

FOLLOWING INITIAL SOIL DISTURBANCES OR RE-DISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: A. THREE (3) CALENDAR DAYS AS TO THE SURFACE OF ALL PERIMETER DIKES, SWALES, DITCHES, PERIMETER SLOPES, AND ALL SLOPES STEEPER THAN 3 HORIZONTAL TO 1 VERTICAL (3:1); AND B. SEVEN (7) CALENDAR DAYS TO ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE NOT UNDER ACTIVE GRADING. ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING

SIGNS BE POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOI., 1. CHAPTER 7. HOWARD COUNTY DESIGN MANUAL, STORM DRAINAGE ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDING, SOD, TEMPORARY SEEDING, AND MULCHING (SEC. G). TEMPORARY STABILIZATION WITH MULCH ALONE SHALL BE DONE WHEN RECOMMENDED SEEDING DATES DO NOT ALLOW FOR PROPER GERMINATION AND ESTABLISHMENT OF GRASSES.

6. ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE MAINTAINED IN OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR

CONTROL INSPECTOR.	COUNTY SEDIMENT
SITE ANALYSIS:	
TOTAL AREA	14.03 ACRES
AREA DISTURBED	8.16 ACRES
AREA TO BE ROOFED OR PAVED	6.91 ACRES
AREA TO BE VEGETATIVELY STABILIZED	1.25 ACRES
TOTAL CUT	4,501 CY*
TOTAL FILL	25,780 CY*
OFFSITE WASTE/BORROW LOCATION	*

ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF UTILITIES MUST BE REPAIRED ON THE SAME

ADDITIONAL SEDIMENT CONTROLS MUST BE PROVIDED, IF DEEMED NECESSARY BY THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.). ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING. OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL BY THE INSPECTION AGENCY

TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGTHS OR THAT WHICH SHALL BE BACK-FILLED AND STABILIZED WITHIN ONE WORKING DAY, WHICHEVER IS SHORTER. * EARTHWORK IS AN ESTIMATE ONLY; CONTRACTOR SHALL VERIFY QUANTITIES TO HIS OWN SATISFACTION. ** TO BE DETERMINED BY CONTRACTOR, WITH PRE-APPROVAL OF THE SEDIMENT CONTROL INSPECTOR WITH AN APPROVED AND ACTIVE GRADING

PLACE EXCAVATED MATERIAL ON UPHILL SIDE OF TRENCH. BACKFILL AND STABILIZE TRENCH AT THE END OF EACH DAY. WITHIN ROAD BED BACKFILL TOP FOOT OF TRENCH WITH GRAVEL

MAINTAIN VEHICLE ACCESS ALONG DRIVEWAY AT ALL TIMES. PEROVE INNING BOAKS OF THE WARD COUNTY

Manuell

CHIEF, DIVISION OF LAND DEVELOPMENT AME

CHIEF, DEVELOPMENT ENGINEERING DIVISION

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

B-4-5 STANDARDS AND SPECIFICATIONS FOR PERMANENT STABILIZATION B-4-2 STANDARDS AND SPECIFICATIONS FOR SOIL PREPARATION.

-----SF------

JE IN MIN. FENCE POST LENGTH DRIVEN MIN. 16 IN INTO GROUND

8 IN MIN. DEPTH

16 IN MIN, HEIGHT OF WOVEN SUIT FILM GEOTEXTILE

DETAIL E-1 SILT FENCE

USE WOOD POSTS 1% X 1% \pm % INCH (MINIMUM) SQUARE CUT OF SOUND QUALITY HARDWOOD. AS AN ALTERNATIVE TO WOODEN POST USE STANDARD "I" OR "U" SECTION STEEL POSTS WEIGHING NO LESS THAN 1 POUND PER LINEAR FOOT.

. USE 36 INCH MINIMUM POSTS DRIVEN 16 INCH MINIMUM INTO GROUND NO MORE THAN 6 FEET APART

USE WOVEN SLIT FILM GEOTEXTILE AS SPECIFIED IN SECTION H-1 MATERIALS AND FASTEN GEOTEXTILE SECURELY TO UPSLOPE SIDE OF FENCE POSTS WITH WIRE TIES OR STAPLES AT TOP AND MID-SECTION.

EMBED GEOTEXTILE A MINIMUM OF 8 INCHES VERTICALLY INTO THE GROUND, BACKFILL AND COMPACT THE SOIL ON BOTH SIDES OF FABRIC.

EXTEND BOTH ENDS OF THE SILT FENCE A MINIMUM OF FIVE HORIZONTAL FEET UPSLOPE AT 45 DEGREES TO THE MAIN FENCE ALIGNMENT TO PREVENT RUNOFF FROM GOING AROUND THE ENDS OF THE SILT FENCE.

MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL FROSION AND SEDIMENT CONTROL

WHERE TWO SECTIONS OF GEOTEXTILE ADJOIN: OVERLAP, TWIST, AND STAPLE TO POST IN ACCORDANCE WITH THIS DETAIL.

CONSTRUCTION SPECIFICATIONS

<u>PURPOSE</u> TO USE LONG-LIVED PERENNIAL GRASSES AND LEGUMES TO ESTABLISH PERMANENT GROUND COVER ON

CONDITIONS WHERE PRACTICE APPLIES EXPOSED SOILS WHERE GROUND COVER IS NEEDED FOR 6 MONTHS OR MORE.

<u>DEFINITION</u> TO STABILIZE DISTURBED SOILS WITH PERMANENT VEGETATION

CRITERIA A. SEED MIXTURES GENERAL USE

DETAIL E-1 SILT FENCE

WOVEN SLIT FILM-

ELEVATION

CROSS SECTION

JOINING TWO ADJACENT SILT FENCE SECTIONS (TOP VIEW

A.SELECT ONE OR MORE OF THE SPECIES OR MIXTURES LISTED IN TABLE 8.3 FOR THE APPROPRIATE PLANT HARDINESS ZONE (FROM FIGURE 8.3) AND BASED ON THE SITE CONDITION OR PURPOSE FOUND ON TABLE 8.2. ENTER SELECTED MIXTURE(S), APPLICATION RATES, AND SEEDING DATES IN THE PERMANENT SEEDING SUMMARY. THE SUMMARY IS TO BE PLACED ON THE PLAN. 3. ADDITIONAL PLANTING SPECIFICATIONS FOR EXCEPTIONAL SITES SUCH AS SHORELINES, STREAM BANKS, OR DUNES OR FOR SPECIAL PURPOSES SUCH AS WILDLIFE OR AESTHETIC TREATMENT MAY BE FOUND IN USDA-NRCS TECHNICAL FIELD OFFICE GUIDE, SECTION 342 - CRITICAL AREA PLANTING. FOR SITES HAVING DISTURBED AREA OVER 5 ACRES, USE AND SHOW THE RATES RECOMMENDED BY THE SOIL TESTING AGENCY. D. FOR AREAS RECEIVING LOW MAINTENANCE, APPLY UREA FORM FERTILIZER (46-0-0) AT 3-1/2 POUNDS PER 1000 SQUARE FEET (150 POUNDS PER ACRE) AT THE TIME OF SEEDING IN ADDITION TO THE

SOIL AMENDMENTS SHOWN IN THE PERMANENT SEEDING SUMMARY. TURFGRASS MIXTURES . AREAS WHERE TURFGRASS MAY BE DESIRED INCLUDE LAWNS, PARKS, PLAYGROUNDS, AND COMMERCIAL SITES WHICH WILL RECEIVE A MEDIUM TO HIGH LEVEL OF MAINTENANCE. 3. SELECT ONE OR MORE OF THE SPECIES OR MIXTURES LISTED BELOW BASED ON THE SITE CONDITIONS OR PURPOSE. ENTER SELECTED MIXTURE(S), APPLICATION RATES, AND SEEDING DATES IN THE PERMANENT SEEDING SUMMARY THE SUMMARY IS TO BE PLACED ON THE PLAN. I. KENTUCKY BLUEGRASS: FULL SUN MIXTURE: FOR USE IN AREAS THAT RECEIVE INTENSIVE MANAGEMENT. IRRIGATION REQUIRED IN THE AREAS OF CENTRAL MARYLAND AND EASTERN SHORE RECOMMENDED CERTIFIED KENTUCKY BLUEGRASS CULTIVARS SEEDING RATE: 1.5 TO 2.0 POUNDS PER 1000 SQUARE FEET. CHOOSE A MINIMUM OF THREE KENTUCKY BLUEGRASS CULTIVARS WITH EACH PANCING FROM 10 TO 35PERCENT OF THE TOTAL MIXTURE BY WEIGHT IL KENTLICKY BLUFGRASS/PERENNIAL RYF: FULL SUN MIXTURE: FOR USE IN FULL SUN AREAS WHERE

RAPID ESTABLISHMENT IS NECESSARY AND WHEN TURF WILL RECEIVE MEDIUM TO INTENSIVE MANAGEMENT. CERTIFIED PERENNIAL RYEGRASS CULTIVARS/CERTIFIED KENTUCKY BLUEGRASS SEEDING RATE: 2 POUNDS MIXTURE PER 1000 SQUARE FEET. CHOOSE A MINIMUM OF THREE KENTUCKY BLUEGRASS CULTIVARS WITH EACH RANGING FROM 10 TO 35 PERCENT OF THE TOTAL MIXTURE BY III. TALL FESCUE/KENTUCKY BLUEGRASS: FULL SUN MIXTURE: FOR USE IN DROUGHT PRONE AREAS AND/OR FOR AREAS RECEIVING LOW TO MEDIUM MANAGEMENT IN FULL SUN TO MEDIUM SHADE. RECOMMENDED MIXTURE INCLUDES; CERTIFIED TALL FESCUE CULTIVARS 95 TO 100 PERCENT CERTIFIED KENTUCKY BLUEGRASS CULTIVARS 0 TO 5 PERCENT. SEEDING RATE: 5 TO 8 POUNDS PER 1000 SOLIARE FEET ONE OR MORE CULTIVARS MAY BE BLENDED. IV KENTUCKY BLUFGRASS/FINE FESCUE: SHADE MIXTURE: FOR USE IN AREAS WITH SHADE IN BLUEGRASS LAWNS. FOR ESTABLISHMENT IN HIGH QUALITY, INTENSIVELY MANAGED TURF AREA. MIXTURE

FFSCUE AND 60 TO 70 PERCENT, SEEDING RATE: 11/2 TO 3 POUNDS PER 1000 SQUARE FEET SELECT TURFGRASS VARIETIES FROM THOSE LISTED IN THE MOST CURRENT UNIVERSITY OF MARYLAND PUBLICATION, AGRONOMY MEMO #77, "TURFGRASS CULTIVAR RECOMMENDATIONS FOR CHOOSE CERTIFIED MATERIAL. CERTIFIED MATERIAL IS THE BEST GUARANTEE OF CULTIVAR PURITY.
THE CERTIFICATION PROGRAM OF THE MARYLAND DEPARTMENT OF AGRICULTURE, TURF AND SEED

LUDES CEPTIFIED KENTUCKY BLUFCRASS CUITIVARS 30 TO 40 PERCENT AND CERTIFIED FINE

SECTION, PROVIDES A RELIABLE MEANS OF CONSUMER PROTECTION AND ASSURES A PURE GENETIC LINE. C. IDEAL TIMES OF SEEDING FOR TURF GRASS MIXTURES WESTEM MD: MARCH 15 TO JUNE 1, AUGUST 1 TO OCTOBER 1 (HARDINESS ZONES: 5B, 6A) CENTRAL MD: MARCH 1 TO MAY 15, AUGUST 15 TO OCTOBER 15 (HARDINESS ZONE: 6B) <u>SOUTHERN MD, EASTERN SHORE</u>: MARCH 1 TO MAY 15, AUGUST 15 TO OCTOBER 15 (HARDINESS

D. TILL AREAS TO RECEIVE SEED BY DISKING OR OTHER APPROVED METHODS TO A DEPTH OF 2 TO 4 INCHES, LEVEL AND RAKE THE AREAS TO PREPARE A PROPER SEEDBED. REMOVE STONES AND DEBRIS OVER 11/2 INCHES IN DIAMETER. THE RESULTING SEEDBED MUST BE IN SUCH CONDITION THAT FUTURE MOWING OF GRASSES WILL POSE NO DIFFICULT E. IF SOIL MOISTURE IS DEFICIENT, SUPPLY NEW SEEDINGS WITH ADEQUATE WATER FOR PLANT GROWTH (1/2 TO 1 INCH EVERY 3 TO 4 DAYS DEPENDING ON SOIL TEXTURE) UNTIL THEY ARE FIRMLY ESTABLISHED. THIS IS ESPECIALLY TRUE WHEN SEEDINGS ARE MADE LATE IN THE PLANTING SEASON, IN ABNORMALLY DRY OR HOT SEASONS, OR ON ADVERSE SITES.

PERMANENT SEEDING SUMMARY

HARDINESS ZONE (FROM FIGURE B.3): ZONE 6b SEED MIXTURE (FROM TABLE B.3): 8					LIME RATÉ			
NO	SPECIES	APPLICATION RATE (LB/AC)	SEEDING DATES	SEEDING DEPTHS	N	P ₂ 0 ₅	к ₂ 0	
1	TALL FESCUE (LOLIUM ARUNDINACEAUM)	T.F. 60 LB / AC K.B. 40 LB / AC	MAR 1 TO MAY 15 AUG 15 TO OCT 15	1/4-1/2 IN.	45 LB/AC (1 LB PER 1000 SF)		90 LB/AC (2 LB PER 1000 SF)	2 TONS/AC (90 LB PER 1000 SF)

B. SOD: TO PROVIDE QUICK COVER ON DISTURBED AREAS (2:1 GRADE OR FLATTER). 1. GENERAL SPECIFICATIONS

BY THE DEVELOPER:

A. CLASS OF TURFGRASS SOD MUST BE MARYLAND STATE CERTIFIED. SOD LABELS MUST BE MADI AVAILABLE TO THE JOB FOREMAN AND INSPECTOR. B. SOD MUST BE MACHINE CUT AT A UNIFORM SOIL THICKNESS OF 3/4 INCH, PLUS OR MINUS 1/4 INCH, AT THE TIME OF CUTTING. MEASUREMENT FOR THICKNESS MUST EXCLUDE TOP GROWTH AND THATCH. BROKEN PADS AND TOM OR UNEVEN ENDS WILL NOT BE ACCEPTABLE.
C. STANDARD SIZE SECTIONS OF SOD MUST BE STRONG ENOUGH TO SUPPORT THEIR OWN WEIGHT AND RETAIN THEIR SIZE AND SHAPE WHEN SUSPENDED VERTICALLY WITH A FIRM GRASP ON THE UPPER 10 PERCENT OF THE SECTION. D. SOD MUST NOT BE HARVESTED OR TRANSPLANTED WHEN MOISTURE CONTENT (EXCESSIVELY DRY OR WFT) MAY ADVERSELY AFFECT ITS SURVIVAL. E. SOD MUST BE HARVESTED, DELIVERED, AND INSTALLED WITHIN A PERIOD OF 36 HOURS. SOD NOT TRANSPLANTED WITHIN THIS PERIOD MUST BE APPROVED BY AN AGRONOMIST OR SOIL SCIENTIST PRIOR

TO ITS INSTALLATION. 2. SOD INSTALLATION A. DURING PERIODS OF EXCESSIVELY HIGH TEMPERATURE OR IN AREAS HAVING DRY SUBSOIL, LIGHTLY IRRIGATE THE SUBSOIL IMMEDIATELY PRIOR TO LAYING THE SOD. B. LAY THE FIRST ROW OF SOD IN A STRAIGHT LINE WITH SUBSEQUENT ROWS PLACED PARALLEL TO IT AND TIGHTLY WEDGED AGAINST EACH OTHER. STAGGER LATERAL JOINTS TO PROMOTE MORE UNIFORM GROWTH AND STRENGTH. ENSURE THAT SOD IS NOT STRETCHED OR OVERLAPPED AND THAT ALL JOINTS ARE BUTTED TIGHT IN ORDER TO PREVENT VOIDS WHICH WOULD CAUSE AIR DRYING OF THE ROOTS. C. WHEREVER POSSIBLE, LAY SOD WITH THE LONG EDGES PARALLEL TO THE CONTOUR AND WITH STAGGERING JOINTS. ROLL AND TAMP, PEG OR OTHERWISE SECURE THE SOD TO PREVENT SLIPPAGE ON SLOPES. ENSURE SOLID CONTACT EXISTS BETWEEN SOD ROOTS AND THE UNDERLYING SOIL SURFACE D. WATER THE SOD IMMEDIATELY FOLLOWING ROLLING AND TAMPING UNTIL THE UNDERSIDE OF THE NEW SOD PAD AND SOIL SURFACE BELOW THE SOD ARE THOROUGHLY WET. COMPLETE THE OPERATIONS OF LAYING, TAMPING AND IRRIGATING FOR ANY PIECE OF SOD WITHIN EIGHT HOURS

A. IN THE ABSENCE OF ADEQUATE RAINFALL, WATER DAILY DURING THE FIRST WEEK OR AS OFTEN AND SUFFICIENTLY AS NECESSARY TO MAINTAIN MOIST SOIL TO A DEPTH OF 4 INCHES. WATER SOD DURING THE HEAT OF THE DAY TO PREVENT WILTING. B. AFTER THE FIRST WEEK, SOD WATERING IS REQUIRED AS NECESSARY TO MAINTAIN ADEQUATE MOISTURE C. DO NOT MOW UNTIL THE SOD IS FIRMLY ROOTED. NO MORE THAN 1/3 OF THE GRASS LEAF MUST BE REMOVED BY THE INITIAL CUTTING OR SUBSEQUENT CUTTINGS. MAINTAIN A GRASS HEIGHT OF AT LEAST 3 INCHES UNLESS OTHERWISE SPECIFIED.

