GENERAL NOTES

1.) THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS/BUREAU OF ENGINEERING/CONSTRUCTION INSPECTION DIVISION AT 410-313-1880 AT LEAST FIVE (5)

2.) THE CONTRACTOR SHALL NOTIFY "MISS UTILITY" AT 1-800-257-7777 AT LEAST 48 HOURS PRIOR TO ANY EXCAVATION WORK BEING DONE.

3.) THIS PROJECT IS IN CONFORMANCE WITH THE LATEST HOWARD COUNTY STANDARDS UNLESS

29.) TRAFFIC CONTROL DEVICES, MARKINGS AND SIGNING SHALL BE IN ACCORDANCE WITH

THE LATEST EDITION OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).

ALL STREET AND REGULATORY SIGNS SHALL BE IN PLACE PRIOR TO THE PLACEMENT OF

ACCORDANCE WITH THE HOWARD COUNTY DESIGN MANUAL, VOLUME III (1993) AND AS

RIGHT-OF-WAY SHALL BE MOUNTED ON A 2" GALVANIZED STEEL, PERFORATED, SQUARE

16.1205(a)(7) TO PROCESS AND USE AN APPROVED GRADING PLAN INSTEAD OF A SITE

1. COMPLIANCE WITH THE REQUIREMENT OF THE FOREST CONSERVATION ACT OF

HOWARD COUNTY FOR THIS PROJECT SHALL BE DEFERED TEMPORARILY TO THE

2. COMPLIANCE WITH THE MSHA COMMENTS INCLUDED WITH THE DED COMMENTS

THIS AMOUNTS TO 15 UNITS (10% OF 150), PER SECTION 13.402A(c) OF THE HOWARD

COUNTY CODE, LOW INCOME HOUSING UNITS (L.I.H.U) SHALL BE PROVIDED IN-LIEU-OF

35.) THE TWO DRIVEWAY PARKING SPACES PER UNIT SHALL ONLY BE USED FOR THE

PARKING OF MOTOR VEHICLES ON A TRANSIENT BASIS PER ZONING REGULATIONS

36.) TRASH COLLECTION: TRASH COLLECTION FOR THIS SUBDIVISION SHALL BECOME

16.115(c)(2), TO BE PERMITTED TO CONSTRUCT A PAVED PATHWAY WITHIN THE ONSITE

PUBLIC ROAD WHICH PROVIDES ACCESS TO THE PROPERTY FOR THE PROPOSED BULK

. THE PRIVATE ROADS SHALL BE MAINTAINED BY THE HOA AND THE OWNERS OF

11/04/2011 FOR THIS WAIVER PETITION RECONSIDERATION SHALL BE RELOCATED

SOUTHWARD TO THE NON-FORESTED AREA OF THE "EXISTING RIGHT-OF-WAY FOR

PARCEL 'A' (ZONED "POR" WITH FRONTAGE ON TWO, EXISTING, VEHICULAR ACCESS

PROJECT FRONTAGE WAS APPROVED ON DECEMBER 1, 2011 WITH THE FOLLOWING

EXHIBIT/PLAN SUBMITTED ON 11/04/2011 FOR THIS WAIVER PETITION

OF HARRIET TUBMAN LANE BRIDGE SHALL BE PROVIDED AS SHOWN ON THE

THE REGULATIONS, REQUESTS, ACTION, CONDITIONS OF APPROVAL, AND THE

38.) EMERGENCY ACCESS ROAD AND SIGNS MUST MEET DFRS REQUIREMENTS. PAVERS

10.656

11,021

7,585

9.682

7.668

10,921

11.083

4,791

11,079 8,572

11,145 8,511

0.96

1.31

1.03

1.02

0.38

1.23

0.99

1.23

0.87

1.33

0.97

0.76

294

261

338

849

375

227

314

396

208

143

376

11,995 24,945

SWM SUMMARY TABLE

836

616

900

700

810

223 445 PASS

674

523

382

605

PASS

529 PASS

552 PASS

4.0

4.0

4.0

4.0

4.0

3.0

4.0

4.0

4.0

4.0

4.0

1353

1442

1614

1936

1208

1196

1189

49.029 43.407

1458

PASS

Provided 75% ESDv

484

234

560

669

554

569

587

0

0

0

0

497

296

565

576

0

0

648

652

356

0 .

0

7,48Z 10,765

EXHIBIT/PLAN SUBMITTED ON 11/04/2011 FOR THIS WAIVER PETITION

USED FOR ACCESS MUST MEET FD WEIGHT REQUIREMENTS.

12,912

8,153

20,627

14.701

12,132

16,147

23,434

13,226

19,745

13,053

16.875

42,431

12,231

19,330

19,630

15.708

19.791

9,332

10,250

16,635

19.476

12.684

8.822

7,173

18,782 7,767

599*750* | 3**33**, 839

17.185

RECONSIDERATION, WITH THE FOLLOWING ADDITIONAL CONNECTIONS:

BUILDABLE BULK PARCEL 'A'.

INTERCEPTOR SEWER 1132/129".

THE STREET.

RECONSIDERATION.

APPROVAL DATE.

(M-6)

(M-6)

(M-6)

(M-6)

(M-8)

(M-6)

(M-6)

(M-6)

(M-6).

(M-6)

10 & 11 (M-6)

MIHU'S AT A RATIO OF 1:3 AS APPROVED BY THE HOWARD COUNTY HOUSING

COMMISSION.. THE LIHU'S SHALL BE LOCATED ON LOTS 2, 4, 9, 129 & 141.

DEVELOPMENT PLAN STAGE, AND TO GRANT PERMISSION FOR THE REMOVAL OF 13

FUTURE SITE DEVELOPMENT PLAN FOR THIS SITE.

COMPLIANCE WITH THE DRP COMMENTS DATED 09/09/2011.

MODIFIED BY "GUIDELINES FOR STREET LIGHTS IN RESIDENTIAL DEVELOPMENTS (JUNE 1993)". A MINIMUM SPACING OF 20' SHALL BE MAINTAINED BETWEEN AND STREET LIGHT

30.) STREET LIGHT PLACEMENT AND THE TYPE OF FIXTURE AND POLE SHALL BE IN

TUBE SLEEVE (12 GAUGE) - 3' LONG. A GALVANIZED STEEL POLE CAP SHALL BE MOUNTED ON TOP OF EACH POST.

32.) ALL PLAN DIMENSIONS ARE TO FACE OF CURB UNLESS OTHERWISE NOTED.

4.) THE SUBJECT PROPERTY IS ZONED R-SA-8 AND POR PER THE 2-2-2004 COMPREHENSIVE ZONING PLAN AND THE "COMP LITE" ZONING AMENDMENTS EFFECTIVE

5.) COORDINATES BASED ON NAD '83, MARYLAND COORDINATE SYSTEM AS PROJECTED BY HÓWARD COUNTY GEODETIC CONTROL STATIONS 3511 AND 3512.

6.) TRACT BOUNDARY IS BASED ON A FIELD RUN BOUNDARY SURVEY PERFORMED BY SILL, ADCOCK AND ASSOCIATES, INC. IN MAY, 2007.

7.) THE EXISTING TOPOGRAPHY ON-SITE SHOWN WAS FIELD SURVEYED BY SILL, ADCOCK AND ASSOCIATES, INC. IN MAY, 2007. THE EXISTING TOPOGRAPHY FOR RAMP IMPROVEMENTS AND TURNAROUND ON GRACE DRIVE WAS FIELD SURVEYED BY BENCHMARK ENGINEERING, INC. IN

8.) A NOISE STUDY WAS PREPARED BY W.T. BALLARD, DATED MARCH, 2008 AND APPROVED ON JUNE 9, 2008. A REVISED STUDY WAS PREPARED BY MARS GROUP, INC. IN FEBRUARY, 2012 AND A REVISED STUDY FOR THE NOISE MITIGATION WAS PREPARED BY MARS GROUP, INC. IN 2012. THE MARS GROUP, INC. REPORTS WERE APPROVED ON MARCH 28. 2012.

9.) AN APFO TRAFFIC STUDY WAS PREPARED BY LEE CUNNINGHAM & ASSOCIATES, INC. DATED FEBRUARY, 2005 AND APPROVED UNDER SP-05-008. IT WAS AMENDED BY TRAFFIC CONCEPTS IN JUNE, 2007 AND THE TRAFFIC GROUP ON JUNE 6, 2008 AND APPROVED ON JULY 29, 2008. A REVISED TRAFFIC IMPACT STUDY WAS PREPARED BY THE TRAFFIC GROUP IN JANUARY, 2012 AND SUBMITTED WITH THIS SDP.

10.) THIS PROPERTY IS LOCATED WITHIN THE METROPOLITAN DISTRICT.

- 11.) WATER IS PUBLIC. THE CONTRACT NUMBER FOR PHASE 1 IS 34-4373-D.
- THE CONTRACT NUMBER FOR PHASE 2 IS 34-4739-D. 12.) SEWER IS PUBLIC. THE CONTRACT NUMBER FOR PHASE 1 IS 34-4373-D. THE CONTRACT NUMBER FOR PHASE 2 IS 34-4739-D.
- 13.) THIS SUBDIVISION IS SUBJECT TO SECTION 18.122B OF THE HOWARD COUNTY CODE PUBLIC WATER AND/OR SEWER SERVICE HAS BEEN GRANTED UNDER THE TERMS AND PROVISIONS, THEREOF, EFFECTIVE 6-14-2012, ON WHICH DATE DEVELOPER AGREEMENT #34-4373 & #34-4739 WAS FILED AND ACCEPTED.
- 14.) WETLAND AND FOREST STAND DELINEATIONS WERE CONDUCTED BY AB CONSULTANTS, INC. AND WERE SUBMITTED AND APPROVED UNDER S-05-008.
- 15.) THE 100YR FLOODPLAIN STUDY WAS PREPARED BY KIDDIE CONSULTANTS, INC. DATED JUNE 23. 1978 FOR HOWARD COUNTY, MARYLAND AND UPGRADED BY AB CONSULTANTS, INC. FOR SP-08-002 AND APPROVED ON JUNE 9, 2008.
- 16.) NO GRADING, REMOVAL OF VEGETATIVE COVER OR TREES, PAVING OR NEW STRUCTURES SHALL SE PERMITTED WITHIN THE REQUIRED WETLANDS, STREAM(S), THEIR BUFFERS, FOREST CONSERVATION EASEMENT AREAS OR 100YR FLOODPLAIN EXCEPT THAT ASSOCIATED WITH THE 8' MACADAM PATHWAY AS APPROVED UNDER WP-12-029.
- 17.) TO THE BEST OF OUR KNOWLEDGE, THERE ARE NO CEMETERY LOCATIONS ON THESE LOTS 18.) THERE ARE NO HISTORIC SITES/FEATURES LOCATED ON THESE LOTS.

19.) THE 65 dBA NOISE CONTOUR LINE DRAWN ON THIS DEVELOPMENT PLAN IS ADVISORY AS REQUIRED BY THE HOWARD COUNTY DESIGN MANUAL, CHAPTER 5, REVISED FEBRUARY, 1992, AND CANNOT BE CONSIDERED TO EXACTLY LOCATE THE 65 dBA NOISE EXPOSURE. THE 65 dBA NOISE LINE WAS ESTABLISHED BY HOWARD COUNTY TO ALERT DEVELOPERS, BUILDERS AND FUTURE RESIDENTS THAT AREAS BEYOND THIS THRESHOLD MAY EXCEED GENERALLY ACCEPTED NOISE LEVELS ESTABLISHED BY THE U.S. DEPARTMENT OF HOUSING AND URBAN DEVELOPMENT.

20.) STORMWATER MANAGEMENT IS PROVIDED IN ACCORDANCE WITH THE STORMWATER MANAGEMENT ACT OF 2007. ENVIRONMENTAL SITE DESIGN (ESD) HAS BEEN IMPLICATED TO THE MAXIMUM EXTENT PRACTICAL (MEP) BY THE USE OF 37 (M-6) MICRO-BIORETENTION FACILITIES AND 2 (M-8) BIO-SWALES. ALL FACILITIES ARE PRIVATELY OWNED AND MAINTAINED BY THE

21.) FINANCIAL SURETY FOR THE REQUIRED PERIMETER LANDSCAPING HAS BEEN POSTED AS PART OF THE DPW DEVELOPER'S AGREEMENT IN THE AMOUNT OF \$120,300.00 (\$61,200.00 FOR 204 SHADE TREES; \$11,100.00 FOR 74 EVERGREENS AND \$48,000.00 FOR 160 PRIVATE

RETENTION OF 4.51 AC. OF NET TRACT AREA FOREST WITHIN A FOREST CONSERVATION EASEMENT AND THE OFFSITE PLANTING OF 5.38 ACRES WITHIN THE FOREST MITIGATION BANK LOCATED WITHIN THE PRESERVE AT CLARKSVILLE SUBDIVISION (F-06-072). FINANCIAL SURETY F-06-072. NO SURETY WAS REQUIRED FOR THE ON-SITE RETENTION.

23.) DRIVEWAYS SHALL BE PROVIDED PRIOR TO ISSUANCE OF A USE AND OCCUPANCY PERMIT FOR ANY NEW DWELLINGS TO INSURE SAFE ACCESS FOR FOR FIRE AND EMERGENCY VEHICLES PER THE FOLLOWING MINIMUM REQUIREMENTS:

- WIDTH 12' (16' SERVING MORE THAN ONE RESIDENCE). b) SURFACE - 6" OF COMPACT CRUSHER RUN BASE WITH TAR AND CHIP COATING (1-
- c) GEOMETRY MAXIMUM 15% GRADE, MAXIMUM 10% GRADE CHANGE AND MINIMUM 45' TURNING RADIUS.
- d) STRUCTURES (CULVERTS/BRIDGES) CAPABLE OF SUPPORTING 25 GROSS TONS (H25 LOADING). e) DRAINAGE ELEMENTS - CAPABLE OF SAFELY PASSING 100 YEAR FLOODPLAIN WITH NO MORE THAN 1 FOOT DEPTH OVER DRIVEWAY.
- f) STRUCTURE CLEARANCES MINIMUM 12 FEET. a) MAINTENANCE - SUFFICIENT TO INSURE ALL WEATHER USE.

24.) IN ACCORDANCE OF SECTION 128 OF THE HOWARD COUNTY ZONING REGULATIONS, BAY WINDOWS, CHIMNEYS OR EXTERIOR STAIRWAYS NOT MORE THAN 16 FEET IN WIDTH MAY PROJECT NOT MORE THAN 4 FEET INTO ANY SETBACKS. PORCHES OR DECKS, OPEN OR ENCLOSED MAY PROJECT NOT MORE THAN 10 FEET INTO THE FRONT OR REAR YARD SETBACK. 25.) ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST STANDARDS AND SPECIFICATIONS OF HOWARD COUNTY PLUS MSHA STANDARDS AND SPECIFICATIONS IF

26.) THE PURPOSE OF LOT 152 IS FOR THE PROTECTION OF STREAM, 100 YR FLOODPLAIN, WETAND AND OTHER ENVIRONMENTALLY SENSITIVE AREAS. THIS LOT SHALL BE OWNED BY HOWARD COUNTY, MARYLAND.

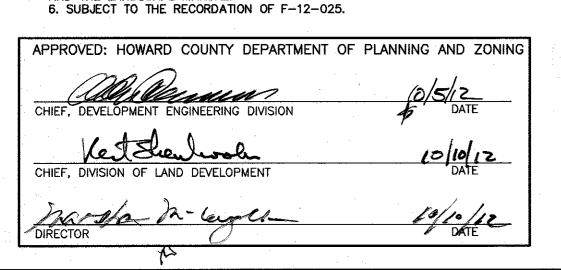
THE PURPOSE OF OPEN SPACE LOT 151 IS FOR PRIVATE ACCESS TO THE LOTS, FOR PRIVATE STORMWATER MANAGEMENT ESD PRACTICES, FOR RECREATIONAL OPEN SPACE AND FOR THE PROTECTION OF EXISTING TREES. THIS LOT IS PRIVATELY OWNED AND MAINTAINED BY THE

27.) THE DEVELOPER SHALL PROVIDE AN 8 FOOT WIDE MACADAM PATHWAY THAT EXTENDS FROM THE END OF GREEN MILL WAY, ALONG THE MIDDLE PATUXENT RIVER AND INTO THE EXISTING BRIDGE AT HARRIET TUBMAN LANE AND CONTINUE UNDER THE CEDAR LANE BRIDGE TO CONNECT TO THE ROBINSON NATURE CENTER.

28.) WP-12-029, A REQUEST TO WAIVE SECTION 16.120(C)(4), TO NOT BE REQUIRED TO PROVIDE ANY OF THE REQUIRED MINIMUM "SINGLE FAMILY ATTACHED" LOT FRONTAGE OF 15 FEET ON AN APPROVED PUBLIC ROAD AND, TO BE PERMITTED TO HAVE THE "SINGLE FAMILY ATTACHED" LOTS FRONT ON A PRIVATE ROAD EXCEEDING A LENGTH OF 200 FEET MEASURED. FROM THE EDGE OF A PUBLIC ROAD RIGHT-OF-WAY WAS APPROVED ON OCTOBER 6, 2011 WITH THE FOLLOWING CONDITIONS:

1. A HOMEOWNERS ASSOCIATION SHALL BE CREATED FOR THIS DEVELOPMENT IN ACCORDANCE WITH SUBDIVISION SECTION 16.121(c). 2. THE PRIVATE ROADS SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE DED, DPW AND DFRS. 3. THE PRIVATE ROADS SHALL BE MAINTAINED BY THE HOA AND THE OWNERS OF

4. SIDEWALKS SHALL BE PROVIDED ALONG BOTH SIDES OF ALL PRIVATE STREETS, AND ALONG CEDAR LANE, IN ACCORDANCE WITH THE REQUIREMENTS OF SUBDIVISION SECTION 5. STREET TREES SHALL BE PROVIDED ALONG BOTH SIDES OF ALL PRIVATE ROADS, AND CEDAR LANE, IN ACCORDANCE WITH THE REQUIREMENTS OF SUBDIVISION SECTION 16.124 AND THE LANDSCAPE MANUAL.

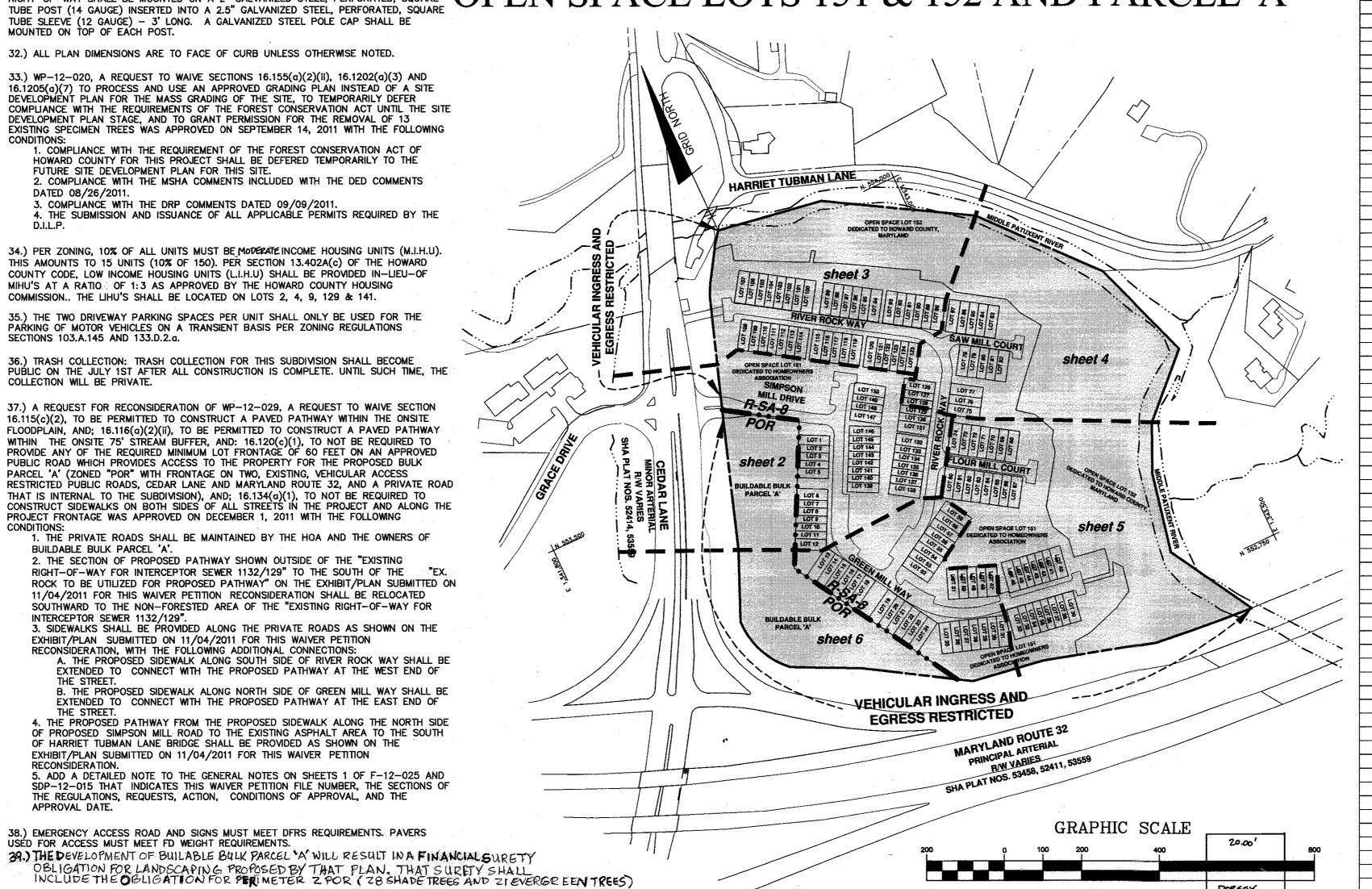


RESIDENTIAL SITE DEVELOPMENT PLAN

SIMPSON MILL

PHASE 1 - LOTS 1 thru 18 & 139 thru 150 PHASE 2 - LOTS 19 thru 138

OPEN SPACE LOTS 151 & 152 AND PARCEL 'A'



Green Mill Way Green Mill Way

6762

6766

6778

6784

6786

6796

6795

6785

6783

6781

7802

7804

7806

7808

7810

7814

7816

6804

6812

6814

6815

6805

6803

7842

7812

Green Mill Way

Green Mill Wav

Green Mill Way

Green Mill Wav

Green Mill Way

River Rock Way

Flour Mill Court

River Rock Way

STAMPED DISC SET ON TOP OF A 3' DEEP COLUMN OF CONCRETE 2" BELOW N 557110.3963' E 1344893.6204' ELEV. 400.034 HO. CO. #3512 STAMPED DISC SET ON TOP OF A 3' DEEP COLUMN OF CONCRETE 2" BELOW E 1342732.9971 N 555100.7743'

6708

6710

7864

7882

7884

7879

7873

7855

7845

7843

7841

7839

7829

7827

7825

6727

6725

6723

6721

6719

6717

6711

6709

146 6715

137 7823

138 7821

125 7853

128

82 6712

River Rock Way

River Rock Way

Saw Mill Court

River Rock Way

River Rock Wav

River Rock Way

River Rock Wav

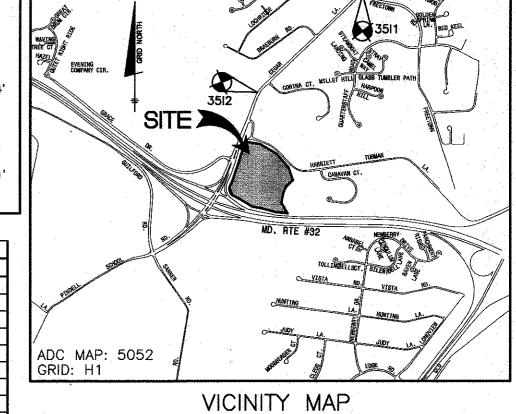
River Rock Way

Green Mill Way

6707 Green Mill Way

150 6705 Green Mill Way

DESIGN: DBT



Site Analysis Data	Sheet
Area of R-SA-8 zoned land	25.90 ac
Area of POR zoned land	3.42 ac
Total Project Area	29.32 ac
Limit of Disturbed Area	20.04 ac
Present Zoning Designation	R-SA-8 & POR
Proposed uses for site	Residential SFA homes
Total Number of Units Allowed	150
Total Number of Units Proposed	150
MIHU Required (10% of total units)	15
MIHU Provided	(see General Note 34
Number of Parking Spaces Required	345
(150 x 2.3 = 345)	340
Number of Parking Spaces Provided	345
Garages (2-car units): 106 x 2 = 212	040
Garages (1-car units): 44 x 1 = 44	(driveway spaces for 2-c
Driveway (1-car units): 44 x 1 = 44	units were not included
Overflow Street Parking Spaces: 45	spaces provided)
R-SA-8 Open Space Required: (25%)	6.47 ac
R-SA-8 Open Space Provided: (73.4%)	19.01 ac
POR Open Space Required:	N/A or 20% if ARAD
POR Open Space Provided:	deferred until developme
Recreational Open Space Required:	60,000 sf
(400 sf/unit)	00,000 81
Recreational Open Space Provided:	`
includes 63,072sf credit for pathway	102,053 sf
(21,024sf X 3 per 11-8-1994 DPZ memo)	
	S-05-008, P-06-115, SP-0
Applicable DPZ File References:	002, WP-07-072, ECP-11-0
repriousio Di Zi ilo Rojororioos.	WP-12-020, WP-12-029,
	F-12-025, F-12-059

Sheet index TITLE TITLE SHEET 2-6 SITE DEVELOPMENT PLAN 7-8 ROAD PROFILES AND DETAILS 9-13 GRADING, SEDIMENT & EROSION CONTROL PLAN SEDIMENT & EROSION CONTROL NOTES AND DETAILS STORM DRAIN DRAINAGE AREA MAP 15-16 17-19 STORM DRAIN PROFILES 20-21 STORMWATER MANAGEMENT DETAILS 22-23 LANDSCAPE PLAN 24 - 25FOREST CONSERVATION PLAN 26-27 SOILS BORING LOGS RETAINING WALL DETAILS

GENERAL NOTES CONTINUED:

11-18-14 ADD LANDSCAPE NOTE & PATHWAY APPROVAL NOTE TO GENERAL NOTES

40) WP-13-035, A PEQUEST TO WAIVE SECTION 16.104(P) TO EXTEND THE MILESTONE DATE FOR THE PAYMENT OF FEES AND POSTING OF FINANCIAL OBLIGATION FOR THE DEVELOPERS AGREEMENT AND SECTION 16.144(4) TO EXTEND THE MILESTONE DATE FOR SUBMISSION OF ORIGINAL SUBDIVISION PLAT WAS APPROVED ON SEPTEMBER 26, 2012 SUBJECT TO

THE FOLLOWING CONDITIONS:

L' THE DEVELOPERS AGREEMENT MUST BE COMPLETED AND THE PLAT ORIGINAL MUST BE SUBMITTED ON OR BEFORE NOVEMBER 3, 2012 FOR F-12-025.
2. COMPLIANCE WITH ALL SUBDIVISION REVIEW COMMITTEE COMMENTS.

