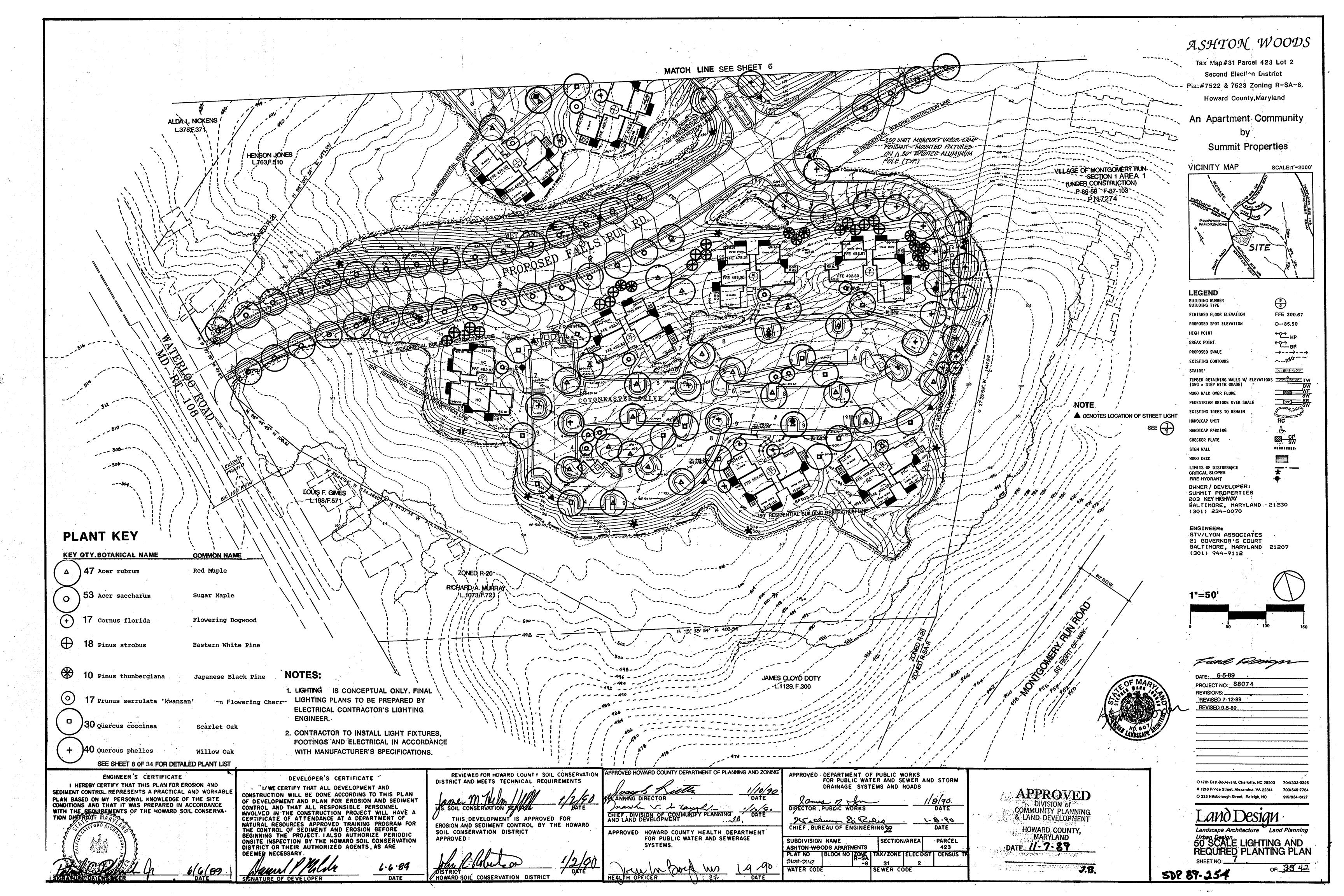


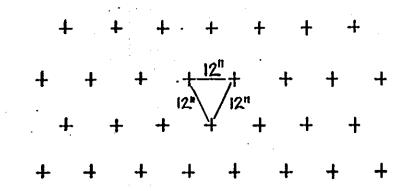












HOTE: EXAMPLE ILLUSTRATES TRIANGULAR SPACING 12" O.C. OTHER SPACING PISTANCES TO BE IMPLEMENTED USING SAME PROPORTION

TRIANGULAR SPACING PLAN (SHRUBS AND GROUND COVER)

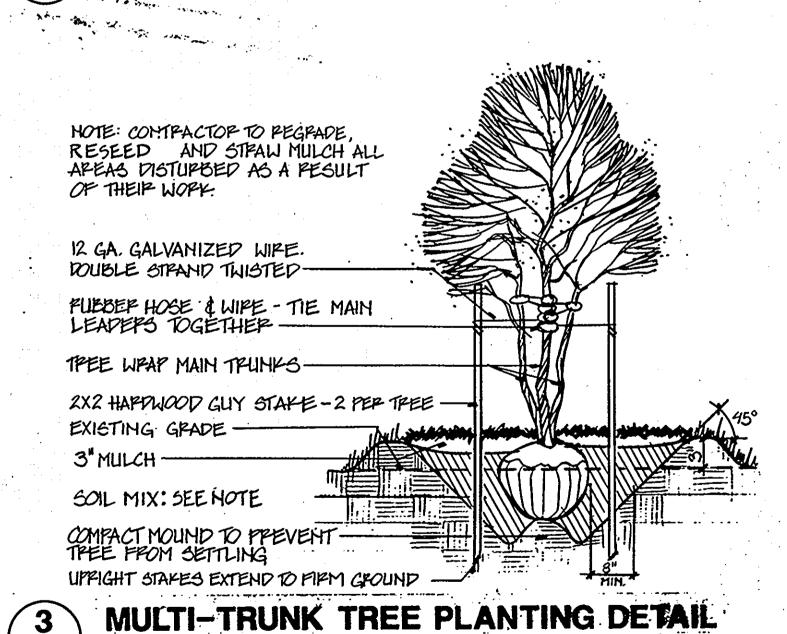
NOTE: PLANT SPACING VAPIES - (SEE PLANT LIST PRUNE ALL BROKEN, DISEASED, AND WEAK BRANCHES

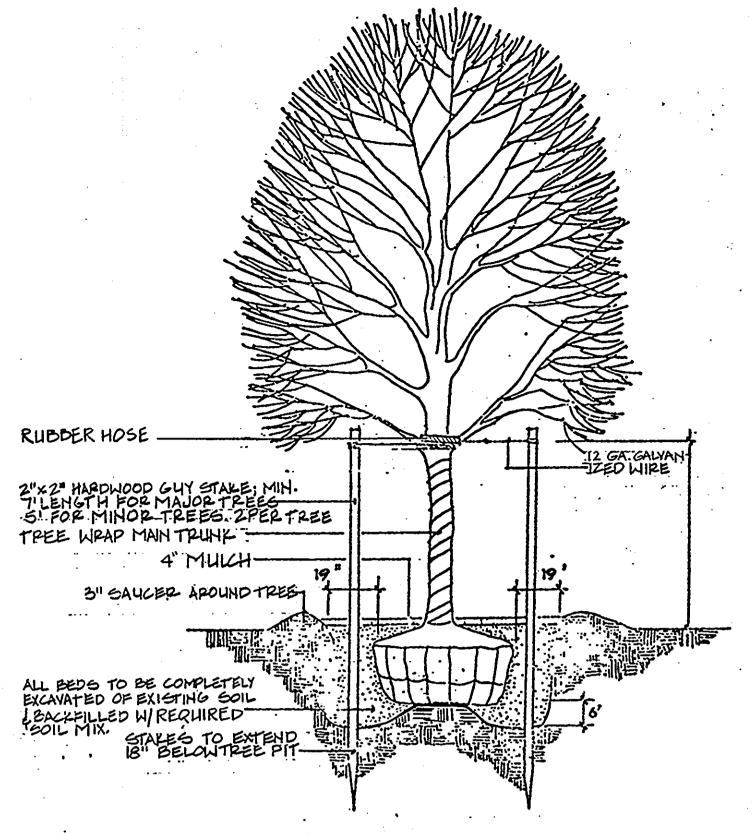
8

ALL SHPUB BERS TO BE COMPLETELY EXCAVATED OF ALL EXIST. SOIL TO PEQ. DEPTH AND BACKFILLED W PEQUIPED SOIL MIX. COMPLETELY PEMOVE ALL STRINGS, PIBBONS AND TAGS FROM POOT BALL SCAPIFY POOTS ON POT BOUND PLANTS

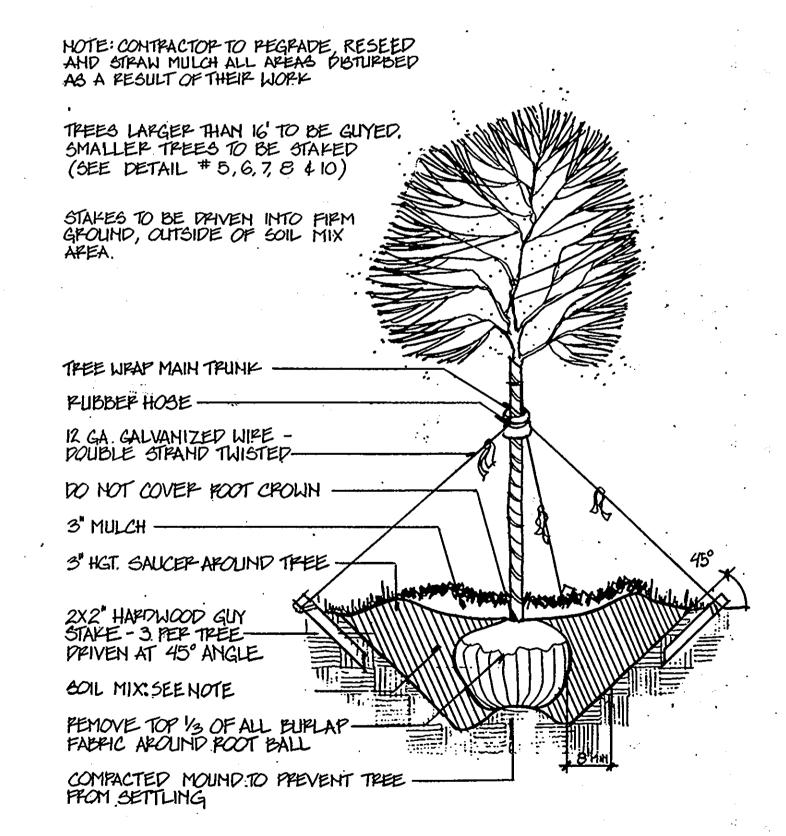
SHRUB PLANTING BED

COMPACT SOIL TO 85% AT OPTIMUM MOISTUPE CONTENT-



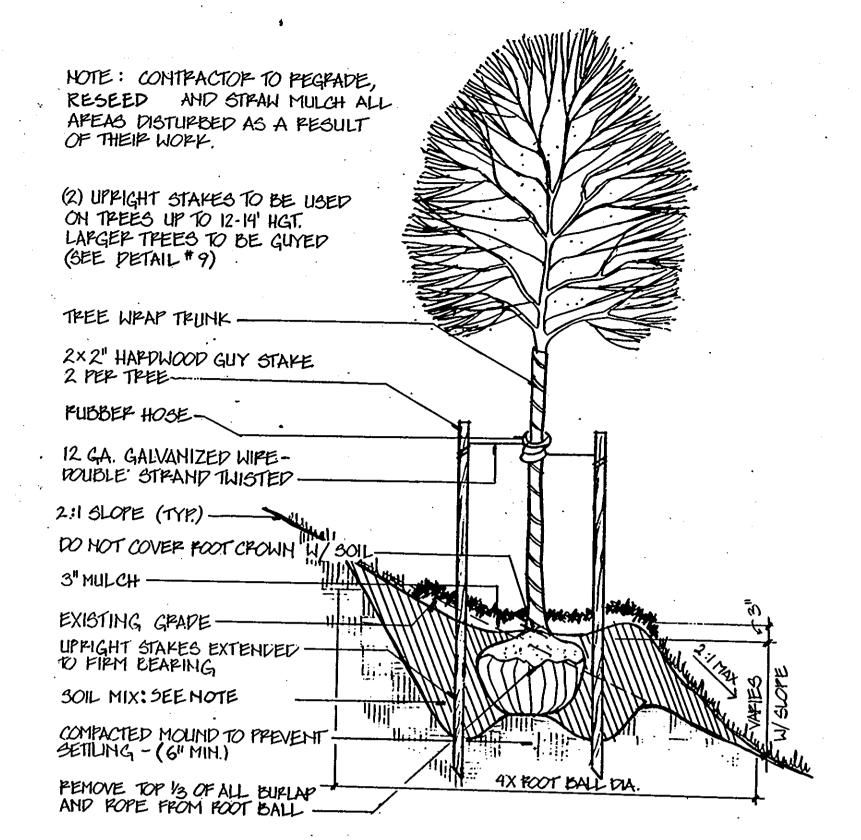


TYPICAL TREE STAKING DETAIL



TREE GUYING DETAIL FOR LARGE SPECIMEN TREES

REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION



TREE ON SLOPE

Flowering Dogwood 8-10'

Japanese Black Pin

PLANTING NOTES

PLANT LIST

Prunus serrulata
PSK 17 'Kwanzan'

B&B; Full Specimen

owing plants are not to be placed directly in windows, under building overhangs or in drainage Adjust in field as necessary. All plants to be off sight lines. No trees allowed within

STV/LYON ASSOCIATES 21 GOVERNOR'S COURT BALTIMORE, MARYLAND 21207 (301) 944-9112

BALTIMORE, MARYLAND 21230

SUMMIT PROPERTIES SO3 KEY HIGHWAY

(301) 234-0070

ENGINEER:



ASHTON WOODS

Tax Map#31 Parcel 423 Lot 2

Second Election District

Plat#7522 & 7523 Zoning R-SA-8.

Howard County, Maryland

An Apartment Community

Summit Properties

SCALE:1'=2000'

VICINITY MAP

| PROJECT NO: 88074 REVISIONS: REVISED 7-12-89 REVISED 9-5-89 | DATE: 6-5-89 | | 11.3 | | |
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Landscape Architecture Land Planning

of: 33 42

SOIL MIX FOR ALL SHRUB BEDS AND TREES SHALL BE:

2/3 TOPSOIL

1/6 COARSE SAND

1/6 PEAT MOSS

ENGINEER'S CERTIFICATE I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVA-

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DEEMED/NECESSARY.

DEVELOPER'S CERTIFICATE

DISTRICT OR THEIR AUTHORIZED AGENTS, AS ARE

APPROVED:

THIS DEVELOPMENT IS APPROVED FOR EROSION AND SEDIMENT CONTROL BY "THE HOWARD SOIL CONSERVATION DISTRICT

HOWARD SOIL CONSERVATION DISTRICT

DISTRICT AND MEETS TECHNICAL REQUIREMENTS

APPROVED HOWARD COUNTY HEALTH DEPARTMENT FOR PUBLIC WATER AND SEWERAGE

APPROVED HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

Messim & Relie 1.8.60 CHIEF , BUREAU OF ENGINEERING SUBDIVISION NAME SECTION/AREA PARCEL ASHTON WOODS APARTMENTS 423
PLAT NO BLOCK NO ZONE TAX/ZONE ELECTRIC CENSUS TR

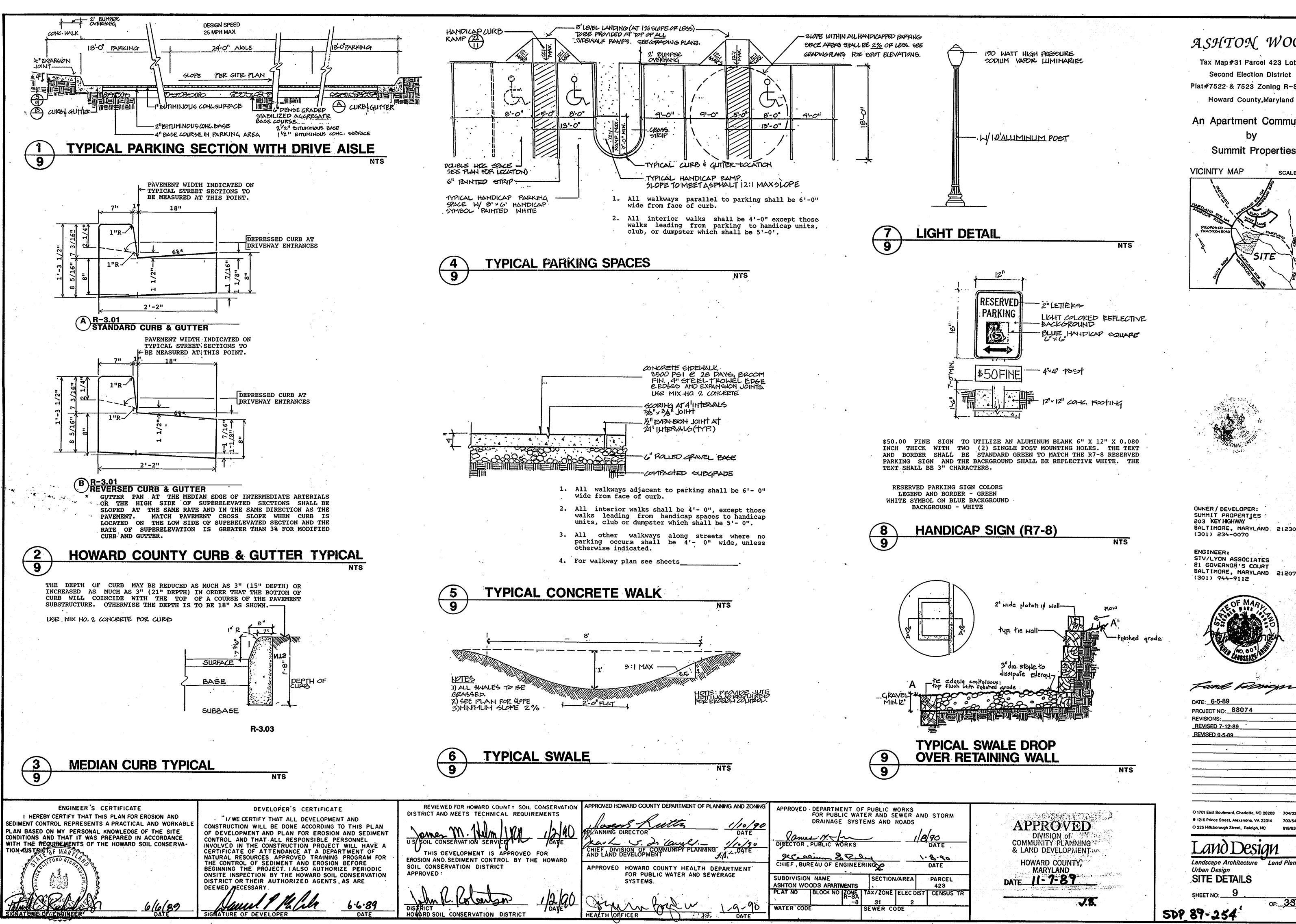
APPROVED DEPARTMENT OF PUBLIC WORKS
FOR PUBLIC WATER AND SEWER AND STORM

DRAINAGE SYSTEMS AND ROADS

APPROVED DIVISION of COMMUNITY PLANNING & LAND DEVELOPMENT HOWARD COUNTY, MARYLAND DATE_11-7-89

Urben Design
PLANTING NOTES, DETAILS AND PLANT LIST SHEET NO: ____8_

SDP 89-254

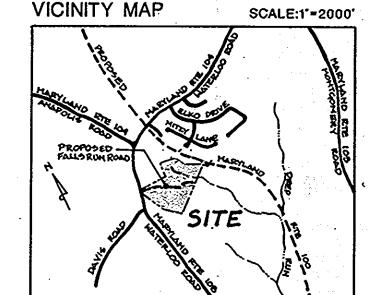


ASHTON WOODS

Tax Map#31 Parcel 423 Lot 2 Second Election District Plat#7522 & 7523 Zoning R-SA-8,

An Apartment Community

Summit Properties





OWNER / DEVELOPER: SUMMIT PROPERTIES BALTIMORE, MARYLAND. 21230 (301) 234-0070

ENGINEER: STV/LYON ASSOCIATES 21 GOVERNOR'S COURT BALTIMORE, MARYLAND 21207 (301) 944-9112

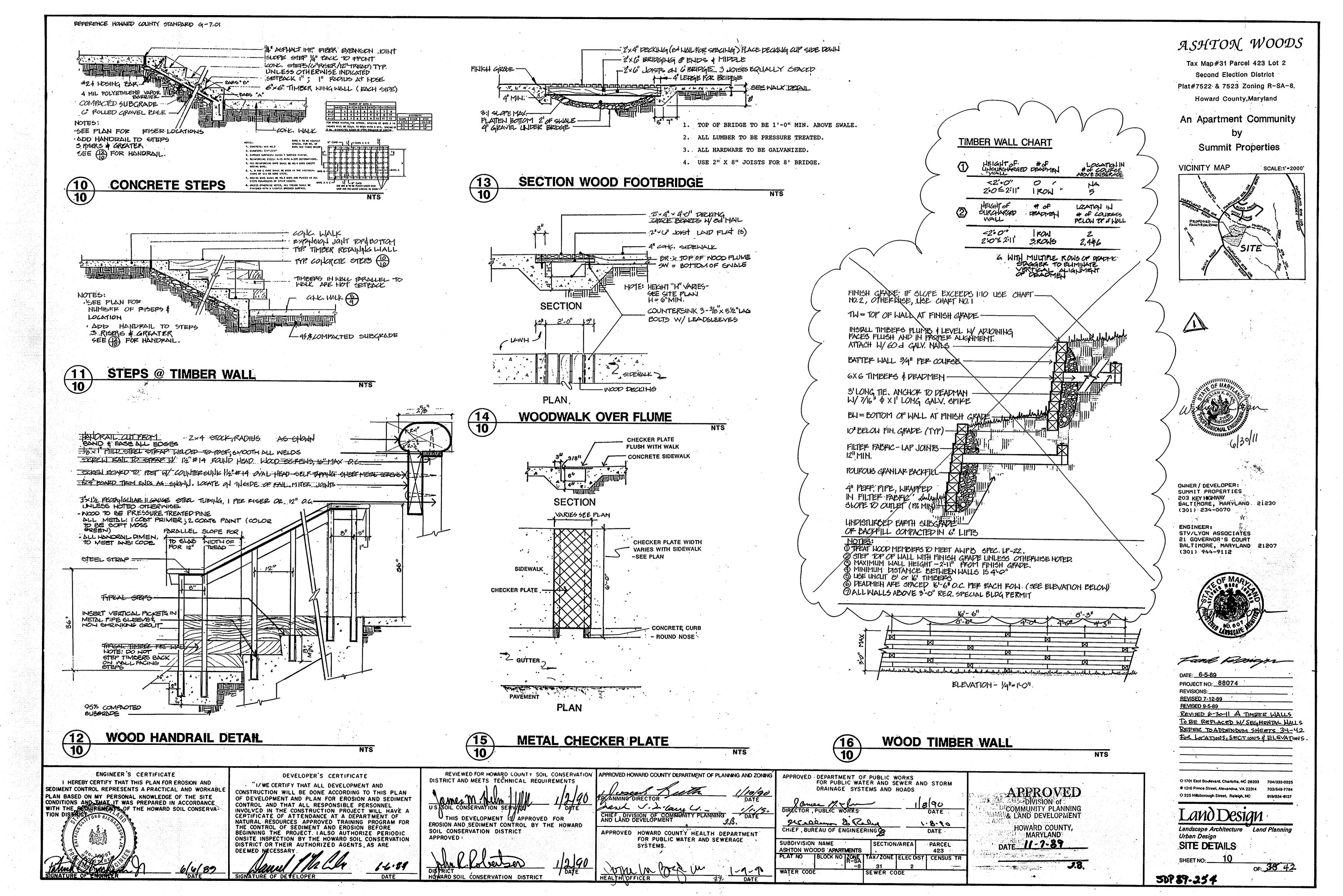


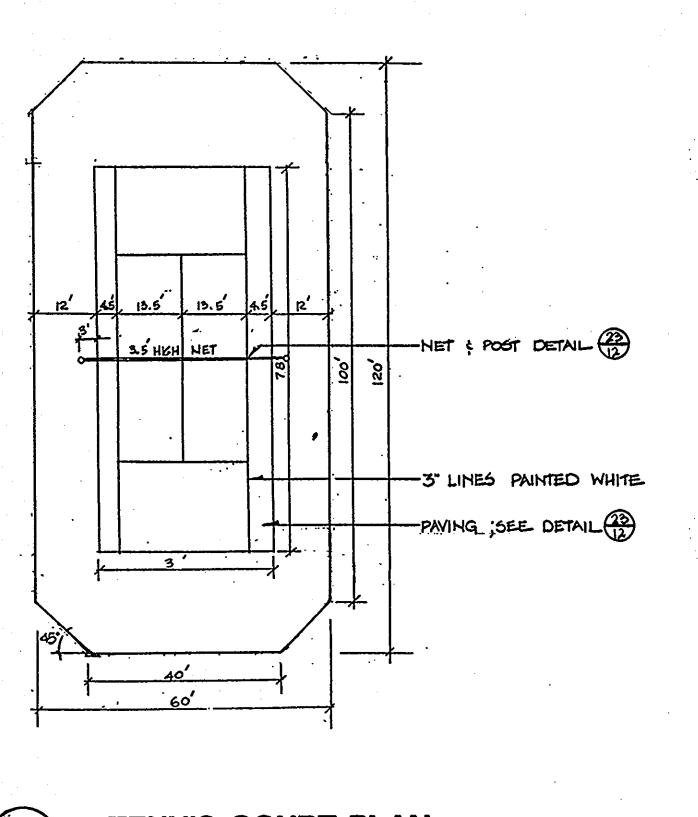
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| DATE: 6-5-89 | | |
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| REVISIONS: | • | |
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Land Design Landscape Architecture Land Planning

SITE DETAILS

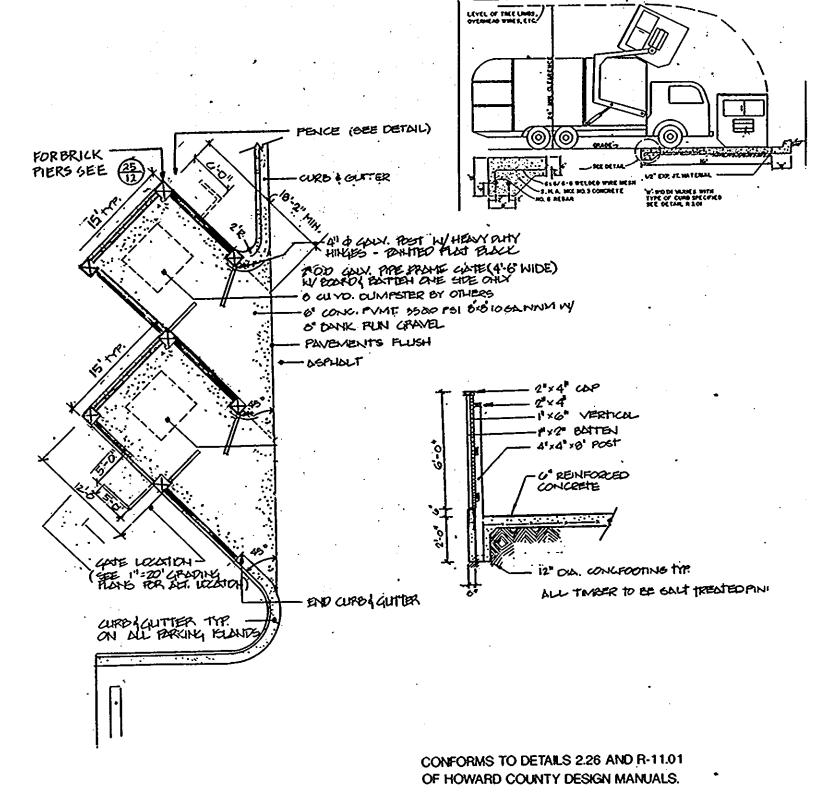
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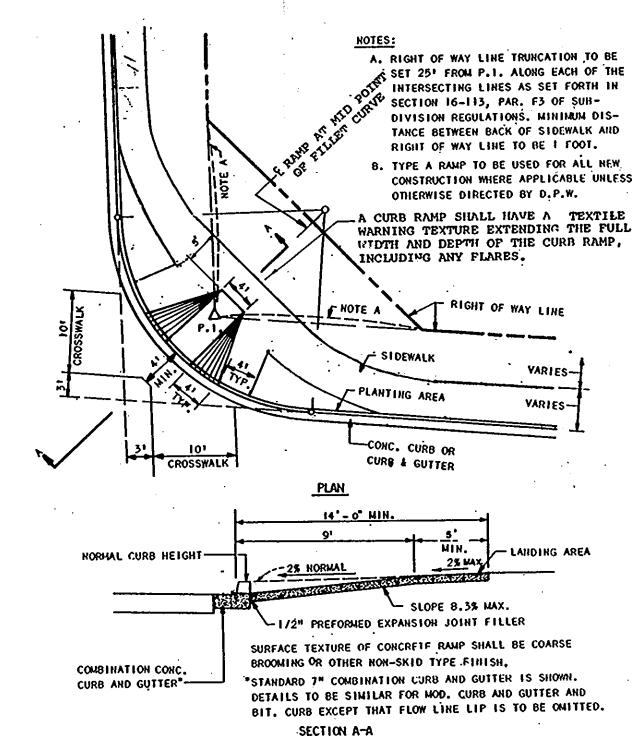




FENCE NOTES

- A. Fabric il gauge 1- 3/4° galvantend steel wire in a chain link mesh with dark green plastic sheathing. The fence to be 10 feet high except as shown otherwise at tennis
- Corner, end and gate posts 3 inch schedule 40 galvanized pipe. Color to be dark green
- Line post 2 1/2 inch schedule 40 galvanized pipe. Color to match corner posts.
- Top rails, intermediate rails and corner braces + 1 5/8 inch .065 galvanized tubing. Color to match corner post.
- E. Bottom tension wire 7 gauge galvanized
- F. Gate 1-5/8 inch .065 galvanized tubing Color to match corner post.
- Post tops All posts shall have malleable iron tops to exclude moisture. Color to match corner posts.





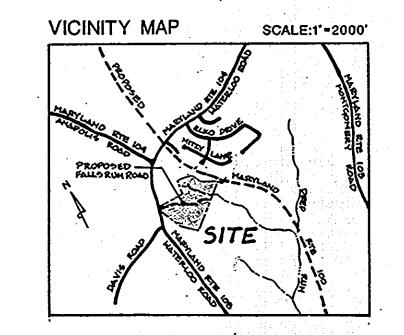
ASHTON WOODS

Tax Map#31 Parcel 423 Lot 2 Second Election District

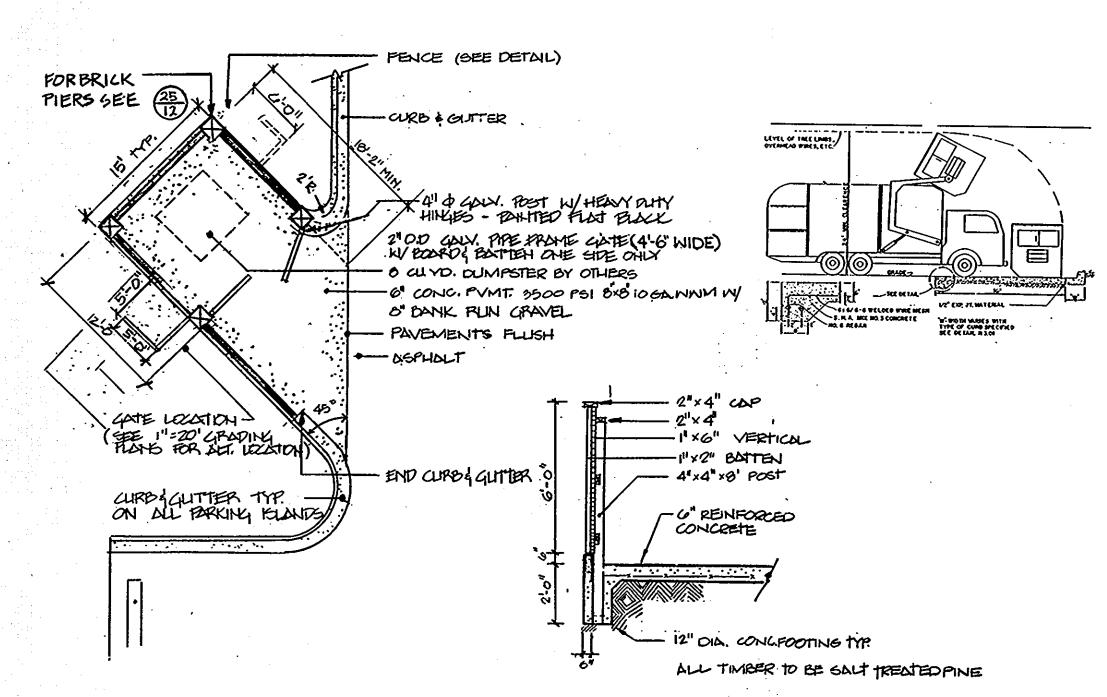
Plat#7522 & 7523 Zoning R-SA-8. Howard County, Maryland

An Apartment Community

Summit Properties

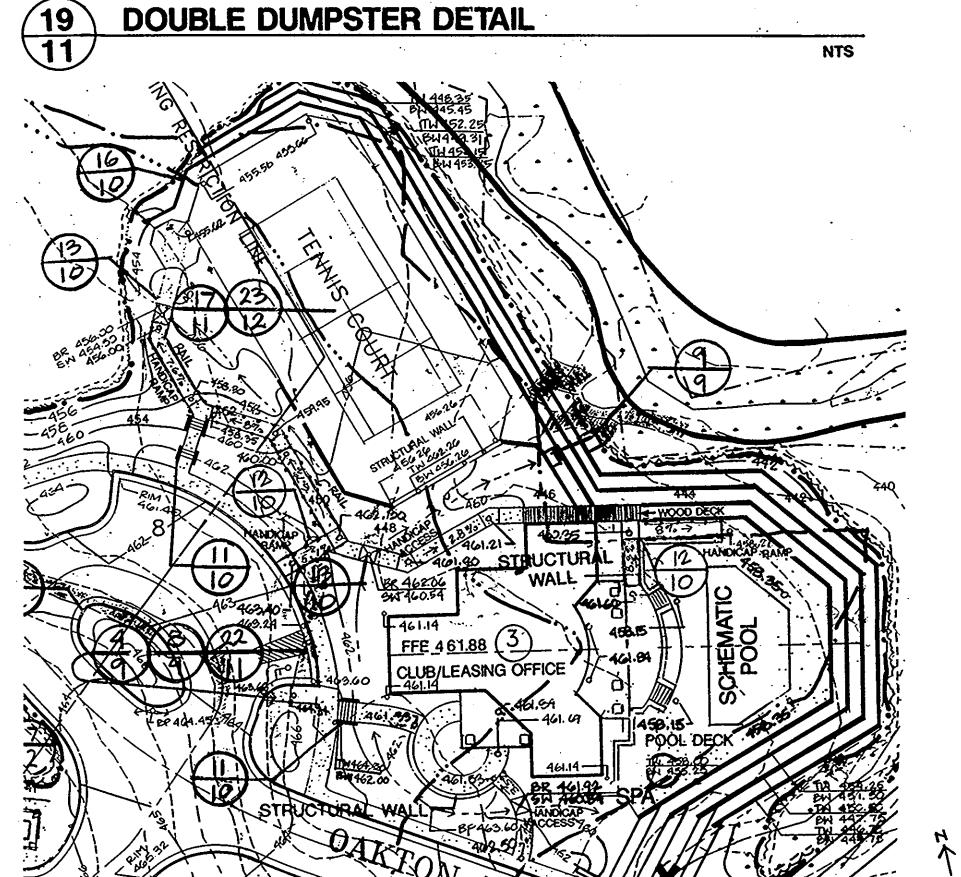






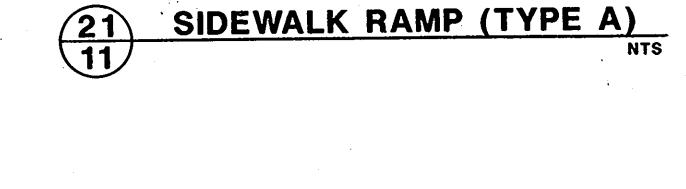
CONFORMS TO DETAILS 2.26 AND R-11.01 OF HOWARD COUNTY DESIGN MANUALS.

