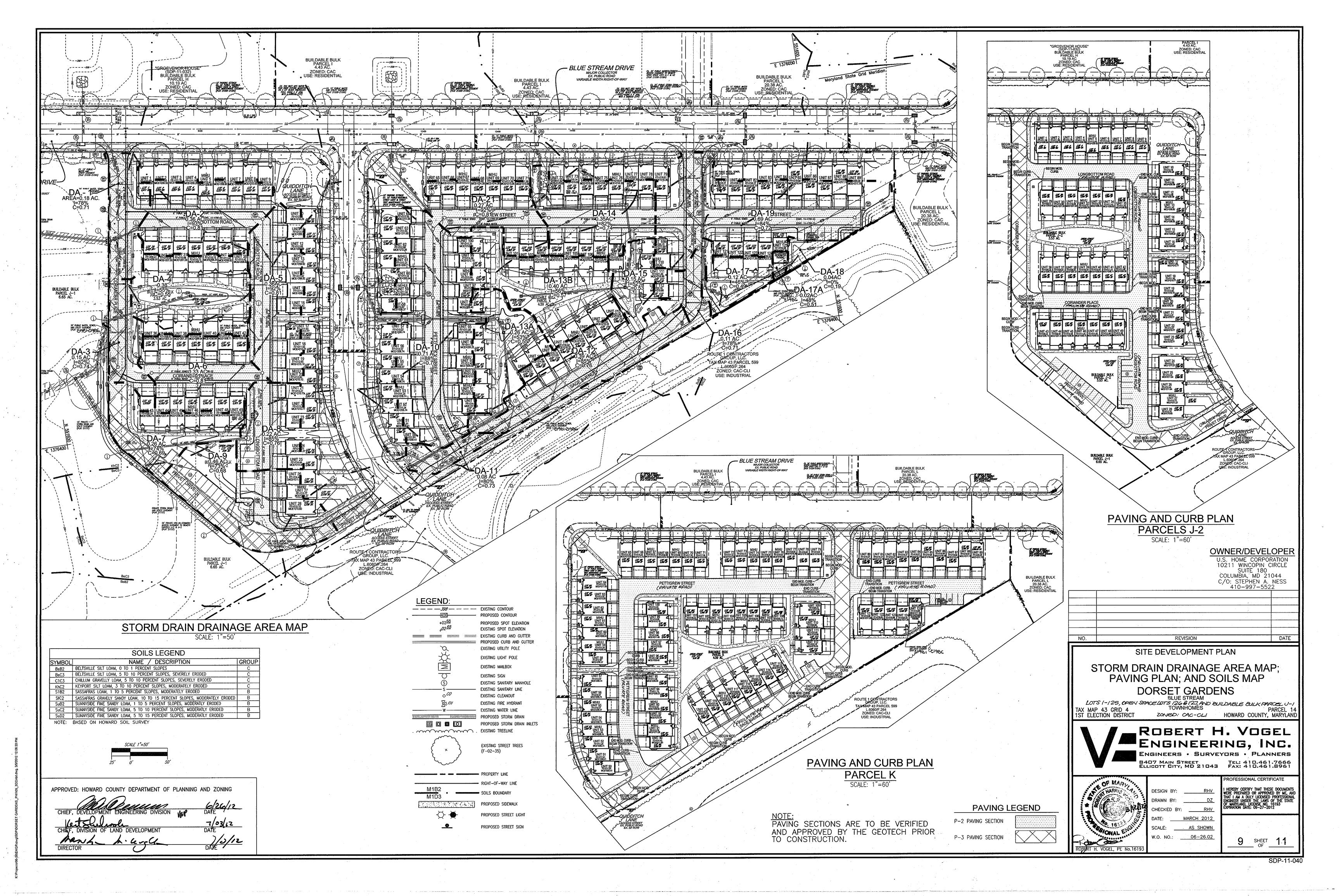
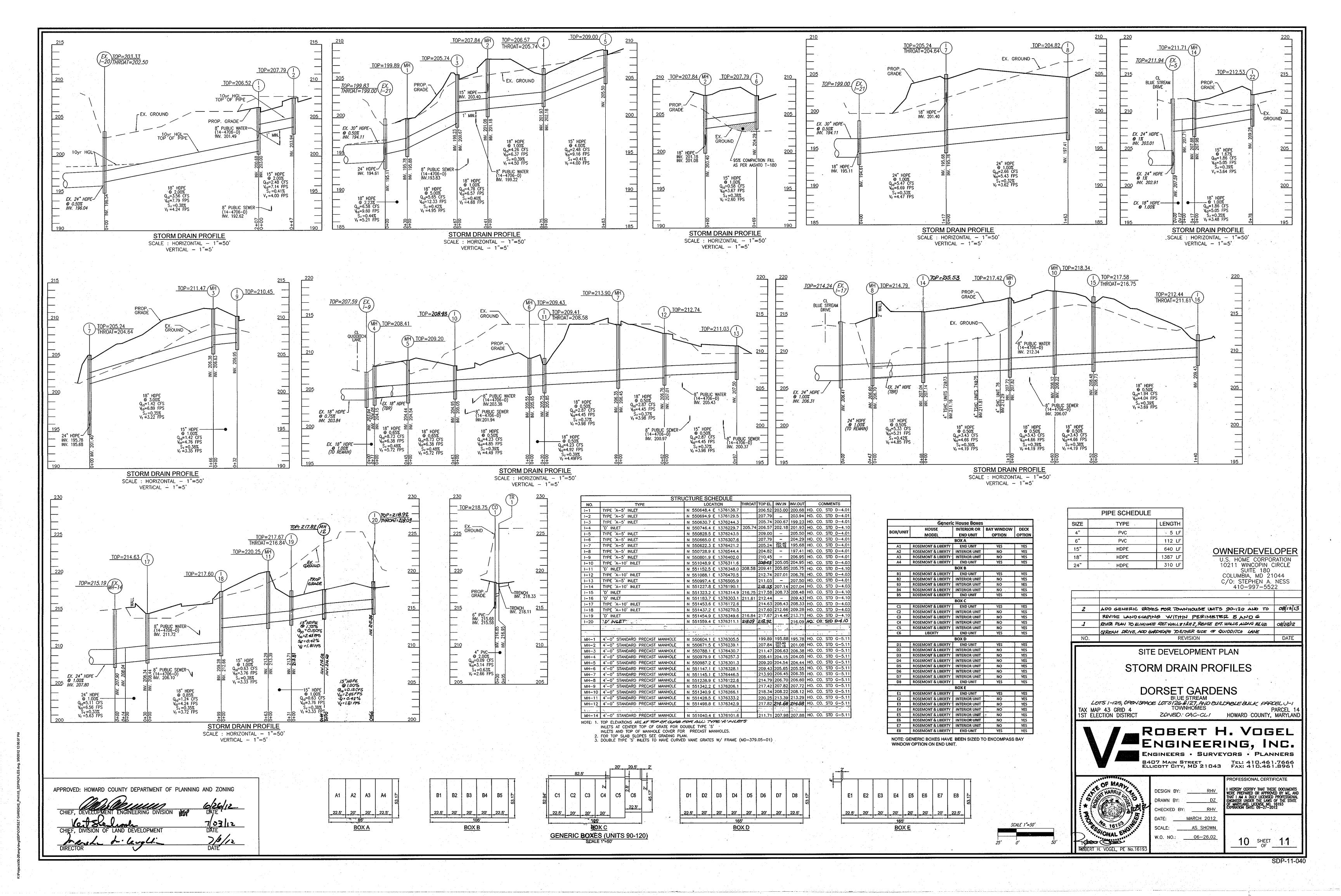