3. PROVIDE THIS NOTE ON F-12-025 AND SDP-12-015.

9-15-2014 SHOW 12'410' SUNKOOM ON DORSEY/LINWOOD ON SAME SIDE AS GARAGE. 1-3-2014 REVISE SWM SUMMARY TABLE FOR D.A. 22 DATE BENCHMARK ENGINEERS & LAND SURVEYORS & PLANNERS ENGINEERING, INC 8480 BALTIMORE NATIONAL PIKE A SUITE 418 A ELLICOTT CITY, MARYLAND 21043 (P) 410-465-6105 (F) 410-465-6644 60 THOMAS JOHNSON DRIVE ▲FREDERICK, MARYLAND 21702

DRAWN: DBT

WWW.BEI-CIVILENGINEERING.COM

were prepared or approved by me, and that I am a duly license professional engineer under the laws of the State of Maryland License No. 28559, Expiration Date: 7-22-2013.

OWNER: PHASE 1 - LOTS 1 thru 18 & 139 thru 150 SIMPSON MILL, LLC PHASE 2 - LOTS 19 thru 138 P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041 OPEN SPACE LOTS 151 & 152 AND PARCEL 'A' 410-465-4244 PARCEL: 116 ZONED: R-SA-8 GRID: 23 PARCEL: 258,476 ZONED: POR **ELECTION DISTRICT NO. 5** HOWARD COUNTY, MARYLAND P.O. BOX 417 TITLE SHEET ELLICOTT CITY, MARYLAND 21041 410-465-4244 AUGUST, 2012 BEI PROJECT NO: 2189

AS SHOWN

SCALE:

approved changing the 52 FROM MACADAM TO 4 9.6.16 correct summary table for DA32 and totals based on revisions to DA32 and DAZZ from Revision # 1.

	[ri]		1,5	7042 NVCI IX	OCK VVAY 100
. :	33.83	20.00' Box 'E'	41.) THE	DIRECTOR OF RECRE PATH WAY WITH IN CONE (CRG & CRS)	ation and parks a dren space Lot is
		12.00'	12.00'		
OPT. AREAWAY	OPT. 12'XID' SUNROOM OPT. 12.00':0 AREAWAY	12.00' EDINBURGH (SUNROOM) GLASGOW FOR SUNROOM) SUNROOM) COUNROOM) COUNROOM	SS. 24 SCHOOOM) CONBOOM) (SUNROOM) (SUNROOM) (SUNROOM)	45.33, DORSEY 45.33, 233,	EDINBURGH GASGOW DORSEY 23, LINWOOD 24,00'
OPT. SUNROOM (X)	OPT. SUNROOM	(SUNROOM) LINWOOD (SUNROOM) 24.00'	20.00'	20.00'	24.00'
2-CAR EE	1-CAR .55	<u> </u>	BOX 'B'	BOX 'C'	BOX 'D'
2-CAR ÉS Q	1-CAR E		GENERIC SCALE:		
24.00'	20.00				•
EDINBURGH	DORSEY	PE	RMIT INFOR	RMATION CH	ART

DORSEY

(SUNROOM)

LINWOOD

(IN FEET)

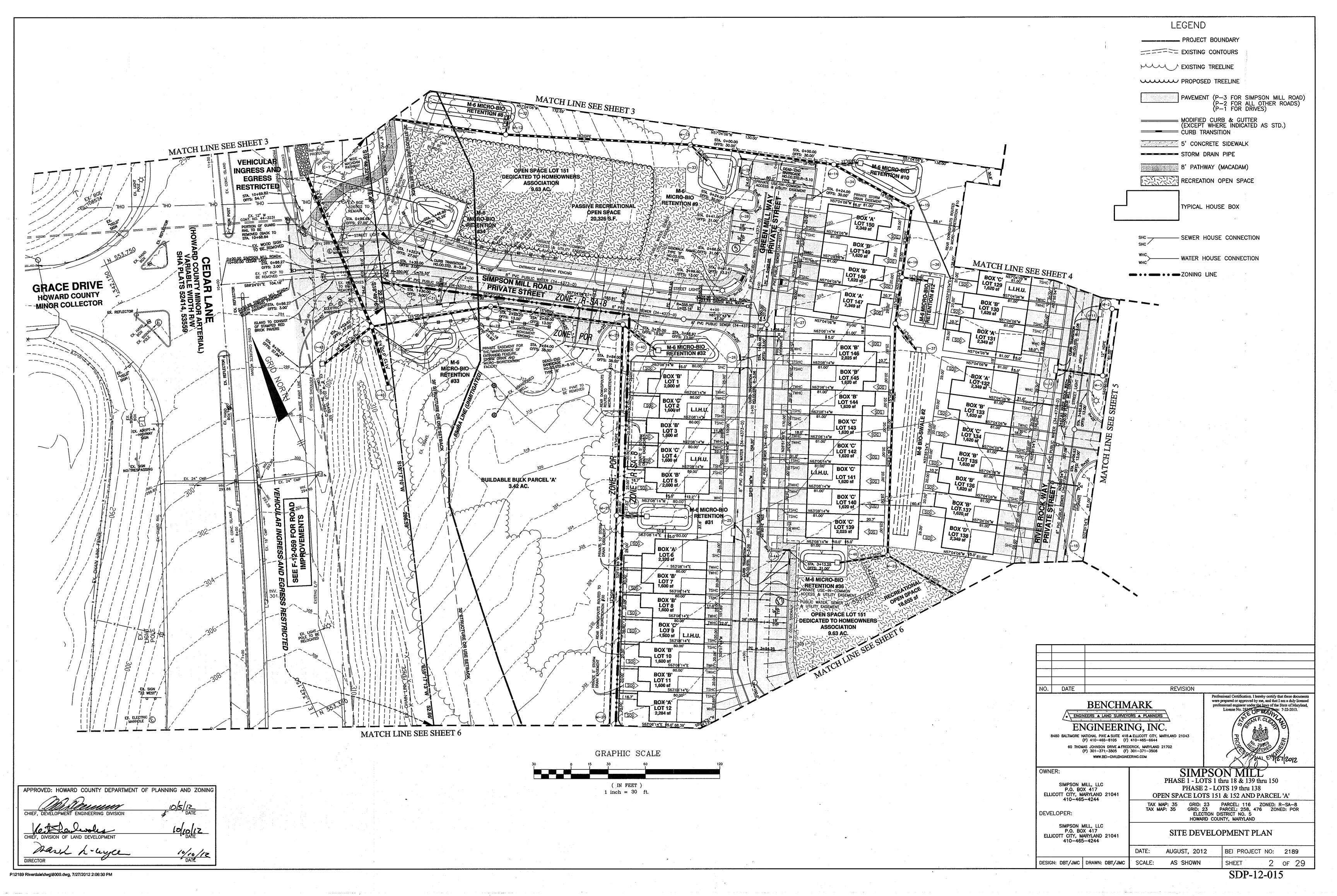
1 inch = 200 ft.

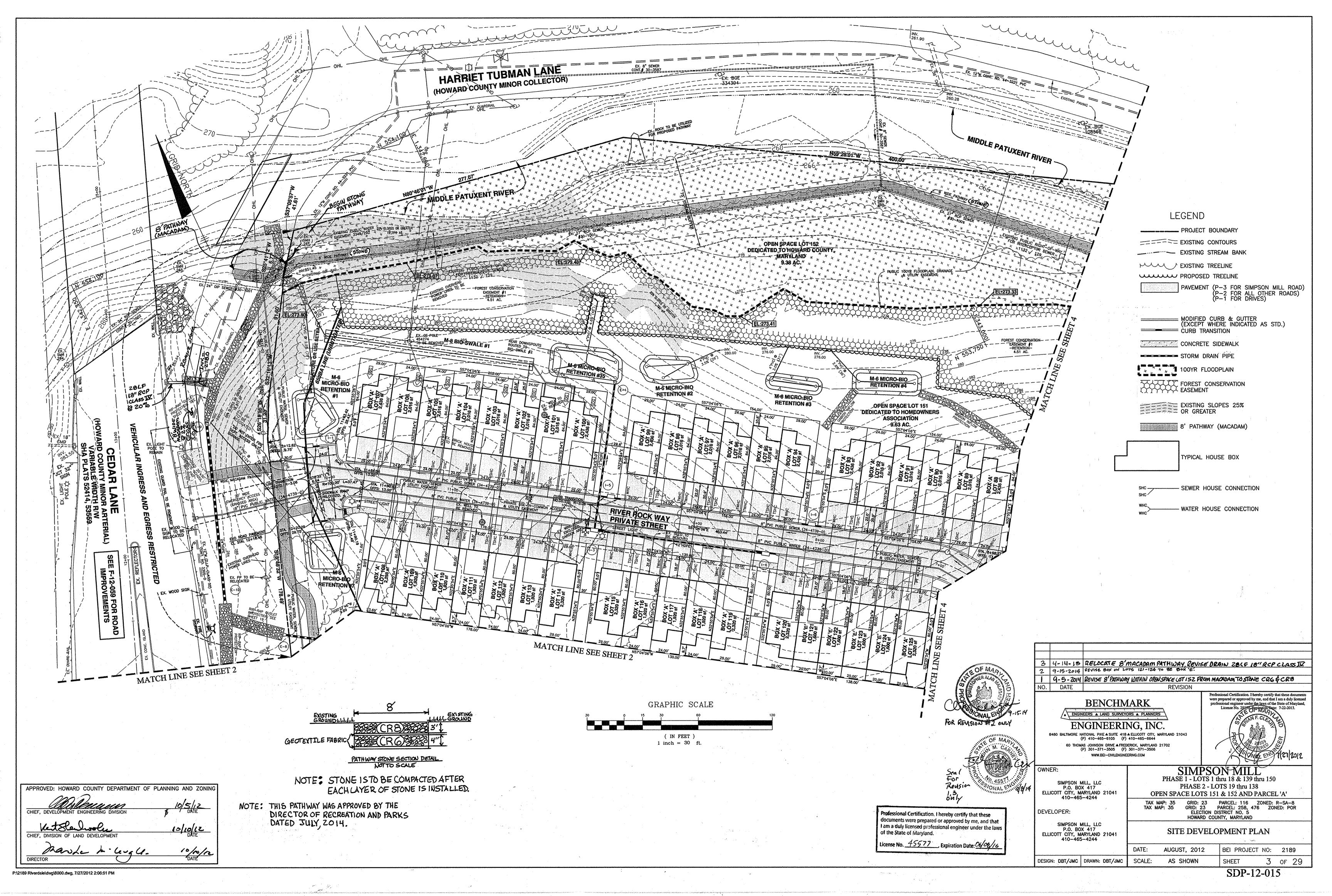
LINWOOD <u>GLASGOW</u> SCALE: 1"=20'

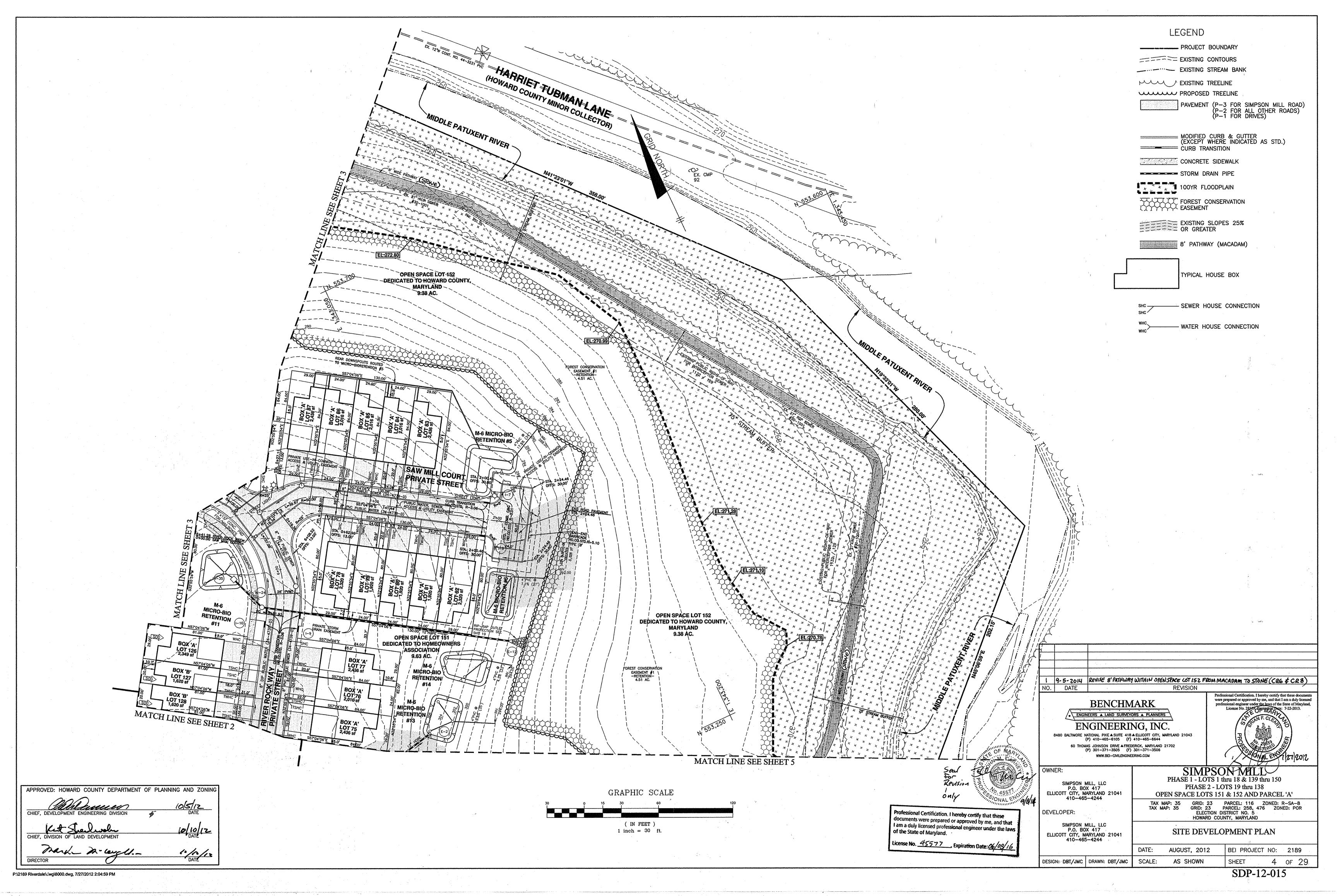
SECTION/AREA LOT/PARCEL # LOTS 1-150, OPEN SPACE LOTS 151-152 AND BUILDABLE PARCEL 'A' TAX MAP NO CENSUS GRID No. ZONE FI ECTION DISTRIC1 TRACT 22093-22101 23 35 6056.02

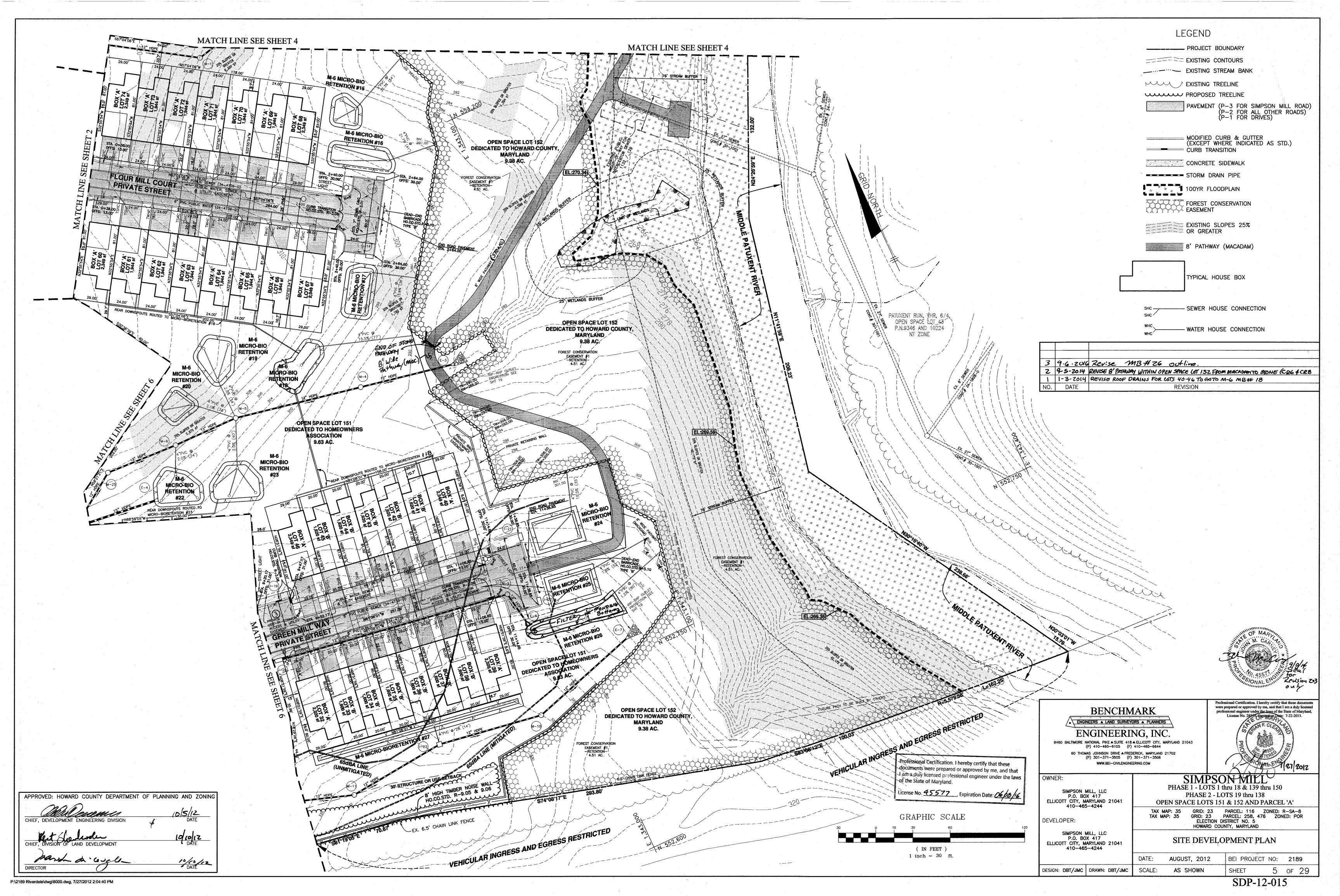
P:\2189 Riverdale\dwg\8009.dwg, 7/27/2012 2:03:28 PM

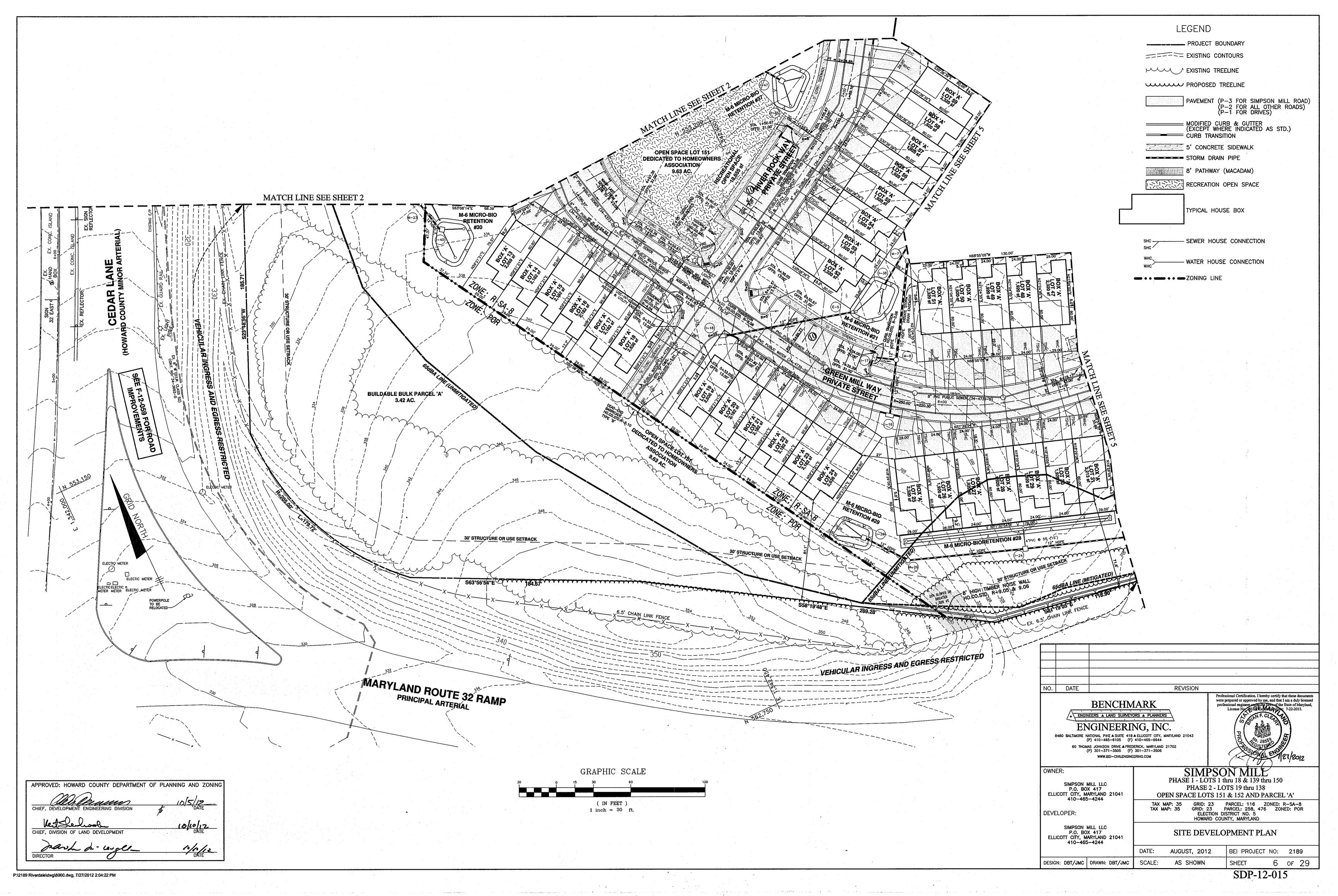
BUILDABLE BULK PARCEL 'A'.