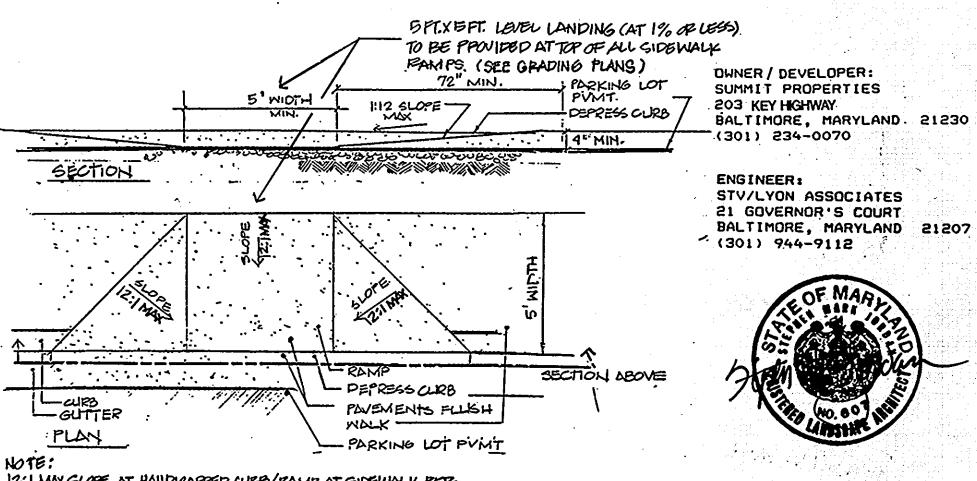
<u>18</u> 11 DUMPSTER DETAIL



CLUB/LEASING OFFICE/POOL/TENNIS COURT GRADING

REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION APPROVED HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING





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| | DATE: 6-5-89 | |
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9 1216 Prince Street, Alexandria, VA 22314

Land Design Landscape Architecture Land Planning Urban Design

SITE DETAILS SHEET NO:_

OF: 3342

ENGINEER'S CERTIFICATE I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THATLIT WAS PREPARED IN ACCORDANCE WITH THE RECURRENT SOF THE HOWARD SOIL CONSERVATION DISTRICK ORD RUA

DEVELOPER'S CERTIFICATE

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DEEMED! NECESSARY.

DATE

THIS DEVELOPMENT IS APPROVED FOR EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT APPROVED : HOWARD SOIL CONSERVATION DISTRICT

DISTRICT AND MEETS TECHNICAL REQUIREMENTS

1/10-/9 • DATE APPROVED HOWARD COUNTY HEALTH DEPARTMENT FOR PUBLIC WATER AND SEWERAGE SYSTEMŠ. DATE

CHIEF, BUREAU OF ENGINEERING 1-8.90 DATE SUBDIVISION NAME PARCEL SECTION/AREA ASHTON WOODS APARTMENTS PLAT NO BLOCK NO ZONE TAX/ZONE ELEC DIST CENSUS TR SEWER CODE

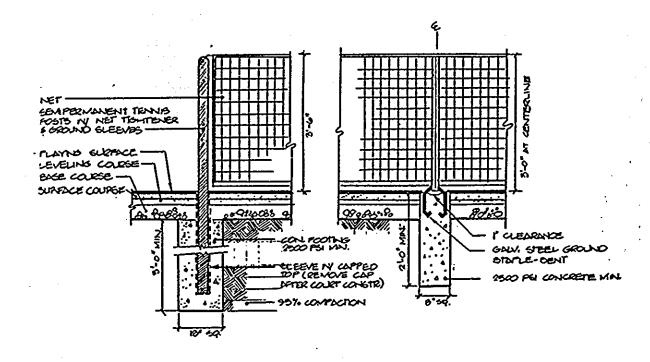
APPROVED DEPARTMENT OF PUBLIC WORKS
FOR PUBLIC WATER AND SEWER AND STORM

DRAINAGE SYSTEMS AND ROADS

COMMUNITY PLANNING & LAND DEVELOPMENT HOWARD COUNTY, MARYLAND and the state of the second state of the secon

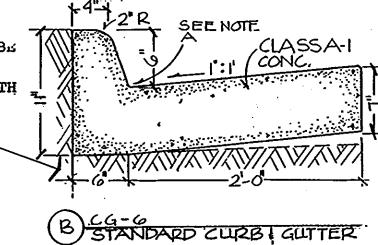
DIVISION of

5DP 89-254

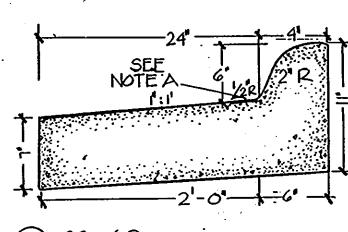


23 12

TENNIS COURT NET & POSTS W/PAVEMENT SECTION



NOTE A: 2" RADIUS WILL BE ALLOWED WITH CURB AND GUTTER.



C CG-GR
REVERSED CURB & GUTTER

12

CURB & GUTTER DETAIL

-95% compacted subgrade

25 12

ENGINEER'S CERTIFICATE

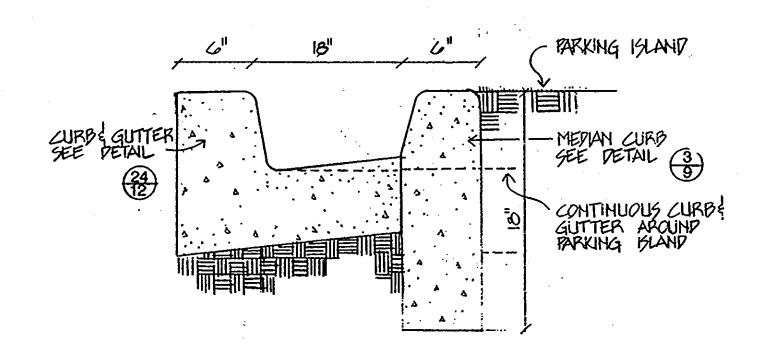
HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND

SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE

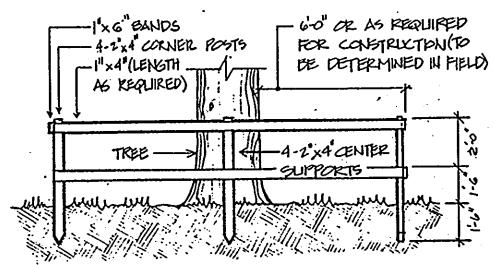
PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE

CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT FORD RICE

BRICK DUMPSTER PIERS & GATE

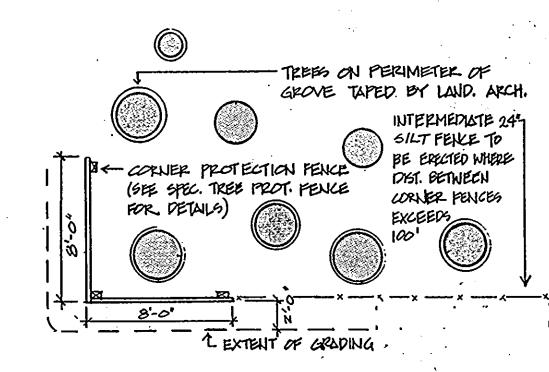


CONCRETE FLUME



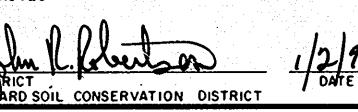
FENCE SHALL BE PAINTED WHITE no building materials, diet, or equipment shall be stored INSIDE THE FENCE.

SPECIMEN TREE PROTECTION FENCE



REVIEWEL FOR HOWARL JUN'T SUIL CONSERVATION DISTRICT AND MEETS TECHNICAL REQUIREMENTS

THIS DEVELOPMENT IS APPROVED FOR EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT APPROVED



APPROVED HOWARD COUNTY HEALTH DEPARTMENT FOR PUBLIC WATER AND SEWERAGE

APPROVED HOWARD COUNTY DEPARTMENT OF PLANISMS AND ZONING

APPROVED DEPARTMENT OF PUBLIC WORKS
FOR PUBLIC WATER AND SEWER AND STORM DRAINAGE SYSTEMS AND ROADS CHIEF, BUREAU OF ENGINEERING SUBDIVISION NAME. ASHTON WOODS APARTMENTS PLAT NO BLOCK NO ZONE TAX/ZONE ELECTIST CENSUS TR

DIVISION of COMMUNITY PLANNING

& LAND DEVELOPMENT HOWARD COUNTY, MARYLAND

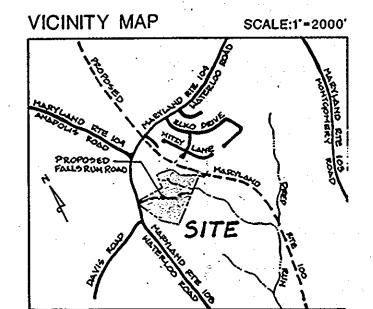
ASHTON WOODS

Tax Map#31 Parcel 423 Lot 2 **Second Election District**

Plat#7522 & 7523 Zoning R-\$A-8. Howard County, Maryland

An Apartment Community

Summit Properties



OWNER / DEVELOPER: SUMMIT PROPERTIES SO3 KEN HIGHMAN BALTIMORE, MARYLAND 21230 (301) 234-0070

ENGINEER: STV/LYON ASSOCIATES 21 GOVERNOR'S COURT BALTIMORE, MARYLAND 21207 (301) 944-9112



| DATE: 6-5-89 | |
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Landscape Architecture Land Planning SITE DETAILS

SHEET NO: 12 509 89-254

OF: 3342

DEEMED NECESSARY

DEVELOPER'S CERTIFICATE

"I/WE CERTIFY THAT ALL DEVELOPMENT AND

CONTROL AND THAT ALL RESPONSIBLE PERSONNEL

CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN

OF DEVELOPMENT AND PLAN FOR EROSION AND SEDIMENT

INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF

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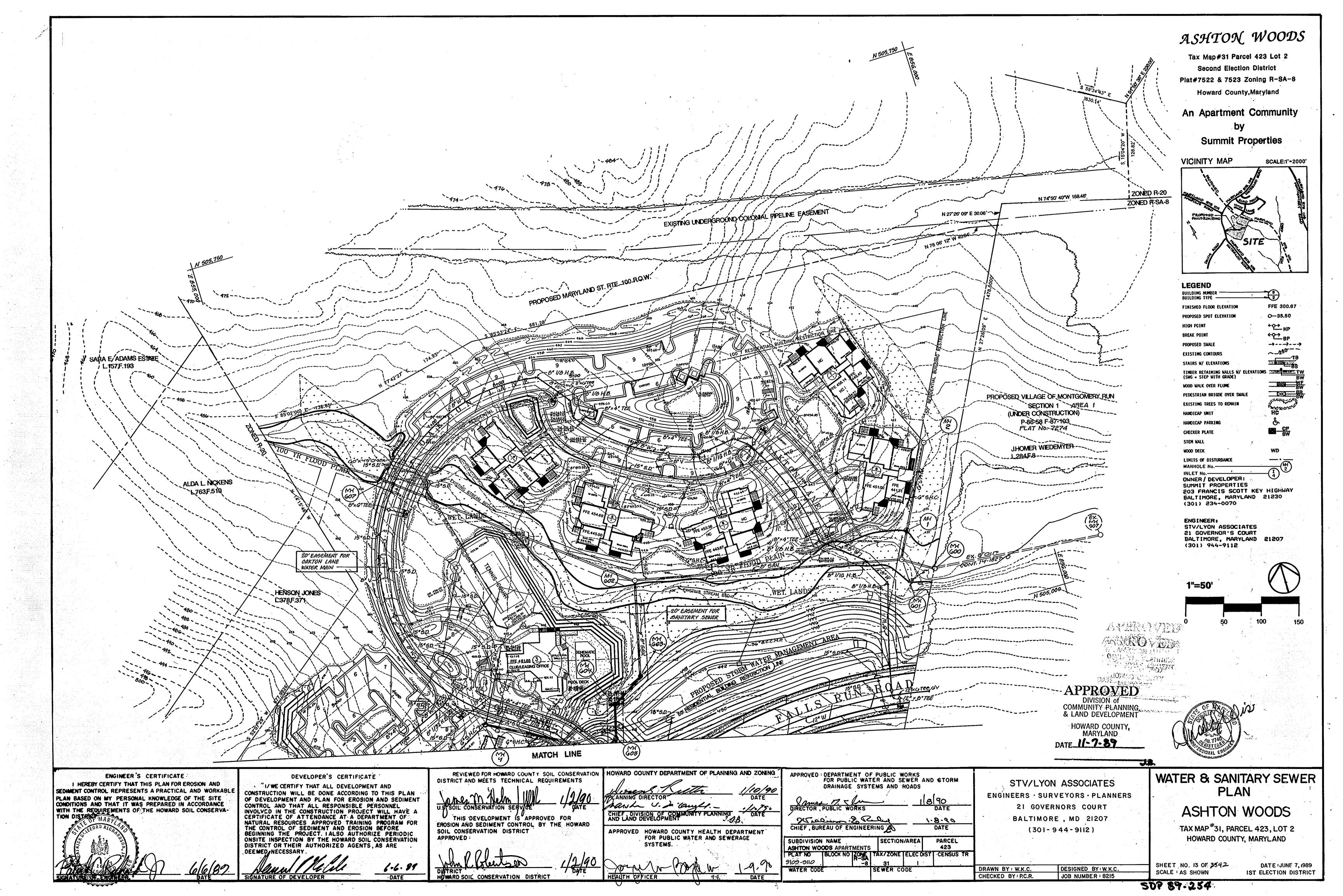
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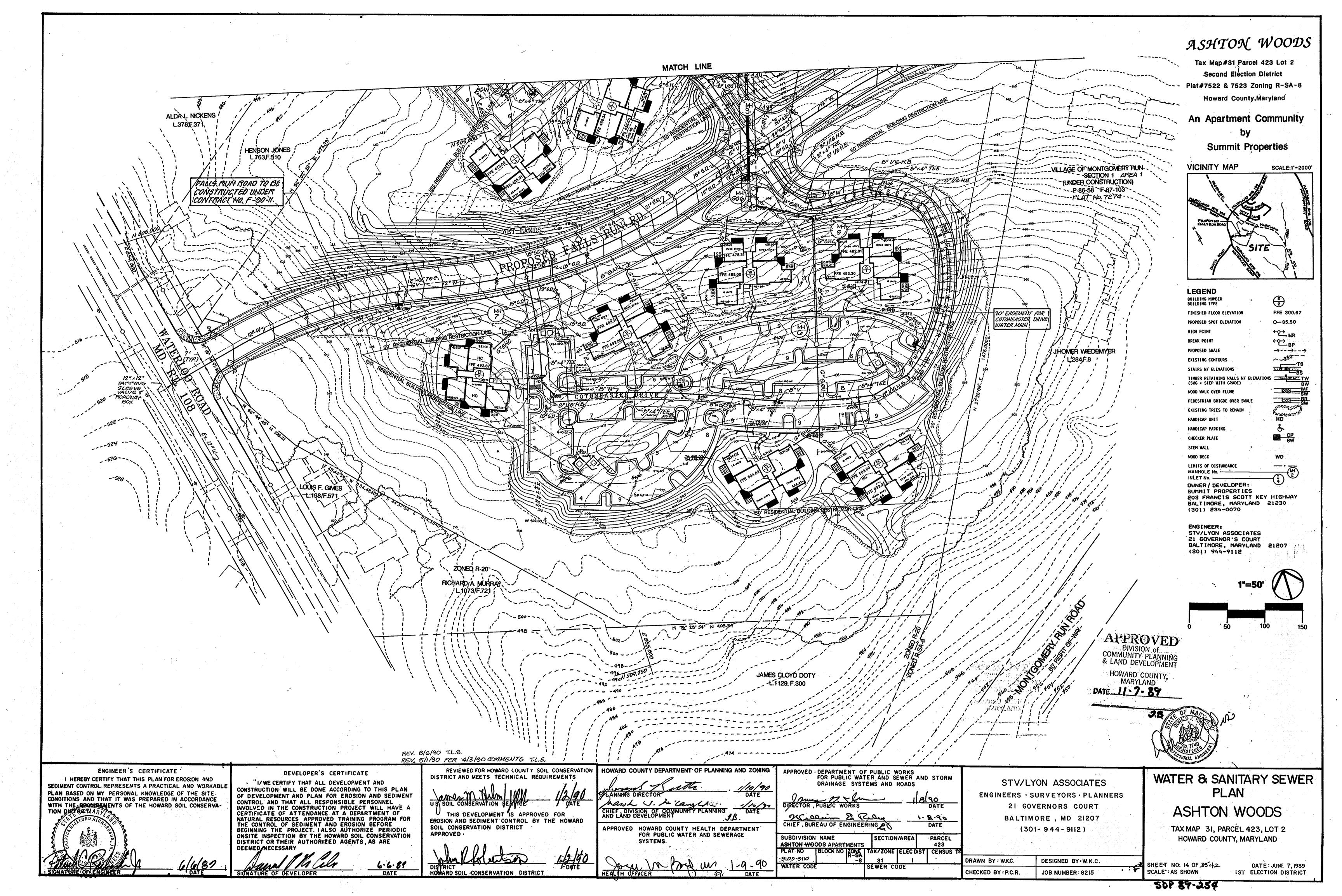
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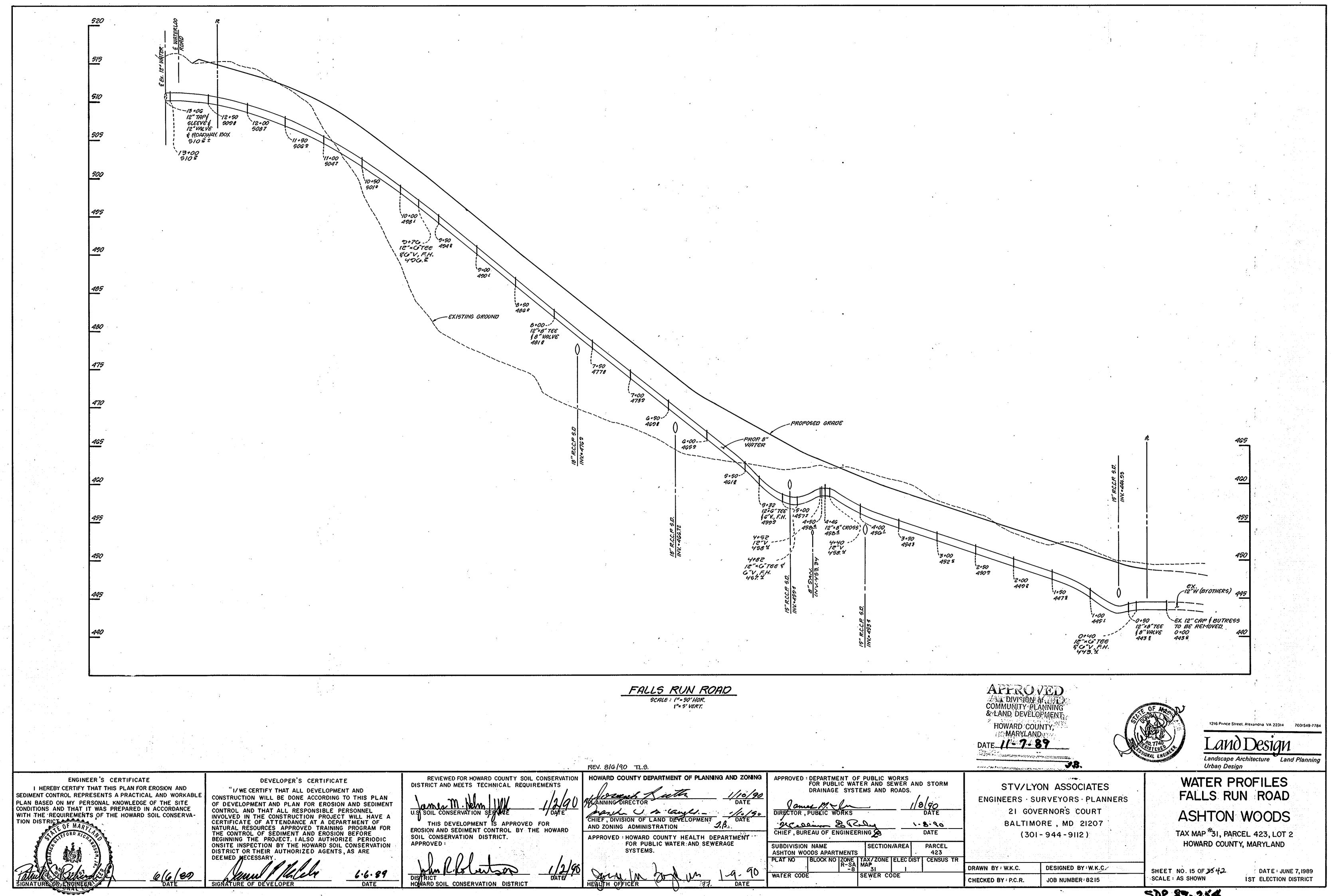
DISTRICT OR THEIR AUTHORIZED AGENTS, AS ARE

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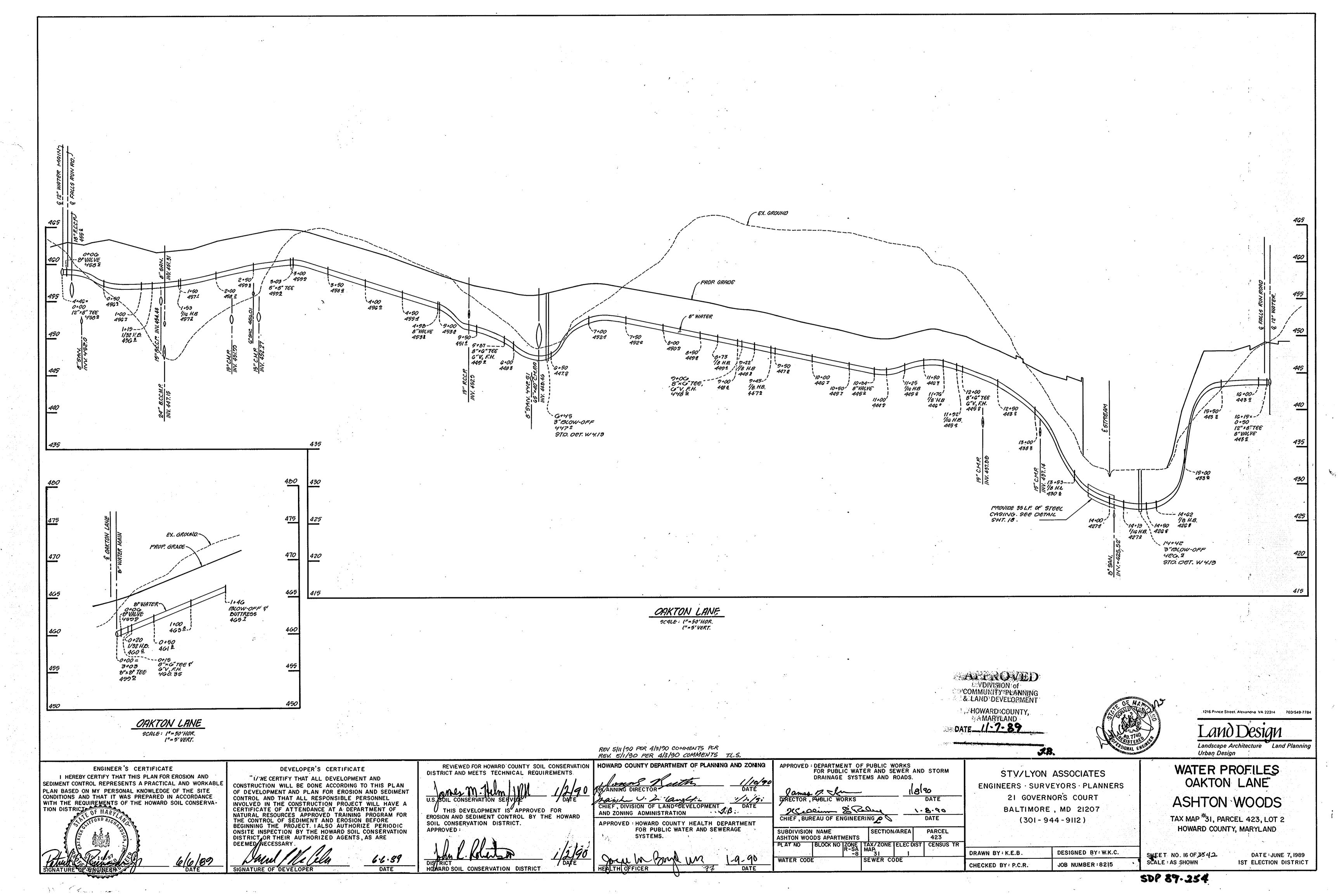
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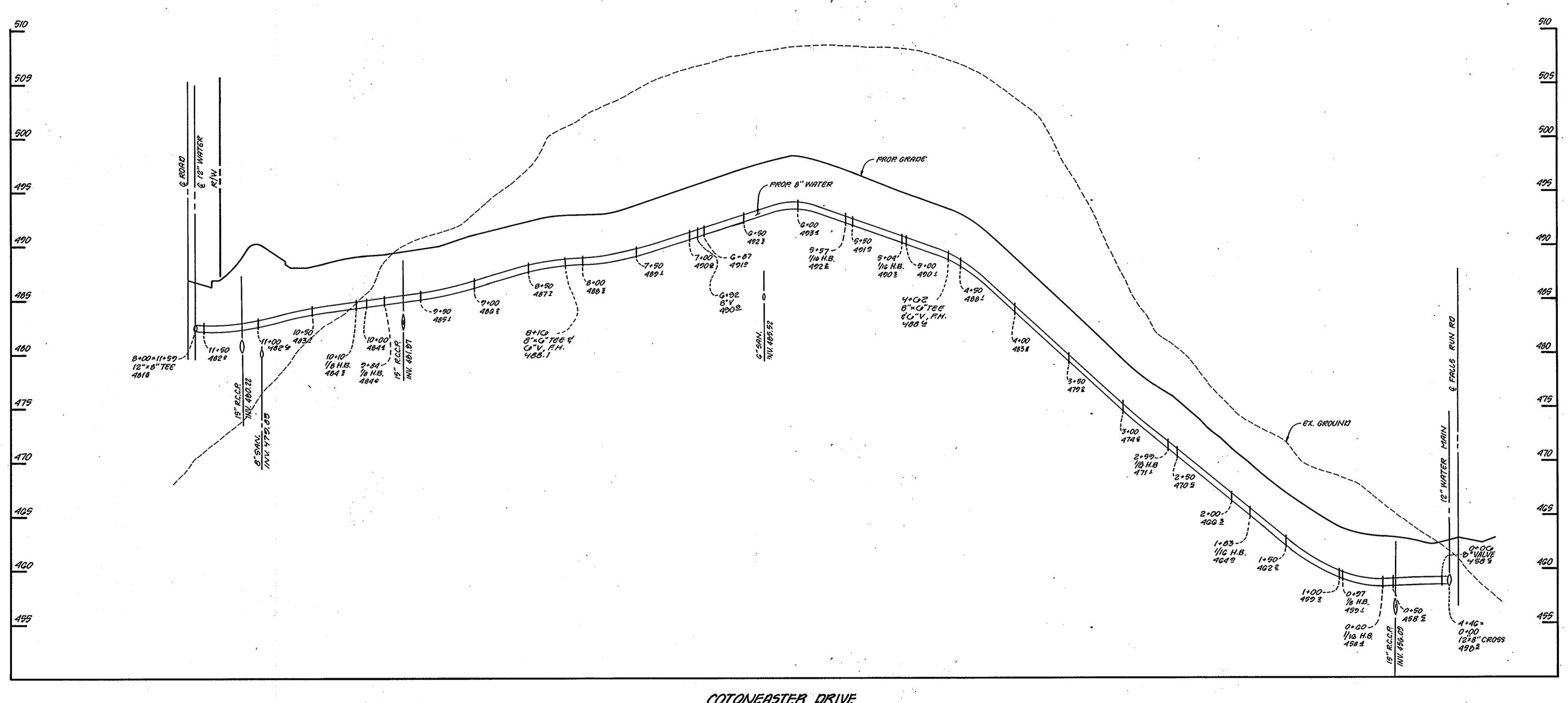






SDP 87-254





COTONEASTER DRIVE

DIVISION OF COMMUNITY PLANNING & LAND DEVELOPMENT HOWARD COUNTY,

CHECKED BY : P.C.R.



1216 Prince Street, Alexandria, VA 22314 703/549 7784

Urban Design

ENGINEER'S CERTIFICATE I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT OF MARY

DEVELOPER'S CERTIFICATE

"I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN OF DEVELOPMENT AND PLAN FOR EROSION AND SEDIMENT CONTROL AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF NATURAL RESOURCES APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ONSITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT OR THEIR AUTHORIZED AGENTS, AS ARE DEEMED, NECESSARY.

| EEMED, NECESSARY. | |
|-----------------------|--------|
| Juni Pla Cilu | 6.6.29 |
| 1000000 1 100 00 | 0.001 |
| IGNATURE OF DEVELOPER | DATE |

THIS DEVELOPMENT IS APPROVED FOR EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.

APPROVED:

REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION DISTRICT AND MEETS TECHNICAL REQUIREMENTS

HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

| 1 | | ŧ |
|---|---|----------|
| • | CHIEF, DIVISION OF LAND DEVELOPMENT DATE | DIR |
| | CHIEF, DIVISION OF LAND ENTELOPMENT J.B. DATE | _2 |
| | APPROVED : HOWARD COUNTY HEALTH DEPARTMENT | CHI |
| | FOR PUBLIC WATER AND SEWERAGE | SUB |
| | SYSTEMS. | ASHI |
| | | PL AT |
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| - | Jack In 1200 100 1-90 10 | WATE |
| | HEAUTH OFFICER C TATE DATE | ! |

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| DIRECTOR | PUBLIC WO | ALL RKS | | _ | | lalq. | | E |
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| SUBDIVISION ASHTON WO | | MENTS | | SECTION | I/AREA | | RCEL 23 | |
| PLAT NO | BLOCK NO | ZONE R-SA -8 | | X/ZONE AP 31 | ELEC D | IST CE | NSUS TR | DRAW |
| WATER COD | Ε | | SI | EWER CO | DDE | | | |

STV/LYON ASSOCIATES ENGINEERS · SURVEYORS · PLANNERS 21 GOVERNOR'S COURT BALTIMORE, MD 21207 (301 - 944 - 9112)

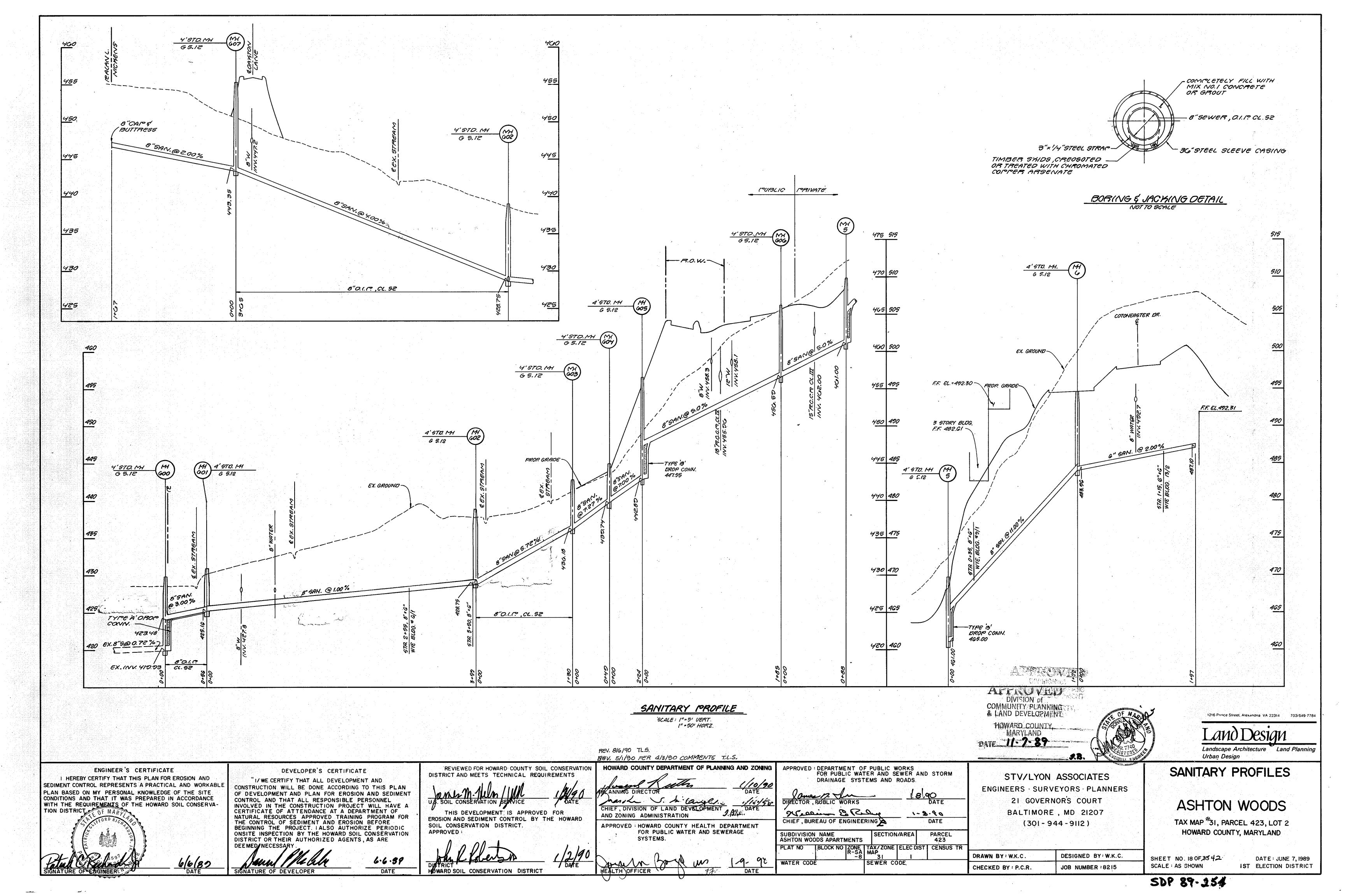
DESIGNED BY : W.K.C. DRAWN BY: K.E.B.