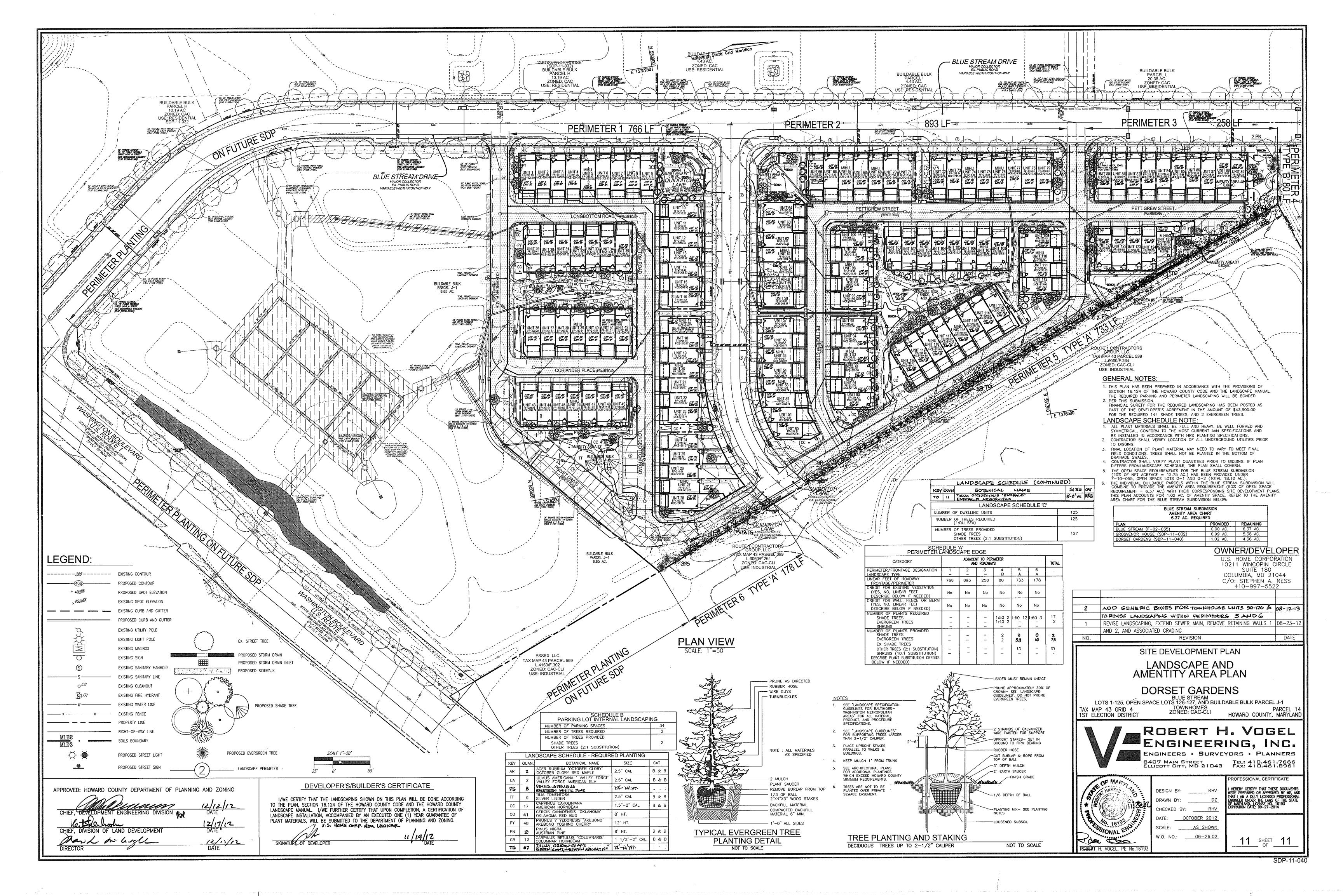
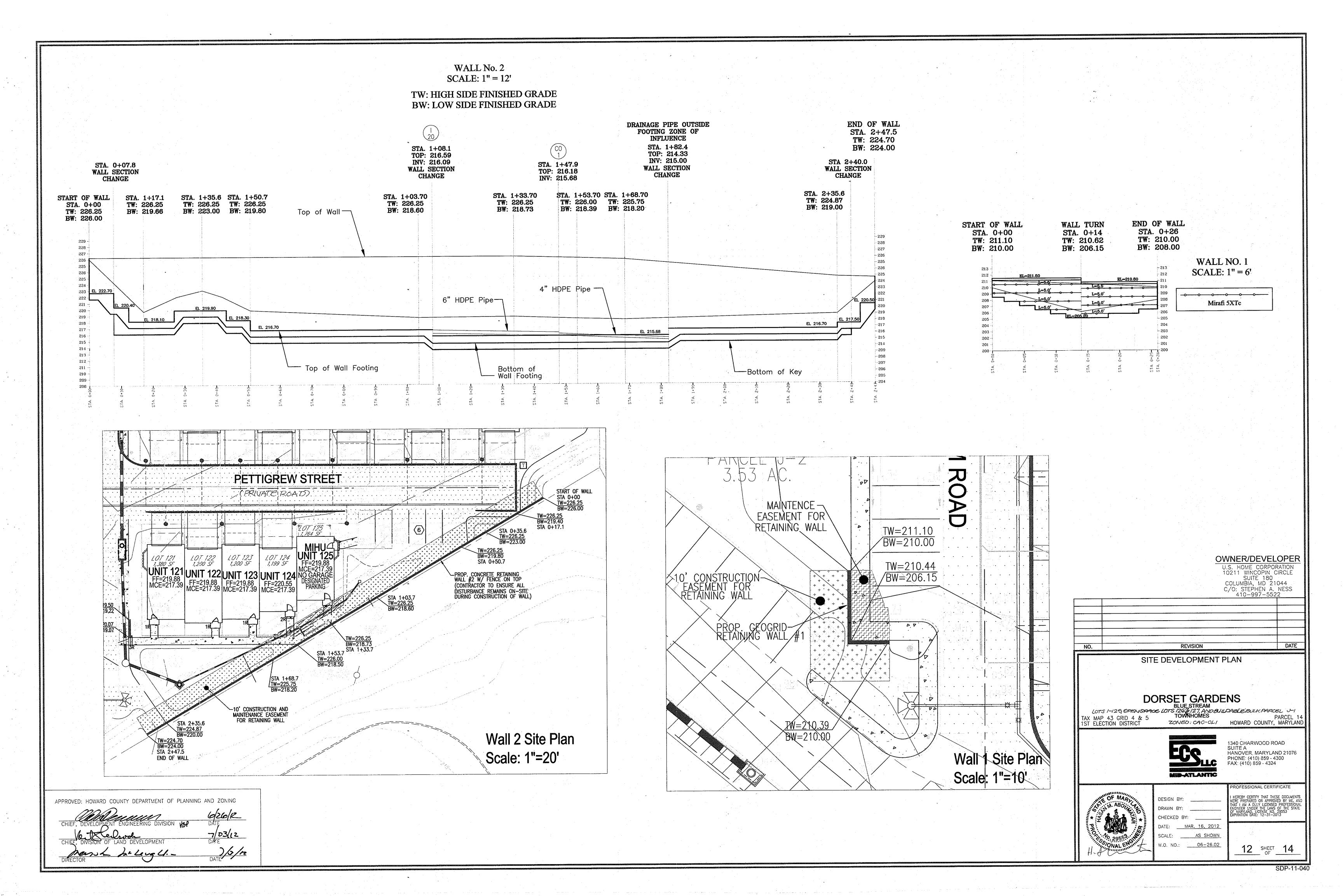