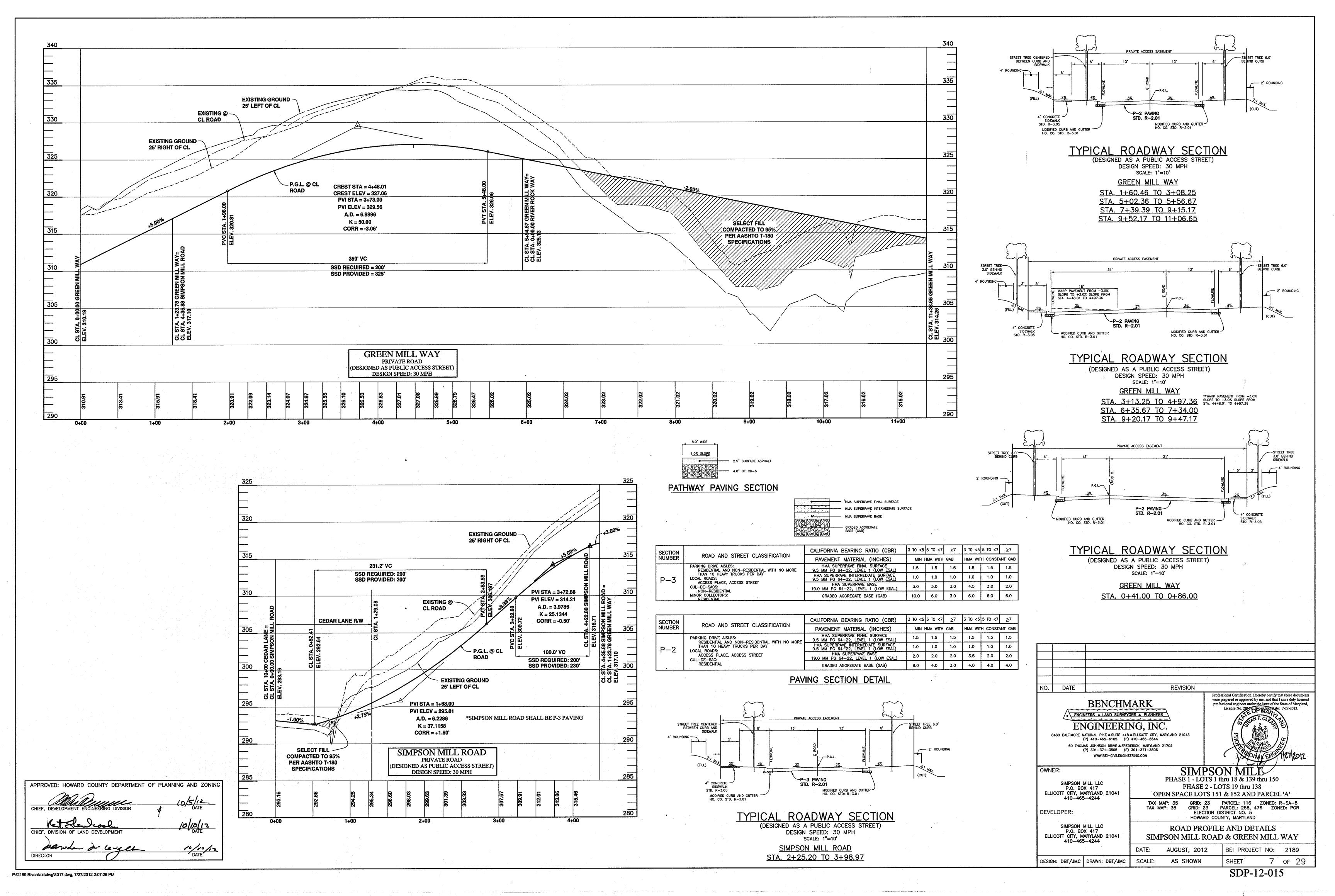


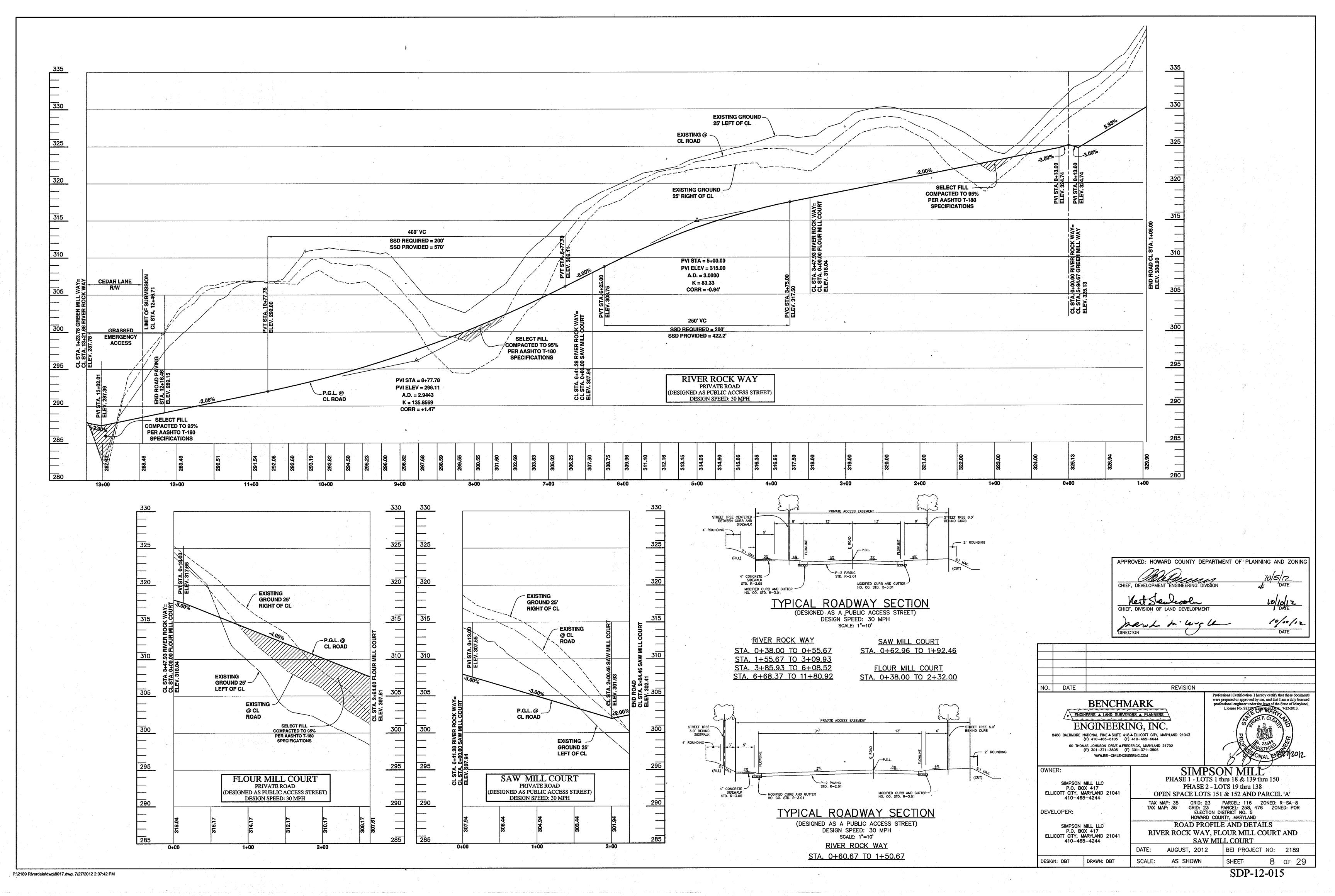


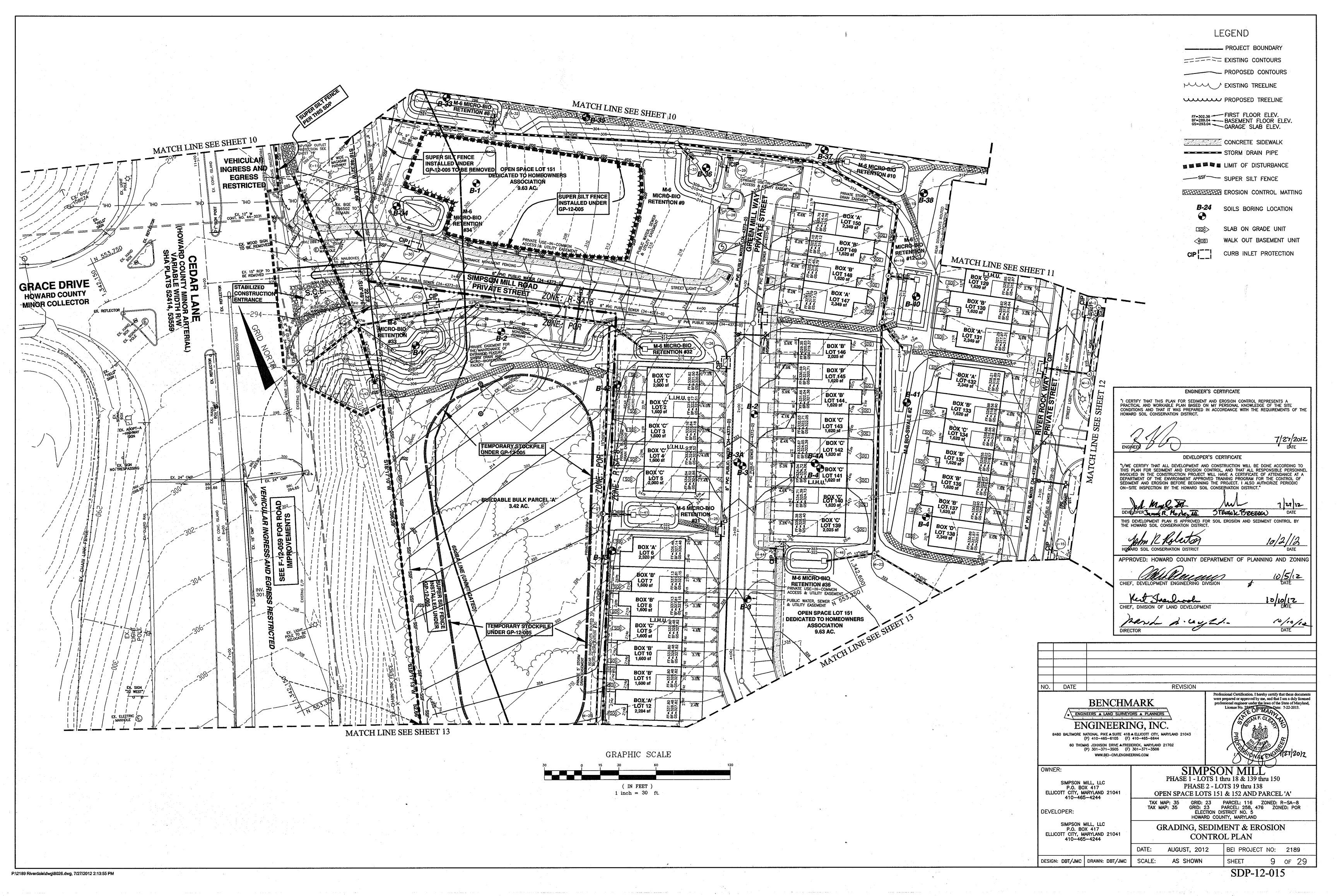


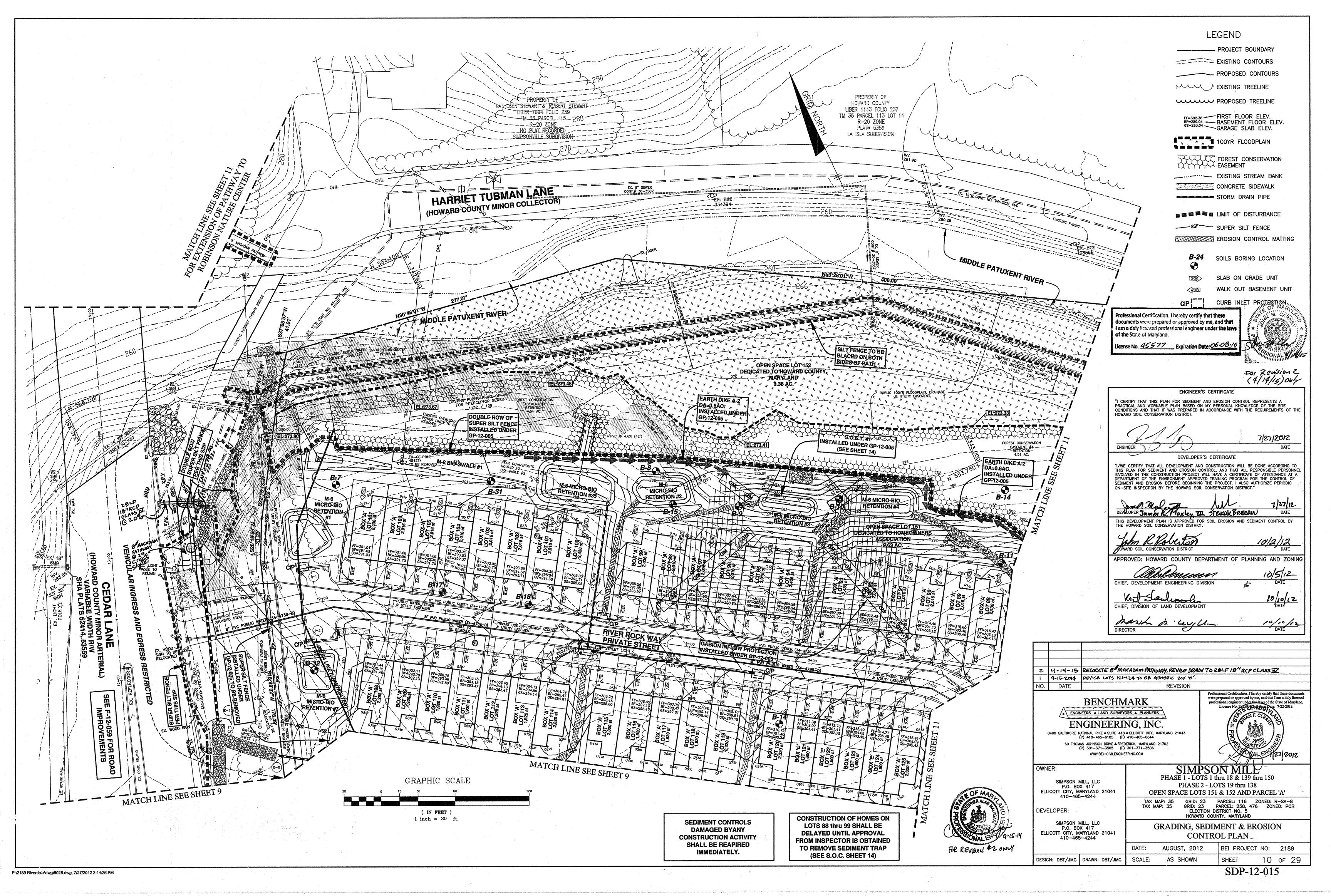


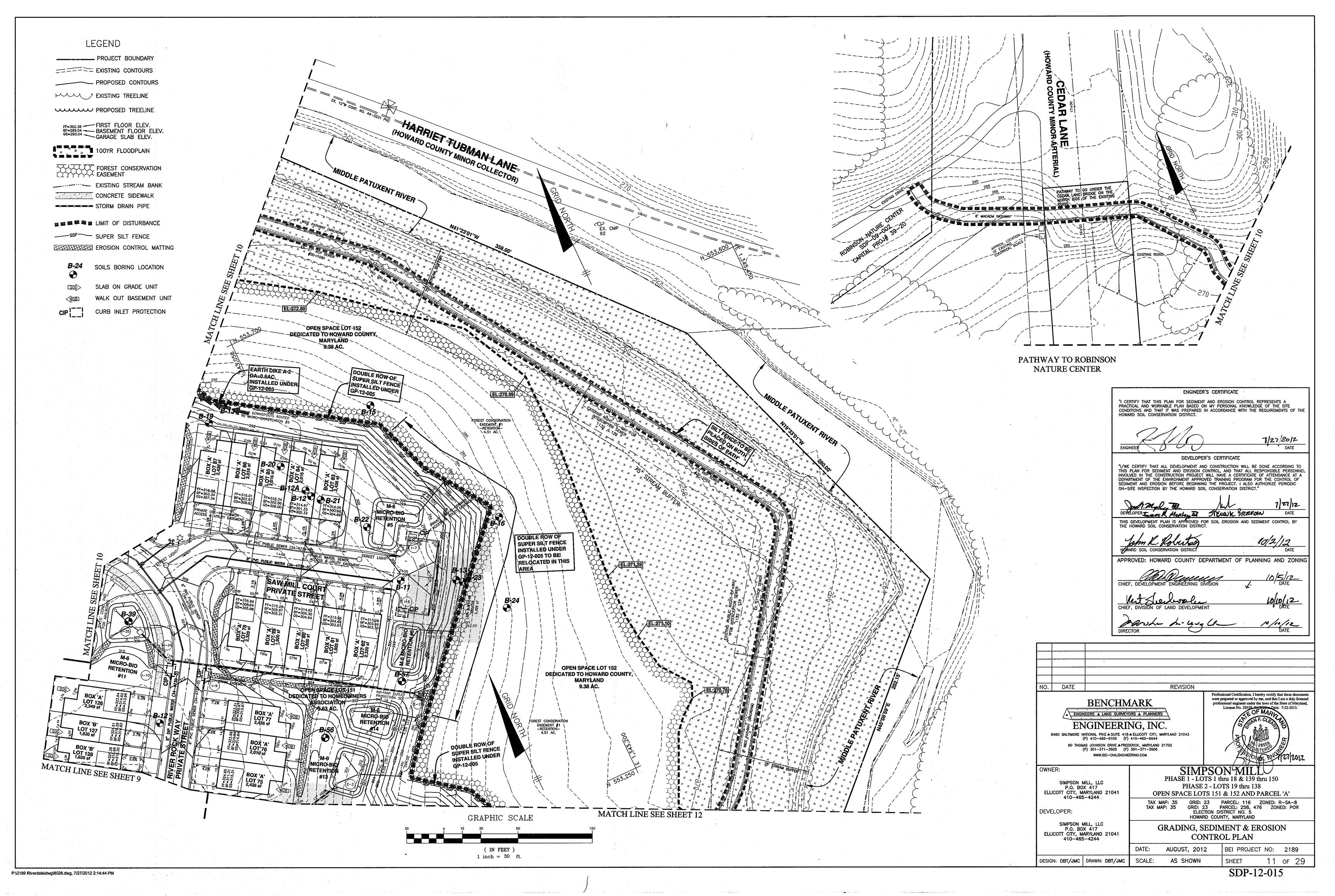


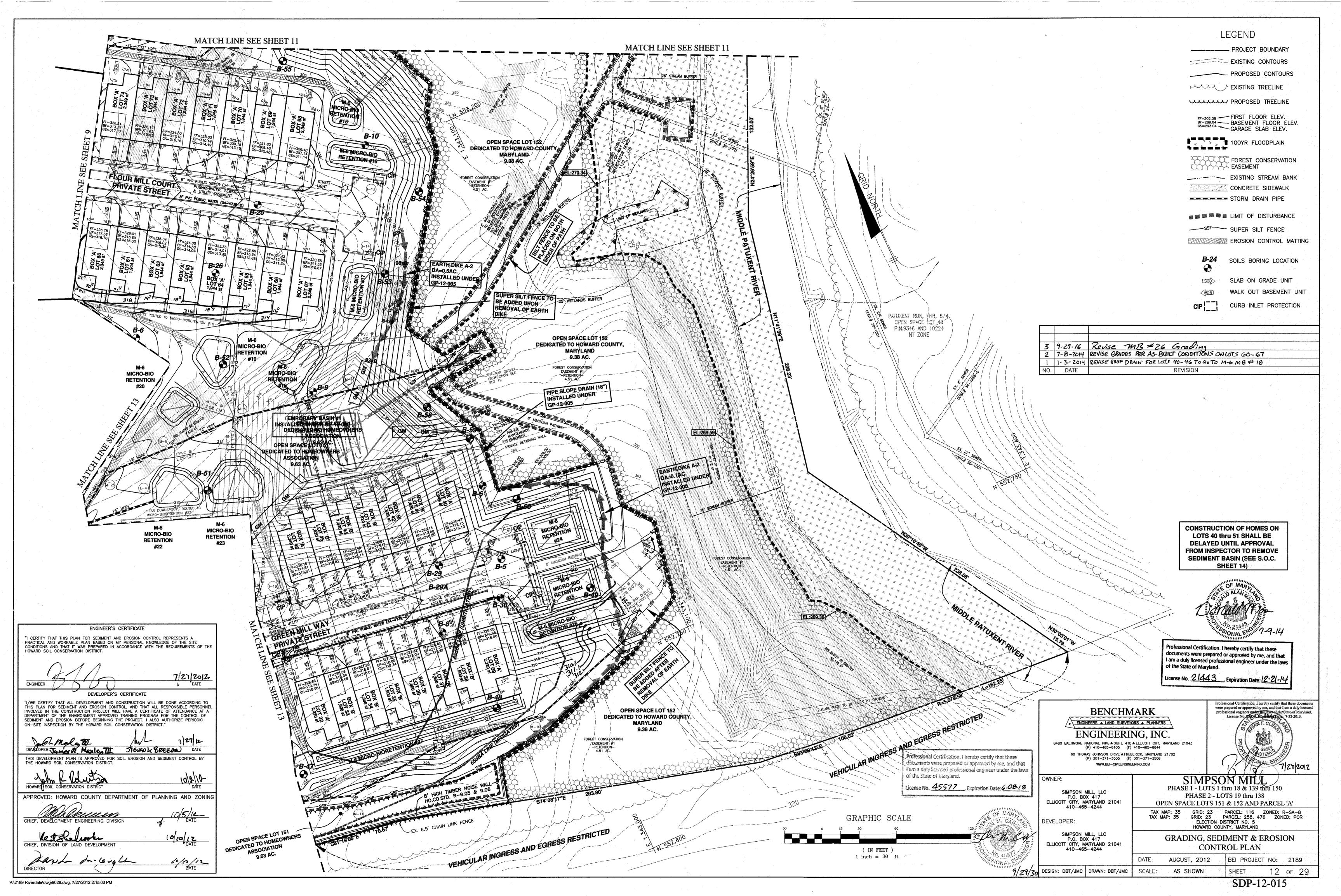


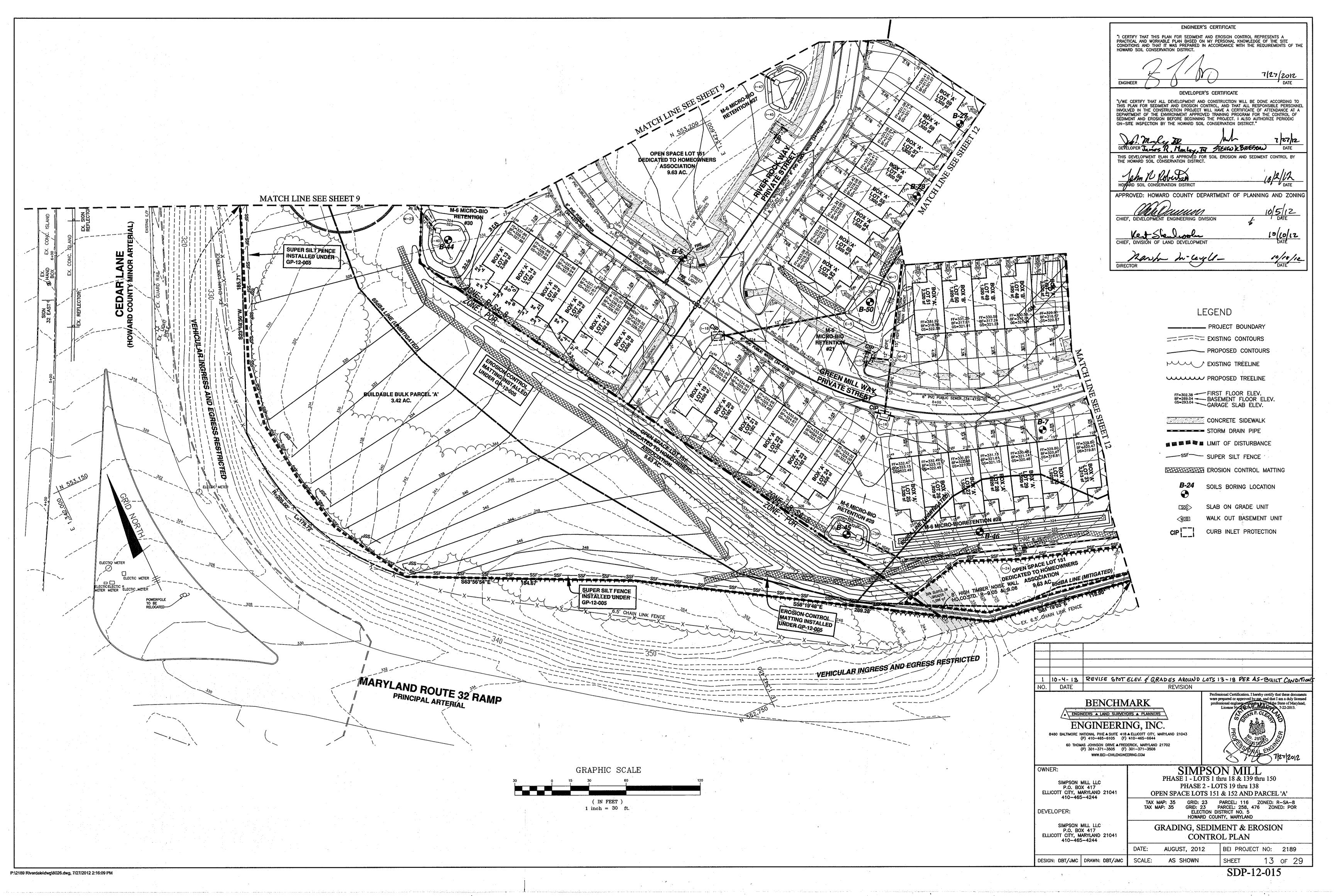


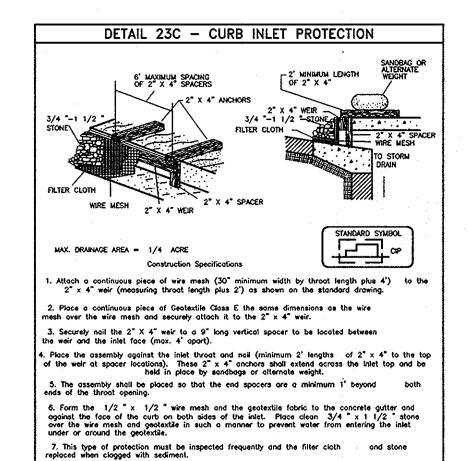






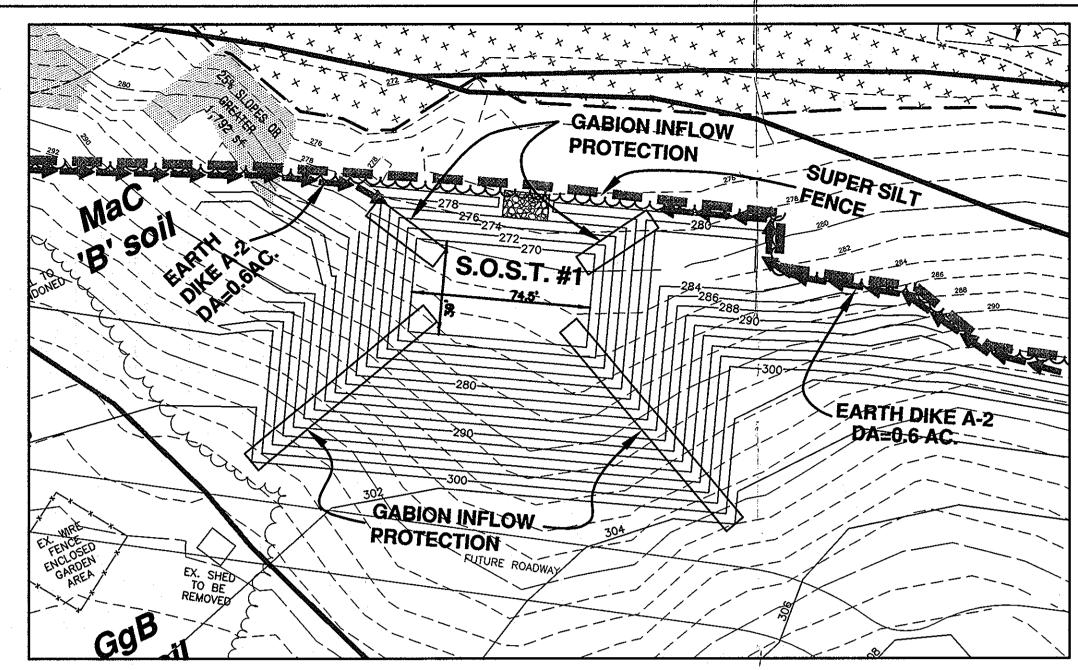




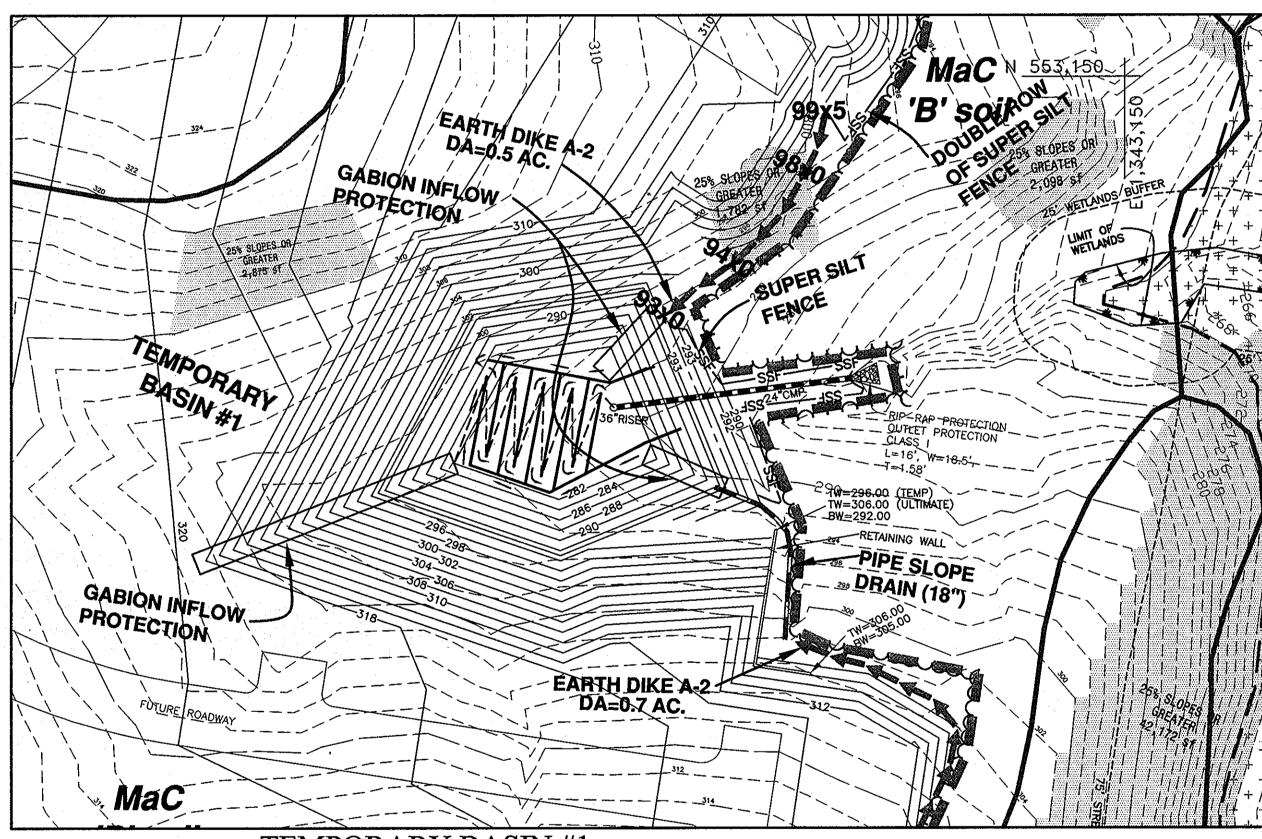


8. Assure that storm flow does not bypass the inlet by installing a temporary earth or asphalt dike to direct the flow to the inlet.