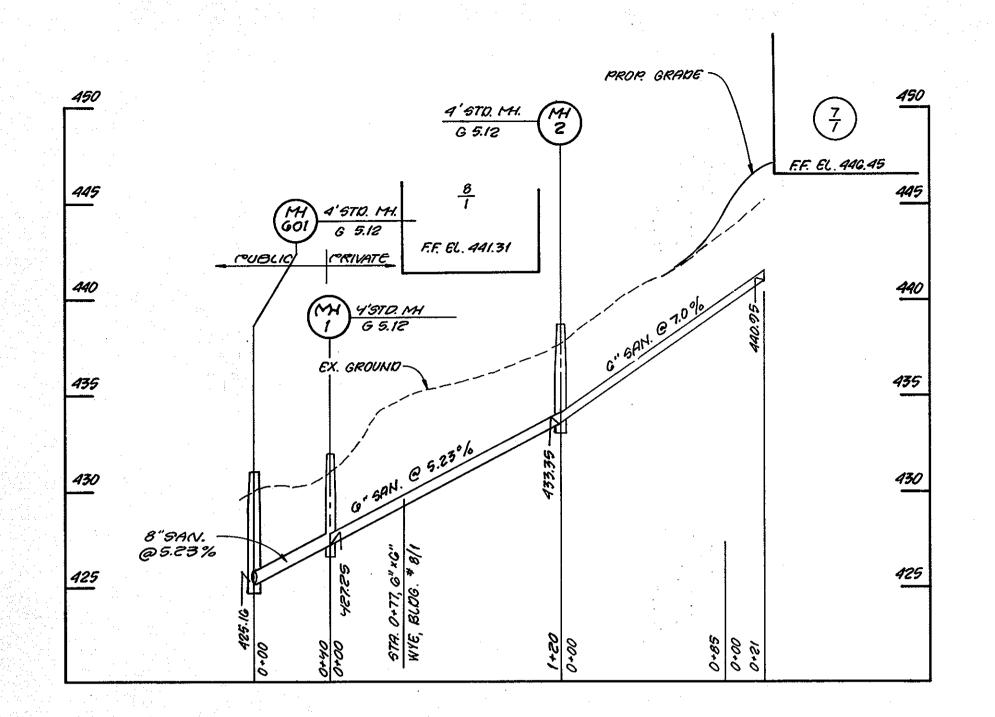
WATER PROFILES COTONEASTER DRIVE ASHTON WOODS

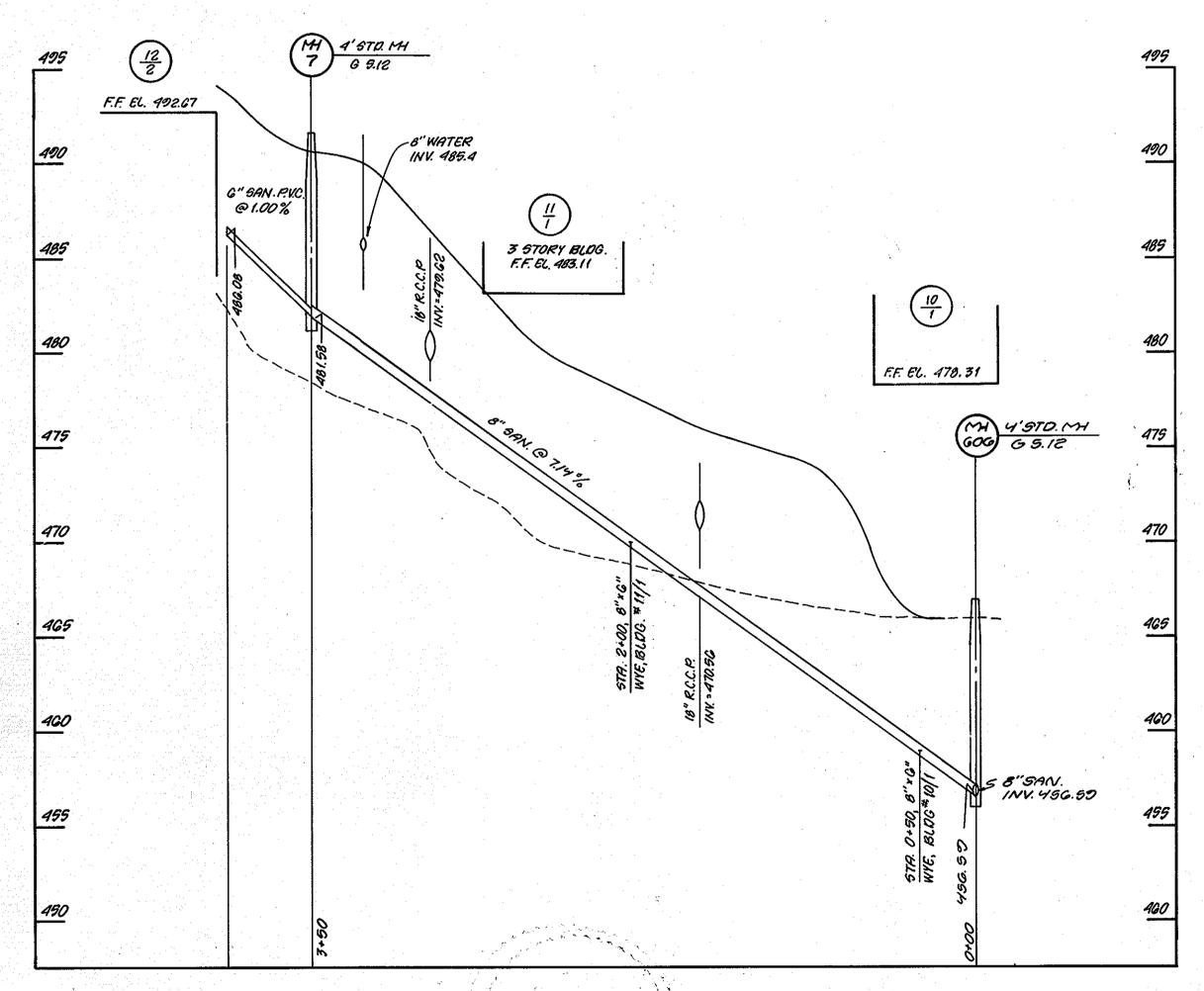
TAX MAP *31, PARCEL 423, LOT 2 HOWARD COUNTY, MARYLAND

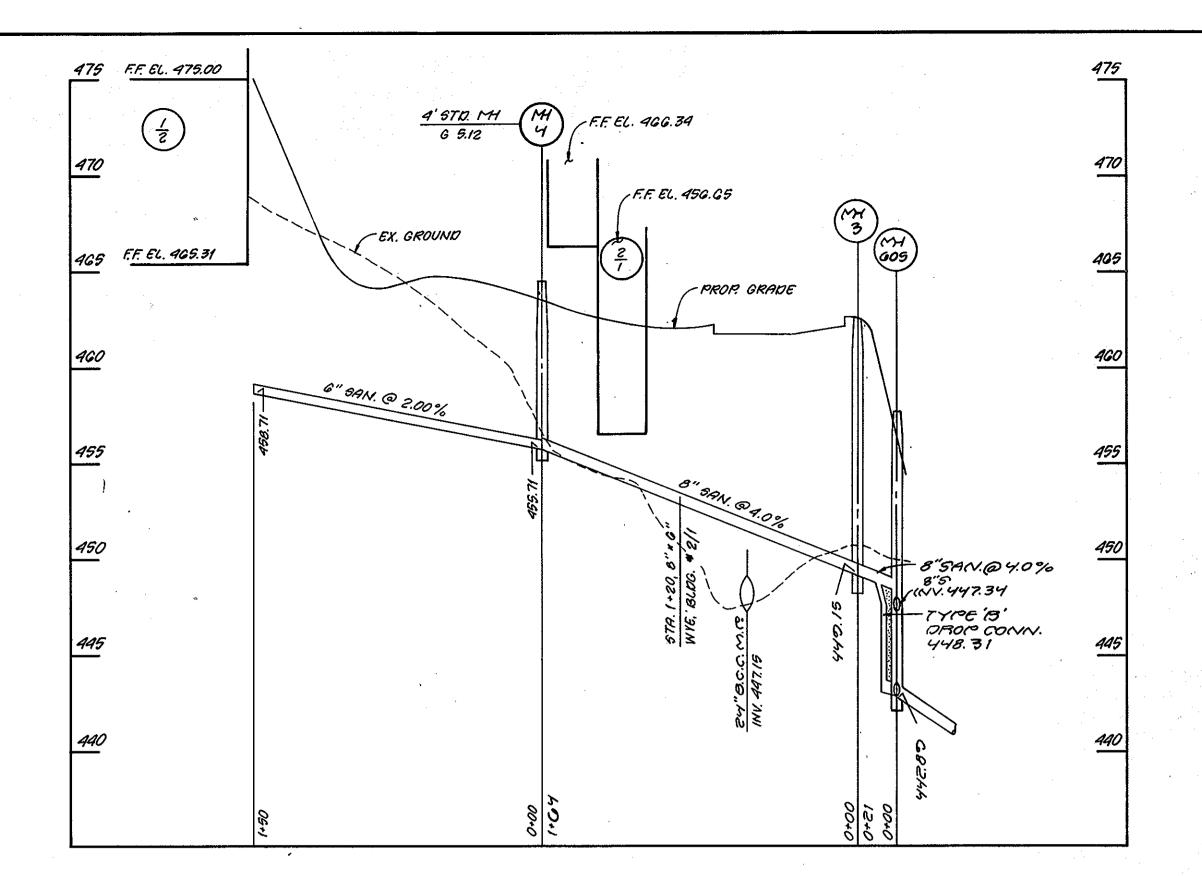
SHEET NO. 17 OF 3342.

, DATE: JUNE 7, 1989 IST ELECTION DISTRICT









APPROVED
DIVISION of COMMUNITY PLANNING & LAND DEVELOPMENT

HOWARD COUNTY, MARYLAND DATE: 11-7-89

CHECKED BY : P.C.R.

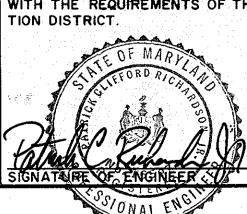


1216 Prince Street, Alexandria, VA 22314 703/549-7784

Landscape Architecture Land Planning Urban Design

DATE: JUNE 7, 1989

ENGINEER'S CERTIFICATE I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND EDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVA-



DEVELOPER'S CERTIFICATE

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EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT. APPROVED:

REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION DISTRICT AND MEETS TECHNICAL REQUIREMENTS CHIEF, DIVISION OF LAND DEVELOPMENT AND ZONING ADMINISTRATION APPROVED : HOWARD COUNTY HEALTH DEPARTMENT FOR PUBLIC WATER AND SEWERAGE

REV. 5/1/90 PER 4/3/90 COMMENTS T.L.S.

APPROVED DEPARTMENT OF PUBLIC WORKS
FOR PUBLIC WATER AND SEWER AND STORM
DRAINAGE SYSTEMS AND ROADS. CHIEF, BUREAU OF ENGINEERING 1.8.90 DATE PARCEL 423 SUBDIVISION NAME ASHTON WOODS APARTMENTS PLAT NO BLOCK NO ZONE TAX/ZONE ELEC DIST CENSUS TR R-SA MAP 31 1

WATER CODE SEWER CODE

STV/LYON ASSOCIATES ENGINEERS · SURVEYORS · PLANNERS 21 GOVERNORS COURT BALTIMORE, MD 21207 (301 - 944 - 9112)

DRAWN BY: W.K.C. DESIGNED BY: W.K.C.

JOB NUMBER : 8215

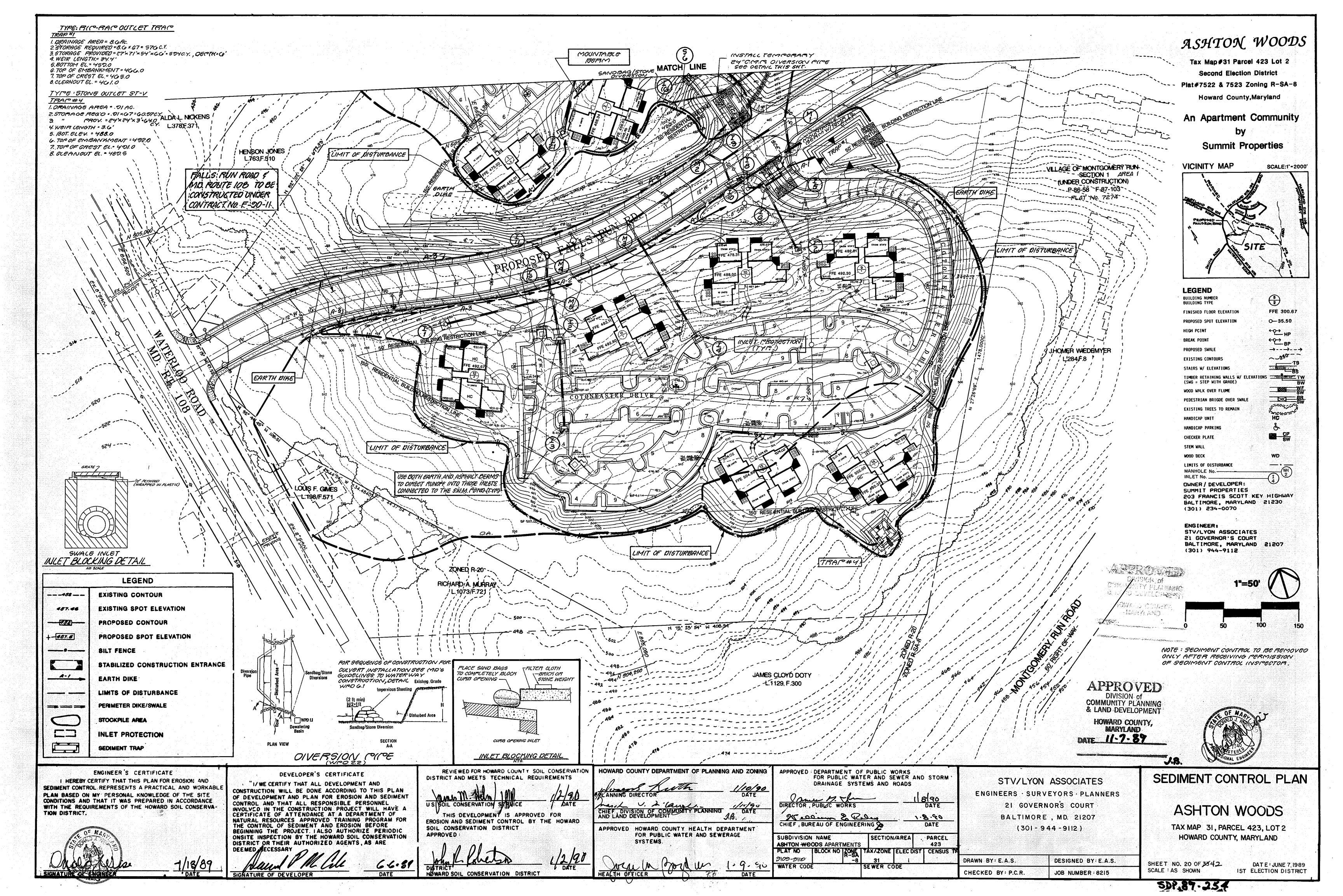
ASHTON WOODS TAX MAP *31, PARCEL 423, LOT 2

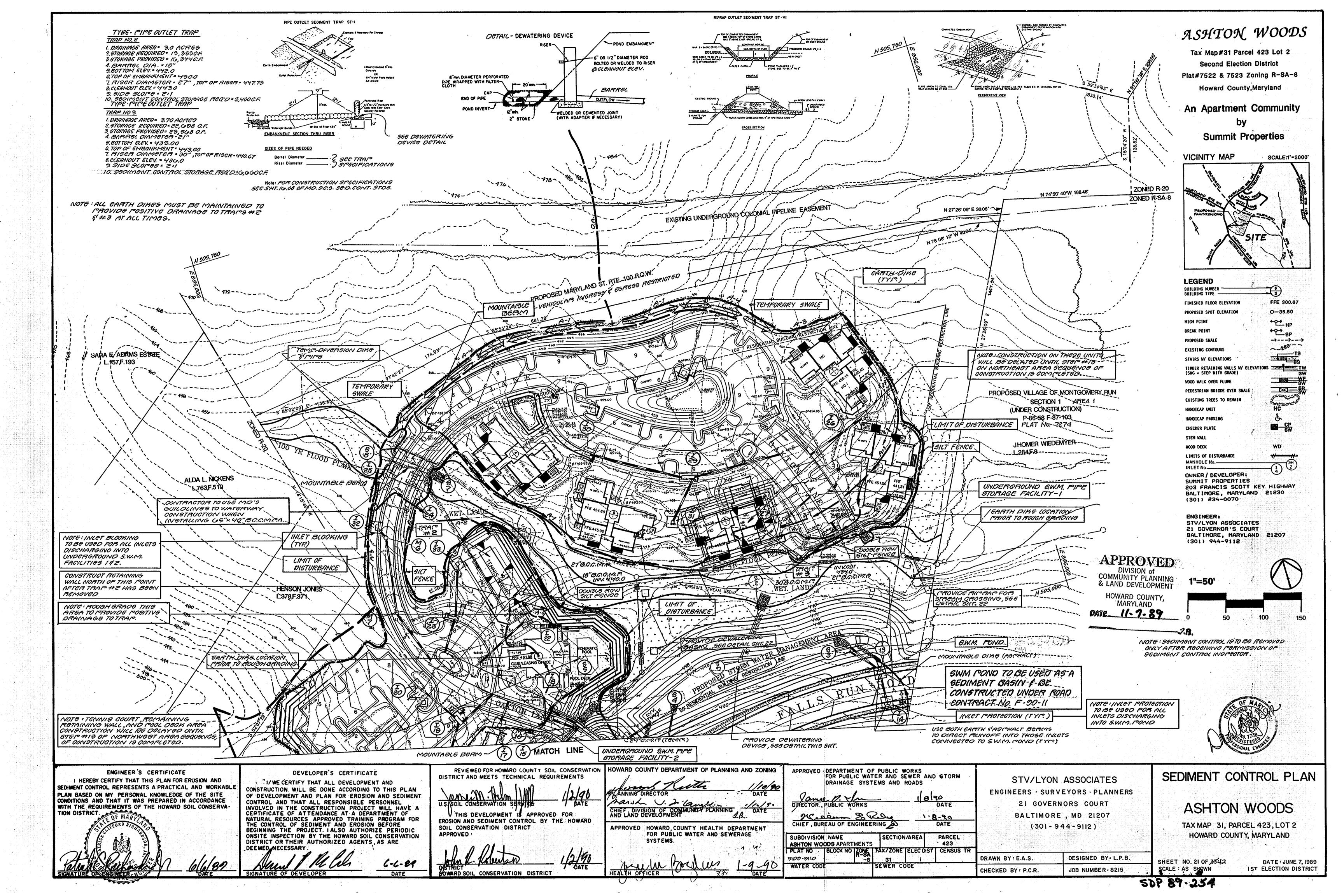
SANITARY PROFILES

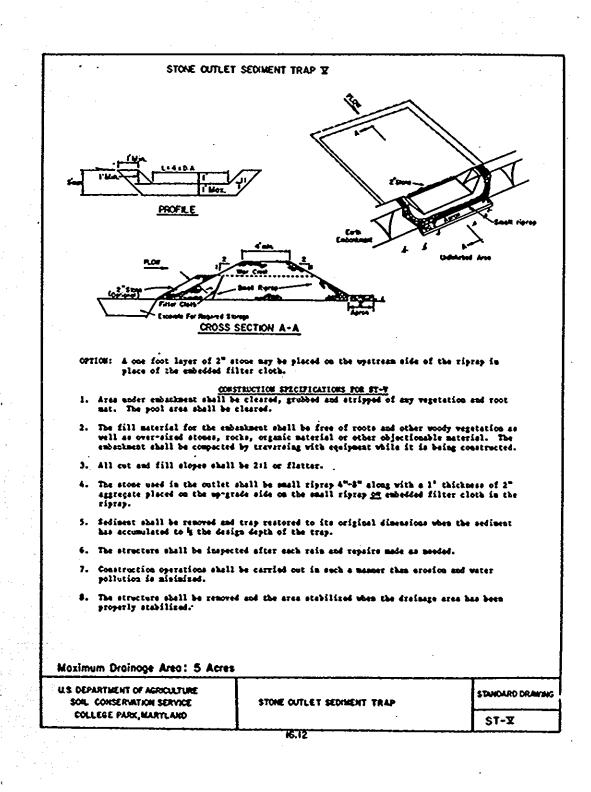
HOWARD COUNTY, MARYLAND

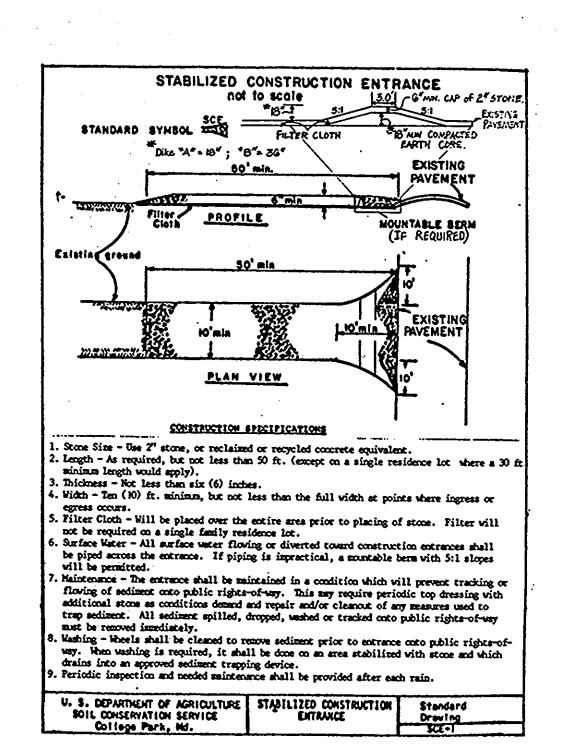
SHEET NO. 19 OF 3842 SCALE : AS SHOWN IST ELECTION DISTRICT

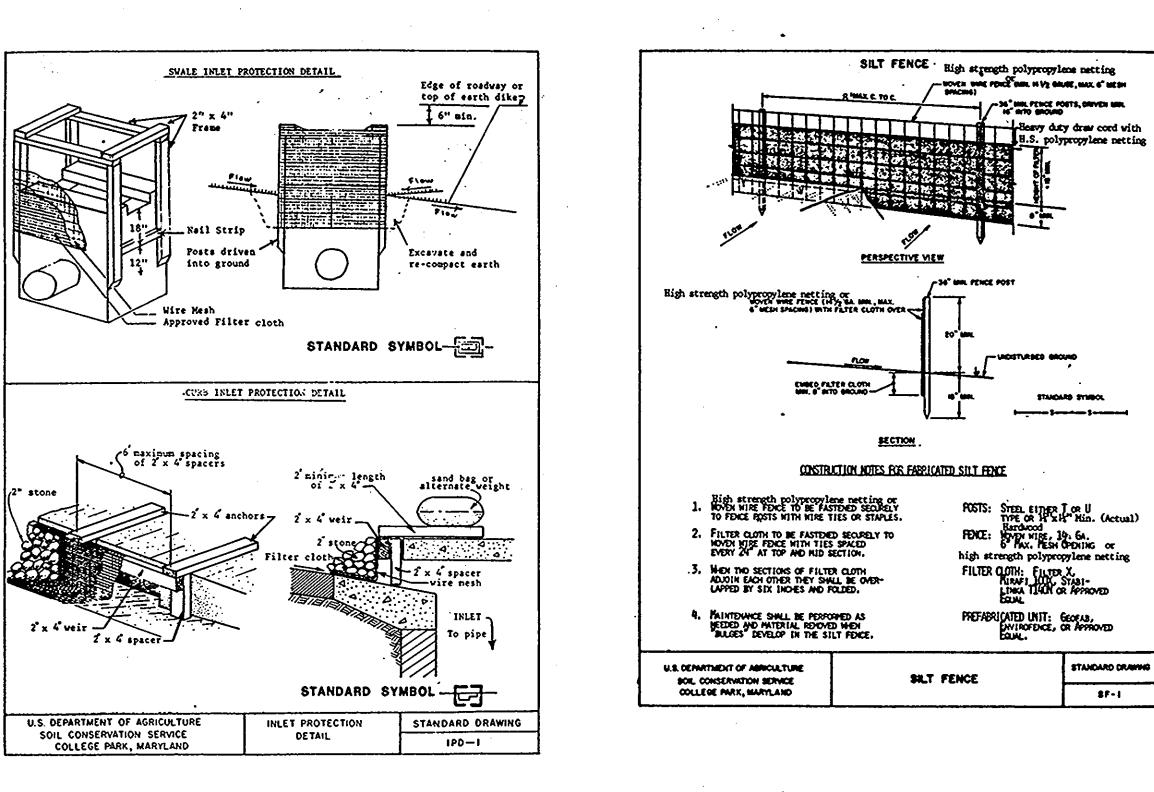
SDP 87-254

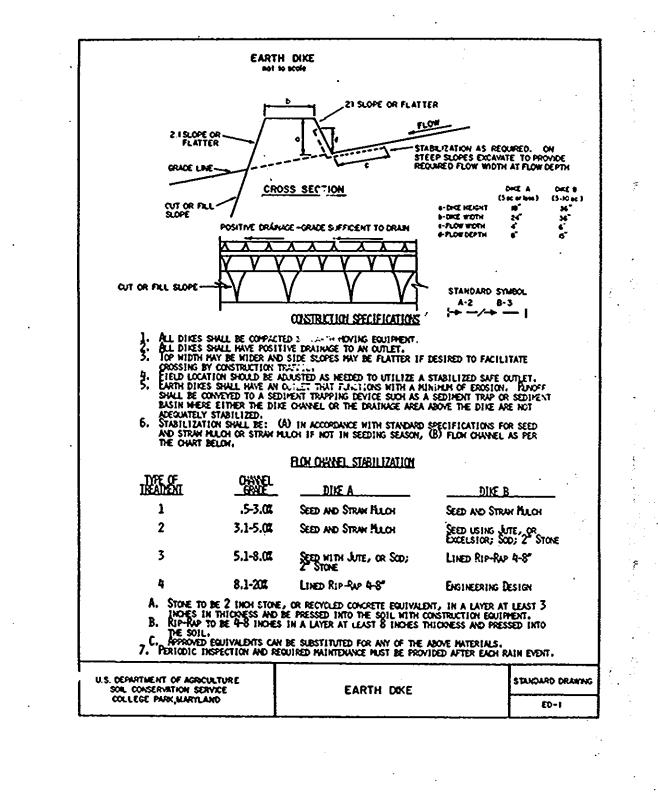


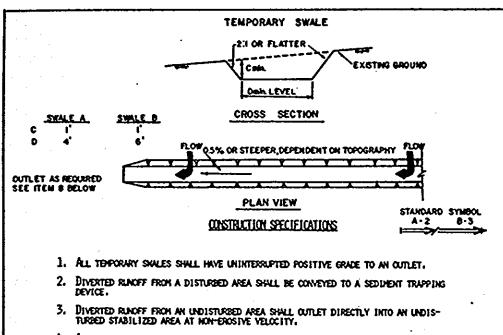










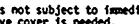


- 4. ALL TREES, BRUSH, STUMPS, OBSTRUCTIONS, AND OTHER OBJECTIONABLE MATERIAL SHALL BE REMOVED AND DISPOSED OF SO AS NOT TO INTERFERE WITH THE PROPER PUNCTIONING OF THE SMALE.
- 5. The same shall be excavated or shaped to line, grade, and cross section as required to meet the criteria specified herein and be free of bank projections or other irregularities which will impede normal flom.
- 6. FILLS SHALL BE COMPACTED BY EARTH MOVING EQUIPMENT.
- 7. ALL EARTH REMOVED AND NOT NEEDED ON CONSTRUCTION SHALL BE PLACED SO THAT IT WILL NOT INTERFERE WITH THE FUNCTIONING OF THE SIGUE.
- 8. STABILIZATION SHALL BE AS PER THE CHART BELON: FLOW CHYNEL STABILIZATION

A (5 AC OR LESS) B (5 AC - 10 AC) 0.5-3.03 SEED AND STRAW MULCH SEED AND STRAW MULCH SEED AND STRAM MULCH SEED USING JUTE OR LINED RIP-RAP 4-8" RECYCLED CONCRETE EQUIVALENT SEED WITH JUTE OR EXCELSION: SOO

LINED 4-8" RIP-RAP ENGINEERED DESIGN 9. PERIODIC INSPECTION AND REQUIRED MAINTENANCE MUST BE PROVIDED AFTER EACH RAIN EVENT, U.S. DEPARTMENT OF AGRICULTUR SOIL CONSERVATION SERVICE TEMPORARY SWALE COLLEGE PARK, MARYLAND T\$ - I

Rip-Rep Shell Consist Of Dense Recke Of Random Shepes And Sizes, Resistant To The Action Of Air And Water And Suitable For Senk Protection, All Reske Shell Weigh A Minimum Of 180 Pounds. L. Erosion Control Pobric Shell Be Poly Filter - X. As Manufactured By Caffbago Mille, Inc., Erosion Control Division, 124 West OG TB Street, Cinnclinati, Ohio; Leurel Erosion Control Cioth As Manufactured By Leurel Piastics Inc., Medison, E Rip-Rop Shell De Plocod G'Eoch Side & Semer Or As Shown, Any Disturbance To Streom Banks Seyond Limits Shown Shall De Protected With Rip-Rep At Contractors Expense. RIP-RAP DETAIL



Apply to graded or cleared areas not subject to immediate further disturbance where a permanent long-lived vegetative cover is needed.

PERMANENT SEEDING NOTES

<u>Seedbed Preparation</u>: Loosen upper three inches of soil by raking, discing or other acceptable means before seeding. <u>Soil Amendments</u>: In lieu of soil test recommendations, use one of the following schedule:

1) Preferred - Apply 2 tons per acre dolomitic limestone (92 lbs/1000 sq ft) and 600 lbs per acre 10-10-10 fertilizer (14 lbs/1000 sq ft) before seeding. Harrows 2000 per fertilizer (14 lbs/1000 sq ft) before seeding, apply 400 lbs per acre 30-0-0 ureaform fertilizer (9 lbs/1000 sq ft).

2) Acceptable - Apply 2 tons per acre dolomitic limestone (92 lbs/1000 sq ft) and 1000 lbs per acre 10-10-10 fertilizer (23 lbs/1000 sq ft) before seeding. Harrow or disc into upper three inches of soil.

Seeding - For the periods March 1 thru April 30, and August 1 thru October 15, seed with 60 lbs per acre (1.4 lbs/1000 sq ft) of Kentucky 31 Tall Fescue. For the period May 1 thru July 31, seed with 60 lbs Kentucky 31 Tall Fescue per acre and 2 lbs per acre (.05 lbs/1000 sq ft) of weeping lovegrass. During the period of October 16 thru February 28, protect site by: Option (1) 2 tons per acre of well anchored straw mulch and as soon as possible in the spring. Option (2) Use sod. Option (3) Seed with 60 lbs/ acre Kentucky 31 Tall Fescue and mulch with 2 tons/acre well anchored straw.

Mulching - Apply 1½ to 2 tons per acre (70 to 90 lbs/1000 sq ft) of unrotted small grain straw immediately after seeding. Anchor mulch immediately after application using mulch anchoring tool or 218 gallons per acre (5 gal/1000 sq ft) of emulsified asphalt on flat areas. On slopes 8 feet or higher, use 348 gallons per acre (8 gal/1000 sq ft)

Maintenance - Inspect all seeded areas as make needed repairs, replacements and reseedings.

Apply to graded or cleared areas likely to be redisturbed where a short-term vegetative

Seedbed Preparation: Loosen upper three inches of soil by raking, discing or other acceptable means before seeding.

Soil Amendments: Apply 600 lbs per acre 10-10-10 fertilizer (14 lbs/1000 sq ft) Seeding: For periods March 1 thru April 30 and from August 15 thru Movember 15, seed with 25 bushel per acre of annual rye (3.2 lbs/1000 sq ft). For the period May 1 thru

August 14, seed with 3 lbs per acre of weeping lovegrass (.07 lbs/1000 sq ft). For

the period November 16 thru February 28, protect site by applying 2 tons per acre of

well anchored straw mulch and seed as soon as possible in the spring, or use sod. Mulching: Apply 1% to 2-tons per acre (70 to 90 lbs/1000 sq ft) of unrotted small grain straw immediately after seeding. Anchor mulch immediately after application using mulch anchoring tool or 218 gal per acre (5 gal/1000 sq ft) of emulsified asphalt on flat areas. On slopes, 8 ft or higher, use 348 gal per acre (8 gal/1000 sq ft) for anchoring.

Refer to the 1983 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL for rate and methods not covered.

5'01e. Or 18"x II" C.M.P. (Min.) Shall

' & Rebor 4' Minimum Depth. (Depth To Be Required To Property Decure Pips) Imp Top Of Rebor 2" (Inches).

Hove A Minimum Length Of 80'

SOIL CONSERVATION MEASURES AT SMALL STREAM CRUSSING

DATE

- . 1) A minimum of 24 hours notice must be given to the Howard County Office of Inspection and Permits prior to the start of any construction. (992-2437)
- 2) All vegetative and structural practices are to be installed according to the provisions of this plan and are to be in conformance with the 1983 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND
- Following initial soil disturbance or redisturbance, permanent or temporary stabilization shall be completed within: a) 7 calendar days for all perimeter sediment control structures, dikes, perimeter slopes and all slopes greater than 3:1, b) 14 days as to all other disturbed or graded areas on the project site.
- 4) All sediment traps/basins shown must be fenced and warning signs posted around their perimeter in accordance with Vol. 1, Chaper 12, of the HOMARD COUNTY DESIGN MANUAL, Storm
- 5) All disturbed areas must be stabilized within the time period specified above in accordance with the 1983 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIT EROSION AND SEDIMENT CONTROL for permanent seedings (Sec. 51) sod (Sec. 54), temporary seeding (Sec. 50) and mulching (Sec. 52.) Temporary stabilization with mulch alone can only be done when recommended seeding dates do not allow for proper germination and establishment of grasses.
- 6) All sediment control structures are to remain in place and are to be maintained in operative condition until permission for their removal has been obtained from. the Howard County Sediment Control Inspector
- Total Area of Site Area Disturbed Area to be roofed or paved 8.2 Acres
 Area to be vegetatively stabilized (2.23 Acres
 Total Cut 64,020 Cu. yds
 Total Fill 90899 Cu. yds
- 8) Any sediment control practice which is disturbed by grading activity for placement of utilities must be

SECTION AA

- repaired on the same day of disturbance. 9) Additional sediment controls must be provided, if
- deemed necessary by the Howard County DPW sediment control inspector. 10) On all sites with disturbed areas in excess of 2 acres, approval of the inspection agency shall be requested upon completion of installation of perimeter erosion and sediment controls, but before proceeding with any other earth disturbance or grading. Other building or grading inspection approvals may not be authorized until this initial approval by the inspection agency is made.