DP-11-04

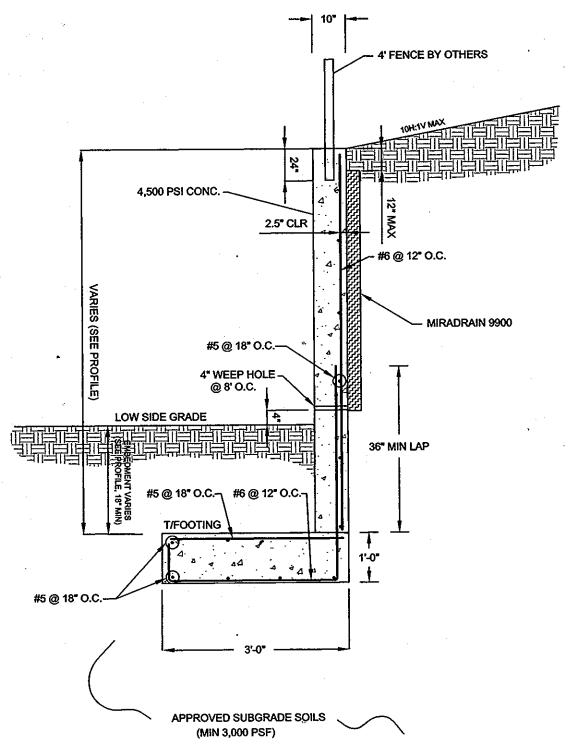








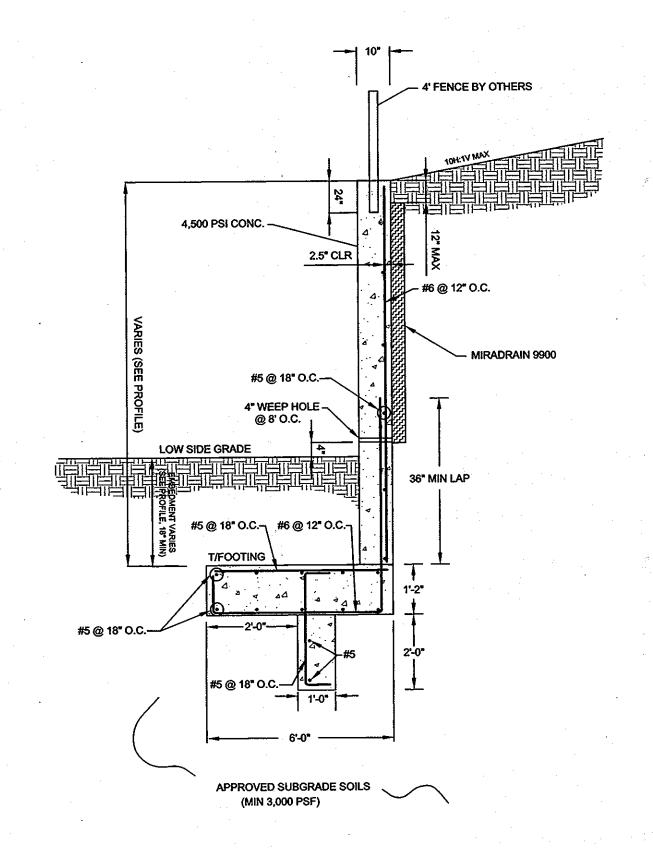
## Retaining Wall Specifications and Guidelines Part 1: General 1.01 Description A. Retaining walls must be constructed under the supervision of a Maryland Registered Professional Engineer. B. Work includes preparation of foundation solls, furnishing all materials, and installing all materials to the lines and grades shown on the construction drawings. A. "International Building Code - 2009", International Code Council, Inc. B. "ACI Manual of Concrete Practice - Parts 1 Through 5 - 2001" C. "Manual of Standard Practice" - Concrete Steel Reinforcing Institute D. "American Society for Testing and Materials" 1.03 Bamage, Storage, and Handling A. The Contractor shall check the materials upon delivery to assure that the proper materials have been received. B. The Contractor shall properly handle and store the materials to prevent damage to the materials. Damaged materials shall not be incorporated into the wall, A. The Owner shall engage a qualified testing agency to provide observation and testing services as described below. 1. The agency shall inspect the formwork and reinforcing steel placement for compliance with the contract documents. Reinforcing steel should be inspected for correct size, quantity, and spacing. 2. Fresh concrete shall be sampled in accordance with ASTM C 172, and tested for slump, air entrainment, and temperature. 3. Test cylinders shall be molded in accordance with ASTM C 31. Four test cylinders shall be molded for each day's pour, or for every 50 cubic yards of concrete placed, whichever is greater. B. Concrete Placement C. Fill Placement 1. All soil fills shall be tested in accordance with ASTM D 2922. 2. A minimum of one compaction test per lift should be made per 2,500 square feet of fill lift area, but not fewer than two tests per lift should be made. 3. The elevations and locations of the field density tests should be clearly identified at the time of fill placement and compaction. Part 2: Materials 2.01 Concrete A. Concrete shall conform to Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414 B. Concrete shall have a minimum 28-day compressive strength of 4,500 psi. C. Concrete shall have a slump range of 2 to 5 inches and shall be air entrained to 6% (+/- 1%) by volume. D. Concrete shall have a minimum density of 145 pcf and a maximum water-to-cement ratio of 0.45. 2.02 Steel Reinforcement A. Steel reinforcing shall conform to ASTM A-615, Grade 60. B. Submit shop drawings at least 15 business days before date reviewed submittals will be needed. Shop drawings shall bear the contractor's stamp of approval which shall constitute that he has verified all field measurements, construction criteria, materials, and similar data, and has checked each drawing for completeness, coordination, and compliance with contract documents. 2.03 Soil Backfill A. Material should consist of soil classified as SM, SC, or more granular, in accordance with ASTM D 2487. B. Material should have no particle larger than 2.5 inches and shall contain no more than 30 percent, by weight, passing the U.S. No. 200 sieve. C. Materials should have a Liquid Limit less than 40, and a Plasticity Index less than 12. D. Material should have a minimum friction angle of 30 degrees and a minimum dry unit weight of 125 pcf. E. The Contractor should submit samples of the proposed backfill soils to the Geotechnical Engineer of Record for approval prior to their use. 2.04 Brainage Board A. Brainage board used behind the walls shall consist of Miradrain 9900. Part 3: Construction A. All existing underground utilities shall be properly marked, and relocated if necessary, prior to construction. B. All proposed underground utilities or structures in the general wall area shall be completely installed prior to the construction of the wall. C. Protect all existing and/or new structures from damage by construction equipment. Immediately repair any damage that may occur. A. The wall foundation shall be excavated to the grades and lines as shown on the construction drawings. Contractor should take care not to disturb foundation soils beyond the lines and grades shown. 