"I/WE CERTIFY THAT ALL DEVELOPEMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN FOR SEDIMENT AND EROSION

CONTROL, AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL: HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. ALSO AUTHORIZE PERIODIC ON—SITE INSPECTION, BY, THE HOWARD SOIL CONSERVATION DISTRICT."

TOPSOILING AND SOIL AMENDMENTS

DETAIL E-9-3 CURB INLET PROTECTION

6 FT MAX. SPACING OF 2 IN x 4 IN SPACERS

L2 IN x 4 IN WEIR

USE NONWOVEN GEOTEXTILE AS SPECIFIED IN SECTION H-1 MATERIALS.

NAIL THE 2x4 WEIR TO 9 INCH LONG VERTICAL SPACERS (MAXIMUM 6 FEET APART).

. PLACE A CONTINUOUS PIECE OF NONWOVEN GEOTEXTILE OF THE SAME DIMENSIONS AS THE HARDWARE CLOTH OVER THE HARDWARE CLOTH AND SECURELY ATTACH TO THE 2x4 WEIR.

PLACE THE ASSEMBLY AGAINST THE INLET THROAT AND NAIL TO 2x4 ANCHORS (MINIMUM 2 FEE LENGTH). EXTEND THE ANCHORS ACROSS THE INLET TOP AND HOLD IN PLACE BY SANDBAGS OF

B. FORM THE HARDWAPE CLOTH AND THE GEOTEXTILE TO THE CONCRETE GUTTER AND FACE OF CURB T SPAN THE INLET OPENING. COVER THE HARDWARE CLOTH AND GEOTEXTILE WITH CLEAN % TO 1% INCI STONE OR EQUIVALENT RECYCLED CONCRETE.

. AT NON-SUMP LOCATIONS, INSTALL A TEMPORARY SANDBAG OR ASPHALT BERM TO PREVENT INLET BYPASS.

INSTALL END SPACERS A MINIMUM OF 1 FOOT BEYOND THE ENDS OF THE THROAT OPENING

LEDGE OF GUTTER PAN

ISOMETRIC

USE NOMINAL 2 INCH x 4 INCH LUMBER

OTHER APPROVED ANCHORING METHOD

ONSTRUCTION SPECIFICATIONS

MAXIMUM DRAINAGE AREA = X ACRE

SECTION A-A

2 IN x 4 IN WEIR-

% TO 1% STONE -

- 2 FT MIN, LENGTI OF 2 IN × 4 IN

SANDBAG OR OTHER APPROVED ANCHORING METH

PROCESS OF PREPARING THE SOILS TO SUSTAIN ADEQUATE VEGETATIVE STABILIZATION.

<u>PURPOSE</u>
TO PROVIDE A SUITABLE SOIL MEDIUM FOR VEGETATIVE GROWTH. CONDITIONS WHERE PRACTICE APPLIES
WHERE VEGETATIVE STABILIZATION IS TO BE ESTABLISHED.

NECESSARY ON NEWLY DISTURBED AREAS.

ACCEPTABLE.

____SF____

TEMPORARY STABILIZATION A. SEEDBED PREPARATION CONSISTS OF LOOSENING SOIL TO A DEPTH OF 3 TO 5 INCHES BY MEANS OF SUITABLE AGRICULTURAL OR CONSTRUCTION EQUIPMENT, SUCH AS DISCHARROWS OR CHISEL PLOWS OR RIPPERS MOUNTED ON CONSTRUCTION FOUIPMENT. AFTER THE SOIL IS LOOSENED, IT MUST NOT BE ROLLED OR DRAGGED SMOOTH BUT LEFT IN THE ROUGHENED CONDITION. SLOPES 3:1 OR FLATTER ARE TO BE TRACKED WITH RIDGES RUNNING PARALLEL TO THE CONTOUR OF THE SLOPE. 3. APPLY FERTILIZER AND LIME AS PRESCRIBED ON THE PLANS. INCORPORATE LIME AND FERTILIZER INTO THE TOP 3 TO 5 INCHES OF SOIL BY DISKING OR OTHER SUITABLE MEANS.

A. A SOIL TEST IS REQUIRED FOR ANY EARTH DISTURBANCE OF 5 ACRES OR MORE. THE MINIMUM SOIL CONDITIONS REQUIRED FOR PERMANENT VEGETATIVE ESTABLISHMENT ARE . SOIL PH BETWEEN 6.0 AND 7.0. IL SOLUBLE SALTS LESS THAN 500 PARTS PER MILLION (PPM) III. SOIL CONTAINS LESS THAN 40 PERCENT CLAY BUT ENOUGH FINE GRAINED MATERIAL (GREATER THAN 30 PERCENT SILT PLUS CLAY) TO PROVIDE THE CAPACITY TO HOLD A MODERATE AMOUNT OF MOISTURE. AN EXCEPTION: IF LOVEGRASS WILL BE PLANTED, THEN A SANDY SOIL (LESS THAN 30 PERCENT SILT PLUS CLAY) WOULD BE

IV. SOIL CONTAINS 1.5 PERCENT MINIMUM ORGANIC MATTER BY WEIGHT. 1. SOIL CONTAINS SUFFICIENT PORE SPACE TO PERMIT ADEQUATE ROOT PENETRATION. B. APPLICATION OF AMENDMENTS OR TOPSOIL IS REQUIRED IF ON-SITE SOILS DO NOT MEET GRADED AREAS MUST BE MAINTAINED IN A TRUE AND EVEN GRADE AS SPECIFIED ON THE APPROVED PLAN, THEN SCARIFIED OR OTHERWISE LOOSENED TO A DEPTH OF 3 TO 5 D. APPLY SOIL AMENDMENTS AS SPECIFIED ON THE APPROVED PLAN OR AS INDICATED BY THE RESULTS OF A SOIL TEST.
E. MIX SOIL AMENDMENTS INTO THE TOP 3 TO 5 INCHES OF SOIL BY DISKING OR OTHER SUITABLE MEANS. RAKE LAWN AREAS TO SMOOTH THE SURFACE, REMOVE LARGE OBJECTS LIKE STONES AND BRANCHES, AND READY THE AREA FOR SEED APPLICATION. LOOSEN SURFACE SOIL BY DRAGGING WITH A HEAVY CHAIN OR OTHER EQUIPMENT TO ROUGHEN THE SURFACE WHERE SITE CONDITIONS WILL NOT PERMIT NORMAL SEEDBED PREPARATION. TRACK SLOPES 3:1 OR FLATTER WITH TRACKED FOUIPMENT LEAVING THE SOIL IN AN IRREGULAR CONDITION WITH RIDGES RUNNING PARALLEL TO THE CONTOUR OF THE SLOPE. LEAVE THE TOP 1 TO 3 INCHES OF SOIL LOOSE AND FRIABLE. SEEDBED LOOSENING MAY BE

TOPSOIL IS PLACED OVER PREPARED SUBSOIL PRIOR TO ESTABLISHMENT OF PERMANENT VEGETATION. THE PURPOSE IS TO PROVIDE A SUITABLE SOIL MEDIUM FOR VEGETATIVE GROWTH. SOILS OF CONCEM HAVE LOW MOISTURE CONTENT, LOW NUTRIENT LEVELS, LOW PH, MATERIALS TOXIC TO PLANTS, AND/OR UNACCEPTABLE SOIL GRADATION TOPSOIL SALVAGED FROM AN EXISTING SITE MAY BE USED PROVIDED IT MEETS THE STANDARDS AS SET FORTH IN THESE SPECIFICATIONS. TYPICALLY, THE DEPTH OF TOPSOIL TO BE SALVAGED FOR A GIVEN SOIL TYPE CAN BE FOUND IN THE REPRESENTATIVE SOIL PROFILE SECTION IN L SURVEY PUBLISHED BY USDA-NRCS 3. TOPSOILING IS LIMITED TO AREAS HAVING 2:1 OR FLATTER SLOPES WHERE:

A. THE TEXTURE OF THE EXPOSED SUBSOIL/PARENT MATERIAL IS NOT ADEQUATE TO PRODUCE B THE SOIL MATERIAL IS SO SHALLOW THAT THE ROOTING ZONE IS NOT DEEP ENOUGH TO SUPPORT PLANTS OR FLIRNISH CONTINUING SUPPLIES OF MOISTURE AND PLANT NUTRIENTS. THE ORIGINAL SOIL TO BE VEGETATED CONTAINS MATERIAL TOXIC TO PLANT GROWTH.

THE SOIL IS SO ACIDIC THAT TREATMENT WITH LIMESTONE IS NOT FEASIBLE. AREAS HAVING SLOPES STEEPER THAN 2:1 REQUIRE SPECIAL CONSIDERATION AND DESIGN. TOPSOIL SPECIFICATIONS: SOIL TO BE USED AS TOPSOIL MUST MEET THE FOLLOWING CRITERIA: A. TOPSOIL MUST BE A LOAM, SANDY LOAM, CLAY LOAM, SILT LOAM, SANDY CLAY LOAM, OR LOAMY SAND. OTHER SOILS MAY BE USED IF RECOMMENDED BY AN AGRONOMIST OR SOI SCIENTIST AND APPROVED BY THE APPROPRIATE APPROVAL AUTHORITY. TOPSOIL MUST NO BE A MIXTURE OF CONTRASTING TEXTURED SUBSOILS AND MUST CONTAIN LESS THAN 5

PERCENT BY VOLUME OF CINDERS, STONES, SLAG, COARSE FRAGMENTS, GRAVEL, STICKS, ROOTS, TRASH, OR OTHER MATERIALS LARGER THAN 1½ INCHES IN DIAMETER.
B. TOPSOIL MUST BE FREE OF NOXIOUS PLANTS OR PLANT PARTS SUCH AS BERMUDA GRASS, QUACK GRASS, JOHNSON GRASS, NUT SEDGE, POISON IVY, THISTLE, OR OTHERS AS

. TOPSOIL SUBSTITUTES OR AMENDMENTS, AS RECOMMENDED BY A QUALIFIED AGRONOMIST OR SOIL SCIENTIST AND APPROVED BY THE APPROPRIATE APPROVAL AUTHORITY, MAY BE USED IN LIEU OF NATURAL TOPSOIL. A. EROSION AND SEDIMENT CONTROL PRACTICES MUST BE MAINTAINED WHEN APPLYING B. UNIFORMLY DISTRIBUTE TOPSOIL IN A 5 TO 8 INCH LAYER AND LIGHTLY COMPACT TO A MINIMUM THICKNESS OF 4 INCHES. SPREADING IS TO BE PERFORMED IN SUCH A MANNER THAT SODDING OR SEEDING CAN PROCEED WITH A MINIMUM OF ADDITIONAL SOIL

PREPARATION AND TILLAGE. ANY IRREGULARITIES IN THE SURFACE RESULTING FROM TOPSOILING OR OTHER OPERATIONS MUST BE CORRECTED IN ORDER TO PREVENT THE FORMATION OF DEPRESSIONS OR WATER POCKETS. C. TOPSOIL MUST NOT BE PLACED IF THE TOPSOIL OR SUBSOIL IS IN A FROZEN OR MUDDY CONDITION, WHEN THE SUBSOIL IS EXCESSIVELY WET OR IN A CONDITION THAT MAY OTHERWISE BE DETRIMENTAL TO PROPER GRADING AND SEEDBED PREPARATION.

SOIL AMENDMENTS (FERTILIZER AND LIME SPECIFICATIONS)

1. SOIL TESTS MUST BE PERFORMED TO DETERMINE THE EXACT RATIOS AND APPLICATION RATES FOR BOTH LIME AND FERTILIZER ON SITES HAVING DISTURBED AREAS OF 5 ACRES OR MORE. SOIL ANALYSIS MAY BE PERFORMED BY A RECOGNIZED PRIVATE OR COMMERCIAL LABORATORY. SOIL SAMPLES TAKEN FOR ENGINEERING PURPOSES MAY ALSO BE USED FOR CHEMICAL FERTILIZERS MUST BE UNIFORM IN COMPOSITION, FREE FLOWING AND SUITABLE FOR ACCURATE APPLICATION BY APPROPRIATE EQUIPMENT. MANURE MAY BE SUBSTITUTED FOR FERTILIZER WITH PRIOR APPROVAL FROM THE APPROPRIATE APPROVAL AUTHORITY. FERTILIZERS MUST ALL B DELIVERED TO THE SITE FULLY LABELED ACCORDING TO THE APPLICABLE LAWS AND MUST BEAR THE NAME, TRADE NAME OR TRADEMARK AND WARRANTY OF THE PRODUCER.

3. LIME MATERIALS MUST BE GROUND LIMESTONE (HYDRATED OR BURNT LIME MAY BE SUBSTITUTED EXCEPT WHEN HYDROSEEDING) WHICH CONTAINS AT LEAST 50 PERCENT TOTAL OXIDES (CALCIUM OXIDE PLUS MAGNESIUM OXIDE). LIMESTONE MUST BE GROUND TO SUCH FINENESS THAT AT LEAST 50 PERCENT WILL PASS THROUGH A #100 MESH SIEVE AND 98 TO 100 PERCENT WILL PASS THROUGH A #20 MESH SIEVE. 4. LIME AND FERTILIZER ARE TO BE EVENLY DISTRIBUTED AND INCORPORATED INTO THE TOP 3 TO 5 INCHES OF SOIL BY DISKING OR OTHER SUITABLE MEANS. . WHERE THE SUBSOIL IS EITHER HIGHLY ACIDIC OR COMPOSED OF HEAVY CLAYS, SPREAD GROUND LIMESTONE AT THE RATE OF 4 TO 8 TONS/ACRE (200-400 POUNDS PER 1,000 SQUARE FEET) PRIOR TO THE PLACEMENT OF TOPSOIL.

"I CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS, AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT."

LOGAL CORRE

SIGNATURE OF ENGINEER

B-4-3 STANDARDS AND SPECIFICATIONS FOR SEEDING AND MULCHING THE APPLICATION OF SEED AND MULCH TO ESTABLISH VEGETATIVE COVER.