1.5 to 2' Rebar or Stakes

SECTION BB

ALL AREAS

1. OBTAIN GRADING PERMIT.

- CLEAR AND GRUB AS REQUIRED FOR INSTALLATION OF SEDIMENT CONTROL

SEQUENCE OF CONSTRUCTION

SILT FENCE

SILT FENCE - Bigh strength polypropylens metting

PERSPECTIVE VIEW

- NOVER WARE POICE DAME IN V2 GALGE, MAX 6" MESH - SPACING)

POSTS: Stepletther I or U

high strength polypropylene netting

TANDARD DRAWN

PENCE: NOVEN HIRE, 19. GA.
6 MAX. PESH OPENING OF

FILTER CLOTH: FILTER X.
RIRAFI HUX, STABILINKA TIMON OR APPROVED
ROMA

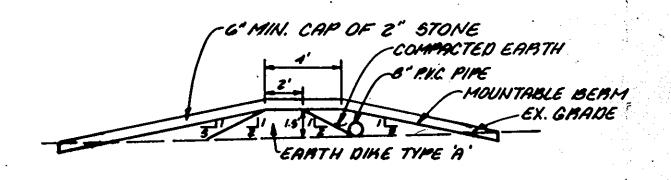
- INSTALL STABILIZED CONSTRUCTION ENTRANCE AS SHOWN AT BOTH ENTRANCES.
- INSTALL TEMPORARY SMALE, EARTH DIKES, SILT FENCES, SEDIMENT TRAPS TEMPORARY S.W.H. FACILITIES (TRAPS #2 AND #3), AND TEMPORARY PIPES.
- INSTALL CULVERT WALLS AND PIPES AS SHOWN ON GAKTON LANE. GRADE OVER CULVERT PIPE AS SHOWN TO PREVENT PONDING BETWEEN CULVERT WALLS.
- 6. CLEAR AND GRUB REMAINDER OF SITE.

NORTHWEST AREA (NORTH OF FALLS RUN RD. - WEST OF EXISTING STREAM)

- CONSTRUCT TIMBER RETAINING WALLS IN CLUB/LEASING OFFICE AREA AND PERHANENT S.W.M. FACILITY NO. 2.
- ROUGH GRADE PROPOSED CLUB/LEASING OFFICE AREA AND ADJACENT OAKTON LANE FROM FALLS RUN ROAD TO PROPOSED PARKING AREA. EARTH DIKES SHOULD BE MOVED TO PROVIDE POSITIVE DRAINAGE TO TEMP. S.W.M. AS GRADES.ARE BROUGHT UP TO FINISHED ELEVATIONS. BEGIN CONSTRUCTION OF CLUB/LEASING OFFICE.
- ROUGH GRADE REMAINDER OF DRAINAGE AREA TO TRAP #2.
- CONSTRUCT STORM DRAINAGE SYSTEM AND UTILITIES. INSTALL INLET BLOCKING UNTIL AREA IS PERMANENTLY STABILIZED.
- 11. CONSTRUCT BUILDINGS AND INSTALL BASE COURSE FOR PAVINGS TO SUBGRADE.
- 12. FINISH GRADING AND INSTALL CURBS, SIDEWALKS (STABILIZE ALL INACTIVE AREAS WITH PERMANENT SEEDING).
- 13. INSTALL FINISH COURSE OF BITUNINOUS PAVING. FINE GRADE AND STABILIZE REMAINING AREAS WITH PERMANENT SEEDING. UNBLOCK ALL INLETS EXCEPT 1-22, 1-33.
- 14. CONSTRUCT TENNIS COURT AREA, REMAINING TIMBER RETAINING WALL AND POOL DECK AREA, AND PERMANENTLY STABILIZE.
- NOTIFY HOWARD COUNTY OFFICE OF INSPECTIONS AND PERMITS FOR FINAL

MORTHEAST AREA (NORTH OF FALLS RUN RD. - EAST OF EXISTING STREAM)

- ROUGH GRADE ENTIRE DRAINAGE AREA TO TRAP #3.
- INSTALL PROPOSED RETAINING WALL ROUGH GRADE BEHIND WALL WHEN
- MOVE EARTH DIKE AS SHOWN SO AS NOT TO INTERFERE WITH CONSTRUCTION. CONSTRUCT PERMANENT S.W.M.
- CONSTRUCT STORM DRAINAGE SYSTEM AND UTILITIES. INSTALL INLET BLOCKING UNTIL AREA IS PERMANENTLY STABILIZED.
- 11. CONSTRUCT BUILDINGS AND INSTALL BASE COURSE FOR PAVINGS TO SUBGRADE.
- FINISH GRADING AND INSTALL CURBS, SIDEWALKS (STABILIZE ALL INACTIVE AREAS WITH PERMANENT SEEDING).
- INSTALL FINISH COURSE OF BITUMINOUS PAVING. FINE GRADE AND STABILIZE REMAINING AREAS WITH PERMANENT SEEDING. UNBLOCK ALL INLETS EXCEPT I-26, I-27, I-31.
- 14. CONSTRUCT REMAINING BLOGS. AND PERMANENTLY STABILIZE
- NOTIFY HOWARD COUNTY OFFICE OF INSPECTIONS AND PERMITS FOR FINAL



MOUNTABLE BERM OVER EARTH DIKE

SOUTH AREA (SOUTH OF FALLS RUN ROAD)

- ROUGH GRADE (STABILIZE ALL INACTIVE AREAS WITHIN 14 DAYS WITH
- 8. INSTALL STORM DRAINAGE AND UTILITIES.
- INSTALL INLET PROTECTION AND PROVIDE EARTH AND ASPHALT BERMS TO DIRECT RUNOFF TO INLETS GOING TO SEDIMENT BASIN.
- CONSTRUCT BUILDINGS AND INSTALL BASE COURSE FOR PAVINGS TO SUBGRADE.
- FINISH GRADING AND INSTALL CURBS, SIDEWALKS (STABILIZE ALL INACTIVE
- AREAS WITH PERMANENT SEEDING).
- REMOVE SEDIMENT CONTROL DEVICES AND INSTALL REMAINING STORM DRAINAGE SYSTEM AND SANITARY SEKER SYSTEM.
- FINISH COURSE OF BITUMINOUS PAVING. FINE GRADE AND STABILIZE REMAINING AREAS WITH PERHANENT SEEDING.
- NOTIFY HOWARD COUNTY OFFICE OF INSPECTION AND PERMITS FOR FINAL

AFFROVED DIVISION of COMMUNITY PLANNING & LAND DEVELOPMENT HOWARD COUNTY.

> MARYLAND 11-7-89

> > DESIGNED BY: E.A.S.

JOB NUMBER: 8215

1216 Prince Street, Alexandria, VA 22314 703/549-778 Landscape Architecture Land Planning



SEDIMENT CONTROL DETAILS

ENGINEER'S CERTIFICATE I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVA

TION DISTRICT.

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DEVELOPER'S CERTIFICATE

REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION DISTRICT AND MEETS TECHNICAL REQUIREMENTS. OIL CONSERVATION SERVE THIS DEVELOPMENT IS APPROVED FOR EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT. APPROVED :

OWARD SOIL CONSERVATION DISTRICT

6 min

PLAN VIEW

CHIEF, DIVISION OF LAND DEVELOPMENT AND ZONING ADMINISTRATION SYSTEMS.

HEALTH DAFICER

HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING 1/10/90 APPROVED : HOWARD COUNTY HEALTH DEPARTMENT FOR PUBLIC WATER AND SEWERAGE

Land to the way

DEWATERING BASIN

NOT TO SCALE

APPROVED: DEPARTMENT OF PUBLIC WORKS FOR PUBLIC WATER AND SEWER AND STORM DRAINAGE SYSTEMS AND ROADS. Measin & Redu CHIEF, BUREAU OF ENGINEERING DATE SECTION/AREA PARCEL SUBDIVISION NAME ASHTON WOODS APARTMENTS 423 BLOCK NO ZONE TAX/ZONE ELECTIST CENSUS SEWER CODE WATER CODE

STV/LYON ASSOCIATES ENGINEERS · SURVEYORS · PLANNERS 21 GOVERNOR'S COURT BALTIMORE, MD 21207 (301 - 944 - 9112)

ASHTON WOODS TAX MAP 31, PARCEL 423, LOT 2 HOWARD COUNTY, MARYLAND

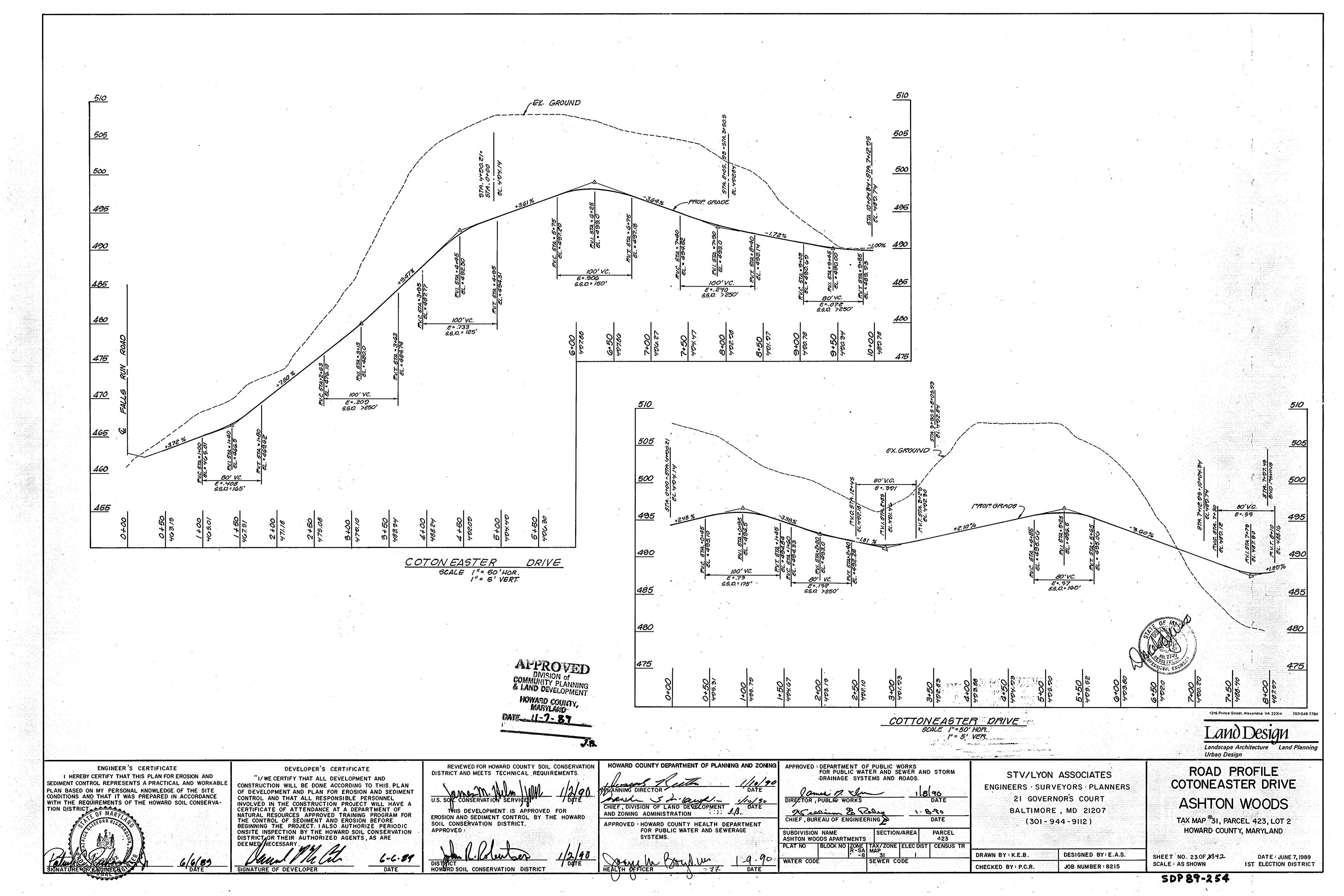
SHEET NO. 22 OF 3342 SCALE : AS SHOWN

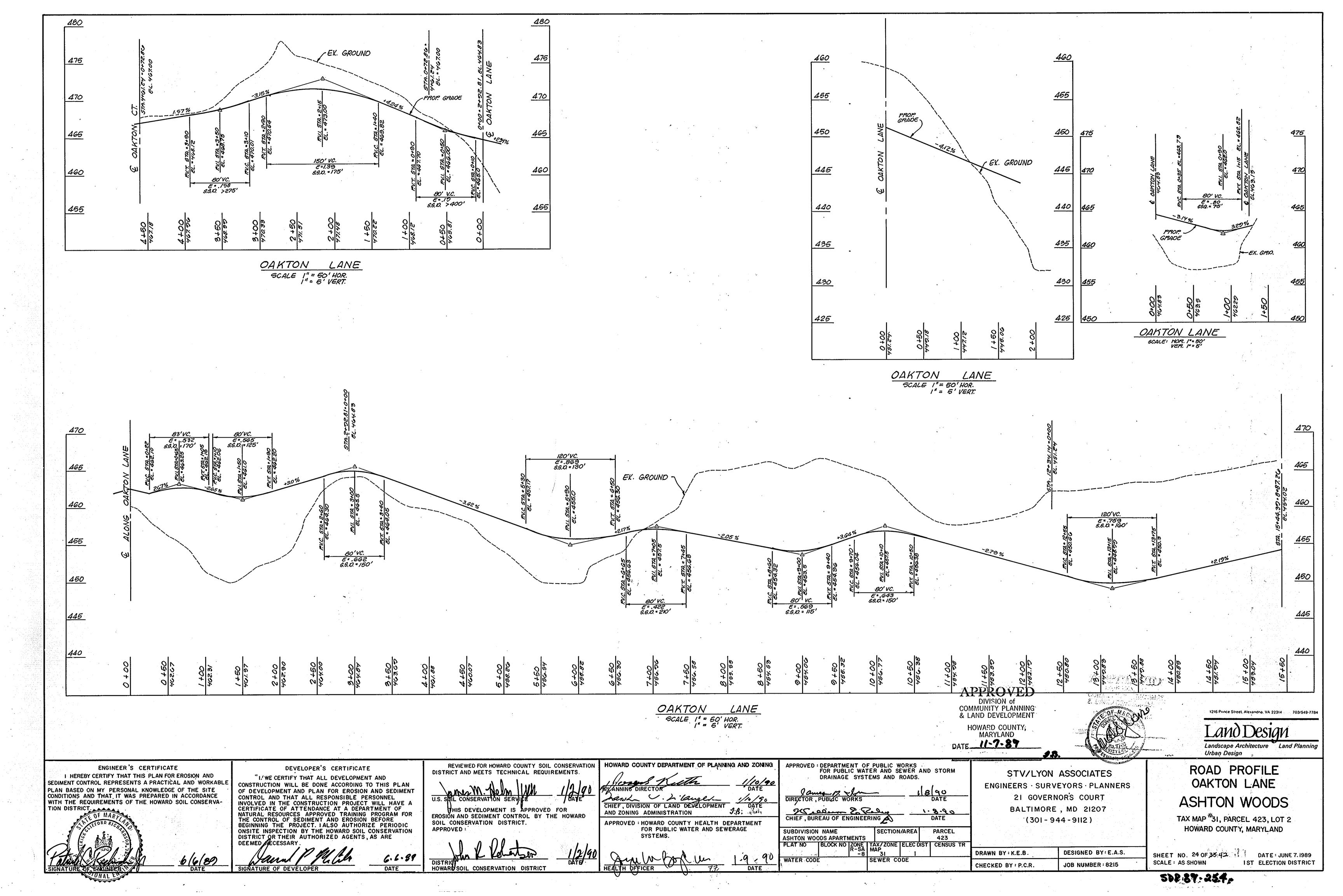
DATE: JUNE 7, 1989 IST ELECTION DISTRICT

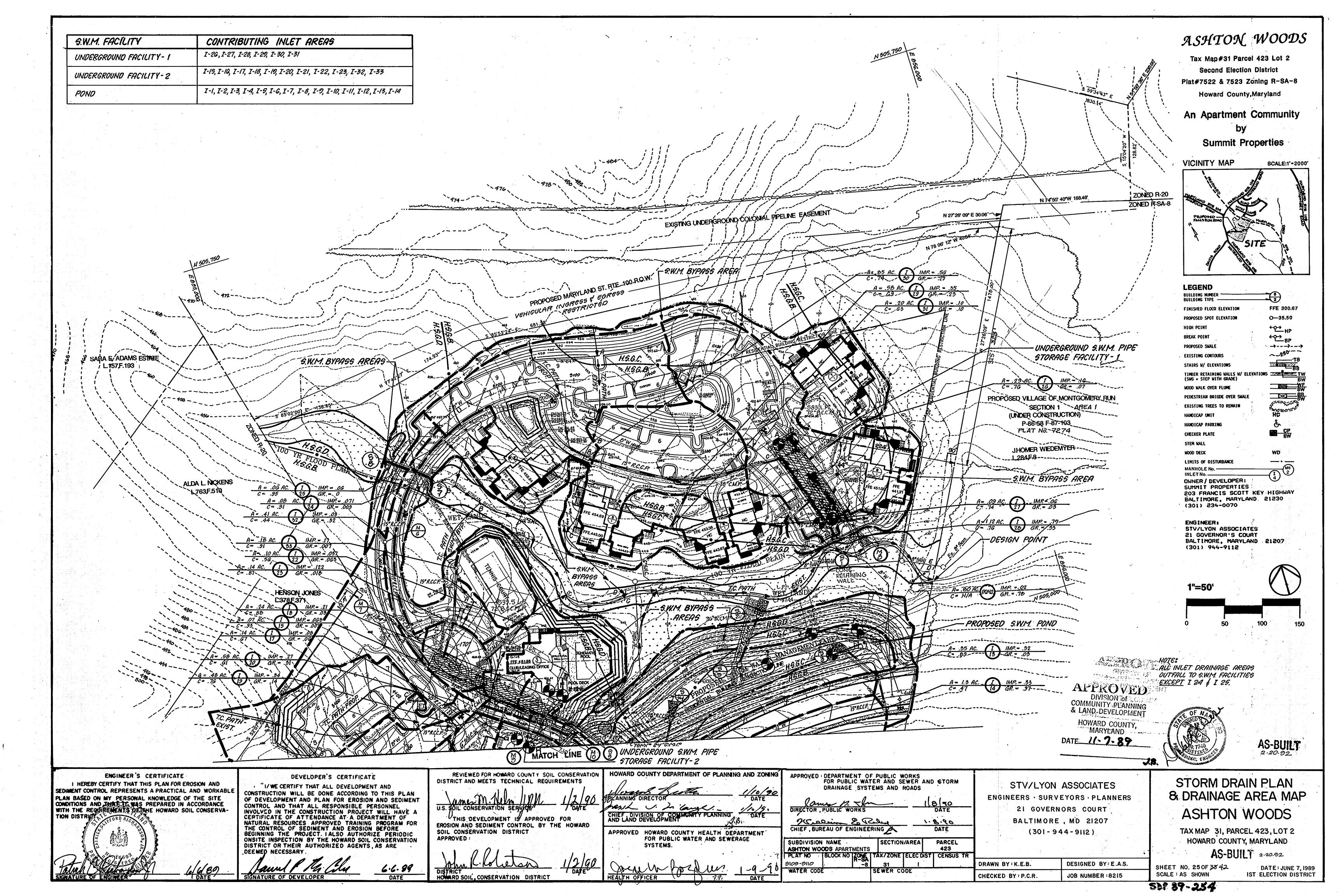
SDP 87-254

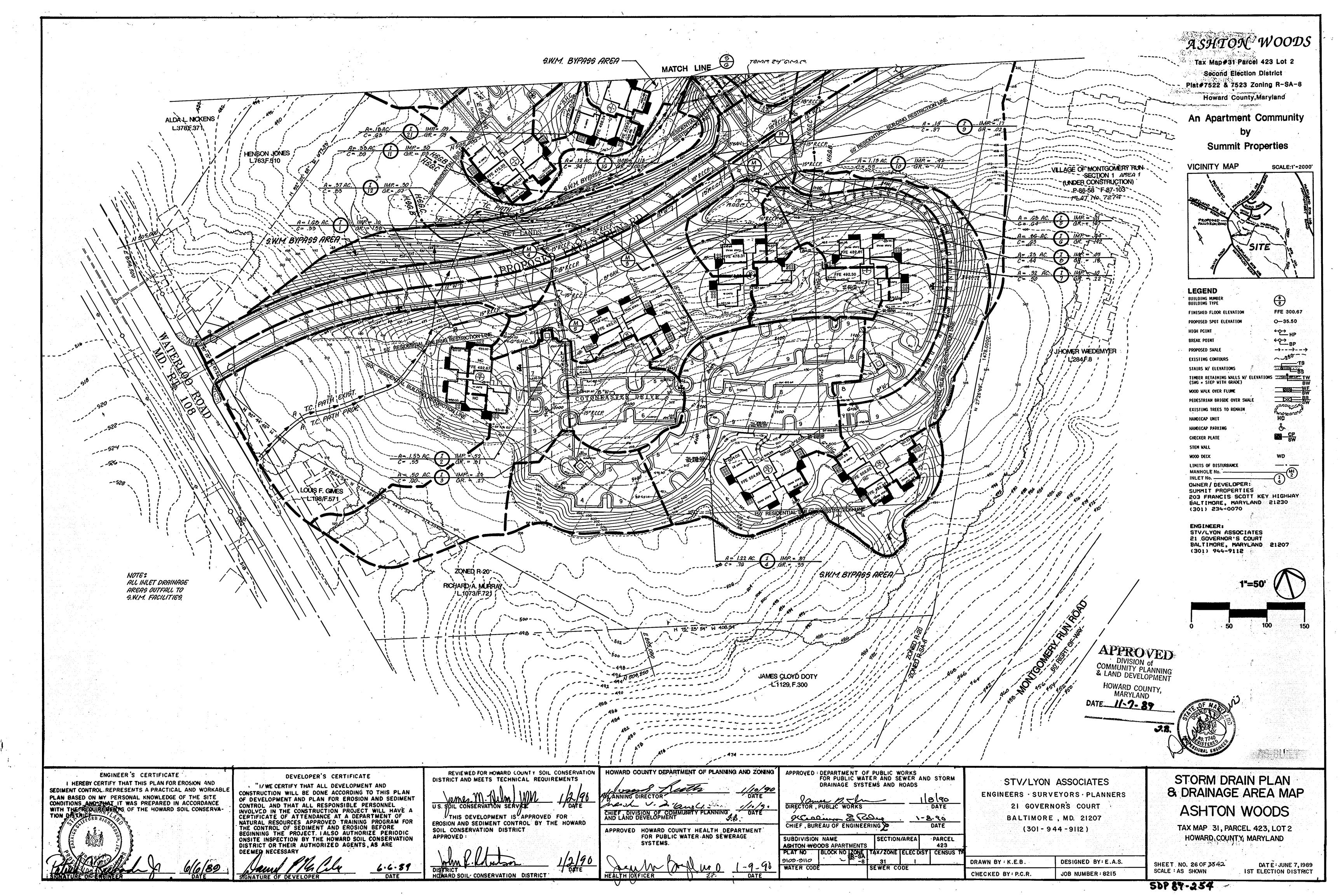
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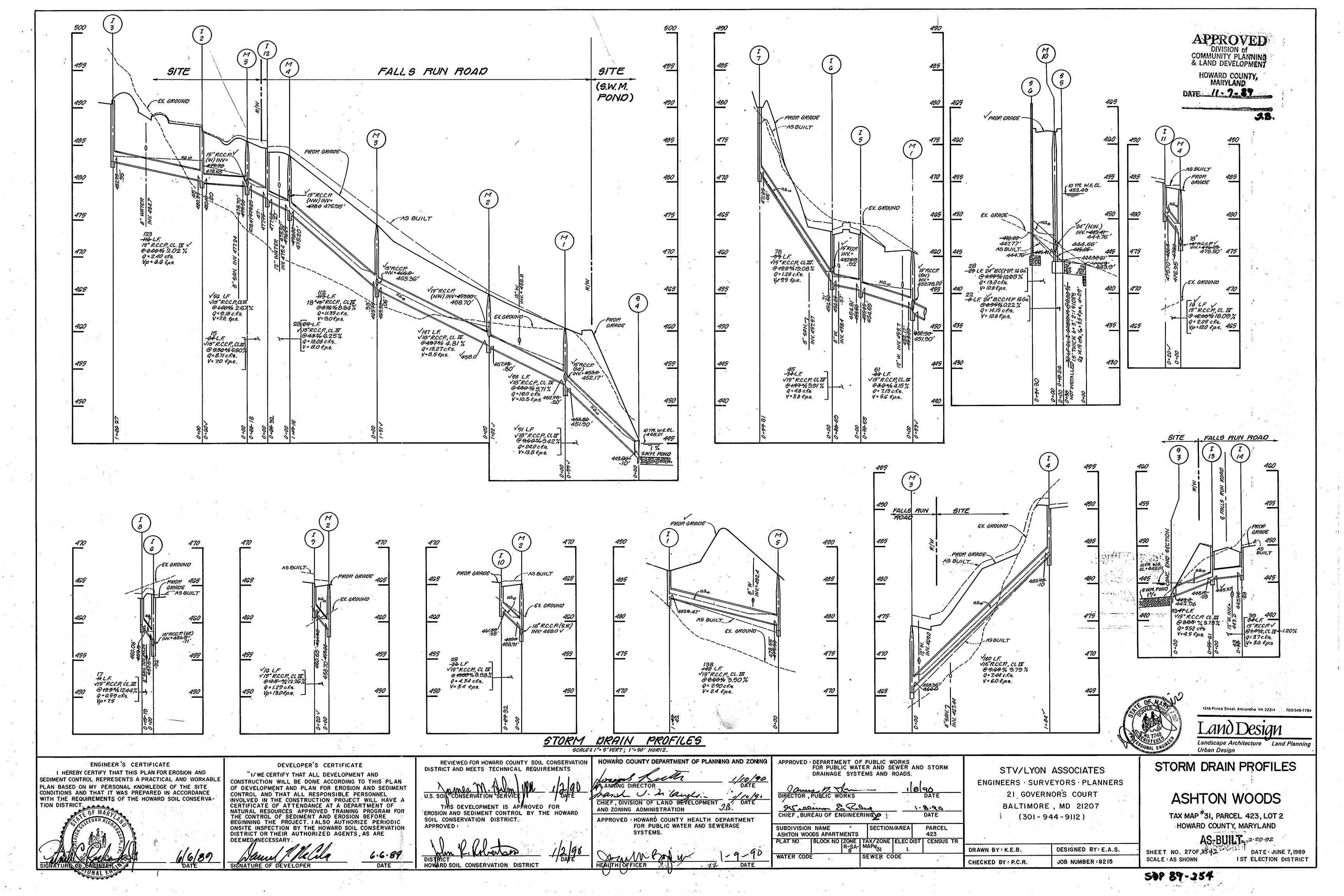
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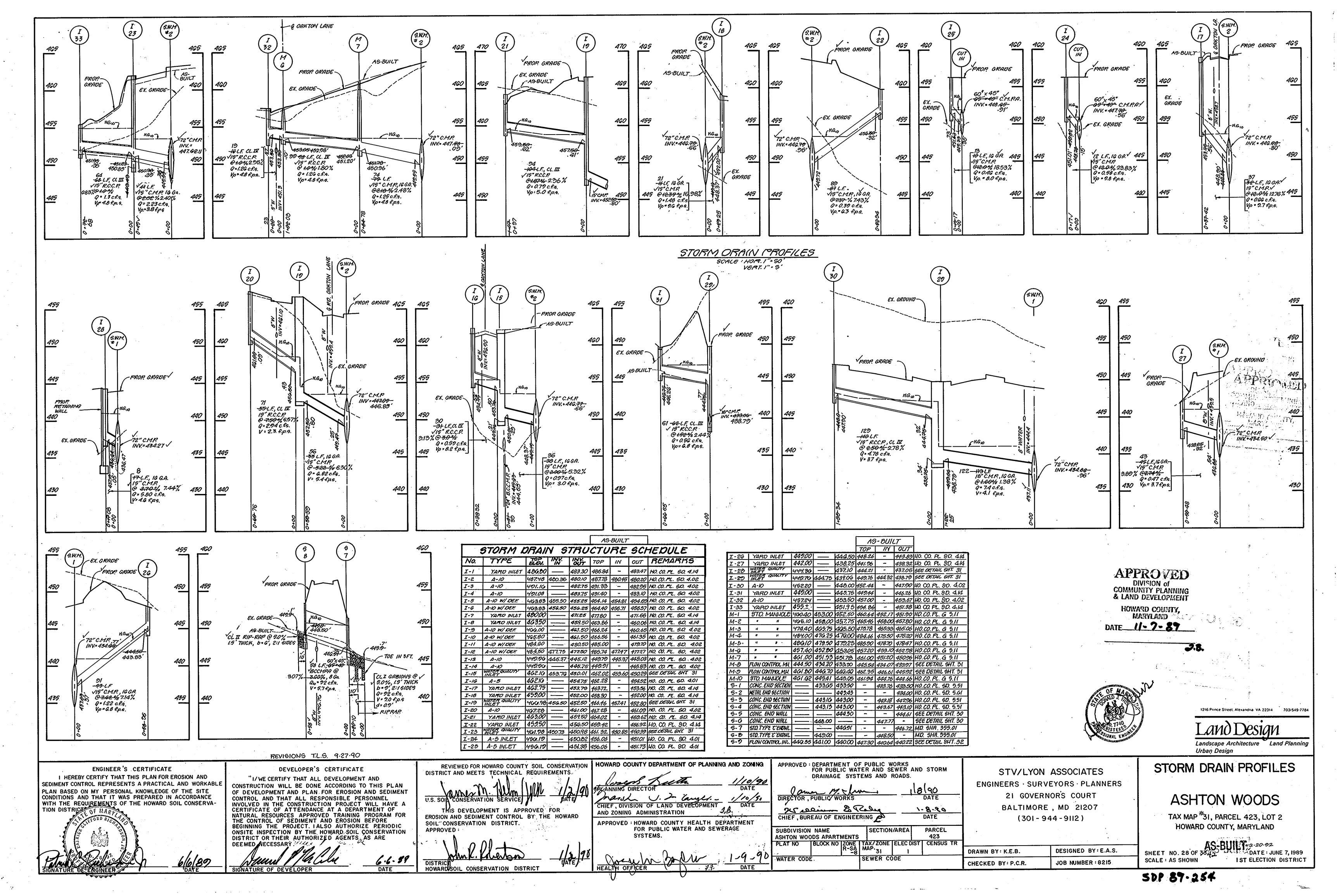