3.02 Foundation B. The foundation shall bear at the minimum embedment depths indicated, as measured from the final grade at the front of the wall. C. The foundation subgrade soils shall be testing by a qualified representative of the Geotechnical Engineer to verify the availability of the design bearing pressure of 3,000 psf. D. If unsuitable soils are encountered at design foundation levels, the unsuitable soils shall be removed and the over-excavated areas shall be replaced with compacted structural fill. 3.03 Steel Reinforcement A. All steel reinforcing shall have a minimum clear cover of 3 inches unless otherwise noted on the contract documents. B. Where applicable, splices for reinforcing steel shall be made by contact tension lap splices. C. Welding and field-bending of reinforcing steel is not permitted. D. Furnish all accessories, chairs, space bars, supports, etc. necessary to secure reinforcing. 3.04 Cast-In-Place Concrete. 1. The vertical faces of the footing and key excavation may be used as forms for placement of foundation concrete. Foundation concrete, or protective mud mats, should be placed the same day that the foundation subgrade is approved 3. Provide concrete protection against freezing during placement and for 5 days thereafter. B. Wall Concrete .. Joined to the lines and grades shown on the construction drawings. 1. Furnish and erect concrete forms to the lines and grades shown on the construction drawings. 2. Locate construction joints as to not impair the strength of the structure, but not more than 60 feet in any direction. Provide continuous bentonite strip waterstrip at all construction joints. 3. Make stops in concrete pours using vertical bulkheads. 4. All reinforcing shall be continuous through Joints and bulkheads. 5. Chamfer exposed concrete corners 3/4" by 3/4" minimum. 6. Provide 4' diameter weep holes every 8 feet along the bottom of the wall and at wall ends. The weep holes should be formed in place prior to concrete placement by using PCV pipe. Weep hole b. Provide 4- diameter weep notes every a feet along the pottom of the wall and at wall ends. The weep notes should be formed in place prior to concrete placement by using rev pipe. Weep note locations must not interfere with steel reinforcing, and shall be no greater than 4 inches above final grade at the front of the wall. 7. Where a fence is required, it is recommended that the fence posts be installed during wall concrete placement. The fence posts shall have a minimum of 24 inches of embedment into the wall, and be located along the center of the wall. Alternatively, provide 4 inch diameter by 24 inch deep post holes at the designated fence post locations along the centerline of the wall. The post holes should be formed in place prior to concrete placement by using PVC pipe. An alternate method for fastening the fence to the wall may be utilized, provided it meets Howard County holes and the provided in place prior to concrete placement by using PVC pipe. An alternate method for fastening the fence to the wall may be utilized, provided it meets Howard County holes with the provided in place prior to concrete placement by using PVC pipe. An alternate method for fastening the fence to the wall may be utilized, provided it meets Howard County holes were provided in the p 3.05 Backfilling A. All soil backfill shall conform to the material requirements of section 2.03. B. Backfill shall be moisture conditioned to within 2 percentage points of the optimum moisture content, as determined in accordance with ASTM D-698. C. Backfill shall be placed in loose lifts, not exceeding 8 inches in thickness, and then compacted to at least 95 percent of the maximum dry density, as determined in accordance with ASTM D-698. D. Backfilling shall not occur against the wall until the wall concrete has attained at least 75 percent of the 28-day design strength, and no earlier than 3 days after placement. E. Where feasible, maintain equal grades on each side of the wall during backfilling to prevent overturning and lateral movements. When the grade differential at the wall exceeds 12 inches, only hand-operated compaction equipment shall be allowed. F. Drainage boards shall be placed against the wall, extending from the weep hole up within 12 inches of final grade at the top of the wall. A. Final grades at the wall shall be established by the Contractor in accordance with the most recent site grading plans. B. Final grades shall be stabilized and seeded per the approved civil plans unless noted otherwise on the site grading plans. C. Install fence at the top of the wall in accordance with project documents. If fence posts are installed subsequent to wall construction, the fence posts shall be grouted into the PVC post holes using 3,000 psi non-shrink grout. D. Concrete wall face shall have faux stone finish. See Architectural or Landscape plans and specifications for additional wall finish details and additional fence details. $\frac{\text{FOOTING STEP}}{\text{NTS}}$ APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING



WALL 2 TYPICAL SECTION

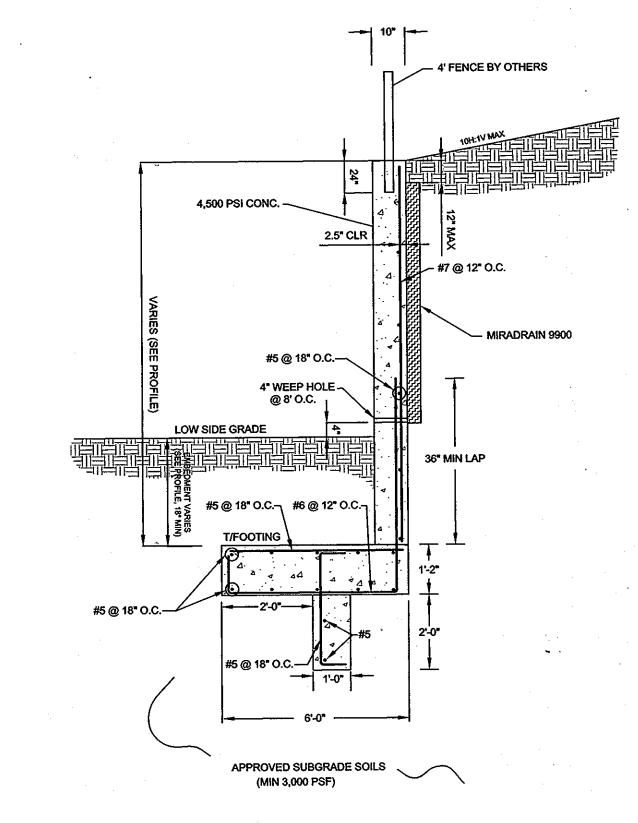
(Start of Wall to STA 0+07.8; STA 2+40.0 to End of Wall)

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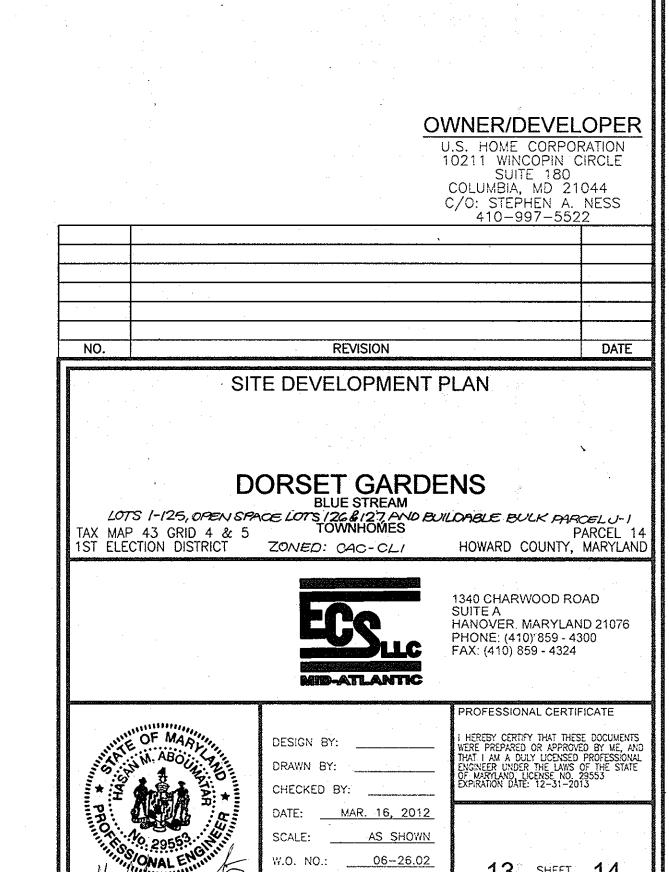


WALL 2 TYPICAL SECTION (STA 0+07.8 - STA 1+08.1) (STA 1+82.4 - STA 2+40.0)

NTS



WALL 2 TYPICAL SECTION
(STA 1+08.1 - STA 1+82.4)
NTS



13 SHEET OF

SDP-11-0

## RETAINING WALL SPECIFICATION GUIDELINES 1.01 Description A. Retaining walls must be constructed under the supervision of a Maryland Registered Professional Engineer. B. Work includes furnishing and installing concrete modular block retaining wall units to the lines and grades shown on the construction drawings and as specified herein. C. Work includes preparing foundation soil, furnishing and installing leveling pad, unit fill and reinforced backfill to the lines and grades shown on the construction drawings. D. Work includes furnishing and installing all related materials required for construction of the retaining wall as shown Load Bearing Concrete Masonry Units. Sampling and Testing Concrete Masonry Units. Sizes of Aggregate for Road and Bridge Construction. Laboratory Compaction Characteristics using Standard Effort. 1.03 Delivery, Storage and Handling Contractor shall check the materials upon delivery to assure that proper materials have been received. Contractor shall prevent excessive mud, wet cement, epoxy, and similar materials (which may affix themselves) from coming in contact with the materials. Contractor shall protect the materials from damage and exposure to sunlight. Damaged materials shall not be incorporated into the retaining wall structure and backfill. A. Owner will be responsible for soil testing and construction observations for quality control during earthwork and retaining wall construction operations. PART 2: MATERIALS A. Modular Wall Units - KEYSTONE or equivalent modular concrete facing and corner units, machine made from portland cement, water, and mineral aggregates. B. Structural Geogrid — a structural geogrid formed by a regular network of integrity connected tensile elements with apertures of sufficient size to allow interlocking with surrounding soil, rock, or earth and function primarily with apertures of sufficient size to allow managers and the interface of the modular concrete units and immediately behind the units to a width of at least 12 inches. 2. Reinforced Backfill — Compacted soil which is within the reinforced soil volume as shown on the plans. 2. Excavation Face — The interface between the reinforced backfill and the retained fill. During construction, measures shall be taken to avoid developing a shear plane at this interface. 3. Retained Backfill — On-site material located behind the reinforced zone of soil. A. Concrete segmental units shall conform to the requirements of NCMA TEK 2-4 and have a minimum 28-day compression strength of 4,000 psi. The units shall also pass 150 freeze thaw cycles in water with less than 1% weight loss for samples tested in accordance with ASIM C-1262.