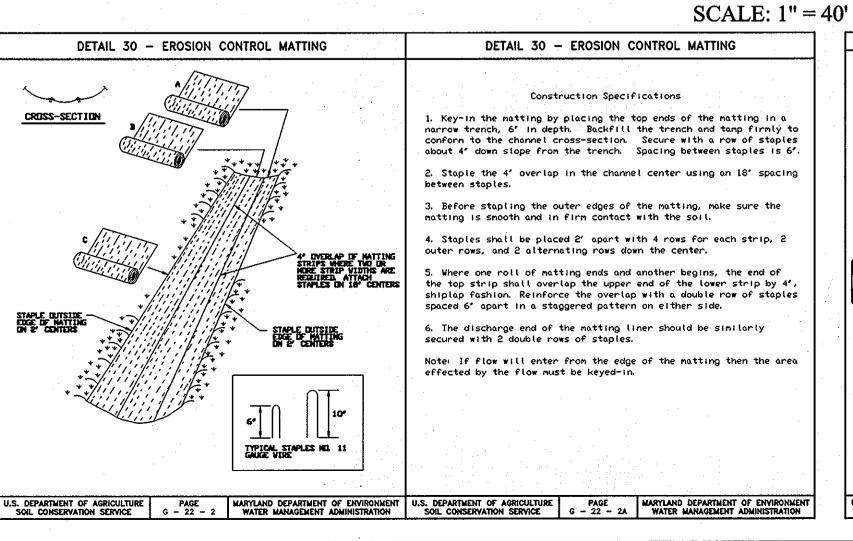
U.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMEN
SOIL CONSERVATION SERVICE E - 16 - 5B WATER MANAGEMENT ADMINISTRATION

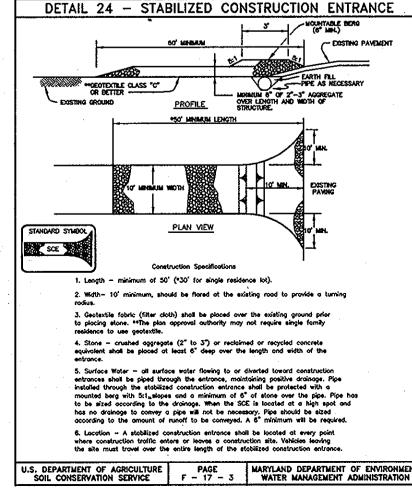


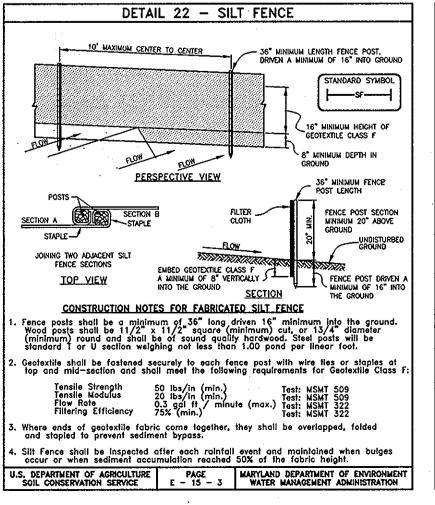
SEDIMENT TRAP #1 **INSTALLED UNDER GP-12-005** SCALE: 1'' = 40'



TEMPORARY BASIN #1 INSTALLED UNDER GP-12-005







SUPER SILT FENCE DETAIL 33 - SUPER SILT FENCE CONSTRUCTION SPECIFICATIONS Fencing shall be 42" in height and constructed in accordance with the latest Maryland State Highway Details for Chain Link Fencing. The specification for a 6' fence shall be used, substituting 42" fabric and 6' length posts. Chain link fence shall be fastened securely to the fence posts with wire ties. The lower tension wire, brace and truss rods, drive anchors and post caps are not required except on the ends of the fence. Filter cloth shall be fastened securely to the chain link fence with ties spaced every 24" at the top and mid section. GROUND SURFACE Filter cloth shall be embedded a minimum of 8" into the ground. When two sections of filter cloth adjoin each other, they shall be overlapped by $6^{\star\prime}$ and folded FLOW STANDARD SYMBOL PERSPECTIVE VIEW CHAIN LINK FENCE ----SUPER SILT FENCE DESIGN CRITERIA 0 - 10:1 Unlimited FLOW 1,000 feet 3:1 - 2:1 100 feet 500 feet

TOPSOIL SPECIFICATIONS

- Topsoil salvaged from the existing site may be used provided that it meets that standards as set forth in these specifications. Typically, the depth of topsoil to be solvaged for a given soil type can be found in the representative soil profile section in the Soil Survey published by USDA—SCS in cooperation with Maryland Agricultural Experimental Station.
- Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay loam, loamy sand. Other soils may be used if recommended by an agronomist or soil scientist and approved by the appropriate approval authority. Regardless, topsoil shall not be a mixture of contrasting texture subsoils and shall contain less than 5% by volume of cinders, stones, slag, coarse fragments, gravel, sticks, roots, trash, or other materials larger than 1-1/2" in diameter.
- Topsoil must be free of plants or plant parts such as Bermuda grass, quack grass, Johnson grass, nutsedge, poison ivy, thistle, or others as specified.
- iii. Where the subsoil is either highly acidic or composed of heavy clays, ground limestone shall be spread at the rate of 4-8 tons/acre (200-400 pounds per 1,000 square feet) prior to the placement of topsoil. Lime shall be distributed uniformly over designated areas and worked into the soil in conjunction with tillage operations as described in the following procedures.
- II. For sites having disturbed areas under 5 acres: Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization — Section I — Vegetative Stabilization Methods and
- V. For sites having disturbed areas over 5 acres:
- On soil meeting Topsoil specifications, obtain test results dictating fertilizer and lime amendments required to bring the soil into compliance with the following:
- a. pH for topsoil shall be between 6.0 and 7.5. If the tested soil demonstrates a pH of less than 6.0, sufficient lime shall be prescribed to raise the pH to 6.5 or higher.
- b. Organic content or topsoil shall be not less than 1.5 percent by weight.
- Topsoil having soluble salt content greater than 500 parts per million shall not be used.

No sod or seed shall be placed on soil which has been treated with soil

- sterilants or chemicals used for weed control until sufficient time has elapsed (14 days min.) to permit dissipation of phyto-toxic materials. Note: Topsoil substitutes or amendments, as recommended by a qualified agronomist soil scientist and approved by the appropriate approval authority, may be used in lieu of
- Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization Section I Vegetative Stabilization Methods and Materials.
- Topsoil Application

SEDIMENT CONTROL NOTES

A MINIMUM OF 24 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT

ALL VEGETATIVE AND STRUCTURAL PRACTICES ARE TO BE INSTALLED ACCORDING TO THE PROVISIONS OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE MOST CURRENT "MARYLAND STANDARDS AND SPECIFICATION FOR SOIL EROSION AND SEDIMENT CONTROL", REVISIONS THERETO.

FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: A) 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, PERIMETER SLOPES AND ALL SLOPES GREATER THAN 3:1, B) 14 DAYS AS TO ALL OTHER DISTURBED OR GRADED

ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 12, OF THE HOWARD

ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDINGS (SEC. 51) SOD

(SEC. 54), TEMPORARY SEEDING (SEC. 50) AND MULCHING (SEC. 52). TEMPORARY STABILIZATION WITH MULCH ALONE CAN ONLY BE DONE WHEN RECOMMENDED SEEDING

DATES DO NOT ALLOW FOR PROPER GERMINATION AND ESTABLISHMENT OF GRASSES.

29.32 ACRES

20.59 ACRES

7.62 ACRES

12.97 ACRES

149,939 cy

54,952 cy

A SITE WITH AN

ACTIVE GRADING

PERMIT

ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE

MAINTAINED IN OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.

ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR

PLACEMENT OF UTILITIES MUST BE REPAIRED ON THE SAME DAY OF DISTURBANCE.

ADDITIONAL SEDIMENT CONTROL MUST BE PROVIDED, IF DEEMED NECESSARY BY THE

10. ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING. OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL BY THE

TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGTHS OR THAT WHICH CAN BE BACK FILLED AND STABILIZED WITHIN ONE WORKING DAY,

TEMPORARY SEEDBED PREPARATIONS

APPLY TO GRADED OR CLEARED AREAS LIKELY TO BE REDISTURBED WHERE A SHORT-TERM

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR

SOIL AMENDMENTS: APPLY 600 LBS PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ FT).

15, SEED WITH 2-1/2 BUSHELS PER ACRE OF ANNUAL RYE (3.2 LBS/1000 SQ FT). FOR THE PERIOD MAY 1 THROUGH AUGUST 14, SEED WITH 3 LBS PER ACRE OF WEEPING LOVEGRASS

(.07 LBS/1000 SQ FT). FOR THE PERIOD NOVEMBER 16 THROUGH FEBRUARY 28, PROTECT SITE BY APPLYING 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ FT) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER

APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ FT) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES, 8 FT. OR HIGHER, USE 348

REFER TO THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR RATE AND METHODS NOT COVERED.

PERMANENT SEEDBED PREPARATIONS

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR

SOIL AMENDMENTS: IN LIEU OF SOIL TEST RECOMMENDATIONS, USE ON OF THE FOLLOWING

SEEDING: FOR THE PERIODS MARCH 1 THROUGH APRIL 30 AND AUGUST 1 THROUGH OCTOBER 15, SEED WITH 60 LBS PER ACRE (1.4 LBS/1000 SQ FT) OF KENTUCKY 31 TALL FESCUE PER ACRE AND 2 LBS PER ACRE (.05 LBS/1000 SQ FT) OF WEEPING LOVEGRASS, DURING THE PERIOD OF OCTOBER 16 THROUGH FEBRUARY 28, PROTECT SITE BY: OPTION (1) 2 TONS

OPTION (2) USE SOD. OPTION (3) SEED WITH 60 LBS PER ACRE OF KENTUCKY 31 TALL FESCUE AND MULCH WITH 2 TONS PER ACRE OF WELL ANCHORED STRAW.

PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ FT) OF UNROTTED

MAINTENANCE: INSPECT ALL SEEDED AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND

SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ FT) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348

ACCEPTABLE - APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS/1000 SQ FT) AND 1000 LBS PER ACRE 10-10-10 FERTILIZER (23 LBS/1000 SQ FT)

SEEDING: FOR PERIOD MARCH 1 THROUGH APRIL 30 AND FROM AUGUST 15 THROUGH NOVEMBE

OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED.

OF INSPECTION, LICENSES AND PERMITS, SEDIMENT CONTROL DIVISION PRIOR TO THE

START OF ANY CONSTRUCTION, (313-1850).

AREAS ON THE PROJECT SITE.

TOTAL AREA OF SITE

AREA TO BE ROOFED OR PAVED

OFFSITE WASTE AREA LOCATION

HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.

AREA TO BE VEGETATIVELY STABILIZED

AREA DISTURBED

TOTAL FILL

INSPECTION AGENCY IS MADE.

AS POSSIBLE IN THE SPRING, OR USE SOD.

GALLONS PER ACRE (8 GAL/1000 SQ FT) FOR ANCHORING

GALLONS PER ACRE (8 GAL/1000 SQ FT) FOR ANCHORING.

WHICHEVER IS SHORTER.

VEGETATIVE COVER IS NEEDED.

SCHEDULES:

7. SITE ANALYSIS:

- When topsoiling, maintain needed erosion and sediment control practices such as diversions, grade stabilization structures, earth dikes, slope silt fence and sediment traps and basins.
- ii. Grades on the great to be topsoiled, which have been previously established, shall be maintained, albeit 4" 8" higher in elevation.
- iii. Topsoil shall be uniformly distributed in a 4" 8" layer and lightly compacted to a minimum thickness of 4". Spreading shall be performed in such a manner that sodding or seeding can proceed with a minimum of additional soil preparation and tillage. Any irregularities in the surface resulting from topsoiling or other operations shall be corrected in order to prevent the formation of depressions or water pockets.
- Topsoil shall not be placed while the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet or in a condition that may otherwise be detrimental to proper grading and seedbed preparation.
- Alternative for Permanent Seeding instead of applying the full amounts of lime and commercial fertilizer, composted sludge and amendments may be applied as specified
- I. Composted Sludge Material for use as a soil conditioner for sites having distributed areas over 5 acres shall be tested to prescribe amendments and for sites having disturbed areas under 5 acres shall conform to the following requirements:
- a. Composted sludge shall be supplied by, or originate from, a person or persons that are permitted (at the time of acquisition of the compost) by the Maryland Department of the Environment under COMAR 26.04.06.
- Composted sludge shall contain at least 1 percent nitrogen, 1.5 percent phosphorus, and 0.2 percent potassium and have a pH of 7.0 to 8.0. I compost does not meet these requirements, the appropriate constituents must be added to meet the requirements prior to use.
- c. Composted sludge shall be applied at a rate of 1 ton/1,000 square feet. iv. Composted sludge shall be amended with a potassium fertilizer applied at the rate
- References: Guidelines Specifications, Soil Preparation and Sodding. MD-VA, Pub. #1, Cooperative Extension Service, University of Maryland and Virginia Polytechnic Institutes, Revised 1973.

30.0 DUST CONTROL

Controlling dust blowing and movement on construction sites and roads. <u>Purpose</u>

damage, health hazards, and improve traffic safety. Conditions Where Practice Applies

This practice is applicable to areas subject to dust blowing and movement where on and off-site

<u>Temporary Methods</u>
1. Mulches — See standards for vegetative stabilization with mulches only. Mulch should be crimped or tracked to prevent blowing.

- 2. Vegetative Cover See standards for temporary vegetative cover.
- 3. Tillage To roughen surface and bring clods to the surface. This is an emergency measure which should be used before soil blowing starts. Begin plowing on windward side of site. Chisel—type plows spaced about 12" apart, spring—toothed harrows, and similiar plows are examples of equipment which may produce the desired effect.
- Irrigation This is generally done as an emergency treatment. Site is sprinkled with water until the surface is moist. Repeat as needed. At no time should the site be irrigated to the point that runoff begins to flow.
- Barriers Solid board fences, silt fences, snow fences, burlap fences, straw bales, and similar material can be used to control air currents and soil blowing. Barriers placed right angles to prevailing currents at intervals of about 10 times their height are effection controlling soil blowing.
- 6. Calcium Chloride Apply at rates that will keep surface moist. May need retreatment
- Permanent Methods

 1. Permanent Vegetation See standards for permanent vegetative cover, and permanent stabilization with sod. Existing trees or large shrubs may afford valuable protection if left in place.

DATE

DESIGN: DBT/JMC | DRAWN: DBT/JMC

- 2. Topsciling Covering with less erosive soil materials. See standards for topsciling. 3. Stone - Cover surface with crushed stone or coarse gravel.
- . Agriculture Handbook 346. Wind Erosion Forces in the United States and Their Use in Predicting Soil Loss. 2. Agriculture Information Bulletin 354. How to Control Wind Erosion, USDA-ARS.

SEQUENCE OF CONSTRUCTION

NOTIFY SEDIMENT CONTROL DIVISION 48 HOURS PRIOR TO START OF WORK

1. Obtain grading permit. (day 1)

PHASE 1

2. Install stabilized construction entrance. Utilize sediment control features installed under GP-12-005 Remove and replace areas of SSF where indicated on the plans (i.e. along Cedar lane ROW and the end of Saw Mill Court) (day 2-7)

3. Upon approval of the Howard County Sediment Control Inspector, bring road beds to subgrade and commence mass grading. Stabilize slopes in accordance with the TEMPORARY seedbed notes. Utilize

dust control methods. (day 8-60) 4. Install storm drains and water and sewer mains. Do NOT install storm drain from E-3 to M-7 at this time. Install curb inlet protection on all inlets within the roadway and for Inlet 23. Wrap Inlet 24 and 43

5. Install curb & gutter and base paving. (day 101-120)

6. Stabilize all areas in accordance with the temporary seedbed notes. (day 121-125)

7. Construct Micro-Bioretention facilities 1-2, 5-7, 9, 11-17, 24-26, and 31-37 complete with outfall pipes. (day 126-155)

8. Once all areas have been stabilized with the PERMANENT seedbed notes and upon approval from the Howard County Sediment Control Inspector, remove Sediment Trap #1 and the related earth dikes and replace with super silt fence. Install Micro-Bioretention facilities 3 and 4. (day 156-165)

9. Once all areas have been stabilized with the PERMANENT seedbed notes and upon approval from the Howard County Sediment Control Inspector, remove Temporary Basin #1 and the related earth dikes and pipe slope drain and replace with super silt fence. Install Micro-Bioretention facilities 18-23 and their outfall (i.e. storm drain from E-3 to M-7). Install remaining portion of the retaining wall not constructed under GP-12-005. (day 166-185).

10. Upon Approval of the Howard County Sediment Control Inspector, remove all remaining sediment control devices and stabilize any remaining areas in accordance with the PERMANENT seedbed notes.

PHASE 2 - (HOUSE CONSTRUCTION*)

1. Install super silt fences around lot stick. (day 1)

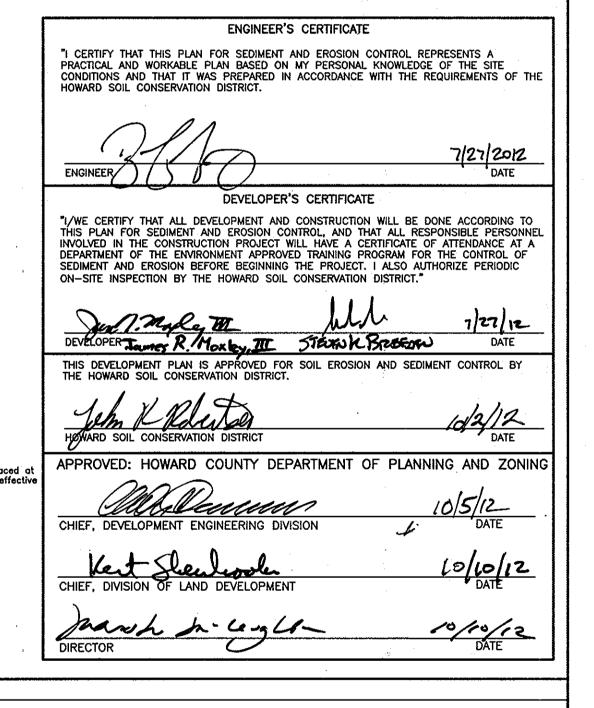
2. Excavate for foundation, rough grade and stabilize in accordance with the temporary seedbed notes. (day 2-5)

3. Construct houses, backfill and construct driveways. (day 6-90)

4. Final grade lot stick and stabilize in accordance with the PERMANENT seedbed notes. (day 91-95)

5. Upon approval from the Howard County Sediment Control Inspector, remove sediment control devices and stabilize any remaining disturbed areas. (day 96-100)

*House construction shall be performed for an entire stick at one time



BENCHMARK professional engineer under the laws of the State of Marylan ENGINEERS & LAND SURVEYORS & PLANNERS ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE & SUITE 418 & ELLICOTT CITY, MARYLAND 21043 60 THOMAS JOHNSON DRIVE ▲ FREDERICK, MARYLAND 21702 WWW.BEI-CIMLENGINEERING.COM SIMPSON MILL OWNER: PHASE 1 - LOTS 1 thru 18 & 139 thru 150 SIMPSON MILL LLC PHASE 2 - LOTS 19 thru 138 P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041 OPEN SPACE LOTS 151 & 152 AND PARCEL 'A' GRID: 23 PARCEL: 116 ZONED: R-SA-8 GRID: 23 PARCEL: 258, 476 ZONED: POR DEVELOPER: FLECTION DISTRICT NO. ! HOWARD COUNTY, MARYLAND SIMPSON MILL LLC SEDIMENT & EROSION CONTROL P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041 NOTES AND DETAILS

SCALE:

AUGUST, 2012

REVISION

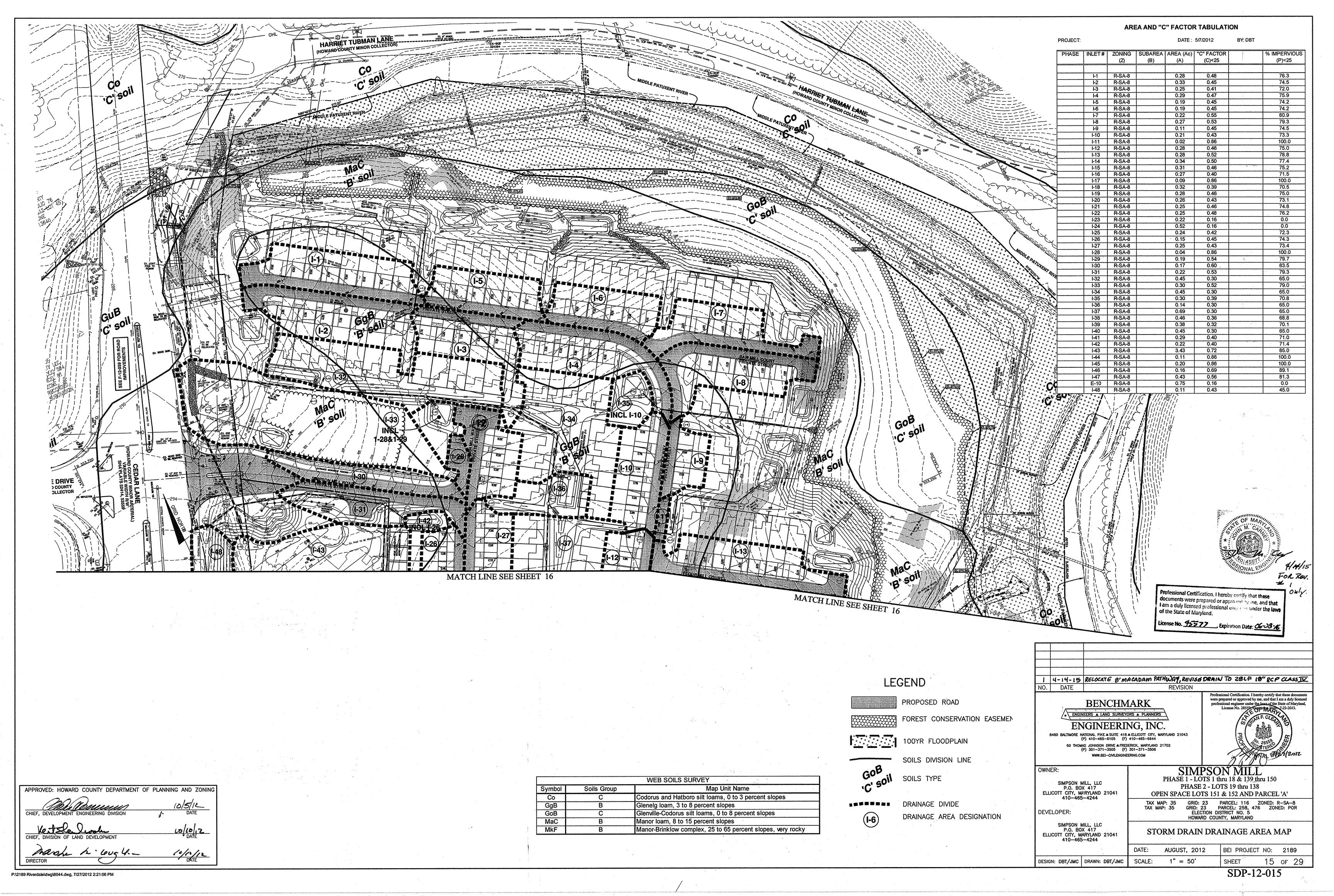
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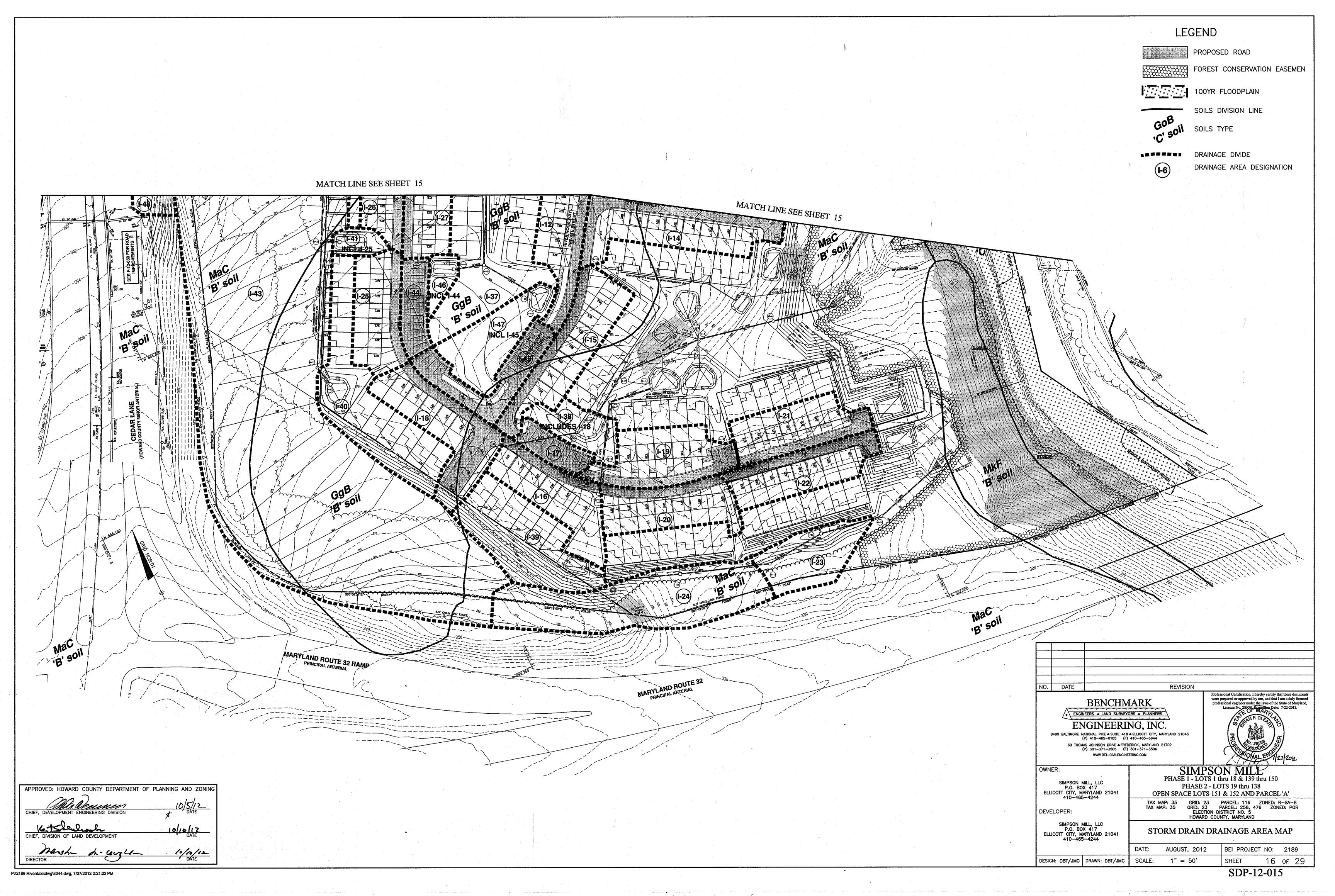
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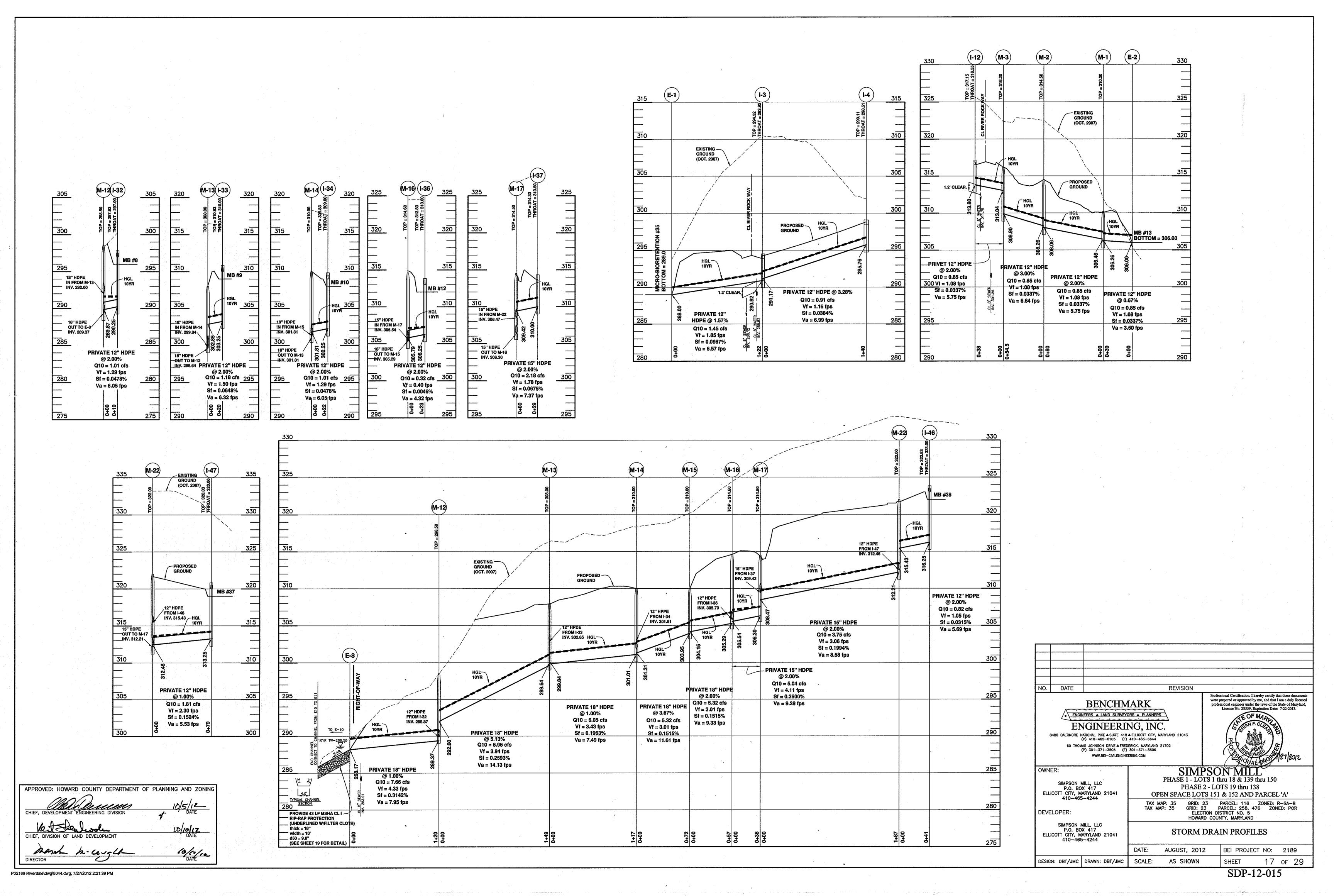
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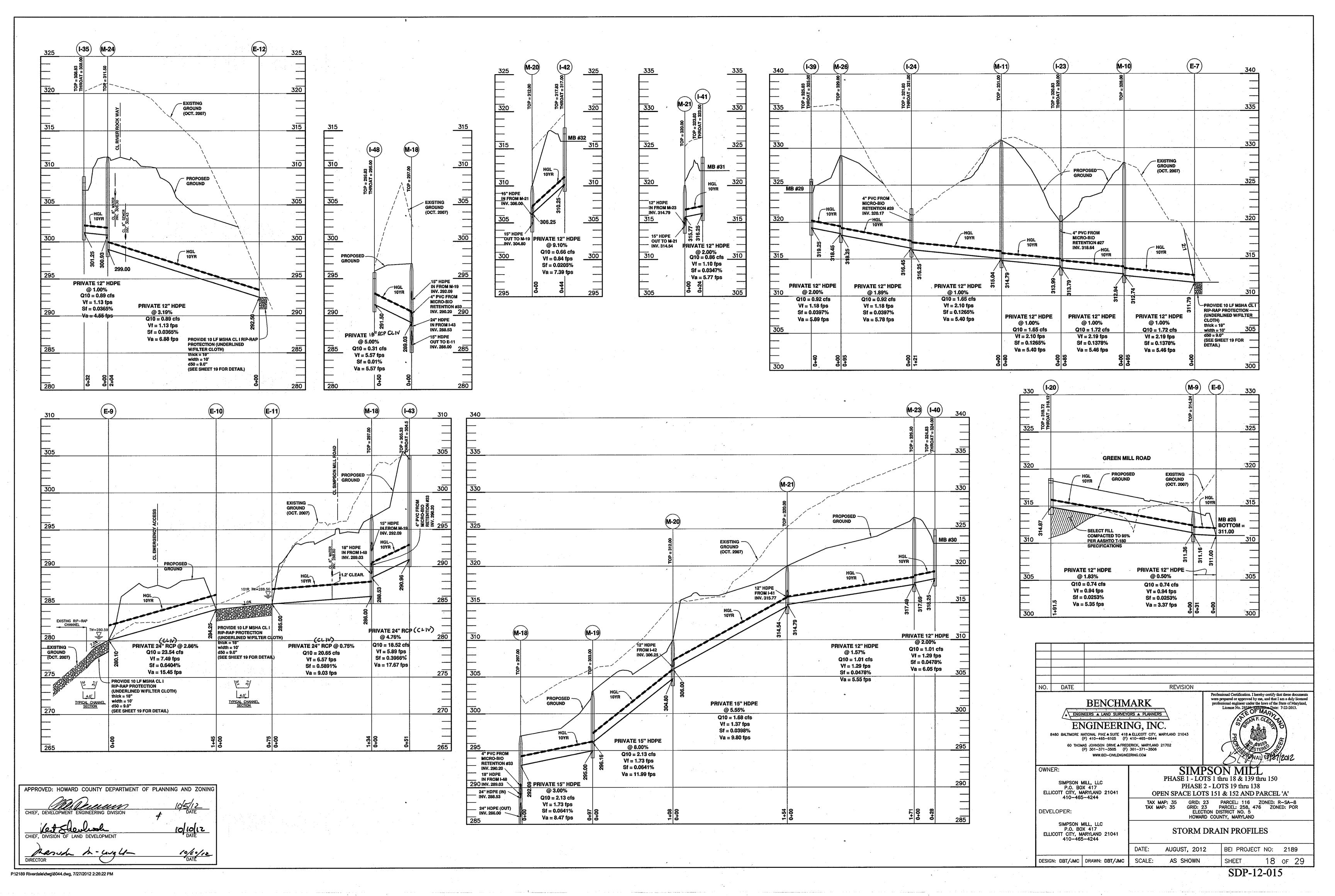
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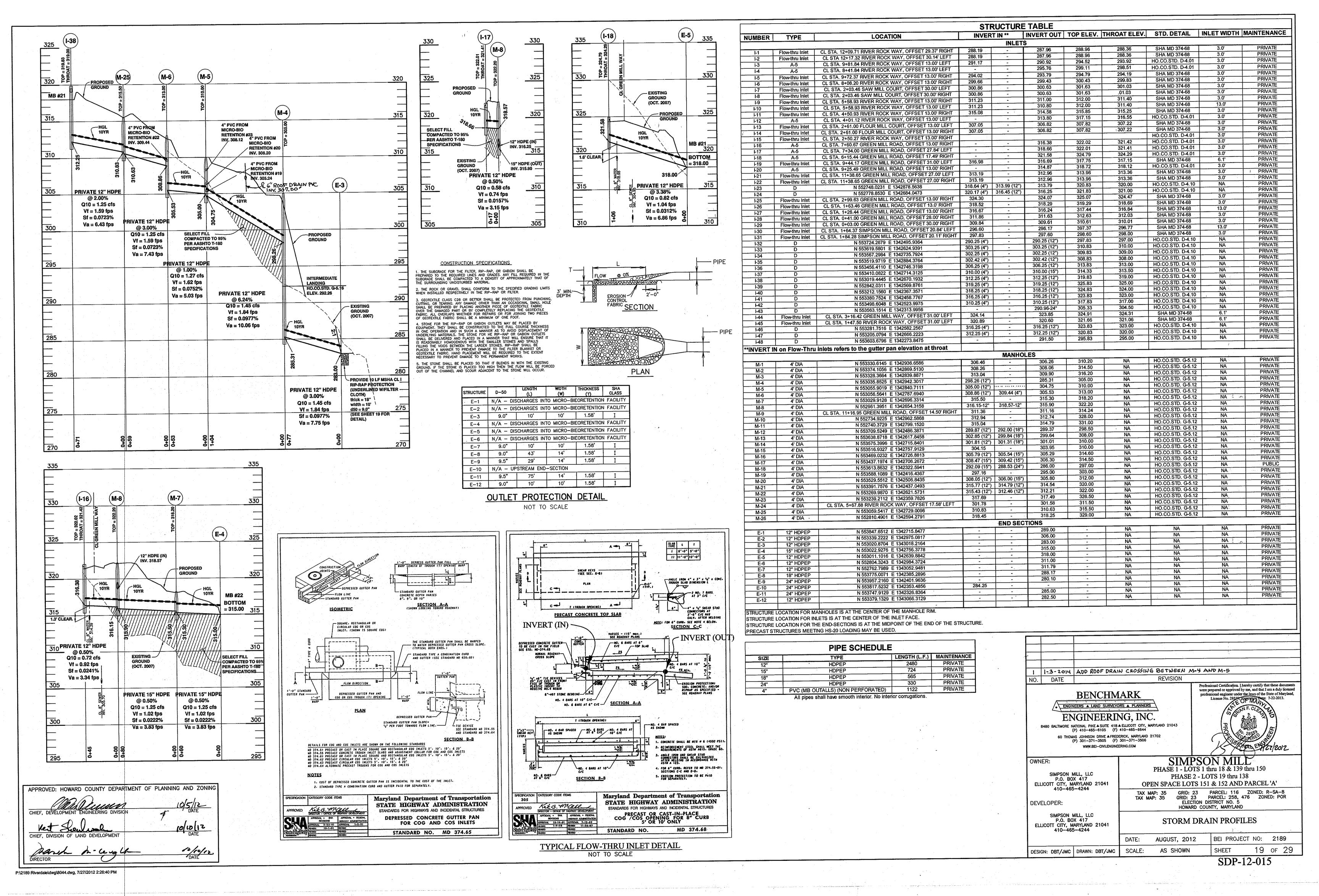
were prepared or approved by me, and that I am a duly license

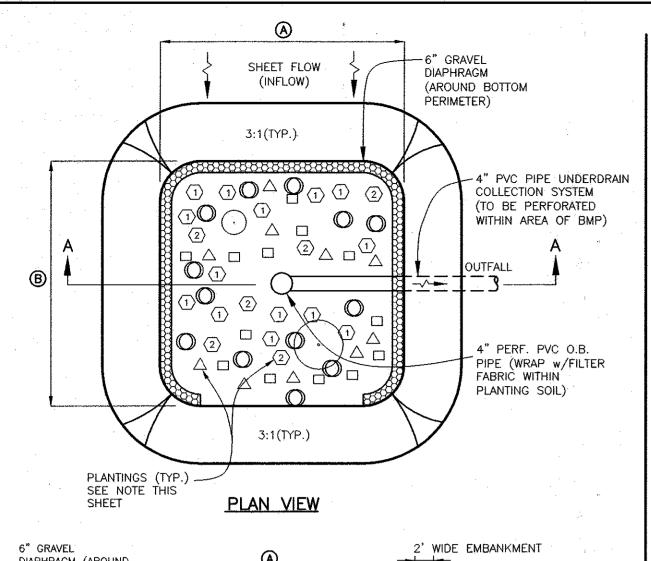


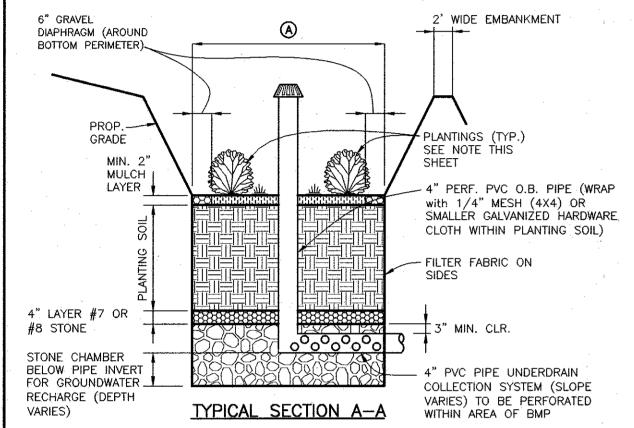


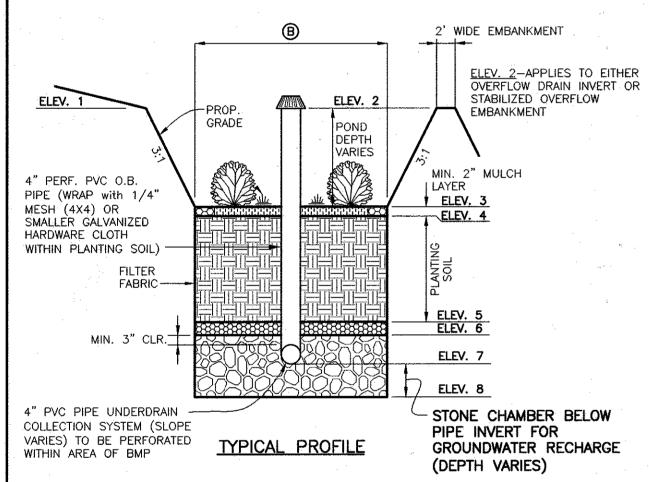








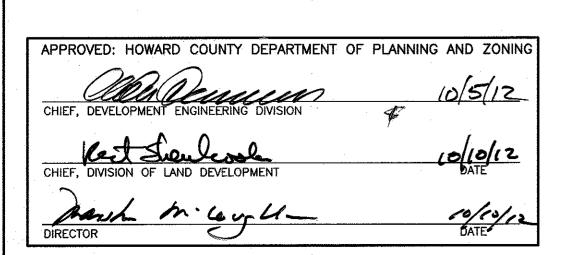


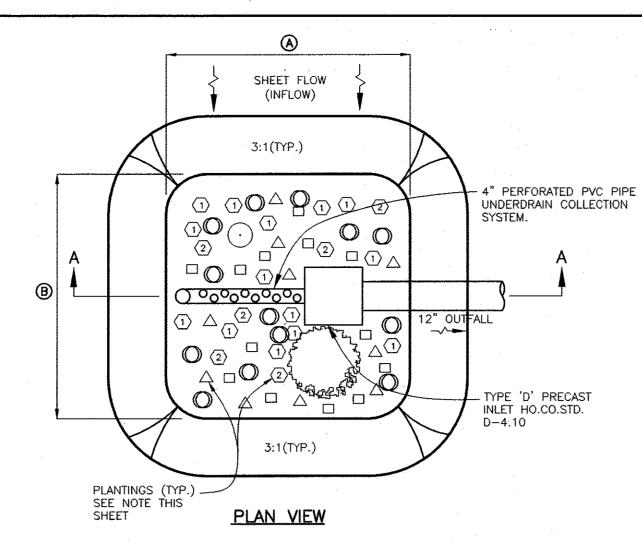


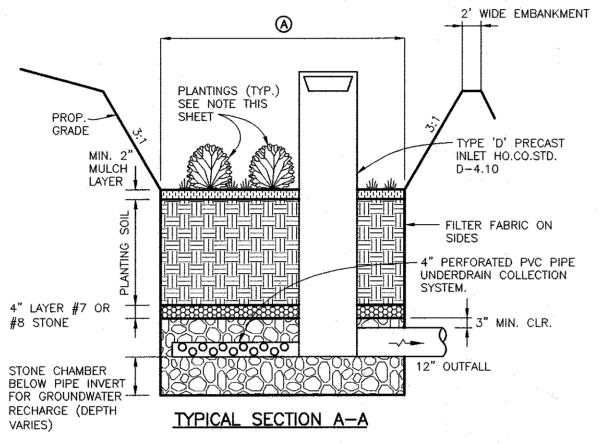
TYPICAL MICRO-BIORETENTION DETAILS

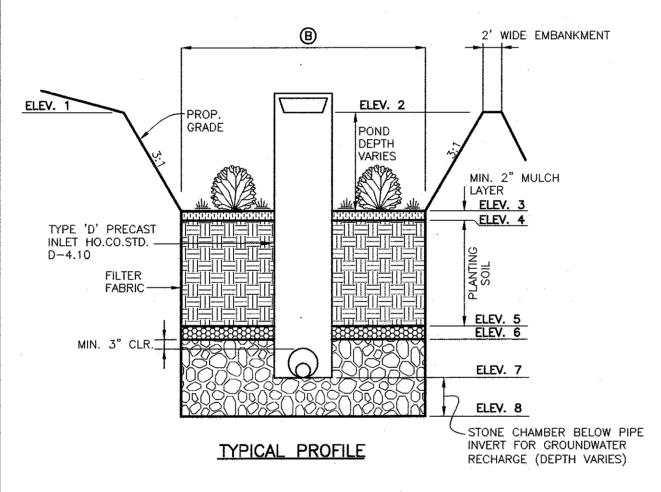
UNDERDRAIN, OVERFLOW AND OUTFALL NOTES

- 1. THE LAST CLEAN-OUT LOCATION WITHIN EACH MICRO-BIORETENTION FACILITY SHALL BE FITTED WITH A NON-CLOGGING SURFACE DRAIN (EXAMPLE: 4" ABS ROOF DRAIN W/CAST ALUMINUM DOME) AT THE POND SURFACE ELEVATION INDICATED IN THE CORRESPONDING TABLE ELEV. 2.
- 2. THE PVC WITHIN THE FACILITY SHALL BE PERFORATED.
- 3. THE UNDER-DRAIN AND PIPE TO OUTFALL SHALL BE INSTALLED TO A MINIMUM DEPTH OF 2' BELOW FINISHED GRADE AND SHALL MAINTAIN A MINIMUM 1% SLOPE AND MAINTAIN A MINIMUM OF 1' OF SEPARATION AT ALL CROSSINGS.









TYPICAL MICRO-WITH TYPE 'D' INLET NOT TO SCALE

MICRO-B	IORETENTION PLANTING LEGEND
SYMBOL	NAME
1	AJUGA REPTANS (CREEPING BUGLEWEED)
2	IRIS VERSICOLOR (IRIS)
	CLETHRA (COMMON PERIWINKLE)
Δ	ELYMUS VIRGINICUS (VIRGINIA WILD RYE)
0	VACCINIUM ATROCOCCUM (HIGHBUSH BLUEBERRY)
	BETULA NIGRA (RIVER BIRCH)

MICRO-BIORETENTION DESIGN TABLES (* - INDICATES FACILITIES WITH 'D' TYPE INLET)

								-							<u> </u>	- <u>-</u>			
#1		#2		#3		#4	-	#5		#6		#7		#8	*	#9	*	#10	*
ELEV. 1	288.00	ELEV. 1 2	287.00	ELEV. 1	283.00	ELEV. 1	283.00	ELEV. 1	300.00	ELEV. 1	300.00	ELEV. 1	288.00	ELEV. 1	297.00	ELEV. 1	310.00	ELEV. 1	309.00
ELEV. 2	288.00	ELEV. 2 2	287.00	ELEV. 2	283.00	ELEV. 2	283.00	ELEV. 2	300.00	ELEV. 2	300.00	ELEV. 2	288.00	ELEV. 2	297.00	ELEV. 2	310.00	ELEV. 2	309.00
ELEV. 3	287.00	ELEV. 3 2	286.00	ELEV. 3	282.00	ELEV. 3	282.00	ELEV. 3	299.00	ELEV. 3	299.00	ELEV. 3	287.00	ELEV. 3	296.00	ELEV. 3	309.00	ELEV. 3	308.00
ELEV. 4	286.83	ELEV. 4 2	285.83	ELEV. 4	281.83	ELEV. 4	281.83	ELEV. 4	298.83	ELEV. 4	298.83	ELEV. 4	286.83	ELEV. 4	295.83	ELEV. 4	308.83	ELEV. 4	307.83
ELEV. 5	282.83	ELEV. 5 2	281.83	ELEV. 5	277.83	ELEV. 5	277.83	ELEV. 5	294.83	ELEV. 5	294.83	ELEV. 5	282.83	ELEV. 5	291.83	ELEV. 5	304.83	ELEV. 5	303.83
ELEV. 6	282.50	ELEV. 6 2	281.50	ELEV. 6	277.50	ELEV. 6	277.50	ELEV. 6	294.50	ELEV. 6	294.50	ELEV. 6	282.50	ELEV. 6	291.50	ELEV. 6	304.50	ELEV. 6	303.50
ELEV. 7	281.92	ELEV. 7 2	280.92	ELEV. 7	276.92	ELEV. 7	276.92	ELEV. 7	293.92	ELEV. 7	293.92	ELEV. 7	281.92	ELEV. 7	290.25	ELEV. 7	303.25	ELEV. 7	302.25
ELEV. 8	279.92	ELEV. 8 2	278.92	ELEV. 8	274.92	ELEV. 8	274.29	ELEV. 8	291.92	ELEV. 8	291.92	ELEV. 8	279.92	ELEV. 8	288.25	ELEV. 8	303.00	ELEV. 8	302.00
DIMENS	IONS	DIMENSIO	NS	DIMENS	IONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS
'A'	varies	'A' \	varies	'A'	40'±	'A'	50'±	'A'	varies	'A'	35'±	'A'	28'±	,Y,	67'±	'A'	23'±	'A'	varies
'B'	varies	'B'	varies	'B'	7.5'±	'B'	14'±	'B'	varies	'B'	20'±	'B'	23'±	,B,	11'±	,B,	23'±	'B'	varies
TOTAL SF	800	TOTAL SF	670	TOTAL SF	292	TOTAL SF	700	TOTAL SF	836	TOTAL SF	692	TOTAL SF	711	TOTAL SF	734	TOTAL SF	552	TOTAL SF	850
OUTFALL	PIPE	OUTFALL F	PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALL	PIPE
SIZE	4"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	12"	SIZE	12"	SIZE	12"
LENGTH	78'	LENGTH	26'	LENGTH	31'	LENGTH	36'	LENGTH	34	LENGTH	27'	LENGTH	124	LENGTH	NA	LENGTH	NA	LENGTH	NA
SLOPE	2.5%	SLOPE	3.5%	SLOPE	3.0%	SLOPE	2.6%	SLOPE	17.5%	SLOPE	7.1%	SLOPE	1.5%	SLOPE	NA	SLOPE	NA	SLOPE	NA
			······································																

SLOPE	2.5%	SLOPE	3.5%	SLUPE	3.0%	SLUPE	2.0%	SLUPE	17.5%	SLUPE	7.176	SLUFE	1.3/6	SLUFE	INA	SLOFE	i jiw	SLUFE	INA
#1	1 *	#12	2 *	#1:	3	#14	1	#1:	5	#16	}	#17	7	#18	В	#19	9	#20)
ELEV. 1	308.00	ELEV. 1	313.00	ELEV. 1	307.00	ELEV. 1	301.00	ELEV. 1	305.00	ELEV. 1	306.00	ELEV. 1	306.00	ELEV. 1	308.00	ELEV. 1	312.00	ELEV. 1	312.00
ELEV. 2	308.00	ELEV. 2	313.00	ELEV. 2	307.00	ELEV. 2	301.00	ELEV. 2	305.00	ELEV. 2	306.00	ELEV. 2	306.00	ELEV. 2	308.00	ELEV. 2	312.00	ELEV. 2	312.00
ELEV. 3	307.00	ELEV. 3	312.00	ELEV. 3	306.00	ELEV. 3	300.00	ELEV. 3	304.00	ELEV. 3	305.00	ELEV. 3	305.00	ELEV. 3	307.00	ELEV. 3	311.00	ELEV. 3	311.00
ELEV. 4	306.83	ELEV. 4	311.83	ELEV. 4	305.83	ELEV. 4	299.83	ELEV. 4	303.83	ELEV. 4	304.83	ELEV. 4	304.83	ELEV. 4	306.83	ELEV. 4	310.83	ELEV. 4	310.83
ELEV. 5	302.83	ELEV. 5	307.83	ELEV. 5	301.83	ELEV. 5	295.83	ELEV. 5	299.83	ELEV. 5	300.83	ELEV. 5	300.83	ELEV. 5	302.83	ELEV. 5	306.83	ELEV. 5	307.83
ELEV. 6	302.50	ELEV. 6	307.50	ELEV. 6	301.50	ELEV. 6	295.50	ELEV. 6	299.50	ELEV. 6	300.50	ELEV. 6	300.50	ELEV. 6	302.50	ELEV. 6	306.50	ELEV. 6	307.50
ELEV. 7	301.25	ELEV. 7	306.25	ELEV. 7	300.92	ELEV. 7	294.92	ELEV. 7	298.92	ELEV. 7	299.92	ELEV. 7	299.92	ELEV. 7	301.92	ELEV. 7	305.92	ELEV. 7	306.92
ELEV. 8	301.00	ELEV. 8	306.00	ELEV. 8	298.92	ELEV. 8	292.92	ELEV. 8	296.92	ELEV. 8	299.92	ELEV. 8	299.92	ELEV. 8	301.67	ELEV. 8	303.92	ELEV. 8	306.67
DIMENS	SIONS	DIMENS	IONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS								
,Y,	varies	'A'	38.5'±	~ 'A'	20'±	'A'	varies	'A'	varies	'A'	49'±	'A'	38'±	'A'	40'±	'A'	varies	'A'	varies
'B'	varies	,B,	13'±	'B'	30'±	'B'	varies	,B,	varies	'B'	15'	'B'	19 ' ±	'B'	15'±	'B'	varies	'B'	varies
TOTAL SF	640	TOTAL SF	670	TOTAL SF	600	TOTAL SF	621	TOTAL SF	370	TOTAL SF	706'	TOTAL SF	720	TOTAL SF	682	TOTAL SF	700	TOTAL SF	395
OUTFALL	PIPE	OUTFALL	_ PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALI	PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALI	_ PIPE	OUTFALL	_ PIPE	OUTFALL	PIPE
SIZE	12"	SIZE	12"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	4"								
LENGTH	NA	LENGTH	NA	LENGTH	35'	LENGTH	22'	LENGTH	54'	LENGTH	41'	LENGTH	38'	LENGTH	77'	LENGTH	34'	LENGTH	36' -
SLOPE	NA	SLOPE	NA	SLOPE	19.8%	SLOPE	4.2%	SLOPE	31.3%	SLOPE	21.7%	SLOPE	15.6%	SLOPE	15.5%	SLOPE	2.0%	SLOPE	2.0%
1																			