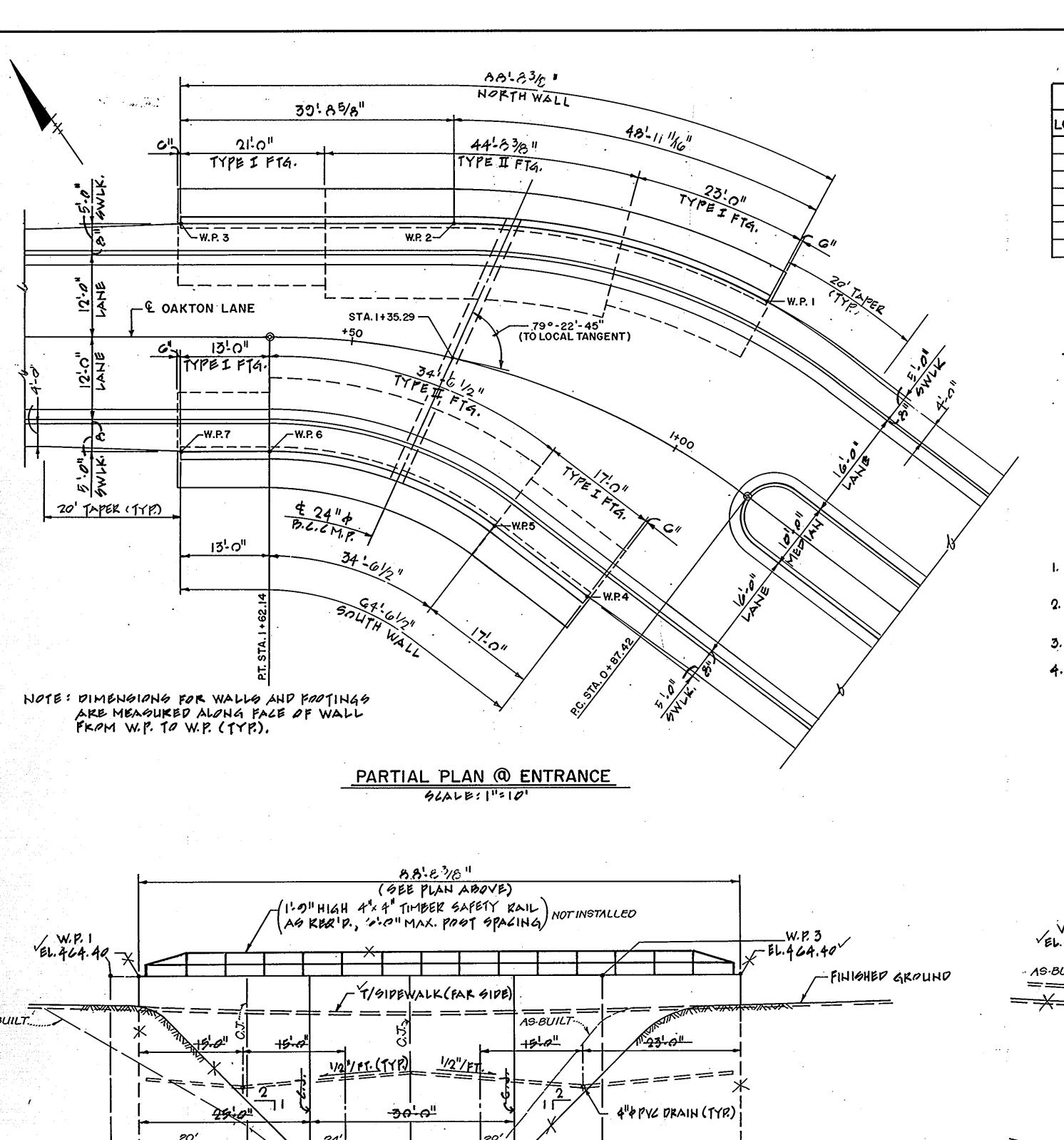


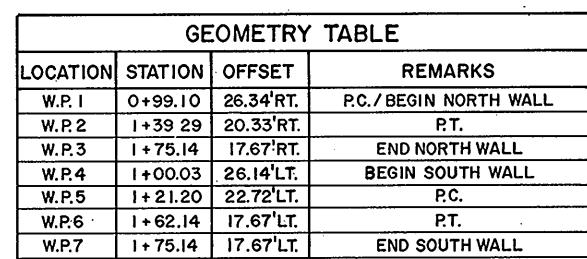










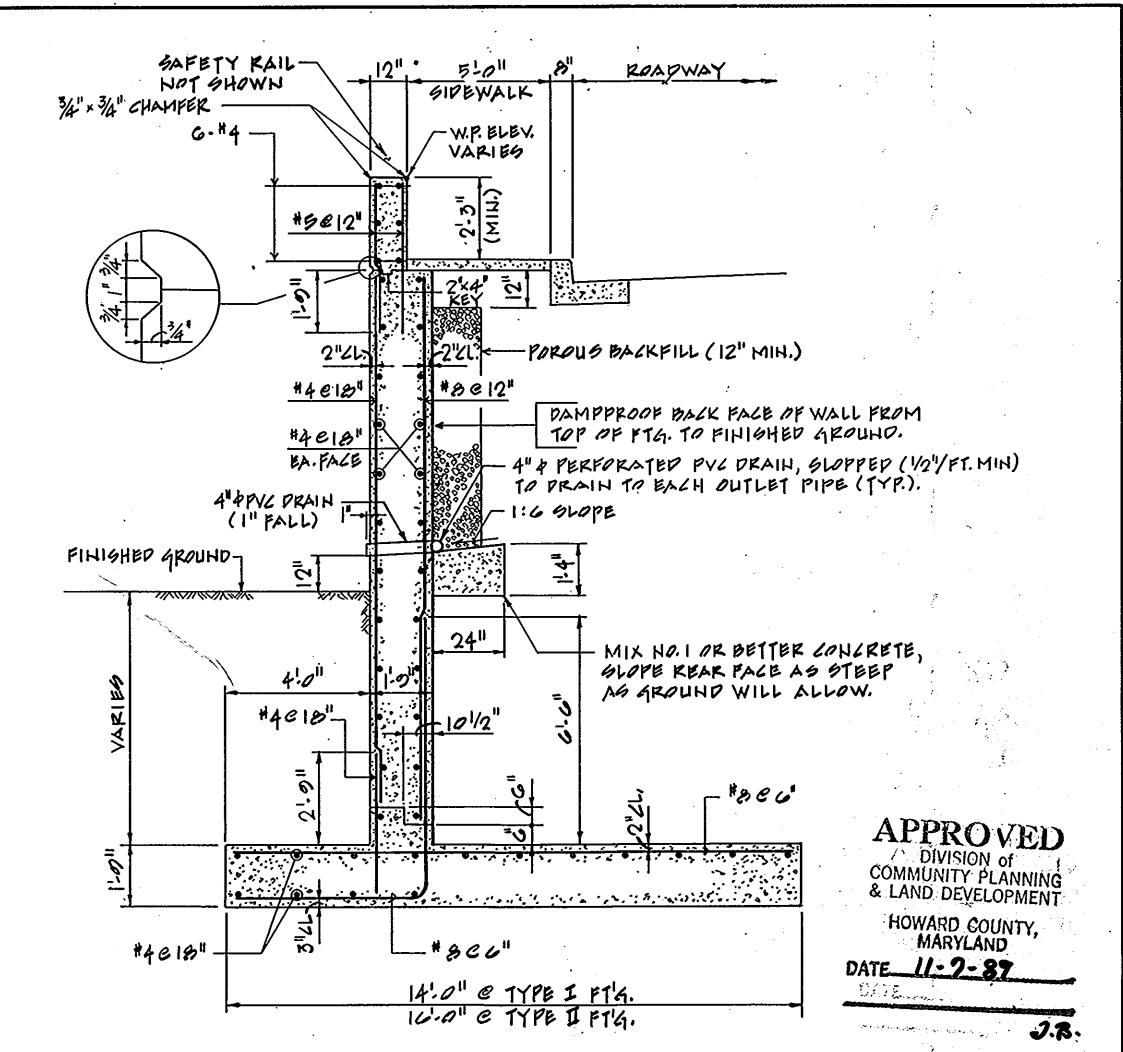


CURVE DATA

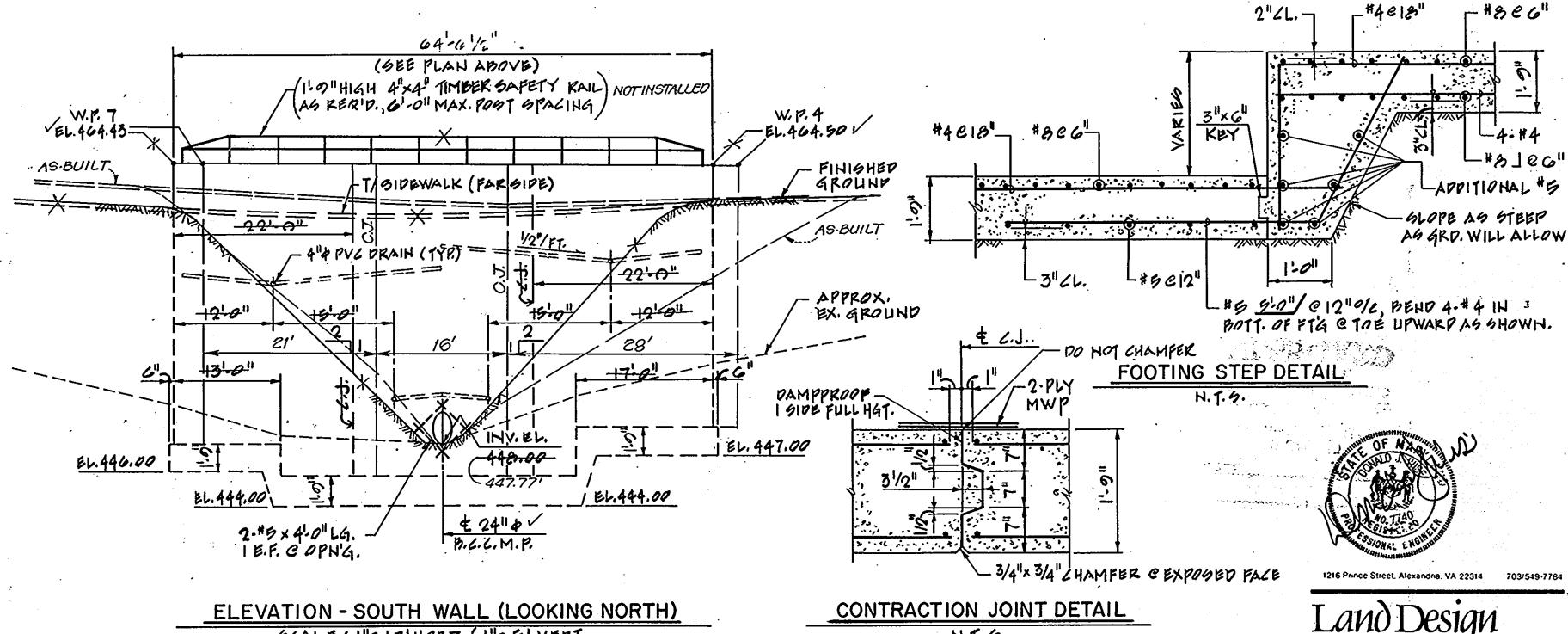
| & OAKTON LANE | NORTH WALL | SOUTH WALL |
|--|--|-------------------|
| $\Delta = 37.^{\circ} - 13^{\circ} - 44^{\circ} LT.$ | $\Delta = 28^{\circ} - 46' - 46'' LT.$ | Δ = 37°-13'-44"L |
| Dc=49°-49'-21" | Dc = 58°-45'-46" | Dc = 107°-46'-24" |
| · R = 115.00' | R = 97.50' | $R = 53.16^{1}$ |
| T = 38.73' | T = 25.02' | T = 17.91' |
| L =74.72' | L = 48.98' | L = 34.54' |
| E = 6.35' | E = 3.16' | E = 2.93' |

GENERAL NOTES

- THE MINIMUM COMPRESSIVE STRENGTH OF REINFORCED LONGRETE & 28 DAYS, FL : 3,000 P.4.1.
- 2. DEFORMED REINFORCING BARG GHALL CONFORM TO AGTM 615, GRADE 60, Fy = 60 K.G.I.
- 3. DEDIGH GOIL BEARING PREGOURE = 3,000 P.G.I.
- 4. THE CONTRACTOR SHALL PREPARE AND SUBMIT SHOP DWGS. FOR REINF. STEEL TO THE ENGINEER, STY/LYDH, FOR REVIEW.



TYPICAL SECTION THRU WALL 96ALE: 3/8": 1:0"



EL.441.00

6.6.89

HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION

1/10/90 DATE CHIEF, DIVISION OF LAND DEVELOPMENT J.B. AND ZONING ADMINISTRATION

APPROVED : HOWARD COUNTY HEALTH DEPARTMENT FOR PUBLIC WATER AND SEWERAGE SUBDIVISION NAME ASHTON WOODS

96ALE: 1" : 10' HORZ. / 1" = 5' VERT.

DRAINAGE SYSTEMS AND ROADS. 18190 Wess & misses 28 1-8-90 CHIEF, BUREAU OF ENGINEERING SECTION/AREA

PLAT NO BLOCK NO ZONE TAX/ZONE ELECTIST CENSUS TR

APPROVED DEPARTMENT OF PUBLIC WORKS
FOR PUBLIC WATER AND SEWER AND STORM

ENGINEERS · SURVEYORS · PLANNERS 21 GOVERNOR'S COURT BALTIMORE, MD 21207 (301 - 944 - 9112)

DESIGNED BY: W.E.S.

STV/LYON ASSOCIATES

N.T. 4.

CULVERT PIPE ELEVATIONS & DETAILS ASHTON WOODS

Urban Design

TAX MAP *31, PARCEL 423, LOT 2 HOWARD COUNTY, MARYLAND

Landscape Architecture Land Planning

AS-BULT 2-20-92 SHEET NO. 29 OF 3542 DATE: DATE: JUNE 7, 1989 IST ELECTION DISTRICT SCALE : AS SHOWN

CONTROL AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF NATURAL RESOURCES APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ONSITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT OR THEIR AUTHORIZED AGENTS, AS ARE DEEMED NECESSARY.

- 2-*5 × 4-0" LG. 1 E.F. € 0PNG.

DEVELOPER'S CERTIFICATE

"I/WE CERTIFY THAT ALL DEVELOPMENT AND

CONTROL AND THAT ALL RESPONSIBLE PERSONNEL

CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN

OF DEVELOPMENT AND PLAN FOR EROSION AND SEDIMENT

ELEVATION - NORTH WALL (LOOKING SOUTH)

SCALE: 1": 10' HOKZ. / 1" = 5' VERT.

23:0

EL. 441.00

EL 444.50

ENGINEER'S CERTIFICATE

I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND

CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE

WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVA-

TION DISTRICT.

SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE

APPROVED:

SOIL CONSERVATION DISTRICT.

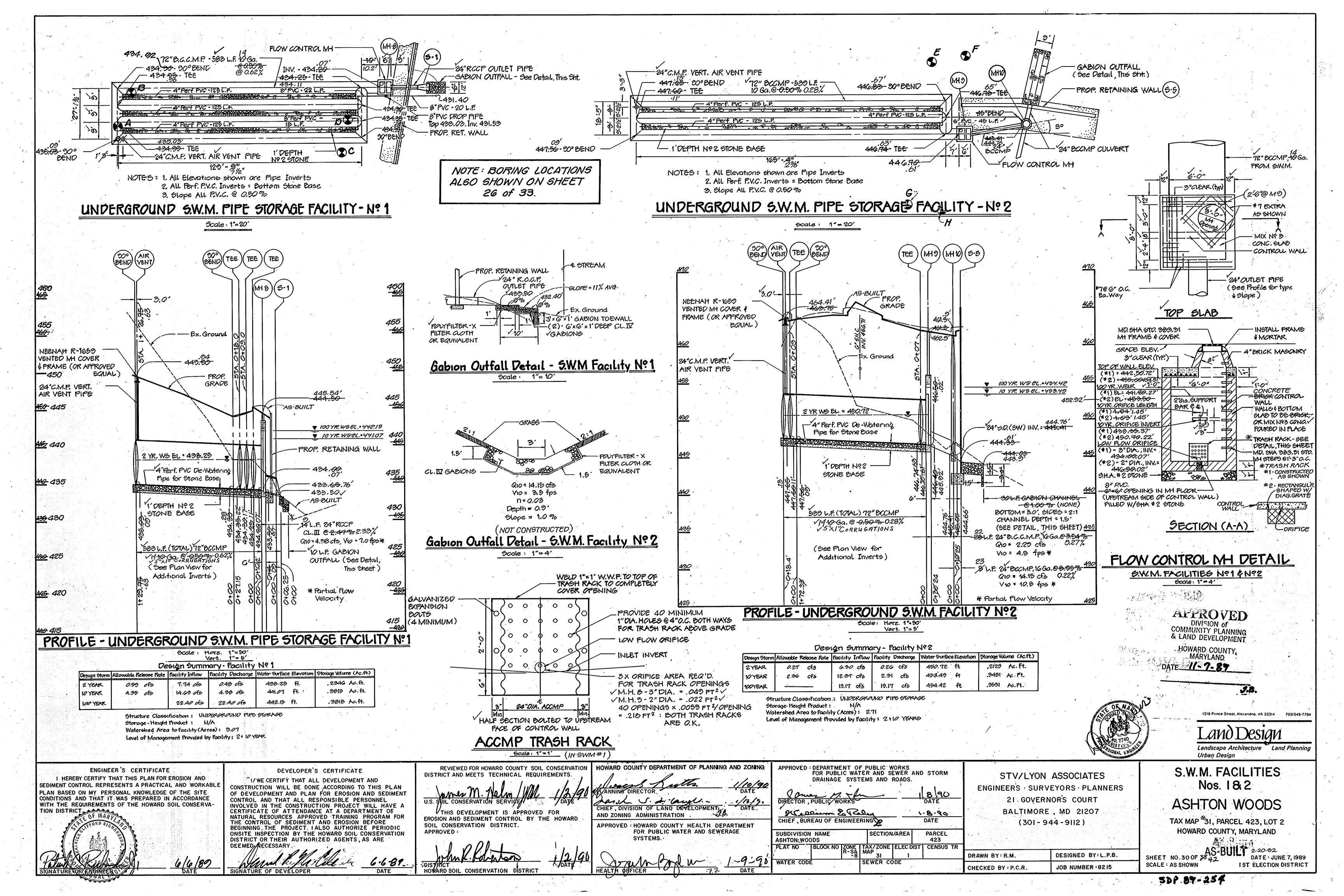
EROSION AND SEDIMENT CONTROL BY THE HOWARD

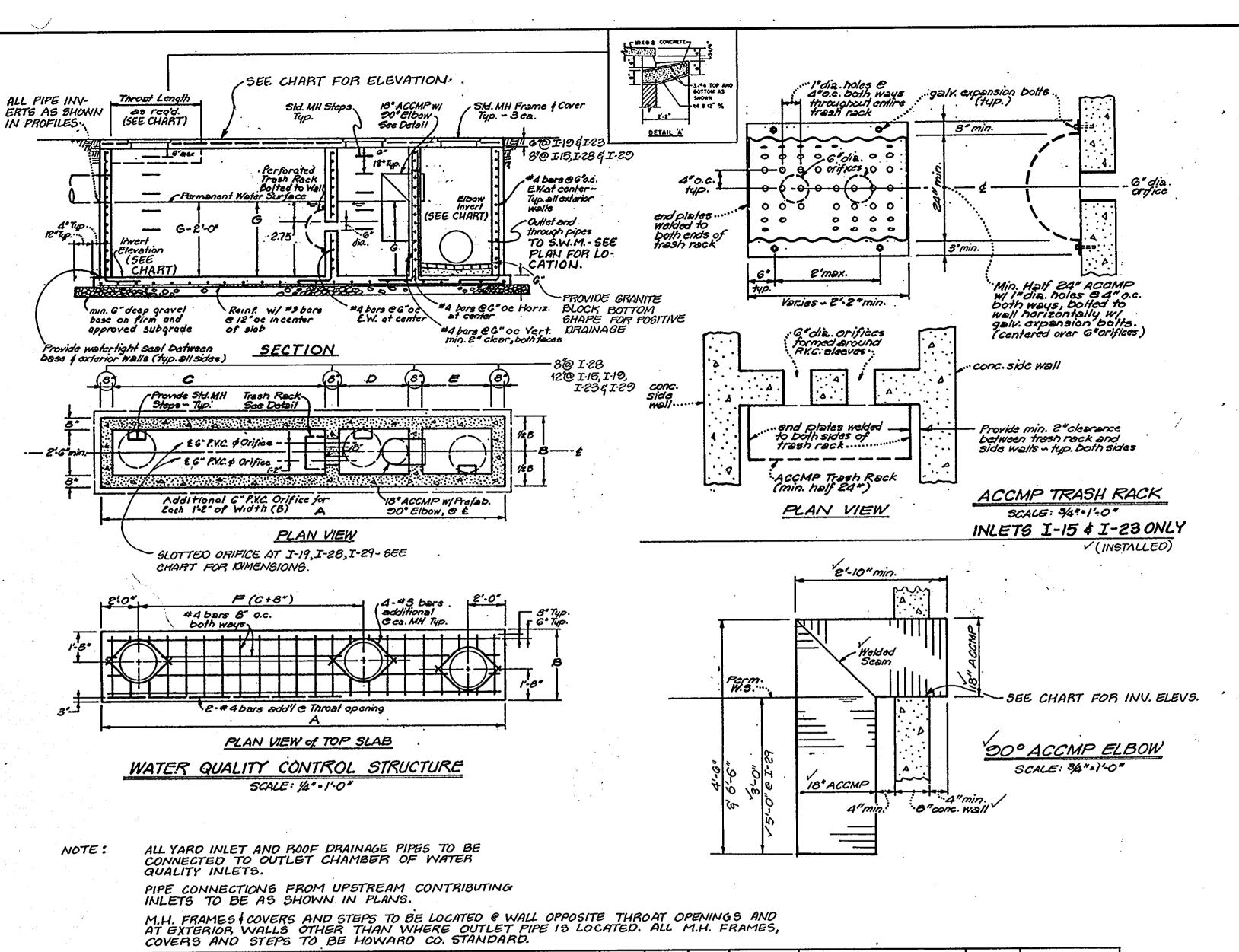
SYSTEMS.

SW 89-254

SEWER CODE WATER CODE CHECKED BY : W.E.S. JOB NUMBER :8215

DRAWN BY : J.D.R.





| | STRUCT DRAIN. VOL. VOL. | | | | , | DIMENSIONS | | | | | | | FLOOR IS" THRO | | ROAT TOP GTH. SLAB IF ELEV. | | HYDRAULIC GRADIENT ELEVATIONS | | | # OF 6" P.V.C. | SLOTTED ORIFICE |
|----|-------------------------|---------------|---------|----------|--------|------------|--------|--------|--------|--------|--------|--------|----------------|--------------|-----------------------------------|--------|-------------------------------------|--------|--------|-------------------|--------------------|
| | NO. | AREA (AC.) | REQ'.D | PROV. | A | В | С | D | E | F | G | ELEV. | INV. | REQ'D | | DN. | INLET | CENTER | | ORIFICES | DIMENSIONS |
| | T-15 | 0.19 | 38 C.F. | 75 C.F. | 16'-8" | 3'-10" | 6'-0" | 4'-0" | 4'-0" | 6'-8" | 4'-6" | 449.26 | 453.76 | + | AG2.16 | 46216 | 454.48 | 454.22 | 453.49 | 2 | |
| | I-19 | | | 120 C.F. | | | 6'-0" | 4'-0" | 4'-0" | 6'-8" | | 452.00 | | | 466.58 | 466.58 | 465.58 | 458.00 | 454.46 | | .6"×15/8" |
| | I-23 | | - | 75 C.F. | | 3'-10" | 6'-0" | 41-0 | 4'-0" | 6'-84 | 4'-6" | 448,99 | 453.49 | , † . | 46229 | 46198 | 454.21 | 453.95 | 453.49 | 2 | |
| | I-28 | 1.12 | **** | 120C.F. | | | 61-0" | 4'-0" | 4' -0" | 6'-8" | 4'-6" | 436,57 | 441.07 | † | 44436 | 444.36 | 443.36 | 442.39 | 441.07 | Number of | 6"×2'-53/8" |
| | I-29 | | | 148 C.F. | | | 71-101 | 41 -0" | 4'-0" | 8'-C" | 6'-6" | 438.25 | 444.75 | Ť, | 449.76 | 449.76 | 448.76 | 445.51 | 441.07 | | 6"×1'-81/2" |
| Г | I-15 | | 38 C.F. | | 18'-0" | 4'-6" | 6'-0" | 4'-0" | 4'-0" | 7'-0" | 4'-6" | 449.27 | 453.80 | 6'-0" | 462.03 | 462.12 | | | | | • |
| | I-19 | | Ī | ICOC.F. | 18'-0" | | 6-0" | 4'-0" | 4'-0" | 7'-0" | 4'-7" | 452.23 | 456.82 | 6'-0" | 466.45 | 466.45 | | | | | |
| | I.23 | | 24 C.F. | | | 4'-5" | 6'-0" | 4'-0" | 4'-0" | 7-0" | 4'-11" | 44940 | 454,32 | 6-0" | 461.44 | 461.60 | | | | | |
| 1 | I·28 | | 92 C.F. | | 16-6" | 5'-4" | 6'-0" | 4'-0" | 4'-0" | 6'-8" | 4'-6" | 436.56 | 441.01 | 6.0 | 444.21 | 444.71 | | • | | | |
| 12 | | | * BOOF | AREAS | AND | SIDEW | ALKS I | JOT IN | CLUDED | IN VOL | UME CO | MPUTAT | MONS. | + LE | NGTH | EQUI | als dim | ENGION | 'C'. | | |

DEVELOPER'S CERTIFICATE

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CONTROL AND THAT ALL RESPONSIBLE PERSONNEL

CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN

OF DEVELOPMENT AND PLAN FOR EROSION AND SEDIMENT

INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE

NATURAL RESOURCES APPROVED TRAINING PROGRAM FOR

BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC

ONSITE INSPECTION BY THE HOWARD SOIL CONSERVATION

CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF

THE CONTROL OF SEDIMENT AND EROSION BEFORE

DISTRICT OR THEIR AUTHORIZED AGENTS, AS ARE

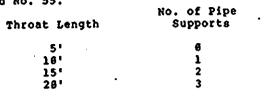
STORAGE REQ'D: 20 C.F. PER O.IAC. OF PAVED AREAS.

FOR STORM DRAIN INVERTS, SEE STORM DRAIN PROFILES. I. 29 | 1.63 | 142 C.F. | 148 C.F. | 19-9" | 4'-6" | 7'-10" | 4'-0" | 4'-0" | 11'-4" | 6'-4" | 438.32 | 444.64 | 7-10" | 449.76 | 449.76

WATER QUALITY STRUCTURES

DESIGN AND GENERAL NOTES:

- 1. Use poured-in place concrete for the entire structure.
- 2. Refer to Maryland State Highway Administration for materials and methods of construction.
- 3. Wall thickness shall be as follows:
- Minimum 8 inches thick for the first 8'-8" of depth, 12 inch thick walls between 8'-6" and 12'-6" of depth and 16 inch thick walls for depth greater than 12'-8". Depth to be measured from top of top slab to crown of outgoing pipe.
- 4. f'c = 3,500 psi at 28 days.
- 5. All reinforcing steel to be ASTM A615, GR.68.
- 6. For details concerning throat openings, refer to MCDOT



pipe supports to be spaced at 5'-0" O.C.

- 7. For details not noted in this standard, refer to MCDOT
- The top 4 inches of walls may be brick masonry for leveling, if required. Brick masonry shall comply with the latest SHA Specification.

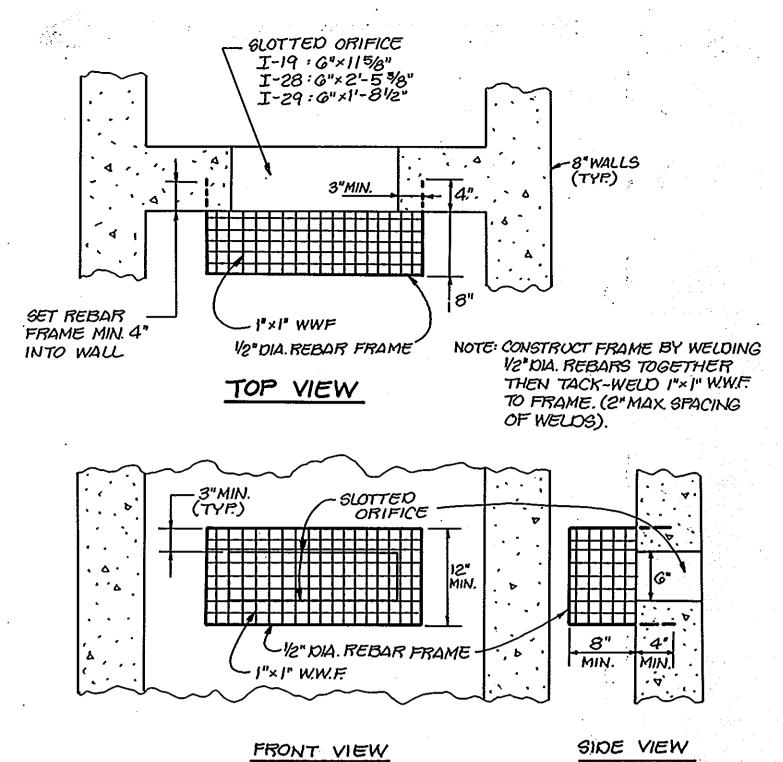
MAINTENANCE NOTES (WATER QUALITY STRUCTURE WASTE)

- 1. Water Quality Structures will require periodical cleaning. Owners of these facilities will have to clean them as needed or on a frequency that the County determines is appropriate. Owners of water quality structures will be notified by the County of the frequency of maintenance.
- 2. Maintenance of these facilities will consist of cleaning out the separator and disposal of the waste and the repair of the facility as needed. Periodic inspections of these facilities will be made by the County Stormwater Management group.
- 3. The disposal of the liquid and solid matter should be as follows:
- A. All liquid material in the separator inlet shall be pumped into a suitable tank truck and disposed of at an approved sanitary district discharge manhole or be taken to an approved sewage treatment plant for discharge.
- B. The solid material shall be landfilled in an approved sanitary landfill.
- The inlet pipes, trash racks, grates, and structural parts shall be repaired as needed.

CONSTRUCTION NOTES:

- 1. Silt and debris shall not be allowed to enter the structures until contributing drainage areas have been permanently stabilized.
- 2. All openings to structures shall be protected with the appropriate sediment control measures during construc-

- . 1. Prior to start of construction on water quality strucinspector must be called 48 hours in advance at
 - . inspector must be notified
 - A. Approval of subgrade for footings.
 - B. Pooting formed and steel set prior to pouring.
 - C. Structure sides formed and steel set prior to
 - D. Prior to top slab and manholes being set, inspector must check structure and all debris and silt in structure removed.
 - E. When site is permanently stabilized and sediment control measures to protect inlet are to be



TRASH RACK FOR I-19, I-28 \$ I-29

1216 Prince Street, Alexandria, VA-22314 703/549-778

Landscape Architecture 🕺 Land Plannin Urban Design

WATER QUALITY INLETS

ASHTON WOODS

TAX MAP 31, PARCEL 423, LOT 2 HOWARD COUNTY, MARYLAND

AS-BUILT 2.20.92 SHEET NO. 31 OF 3842.

DATE: NOV. 29,1989 IST ELECTION DISTRICT

REVISIONS T.L.S. 9.27.90

REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION DISTRICT AND MEETS TECHNICAL REQUIREMENTS.

EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.

APPROVED:

AND ZONING ADMINISTRATION APPROVED : HOWARD COUNTY HEALTH DEPARTMENT FOR PUBLIC WATER AND SEWERAGE SYSTEMS.

HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

1-8.60 DATE CHIEF, BUREAU OF ENGINEERING PARCEL SUBDIVISION NAME 423 'ASHTON WOODS APARTMENTS ONE TAX/ZONE ELEC DIST CENSUS TR PLAT NO BLOCK NO ZONE

SEWER CODE

APPROVED : DEPARTMENT OF PUBLIC WORKS
FOR PUBLIC WATER AND SEWER AND STORM

DRAINAGE SYSTEMS AND ROADS.

(301 - 944 - 9112)DRAWN BY : E.A.S./L.P.B. DESIGNED BY: L.P.B.

STV/LYON ASSOCIATES

ENGINEERS · SURVEYORS · PLANNERS

21 GOVERNOR'S COURT

BALTIMORE, MD 21207

SCALE : AS SHOWN

ENGINEER'S CERTIFICATE

I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND

SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE

PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE

CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE

TION DISTRICT.

WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVA-

DEEMED NECESSARY.

JOB NUMBER: 8215 CHECKED BY: L.P.B./C.V.S.

3DP 87-254

SOIL CONSERVATION SERVICE MARYLAND CONSTRUCTION SPECIFICATIONS

These specifications are appropriate to ponds within the scope of the Standard for practice 378.

I. SITE PREPARATION

Areas designated for borrow areas, embankment, and structural works shall 'be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1.

Areas to be covered by the pond or reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush and stumps shall be cut approximately level with the ground surface.

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

II. EARTH FILL

Material

The fill material shall be taken from approved designated borrow area or areas. It shall be free of roots, stumps, wood, rubbish, oversize stones, frozen or other objectionable materials. The embankment shall be constructed to an elevation which provides for anticipated settlement to the design elevation. The fill height all along the length of the embankment shall be increased above the design elevation (including freeboard) as shown on the plans.

Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in 8-inch maximum thickness (before compaction) layers which are to be continuous over the entire Tength of the fill. The most porous borrow material shall be placed in the downstream portions of the embankment.

Compaction

The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of the equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction can be obtained with the equipment used.

Where a minimum required density is specified, each layer of fill shall be compacted as necessary to obtain that density and is to be certified by the Engineer.

Cutoff Trench

Where specified, a cutoff trench shall be excavated along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be as shown on the drawings, with the minimum width being four feet. The depth shall be at least four feet on as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill material for the cutoff trench shall be the most impervious material available and shall be compacted with equipment or rollers to assure maximum density and minimum permeability.

III. STRUCTURAL BACKFILL

Backfill material shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe unless there is a compacted fill of twenty-four inches or greater over the structure or pipe.

IV. PIPE CONDUITS

All pipes shall be circular in cross section.

A. Corrugated Metal Pipe

1. Materials - (Steel Pipe) - This pipe and its appurtenances shall be galvanized and fully bituminous coated and shall conform to the requirements of AASHTO Specification M-190 Type A with watertight coupling bands. Any bituminous coating damaged or otherwise removed shall be replaced with cold applied bituminous coating

Steel pipes with polymeric coatings shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. The following coatings are commercially available: Nexon, Plasti-Cote, Blac-Klad, and Beth-Cu-Loy. Coated corrugated steel pipe shall meet the requirements of AASHTO M-245 and M-246.

Materials - (Aluminized Steel Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-274-791 with watertight coupling bands or flanges.

Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-196 or M-211 with watertight coupling bands or flanges. Coupling bands, anti-seep collars, end sections, etc. must be composed of the same material as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at least 24 mils in thickness. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be less than 9 and greater than 4.

- 2. Connections All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Watertight coupling bands or flanges shall be used at all joints. Anti-seep collars shall be connected to the pipe in such a manner as to the completely watertight. Dimple bands are not considered to be watertight.
- 3. Bedding The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.
- 4. Laying pipe The pipe shall be placed with inside circumferential laps pointing downstream and with the longitudinal laps at the
- 5. Backfilling shall conform to structural backfill as shown above.
- 6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

B. Reinforced Concrete Pipe

- 1. Materials Reinforced concrete pipe shall have a rubber gasket joint and shall equal or exceed ASTM Specification C-361. An approved equivalent is AWWA Specification C-301.
- 2. Bedding All reinforced concrete pipe conduits shall be laid in a concrete bedding for their entire length. this bedding shall consist of high slump concrete plac. Under the pipe and up the sides of the pipe at least 10% of its outage diameter with a
- minimum thickness of 3", or as shown on the drawings.
- 3. Laying pipe Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe.
- 4. Backfilling shall conform to structural backfill as shown above.
- 5. Other details (anti-seep collars, valves, etc.) shall be as shown
- C. For pipes of other materials, specific specifications shall be shown on the drawings.

CONCRETE

1. Materials

- a. Cement Normal Portland cement shall conform to the latest ASTM Specification C-150.
- b. Water The water used in concrete shall be clean, free from oil, acid, alkali, scales, organic matter or other objectionable
- c. Sand The sand used in concrete shall be clean, hard, strong and durable, and shall be well graded with 100 percent passing a one-quarter inch sieve. Limestone sand shall not be used.
- d. Coarse Aggregate The coarse aggregate shall be clean, hard, strong and durable, and free from clay or dirt. It shall be well graded with a maximum size of one and one-half (1-1/2) inches.
- e. Reinforcing Steel The reinforcing steel shall be deformed bars of intermediate grade billet steel or rail steel conforming to ASTM Specification A-615.
- 2. Design Mix The concrete shall be mixed in the following proportions, measured by weight. The water-cement ratio shall be 5-1/2 to 6 U.S. Gallons of water per 94 pound bag of cement. The proportion of materials for the trial mix shall be 1:2:3-1/2. The combination of aggregates may be adjusted to produce a plastic and workable mix that will not produce harshness in placing or honeycombing in the structure.
- 3. Mixing The concrete ingredients shall be mixed in batch mixers until the mixture is homogeneous and of uniform consistency. The mixing of each batch shall continue for not less than one and one-half minutes after all the ingredients, except the full amount of water, are in the mixer. The minimum mixing time is predicted on proper control of the

speed of rotation of the mixer and of the introduction of the materials, including water, into the mixer. Water shall be added prior to, during, and following the mixer-charging operations. Excessive overmixing requiring the addition of water to preserve the required concrete consistency shall not be permitted. Truck mixing will be allowed provided that the use of this method shall cause no violation of any applicable provisions of the specifications given

4. Forms - The forms shall have sufficient strength and rigidity to hold the concrete and to withstand the necessary pressure, tamping, and vibration without deflection from the prescribed lines. They shall be mortar-tight and constructed so that they can be removed without hammering or prying against the concrete.

The inside of forms shall be oiled with a non-staining mineral oil or thoroughly wetted before-concrete is placed.