 B. Wall Face Units for general wall construction shall be KEYSTONE Compac II Units or equivalent. Sculptured face or straight (flat) face may be used.
 C. Top of wall Cap Units shall be KEYSTONE Cap Units or equivalent with fiberglass connecting pins. 2.03 Fiberglass Connecting Pins

A. Connecting pins shall be 1/2" diameter thermoset isopathalic polyester resin-pultruded fiberglass reinforcement rods supplied by the unit manufacturer. 2.04 Construction Adhesive . Construction adhesive for top of wall cap blocks shall be KEYSTONE KapSeaTM or an approved equivalent construction adhesive. Material shall conform to ASTM 2339 and shall be supplied by the block unit supplier.

2.05 Drainage Pipe A. Continuous collection pipe shall consist of 4-inch diameter slotted or perforated PVC pipe (Schedule 40)
 B. Outlet (discharge) pipe shall consist of 4-inch diameter solid PVC pipe (Schedule 40).
 C. All pipe fittings shall be appropriate for the pipe size and schedules used.

A. Base Leveling and Pad Material 1. Material shall consist of crushed stone (GA S/B) as shown on the construction drawing. The leveling pad shall be, at a minimum, 6-inches thick. MSHA No. 57 Stone or peo gravel is not permitted.

1. Fill for units shall be free draining crushed stone or gravel, with a maximum aggregate size of 1/2" to 3/4" and no more than 5% passing the No. 50 sieve and conforming to ASTM D 448. Gradation of the unit fill shall be approved by the Geotechnical Engineer. Pea gravel shall not be used. MSHA No. 57 stone may be used. C. Reinforced Backfill

1. Material shall consist of soil classified as SM, SC or more granular soils per USCS with minimum soil parameters as indicated under design parameters. The backfill material shall contain no particles greater than 2.5 inches in diameter. The backfill material shall contain at least 30 percent by weight retained on the US Standard No. 200 sieve. Other backfill materials my be approved by the Geotechnical Engineer. Material may be imported or site excavated soils exhibiting a USCS designation of a lean clay (CL) or clayey sand (SC). The material shall contain no less than 40 percent by weight passing the US Standard No. 200 sieve and exhibit a plasticity index no less than 4 and no greater than 20. Other materials may be approved

 The contractor shall submit samples and material specifications of the proposed backfill soils (unit fill, pad material, reinforced backfill) to the Geotechnical Engineer for approval. Soil must meet or exceed the friction angle specified in design parameters.
 Direct shear testing is required for all soil samples used for Reinforced Backfill.

2.07 Structural Geogrid

A. The geogrid identified for the retaining wall consists of the following:

by the Geotechnical Engineer.

Mirafi 5XTc and 7XTc. . Other geogrid may be utilized provided the materials meet or exceed the minimum strength with similar or better strain characteristics of the Mirafi geogrid and are approved by the Geotechnical Engineer for use with soil backfill. The material shall be protected from sunlight and weather while stored on site in accordance with the

Additional Geogrids L = 15 ft

A. A non-woven geotextile shall be utilized as shown on the plans to provide a filter between the unit fill/drainage aggregate and the minforced backfill.

Manhole

Additional Geogrids L = 15 ft

The geotextile shall consist of a Mirafi 140N, or an approved equivalent.

Where geograds are located, the geotextile shall be placed as illustrated on the plans. At junctions and ends, the geotextile shall be overlapped at least 12 inches. The geotextile shall be placed so that intimate contact is made between the geotextile and the backfill material.

Ripped or otherwise damaged material shall not be used. The material shall be protected from sunlight and weather while stored on site in accordance with the manufacturer's recommendation.

A. Contractor shall excavate to the lines and grades shown on the construction drawings. Contractor shall be careful not to disturb embankment and foundation materials beyond lines shown.
B. All existing topsoil, rootmot and other soft or unsuitable materials shall, at a minimum, be removed from the footprint of the retained soil mass.
C. If groundwater is encountered during the excavation of the backslope, a backslope drainage system shall be utilized. The system shall tie into the internal wall drainage system to provide adequate release of any water which accumulates behind the reinforced zone. A. Foundation shall be excavated as required for leveling pad dimensions shown on the construction drawings, or as directed by the Geotechnical Engineer.
 B. The required bearing pressure beneath the footing of the wall must be verified in the field by a Geotechnical Engineer. Unauitable soils shall be removed and replaced with approved material.

Over—excavated areas shall be backfilled with approved, compacted backfill material or as approved by the A. Leveling pad materials shall be placed upon an approved foundation as shown on the construction drawings to a minimum thickness of 6 inches. Aggregate material shall be compacted to provide a dense, level surface on which to place the first course of modular units. Compaction shall be to at least 95% of the maximum dry density as determined by the Standard Proctor Compaction Test (ASTM D 698). Leveling pad shall be prepared and leveled to ensure complete contact of retaining wall unit with base. Unit Installation

A. The first course of concrete modular units shall be carefully placed on the base leveling pad. Each unit shall be checked for level (in both directions) and alignment.

B. Install fiberglass connecting pins and fill oil voids in and around the modular units with unit fill material. Tamp or rod unit fill to ensure that all voids are completely filled.

C. Sweep excess material from top of units and install the next course. Ensure that the units of each course are completely filled, backfilled and compacted prior to proceeding to next course.