																- -			
#21 >	*	#22	2	#23	3	#24	-	#25	5	#26	5	#27	7	#28	3	#29	9 🔭	#30	*
ELEV. 1	319.00	ELEV. 1	316.00	ELEV. 1	315.00	ELEV. 1	313.00	ELEV. 1	312,00	ELEV. 1	311./	ELEV. 1	323.00	ELEV. 1	325.00	ELEV. 1	325.00	ELEV. 1	324.00
ELEV. 2	319.00	ELEV. 2	316.00	ELEV. 2	315.00	ELEV. 2	313.00	ELEV. 2	312.00	ELEV. 2	311.7	ELEV. 2	323.00	ELEV. 2	325.00	ELEV. 2	325.00	ELEV. 2	324.00
ELEV. 3	318.00	ELEV. 3	315.00	ELEV. 3	314.00	ELEV. 3	312.00	ELEV. 3	311.00	ELEV. 3	3101	ELEV. 3	322.00	ELEV. 3	324.00	ELEV. 3	324.00	ELEV. 3	323.00
ELEV. 4	317.83	ELEV. 4	314.83	ELEV. 4	313.83	ELEV. 4	311.83	ELEV. 4	310.83	ELEV. 4	309.93	ELEV. 4	321.83	ELEV. 4	323.83	ELEV. 4	323.83	ELEV. 4	322.83
ELEV. 5	313.83	ELEV. 5	310.83	ELEV. 5	309.83	ELEV. 5	307.83	ELEV. 5	30%.83	ELEV. 5	305,93	ELEV. 5	319.83	ELEV. 5	321.83	ELEV. 5	320.83	ELEV. 5	319.83
ELEV. 6	313.50	ELEV. 6	310.50	ELEV. 6	309.50	ELEV. 6	307.50	ELEV. 6	30%,50	ELEV. 6	305.60	ELEV. 6	319.50	ELEV. 6	321.50	ELEV. 6	320.50	ELEV. 6	319.50
ELEV. 7	312.25	ELEV. 7	309.92	ELEV. 7	308.92	ELEV. 7	306.92	ELEV. 7	3 0 £ ,92	ELEV. 7	305.0	ELEV. 7	318.92	ELEV. 7	320.92	ELEV. 7	319.25	ELEV. 7	318.25
ELEV. 8	312.00	ELEV. 8	309.67	ELEV. 8	306.92	ELEV. 8	304.92	ELEV. 8	30392	ELEV. 8	303. <i>O</i> .	ELEV. 8	318.67	ELEV. 8	320.67	ELEV. 8	319.00	ELEV. 8	318.00
DIMENSIO	ONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS	DIMENS	SIONS
'A'	varies	'A'	varies	'A'	varies	'A'	35'±	'A'	35'±	Ar 'A'	48±	'A'	168'±	'A'	168'±	'A'	26.5'±	'A'	varies
'B'	varies	'B'	varies	~ 'B'	varies	,B,	23'±	,B,	20'±	Ac 'B'	9.5±	'B'	4'±	'B'	4'±	'B'	15.5°±	'B'	varies
TOTAL SF	692	TOTAL SF	700	TOTAL SF	810	TOTAL SF	815	TOTAL SF	700	TOTAL SF	445	TOTAL SF	672	TOTAL SF	672	TOTAL SF	400	TOTAL SF	674
OUTFALL I	PIPE	OUTFALL	. PIPE	OUTFALL	PIPE	OUTFALL	. PIPE	OUTFALL	_ PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALL	PIPE	OUTFALL	_ PIPE	OUTFALL	PIPE
SIZE	12"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	4"	SIZE	12"	SIZE	12"
LENGTH	NA	LENGTH	24'	LENGTH	40'	LENGTH	35'	LENGTH	60'	LENGTH	80'	LENGTH	14'	LENGTH	15'	LENGTH	NA	LENGTH	NA
SLOPE	NA	SLOPE	2.0%	SLOPE	2.0%	SLOPE	15.5%	SLOPE	1 <i>.5</i> %	SLOPE	1.3%	SLOPE	2.0%	SLOPE	2.0%	SLOPE	NA	SLOPE	NA
										+ Total BH + Bottom E	m. area= imensions	755SF Vary				·			

-BIORETENTION DETAILS	•						* Total BHM. area = *Bottom Dimensions
-DIUREIENTIUN DETAILS		#74 > -	1/70 >14	1177	1174	1175	1176 1

													٠,	130 11 0000 -			
#31	1*		#32	*	#3.	3	ſ	#34	-		#35	5	Γ	#36	*	#37	*
ELEV. 1	323.00		ELEV. 1	317.00	ELEV. 1	297.00		ELEV. 1	296.00		ELEV. 1	290.00		ELEV. 1	323.00	ELEV. 1	320.00
ELEV. 2	323.00		ELEV. 2	317.00	ELEV. 2	297.00		ELEV. 2	296.00		ELEV. 2	290.00		ELEV. 2	323.00	ELEV. 2	320.00
ELEV. 3	322.00		ELEV. 3	316.00	ELEV. 3	296.00		ELEV. 3	295.00		ELEV. 3	289.00		ELEV. 3	322.00	ELEV. 3	319.00
ELEV. 4	321.83		ELEV. 4	315.83	ELEV. 4	295.83		ELEV. 4	294.83		ELEV. 4	288.83		ELEV. 4	321.83	ELEV. 4	318.83
ELEV. 5	317.83		ELEV. 5	311.83	ELEV. 5	291.83		ELEV. 5	290.83		ELEV. 5	284.83		ELEV. 5	317.83	ELEV. 5	314.83
ELEV. 6	317.50		ELEV. 6	311.50	ELEV. 6	291.50		ELEV. 6	290.50		ELEV. 6	284.50		ELEV. 6	317.50	ELEV. 6	314.50
ELEV. 7	316.25		ELEV. 7	310.25	ELEV. 7	290.92		ELEV. 7	289.92		ELEV. 7	283.92		ELEV. 7	316.25	ELEV. 7	313.25
ELEV. 8	316.00	Ì	ELEV. 8	310.00	ELEV. 8	290.67		ELEV. 8	287.92		ELEV. 8	281.92		ELEV. 8	316.00	ELEV. 8	313.00
DIMENS	SIONS	Ī	DIMENS	SIONS	DIMENS	SIONS	ſ	DIMENS	SIONS	ſ	DIMENS	IONS		DIMENS	IONS	DIMENS	IONS
, V ,	38'±	ſ	'A'	11'±	'A'	varies		'A'	29'±		'A'	varies		,V,	30'±	'A'	varies
'B'	14.5'±	I	'B'	44'±	,B,	varies	I	'B'	18'±	I	'B'	varies		'B'	13'±	'B'	varies
TOTAL SF	540	I	TOTAL SF	523	TOTAL SF	390		TOTAL SF	494	I	TOTAL SF	1,100		TOTAL SF	283'	TOTAL SF	529
OUTFALL	PIPE	I	OUTFALL	PIPE	OUTFALL	PIPE		OUTFALL	PIPE		OUTFALL	PIPE		OUTFALL	PIPE	OUTFALL	PIPE
SIZE	12"	• [SIZE	12"	SIZE	4"		SIZE	4"	I	SIZE	4"		SIZE	12"	SIZE	12"
LENGTH	NA		LENGTH	NA	LENGTH	36'		LENGTH	91		LENGTH	42'		LENGTH	NA	LENGTH	NA
SLOPE	NA		SLOPE	NA	SLOPE	2.0%	Ĺ	SLOPE	2.1%		SLOPE	4.6%		SLOPE	NA	SLOPE	NA

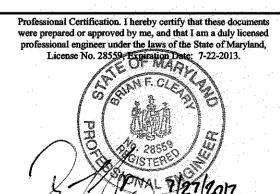
Professional Certification Thereby contity was these documents were propared or approved by line, and that Fam a duty licensed professional engineer under the laws of the State of Maryland. License No. 45577 Expiration Date: 96.08.18

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Z	9.6.2016	Revise MB #26 = #25 Chart based on As-built Conditi	ouse
			4 Volume
NO.	DATE	REVISION	

BENCHMARK ENGINEERS A LAND SURVEYORS A PLANNERS ENGINEERING, INC.

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8480 BALTIMORE NATIONAL PIKE & SUITE 418 & ELLICOTT CITY, MARYLAND 21043 (P) 410-465-6105 (F) 410-465-6644 60 THOMAS JOHNSON DRIVE ▲ FREDERICK, MARYLAND 21702 (P) 301-371-3505 (F) 301-371-3506



OWNER: SIMPSON MILL LLC P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041 410-465-4244

PHASE 1 - LOTS 1 thru 18 & 139 thru 150 PHASE 2 - LOTS 19 thru 138 OPEN SPACE LOTS 151 & 152 AND PARCEL 'A' GRID: 23 PARCEL: 116 ZONED: R-SA-8 GRID: 23 PARCEL: 258, 476 ZONED: POR ELECTION DISTRICT NO. 5 HOWARD COUNTY, MARYLAND TAX MAP: 35

SIMPSON MILL

STORMWATER MANAGEMENT DETAILS

410-465-4244 DATE: AUGUST, 2012 BEI PROJECT NO: 2189

AS SHOWN

SCALE:

SDP-12-015

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DEVELOPER:

SIMPSON MILL LLC P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041

9/79/16 DESIGN: DBT/JMC DRAWN: DBT/JMC

CONSTRUCTION SPECIFICATIONS

These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent

Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped to topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the toe of the embankment. Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish

and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet structure shall be

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

<u>Moterial</u> — The fill material shall be taken from approved designated borrow areas. If shall be free of roots, stumps, wood, rubbish, stones greater than 6", frozen or other

objectionable material. Fill material for the center of the embankment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer.

Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment

<u>Placement</u> — Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment

Compaction - the movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with teh equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not be so wet that water can be squeezed out.

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within \pm 2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

Cut Off Trench - The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be a least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the cores shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed concurrently with the outer shell of the embankment

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistively of 2,000 ohm—cm. Material shall be placed such that a minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sided of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24 or greater over the structure or pipe. Backfill material outside the structural backfill (flowable fill) zone shall be of the type and quality conforming to that specified for the core of the embankment or other embankment

All pipes shall be circular in cross section

Planting soil
[2' to 4' deep]

Organic content

Curtain drain

Geotextile

Pea gravel diaphragn

Gravel (underdrains and

Poured in place concrete (if

P:\2189 Riverdale\dwg\8024.dwg, 7/27/2012 2:29:04 PM

nfiltration berms)

Underdrain piping

Corrugated Metal Pipe - all of the following criteria shall apply for corrugated metal pipe: 1. Materials — (Polymer Coated steel pipe) — Steel pipes with polymeric coatings shall bave a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 &

Maerials — (Aluminum Coated Steel Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M—274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.

Materials — (Aluminum Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M—196 or M—211 with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be

2. Coupling bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at least 24 mils in thickness.

3. Connections - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti—seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.

Appendix B.4. Construction Specifications for Environmental Site Design Practices

Specification

sandy loam (30%),

coarse sand (30%) & compost (40%)

(ASTM D 2974)

shredded hardwood

pea gravel: ASTM-D-448

ornamental stone: washed

MSHA Mix No. 3; f = 3500

psi @ 28 days, normal weight,

r-entrained; reinforcing to

meet ASTM-615-60

see Appendix A, Table A.4

loamy sand (60 - 65%) &

Table B.4.1 Materials Specifications for Micro-Bioretention, Rain Gardens & Eandscape Infiltration:

(1/8" TO 3/8")

stone: 2" to 5

NO. 57 OR NO. 6

AGGREGATE

(3/8" to 3/4")

PVC or SDR35

B.4.7

F 758, Type PS 28 or AASHTO 4" to 6" rigid schedule 40

AASHTO-M-6 or ASTM-C-33 0.02" to 0.04"

All connection shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches in diameter: flanges on both ends of the pipe with a circular 3/8 inch closed cell neoprene aasket, prepunched to the flange bolt circle, sandwiched between adjacent flanges; a 12-inch wide standard lap type band with 12-inch wide by 3/8-inch thick closed cell circular neoprene gasket; and a 12—inch wide hugger typé bánd with o—ring gaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by a 24 inch long annular corrugated band using minimum of 4 (four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8 inch closed cell gaskets the full width of the

Helically corrugated pipe shall have either continuously welded seams or have lock seams

4. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

5. Backfilling shall conform to "Structure Backfill".

6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings. Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete

1. Materials — Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C—361.

2. Bedding — Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used a described in the "Structure Backfill" section of this standard. Grovel

3. Laying pipe — Bell and spigot pipe shall be places with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation form the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

292

286

280

4. Backfilling shall conform to "Structure Backfill".

5. Other details (anti-seep collars, valves, etc.) shall be shown on the drawings. Plastic Pipe - The following criteria shall apply for plastic pipe:

. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4' - 10" inch pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" inch shall meet the requirements of AASHTO M294 Type S.

- 2. Joints and connections to anti-seep collars shall be completely watertight 3. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.
- 4. Backfilling shall conform to "Structure Backfill".
- 5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings. <u>Drainage Diaphragms</u> - When a drainage diaphragm is used, a registered professional engineer will supervise the design and construction inspection.

Rock Riprap

plantings are site-specific

E Type I nonwoven

galvanized hardware cloth

ISDA soil types loamy sand or sandy loam; clay content < 5%

Slotted or perforated pipe; 3/8" perf. @ 6" on center, 4 holes per

row; minimum of 3" of gravel over pipes; not necessary underneath pipes. Perforated pipe shall be wrapped with 14-inch

28 day strength and slump test; all concrete design (cast-in-place

standards requires design drawings sealed and approved by a

[H-10 or H-20]; allowable horizontal loading (based on soil

Sand substitutions such as Diabase and Graystone (AASHTO) #10 are not acceptable. No calcium carbonated or dolomitic sand

substitutions are acceptable. No "rock dust" can be used for sand.

professional structural engineer licensed in the State of Marylan

- design to include meeting ACI Code 350.R/89; vertical loading

or pre-cast) not using previously approved State or local

on-site testing of poured-in-place concrete required:

pressures); and analysis of potential cracking

aged 6 months, minimum; no pine or wood chips

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414,

Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 311. Geotextile shall be placed under all riprap and shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

All work on permanent structures shall be carried out in areas free from water. The contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect the areas to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the evacuations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the location being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall be pumped.

All borrow areas shall be graded to provide proper drainage and left in a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting

Construction operations will be carried out in such a manner that erosion will be controlled

and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures.

(MD-342) or as shown on the accompanying drawings.

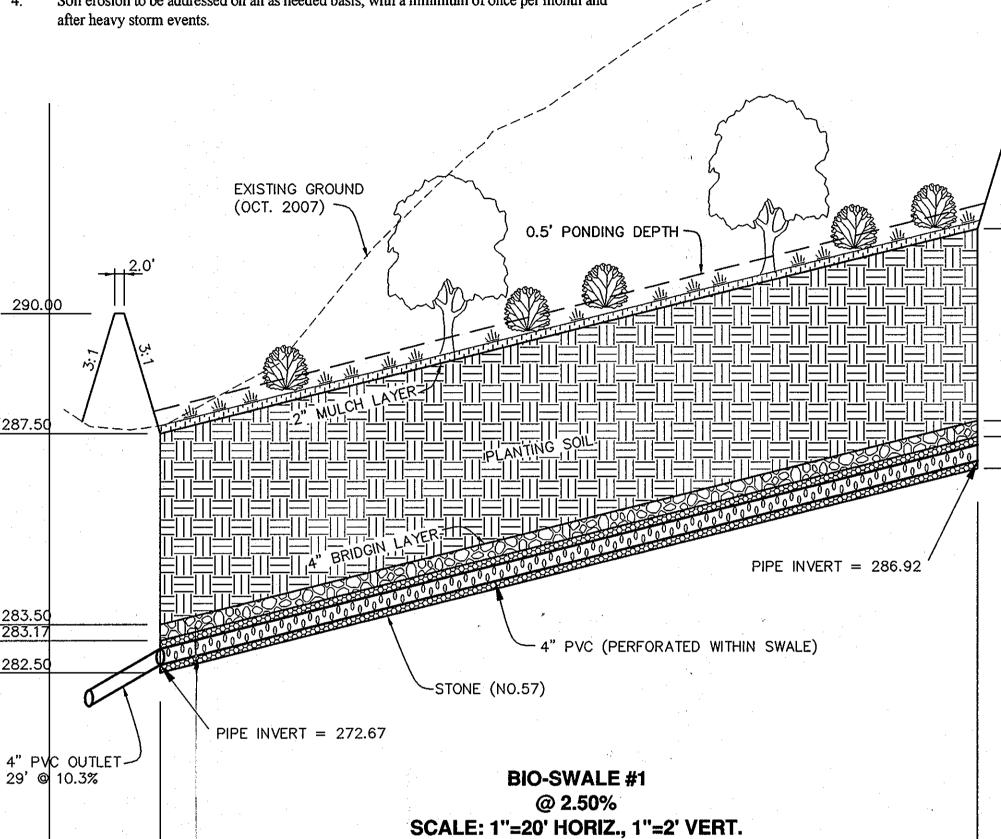
OPERATION AND MAINTENANCE SCHEDULE FOR LANDSCAPE INFILTRATION (M-3) MICRO-BIORETENTION (M-6). RAIN GARDENS (M-7), BIORETENTION SWALE (M-8), ENHANCED FILTERS (M-9)

Annual maintenance of plant material, mulch layer and soil layer is required. Maintenance of mulch and soil is limited to correcting areas of erosion or wash out. Any mulch replacement shall be done in the spring. Plant material shall be checked for disease and insect infestation and maintenance will address dead material and pruning. Acceptable replacement plant material is limited to the following: 2000 Maryland Stormwater Design Manual Volume II, Table A.4.1 and 2.

Schedule of plant inspection will be twice a year in spring and fall. This inspection will include removal of dead and diseased vegetation considered beyond treatment, treatment of all diseased trees and shrubs and replacement of all deficient stakes and wires.

Mulch shall be inspected each spring. Remove previous mulch layer before applying new layer once every 2 to 3 years.

Soil erosion to be addressed on an as needed basis, with a minimum of once per month and



326 296 326 324 290 318 318 (1-37) ∠0.5' PONDING DEPTI 316 316 315.00 284 PIPE INVERT = 314.67 282 312 312 4" PVC (PERFORATED WITHIN SWALE) -STONE (NO.57) -310 PIPE INVERT = 309.67 **BIO-SWALE #2** @ 3.00% 15" HDPEP-SCALE: 1"=20' HORIZ., 1"=2' VERT. 308 308 278 (OUT) INV. 309.67 306 APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING 10/5/12 Millennum

EXISTING GROUND

(OCT. 2007)

- PLANTINGS ELEV. 1 2" MULCH LAYER 4" BRIDGING LAYER (1/8"-3/8" STONE)— ELEV. 5 8" STONE RESERVOIR ELEV. 6 (NO.57 STONE)

BIO-SWALE TYPICAL SECTION DETAIL

4"ø MIN.

PERF. PVC

UNDERDRAIN

ELEV. 7

WIDTH

BIO-SWALE DESIGN TABLES

328

NOTE: ELEVATIONS LISTED IN CHART ARE ELEVATIONS AT BOTTOM OF THE SWALE.

SWALE	#1
Α	1.0'
В	4.0'
С	3:1
SLOPE	2.5%
ELEV. 1	290.00
ELEV. 2	289.50
ELEV. 3	287.50
ELEV. 4	283.50
ELEV. 5	283.17
ELEV. 6	282.67
ELEV. 7	282.50
BOTTOM	DIMS
LENGTH	170'
WIDTH	4.0'
TOTAL SF	680

		HTDIV			4.0'	
	TO	TAL SF	-		680	
1 10	ORM YR YR OYR	Q (RU 0.30 1.01 1.73	CF CF	ร์ ร	0.69 1.09	FI

SWALE	#2
Α	1.0'
В	3.0'
Ô	3:1
SLOPE	3.0%
ELEV. 1	314.50
ELEV. 2	314.00
ELEV. 3	313.50
ELEV. 4	310.50
ELEV. 5	310.17
ELEV. 6	309.67
ELEV. 7	309.50
воттом	DIMS
LENGTH	150'
WIDTH	6.0'
TOTAL SF	900

0.57 CFS 0.80 FPS 2.35 CFS 1.39 FPS

4.07 CFS 1.70 FPS

10YR

ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE A SUITE 418 A ELLICOTT CITY, MARYLAND 21043 (P) 410-465-6105 (F) 410-465-6644 60 THOMAS JOHNSON DRIVE ▲ FREDERICK, MARYLAND 21702 (P) 301-371-3505 (F) 301-371-3506 WWW.BEI-CIMLENGINEERING.COM OWNER: P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041 410-465-4244

NO.

DATE

DEVELOPER: SIMPSON MILL LLC P.O. BOX 417 ELLICOTT CITY, MARYLAND 21041 410-465-4244

BENCHMARK

ENGINEERS & LAND SURVEYORS & PLANNERS

REVISION Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly license

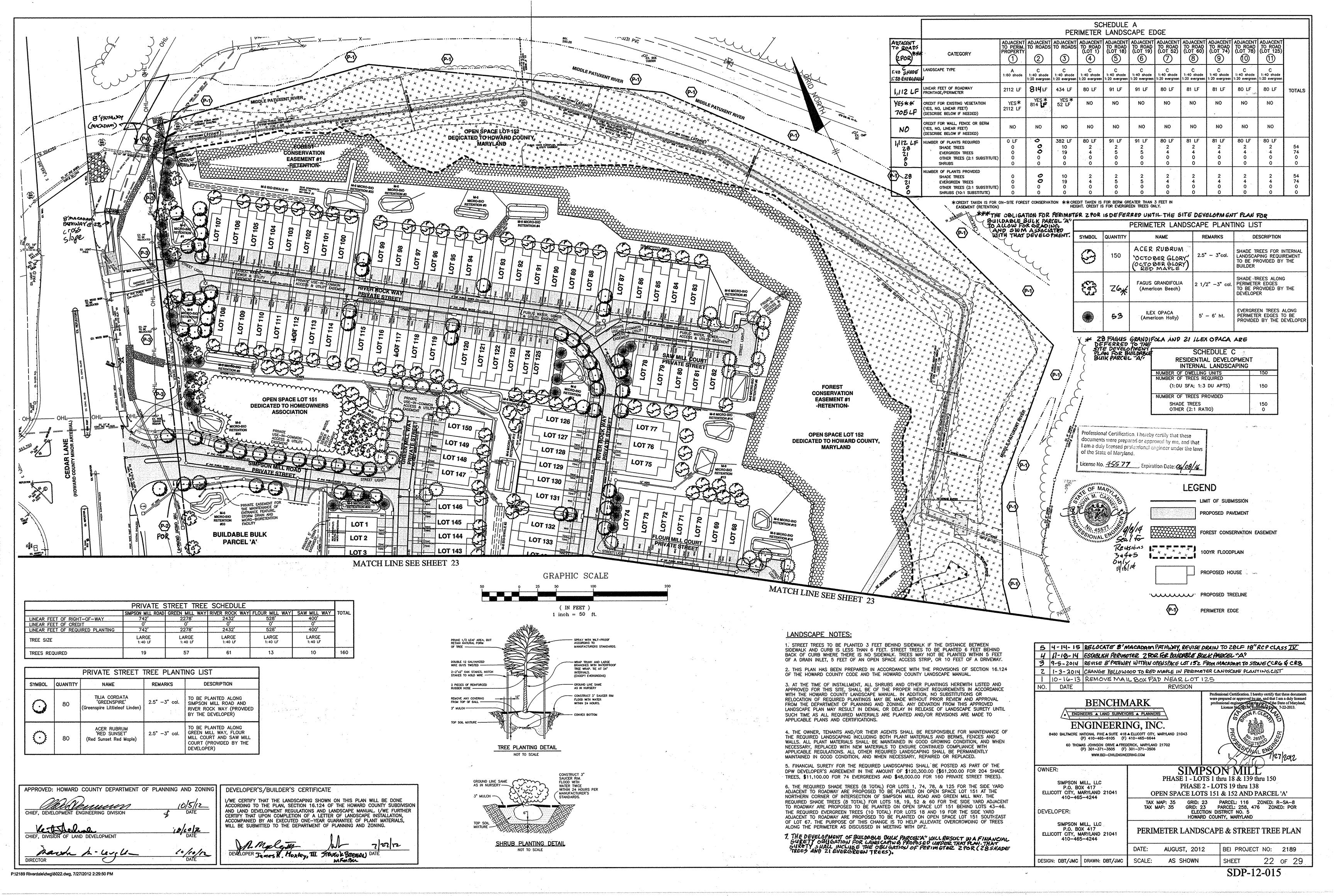
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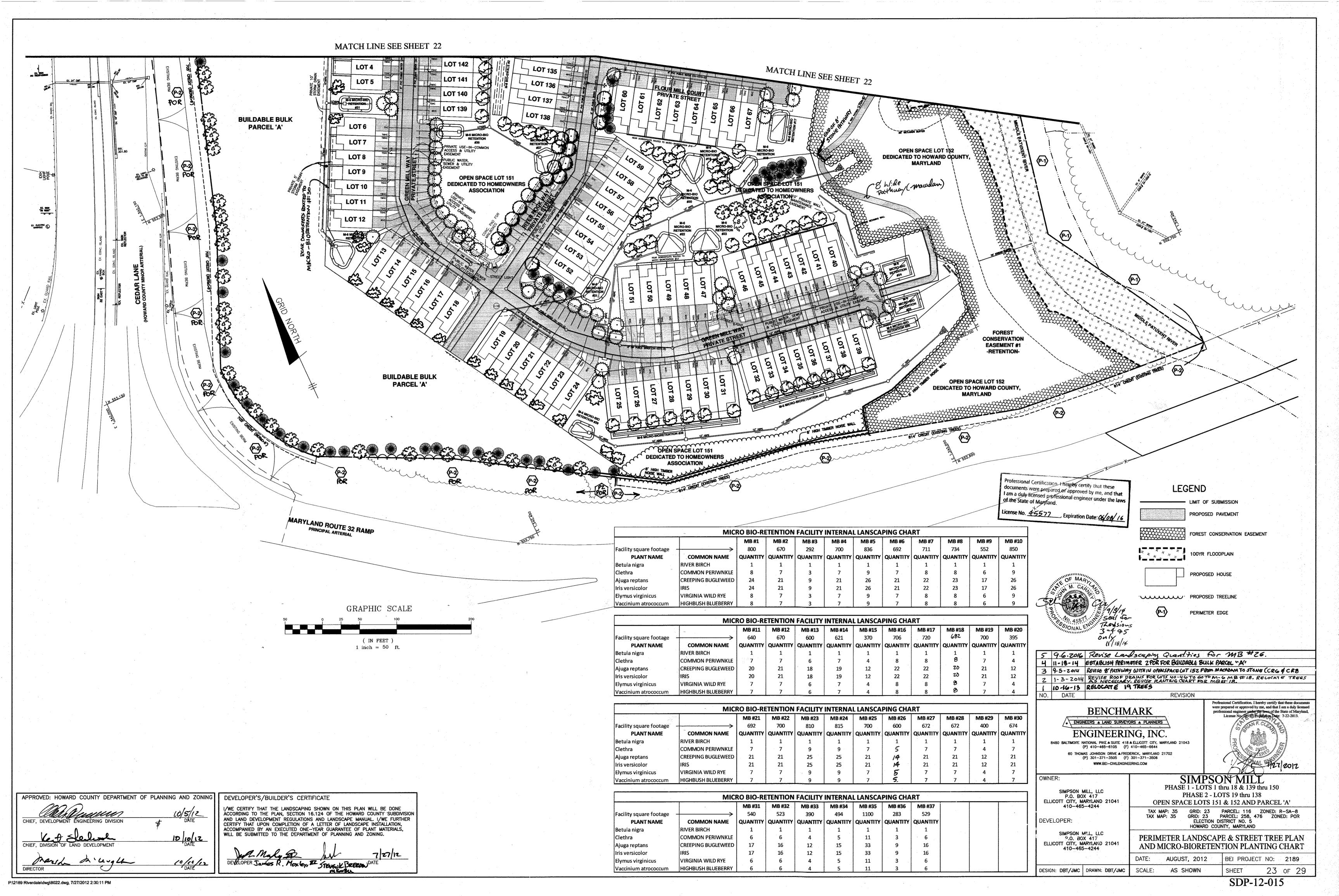
SIMPSON MILL PHASE 1 - LOTS 1 thru 18 & 139 thru 150 PHASE 2 - LOTS 19 thru 138 OPEN SPACE LOTS 151 & 152 AND PARCEL 'A' PARCEL: 116 ZONED: R-SA-8 GRID: 23 PARCEL: 258, 476 ZONED: POR ELECTION DISTRICT NO. 5 HOWARD COUNTY, MARYLAND