Forms may be removed 24 hours after the placement of concrete. All wire ties and other devices used shall be recessed from the surface of

- 5. Reinforcing Steel All reinforcing material shall be free of dirt, rust, scale, oil, paint or any other coatings. The steel shall be accurately placed and securely tied and blocked into position so that no movement of the steel will occur during placement of concrete.
- 6. Consolidating Concrete shall be consolidated with internal type mechanical vibrators. Vibration shall be suplemented by spading and hand tamping as necessary to insure smooth and dense concrete along form surfaces, in corners, and around embedded items.
- 7. Pinishing Defective concrete, honeycombed areas, voids left by the removal of tie rods, ridges on all concrete surfaces permanently exposed to view or exposed to water on the finished structure, shall be repaired immediately after the removal of forms. All voids shall be reamed and completely filled with dry-patching mortar.
- 8. Protection and Curing Exposed surfaces of concrete shall be . protected from the direct rays of the sun for at least the first three (3) days. All concrete shall be kept continuously moist for at least ten (10) days after being placed. Moisture may be applied by spraying or sprinkling as necessary to prevent the concrete from drying. Concrete shall not be exposed to freezing during the curing period. Curing compunds may also be used.
- 9. Placing Temperature Concrete may not be placed at temperatures below . 370 F with the temperature falling, or 340 with the temperature

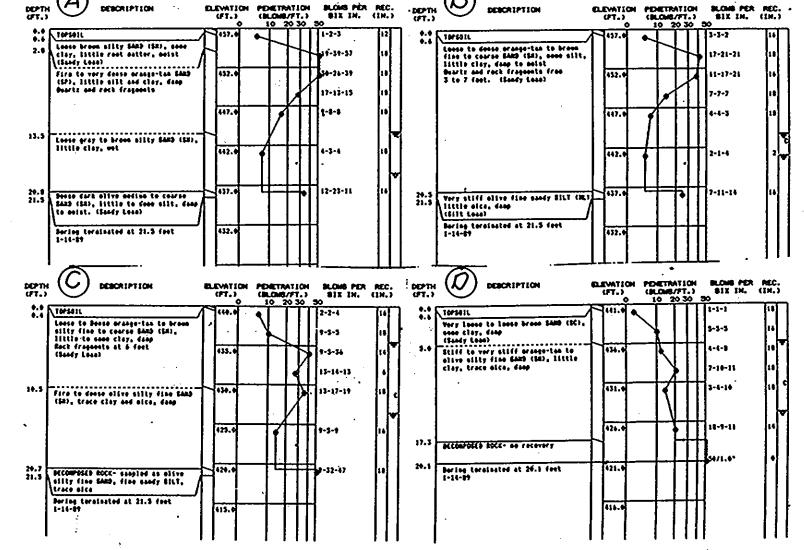
VI. STABILIZATION

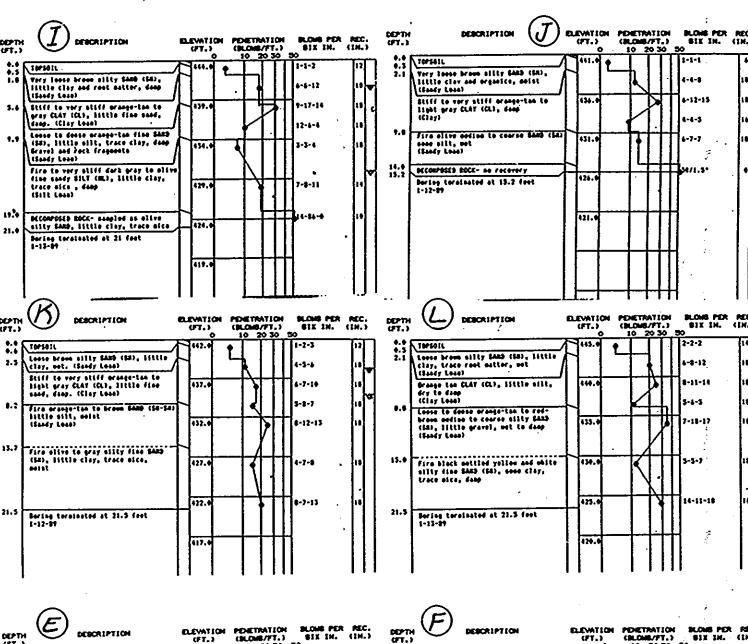
All borrow areas shall be graded to provide proper drainage and left in a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching (if required) in accordance with the vegetative treatment specifications or as shown on the accompanying drawings.

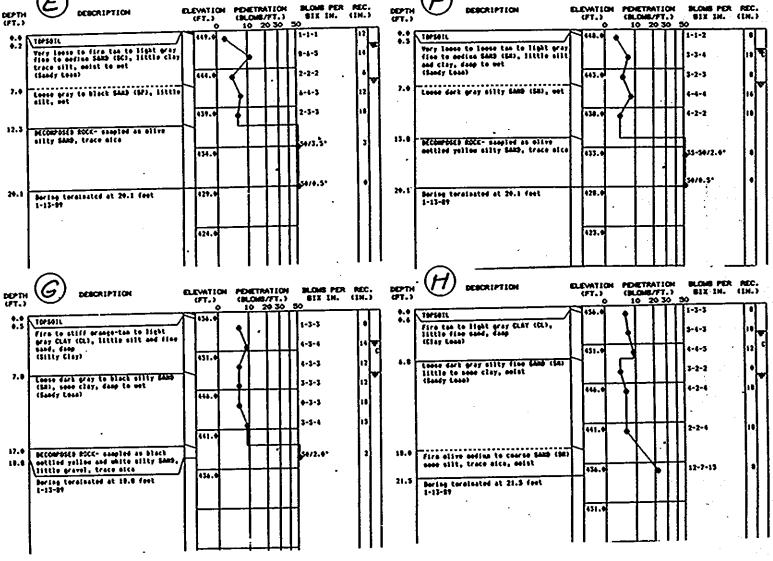
VII. EROSION AND SEDIMENT CONTROL

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures to be employed during the construction process.

TEST BORING RECORD







(A): BORING ID. - SEE S.W.M. PLAN FOR LOCATION.

NOTE: SEE STORMWATER MANAGEMENT COMPUTATIONS FOR LABORATORY TESTS PERFORMED ON THE ABOVE BORINGS.

> COMMUNITY PLANNING & LAND DEVELOPMENT HOWARD COUNTY. MARYLAND



STV/LYON ASSOCIATES

ENGINEERS · SURVEYORS · PLANNERS

21_GOVERNORS COURT

BALTIMORE, MD 21207

(301 - 944 - 9112)

DRAWN BY : L.P.B.

CHECKED BY: P.C.R.

DESIGNED BY : L.P. B.

JOB NUMBER:8215

216 Prince Street, Alexandria, VA 22314 703/549-7784

and Design Landscape Architecture Land Planning

Urban Design S.W.M. NOTES AND **BORING LOGS**

ASHTON WOODS

TAX MAP *31, PARCEL 423, LOT 2

HOWARD COUNTY, MARYLAND

SHEET NO. 3 20F 38 42 SCALE : AS SHOWN

DATE: JUNE 7, 1989 IST ELECTION DISTRICT

I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVA-

ENGINEER'S CERTIFICATE

DEVELOPER'S CERTIFICATE

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REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION DISTRICT AND MEETS TECHNICAL REQUIREMENTS.

THIS DEVELOPMENT IS APPROVED FOR EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.

APPROVED :

AND ZONING ADMINISTRATION APPROVED : HOWARD COUNTY HEALTH DEPARTMENT FOR PUBLIC WATER AND SEWERAGE

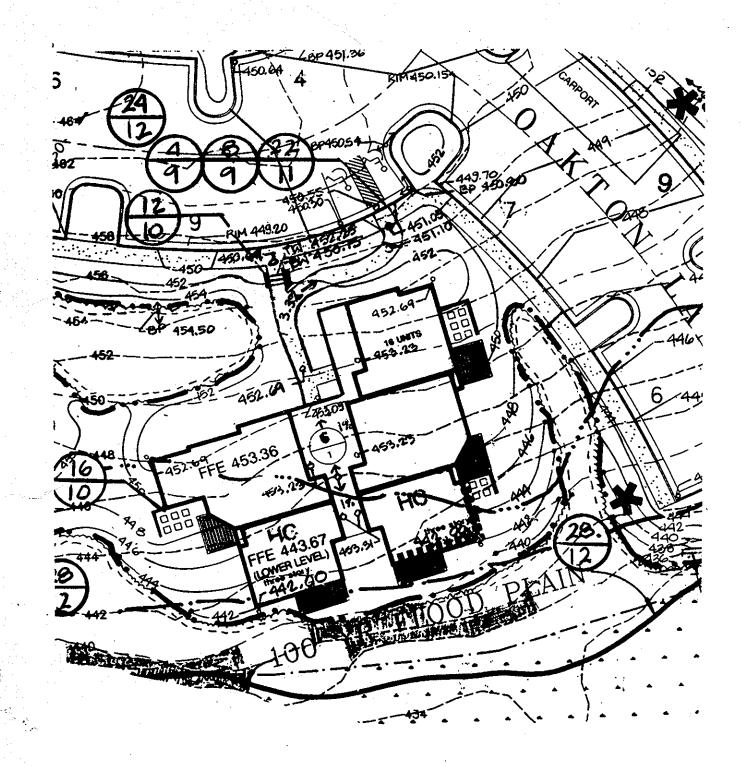
HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

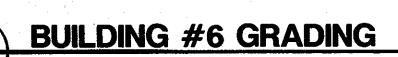
CHIEF, BUREAU OF ENGINEERING SECTION/AREA PARCEL SUBDIVISION NAME 423 ASHTON WOODS PLAT NO BLOCK NO ZONE TAX/ZONE ELECTIST CENSUS TR SEWER CODE WATER CODE

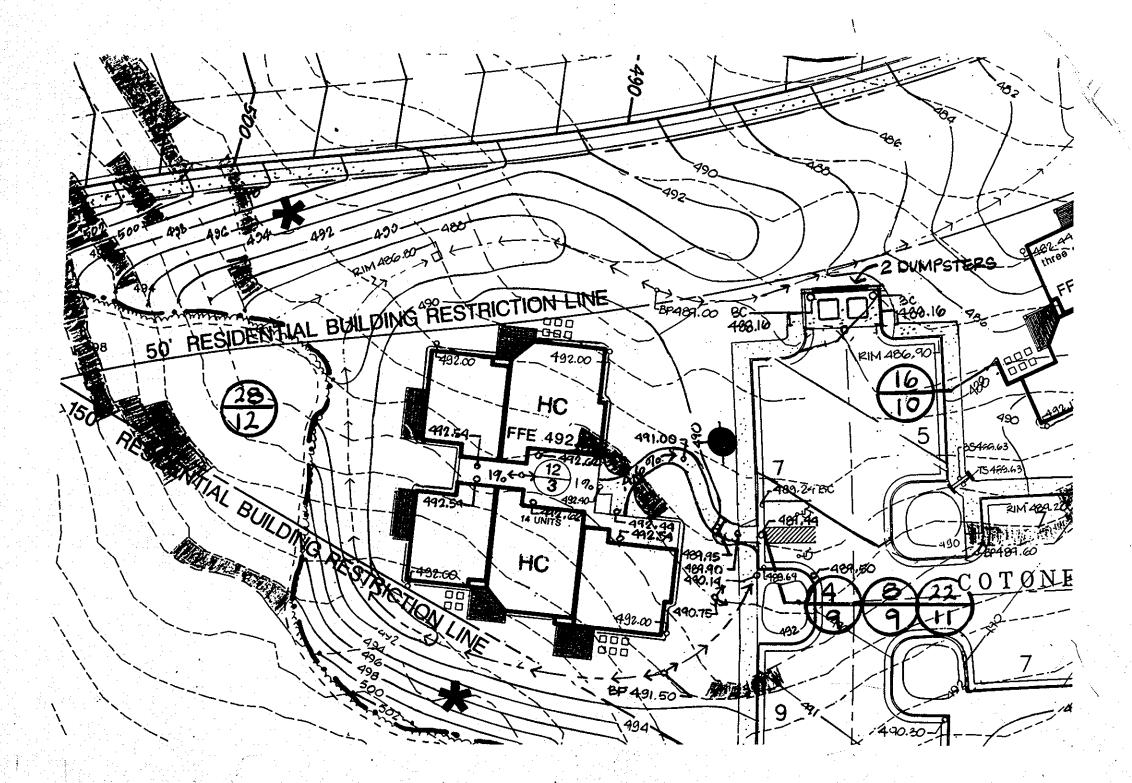
APPROVED : DEPARTMENT OF PUBLIC WORKS FOR PUBLIC WATER AND SEWER AND STORM

DRAINAGE SYSTEMS AND ROADS.

5DP 87-254

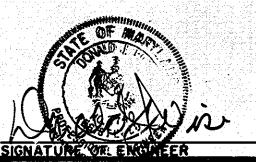






BUILDING #12 GRADING

ENGINEER'S CERTIFICATE I HEREBY CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.



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DEFMED NECESSARY

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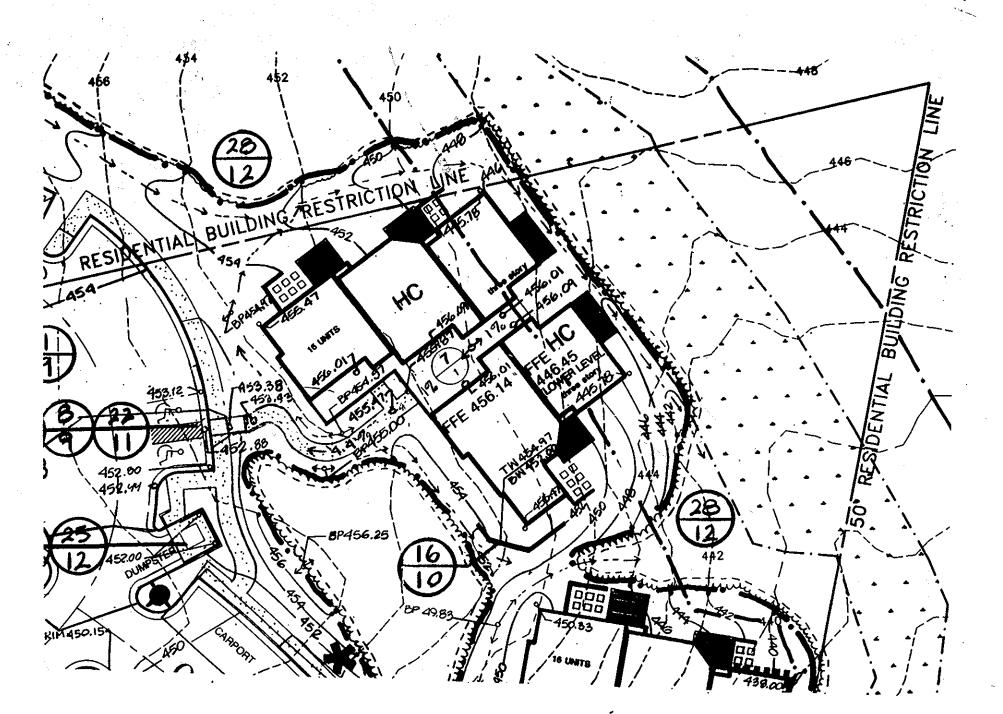
CHOWARD SOIL CONSERVATION DISTRICT

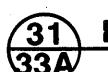
REVIEWED FOR HOWARD COUNTY SOIL CONSERVATION DISTRICT AND MEETS TECHNICAL REQUIREMENTS

THIS DEVELOPMENT IS APPROVED FOR EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT APPROVED :

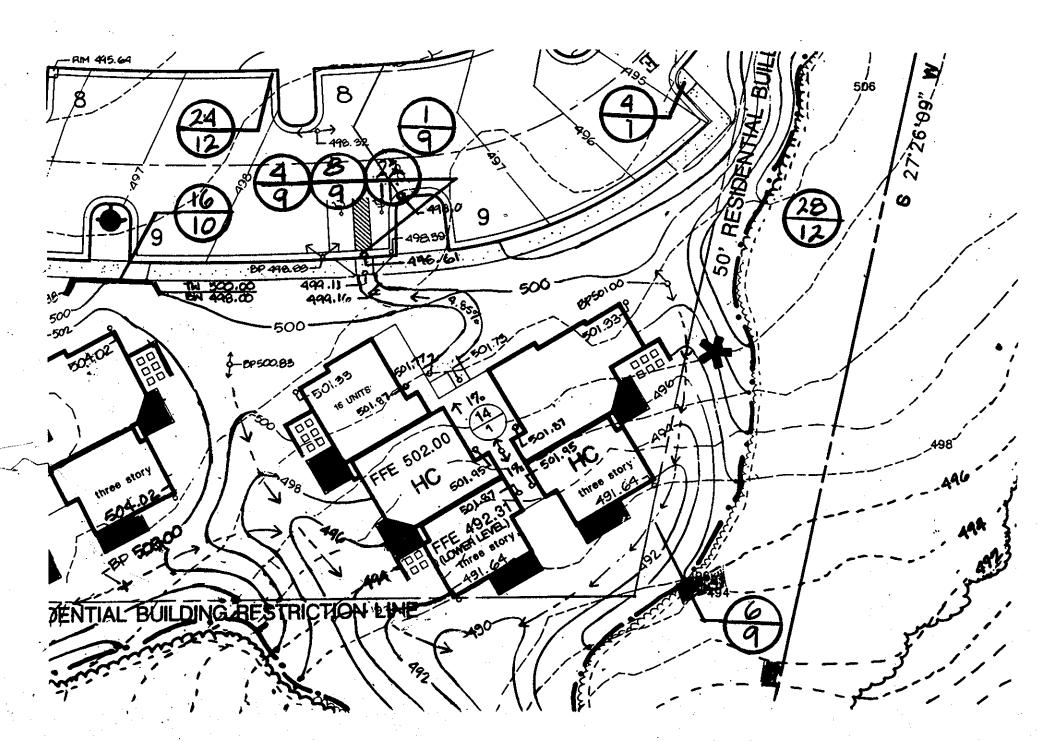
APPROVED HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING APPROVED HOWARD COUNTY HEALTH DEPARTMENT FOR PUBLIC WATER AND SEWERAGE

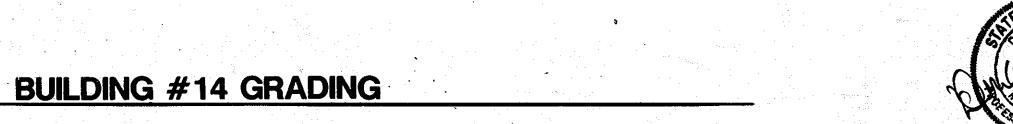
APPROVED DEPARTMENT OF PUBLIC WORKS
FOR PUBLIC WATER AND SEWER AND STORM
DRAINAGE SYSTEMS AND ROADS 118190 Masim & Ream 1-8-60 CHIEF, BUREAU OF ENGINEERING DATE SUBDIVISION NAME SECTION/AREA PARCEL ASHTON WOODS APARTMENTS PLAT NO BLOCK NO ZONE TAX/ZONE ELEC DIST CENSUS TR





BUILDING #7 GRADING



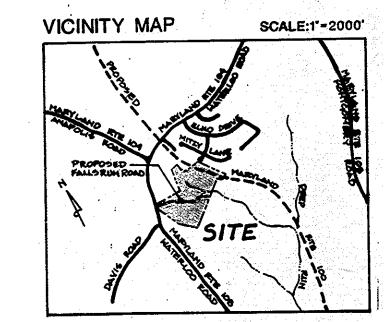


ASHTON WOODS

Tax Map#31 Parcel 423 Lot 2 Second Election District Plat#7522 & 7523 Zoning R-SA-8 Howard County, Maryland

An Apartment Community

Summit Properties



0-35.50 CRITICAL SLOPES

OWNER / DEVELOPER: SUMMIT PROPERTIES 203 KEY HIGHWAY BALTIMORE, MARYLAND (301) 234-0070

ENGINEER: STV/LYON ASSOCIATES 21 GOVERNOR'S COURT BALTIMORE, MARYLAND 21207 (301) 944-9112

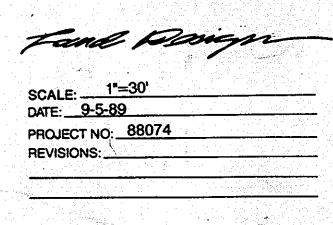


APPROVED

DIVISION of COMMUNITY PLANNING

& LAND DEVELOPMENT

HOWARD COUNTY, MARYLAND



Landscape Architecture Land Planning

SITE DETAILS

OF: 38 42

50P 29-254

Scope: The retaining wall engineer's (Ryan & Associates) scope consists of preparing the wall design to enable the contractor to obtain necessary permits and properly construct the wall. The design considers the internal and local stability of the reinforced soil mass and is in accordance with acceptable engineering practice and these specifications. In areas where the wall's front slope meets or exceeds a 3:1 ratio a global stability analysis was performed. Services outside this scope such as responding to the owner's engineering firm (civil, structural, geotechnical or otherwise), provision of quality control testing & inspection, certification of wall construction, investigation of failed or non-conforming walls or any other services may be provided on a time & materials basis or for a negotiated fee. The scope of Ryan & Associates (RA) for this project does not include wall stakeout or any other civil engineering/surveying.

Contactor's Responsibility: This design has been done in an effort to achieve the required grade changes shown on the civil plans for this project. The wall installer must verify the accuracy of this design by comparing it to the civil plans prior to wall construction. The civil plans govern and their grades must always be met. The lineal footage of the wall and the TW/BW elevations must be checked. If an error is discovered, RA shall be notified immediately and revisions will be provided. RA will not be responsible for correcting a wall that is being built or has been built incorrectly due to this design being followed rather than the civil drawings.

Stormwater Management: The segmental retaining wall is not a stormwater management structure (unless designed as a "water application"). Therefore, it is absolutely essential that surface water be prevented from entering and/or ponding above the wall's reinforced geogrid zone and retained zone (2x the wall height). This is usually accomplished by the site engineer (owner's civil engineer) grading the surface behind the wall to direct surface water to swales that divert the water around the wall ends, to inlets or over the top of the wall through scuppers. If water is directed to the wall, the top eight inches of compacted fill over the reinforced geogrid zone must have impermeable soils such as clay (CL, GC or SC) or low permeable soils (ML meeting the parameters listed in the RA Specifications under Section 3.16A). An underlying geomembrane may also be used to line the swale or be laid over top of the part of the reinforced geogrid zone that is prone to ponding (see RA Specifications for details) if clayey or silty soils are not readily available.

INSTALLATION MUST CONFORM TO THE ATTACHED "Ryan & Associates segmental retaining wall specifications and installation guidelines for Anchor Diamond Pro".

CONSTRUCTION REVIEW & CERTIFICATION: CONSTRUCTION INSPECTION, TESTING AND CERTIFICATION BY A STRUCTURAL/GEOTECHNICAL ENGINEER QUALIFIED IN THE DESIGN OF REINFORCED EARTH STRUCTURES IS A REQUIREMENT OF THESE PLANS. ACCEPTANCE OF THE USE OF THESE PLANS INDICATES AGREEMENT FOR PROFESSIONAL ENGINEERING CONSTRUCTION REVIEW AND CERTIFICATION

ASHTONWOODS

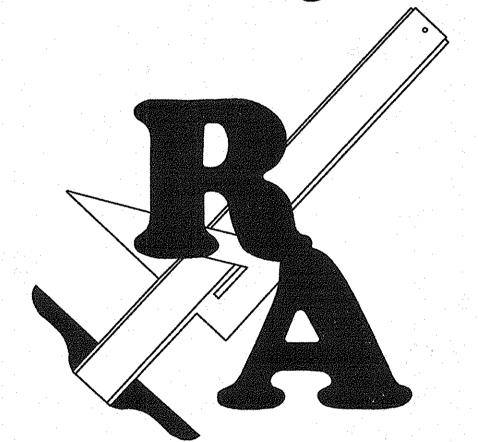
FALLS RUN ROAD, ELLICOTT CITY, MD 21043-7260

ANCHOR DIAMOND PRO RETAINING WALL DESIGNS

Ryan & Associates

Real World Engineering

Hagerstown, MD Office 1825 Howell Road, Suite 3 Hagerstown, MD 21740



www.ryanandassociates.net info@ryanandassociates.net 301-671-3200 (ph) 301-360-9574 (fx)

DRAWING INDEX

Sheet 34 - Cover Sheet

Sheet 35 - Wall Plans/Profiles & Materials Estimate

Sheet 36 - Wall Plans/Profiles & Structural Notes

Sheet 37 - Wall Plans/Profiles & Load Table

Sheet 38 - Wall Plans/Profiles

Sheet 39 - Wall Plans/Profiles

Sheet 40 - Typical Wall Sections & Details

Sheet 41 - Typical Block Details

Sheet 42 - Specifications

PROJECT INFORMATION

Project : Ashton Woods

Location: Falls Run Road, Ellicott City, MD 21043-7260

Wall Installer: U.S. Lawns of Baltimore Contractor: U.S. Lawns of Baltimore Owner/Developer: The Dolben Companies RA Project Manager: Charles R. Dunn

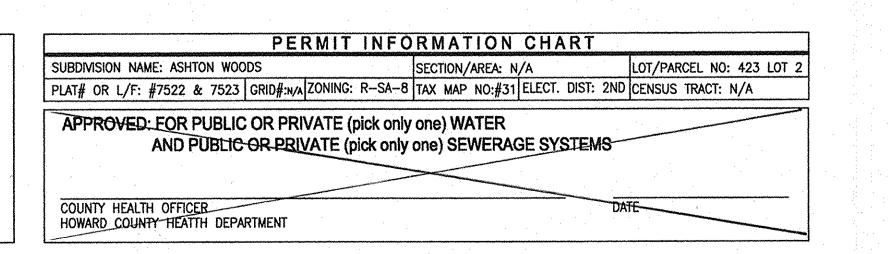
CALL "MISS UTILITY"

TELEPHONE 1-800-257-7777 FOR UTILITY LOCATIONS AT LEAST 48 HOURS BEFORE CONSTRUCTION.

THESE DRAWINGS ARE THE PROPERTY OF RYAN & ASSOCIATES. UNAUTHORIZED REPRODUCTION FOR ANY PURPOSE IS AN INFRINGEMENT UPON COPYRIGHT LAWS. VIOLATORS WILL BE SUBJECT TO PROSECUTION BY THE FULLEST EXTENT OF THE LAW.

WRITTEN DIMENSIONS ON THE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALE DIMENSIONS. CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THIS OFFICE MUST BE NOTIFIED OF ANY VARIATION FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THESE DRAWINGS.

APPROVED: DEPARTMENT OF PLANNING AND ZONING 8/25/V DATE DATE 8/29/11 DATE

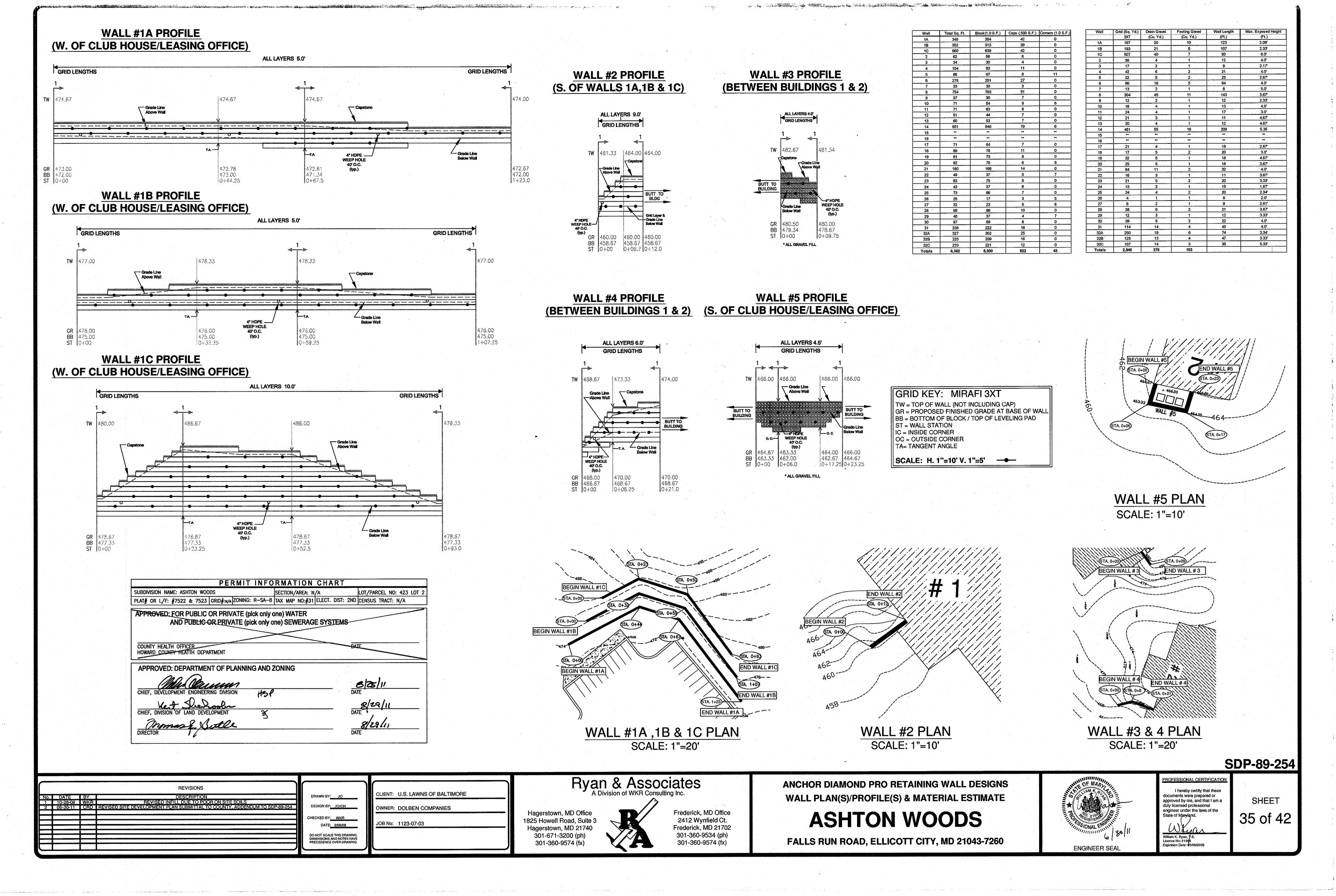




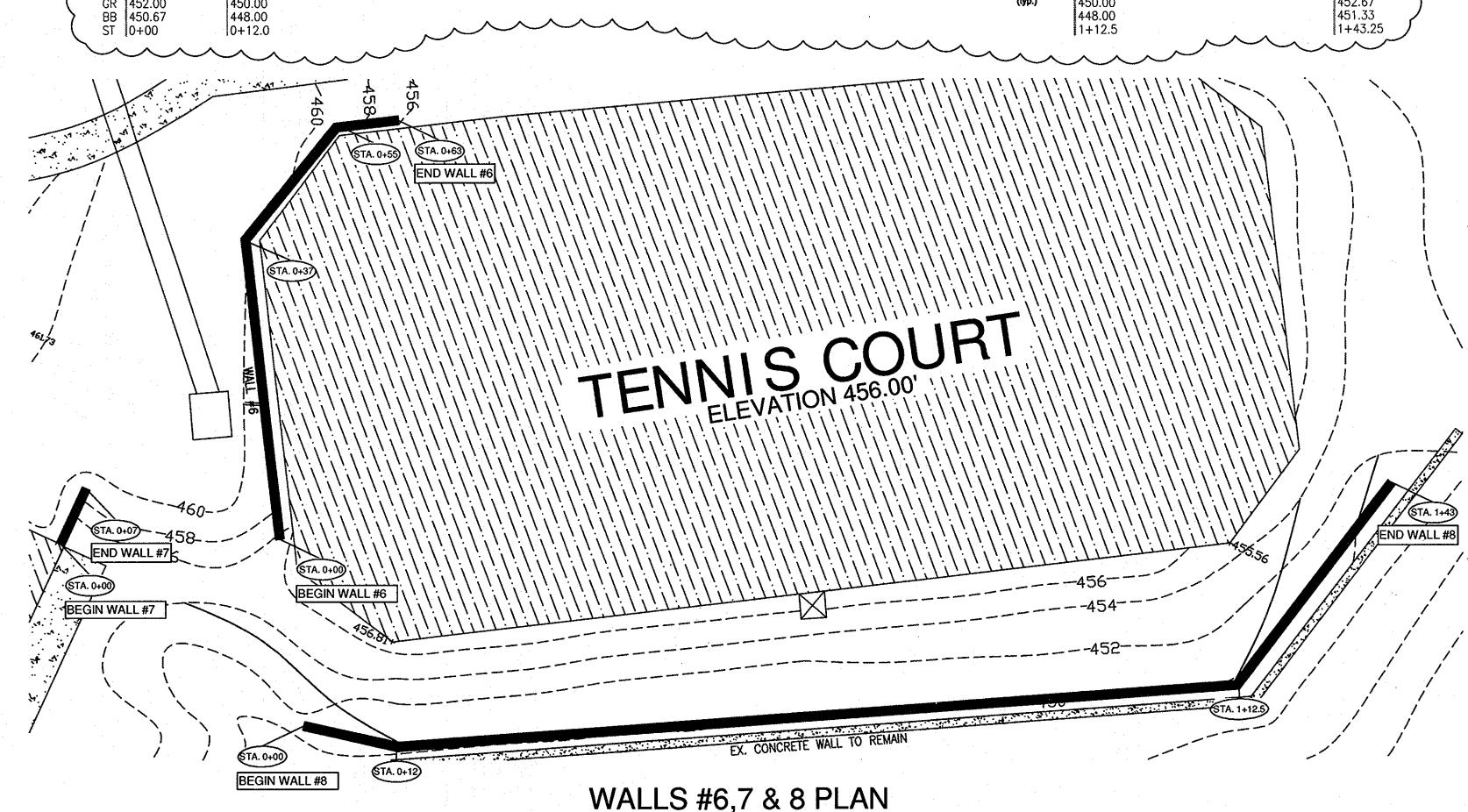
SHEET

SDP-89-254

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WALL #6 PROFILE (WRAPPING AROUND TENNIS COURT) WALL #7 PROFILE (@ CLUB HOUSE) ALL LAYERS 5.0' (3 LAYERS) NOTE: ALL GRAVEL BACKFILL (#57 **GRID LENGTHS** STONE OR CLEAN GRADE CR6) TW 456.00 460.00 4" HDPE -40' O.C. GR 456.00 456.00 BB 454.67 ST 0+00 454.67 0+55.5 454.67 0+63.75 454.67 455.00 454.00 ST [0+00 WALL #8 PROFILE (ABOVE EXISTING C.I.P WALL @ TENNIS COURT) GRID LENGTHS **GRID LENGTHS** NOTE: ALL GRAVEL BACKFILL (#57 STONE OR CLEAN GRADE CR6) WITH MINIMUM UNIT WEIGHT OF 100 pcf. 452.67 GR 452.00 BB 450.67 ST 10+00



SCALE: 1"=10'

STRUCTURAL NOTES

1. SOIL PARAMETERS: Ryan and Associates has performed a geotechnical investigation for this site and due to the wide range of soil types present throughout the site an internal angle of friction of 27° was assumed for all walls except those supporting AC units (see wall profiles). Due to the potential for water infiltration and limited space for excavation for walls supporting AC units, Ryan & Associates (RA) has substituted all gravel infill. The internal angle of friction of 27° is for worst case CL (Lean Clay) soil types and must be verified prior to wall construction by the site geotechnical engineer. CH (fat clay), MH (elastic silt) and OH/OL/PT (organic) soils are not acceptable for wall construction. If these unsuitable soils are encountered they must be removed and replaced with soils that meet or exceed the design friction angle of 27°. The on site geotechnical engineer must monitor this closely during the construction process.