MAXIMUM DRAINAGE AREA = 1 ACRE

- ¾ TO 1½ IN STONE

DETAIL E-9-2 AT-GRADE INLET PROTECTION

PLAN / CUT AWAY VIEW

CROSS SECTION

LIFT GRATE AND WRAP WITH NONWOVEN GEOTEXTILE TO COMPLETELY COVER ALL OPENINGS. SECURE WITH WITE TIES AND SET GRATE BACK IN PLACE.

PLACE CLEAN 1/4 TO 11/2 INCH STONE OR EQUIVALENT RECYCLED CONCRETE 6 INCHES THICK ON THE CRATE

. USE NONWOVEN GEOTEXTILE AS SPECIFIED IN SECTION H-1 MATERIALS.

CONSTRUCTION SPECIFICATIONS

PROTECT DISTURBED SOILS FROM EROSION DURING AND AT THE END OF CONSTRUCTION. CONDITIONS WHERE PRACTICE APPLIES
TO THE SURFACE OF ALL PERIMETER CONTROLS, SLOPES, AND ANY DISTURBED AREA NOT UNDER ACTIVE

. SPECIFICATIONS A. ALL SEED MUST MEET THE REQUIREMENTS OF THE MARYLAND STATE SEED LAW. ALL SEED MUST BE SUBJECT TO RE-TESTING BY A RECOGNIZED SEED LABORATORY. ALL SEED USED MUST HAVE BEEN TESTED WITHIN THE 6 MONTHS IMMEDIATELY PRECEDING THE DATE OF SOWING SUCH MATERIAL ON ANY PROJECT. REFER TO TABLE B.4 REGARDING THE QUALITY OF SEED. SEED TAGS MUST BE AVAILABLE UPON REQUEST TO THE INSPECTOR TO VERIFY TYPE OF SEED AND SEEDING RATE. B. MULCH ALONE MAY BE APPLIED BETWEEN THE FALL AND SPRING SEEDING DATES ONLY IF THE GROUND IS FROZEN. THE APPROPRIATE SEEDING MIXTURE MUST BE APPLIED WHEN THE GROUND THAWS.
INOCULANTS: THE INOCULANT FOR TREATING LEGUME SEED IN THE SEED MIXTURES MUST BE A PURE CULTURE OF NITROGEN FIXING BACTERIA PREPARED SPECIFICALLY FOR THE SPECIES. INOCULANTS MUST NOT BE USED LATER THAN THE DATE INDICATED ON THE CONTAINER, ADD FRESH INOCULANTS AS DIRECTED ON THE PACKAGE. USE FOUR TIMES THE RECOMMENDED RATE WHEN HYDROSEEDING. NOTE: IT IS VERY IMPORTANT TO KEEP INOCULANT AS COOL AS POSSIBLE UNTIL USED. TEMPERATURES ABOVE 75 TO 80 DEGREES FAHRENHEIT CAN WEAKEN BACTERIA AND MAKE THE INOCULANT LESS EFFECTIVE. SOD OR SEED MUST NOT BE PLACED ON SOIL WHICH HAS BEEN TREATED WITH SOIL STERILANTS OR CHEMICALS USED FOR WEED CONTROL UNTIL SUFFICIENT TIME HAS ELAPSED (14 DAYS MIN.) TO PERMIT DISSIPATION OF PHYTO-TOXIC MATERIALS.

A. DRY SEEDING: THIS INCLUDES USE OF CONVENTIONAL DROP OR BROADCAST SPREADERS. I. INCORPORATE SEED INTO THE SUBSOIL AT THE RATES PRESCRIBED ON TEMPORARY SEEDING TABLE , PERMANENT SEEDING TABLE 8.3, OR SITE-SPECIFIC SEEDING SUMMARIES. II. APPLY SEED IN TWO DIRECTIONS, PERPENDICULAR TO EACH OTHER. APPLY HALF THE SEEDING RATE IN EACH DIRECTION. ROLL THE SEEDED AREA WITH A WEIGHTED ROLLER TO PROVIDE GOOD B. DRILL OR CULTIPACKER SEEDING: MECHANIZED SEEDERS THAT APPLY AND COVER SEED WITH SOIL. I, CULTIPACKING SEEDERS ARE REQUIRED TO BURY THE SEED IN SUCH A FASHION AS TO PROVIDE AT LEAST 1/4 INCH OF SOIL COVERING. SEEDBED MUST BE FIRM AFTER PLANTING.

II. APPLY SEED IN TWO DIRECTIONS, PERPENDICULAR TO EACH OTHER. APPLY HALF THE SEEDING RATE IN EACH DIRECTION. HYDROSEEDING: APPLY SEED UNIFORMLY WITH HYDROSEEDER (SLURRY INCLUDES SEED AND FERTILIZER). I. IF FERTILIZER IS BEING APPLIED AT THE TIME OF SEEDING, THE APPLICATION RATES SHOULD NOT EXCEED THE FOLLOWING: NITROGEN, 100 POUNDS PER ACRE TOTAL OF SOLUBLE NITROGEN; P205 (PHOSPHOROUS), 200 POUNDS PER ACRE; K20 (POTASSIUM), 200 POUNDS PER ACRE. II. LIME: USE ONLY GROUND AGRICULTURAL LIMESTONE (UP TO 3 TONS PER ACRE MAY BE APPLIED BY HYDROSEEDING). NORMALLY, NOT MORE THAN 2 TONS ARE APPLIED BY HYDROSEEDING AT ANY ONE TIME DO NOT USE BURNT OR HYDRATED LIME WHEN HYDROSEEDING III. MIX SEED AND FERTILIZER ON SITE AND SEED IMMEDIATELY AND WITHOUT INTERRUPTION.

1. MULCH MATERIALS (IN ORDER OF PREFERENCE) A. STRAW CONSISTING OF THOROUGHLY THRESHED WHEAT, LYE, OAT, OR BARLEY AND REASONABLY BRIGHT IN COLOR. STRAW IS TO BE FREE OF NOXIOUS WEED SEEDS AS SPECIFIED IN THE MARYLAND SEED LAW AND NOT MUSTY, MOLDY, CAKED, DECAYED, OR EXCESSIVELY DUSTY. NOTE USE ONLY STERILE STRAW MULCH IN AREAS WHERE ONE SPECIES OF GRASS IS DESIRED. B. WOOD CELLULOSE FIBER MULCH (WCFM) CONSISTING OF SPECIALLY PREPARED WOOD CELLULOSE

IV. WHEN HYDROSEEDING DO NOT INCORPORATE SEED INTO THE SOIL.

PROCESSED INTO A UNIFORM FIBROUS PHYSICAL STATE.

I. WCFM IS TO BE DYED GREEN OR CONTAIN A GREEN DYE IN THE PACKAGE THAT WILL PROVIDE AN APPROPRIATE COLOR TO FACILITATE VISUAL INSPECTION OF THE UNIFORMLY SPREAD SLURRY. WCFM, INCLUDING DYE, MUST CONTAIN NO GERMINATION OR GROWTH INHIBITING FACTORS. III WCFM MATERIALS ARE TO BE MANUFACTURED AND PROCESSED IN SUCH A MANNER THAT TH WOOD CELLULOSE FIBER MULCH WILL REMAIN IN UNIFORM SUSPENSION IN WATER UNDER AGITATION 12. WITH PERMISSION FROM INSPECTOR, CLEAR AND GRUB SITE. AND WILL BLEND WITH SEED, FERTILIZER AND OTHER ADDITIVES TO FORM A HOMOGENEOUS SLURRY. THE MULCH MATERIAL MUST FORM A BLOTTER-LIKE GROUND COVER, ON APPLICATION HAVING MOISTURE ABSORPTION AND PERCOLATION PROPERTIES AND MUST COVER AND HOLD GRASS SEED IN CONTACT WITH THE SOIL WITHOUT INHIBITING THE GROWTH OF THE GRASS SEEDLINGS. IV. WCFM MATERIAL MUST NOT CONTAIN ELEMENTS OR COMPOUNDS AT CONCENTRATION LEVELS THAT V. WCFM MUST CONFORM TO THE FOLLOWING PHYSICAL REQUIREMENTS: FIBER LENGTH C APPROXIMATELY 10 MILLIMETERS, DIAMETER APPROXIMATELY 1 MILLIMETER, PH RANGE OF 4.0 TO

8.5. ASH CONTENT OF 1.6 PERCENT MAXIMUM AND WATER HOLDING CAPACITY OF 90 PERCENT A APPLY MULCH TO ALL SEEDED AREAS IMMEDIATELY AFTER SEEDING B. WHEN STRAW MULCH IS USED, SPREAD IT OVER ALL SEEDED AREAS AT THE RATE OF 2 TONS PER ACRE TO A UNIFORM LOOSE DEPTH OF 1 TO 2 INCHES. APPLY MULCH TO ACHIEVE A UNIFORM DISTRIBUTION AND DEPTH SO THAT THE SOIL SURFACE IS NOT EXPOSED. WHEN USING A MULCH NCHORING TOOL, INCREASE THE APPLICATION RATE TO 2.5 TONS PER ACRE. WOOD CELLULOSE FIBER USED AS MULCH MUST BE APPLIED AT A NET DRY WEIGHT OF 1500 POUNDS

PER ACRE. MIX THE WOOD CELLULOSE FIBER WITH WATER TO ATTAIN A MIXTURE WITH A MAXIMUM OF

50 POUNDS OF WOOD CELLULOSE FIBER PER 100 GALLONS OF WATER. A. PERFORM MULCH ANCHORING IMMEDIATELY FOLLOWING APPLICATION OF MULCH TO MINIMIZE LOSS BY WIND OR WATER. THIS MAY BE DONE BY ONE OF THE FOLLOWING METHODS (LISTED BY PREFERENCE), DEPENDING UPON THE SIZE OF THE AREA AND EROSION HAZAR I. A MULCH ANCHORING TOOL IS A TRACTOR DRAWN IMPLEMENT DESIGNED TO PUNCH AND ANCHOR MULCH INTO THE SOIL SURFACE A MINIMUM OF 2 INCHES. THIS PRACTICE IS MOST EFFECTIVE ON LARGE AREAS, BUT IS LIMITED TO FLATTER SLOPES WHERE EQUIPMENT CAN OPERATE SAFELY. IF SED ON SLOPING LAND, THIS PRACTICE SHOULD FOLLOW THE CONTOUR. II. WOOD CELLULOSE FIBER MAY BE USED FOR ANCHORING STRAW. APPLY THE FIBER BINDER AT A NET DRY WEIGHT OF 750 POUNDS PER ACRE. MIX THE WOOD CELLULOSE FIBER WITH WATER AT A

MAXIMUM OF 50 POUNDS OF WOOD CELLULOSE FIBER PER 100 GALLONS OF WATER.

II. SYNTHETIC BINDERS SUCH AS ACRYLIC DLR (AGRO—TACK), DCA—70, PETROSET, TERRA TAX II, TERRA TACK AR OR OTHER APPROVED EQUAL MAY BE USED. FOLLOW APPLICATION RATES AS SPECIFIED BY THE MANUFACTURER. APPLICATION OF LIQUID BINDERS NEEDS TO BE HEAVIER AT THE EDGES WHERE WIND CATCHES MULCH, SUCH AS IN VALLEYS AND ON CRESTS OF IV. LIGHTWEIGHT PLASTIC NETTING MAY BE STAPLED OVER THE MULCH ACCORDING TO MANUFACTURER RECOMMENDATIONS. NETTING IS USUALLY AVAILABLE IN ROLLS 4 TO 15 FEET WIDE AND 300 TO

> 3.053.00° GRADED AGGREGATE BASE (GAB) HOT MIX ASPHALT CURB R-3.03

SEQUENCE OF CONSTRUCTION

DETAIL C-1 EARTH DIKE

CONTINUOUS GRADE 0.5% MIN. TO 10% MAX. SLOPE

BAAAAAA

PLAN VIEW

FLOW CHANNEL STABILIZATION

COMPACT FILL

CROSS_SECTION

A-2/B-2 SEED WITH SOIL STABILIZATION MATTING OR LINE WITH SOD.

PROVIDE OUTLET PROTECTION AS REQUIRED ON APPROVED PLAN

SEED WITH STRAW MULCH AND TACK, (NOT ALLOWED FOR CLEAR WATER DIVERSION.)

REMOVE AND DISPOSE OF ALL TREES, BRUSH, STUMPS, OBSTRUCTIONS, AND OTHER OBJECTIONABLE MATERIAL SO AS NOT TO INTERFERE WITH PROPER FUNCTION OF EARTHDIKE.

CONSTRUCT FLOW CHANNEL ON AN UNINTERRUPTED, CONTINUOUS GRADE, ADJUSTING THE LOCATION DUE TO FIELD CONDITIONS AS NECESSARY TO MAINTAIN POSITIVE DRAINAGE.

STABILIZE EARTH DIKE WITHIN THREE DAYS OF INSTALLATION. STABILIZE FLOW CHANNEL FOR CLEAR WATER DIVERSION WITHIN 24 HOURS OF INSTALLATION.

MAINTAIN LINE, GRADE, AND CROSS SECTION. REMOVE ACCUMULATED SEDIMENT AND DEBRIS, AND MAINTAIN POSITIVE DRAINAGE, KEEP EARTH DIKE AND POINT OF DISCHARGE FREE OF EROSION, AND CONTINUOUSLY MEET REQUIREMENTS FOR ADEQUATE VEGETATIVE ESTABLISHMENT IN ACCORDANCE WITH SECTION B-4 VEGETATIVE STABILIZATION.

UPON REMOVAL OF EARTH DIKE, GRADE AREA FLUSH WITH EXISTING GROUND. WITHIN 24 HOURS OF REMOVAL STABILIZE DISTURBED AREA WITH TOPSOIL, SEED, AND MULCH, OR AS SPECIFIED ON APPROVED PLAN.

MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL

A-3/B-3 4 TO 7 INCH STONE OR EQUIVALENT RECYCLED CONCRETE PRESSED INTO SOIL A MINIMUM OF 7 INCHES AND FLUSH WITH GROUND.

EXCAVATE OR SHAPE EARTH DIKE TO LINE, GRADE, AND CROSS SECTION AS SPECIFIED. BANK PROJECTIONS OR OTHER IRREGULARITIES ARE NOT ALLOWED.

_____2:1 SLOPE OR FLATTER

a - DIKE HEIGHT 18 IN MIN. 30 IN MIN.

b - DIKE WIDTH 24 IN MIN. 36 IN MIN.

c - FLOW WIDTH 4 FT MIN. 6 FT MIN.

d - FLOW DEPTH 12 IN MIN. 24 IN MIN.

I. OBTAIN HOWARD COUNTY GRADING PERMIT AND MDE PERMIT FOR STORMWATER ASSOCIATED WITH CONSTRUCTION ACTIVITY. (1 DAY) 2. OBTAIN PUBLIC WATER AND SEWER CONTRACT DRAWINGS (CONT. 24-4649-D) FROM HOWARD 3. NOTIFY HOWARD COUNTY AT LEAST 48 HOURS PRIOR TO START OF CONSTRUCTION. (2 DAYS)

4. CONDUCT A PRE-CONSTRUCTION MEETING WITH THE SEDIMENT CONTROL INSPECTOR PRIOR TO

DETAIL E-3 SUPER SILT FENCE

CHAIN LINK FENCING

WOVEN SLIT FILM GEOTEXTILE-FLOW _

EMBED GEOTEXTILE AND -CHAIN LINK FENCE 8 IN MIN. INTO GROUND

-36 IN MIN

GALVANIZED CHAIN LINK FENCE WITH WOVEN SLIT FILM GEOTEXTILE

ELEVATION

CROSS SECTION

FASTEN 9 GAUGE OR HEAVIER GALVANIZED CHAIN LINK FENCE (2% INCH MAXIMUM OPENING) 42 INCHES IN HEIGHT SECURELY TO THE FENCE POSTS WITH WIRE TIES OR HUG RINGS.