Place each subsequent course, ensuring that pins protrude into adjoining courses a minimum of 1 inch.

Two pins are required per unit. Pull each unit forward to obtain the desired offset (as noted on the plans), away from the fill zone, locking against the pins in the previous course and backfill as the course is completed.

E. Repeat procedure to the extent of wall height. Wall construction shall not exceed 2 courses in height before reinforced backfill is placed.

F. Follow wall erection and unit fill placement closely with any other backfilling required. Compaction of all solls shall be to 95% of the maximum dry density as determined in accordance with ASTM D 698.

G. As appropriate where the wall changes elevation, units can be stepped with the grade or turned into the embankment with a convex return end. Provide appropriate buried units on compacted leveling pad in area of convex return end. A. The geogrid type and length (direction perpendicular to the wall face) shall conform to those indicated on the construction drawings. Geogrid shall be laid continuously at the proper elevations and orientation as shown on the construction drawings or as directed by the Geotechnical Engineer.
B. Correct orientation (roll direction) of the geogrid shall be verified by the Contractor.
C. The geogrid shall be connected to the modular wall units by placing the geogrid over fiberglass pins and laying the grid back to the fill side.
D. A filtering, non-woven geotextile shall be located between the drainage aggregate/unit fill and the reinforced backfill. The geotextile shall be folded back parallel, above and below the geogrid as necessary to ensure continuous and placement.

continuous grid placement.

E. The geogrid shall be pulled taut to set the geogrid against the fiberglass pins and to eliminate loose folds in the material. The fill surface shall be level. To tension the geogrid, backfill shall be placed over the geogrid from immediately behind the wall to the back end of the geogrid, backfill shall be placed over the geogrid F. No geogrid overlaps will be allowed in any length of geogrid perpendicular to the wall face except at corners or angled locations. The geogrid shall overlap rather than provide no coverage. A minimum of 4 inches of soil cover is required between over lapping layers of geogrid.

A. Provide continuous 4-inch slotted or perforated PVC pipe behind the wall, no greater than 4 inches above finished grade at the bottom of the wall.
B. Provide 4-inch solid PVC pipe outlets every 10 feet along the wall, and at each end of the wall.

A. Backfill material shall be placed in 8 inch loose lifts and compacted to at least 95% of the maximum dry density as determined by ASTM D 698. The in-place moisture content shall be in the range of at the optimum moisture content to 2 percentage points higher than the optimum moisture content, as determined in accordance with ASTM D 698. B. Backfill shall be placed, spread and compacted in such a manner that minimizes the development of slack or loss of pretension of the geogrid. Backfill shall be placed in horizontal layers. The excavation face shall be stepped or notched to provide compaction of backfill on a level surface and to increase the interlock between the retained soils and the reinforced backfill.

C. Only hand-operated compaction equipment shall be soils and the reinforced backfill.

Only hand—operated compaction equipment shall be allowed within 5 feet of the back surface of the KEYSTONE or equivalent units.

Backfill shall be placed from immediately behind the wall towards the excavation face/retained soils and compacted

 Backfill shall be placed from immediately behind the wall towards the excavation face/retained soils and compacted to the specifications presented herein with appropriate compaction equipment.
 Tracked construction equipment shall not be operated directly on the geogrid. A minimum backfill thickness of 6 inches is required prior to operation of tracked vehicles over the geogrid. Turning of tracked vehicles shall not be permitted overtop the geogrid.
 Rubber—tired equipment may pass over the geogrid reinforcement at slow speeds (less than 10 mph). Avoid sudden braking and sharp turning.
 The suitability of the fill material must be confirmed by a Geotechnical Engineer. H. The upper 8 inches of wall backfill shall consist of impervious soil, compacted to at least 95% of the maximum dry density as determined by ASTM D 698. The in-place moisture content shall be in the range of at the optimum moisture content to 2 percentage points higher than the optimum moisture content, as determined in accordance with ASTM D 698.

A. Provide permanent mechanical connection to wall units with KEYSTONE KapSeaITM or equivalent construction adhesive. Apply adhesive to top surface of lower unit and place cap unit atop adhesive. B. Place Cap Units over projecting pins from the units below. Pull forward to setback position.

DESIGN PARAMETERS

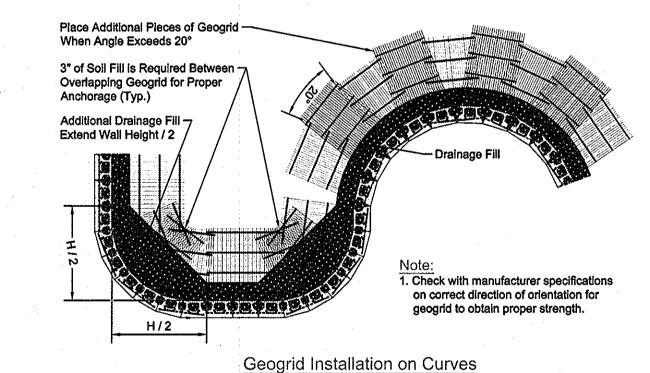
C. Backfill and compact to finished grade.