Select infill soil (gravel-#57 stone) will be imported and used to backfill walls supporting AC units and walls #6, #7 and #8. An internal angle of friction of 36° was used for this material; therefore the infill soil must be classified as gravel (GP) or (GW) with a minimum density of 100 pcf. This material must also meet the additional requirements listed in the RA Specifications under section 2.07A. Assumed unit weights (maximum wet densities) of 120 PCF for the 27° soils and 95 PCF for the 36° soils were used. All 27° soils used for wall backfill must also meet the following additional requirements: maximum liquid limit of 40, maximum optimum moisture of 20%, minimum dry unit weight of 100 PCF and maximum of 65% passing the #200 sieve (minimum of 35% retained on the #200 sieve). Fluctuations of 5 PCF higher or lower will not affect this design, however if the unit weights vary by more than 5 PCF Ryan & Associates (RA) must be notified and the cross sections rerun to verify that all factors of safety are still met.

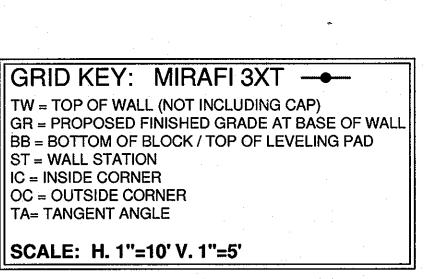
2. BEARING CAPACITY: The sub-grade (the soils under the wall's gravel leveling pad and the soils under the wall's reinforced geogrid zone) must be tested by the site geotechnical engineer prior to wall construction and have a minimum allowable bearing capacity of 3,000 PSF. The actual bearing pressure exerted by each specific wall section is shown on the Cross Section Details and Factors of Safety table so that the site geotechnical engineer may determine specifically how to handle any areas where low bearing capacity soils are encountered on an individual wall section basis. Areas of the sub-grades that do not meet these maximum pressures will require undercutting or geogrid reinforcing. The sub-grade must be virgin (natural undisturbed soil with blow counts >12) or suitable fill (>27°) compacted to 95% of a standard proctor maximum dry density.

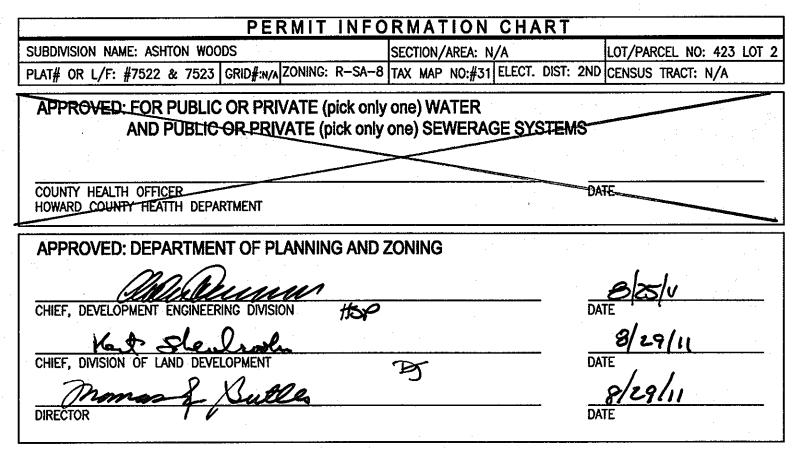
- 3. SLOPES & SUPCHARGES: Refer to load table (sheet 7-this submittal).
- 4. SPECIFICATIONS: Construction and materials must conform to the attached 'Ryan & Associates segmental retaining wall specifications and installation guidelines for Anchor Diamond Pro".
- 5. BLOCK SYSTEM: This design is valid only for the Anchor Diamond Pro system. Each segmental wall system has unique dimensions, connection devices and interacts differently with geogrids; therefore other block types may not be substituted without a partial or total redesign.

6. WALL BATTER: The Anchor Diamond Pro Blocks have a 7.1° batter (1" setback per block course.) It is important for the wall installer, general contractor and the civil engineer/surveyor to predetermine the wall's batter during stake out. The base of the wall will need to be moved forward if there are critical dimensions that need to be met on the high side of the wall (such as distances to curbs or guardrails).

- 7. GEOGRIDS: These walls were designed with Mirafi 3XT which has a LTDS (Long Term Design Strength) of 1705. All geogrid substitutions must have prior approval of RA.
- 8. DESIGN BUILD: These walls are a "design build" situation and are based on field measurements taken during RA's site visit conducted on 1/07/08 & 01/08/08. Since it is a "design build" situation the wall installer may need to slightly modify the wall lengths and heights to accomplish the necessary grade changes. Lineal footage may be added or subtracted as needed if the walls' heights are equal to or less than the design heights. If the walls need to be raised in height, RA shall be notified and new structural cross sections must be provided before the wall installer proceeds
- 9. EMBEDMENT: Wall embedment varies from one to three blocks. The exact amount of buried blocks can be determined by subtracting the "BB" elevations from the "GR" elevations on the RA profile drawing.
- 10. WALL PROFILE The elevation drawings were done to represent the grade changes necessary on the civil drawings and was done in even block course increments of 0.667' (8.0"). Minor field changes may be necessary by the wall installer. Lineal footage may be added or subtracted as needed if the wall's height is equal to or less than the design height. If the wall needs to be raised in height, RA shall be notified and new structural cross sections must be provided before the installer proceeds. The cap height of 0.33' (4.0") is not shown on the profile drawing however its height may have been used in some cases to achieve the desired TW elevations.
- 11. TANGENT ANGLES: Any angle points as drawn on the civil plans (refer to wall plans/profiles for specific stationing) must be built as radii (inside or outside curves) or as structural inside or outside corners (with the blocks interlocked and overlapped in accordance with the Anchor Diamond Pro construction guidelines). Continuous vertical joints are not permitted.
- 12. INSIDE & OUTSIDE CORNERS: Any inside or outside corners as drawn on the civil plans (see wall plans/profiles for specific stationing) must be built structurally (with the blocks interlocked and overlapped) in accordance with the Anchor Diamond Design Manual. The outside corners must be built structurally with the special Anchor Diamond Pro corner units. Continuous vertical joints are not permitted.
- 13. GLOBAL STABILITY: Global stability analyses were required in areas where the walls front slope meets or exceeds a 3:1 ratio or interraced wall situations. Analyses were done with G-Slope design software and the geogrid layers were lengthened as necessary until a factor of safety of 1.3 was met for external stability. A minimum of 25# of cohesion was assumed for all foundation soils in the analyses. The assumed cohesion values can be verified by the site geotechnical engineer by conducting direct shear, triaxial or unconfined compression tests. A factor of safety of 1.5 was also met for internal stability (geogrid rupture). A copy of these analyses are included behind the structural cross section calculations.
- 14. DESIGN SOFTWARE: Internal and external wall calculations were performed with SRWall (version 3.22). A table has been included ("Cross Section Details and Factors of Safety") which has the following information: section location (area of wall referenced), total wall height, loads applied, factors of safety (for sliding, overturning and bearing capacity) and bearing pressure (the weight exerted by the wall structure- block and geogrid zone). Factors of safety of 1.5 were also met for: geogrid pullout (from the soil and from the block), geogrid overstress (geogrid rupture) and connection (block to geogrid).
- 15. BACK SLOPES: Water management is especially critical since there are back slopes above these walls. Since water is being directed to the walls, the water must be directed over them (sheet flow- the fill soils must come to tops of caps) or swales must be constructed behind the walls to divert the water around the end(s) of the walls. The surface water runoff must not be permitted to enter or pond above the reinforced geogrid zones or be introduced into the 12" gravel drainage layers (saturation of the reinforced geogrid zones will cause the reinforced backfill soils to loose their shear strength and may ultimately lead to wall failure). If swales are done they must have minimum depths of 8" and minimum 1-2% slopes laterally from the high points to the end(s) of the walls (see swale details for clarification). They shall be lined with asphalt, concrete, impermeable soils (clay: CL, GC or SC), low permeable soils (ML meeting the requirements in the RA Specifications) or an underlying geomembrane (see Section 3.16A of the RA Specifications for details on low permeable soils and the geomembrane). The soils in the back slopes and the retained zones (within the walls' zones of influence: behind the reinforced geogrid zones and extending to distances that are twice the walls' exposed heights) must be virgin (natural undisturbed soil with blow counts > 12) or suitable fill (>27°) compacted to 95% of a standard proctor maximum dry density. This must be verified by the site geotechnical engineer. The requirements for the impermeable layer/geomembrane may be waived if the infill soils are free-draining gravel or sand (classified by USCS as GP, GW, SP or SW).
- 16. FENCE/RAILING: Fence/railing installation behind the wall(s) for a wall exposed height of 30 inches or taller (varies by local jurisdiction) is required. Fence/railing post (non-wind/non-load bearing) foundation details are shown on sheet 8.
- 17. WEEP HOLES: Weep hole(s) shown in wall profile(s) are for guidance purposes only, actual locations to be decided at site in consultation with site geotech engineer with a maximum spacing of 40 O.C.
- 18. CONSTRUCTION OVERSIGHT: The construction of this wall must be performed under the observation/review of a Maryland Registered Professional Engineer or their designated representative to ensure that it is built in accordance with the RA Structural Notes and Specifications. All wall construction must be certified by a registered professional geotechnical/structural engineer.

19. SAFETY: The contractor is responsible for the following: a) safety and protection within and adjacent to the site; b) Adhere to OSHA's health & safety laws; c) any special inspections required by the building codes; d) any temporary bracing or shoring; e) verification of all conditions, dimensions & elevations; f) erosion and sediment control of the site; g) notifying the design engineer (RA) of any conflicts or discrepancies between the design condition and site condition.





SDP-89-254

REVISIONS

No. DATE BY DESCRIPTION DRAWN BY: CJH
1 10-28-08 WKR REVISED INFILL DUE TO POOR ON SITE SOILS
2 06-30-11 CRD REVISED SITE DEVELOPMENT PLAN SUBMITTAL TO COUNTY, ADDENDUM TO SDP-89-254

CHECKED BY: WKR

DATE: 3/06/08

DO NOT SCALE THIS DR

DIMENSIONS AND NOTE
PRECEDENCE OVER DR

DESIGN BY: WKR

DESIGN BY: WKR

DATE: 3/06/08

DO NOT SCALE THIS DRAWIN'S: DIMENSIONS AND NOTES HAVE PRECEDENCE OVER DRAWING

Ryan & Associates
A Division of WKR Consulting Inc.

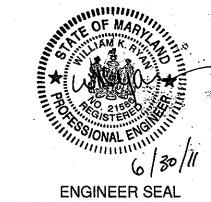
Hagerstown, MD Office 1825 Howell Road, Suite 3 Hagerstown, MD 21740 301-671-3200 (ph) 301-360-9574 (fx)



Frederick, MD Office 2412 Wynfield Ct. Frederick, MD 21702 301-360-9534 (ph) 301-360-9574 (fx) ANCHOR DIAMOND PRO RETAINING WALL DESIGNS WALL PLAN(S)/PROFILE(S) & STRUCTURAL NOTES

ASHTON WOODS

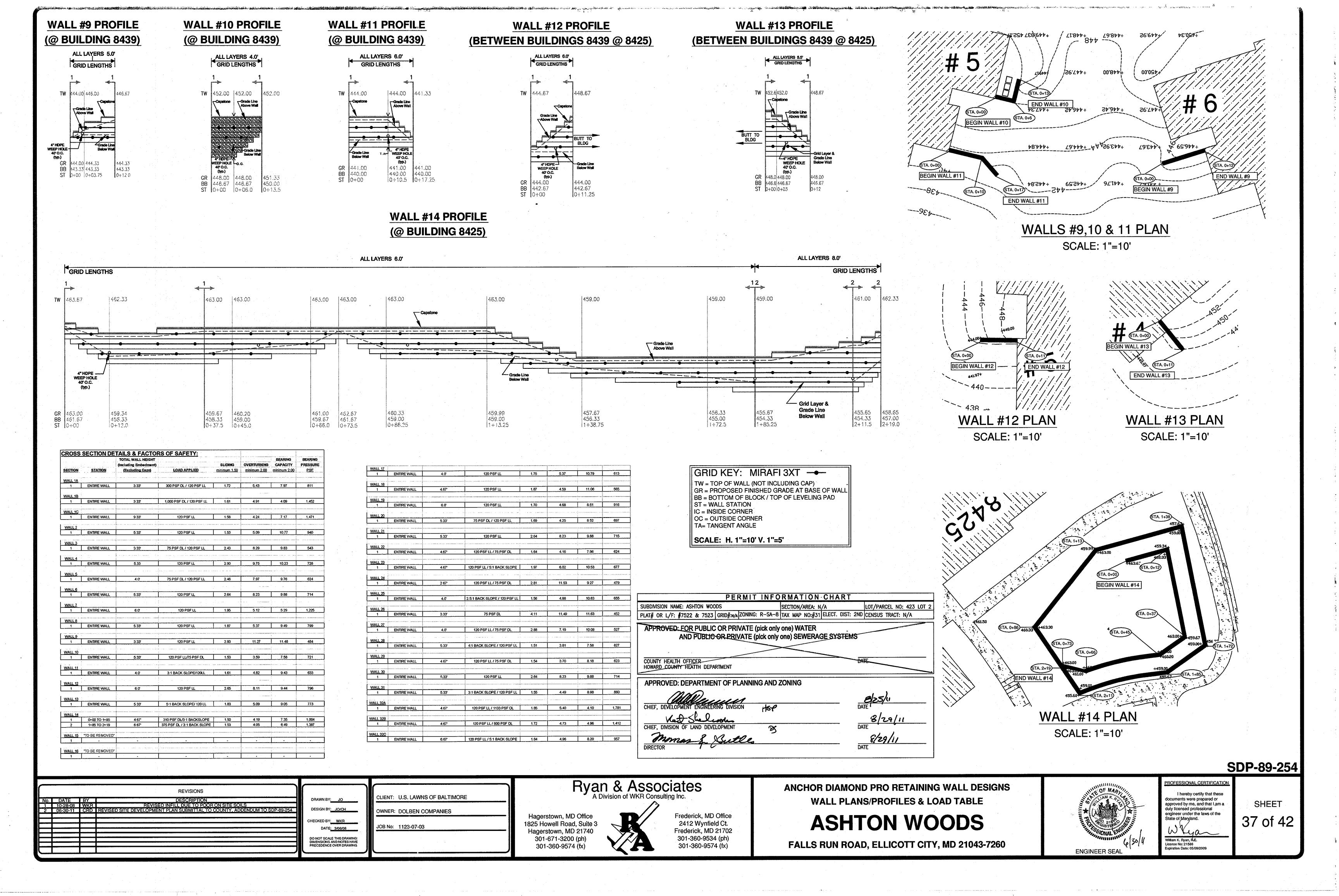
FALLS RUN ROAD, ELLICOTT CITY, MD 21043-7260

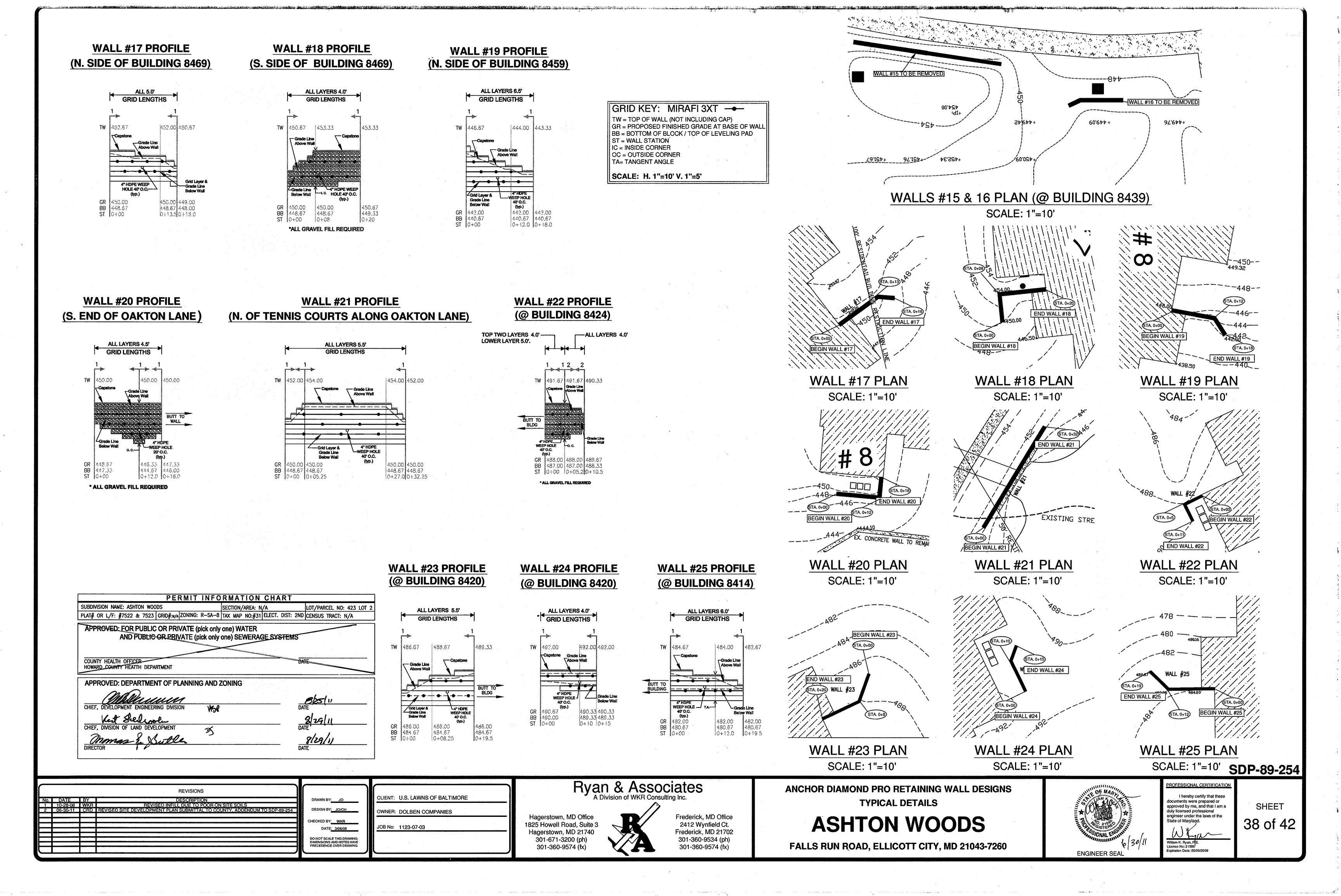


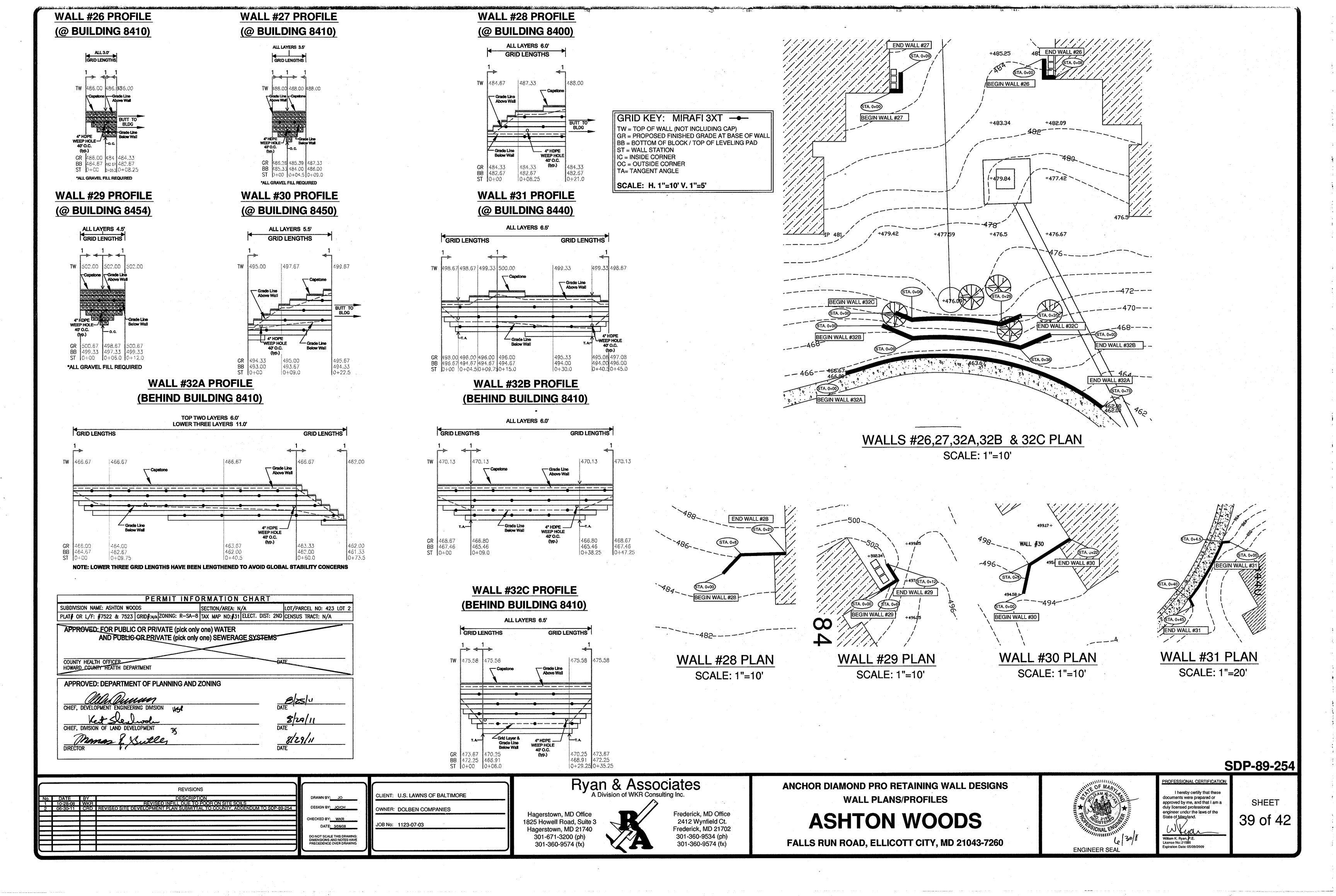
I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

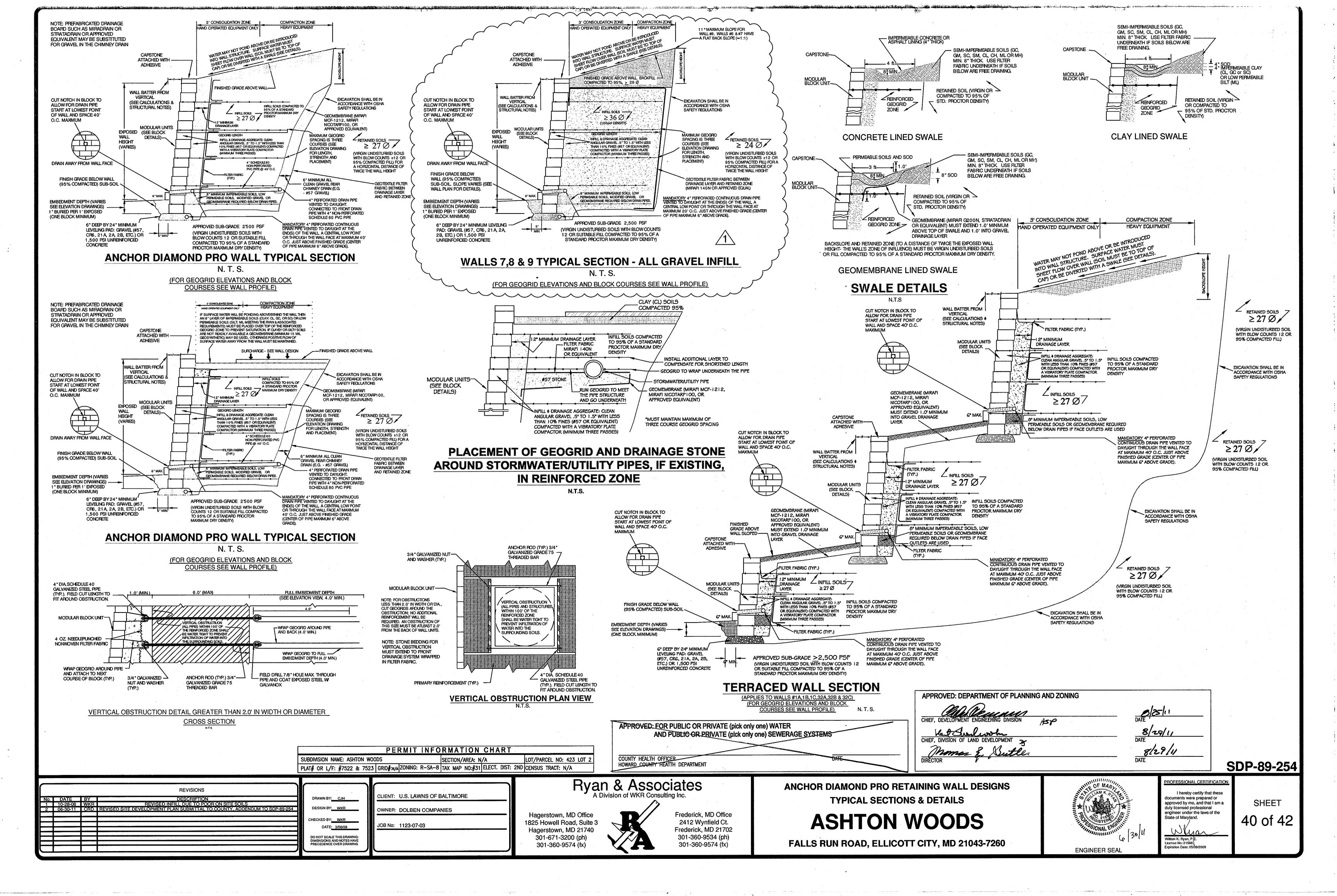
William K. Ryan, F.E. License No: 2158

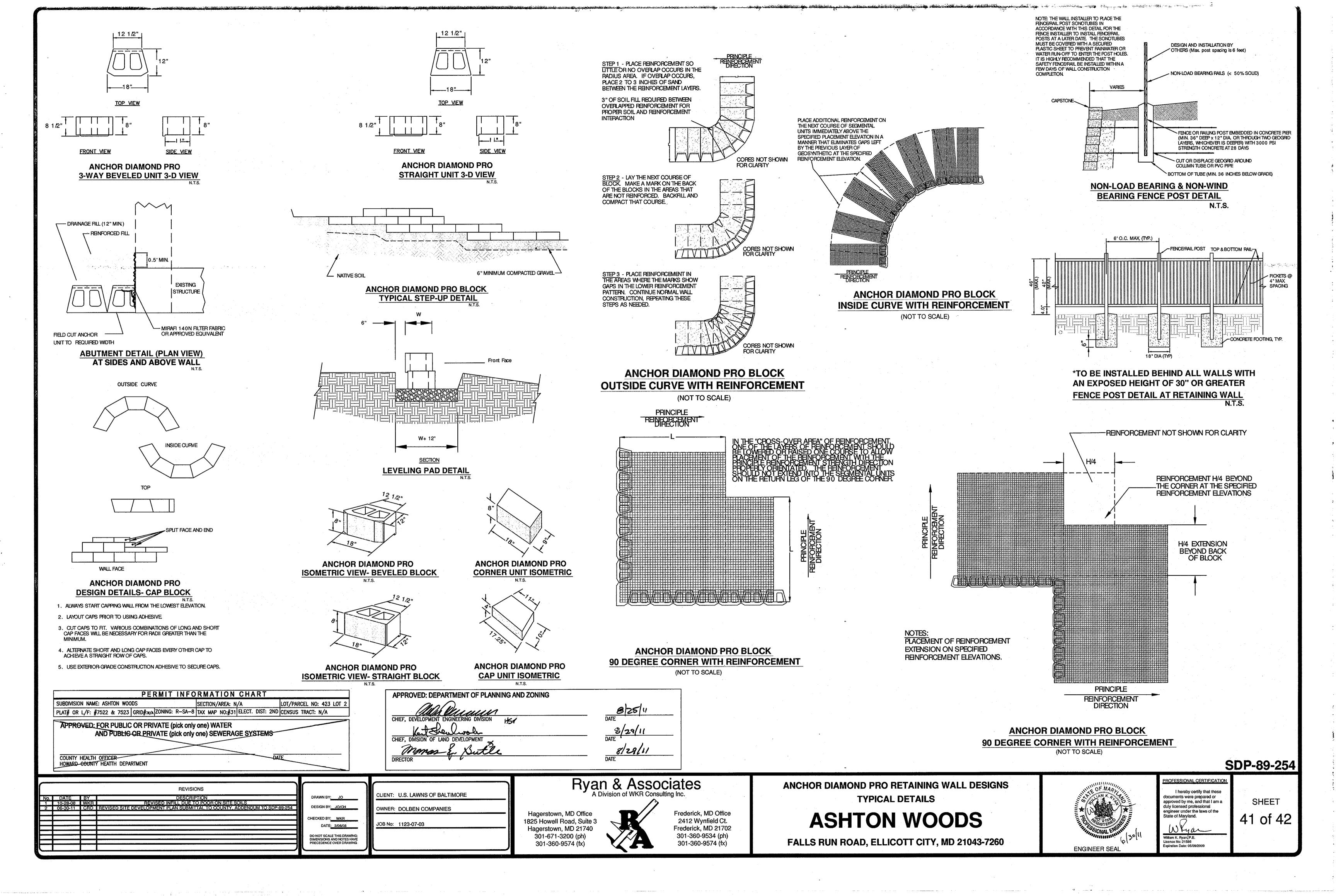
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SPECIFICATIONS FOR SEGMENTAL RETAINING WALL SYSTEMS

PART 1: GENERAL

1.01 Description

A. Work includes furnishing and installing segmental retaining wall (SRW) Units to the lines and grades designated on the Final Design prepared by Ryan & Associates (RA). Also included are furnishing and installing appurtenant materials required for construction of the retaining wall as shown on the RA Final Design.

1.02 Reference Standards

ASTM 3034- Specification for Polyvinyl Chloride (PVC) Plastic Pipe

ASTM D 2166- Unconfined Compressive Strength of Cohesive Soil

- ASTM C 140- Sampling and Testing Concrete Masonry Units and related units ASTM C 1372- Standard Specification for Segmental Retaining Wall Units
- ASTM D 422- Gradation of Soils ASTM D 698- (AASHTO T99) Standard Test Methods for Laboratory Compaction Characteristics of Soil using Standard Effort ASTM D 1248- Polyethelene Plastics Extrusion Materials for wire and Cable
- ASTM D 1557- (AASHTO T 180) Standard Test Method for Laboratory Compaction Characteristics of Soil using Modified Effort
- ASTM D 1586- Standard Test Method for Penetration Test and Split-Barrel Sampling of Soils
- ASTM D 2487- Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System) ASTM D 3080- Direct Shear Test of Soils Under Consolidated Drained Conditions
- ASTM D 4318- Liquid Limit, Plastic Limit and Plasticity Incex of Soils ASTM D 4595- Tensile Properties of Geotextiles by the Wide-Width Strip Method.
- ASTM D 5262- Test Method for Evaluating the Unconfined Creep and Creep Rupture Behavior of Geosynthetics ASTM D 2850- Unconsolidated, Undrained Compressive Strength of Cohesive Soils in Triaxial Compression GRI:GG1- Single Rib Geogrid Tensile Strength
- GRI:GG5- Geogrid Pullout

1.03 Design Standards

A. The following factors of safety have been met in this design: Sliding 1.5, Overturning 2.0, Bearing Capacity 2.0, Tensile Overstress 1.0, Geogrid Facing Connection (between the block and the geogrid) 1.5 Geogrid Pullout 1.5 (from the soil) and Global Stability 1.3 (if applicable-typically for terraced walls and/or walls situated on front slopes ≥3:1).

1.04 Scope of Work

A. The retaining wall design engineer's (Ryan & Associates) scope consists of preparing the wall design and professionally sealing to enable the contractor to obtain the necessary permits and properly construct the wall. The design considers the internal and local stability of the reinforced soil mass and is in accordance with acceptable engineering practice and these specifications. Services outside this scope such as responding to the owner's engineering firm (civil, structural, geotechnical or otherwise), provision of quality control testing & inspection, investigation of failed or non-conforming walls or any other services may be provided on a time & materials basis or for a negotiated fee. The scope of work for Ryan & Associates (RA) on this project does not include the preparation of a grading plan, site plan, geotechnical investigation, resolution of groundwater issues, sub-grade preparation, sub-grade repair, sub-grade failure (including but not limited to mine subsidence and sinkholes), surveying, wall stakeout or any other civil or geotechnical engineering.

1.05 Contractor's Responsibility and Qualifications

A. This design has been done in an effort to achieve the required grade changes shown on the civil plans for the project or the owner's (or wall installer's) desired grade changes (in the case of design build projects). The wall installer must verify the accuracy of these grades (elevations) by comparing them to the civil plans or actual field conditions prior to wall construction. The civil plans and actual field conditions govern and their grades must always be met. The lineal footage of the wall and the TW/BW (top of wall/bottom of wall) elevations must be checked. If an error is discovered, RA shall be notified immediately and revisions will be provided. RA will not be responsible for correcting a wall that is being built or has been built incorrectly due to this design not being checked for accuracy prior to wall construction. Deviations from the RA Final Design will have ramifications; therefore all changes must be approved by RA in writing prior to wall construction. RA is not responsible for any future problems that result from unauthorized changes to the RA Final Design. The contractor must be qualified to do this work with a minimum of three years successful experience and minimum installation square footage of 100,000 s.f.

PART 2: MATERIALS & DESIGN PARAMETERS

2.01 Segmental Retaining Wall Units

A. SRW Units shall be machine formed, Portland Cement concrete blocks specifically designed for retaining wall applications. The SRW Unit currently approved for this project is:

Anchor Diamond Pro NOTE: Where Anchor specifications and reference documents conflict with these specifications the RA specifications

B. SRW Units shall be sound and free of cracks or other defects that would interfere with the proper placing of the units or significantly impair the strength or permanence of the structure. Cracking or excessive chipping may be

grounds for rejection. Units showing cracks longer than 1/2 inch shall not be used within the wall. Units showing chips visible at a distance of 30 feet from the wall shall not be used within the wall.

C. Concrete used to manufacture SRW Units shall have a minimum 28 days compressive strength of 3,000 PSI and a maximum moisture absorption rate, by weight, of 8% as determined in accordance with ASTM C 140. Compressive strength test specimens shall conform to the saw-cut coupon provisions of Section 5.2.4 of ASTM C 140 with the following exception: Coupon shall be taken from the least dimension of the unit of a size and shape representing

D. SRW Units molded dimensions shall not differ more than + 1/8 inch from that specified, except height which shall be ±1/16 inch as measured in accordance with ASTM C 140.