EXTEND BOTH ENDS OF THE SUPER SILT FENCE A MINIMUM OF FIVE HORIZONTAL FEET UPSLOPE AT 45 DEGREES TO THE MAIN FENCE ALIGNMENT TO PREVENT RUNOFF FROM GOING AROUND THE ENDS OF THE SUPER SILT FENCE.

MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL

2011 WARYLAND DEPARTMENT OF ENVIRONMEN
WATER MANAGEMENT ADMINISTRATION

GROUND SURFACE

STEEL OF

CONSTRUCTION SPECIFICATIONS

ANY LAND DISTURBANCE. (1 WEEK) INSTALL STABILIZED CONSTRUCTION ENTRANCES WITH MOUNTABLE BERMS, AND DIVERSION FENCE ALONG NORTHERN LOD (ADJACENT TO LINCOLN PARKING ARES). (3 DAYS). 6. INSTALLATION OF STORM DRAIN SYSTEM (HW-2 TO I-6):

A. REMOVE AND REPLACE STORM DRAIN SYSTEM FROM HW-2 TO MH-1, BUILDING MH-1 OVER EXISTING 36" RCP WHILE MAINTAING FLOW THROUGH EXISTING STORM DRAIN B. CONSTRUCT MH-1 TO MH-2 AND BLOCK TEMPORARILY. C. CONSTRUCT FOOTER AND FOUNDATION FOR WESTERN PORTION OF BUILDINGS #1 AND #2

TO 20' FROM EDGE OF BUILDING. D. CONSTUCT I-6 OVER EXISTING 18" RCP, AND BUILD REMAINING STORM DRAIN SYSTEM FROM MH-2 TO I-6. REMOVE PORTION OF EXISTING 18" RCP FROM EX. I-22 TO PROP. 1-6. UPON COMPLETION, IMMEDIATELY REMOVE TEMPORARY BLOCKING AT MH-1. SEVER AND BULKHEAD EXISTING 36" CONNECTION TO MH-1. DURING INSTALLATION OF STORM DRAIN, CONTRACTOR TO USE DIVERSION PIPE (MGWC 1.4) AS NEEDED, DAILY, TO KEEP CLEAN WATER SEPARATE FROM ENTERING AND MIXING WITH THE LOD. E. WITH PERMISSION FROM THE SEDIMENT CONTROL INSPECTOR, STABILIZE AREA BETWEEN WESTERN LOD AND BUILDINGS #1 & #2 WITH ECM, AND REMOVE DIVERSION OF SAME LOCATION, ALLOWING ANY OFFSITE WATER TO ENTER THIS SYSTEM, THEREFORE BYPASSING THE SITE. (DIVERSION FENCE ADJACENT TO LINCOLN PARKING TO REMAIN IN

PLACE.) . REMOVE THE EXISTING CURB ADJACENT TO I-6 AS INDICATED ON PLAN, AND INSTALL NOSE DOWN CURB AS SHOWN. INSTALL TEMPORARY AND PERMENANT CURB ALONG NORTHERN LOD (ADJACENT TO LINCOLN PARKING). WITH CURB INSTALLED AND PROPER FLOW ESTABLISHED TO INLETS (ALLOWING OFFSITE WATER TO BYPASS SITE), REMOVE REMAINDER OF DIVERSION

A. EXCAVATE POOL AREA. EXCAVATED MATERIAL SHALL BE PLACED ABOVE THE BASIN AREA OR TO THE DESIGNATED SPOIL AREA. INSTALL SILT FENCE BELOW THE DESIGNATED SPOIL AREA PRIOR TO TRANSPORTING THE EXCESS SPOIL MATERIAL. B. INSTALL PRINCIPAL SPILLWAY (METAL RISER, CONCRETE BASE, BARREL AND DEWATERING DEVICE).

THE SEDIMENT BASIN IS EXCAVATION AND REQUIRES NO EMBANKMENT CONSTRUCTION. SHAPE POOL AREA TO DESIGNED CONFIGURATION. INSTALL REMOVABLE PUMPING STATION (RPS) IN BASIN POOL AREA AS SHOWN ON THE PLAN. STABILIZE THE SEDIMENT BASIN PER SPECIFICATIONS. 9. WITH PERMISSION FROM INSPECTOR, INSTALL PERIMETER CLEANWATER DIKES, SF. AND SS

ALONG EASTERN PROPERTY LINE. 10. INSTALLATION OF STORM DRAIN SYSTEM (MH-1 TO I-11): A. INSTALL STORM DRAIN SYSTEM FROM MH-1 TO I-11. DURING INSTALLATION OF STORM DRAIN, CONTRACTOR TO USE DIVERSION PIPE (MGWC 1.4) AS NEEDED, DAILY, TO KEEP CLEAN WATER SEPARATE FROM ENTERING AND MIXING WITH THE LOD. B. PROVIDE INLET PROTECTION FOR INLETS IN THIS RUN, EXCEPT FOR I-11. WRAP INLETS 1-7, 1-8, 1-9, AND 1-10 IN SSF.

C. STABILIZE AREA AROUND I-11 WITH ECM AND INSTALL PERIMETER EARTH DIKES ALONG NORTH EAST PROPERTY LINE INSTALL EARTHDIKES TO SEDIMENT BASIN, AND PERIMETER SEDIMENT CONTROL DEVICES.

13. BEGIN MASS GRADING 14. AS SITE IS BROUGHT TO PROPOSED GRADE, BEGIN CONSTRUCTION OF RETAINING WALLS ALONG WESTERN EDGE OF SITE, RETAINING WALL #3 TO BE BUILT FROM STATION 0+00 TO 0+28. REMAINDER OF RETAINING WALL #3 TO BE BUILT AFTER REMOVAL OF SEDIMENT BASIN. 15 REGIN INSTALLATION OF UTILITIES AND UNDERGROUND SWM FACILITIES. AS STORMDRAIN SYSTEM IS CONSTRUCTED, BULKHEAD BYPASS PIPES AT STRUCTURES CS-1, CS-2, CS-3, AND CS-4. WRAP INLETS I-15, I-16, I-17, I-18, I-19 AND I-20 IN SSF. AS NECESSARY DEWATER UTILITY EXCAVATION AND USE FILTER BAG FOR SEDIMENT CONTROL. (12 WEEKS)

16. BEGIN BUILDING CONSTRUCTION. 17. WITH UTILITIES INSTALLED AND SITE BROUGHT TO GRADE, BEGIN INSTALLATION OF SUB-BASE PAVING AND CURB, BEGINNING AT I-7 AND CONTINUING NORTH. CURB AT FRONT OF BUILDLING #2 TO BE INSTALLED AFTER REMOVAL OF SEDIMENT BASIN. 18. INSTALL ALL BASE PAVING NORTH OF BUILDING #2.

STABILIZE ALL NON-PAVED AREAS NORTH OF BASIN WITH SEED AND MULCH. 20. AFTER ALL CONTRIBUTING AREAS HAVE BEEN STABILIZED, AND WITH THE PERMISSION OF THE SEDIMENT CONTROL INSPECTOR, REMOVE ANY SEDIMENT CONTROLS WHICH MAY BE REMAINING, WITH THE EXCEPTION OF THE SUPER SILT FENCE AROUND SURFACE SAND FILTER AREA. PERMANENTLY STABILIZE ALL AREAS DISTURBED AS A RESULT OF THE REMOVAL OF SEDIMENT CONTROL MEASURES. UNDERGROUND SWM STORMFILTER SYSTEMS MAY BE MADE OPERTAIONAL WHEN THE DRAINAGE AREA IS STABILIZED WITH ESTABLISHED VEGETATION. (2 DAYS)

21. WITH PERMISSION OF THE SEDIMENT CONTROL INSPECTOR, REMOVE SEDIMENT BASIN, AND INSTALL PHASE 2 SEDIMENT CONTROL MEASURES. BASIN AREA IS TO RECEIVE CONTROLLED COMPACTED FILL AS REQUIRED BY THE GEOTECHNICAL ENGINEER. (1 WEEK) BRINGING AREA SOUTH OF BUILDING #2 TO GRADE AND INSTALL REMAINING UTILITIES. 23. AS GRADING PROGRESSES COMPLETE RETAINING WALL CONSTRUCTION. (3 WEEKS)

24. INSTALL CURB AND GUTTER AND STONE SUB-BASE SOUTH OF BUILDLING #2. 25. AS PARKING LOT CONSTRUCTION IS COMPLETED, STABILIZE REMAINING NON-PAVED AREAS WITH SEED AND MULCH. (3 WEEKS) 26. AFTER ALL CONTRIBUTING AREAS HAVE BEEN STABILIZED, FLUSH OUT THE STORM DRAIN SYSTEM THROUGH FILTERBAG. (1 DAY)

27. WITH PERMISSION OF THE SEDIMENT CONTROL INSPECTOR, INSTALL BIORETENTION AND SURFACE SANDFILTER. THE SWM AREAS SHALL BE SHAPED TO FINAL LINE, PRETREATMENT AREAS AND SAND FILTERS TO BE INSTALLED AND GRADED AS SHOWN ON THE APPROVED STORM WATER MANAGEMENT PLANS. (1 WEEK)* *OUTFALLS FROM CS-4 TO SURFACE SAND FILTER IS TO REMAIN BLOCKED DURING

CONSTRUCTION. ALL INLETS TO HAVE INLET PROTECTION. 28. COMPLETE ANY REMAINING UTILITY AND SWM CONSTRUCTION. A COMPLETE AS-BUILT SURVEY AND STUDY AND SUBMIT TO HOWARD COUNTY DED WITHIN 30 DAYS OF COMPLETION OF

STORMWATER MANAGEMENT FACILITIES. 29. COMPLETE BUILDING CONSTRUCTION 30. COMPLETE SURFACE COURSE PAVING AND INSTALL SIDEWALKS.

VERTICAL (3:1); AND

31. INSTALL SITE LANDSCAPING. (3 DAYS) 32. WITH INSPECTOR'S APPROVAL, FINE GRADE AND STABILIZE ALL AREAS OF SITE INCLUDING ANY EXPOSED EARTH AREAS OUTSIDE THE LOD. REMOVE ALL TRASH JUNK AND DEBRIS FROM ENTIRE PARCEL. (2 WEEKS) 33. WITH THE PERMISSION OF THE SEDIMENT CONTROL INSPECTOR, REMOVE ANY SEDIMENT CONTROLS WHICH MAY BE REMAINING. STABILIZE ALL AREAS DISTURBED AS A RESULT OF THE REMOVAL OF SEDIMENT CONTROL MEASURES.

1. DURING GRADING AND AFTER EACH RAINFALL, CONTRACTOR WILL INSPECT AND PROVIDE NECESSARY MAINTENANCE TO THE SEDIMENT CONTROL MEASURES ON THIS PLAN. 2. FOLLOWING INITIAL SOIL DISTURBANCES OR RE-DISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: A. THREE (3) CALENDAR DAYS AS TO THE SURFACE OF ALL PERIMETER DIKES, SWALES, DITCHES, PERIMETER SLOPES, AND ALL SLOPES STEEPER THAN 3 HORIZONTAL TO 1

PROJECT SITE NOT UNDER ACTIVE GRADING. 3. ALL UNDERGROUND SWM STRUCTURES MUST REMAIN SEPARATE OR BULKHEADED FROM ENTERING STORMDRAIN UNTIL ALL CONTRIBUTING AREAS ARE PERMANENTLY STABILIZED AND WRITTEN PERMISSION IS PROVIDED BY INSPECTOR TO ALLOW OPENING FOR FLOW. ANY WATER COLLECTED IN WORK AREA SHALL BE PUMPED THROUGH FILTERBAG. 4. ALL SEDIMENT CONTROLS THAT ARE DAMAGED DURING CONSTRUCTION ARE TO BE REPAIRED IMMEDIATELY.

40 AS-BULT INFORMATION ON THIS SHEET

B. SEVEN (7) CALENDAR DAYS TO ALL OTHER DISTURBED OR GRADED AREAS ON THE

- WOVEN SLIT FILM GEOTEXTILE TYPE A TYPE B ISOMETRIC VIEW EDGE OF ROADWAY OR TOP-OF EARTH DIKE INSTALL 2% INCH DIAMETER GALVANIZED STEEL POSTS OF 0.095 INCH WALL THICKNESS AND SIX FOOLENGTH SPACED NO FURTHER THAN 10 FEET APART. DRIVE THE POSTS A MINIMUM OF 36 INCHES INTO THE GROUND. WHERE ENDS OF THE GEOTEXTILE COME TOGETHER, THE ENDS SHALL BE OVERLAPPED BY 6 INCHES/FOLDED, AND STAPLED TO PREVENT SEDIMENT BY PASS. PROVIDE MANUFACTURER CERTIFICATION TO THE INSPECTION/ENFORCEMENT AUTHORITY SHOWING THAT GEOTEXTILE USED MEETS THE REQUIREMENTS IN SECTION H-1 MATERIALS. SECTION FOR TYPE A AND B REMOVE ACCUMULATED SEDIMENT AND DEBRIS WHEN BULGES DEVELOP IN FENCE OR WHEN SEDIMENT REACHES 25% OF FENCE HEIGHT. REPLACE GEOTEXTILE IF TORN. IF UNDERMINING OCCURS, REINSTALL CHAIN LINK FENCING AND GEOTEXTILE.

DETAIL E-9-1 STANDARD INLET PROTECTION

-TOP ELEVATION

-16 IN MIN. -NOTCH ELEVATION

2011 DETAIL E-9-1 STANDARD INLET PROTECTION

USE WOVEN SLIT FILM GEOTEXTILE AS SPECIFIED IN SECTION H-1 MATERIALS. EXCAVATE COMPLETELY AROUND THE INLET TO A DEPTH OF 18 INCHES BELOW THE NOTCH ELEVATION FOR TYPE A, USE NOMINAL 2 INCH X 4 INCH CONSTRUCTION GRADE LUMBER POSTS, DRIVEN 1 FOOT INTO THE GROUND AT EACH CORNER OF THE INLET. PLACE NAIL STRIPS BETWEEN THE POSTS ON THE ENDS OF THE INLET. ASSEMBLE THE TOP PORTION OF THE 2X4 FRAME AS SHOWN. STRETCH & INCH GALVANIZED HARDWARE CLOTH TIGHTLY AROUND THE FRAME AND FASTEN SCURELY, FASTEN GEOTEXTILE SECURELY TO THE HARDWARE CLOTH WITH TIES SPACED EVERY 24 INCHES AT THE TOP AND MID SECTION. EMBED GEOTEXTILE AND HARDWARE CLOTH A MINIMUM OF 18 INCHES BELOW THE WERR CREST. THE ENDS OF THE GEOTEXTILE MUST MEET AT A POST, BE OVERLAPPED AND FOLDED; THEN FASTENED TO THE POST. FOR TYPE B, USE 2½ INCH DIAMETER GALVANIZED STEEL POSTS OF 0.095 INCH WALL THICKNESS A 6 FOOT LENGTH, DRIVEN A MINIMUM OF 36 INCHES BELOW THE WEIR CREST AT EACH CORNER OF T STRUCTURE. FASTEN 9 GAUGE OR HEAVIER CHAIN LINK FENCE, 42 INCHES IN HEIGHT, SECURELY TO THE FENCE POSTS WITH WIRE TIES. FASTEN GEOTEXTILE SECURELY TO THE CHAIN LINK FENCE WITH TIES SPACED EVERY 24 INCHES AT THE TOP AND MID SECTION. EMBED GEOTEXTILE AND CHAIN LINK FENCE WITH FENCE A MINIMUM OF 18 INCHES BELOW THE WEIR CREST.