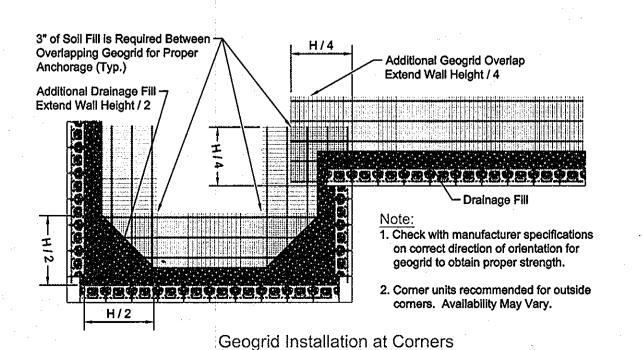
3.02 Foundation Preparation

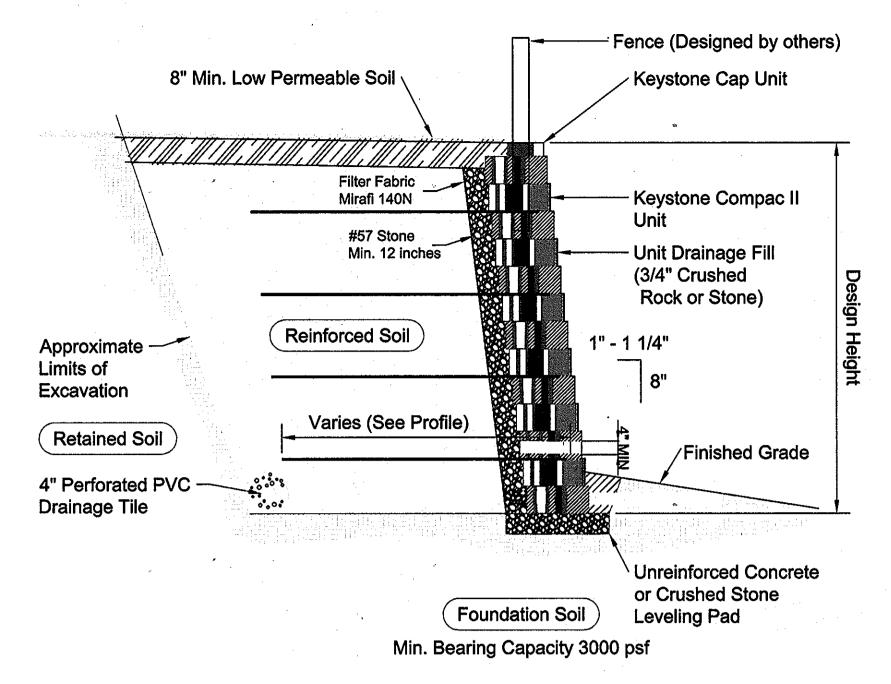
3.03 Base Leveling Pad

3.04 Unit Installation

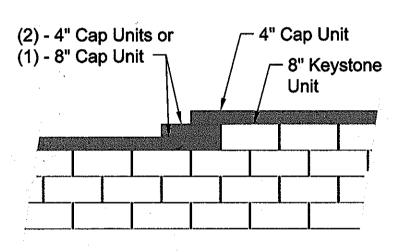
Characteristics: Soil Parameters: Configuration: Soil Type Varies (10H:1V maximum) Varies (2.5H:1V maximum) Backslope Angle: Retained soils Toe Slope Angle:





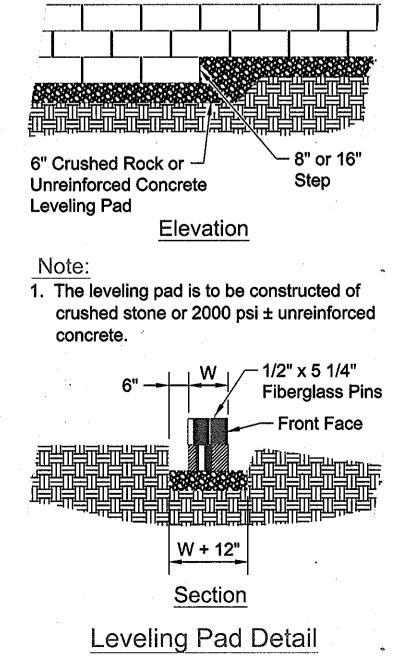


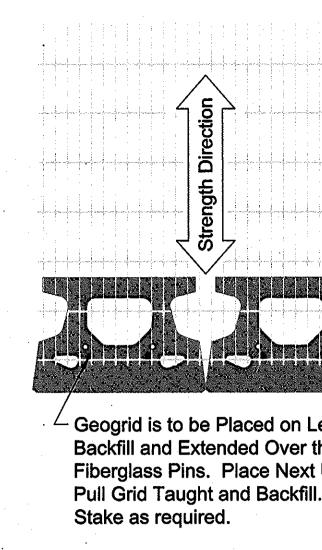
Typical Reinforced Wall Section Standard Unit - 1" Setback



1. Secure all cap units with Keystone Kapseal or equal.

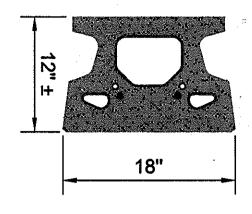
Top of Wall Steps





Geogrid is to be Placed on Level Backfill and Extended Over the Fiberglass Pins. Place Next Unit

Grid & Pin Connection

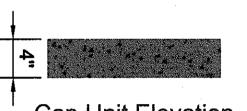


Compac II Elevation

Compac II Plan

Compac II Unit \* Dimensions May Vary by Region

Cap Unit Elevation

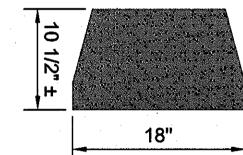


Cap Unit Elevation



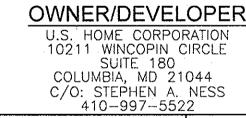
Cap Unit Plan

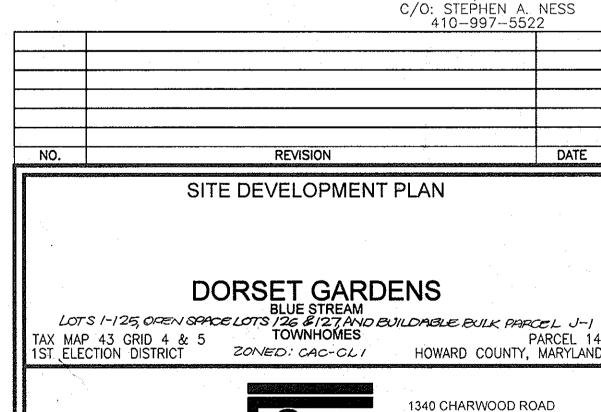
Universal Cap Unit Option \* Dimensions & Availability Will Vary by Region



Cap Unit Plan

Straight Split Cap Unit Option \* Dimensions & Availability Will Vary by Region



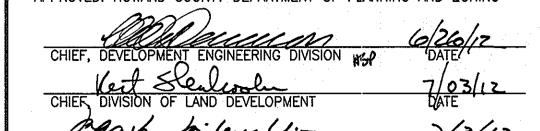




SUITE A HANOVER, MARYLAND 21076 PHONE: (410) 859 - 4300 FAX: (410) 859 - 4324 MID-ATLANTIC ROFESSIONAL CERTIFICATE

CHECKED BY: DATE: <u>MAR. 16, 2012</u> W.O. NO.: 06-26.02

14 SHEET 14



Park to leng 4:-

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

Detail "B" (NTS)

Additional Geogrid