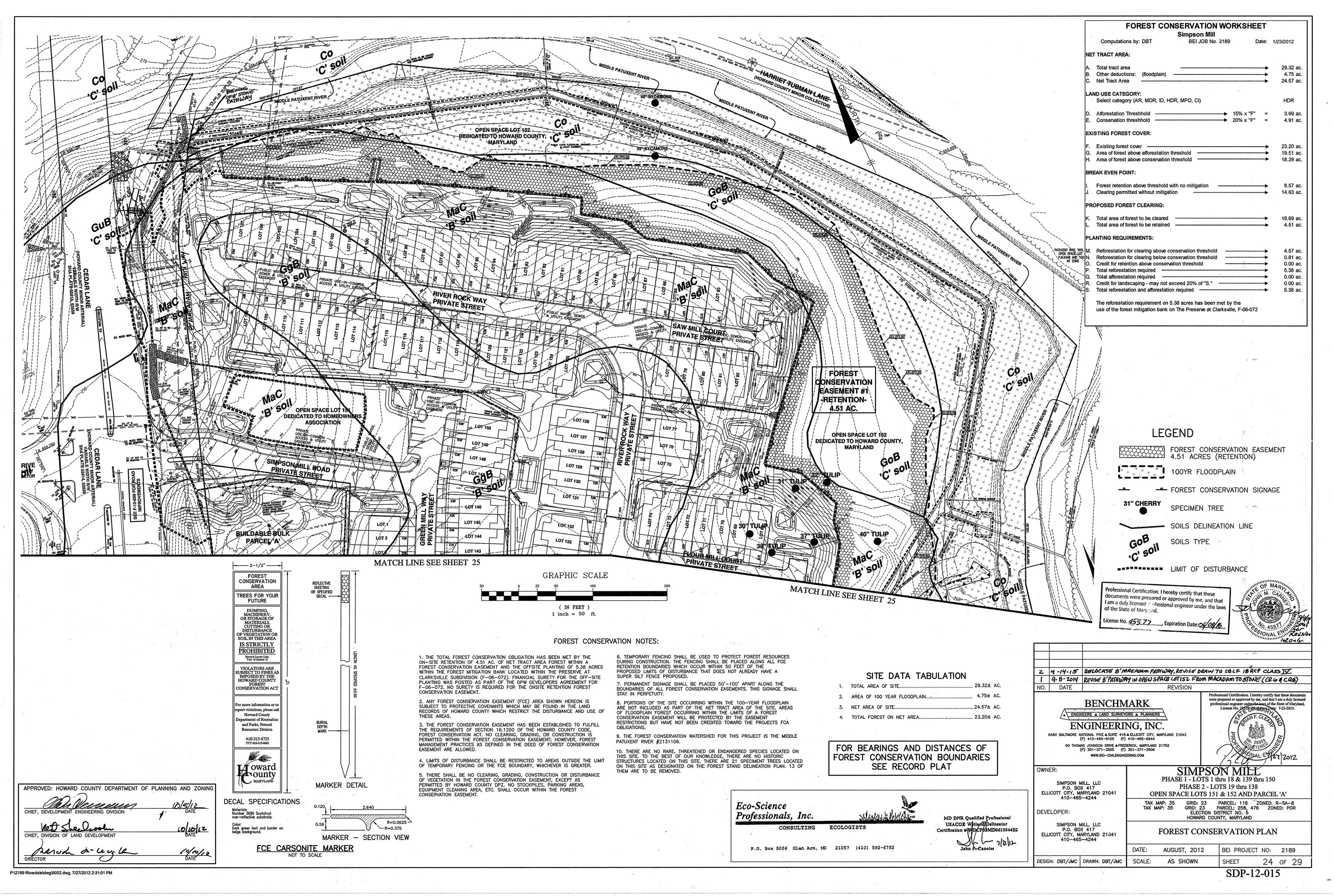
STORMWATER MANAGEMENT DETAILS

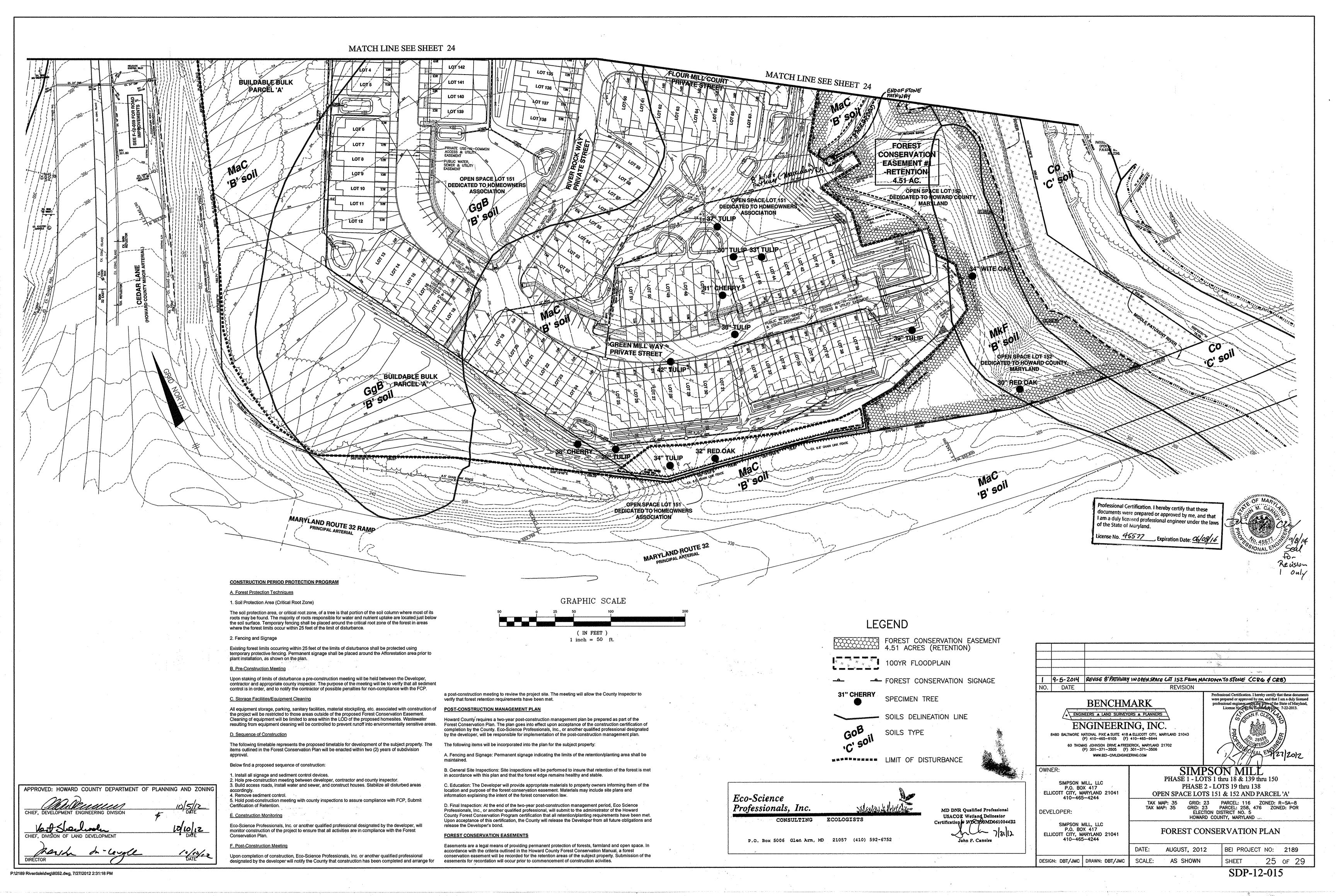
BEI PROJECT NO: 2189 DATE: AUGUST, 2012 DESIGN: DBT/JMC DRAWN: DBT/JMC 21 of 29 SCALE: AS SHOWN

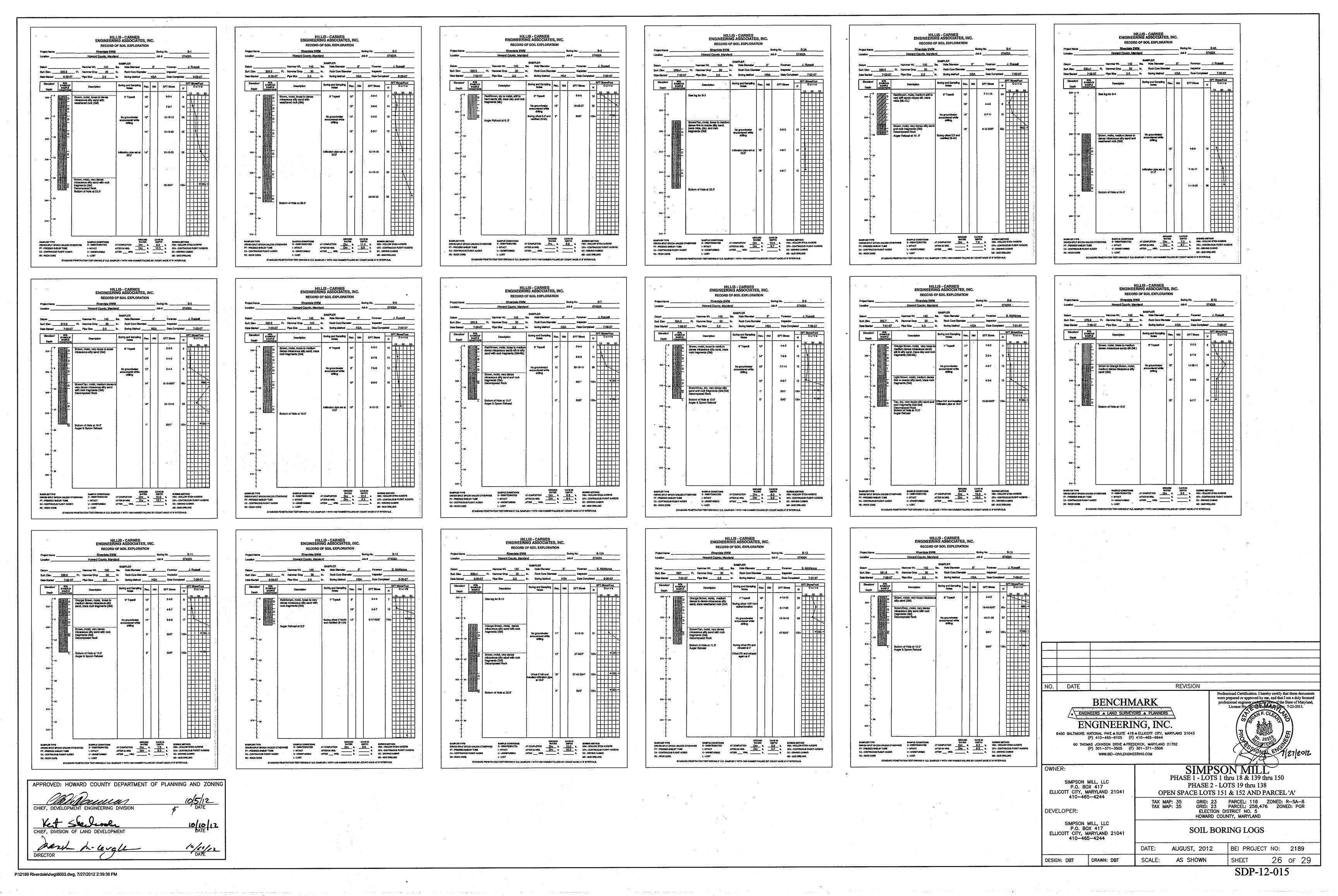
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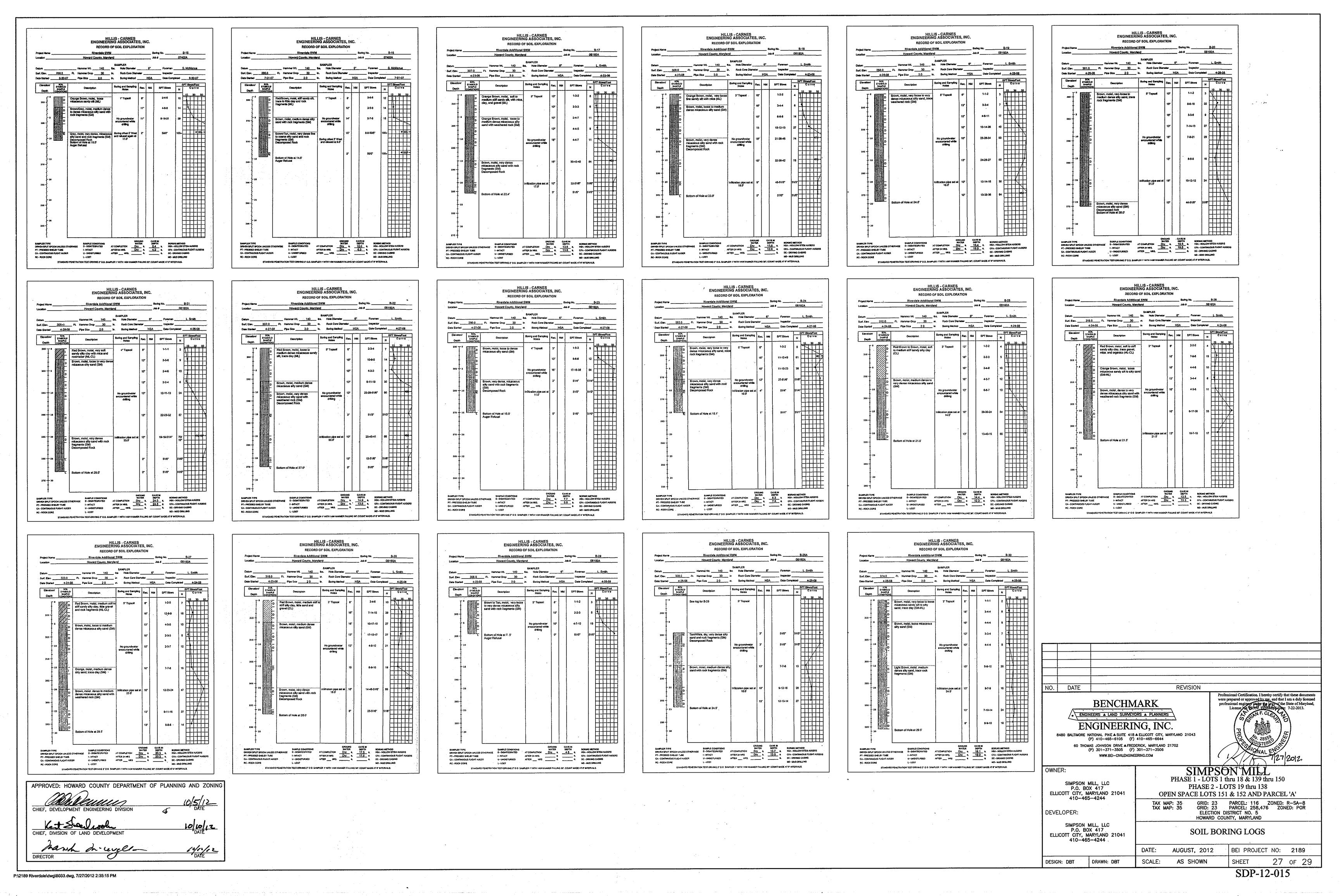


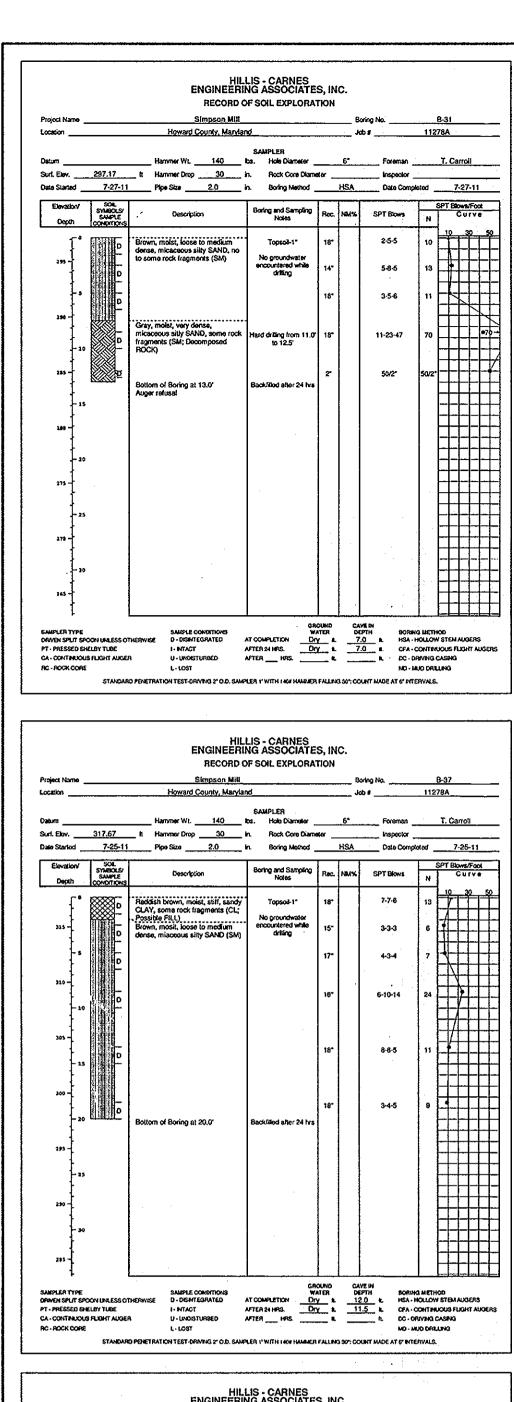


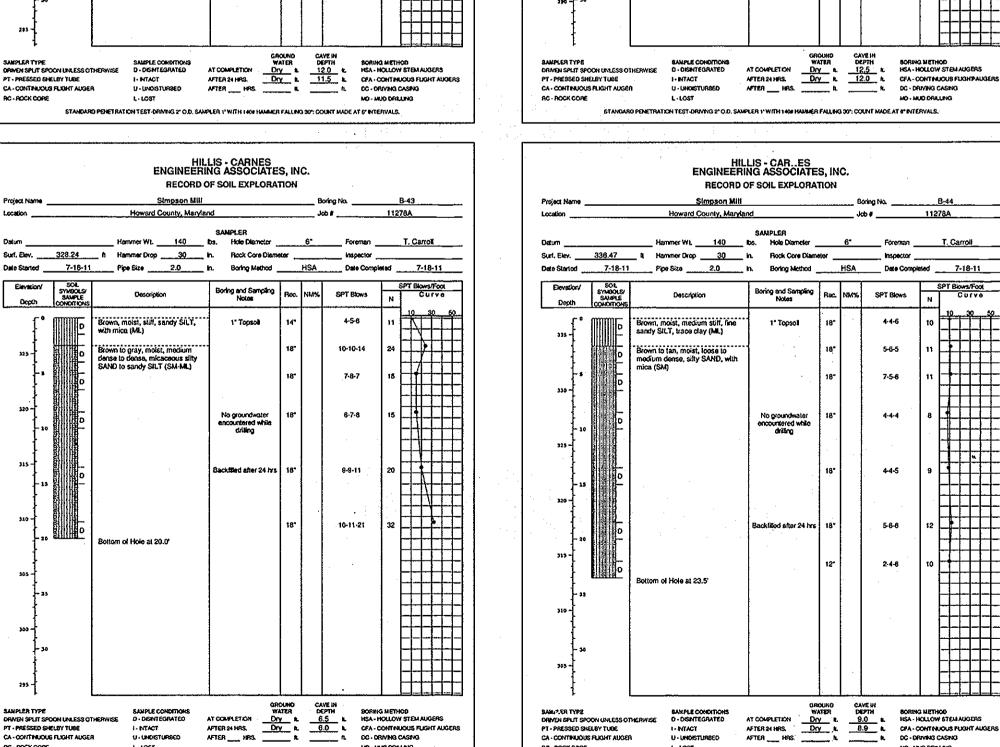












10/10/12 DATE

DATE

SAMPLER TYPE DRIVEN SPLIT SPOON UNLESS OTHERWISE PT - PRESSED SHELBY TUBE CA - CONTINUOUS FLIGHT AUGER

HILLIS - CARNES ENGINEERING ASSOCIATES, INC.

 Datum
 Hammor Wt.
 140
 bs.
 Hole Diameter
 6"
 Foremen
 T. Carroll

 Surf. Elev.
 320.17
 ft
 Hammor Drop
 30
 in.
 Reck Core Diameter
 Inspector

Date Started 7-25-11 Pipe Size 2.0 In. Boring Method HSA Date Completed 7-25-11

STANDARD PERETRATION TEST DRAWING 2" OLD. SAMPLER 1" WISH 1404 HAWKER FALLING 30"; COUNT MADE AT 8" INTERVALS

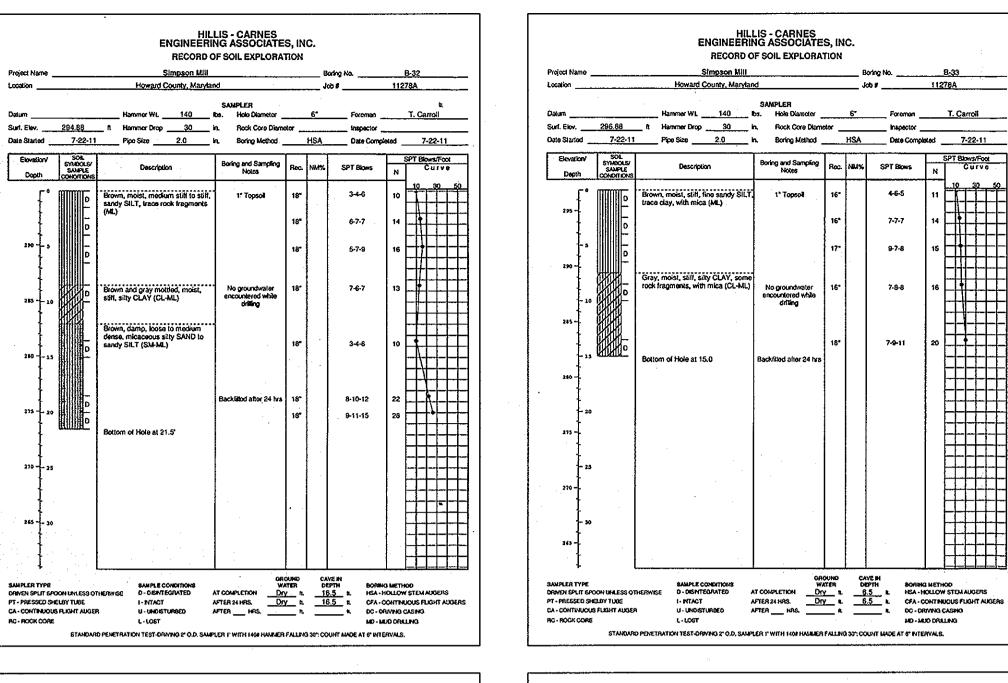
Brown, moist, stilf, fine sandy SILT.