SP SP

STORM DRAIN INLET PROTECTION REQUIRES FREQUENT MAINTENANCE. REMOVE ACCUMULATED SEDIMENT AFTER EACH RAIN EVENT TO MAINTAIN FUNCTION AND AVOID PREMATURE CLOGGING. IF INLET PROTECTION DOES NOT COMPLETELY DRAIN WITHIN 24 HOURS AFTER A STORM EVENT ITS CLOGGED. WHEN THIS OCCURS, REMOVE ACCUMULATED SEDIMENT AND CLEAN, OR REPLACE GEOTEXTILE AND

MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL

THROUGH FILTERBAG.



AS-BUILT CERTIFICATION FOR PSWI

EREBY CERTIFY THAT THE FACILITY SHOWN ON THIS WAS CONSTRUCTED AS SHOWN ON THE "AS-BUILT" PLATS AND COMPLIES WITH THE APPROVED PLANS AND SPECIFICATIONS. ERIFIED THAT THE CONTRIBUTING DRAINAGE SUFFICIENTLY STABILIZED TO PREVENT CLOGGING

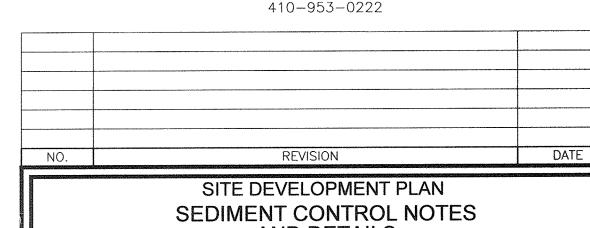
UNDERGROUND SWM FACILITY.

FROM ENTERING STORMDRAIN UNTIL ALL CONTRIBUTING AREAS ARE PERMANENTLY STABILIZED AND WRITTEN PERMISSION IS PROVIDED BY INSPECTOR TO ALLOW OPENING FOR FLOW. ANY WATER COLLECTED IN WORK AREA SHALL BE PUMPED

ALL UNDERGROUND SWM STRUCTURES MUST REMAIN SEPARATE OR BULKHEADED

OWNER/DEVELOPER

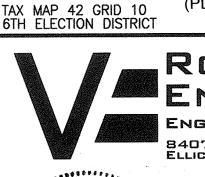
TSC/JMJ SNOWDEN RIVER SOUTH LLC A MD LLC 8600 SNOWDEN RIVER PKWY, SUITE 207 COLUMBIA MD 21045



AND DETAILS MIDWAY BUSINESS CENTER

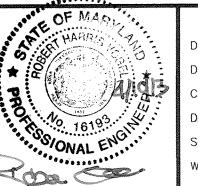
SNOWDEN RIVER SOUTH SECTION 1, AREA 1 INDUSTRIAL / FLEX SPACE

(PLAT 8795, L.10008/F.485) PARCEL 319, PARCEL HOWARD COUNTY, MARYLAND



ROBERT H. VOGEL ENGINEERING, INC. ENGINEERS . SURVEYORS . PLANNERS

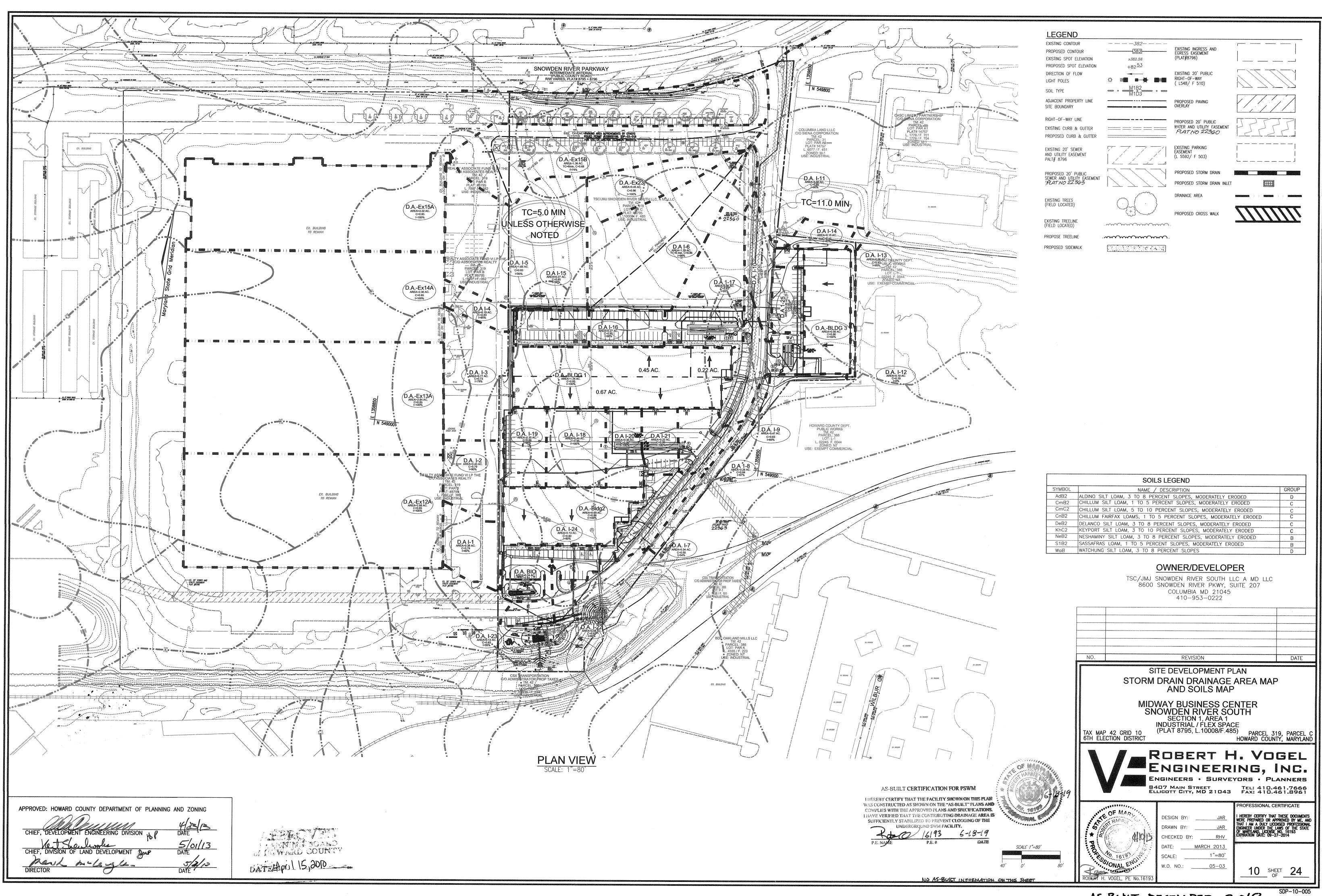
8407 MAIN STREET TEL: 410.461.7666 ELLICOTT CITY, MD 21043 FAX: 410.461.8961

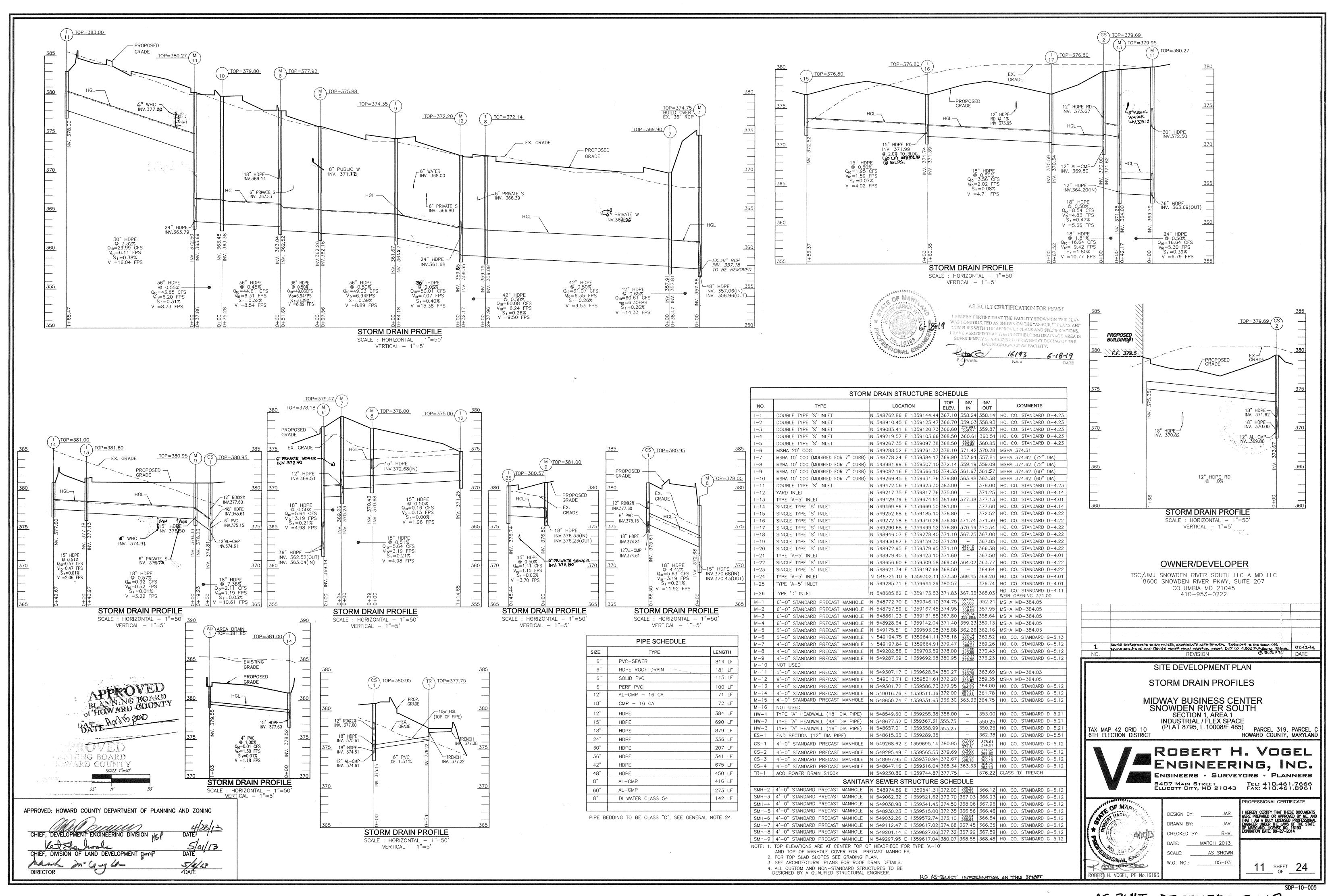


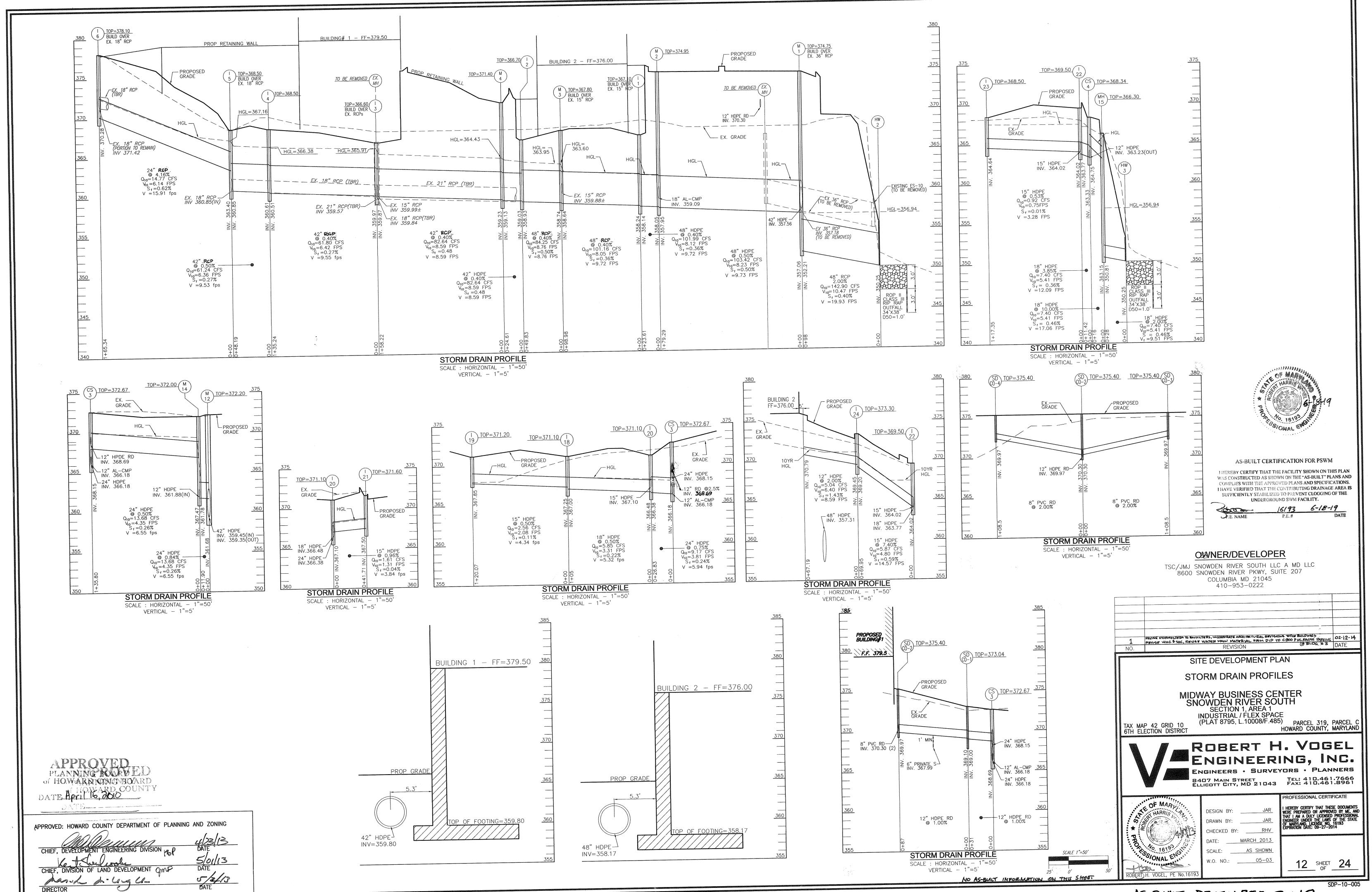
DESIGN BY: JAR DRAWN BY: RHV CHECKED BY: SCALE: AS SHOWN W.O. NO.: 05-03