2.02 Geosynthetic Reinforcemen

A. Geosynthetic reinforcement shall consist of geogrids as indicated on the RA Final Design. No geogrid substitutions shall be permitted without the prior approval of RA (a partial redesign may be necessary if geogrids are substituted). NOTE: it is always acceptable to substitute a higher strength geogrid (of the same manufacturer) for

2.03 Connectors

A. The Anchor block system does not have separate connectors (clips or pins). The rear lower lip (protrudes downward) is the connector and alignment guide; therefore this rear lip must be in tact and structurally sound (cannot be cracked, loose or partially missing). The rear lip also determines the wall's batter (7.1°: 1° setback per

2.04 Leveling Pad

A. Material for the leveling pad shall consist of compacted gravel or unreinforced concrete. Typical gravels used for this leveling pad are #57, CR6, 21A, 2A modified, 2B, RC6, RC57, etc. Lean un-reinforced concrete with a strength of 1,500 PSI may also be

2.05 Drainage Aggregate

A. Drainage aggregate shall be clean angular gravel (#57 or equivalent) with a size of 1/2 inch to 1 1/2 inches and less than 10% fines (passing the #200 sieve). Rounded "pea gravel" type aggregate is not permissible since it does not have the necessary frictional properties. Recycled gravel may be used if it meets the above criteria.

2.06 Drainage Pipe

A. The drainage collection pipe shall be a 4 inch perforated or slotted PVC or corrugated HDPE pipe

2.07 Infill Soils: the reinforced geogrid zone

A. The soils used must meet or exceed the friction angle stated in the RA Final Design (in the Structural Notes, on the typical wall section and on the structural cross sections). The reinforced material shall be free of debris and organic material (i.e.- no plants, roots, sod, top soil, trash, wood, etc.). The infill soils shall not consist of CH (fat clay), MH (elastic silt) or OH/OL/PT (organic) soils. All soils used for wall infill must always meet the following requirements, regardless of the friction angle: maximum liquid limit of 40, maximum optimum moisture of 20%, maximum of 65% passing the #200 sieve (minimum of 35% retained on the #200 sieve) and minimum dry unit weight of 100 PCF (if fine soil). Soil moisture must be within ±2% of optimum to obtain proper compaction results (no exceptions).

B. Rocks may be used as infill material as long as they have a maximum size of 4 inches and a mean diameter of 2 inches. Recycled concrete is permissible for infill except with certain polyester geogrids in water applications. In the case of water applications the geogrid manufacturer shall be consulted to see if the alkali in the recycled material will cause corrosive damage

C. Select gravet (classified by USCS as GP or GW) is normally an acceptable substitution in the event suitable soils (those meeting RA's and the site geotechnical engineer's requirements) are not readily available or if there are difficulties due to frost or moisture levels. However, the unit weights of gravel can vary widely (clean gravel is typically 90-100 PCF and "crusher run" gravel is typically 130-140 PCF) so RA must be notified so that revised sections can be run prior to making any substitutions. In some cases clean gravel actually requires longer geogrid lengths because of its extremely light unit weight.

D. Asphalt tailings (blacktop millings) are only acceptable for wall infill or the leveling pad if authorized in writing by the municipality (county, township, etc.) and/or regulatory authority since they may be a potential environmental hazard. These materials must also meet the requirements for the backfill soils in the RA Final Design; therefore RA must be given a proctor test and gradation (sieve analysis) for approval prior to wall construction

E. For soils that have been treated with lime, only HDPE geogrids (such as Tensar) may be used. Polyester geogrids may only be used with lime treated soils when authorized in writing by the geogrid manufacturer

2.08 Retained Soils: the area beyond the infill soils and extending to a distance that is twice the wall's exposed height

A. These soils must meet or exceed the friction angle stated in the RA Final Design (shown in the Structural Notes, on the typical wall section and on the structural cross sections) and be virgin (natural undisturbed soils with blow counts ≥12) or suitable fill (friction angle ≥ the RA Final Design requirement) compacted to 95% of a standard proctor (or 93% of a modified proctor) maximum dry density.

B. Surface water or groundwater must not be permitted to drain/flow into the retained zone. This could cause hydrostatic pressure to build against the wall structure, leading to failure. The exception is when the wall has been designed as a "water application" with free draining infill gravel or sand (GP, GW, SP or SW).

2.09 Foundation Soils: the soils under the wall's gravel leveling pad and the soils under the rienforced geogrid zone

A. The foundation soils (wall sub-grade) must meet or exceed the minimum allowable bearing capacity stated in the RA Final Design (in the General Notes and on the typical wall section). The sub-grade must be virgin (natural undisturbed soils with blow counts ≥12 blows/foot) or suitable fill (friction angle ≥ the RA Final Design requirement) compacted to 95% of a standard proctor maximum dry density (or 93% of a modified proctor). If highly plastic soils (CH or MH) or organic soils (OH, OL or PT) are encountered in the sub-grade they must be removed and replaced with suitable soils or gravel that is placed in controlled lifts and compacted to 95% of a standard proctor maximum dry density. If the organic or plastic soils extend so deep that they cannot be totally removed, they shall be undercut a minimum of 4' (or as directed by the site geotechnical engineer) and replaced with suitable soils or gravel. It must be emphasized that the wall's sub-grade is not only the soils under the leveling pad, but also includes all soils under the entire reinforced geogrid zone. Therefore, the foundation soils extend from 6' in front of the base block to the back of the reinforced geogrid zone (back edge of the lowest geogrid layer). 2.10 Soil Investigation

A. RA recommends that every retaining wall design be preceded by an in-situ soil investigation with borings spaced 100 on center under the wall face and at the back of the proposed reinforced geogrid zone by a licensed geotechnical engineer. However, if the owner and/or wall installer elects not to have an investigation conducted, RA may assume soil design parameters based on: published data by the Soil Conservation Service (soil maps), a verbal description by the owner and/or wall installer or by RA's previous experience in certain geographic areas. It must be understood that the owner and/or wall installer bears full responsibility to the election not to have a soil investigation performed.

2.11 Site History & Information

A. Many factors other than soil information affect the performance and design of the retaining wall. RA relies on information provided by the owner and/or wall installer when designing a retaining wall. RA bears no responsibility if the owner and/or wall installer omit critical information required to properly design the wall. Information critical to wall design from the site consists of: topographic features (such as slopes), soil types, utilities, storm water management, structures (including buildings, other existing or proposed walls, swimming pools, etc.), site geological phenomenon, groundwater, loads within the wall's zone of influence (such as driveways, other walls, patios, roadways, sidewalks, etc.) and any other readily known site factors that could potentially impact the RA Final Design.

2.12 Civil Plans

A. These designs are supplemental to the approved land development plans (civil plans). All erosion and sediment controls must be followed in accordance with the approved land deverlopment plans (civil plans).

PART 3: CONSTRUCTION

3.01 Inspection

A. RA considers all retaining walls to be critical structures, meaning most walls require a considerable financial investment by the owner and failure of a wall will negatively impact a property both financially and from a public safety perspective. The owner or owner's representative is responsible for verifying that the wall installer meets all of the requirements of the RA Final Design (as stated in these specifications and the project's Structural Notes). This includes all submittals for materials and design, qualifications and proper installation of the wall system. All walls with an exposed height of 4 feet or greater, or those which require a building permit, must have the construction monitored and the completed wall certified by a licensed geotechnical/structural engineer registered in the jurisdiction of the project. Additionally, after the wall has been completed it is highly recommended that it be surveyed to establish the wall's current horizontal and vertical alignment.

B. The wall installer's field construction supervisor shall have demonstrated experience and be qualified to direct all work at the

C. RA provides construction review on some retaining wall projects. RA verifies general compliance with the RA Final Design; however, it is the wall installer's ultimate responsibility to construct the structure properly in accordance with the RA Final Design. RA's liability is limited to the amount of our fees for the scope of work provided for the wall designs and construction

A. The wall installer shall excavate to the lines and grades shown on the RA Final Design and the project's civil plans. The wall installer shall take precautions to minimize over-excavation. Over-excavated areas shall be filled with compacted soils (friction angle ≥ RA design parameters) or gravel (soils or gravel must be compacted to 95% of a standard proctor) as directed by the

B. The wall installer shall verify the location of existing structures and utilities prior to excavation. The wall installer shall ensure that all surrounding structures are protected from the effects of wall excavation. Excavation support (shoring), if required, is the responsibility of the general contractor and/or site contractor. All excavation must be conducted in accordance with OSHA (federal) and state safety regulations. All work to construct the wall must be in accordance with 29CFR1926 sub-part P (OSHA Excavation Safety Requirements).

3.03 Foundation Preparation

A. Following excavation, the foundation soils (the soils under the wall's gravel leveling pad and the soils under the wall's reinforced geogrid zone) shall be examined by the site geotechnical engineer to assure that the actual foundation soil strength meets or exceeds the minimum allowable bearing capacity in the RA Final Design (stated in the Structural Notes and shown on the typical wall section). Soils that do not meet the required strength shall be removed and replaced with approved select structural fill or gravel and be compacted to 95% of a standard proctor (or 93% of a modified proctor) maximum dry density for the full depth.

B. In cases of poor bearing capacity, deep fill soils, or when groundwater is encountered it may not be possible or practical to undercut to suitable soils. As an alternative, gravel, geogrid reinforcing (or a combination of both) or an reinforced concrete footing (may be steel reinforced if required) may be utilized to meet the required soil bearing capacity. RA can provide a supplemental sub-grade design for an additional fee (it is not within RA's original scope of work for this project). However, it is the site geotechnical engineer's ultimate responsibility to ensure that the sub-grade meets or exceeds that specified by RA for this project (stated in the RA Final Design). The site geotechnical engineer must add a minimum factor of safety of 2.0 to the RA specified capacity (i.e. if RA requires 2,500 PSF the actual field ultimate bearing capacity must meet or exceed 5,000 PSF).

C. If competent rock (blow counts ≥50 blows/inch) is encountered in the sub-grade, or if the wall needs to span a concrete structure, the embedment may need to be reduced. RA must be consulted for an alternate embedment design (pinning to rock or the structure is typically an option).

A. The leveling pad shall be placed so that its top elevation is the same as the bottom of block ("BB") elevation on the RA Final Design profile drawing. It shall have a minimum thickness of 6 inches and a minimum width of 2 feet (for 12" wide blocks). The leveling pad should, at a minimum, extend laterally at least a distance of 6 inches from the toe and heel of the lower most SRW Unit. The leveling pad must have a minimum width of 30 inch for 18 inch wide blocks and a minimum width of 36 inch for 21.5 inch wide blocks

B. The leveling pad material shall be compacted to 95% of a standard proctor maximum dry density with a vibratory plate compactor to provide a firm level-bearing surface on which to place the first course of SRW Units. A thin layer (not to exceed 1/2 inch) of well-graded sand or stone dust may be used to smooth the top of the leveling pad.

3.05 SRW Unit Installation

A. Embedment shall be a minimum of 1 inch buried for every 1 foot of exposed wall height with one block minimum when the front slope is 4H:1V or greater (more level). Walls constructed on 3H:1V front slopes or less (more steep) require additional buried blocks. See the profile drawing in the RA Final Design for the exact amount of embedment (the amount of buried block can be determined at each wall station by subtracting the "BB" elevations from the "GR" i.e. grade elevations). The wall's toe (front of wall at base course) must be backfilled up to the proposed civil plan elevations with 95% compacted approved sub-soils no later than after the second course above the final grade is placed (two courses exposed). The wall's foundation shall not be left exposed/open for more than two to five days during clear weather, however it must be backfilled and compacted (in accordance with the RA Final Design) prior to a rain event. Clean gravel may not be used to fill against the buried blocks. They must be "locked in" with suitable sub-soils or modified gravel.

B. All SRW Units shall be installed at the proper elevation and orientation as shown on the RA Final Design profile drawing and in conjunction with the project's civil plans. The SRW Units shall be installed in general accordance with the manufacturer's recommendations (RA's Final Design shall govern in any conflict between the two requirements).

C. The first course of SRW Units shall be placed on the leveling pad. The units shall be leveled side-to-side, front-to-rear and with adjacent units, and aligned to ensure intimate contact with the leveling pad. The first course is the most important for accurate and acceptable results. Alignment may be done by means of a string line or an offset from the base line to the backs of the blocks. SRW units shall have a minimum 4 inch overlap of units on each successive course so that the wall is interlocked and continuous. No horizontal gaps greater than 1/4 inch between the faces of adjacent units are permitted

D. Because the wall has a setback, its batter must be predetermined during the stake out process by the civil engineer/surveyor and wall installer. If there are critical dimensions that must be met on the high side of the wall then the base (at the toe)

E. Lay out of curves and comers shall be installed in accordance with the civil plans and the RA Final Design. Construction techniques for curves and corners shall be in accordance with the SRW manufacturer's installation guidelines. In general, all tangent angles shown on the civil drawings should be changed into radii (inside and outside curves) to enhance the wall's strength and appearance. Continuous vertical joints are not permitted. Inside and outside 90° corners may be constructed without compromising the wall's integrity if they are properly built. Inside comers should be constructed so that the SRW Units interlock and overlap (according to the manufacturer's recommendations) and outside corners should incorporate special corner blocks when possible. If special outside corner blocks are not available from the block manufacturer for this project then the manufacturer's guidelines for building structural outside corners shall be followed. Outside corners must be built so that the blocks interlock and overlap. If gluing is necessary (when there is not a positive connection from clips, lips, lugs, pins, etc.) only industrial grade adhesives or sealants designed for concrete-to-concrete applications may be used (adhesives designed for plastic or wood applications are not acceptable).

F. Clean all excess debris from the tops of the SRW Units and install the next course.

G. Repeat procedures to the extent of wall height

H. A ±2° construction tolerance is permitted horizontally for wall batter (block setback). In no case shall a wall go beyond vertical (have a negative batter). Some block systems have an optional near vertical batter (typically 0.5%. If the wall is to be built with a near vertical batter, the base course should be laid so that it is tilted back a minimum of 1/4 inch to compensate for movement that will likely occur from compaction equipment and from the geogrid losing its slack. Walls shall be built level from left to right (not with grade), howe ver a ±1.5 inch tolerance over a 10 foot distance is permitted vertically (as checked from left

A. All geogrid reinforcement shall be installed at the proper elevation, length (measured from the face of block) and strength as shown on the profile drawing and structural cross section(s) in the RA Final Design. Partial geogrid coverage is not acceptable: no gaps shall be present between geogrid layers. 100% coverage is required, however it is not necessary to overlap the geogrid pieces (overlapping the geogrid may be detrimental since it will likely cause a "bump" in the wall). The geogrid shall be laid horizontally on the compacted infill soil and on top of the concrete SRW Units. The geogrid must be embedded into the SRW Units to the face. The wall installer shall verify that the orientation of the geograd is in accordance with the geograd manufacturer's

recommendations. The highest strength direction of the geogrid must be perpendicular to the wall face (the geogrid must not be laid parallel to the wall-cannot be rolled out with the wall). The geogrid must be level (cannot be going "down hill" or "up hill"). B. Geogrid reinforcement layers shall be one continuous piece for their entire embedment length. Overlapping of the geogrid in the design strength direction (perpendicular to the wall face) is not permitted.

C. Tracked construction equipment shall not be operated directly on the geogrid. A minimum of 6 inches of backfill is required prior to operation of tracked vehicles over the geogrid. Turning should be kept to a minimum. Rubber-tired equipment may pass over the geogrid reinforcement at slow speeds (less than 5 MPH).

D. The geogrid shall be in tension and free of wrinkles prior to placement of the infill soil. Nominal tension shall be applied to the geogrid and secured in place with staples, stakes or by hand until it is covered by 6 inches of infill soil.

E. For inside & outside corners and inside & outside curves the geogrid shall be placed according to the manufacturer's instructions to provide total geogrid coverage. On outside corners the geogrid should be shifted up or down one course and alternated so that the geogrid comes into the reinforced geogrid zone from both legs of the 90° angle. Geogrid layers should never be placed on top of one another: there must be a minimum of 3 inches of compacted infill soil between geogrid layers.

F. RA must be notified if the geogrid cannot be placed to its full design length. This is typically caused by restrictions on the site such as rock being encountered, property line restraints or unsafe excavation (embankment too steep). In some cases the geogrid can be shortened (by using gravel infill, if the retained soils are competent rock, if the site retained soils exceed the RA Final Design requirements, etc.). However, the geogrid length may never be less than .6 (60%) of the total wall height (exposed height plus embedment). In the absence of all lateral loads (only when the retained zone is all competent rock with blow counts ≥50 or when there is a full depth structure removing all pressure) the geogrid can never be less than .5 (50%) of the exposed wall height to maintain facial stability.

3.07 Wall Drainage

3.06 Geogrid Reinforcement Placement

A. Drainage aggregate (clean gravel such as #57 or approved equivalent) shall be installed behind the entire wall face from the first course below grade to one course from the top of the wall. The drainage gravel shall be placed to a minimum thickness of 12 inches behind the SRW Units. Drainage gravel shall also fill all voids between and within (if hollow) the SRW Units. SRW Units must be filled with drainage aggregate in one course lifts (SRW Units may not be stacked in two or three course lifts and then have the gravel dumped in from the top through multiple courses). An impermeable clay layer (CL, GC or SC) shall be placed on top of the 12" drainage layer. If clayey soils are not readily available, a layer of filter fabric (Mirafi 140N or equivalent) shall be placed on top of the gravel (below the topsoil) to prevent the downward migration of fines.

B. A continuous drainpipe is mandatory and shall be vented to daylight. The continuous drainpipe may run to the end(s) of the wall or to a central low point of the wall. Or, the continuous drainpipe may be vented through the wall face at maximum intervals of 40 feet on center (no more than 6 inches above finished grade). The pipe must maintain gravity flow o water outside the reinforced geogrid zone. Water must drain to an outlet and have positive flow. If a pipe is run below grade, it shall daylight into a storm sewer manhole or along a slope at an elevation lower than the lowest point of the pipe within the drainage aggregate. When drainpipes are daylighted at the end(s) of a wall they must be visible and unobstructed. The drainpipes should be checked by the owner on a regular basis to ensure that they remain open (not blocked, filled in, grown over, pinched).

C. Rear drainpipes are required in the following situations: when groundwater can rise and approach within 1 foot of the wall's subgrade and for walls that have an exposed height of 10.0' or greater in "cut" situations (since the potential exists for water to enter the interface between the reinforced geogrid zone and the retained zone). This rear drainpipe shall be surrounded by a minimum of 12 inches (6 inches on each side) of clean gravel (#57 or equivalent) and surrounded with filter fabric to prevent the migration of fines. The rear drainpipe must vent to daylight or be directed to a storm sewer manhole (see instructions for front drainpipe in section 3.07B above).

D. A chimney drain (a second 12 inch layer of drainage aggregate comprising the rear 1 foot of the reinforced geogrid zone or directly behind the reinforced geogrid zone) or composite blanket (Mirafi MCF-1212 or approved equivalent) must be installed when groundwater is present or likely (to an elevation that is a minimum of 1 foot above predicted levels as given by the sitegeotechnical engineer), when stated in the RA Final Design or when required by the site geotechnical engineer.

E. All drainage zone aggregate shall be compacted with a vibratory plate compactor (minimum of three passes).

3.08 Backfill Placement

A. The infill soils shall be placed as shown in the RA Final Design in the maximum compacted lift thickness of 8 inches and shall be compacted to a minimum of 95% of a standard proctor (or 93% of a modified proctor) maximum dry density (ASTM D 698) at a moisture content within 2% of optimum. The backfill shall be placed and spread in such a manner as to eliminate wrinkles or movement of the geogrid and the SRW units. Compaction testing shall be done with a minimum of one compaction test per lift per 2,500 sqft, but not fewer than two tests per lift for any lift. Approved compaction tests are the Nuclear Density Gauge or Sand cone.

B. Only a vibratory plate or small-scale vibratory smooth drum compactor equipment shall be allowed within 3 feet of the front of the wall face. Compaction within the 3 feet behind the wall face shall be achieved by at leastthree (3) passes of the lightweight mechanical plate compactor or roller. Heavy equipment (such as track hoes, ride on rollers, pans, etc.) must be kept back a minimum of 3 feet from the rear of the wall

C. At the end of each day's operation, the wall installer shall slope the last level of backfill away from the wall facing to direct water runoff away from the wall face and preferably covered with plastic sheets.

D. At completion of wall construction if final grading, paving, landscaping and/or storm drainage installation adjacent to the wall is not placed immediately after wall completion, temporary grading shall be provided to ensure that water runoff is not allowed to collect or pond behind the wall until final construction adjacent to the wall is completed.

E. Filter fabric (Mirafi 140N or equivalent) is required as shown on the details/sections and when the infill soil is classified as poorly graded sand (SP) or well graded sand (SW) since these soils are non-cohesive and could potentially slough, clogging the gravel drainage layer. Filter fabric is optional between the 12 inch gravel drainage layer and the compacted intili soil if the backfillsoils are clayey (CL or SC), gravelly (GC, GM, GP or GW) or silty (ML or SM).

3.09 SRW Caps

A. SRW caps shall be properly aligned and glued (for safety reasons) to the underlying SRW Units with a flexible high-strength concrete adhesive or sealant designed for "concrete to concrete" applications (not for plastic or wood). Rigid adhesive or mortar

3.10 Wall Geometry

A. It is acceptable to modify the wall's configuration (angles to curves, corners to curves, etc.) from the civil plan layout as long as the RA design height is not exceeded and the loads do not increase (slopes do not get steeper, surcharges do not get greater, etc.). There may be differences between the civil plan dimensions and the RA Final Design wall length. This is due to differences in measuring techniques and/or due to copying and scaling of plans. Also, as a conservative measure, RA measures the longest dimension and rounds up. When there is a conflict, the civil plans govern for wall stakeout.

B. Some projects, such as "design build", will require the wall installer to change the wall length and height as needed to meet the field conditions (since there may not be a specific grading plan). This is acceptable, but the wall height in the RA Final Design must not be exceeded. If the wall must be built to a height that exceeds the design, RA must be notified and a revised (taller) cross section provided prior to wall construction.

A. When walls are installed in water applications (such as storm water ponds, streams, bulkheads, areas adjacent to flood plains, etc.) all free-draining gravel or sand (classified by USCS as GP, GW, SP or SW) must be used as infill up to 1 foot above the 100 year flood elevation, the high water level or the top of berm/spillway. This gravel or sand must be free draining and have less than 10% fines. Filter fabric (Mirafi 140N or approved equivalent) must go in front of the buried block, under the leveling pad, under the reinforced geogrid zone, behind the reinforced geogrid zone (vertically up to the extent of the gravel or sand infill) and on top of the gravel or sand infill (horizontally). This is required to prevent the migration of fines into the gravel or sand infill. Rip rap is required in front of the bottom three courses on walls installed near tidal waters or waters subject to wave action. Rip rap is also required when indicated on the civil plans and where pipes with active water flow exit through the wall (to prevent scour) The scope of work for RA on this project does not include a scour analysis (walls that have interface with flowing water must have a scour analysis performed to prevent undermining of the sub-grade). Contact RA if a scour analysis is needed.

B. Walls installed in ponds that hold water for extended periods of time may require special precautions. If the foundation soils (subgrade) are not free-draining (classified by USCS as GP, GW, SP or SW) the following shall be done to prevent the wall's sub-grade from being compromised: an 8" impermeable clay layer (soils classified by USCS as CL, GC or SC) must be placed as the final 8" of soils (in front of the first block below grade). The clay layer shall extend forward from the base of the wall a minimum of 20' laterally into the pond. This will ensure that any water seepage does not intersect a 45° slope (1H:1V) downward from the front edge of the wall's leveling pad.

3.12 Responsibilities of the Site Geotechnical Engineer (construction oversight)

A. The site geotechnical engineer's responsibilities include, but are not limited to, verifying the following: sub-grade bearing capacity (must meet or exceed that stated in the RA Final Design); soil friction angles (foundation, infill and retained); soil compaction (≥95% of a standard proctor or ≥93% of a modified proctor maximum dry density); geogrid type (manufacturer and strength), length, coverage, orientation and elevation; placement of drainage aggregate (12" minimum); placement and proper flow of drainpipes; wall geometry (wall must not exceed height of RA Final Design); site geometry (slopes and surcharges must be equal to or less than the RA Final Design and the civil plans); construction techniques (must be in accordance with previously stated procedures); water management (placement of swales, clay layers/geomembranes, flow of surface water to inlets, etc.), presence of groundwater under and behind the walls (retained zone) and all other activity that may impact the wall's integrity. RA must be notified if actual field conditions differ from the Final Design parameters and/or if there are activities on site that are not in accordance with these Specifications.

B. Field changes to the RA Final Design must be approved by RA in writing. Request(s) for design changes/field modifications must be made to RA in writing prior to RA issuing a written response. All correspondence must be documented with formal written response. These additional services are not included in RA's original scope of work and design fee. Therefore, a contract modification/change order must be signed by RA and the client prior to these additional services being provided.

3.13 Rails, Fences & Other Structures

A. RA recommends installation of safety fence or railing for walls with exposed height 30" or taller (varies by local jurisdiction). Typical non-load bearing fence/rail post foundation details has been provided (see sheet 8) for the wall installer. This should be co-ordinated with the fence/rail installer.

B. As a general guidance, all post foundations, including guard rails subject to vehicular impact must be kept back a minimum of 3 feet from the face of the wall to prevent any loading on the wall.

3.14 Storm Structures & Utilities

A. Reinforced Concrete Pipes (RCP) may pass through the leveling pad or wall structure without additional means of support (it must be verified from the pipe manufacturer that the pipe can withstand a load equal to or greater than that exerted by the wall-as stated in the RA Final Design Structural Notes under "Bearing Capacity"). The SRW units may be cut to fit around the pipe and the voids filled with non-shrink grout or type "M" mortar. A concrete collar may be cast around the structure if desired for ease of construction and aesthetic considerations. When a collar is cast, the top of the collar must line up with an even block course to maintain proper alignment, neat workmanship and to eliminate horizontal cutting of blocks. See structural notes for specific design requirements

B. The wall may not bear on plastic or steel pipes (such as ADS, CMP, HDPE, PVC, SLCPP, etc.) or utilities (such as electric, gas, phone lines, sewer or water lines, etc.). Grade beams or lintels must be used to bridge these non-load bearing structures. If a specific grade beam or lintel is not specified in the RA Final Design, RA shall be consulted to determine the size, strength and reinforcing of the grade beam or lintel. If these non-load bearing pipes or utilities are located at minimum of 42 inches below the wall's leveling pad then a grade beam or lintel is not necessary.

C. Concrete storm structures may be located behind a wall and be within the reinforced geogrid zone as dictated by the project's civil drawings. If the structure(s) cannot be moved out of the reinforced geogrid zone and the geogrid cannot be installed to its full design length the following shall apply. On small structures (such as collection boxes, concrete pipes less than 18 inches, inlets, manholes, etc.) it is acceptable to shorten the geogrid from the design length and meet the structure. The area between the wall and structure where the geogrid has been shortened must be filled with gravel (#57 or equivalent) and not soils. The gravel must be placed in maximum loose lifts of 8" and be compacted with a minimum of three passes by a vibratory plate compactor. For tall narrow structures the geogrid should be "looped" around the structures for additional lateral support. On large structures and in cases where pipes parallel the wall for long distances, RA shall be consulted to determine the impact on the wall before allowing this to be done. Where the geogrid is shortened an extra layer of geogrid is required immediately above and below the structure. See structural details and notes for further guidance.

D. The wall's integrity may be compromised if pipes or structures burst or develop leaks and allow water or fluids to saturate the reinforced geogrid zone. RA is not responsible for wall failure that results from pipes or structures that burst or leak and allow water or fluids to saturate the reinforced geogrid zone. To prevent damage to the wall structure a bed of clean gravel shall be placed under all pipes (minimum of 6" deep), shall extend up to the "spring line" (half way up) and connect to the front drainage layer (see detail(s) on previous sheets). Positive flow of the gravel must be maintained away from the wall structure.

3.15 Construction Adjacent to Completed Wall

A. The owner or owner's representative is responsible for ensuring that construction adjacent to the wall by others does not disturb the wall or place temporary construction loads on the wall that exceed design loads, including loads such as water pressure, temporary grades, or equipment loading. Heavy paving or grading equipment shall be kept a minimum of three feet behind the back of the wall face. Equipment with wheel loads in excess of 150 PSF live load shall not be operated with 10 feet of the face of the retaining wall during construction adjacent to the wall. Care should be taken by the general contractor or owner to ensure water runoff is directed away from the wall structure until final grading and surface drainage collection systems are completed.

B. Care must be taken when installing appurtenances (such as generators, transformers, etc.) or utilities above or behind the wall (within the reinforced geogrid zone). The compaction integrity of the reinforced geogrid zone must be maintained, both below and beside (around) the appurtenance or utility. Neglecting to do so may cause hydrostatic pressure and wall failure.

3.16 Storm Water Management & Slopes

A. The segmental retaining wall is not a storm water management structure. The wall can accommodate the rainfall above the reinforced geogrid zone but not the watershed (including the retained zone). Therefore it is absolutely essential that surface water be prevented from entering (and ultimately saturating) and ponding above the reinforced geogrid zone. This is usually accomplished by the site engineer (owner's civil engineer) grading the surface behind the wall to direct surface water to swales that divert the water around the wall ends, to inlets or over the top of the wall (sheet flow or through scuppers). If water is directed to the wall (such as applications with back slopes), the top 8 inches of compacted fill over the reinforced geogrid zone must be impermeable soils (CL, GC or SC) or low impermeable soils (classified by USCS as ML with LL ≥25, PI between 4 and 22, ≥80% passing the #200 sieve and ≤20% retained on the #200 sieve). If clayey soils are not readily available an underlying geomembrane (geosynthetic liner) may also be used. This geomembrane shall be a composite structure (Mirafi MCF-1212, Mirafi NicoTarp 100 or approved equivalent) and should be laid under the swale or over top of the part of geogrid zone that is prone to ponding.

B. The site geotechnical engineer is responsible for verifying the stability of slopes and management of sub-grade groundwater issues on the project. RA's scope includes only the wall design, not the evaluation of back slopes (above walls), front slopes (at the base of walls) or resolving groundwater issues. RA performs global stability analyses on walls that rest on major (4:1 or steeper) front slopes, however only the wall is analyzed, not the actual slopes above or below the wall. It is the responsibility of the site geotechnical engineer to determine if the site soils are able to sustain the proposed grades. If not, they shall determine and specify the additional reinforcement that is necessary to provide the proper slope stability and prevent erosion.

C. The general contractor, owner, site contractor and/or wall installer must provide for proper wall drainage to prevent the buildup of hydrostatic pressures over the service life of the structure. In the event additional water is introduced into the general wall area, either above or below grade, the RA Final Design will be invalid (the exception is "water applications" where free-draining gravel or sand is used for infill and it is wrapped in filter fabric with the design intent for the wall to be interacting with water).

D. Drains (such as downspouts, roof drains, sump pumps, etc.) for other structures (such as buildings, houses, other walls, etc.) must pass through the wall (solid pipe- not perforated or slotted). They cannot be connected to the wall's drainage system or discharge into the wall's reinforced geogrid zone.

3.17 Parking Lots & Roadways

A. If a paved parking lot or roadway is proposed above the wall it must be properly maintained and have proper suface water management. Typically curbing is installed to catch and divert the surface water runoff to inlets or ponds. All joints between the asphalt and the curbing must be sealed to prevent water from seeping into the walls reinforced geogrid zone. The owner must regularly (semi-annually) inspect the pavement for cracks and repair them immediately if discovered. If cracks remain open they could allow surface water runoff or melting snow to saturate the wall's reinforced geograd zone or retained zone which can lead to wall failure.

B. RA makes an effort to keep the topmost layer of geogrid two or three courses down from the top of the wall when there is pavement above. This is done so that the geogrid is properly embedded in sub-soils and not lying on top (since there is the depth of the granular sub-base material and the pavement depth to account for). Also, in many cases, RA designs the wall with the "TW" elevation high enough so that the back of the curb is covered (normally required by the civil engineer and required for support of the curbing). Therefore, the top layer of geogrid in some cases must be cut during pavement and/or curb installation (especially when a wall steps continuously on top). It is acceptable to cut the topmost layer of geogrid only if it is at the same elevation as the pavement or curbing (since in this case the wall is not absorbing a lateral load-only holding back "dead space" between the pavement or curbing and itself). However, the second layer of geogrid down from the top must never be cut.

C. If snow is plowed/pushed and stored above a wall its load can not exceed a height of 5'. If there are definite plans for storage of accumulated snow before the wall is built, RA, upon request, can design the wall to accommodate more than 5' of snow (additional geogrid will be required). Snow melt must not be allowed to saturate the wall's reinforced geogrid zone.

3.18 Landscaping

A. The walls on this project were designed ONLY for grass seeding and no other plants. If some other plantings are planned, RA must review the landscaping plan and design a water management system to protect the walls. No plants are allowed on top of the walls without RA's permission

A. All walls on this project must be inspected by a qualified geotechnical engineer licensed in the state of construction (or his or her designated representative) quarterly for walls equal to and over 15' in exposed height and semi-annualy for walls 6' and over and under 15' in exposed height. Digital photographs should also be taken at the time of inspection.

3.20 Post Construction Responsibilities

A. Retaining walls are a substantial financial investment. Therefore it is in the owner's best interest that a wall maintenance budget be established within the overall property management budget to monitor and provide preventative maintenance. Retaining wall maintenance, at a minimum, should consist of: checking drainage, inspecting for settling and surveying to verify alignment and batter. This service should be by qualified personnel under the supervision of a licensed geotechnical/structural engineer. RA can provide this service for an additional fee.

B. RA SHOULD BE NOTIFIED AS SOON AS REASONABLY POSSIBLE IF THE RETAINING WALL EXHIBITS CONDITIONS CONTRARY TO THE RA FINAL DESIGN SO THAT RA MAY BE CONSULTED TO PROTECT THE OWNER'S INVESTMENT.

Part 4: Safety 4.0 Site Safety

A. Site Safety is the sole responsibility of the contractor(s) (GC, all subcontractors including segmental wall installer) on-site in strict accordance with all state and federal safety regulations END OF SECTION Revised 12-07-07

The information contained herein is proprietary and is the sole property of RA. It is only intended for use on this project. Reuse of these drawings, sketches, and design computations in any manner is strictly prohibited without written approval from RA. Any other use is subject to penalty of law.

SDP-89-254

REVISIONS REVISED INFILL DUE TO POOR ON SITE SOILS SITE DEVELOPMENT PLAN SUBMITTAL TO COUNTY, ADDENDUM TO SDP-89-254

CLIENT: U.S. LAWNS OF BALTIMORE DRAWN BY: JO DESIGN BY: JO/CH OWNER: DOLBEN COMPANIES CHECKED BY: WKR IOB No: 1123-07-03 DATE: 3/06/08 O NOT SCALE THIS DRAW

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ANCHOR DIAMOND PRO RETAINING WALL DESIGNS SPECIFICATIONS

ASHTON WOODS

FALLS RUN ROAD, ELLICOTT CITY, MD 21043-7260



I hereby certify that these locuments were prepared or approved by me, and that I am a fully licensed professional engineer under the laws of the State of Maryland. WWar-Villam K. Ryan, P∖E.

icense No: 21586

xpiration Date: 05/09/2009

PROFESSIONAL CERTIFICATION

SHEET