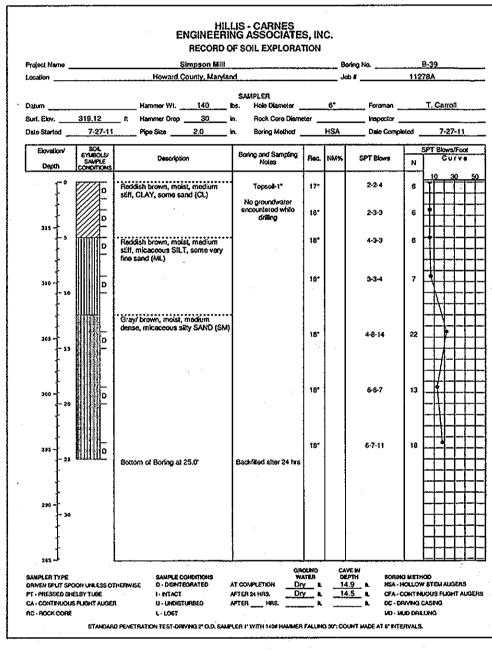
Brwon, moist, medium dense, micaceous silly SAND (SM)

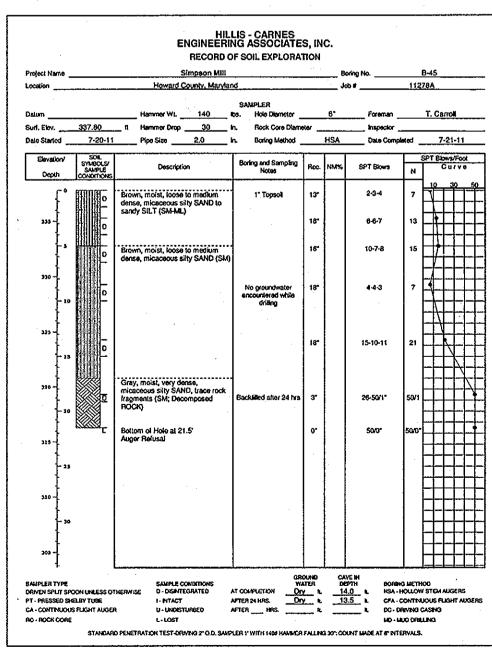
Brown, moist, very dense, micaceous sitty SAND (SM; Decomposed ROCK) Bottom of Boring at 18.8'

Boring and Sampling Rec. NM% SPT Blows

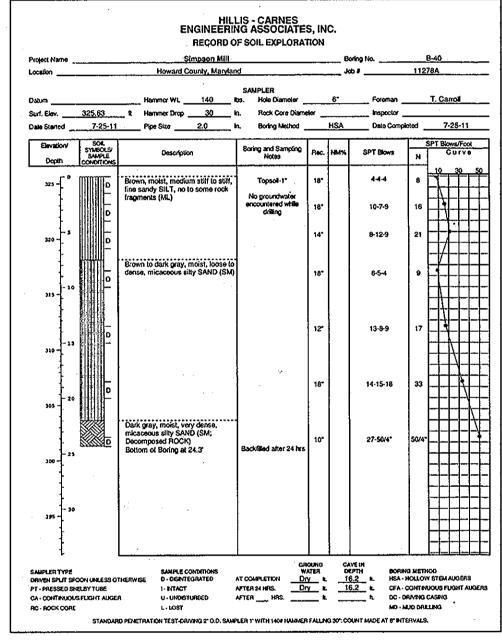
RECORD OF SOIL EXPLORATION

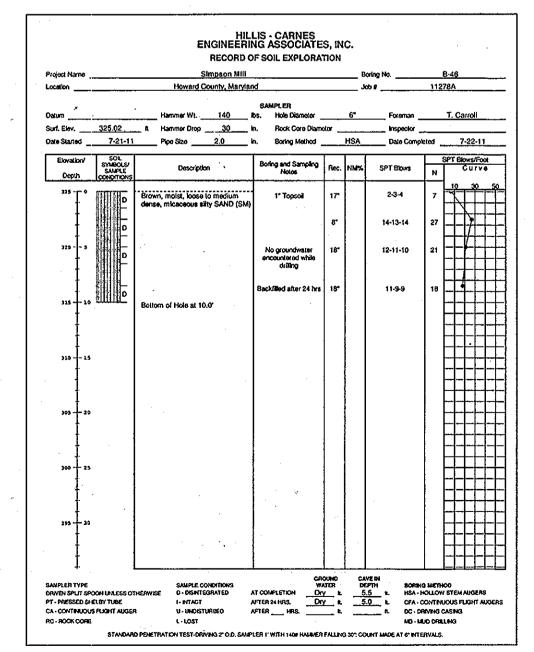


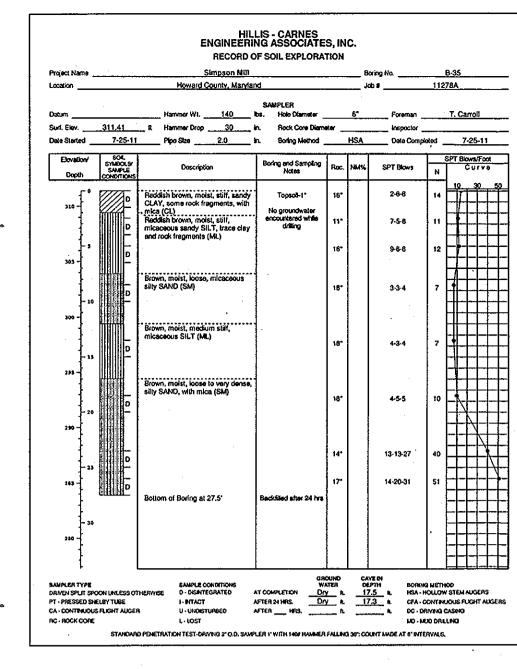


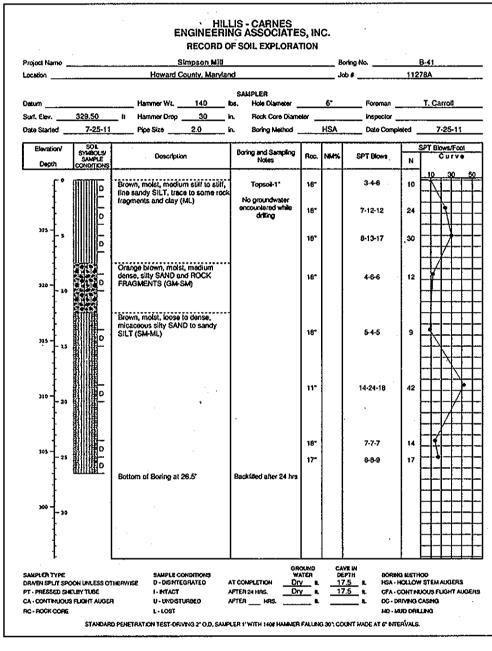


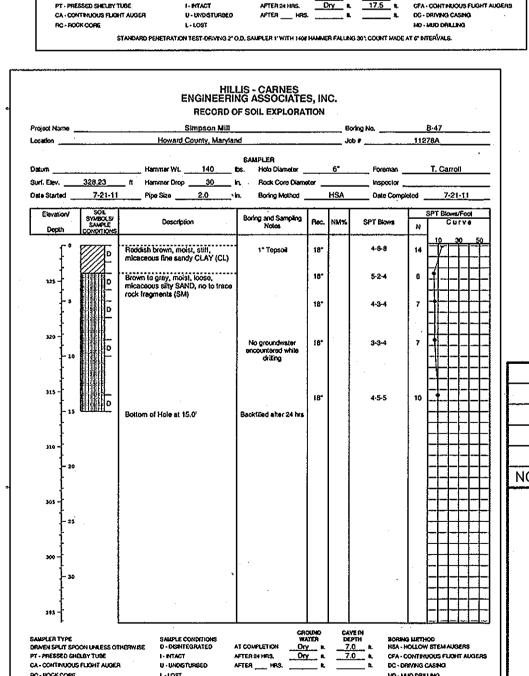
	_		ING ASSOCIATE OF SOIL EXPLORA				
Project Name		Simpson Mill				ng No	
Location		Howard County, Maryl	and		Job	<i></i>	11278A
Datum		Hammer WI140	SAMPLER Tos. Holo Diamotor		.6"	Foreman	T. Carroll
Surl. Elev	298.06	ft Hammer Drop30					
Date Started _	7-22-11	Pipe Size	in. Boring Method		HSA	Date Comp	xieted 7-22-11
Elevation/ Depth	SOL SYMBOLS/ SAMPLE CONDITIONS	Description	Boring and Sampling Notes	Rec.	NM%	SPT Blows	SPT Blows/Fool
Į,	‱ •	Brown, moist, loose, sifty SANO, trace rock fragments, with mica	1º Topsol	17*		5-5-5	10 30
293 -	<u> </u>	(SM; Possible FILL) Brown, moist, stiff, micaceous sandy SILT, some rock fragments		8"		4-4-8	12
	101	(ML)		18*		5-6-7	13
290	0	Orange brown to brown, moist, medium dense, micaceous silty SAND, trace to some rock fragments (SM)	No groundwater encountered while dhiling	15*		7-7-10	17
245	10	asymons (only	Backitied after 24 hrs.	18*		8-8-11	19
- 15	HISTA.	Bottom of Hole at 15.0"					
280							
275 -		•					
25							
270 -							
30		ı.					
265			<u></u>	<u> </u>			
SAMPLER TYPE DRIVEN SPLIT SPO PT - PRESSED SK CA - CONTINUOU: RC - ROCK CORE	ELBY TUBE	I - INTACT	AT COMPLETION Dr	OUND ATER Y IL Y IL	7.0 7.0	H 80RI 1L HSA- 1L CFA- 1L DC-1	NG METROD HOLLOW STEM AUGERS CONTINUOUS FUCHT AU DRIVING CASING MUO DRILLING

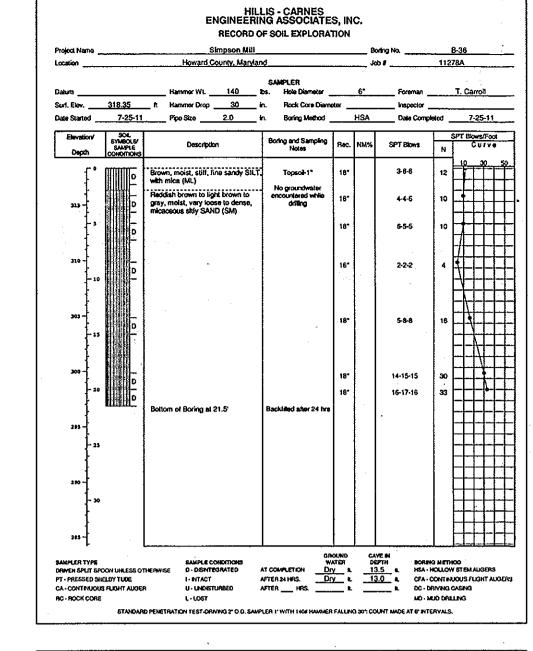


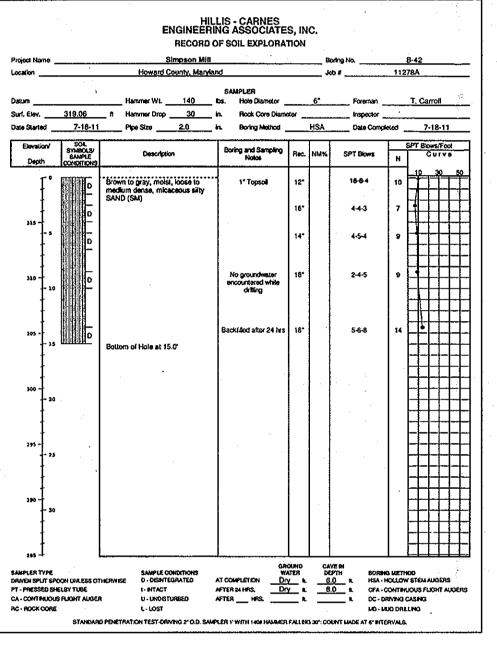


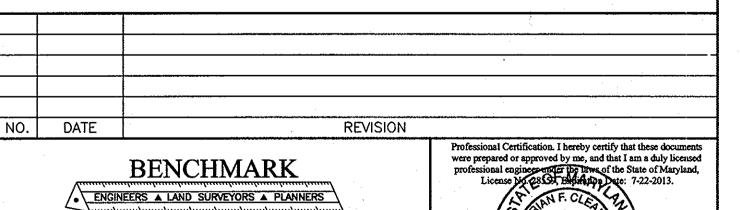












ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE & SUITE 418 & ELLICOTT CITY, MARYLAND 21043 (P) 410-465-6105 (F) 410-465-6644 60 THOMAS JOHNSON DRIVE ▲ FREDERICK, MARYLAND 21702 (P) 301-371-3505 (F) 301-371-3506 WWW.BEI-CIVILENGINEERING.COM

OWNER:	SIMPSON MILL				
SIMPSON MILL, LLC	PHASE 1 - LOTS 1 thru 18 & 139 thru 150				
P.O. BOX 417	PHASE 2 - LOTS 19 thru 138				
ELLICOTT CITY, MARYLAND 21041 410-465-4244	OPEN SPACE LOTS 151 & 152 AND PARCEL 'A'				
DEVELOPER:	TAX MAP: 35 GRID: 23 PARCEL: 116 ZONED: R-SA-8				
	TAX MAP: 35 GRID: 23 PARCEL: 258,476 ZONED: POR ELECTION DISTRICT NO. 5				
	HOWARD COUNTY, MARYLAND				
SIMPSON MILL, LLC P.O. BOX 417	SOIL BORING LOGS				
ELLICOTT CITY, MARYLAND 21041 410-465-4244	2011 2014110 2000				

AUGUST, 2012

AS SHOWN

DATE:

SCALE:

DESIGN: DBT

DRAWN: DBT

28 of 29 SHEET

BEI PROJECT NO: 2189

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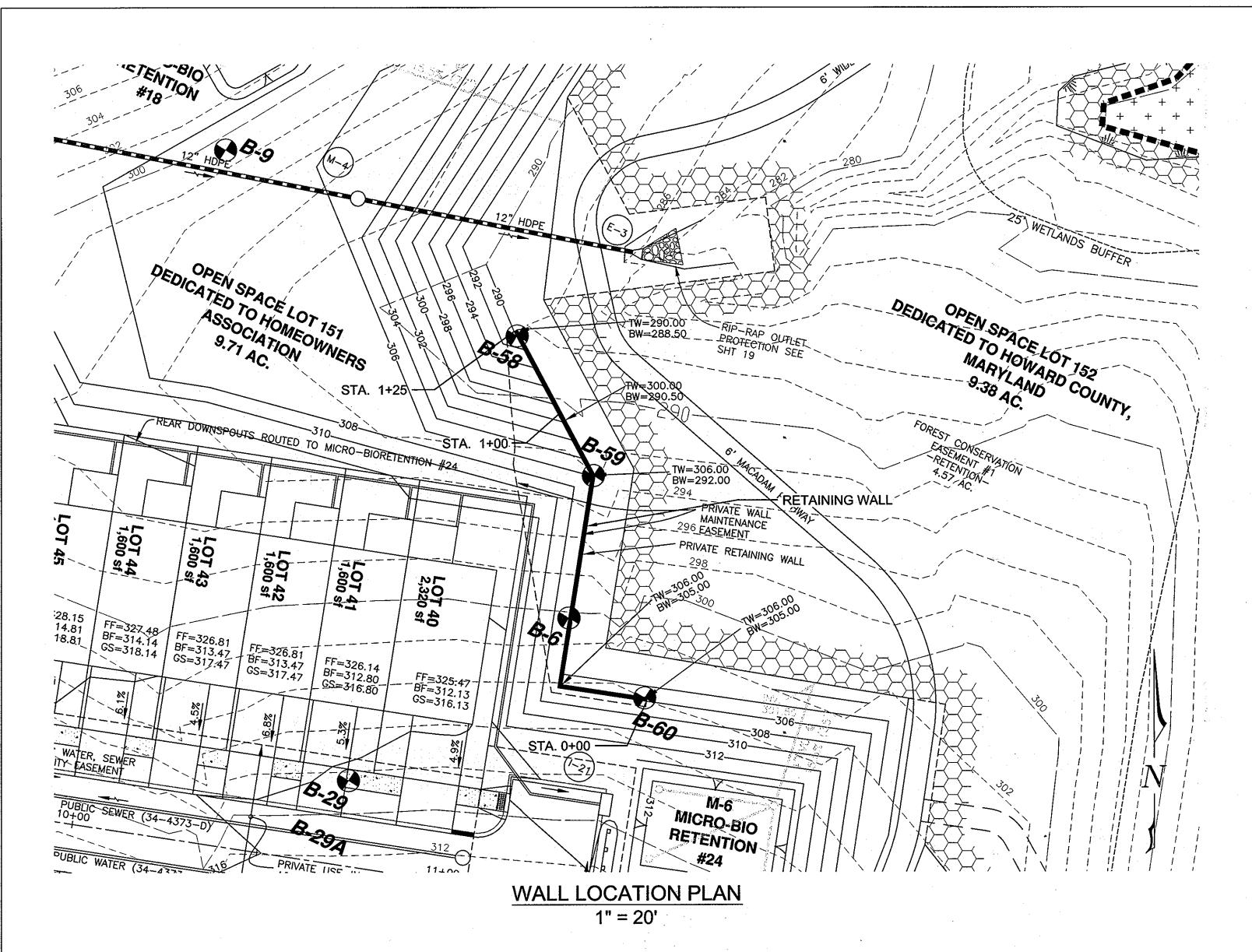
Mender DIVISIO

CHIEF, DEVELOPMENT ENGINEERING DIVISION

march ho length

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

SDP-12-015



Cap_ Top Grade | Grid Embedment Length 307 301 Miragrid Geogrid (Typ.) 292 Additional full length geogrid for bend reinforcement where showh. Provide 3" separation between Geogrid Key overlapping grid layers -◆ | Miragrid 3XT Geogrid Bottom Grade — → Miragrid 5XT Geogrid Block Bottom 🛏 Miragrid 7XT Geogri 1+20 0+60 0+70 0+90 1+00 0+50 1+10 Wall Station 1" = 10'

WALL ELEVATION

SPECIFICATIONS

MODULAR CONCRETE BLOCK RETAINING WALL

PART 1: GENERAL

1.01 Description

- A. Work shall consist of furnishing and construction of a Modular Retaining Wall System in accordance with these specifications and in reasonably close conformity with the lines, grades, design, and dimensions shown on the plans.
- B. Work includes preparing foundation soil, furnishing and installing leveling pad, unit drainage fill and backfill to the lines and grades shown on the construction
- C. Work includes furnishing and installing geogrid soil reinforcement of the type, size, location, and lengths designated on the construction drawings.

1.02 Delivery, Storage and Handling

- A. Contractor shall check all materials upon delivery to assure that the proper type, grade, color, and certification has been received.
- B. Contractor shall protect all materials from damage due to job site conditions and in accordance with manufacturer's recommendations. Damaged materials shall not be incorporated into the work.

PART 2: PRODUCTS

2.01 Modular Concrete Retaining Wall Units

A. Modular concrete units shall conform to the following architectural requirements: face color - color may be specified by the Owner.

face finish - sculptured rock face in angular tri-planer or flat configuration. Other face finishes will not be rallowed without written approval of Owner.

bond configuration - running with bonds nominally located at midpoint vertically adjacent units, in both straight and curved alignments.

exposed surfaces of units shall be free of chips, cracks or other imperfections when viewed from a distance of 10 feet under diffused lighting.

- B. Modular concrete materials shall conform to the requirements of ASTM C1372 - Standard Specifications for Segmental Retaining Wall Units.
- C. Modular concrete units shall conform to the following structural and geometric requirements measured in accordance with appropriate references:

compressive strength = 3000 psi minimum; absorption = 8% maximum (6% in northern states) for standard weight aggregates;

dimensional tolerances = $\pm 1/8$ " from nominal unit dimensions not including rough split face, ±1/16" unit height - top and bottom planes; unit size - 8" (H) x 18" (W) x 12 (D) minimum;

unit weight - 75 lbs/unit minimum for standard weight

inter-unit shear strength - 1000 plf minimum at 2 psi normal pressure; at 2 psi normal force.

geogrid/unit peak connection strength - 1000 plf

- D. Modular concrete units shall conform to the following constructability requirements: (if applicable)
- vertical setback = 1/8"± per course (near vertical) or

1"+ per course per the design; alignment and grid positioning mechanism - fiberglass

maximum horizontal gap between erected units shall be - 1/2 inch.

pins, two per unit minimum;

2.02 Shear Connectors (if applicable)

A. Shear connectors shall be 1/2 inch diameter thermoset isopthalic polyester resin-protruded fiberglass reinforcement rods or equivalent to provide connection between vertically and horizontally adjacent units. Strength of shear connectors between vertical adjacent units shall be applicable over a design temperature of 10 degrees F to + 100 degrees F. B. Shear connectors shall be capable of holding the geogrid in the proper design position during grid

pre-tensioning and backfilling. 2.03 Base Leveling Pad Material

A. Material shall consist of a compacted #57 crushed stone base as shown on the construction drawings.

2.04 Unit Drainage Fill

A. Unit drainage fill shall consist of #57crushed stone

2.05 Reinforced Backfill

A. Reinforced backfill shall type SM, be free of debris and meet the following gradation tested in accordance with ASTM D-422 and meet other properties shown on the

<u>Sieve Size</u>	Percent Passi
2 inch	100-75
3/4 inch	100-75
No. 40	0-60
No. 200	0-40

Plasticity Index (PI) <10 and Liquid Limit <35 per ASTM D-4318.

 B. Material can be site excavated soils where the above requirements can be met. Unsuitable soils for backfill (high plastic clays or organic soils) shall not be used in the reinforced soil mass.

2.06 Geogrid Soil Reinforcement

A. Geosynthetic reinforcement shall consist of geogrids manufactured specifically for soil reinforcement

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applications and shall be manufactured from high tenacity polyester yarn.

2.07 Drainage Pipe

A. The drainage pipe shall be perforated corrugated HDPE pipe manufactured in accordance with ASTM

PART 3 EXECUTION

3.01 Excavation

A. Contractor shall excavate to the lines and grades shown on the construction drawings. Owner's representative shall be responsible for inspecting and approving the excavation prior to placement of leveling material or fill soils.

3.02 Base Leveling Pad

- A. Leveling pad material shall be placed to the lines and grades shown on the construction drawings, to a minimum thickness of 6 inches and extend laterally a minimum of 6" in front and behind the modular wall
- B. Leveling pad shall be prepared to insure full contact to the base surface of the concrete units.

3.03 Modular Unit Installation

- A. First course of units shall be placed on the leveling pad at the appropriate line and grade. Alignment and level shall be checked in all directions and insure that all units are in full contact with the base and properly seated.
- B. Place the front of units side-by-side. Do not leave gaps between adjacent units. Layout of corners and curves shall be in accordance with manufacturer's recommendations.
- C. Install shear/connecting devices per manufacturer's recommendations.
- D. Place and compact drainage fill within and behind wall units. Place and compact backfill soil behind drainage fill. Follow wall erection and drainage fill closely with structure backfill.
- E. Maximum stacked vertical height of wall units, prior to unit drainage fill and backfill placement and compaction, shall not exceed three courses

3.04 Structural Geogrid Installation

- A. Geogrid shall be oriented with the highest strength axis perpendicular to the wall alignment.
- B. Geogrid reinforcement shall be placed at the strengths. lengths, and elevations shown on the construction design drawings or as directed by the Engineer.
- C. The geogrid shall be laid horizontally on compacted backfill and attached to the modular wall units. Place the next course of modular concrete units over the geogrid. The geogrid shall be pulled taut, and anchored prior to backfill placement on the geogrid.

D. Geogrid reinforcements shall be continuous throughout their embedment lengths and placed side-by-side to provide 100% coverage at each level. Spliced connections between shorter pieces of geogrid or gaps between adjacent pieces of geogrid are not

3.05 Reinforced Backfill Placement

permitted.

- A. Reinforced backfill shall be placed, spread, and compacted in such a manner that minimizes the development of slack in the geogrid and installation damage.
- B. Reinforced backfill shall be placed and compacted in lifts not to exceed 6 inches where hand compaction is used, or 8 - 10 inches where heavy compaction equipment is used. Lift thickness shall be decreased to achieve the required density as required.
- C. Reinforced backfill shall be compacted to 95% of the maximum density as determined by ASTM D698. The moisture content of the backfill material prior to and during compaction shall be uniformly distributed throughout each layer and shall be + 3% to - 3% of optimum.
- D. Only lightweight hand-operated equipment shall be allowed within 3 feet from the tail of the modular concrete unit.
- E. Tracked construction equipment shall not be operated directly upon the geogrid reinforcement. A minimum fill thickness of 6 inches is required prior to operation of tracked vehicles over the geogrid. Tracked vehicle turning should be kept to a minimum to prevent tracks from displacing the fill and damaging the geogrid.
- F. Rubber tired equipment may pass over geogrid reinforcement at slow speeds, less than 10 MPH. Sudden braking and sharp turning shall be avoided.
- G. At the end of each day's operation, the Contractor shall slope the last lift of reinforced backfill away from the wall units to direct runoff away from wall face. The Contractor shall not allow surface runoff from adjacent areas to enter the wall construction site.

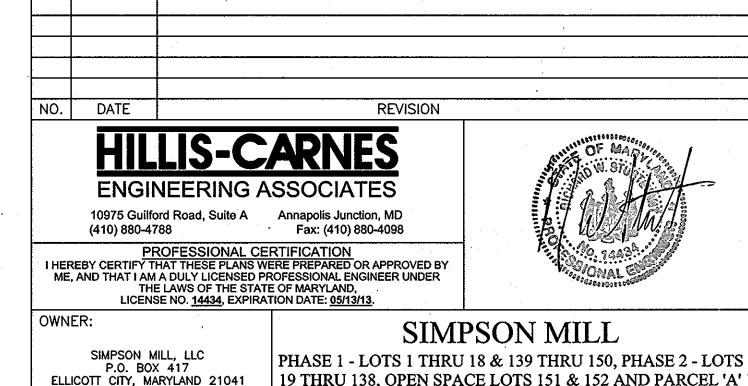
3.06 Cap Installation

A. Cap units shall be glued to underlying units with an all-weather adhesive recommended by the manufacturer.

3.07 Field Quality Control

- A. The Owner shall engage inspection and testing services, including independent laboratories, to provide quality assurance and testing services during construction.
- B. As a minimum, quality assurance testing should include foundation soil inspection, soil and backfill testing, verification of design parameters, and observation of construction for general compliance with design drawings and specifications.

- No trees shall be planted within 10 feet of the top of the retaining wall.
- Retaining walls shall only be constructed under the observation of a registered professional engineer and a (NICET, WACEL, or equiv.) certified soils technician.
- One soil boring shall be required every one hundred feet along the entire length of the wall. Copies of all boring reports shall be provided to the Howard County Inspector Prior to the start of
- The required bearing pressure beneath the wall system shall be verified in the field by a certified soils technician. Testing documentation must be provided to the Howard County Inspector prior to start of construction. The required bearing test shall be the Dynamic Cone Penetrometer test ASTM
- The suitability of fill material shall be confirmed by the on-site soils technician. Each 8" lift must be compacted to a minimum 95% standard proctor density and the testing report shall be made available to the Howard County Inspector upon completion of construction.
- Walls shall not be constructed on uncertified fill materials.
- Walls shall not be constructed within a Howard Co. right-of-way or easement.



19 THRU 138, OPEN SPACE LOTS 151 & 152 AND PARCEL 'A'

410-465-4244 GRID: 23 PARCEL: 116 ZONED: R-SA-8 GRID: 23 PARCEL: 258, 476 ZONED: POR ELECTION DISTRICT NO. 5 **DEVELOPER:** SIMPSON MILL, LLC P.O. BOX 417

410-465-4244

HOWARD COUNTY, MARYLAND **RETAINING WALL** CONSTRUCTION DETAILS DATE: AUGUST, 2012 HCEA PROJECT NO: 11278-B SCALE: AS SHOWN SHEET 29 of 29

SDP-12-015