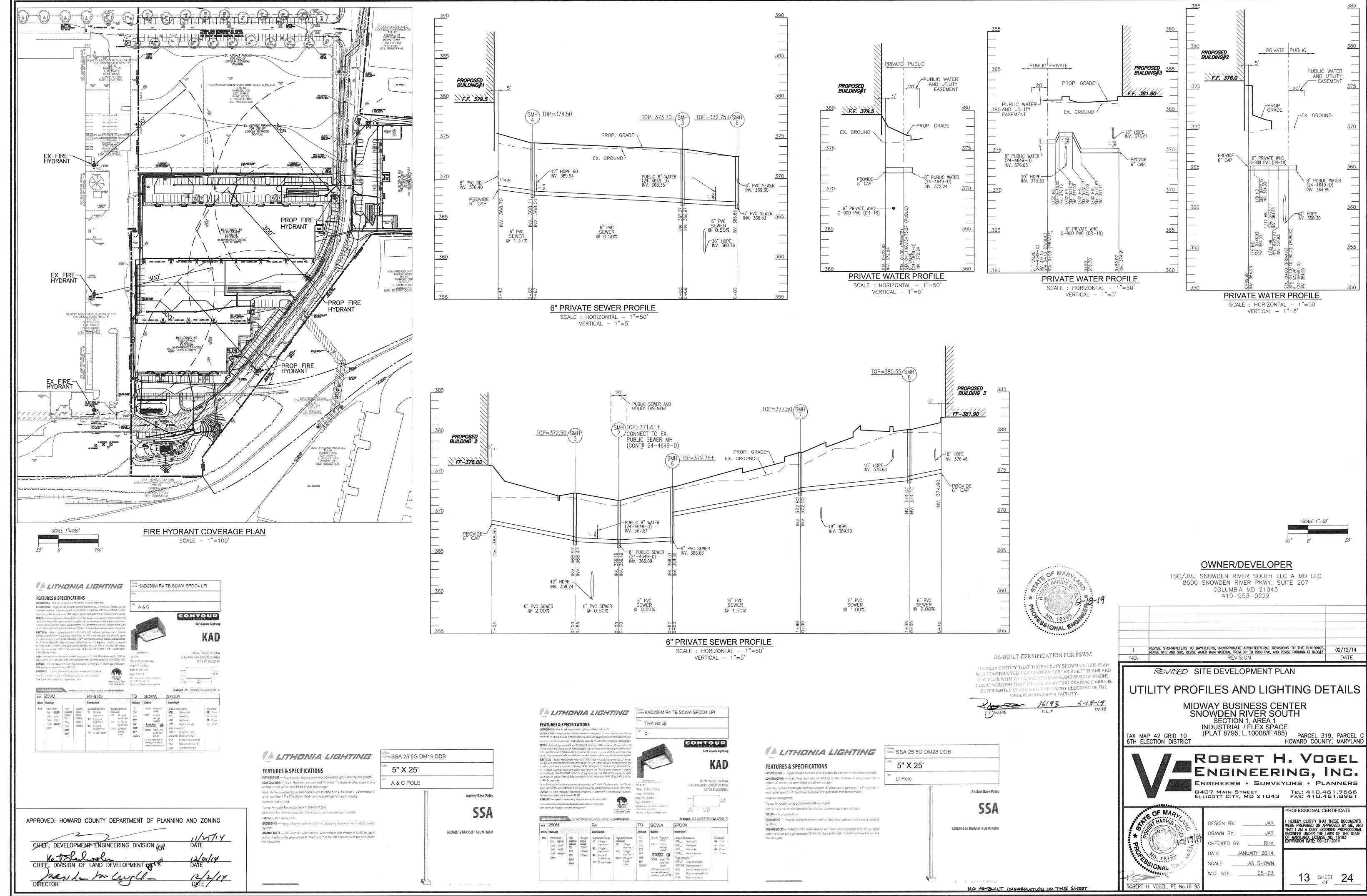
I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO, 16193 EXPIRATION DATE: 09-27-2014

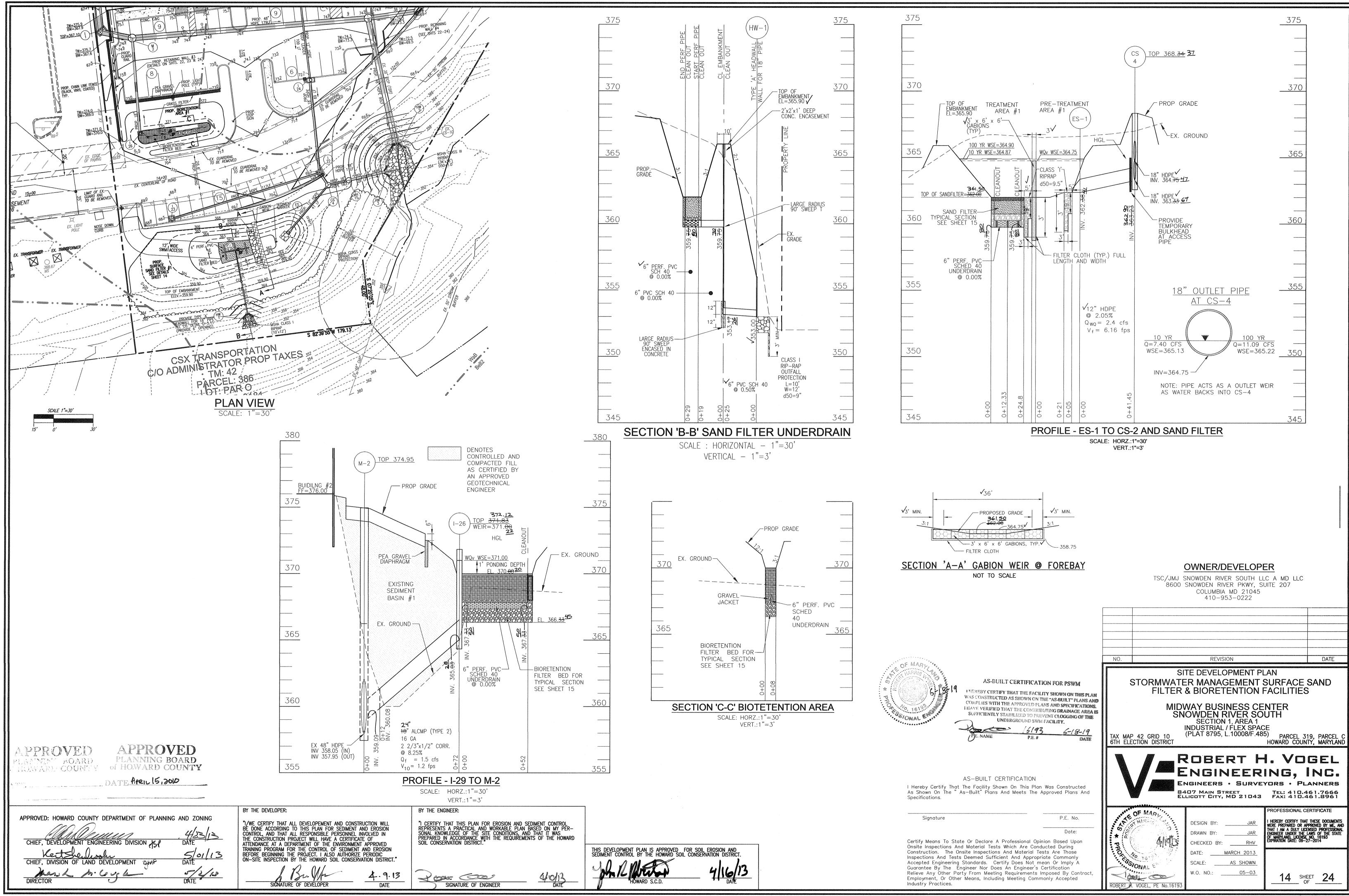
SHEET 24

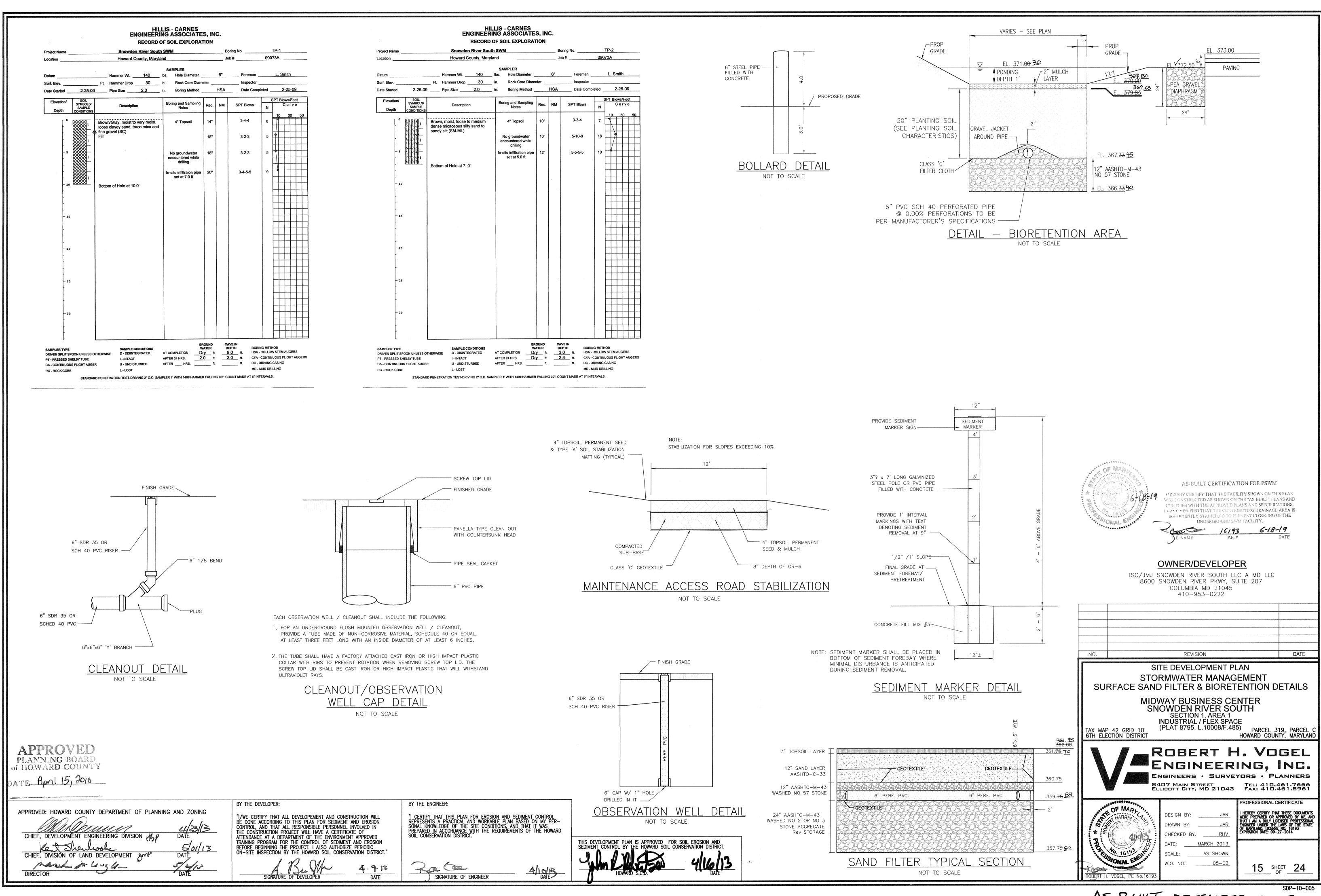


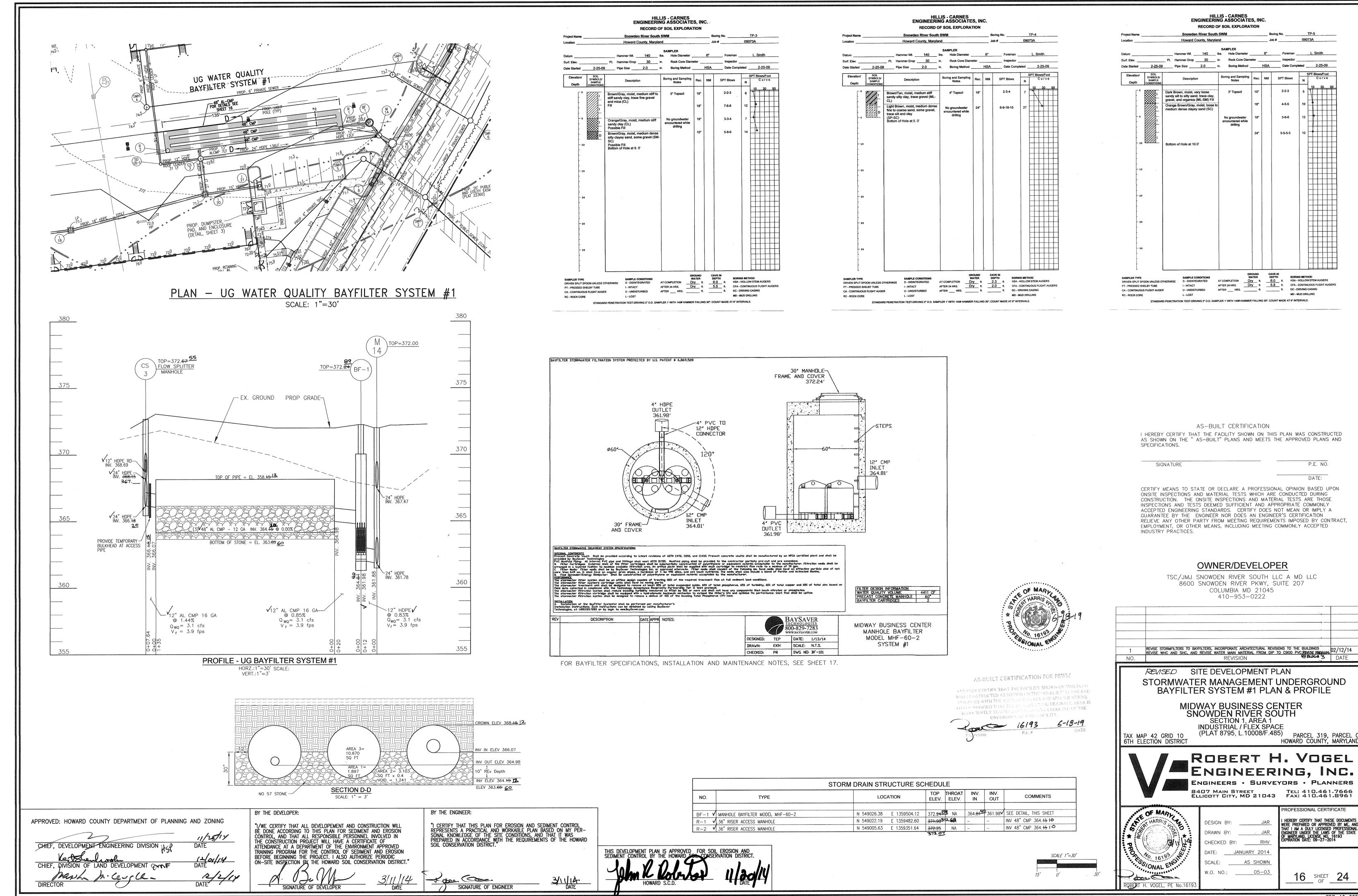




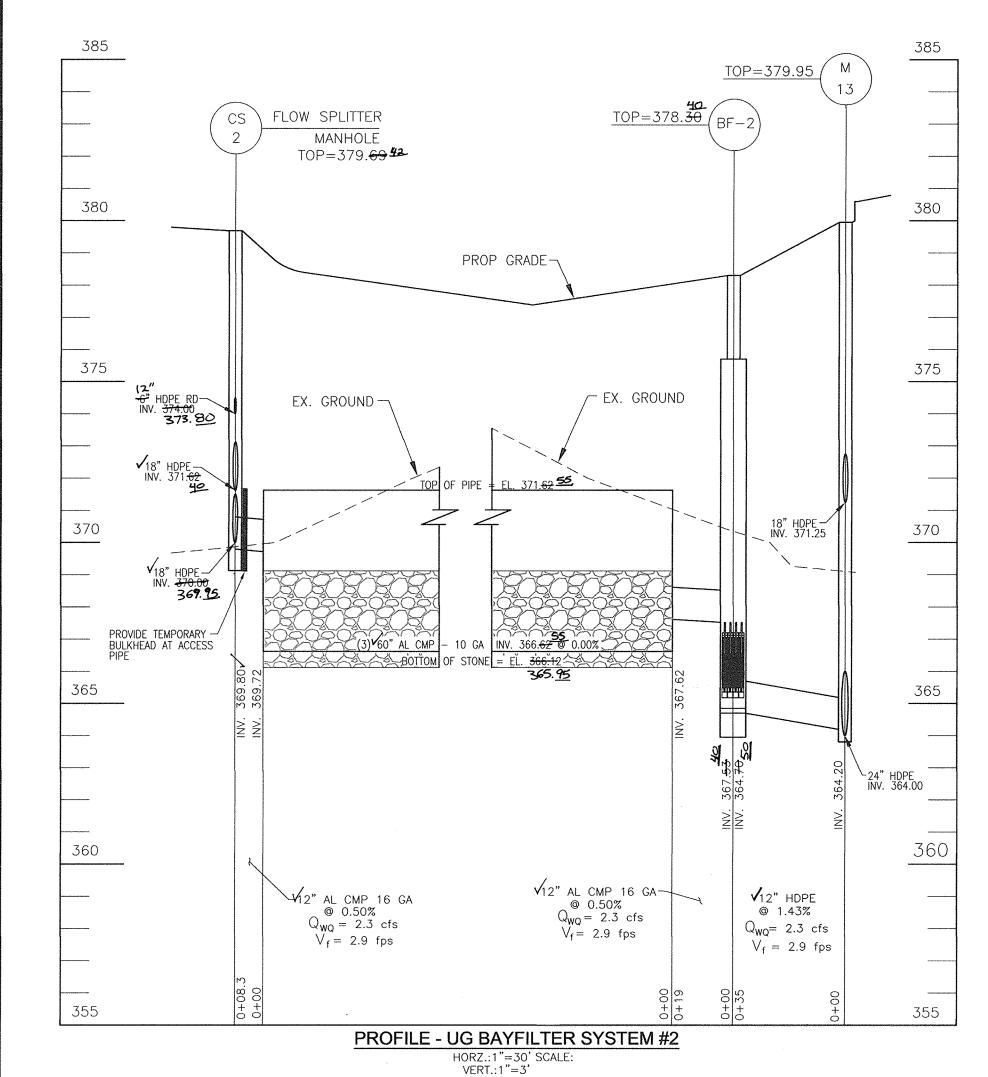








K:\Projects\05-03\ENGR\dwg\SDP\\REDL\\NE\\REDL\\NE\\REPLACE-SHEET16-18_SWMDETAILS.dwg, 3/27/20'



	STORM DRAIN STRUCTURE SCHEDULE								
NO.	TYPE	LOCATION		TOP ELEV.	THROAT ELEV.	INV. IN	INV. OUT	COMMENTS	
				LIC.		ц	90		
BF-2	MANHOLE BAYFILTER MODEL MHF-60-2	N 549299.19 E 135	9566.51	378. 30	NA	367. 53	364. 70	SEE DETAILS, THIS SHEET	
R-3	36" RISER ACCESS MANHOLE ✓	N 549270.39 E 135	9545.08	378. 30 19	P NA	_	_	INV 60" CMP 366.62 55	
R-4	36" RISER ACCESS MANHOLE ✓	N 549277.48 E 135	9466.51	377. 504	⊇ NA			INV 60" CMP 366.62-55	
R-5	36" RISER ACCESS MANHOLE ✓	N 549288.49 E 135	9568.35	378 .80 9	≦ NA	_	_	INV 60" CMP 366.6255	
	*** *** *** *** *** *** *** *** *** **								

BY THE DEVELOPER: APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

"I/WE CERTIFY THAT ALL DEVELOPEMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN FOR SEDIMENT AND EROSION CONTROL, AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED

TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ON—SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT."

BY THE ENGINEER: "I CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS, AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT."

4 con Con-

SIGNATURE OF ENGINEER

Silicone Sealant: Shall be pure RTV silicone conforming to Federal Specification Number TT S001543A or TT S00230C or Engineer

Grout: Shall be non-shrink grout meeting the requirements of Corps of Engineers CRD-C588. Specimens molded, cured and tested in accordance

Backfill: Backfill shall be 4-inch minus rock at 95% compaction. A. The quality of materials, the process of manufacture, and the finished sections shall be subject to inspection by the Engineer. Such inspection may be made at the place of manufacture, or on the worksite after delivery, or at both places, and shall be subject to rejection at any time if material conditions fail to meet any of the specification requirements. If a PRECAST CONCRETE VAULT BayFilter system component(s) is rejected after delivery to the site, it shall be marked for identification and removed from the site. Any BayFilter

system component(s) which have been damaged beyond repair during delivery will be rejected. 1.3 SUBMITTALS

1.2 QUALITY CONTROL INSPECTION

A Plan elevation and profile dimensional drawings shall be submitted to the Engineer for review and approval. The Contractor shall be provided with the approved plan, elevation, and profile dimensional drawings.

BAYFILTERTM SPECIFICATIONS

A. The BayFilterTM system's internal components manufacturer selected

attached drawing(s) and these specifications.

by the Contractor and approved by the Engineer, shall furnish all labor,

materials, equipment and incidentals required to manufacture the

BayFilter system components(s) specified herein in accordance with the

B. Concrete structures and any appurtenances that form an integral part of the BayFilter™ system shall be described in Part 2.00 of these specifications.

PART 2.00 PRODUCTS

PART 1.00 GENERAL

1.1 <u>DESCRIPTION</u>

2.1 <u>INTERNAL COMPONENTS</u> All components including concrete structure(s), PVC manifold piping and filter cartridges, shall be provided by BaySaver Technologies Inc., 1030 Deer Hollow 3.3 Drive, Mount Airy, MD (800,229,7283).

A. PVC Manifold Piping: All internal PVC pipe and fittings shall meet ASTM D1785. Manifold piping shall be provided to the contractor partially pre-cut and pre assembled B. Filter Cartridges: External shell of the filter cartridges shall be substantially constructed of polyethylene or equivalent material acceptable to the manufacturer. Filtration media shall be arranged in a layered fashion to maximize available filtration area. An orifice plate shall be supplied

with each cartridge to restrict flow rate to a maximum of 30 gpm. C. Filter Media: Filter media shall be by BaySaver Technologies Inc. or approved alternate. Filter media shall consist of the following mix. Sand media shall have an effective particle size of not more than 0.49mm, it shall have an angular grain shape, a hardness of 7, be 99% silica, and not leach nutrients. The media shall also include a blend of Perlite and Activated Alumina

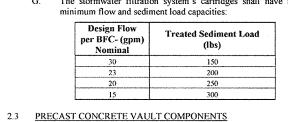
2.2 PERFORMANCE

A. The stormwater filter system shall be capable of treating 100% of the required treatment flow at full sediment load conditions.

B. The stormwater filter system's cartridges shall have no moving parts. C. The stormwater treatment unit shall be designed to remove at least 80% of the suspended solids load. Said removal shall be based on full-scale testing using SIL-CO-SIL 106 media gradation with a d₅₀ of 23 microns (manufactured by US Silica) or equivalent. Said full scale testing shall have included sediment capture based on actual total mass collected by the PART 4.00 EXECUTION stormwater filtration system.

D. The stormwater filtration system shall reduce incoming turbidity (measured as NTUs) by 50% or more and shall not have any components that leach nitrates or phosphates E. The stormwater filtration cartridge shall be equipped with a hydrodynamic backwash mechanism to extend the filter's life and optimize its performance. Inlet flow shall be upflow.

F. The stormwater filtration system shall be designed to remove a minimum Installation of a BayFilterTM System of 50% of the incoming Total Phosphorus (TP) load. G. The stormwater filtration system's cartridges shall have the following



A. Concrete structures shall be designed for H-20 traffic loading and applicable soil loads or as otherwise determined by a Licensed Professiona Engineer. The materials and structural design of the devices shall be per

B. The minimum compressive strength of the concrete shall be 4000 psi. C. Cement shall conform to the requirements for Portland cement of Specification C150.

D. Aggregates shall conform to Specification C33, except that the requirement for gradation shall not apply. E. Reinforcement shall consist of wire conforming to Specification A82 or

Specification A496, of wire fabric conforming to Specification A185 or specification A497, or of bars of Grade 40 steel conforming to Specification A615/A615M. F. The access cover shall be designed for HS20-44 traffic loading and shall

provide a minimum 30 inch clear opening. G. All joints shall be waterproof with wrapped gaskets or sealed with a mastic

H. Any grout used within the system shall meet the ASTM C 1107 "Standard Specification for Packaged Dry, Hydraulic-Cement Grout (Non-Shrink)". Grades A, B and C at a pourable and plastic consistency at 70°F. CRD C

621 "Corps of Engineers Specification For Non-Shrink Grout." 2.4 CONTRACTOR PROVIDED COMPONENTS

ASTM C857 and ASTM C858.

Specifications for all contractor-provided components are minimum requirement If a higher standard is shown on the plans or described in another section of the technical specifications, then the higher standard shall govern.

Sub-Base: Sub-base shall be six-inch minimum of 3/4-inch minus rock, 95% compaction. Compact undisturbed sub-grade materials to 95% of maximum density at +/-2% of optimum moisture content. Unsuitable material below sub-grade shall be replaced to engineer's approval.

B. The minimum compressive strength of the concrete for cast in place structures shall be 4000 psi.

ith ASTM C-109 shall have minimum compressive strength of 6,200 psi. Grout shall not exhibit visible bleeding.

Vault top finish grade shall be even with surrounding finish grade surface Contractor shall grout all inlet and outlet pipes flush with vault interior

Sanded PVC fittings shall be used on all PVC inlet and outlet pipes.

ANTI-FLOTATION BALLAST (Where Required)

Ballast shall be to the dimensions specified by the engineer and noted on the data block. Ballast shall run the entire length of the long side of the vault on both sides. Ballast shall not encase the inlet and/or outlet piping. Provide 12" clearance from outside diameter of pipe.

Remove all excess materials, rocks, roots, or foreign debris, leaving the site in a clean, complete condition approved by the engineer. All filter components shall be free of any foreign materials including concrete. FILTER CARTRIDGES

Filter cartridges shall not be installed until the project site is clean and stabilized or if the inlet and outlet pipes are temporarily blocked off. The project site includes any surface that contributes stormwater runoff to the syFilter system. All impermeable surfaces shall be clean and free of dirt and debris. All catch basins, manholes and pipes shall be free of dirt and

INSTALLATION NOTES

Contractor to strictly follow the approved design and construction specifications. Any substitutions are to be pre-approved by the inspector and design engineer in writing prior to placement of materials.

B. The stormwater filtration system(s) may not be activated until all contributing drainage areas to each facility are stabilized. Construction of the facility shall not proceed without prior authorization of the inspector.

Contact "Miss Utility" at 1-800-257-7777 at least 48 hours prior to the

No "rock dust" can be used for sand.

Installation of the BayFilter System(s) shall be performed per manufacturer's Installation Instructions. Such instructions can be obtained by calling BaySaver Technologies, Inc. at 1.800.229.7283 or by login to www.BaySaver.com.

Contact utility locator to mark any nearby underground utilities and make sure it is safe 2. Reference the site plan and stake out the location of the BayFilterTM manhole/vault.

3. Excavate the hole, providing any sheeting and shoring necessary to comply with all federal, state and local safety regulation

4. Level the subgrade to the proper elevation. Verify the elevation against the

nanhole/vault dimensions, the invert elevations, and the site plans. Adjust the base aggregate, if necessary. . Have the soil bearing capacity verified by a licensed engineer for the required load

earing capacity. On solid subgrade, set the first section of the BayFilter™ 6. Check the level and elevation of the first section to ensure it is correct before adding any

. If additional section(s) are required, add a watertight seal to the first section of the BayFilterTM manhole/vault. Set additional section(s) of the manhole/vault, adding a watertight seal to each joint.

8. Install the trolley system (if applicable). See separate instruction sheet. 9. Install the PVC outlet manifold. Glue all PVC joints with the exception of the BayFilter

cartridge coupling. See separate instruction sheet. 10. Install the PVC outlet pipe in BayFilter^{1M} manhole/vault.

11. Install the inlet pipe to the BayFilter™ manhole/vault After the site has stabilized, remove any accumulated sediment or debris from the vault
and install the Bayfilter Drain Down Modules (DDM) with red mark aligned to the top of the manifold system.

TOOL LIST PVC GLUE AND PRIMER

CRANE / LIFTING MECHANISM TO LOWER THE CARTRIDGES IN THE VAULT (EACH CARTRIDGE WEIGHS 350 LB) SCREWDRIVER OR NUT DRIVER FOR FERNCO COUPLERS SOFT BLOW HAMMER

SAW (IN CASE PVC SCH 40 PIPING LENGTH NEEDS TO BE ADJUSTED).

13. Install the flow disks and the BayFilter** cartridges.

Maintenance of the BayFilter System

Maintenance Procedures

efficiency. The maintenance process comprises the removal and replacement of each BayFilterTM cartridge and drain down module and the cleaning of the vault or manhole with a vacuum truck. BayFilter maintenance should be performed by a BaySaver Technologies, Inc. certified maintenance

The maintenance cycle of the BayFilter^{1M} system will be driven mostly by the actual solids load on the filter. The system should be periodically monitored to be certain it is operating correctly. Since stormwater solids loads can be variable, it is possible that the maintenance cycle could be more or less The BayFilter systems in New Development applications are designed to treat the WQv in 24 hours initially. Later in the cycle these cartridges will flow at a slower rate, and when the WQv does not

drain down within 1/2 40 hours after the storm event, the system must be maintained. When a BayFilter™ system is first installed, it is recommended that it be inspected every six (6) months. When the filter system exhibits flows below design levels the system should be maintained. Filter cartridge replacement should also be considered when sediment levels are at or above the level of the 4 inch manifold system. Please contact the BaySaver Technologies Inc. Engineering Department for maintenance cycle estimations or assistance at 1.800.229.7283.

1. Remove the manhole covers and open all access hatches.

2. Before entering the system make sure the air is safe per OSHA Standards or use a breathing apparatus. Use low O2, high CO, or other applicable warning devices per

3. Using a vacuum truck remove any liquid and sediments that can be removed prior to 4. Using a small lift or the boom of the vacuum truck, remove the used cartridges by lifting

5. Any cartridges that cannot be readily lifted directly out of the vault should be removed Vault (if applicable), 6. When all cartridges and drain down modules are removed, remove the balance of the solids and water; then loosen the stainless clamps on the Ferneo couplings in the pipe manifold; remove the drain pipes as well. Carefully cap the manifold and the Ferneo's

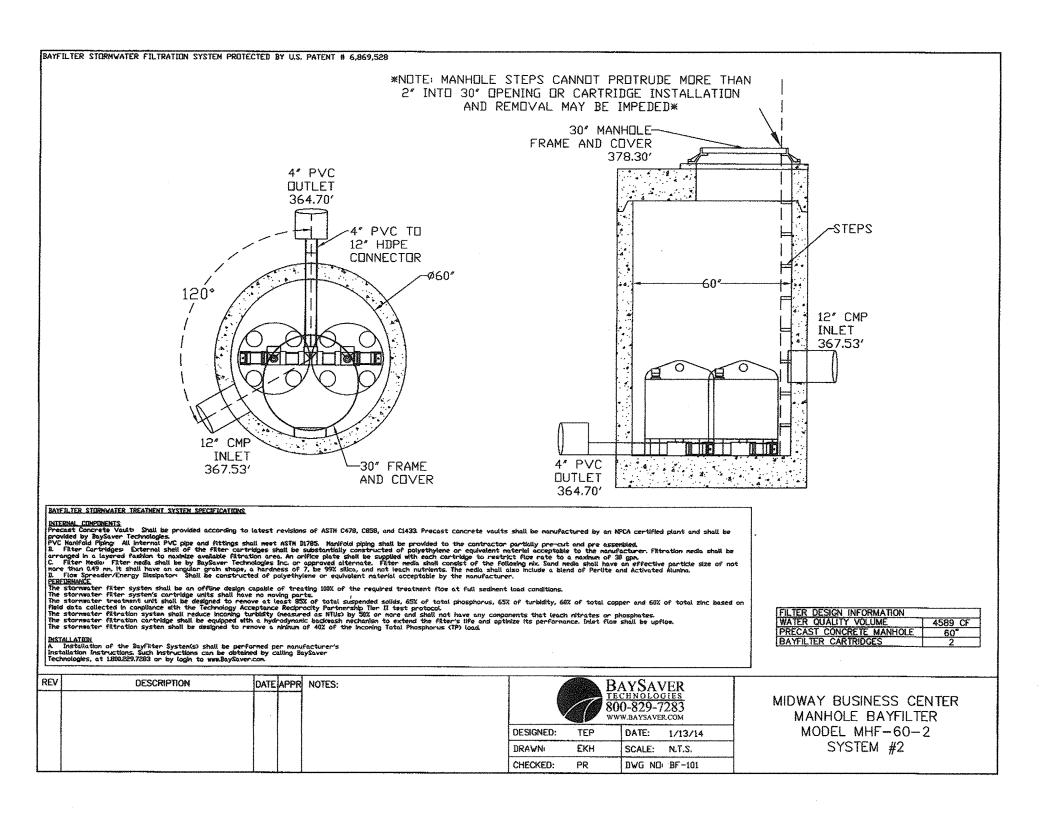
Clean the manifold pipes, inspect, and reinstall. 8. Install the exchange cartridges and close all covers.

and rinse the floor removing the balance of the collected solids.

9. The used cartridges must be sent back to BaySaver Technologies, Inc. for ge/recycling and credit on undamaged units (1030 Deer Hollow Drive, Mt. Airv, MD 21771. Phone: 800 229-7283).

BAYSAVER

Midway Business Center						
Howard County, MD						
December 5, 2013	BayFilter					
Maryland Sizing Calculator	#1	#2	#3			
Treatment Volume (ct)	4,411	4,589	2,179			
Treatment Capacity Per Cartridge (cf)	2,500	2,500	2,500			
Number of Cartridges Required	2	2	1			
Cartridge Type	BFC	BFC	BFC			
Treatment Capacity Provided (cf)	5,000	5,000	2,500			
Vault Required (see below)	60" MH	60" MH	48" MH			



OWNER/DEVELOPER

TSC/JMJ SNOWDEN RIVER SOUTH LLC A MD LLC 8600 SNOWDEN RIVER PKWY, SUITE 207 COLUMBIA MD 21045 410-953-0222



CROWN ELEV 371.62 55

INV IN ELEV 369.72

12" REv Depth

365.<u>95</u>

AS-BUILT CERTIFICATION FOR PSWM I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN WAS CONSTRUCTED AS SHOWN ON THE "AS-BUILT" PLANS AND

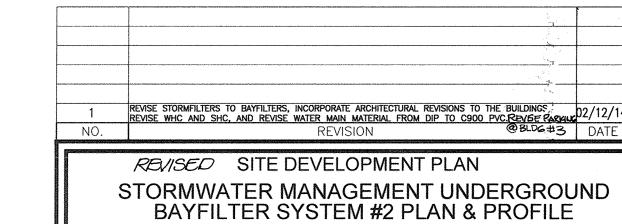
SCALE 1"=30'

COMPLIES WITH THE APPROVED PLANS AND SPECIFICATIONS. I HAVE VERIFIED THAT THE CONTRIBUTING DRAINAGE AREA IS SUFFICIENTLY STABILIZED TO PREVENT CLOGGING OF THE UNDERGROUND SWM FACILITY.

AS-BUILT CERTIFICATION I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN WAS CONSTRUCTED AS SHOWN ON THE " AS-BUILT" PLANS AND MEETS THE APPROVED PLANS AND SPECIFICATIONS.

SIGNATURE P.E. NO. DATE:

CERTIFY MEANS TO STATE OR DECLARE A PROFESSIONAL OPINION BASED UPON ONSITE INSPECTIONS AND MATERIAL TESTS WHICH ARE CONDUCTED DURING CONSTRUCTION? THE ONSITE INSPECTIONS AND MATERIAL TESTS ARE THOSE INSPECTIONS AND TESTS DEEMED SUFFICIENT AND APPROPRIATE COMMONLY ACCEPTED ENGINEERING STANDARDS. CERTIFY DOES NOT MEAN OR IMPLY A GUARANTEE BY THE ENGINEER NOR DOES AN ENGINEER'S CERTIFICATION RELIEVE ANY OTHER PARTY FROM MEETING REQUIREMENTS IMPOSED BY CONTRACT, EMPLOYMENT, OR OTHER MEANS, INCLUDING MEETING COMMONLY ACCEPTED INDUSTRY PRACTICES.

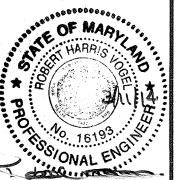


MIDWAY BUSINESS CENTER SNOWDEN RIVER SOUTH SECTION 1, AREA 1 INDUSTRIAL / FLEX SPACE

(PLAT 8795, L.10008/F.485) TAX MAP 42 GRID 10 6TH ELECTION DISTRICT 5) PARCEL 319, PARCEL C HOWARD COUNTY, MARYLAND

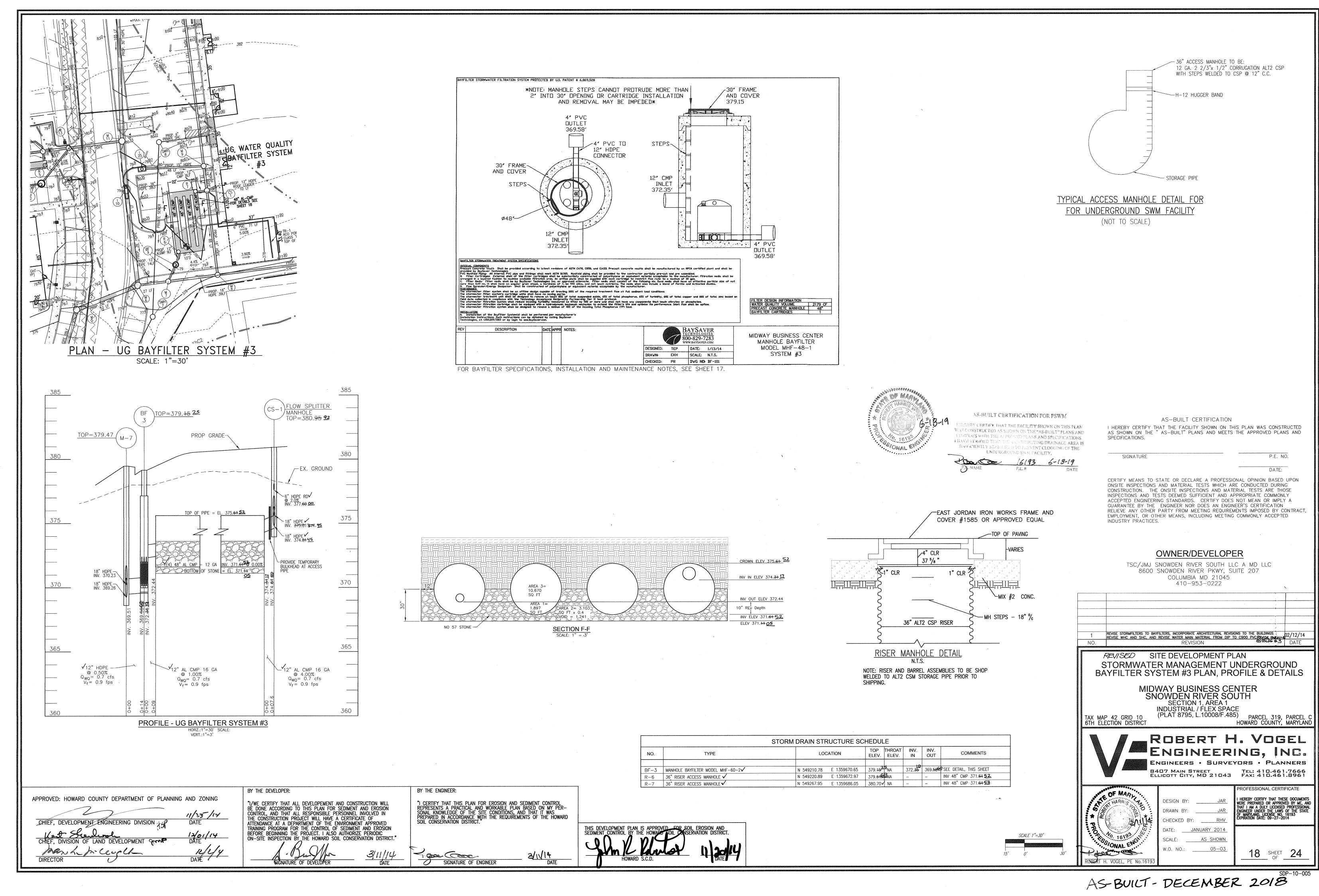


ROBERT H. VOGEL ENGINEERING, INC. Engineers • Surveyors • Planners 8407 MAIN STREET TEL: 410.461.7666 ELLICOTT CITY, MD 21043 FAX: 410.461.8961



CHECKED BY: JANUARY 2014

SHEET 24



MARYLAND 378

STORMWATER MANAGEMENT POND CONSTRUCTION SPECIFICATIONS

CONSTRUCTION SPECIFICATIONS

These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent version.

Site Preparation

Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the toe of the

Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius ground the inlet structure shall be cleared.

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

Material - The fill material shall be taken from approved designated borrow areas. It shall be free of roots, stumps, wood, rubbish, stones greater than 6", frozen or other objectionable materials. Fill material for the center of the embankment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer. Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment.

Placement - Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment.

Compaction - The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not be so wet that water can be saucezed out.

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within +\-2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

Cut Off Trench - The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability.

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed concurrently with the outer shell of the embankment.

Structure Backfill

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operated closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100—200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2.000 ohm-cm. Material shall be placed such that minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sides of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of the structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24" or greater over the structure or pipe. Backfill (flowable fill)zone shall be of the type and quality conforming to that specified for the core of the embankment or other embankment materials.

Pipe Conduits

All pipes shall be circular in cross section.

Corrugated Metal Pipe - All of the following criteria shall apply for corrugated metal pipe:

. Materials - (Polymer Coated steel pipe) - Steel pipes with polymeric coating shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 & M-246 with watertight coupling bands or flanges.

Materials — (Aluminum Coated Steel Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.

POND BOTTOM SOIL CONDITIONS

If broken rock fragments are encountered at finished pond bottom, under cut a minimum of 12" below basin grade and to a horizontal distance of at least 18" beyond each edge of the broken rock and backfill with fine-grained ML or CL soils compacted to a firm condition. This procedure should be performed under the supervision of the project Geotechnical Engineer.

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

PLANNING BOARD of HOWARD COUNTY

CHIEF. DEVELOPMENT ENGINEERING DIVISION , VAD

Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-196 or M-211 with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts

may be used for connections. The pH of the surrounding soils shall be between 4 and 9. 2. Coupling, bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at lease 24 mils in thickness.

3. Connections - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.

All connections shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be rerolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches diameter: flanges on both ends of the pipe with a circular 3/8 inch thick closed cell circular neoprene aasket; and a 12-inch wide hugger type band with o-ring aaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by a 24 inch long annular corrugated band using a minimum of 4(four) rods and lugs, 2 on each connecting pipe end. 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8'inch closed cell qaskets the full width of the flange is also acceptable.

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or a neoprene bead.

4. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support

Backfilling shall conform to "Structure Backfill." 6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete pipe: 1. Materials - Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C-361.

2. Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in the "Structure Backfill" section of this standard. Gravel SUPPLY SOILS AND SAND. GRADE BIORETENTION MATERIALS WITH LIGHT EQUIPMENT SUCH AS A COMPACT LOADER OR bedding is not permitted.

3. Laying pipe - Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser. 4. Backfilling shall conform to "Structure Backfill."

5. Other details (anti-seep collars, valves, etc.) shall be shown on the drawings. Plastic Pipe - The following criteria shall apply for plastic pipe:

. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" -10" inch pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" inch shall meet the requirements of AASHTO M294 Type S.

2. Joints and connections to anti-seep collars shall be completely watertight.

Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

4. Backfilling shall conform to "Structure Backfill."

5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Drainage Diaphragms - When a drainage diaphragm is used, a registered professional engineer will supervise the design and construction inspection.

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3.

Rock Riprap

Concrete

Rock riprop shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction Materials, Section 311

Geotexile shall be placed under all riprap and shall meet requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C. Care of Water during Construction

All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the excavations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall be pumped.

Stabilization All borrow areas shall be graded to provide proper drainage and left I a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

"I CERTIFY THAT THIS PLAN FOR EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS, AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT."

SIGNATURE OF ENGINEER

Erosion and Sediment Control

BY THE DEVELOPER:

I/WE CERTIFY THAT ALL DEVELOPEMENT AND CONSTRUCTION WILL

BE DONE ACCORDING TO THIS PLAN FOR SEDIMENT AND EROSION

CONTROL, AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN

ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED

TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ON—SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT."

4.9.13

THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures.

APPENDIX B.3. CONSTRUCTION SPECIFICATIONS FOR SAND FILTERS BIORETENTION AND OPEN CHANNELS SPECIFICATIONS FOR BIORETENTION

1. MATERIAL SPECIFICATIONS THE ALLOWABLE MATERIALS TO BE USED IN BIORETENTION AREA ARE DETAILED IN TABLE B.3.2

2. PLANTING SOIL THE SOIL SHALL BE A UNIFORM MIX. FREE OF STONES. STUMPS. ROOTS OR OTHER SIMILAR OBJECTS LARGER THAN TWO INCHES. NO OTHER MATERIALS OR SUBSTANCES SHALL BE MIXED OR DUMPED WITHIN THE BIORETENTION AREA THAT MA BE HARMFUL TO PLANT GROWTH, OR PROVE A HINDRANCE TO THE PLANTING OR MAINTENANCE OPERATIONS. THE PLANTING SOIL SHALL BE FREE OF BERMUDA GRASS, QUACKGRASS, JOHNSON GRASS, OR OTHER NOXIOUS WEEDS AS

THE PLANTING SOIL SHALL BE TESTED AND SHALL MEET THE FOLLOWING CRITERIA: PH RANGE 5.2 - 7.0

ORGANIC MATTER 1.5-4 % (BY WEIGHT) MAGNESIUM 35 LB. /AC PHOSPHORUS (PHOSPHATE - PO25) 75

POTASSIUM (POTASH - K20) 85 LB./AC

SPECIFI**ED UND**ER COMAR 15.08.01.05

ALL BIORETENTION AREAS SHALL HAVE A MINIMUM OF ONE TEST. EACH TEST SHALL CONSIST OF BOTH THE STANDARD SOIL TEST FOR PH, PHOSPHORUS, AND POTASSIUM AND ADDITIONAL TEST OF ORGANIC MATTER, AND SOLUBLE SALTS. A TEXTURAL ANALYSIS SHALL BE PERFORMED FOR EACH LOCATION WHERE THE TOP SOIL WAS EXCAVATED. SINCE DIFFERENT LAB CALIBRATE THEIR TESTING EQUIPMENT DIFFERENTLY, ALL TESTING RESULTS SHALL COME FROM THE SAME TESTING FACILITY. SHOULD THE PH FALL OUT OF THE ACCEPTABLE RANGE, IT MAY BE MODIFIED (HIGHER) WITH LIME OR (LOWER) WITH IRON SULFATE PLUS SULFUR.

3. COMPACTION

IR /AC

IT IS VERY IMPORTANT TO MINIMIZE COMPACTION OF BOTH THE BASE OF THE BIORETENTION AREA AND THE REQUIRED BACKFILL. WHEN POSSIBLE, USE HOES TO REMOVE ORIGINAL SOIL. IF BIORETENTION AREAS ARE EXCAVATED USING LOADER, THE CONTRACTOR SHOULD USE WIDE TRACK OR MARSH TRACK EQUIPMENT, OR LIGHT EQUIPMENT WITH TURE YPE TIRE. USE OF EQUIPMENT WITH NARROW TRACKS OR NARROW TIRES, RUBBER TIRES WITH LARGE LUGS, OR HIGH PRESSURE TIRES WILL CAUSE EXCESSIVE COMPACTION RESULTING IN REDUCED INFILTRATION RATES AND IS NOT ACCEPTABLE. COMPACTION WILL SIGNIFICANTLY CONTRIBUTE TO DESIGN FAILURE

COMPACTION CAN BE ALLEVIATED AT THE BASE OF THE BIORETENTION FACILITY BY USING A PRIMARY TILLING OPERATION SUCH AS CHISEL PLOW, RIPPER, OR SUBSOILER. THESE TILLING OPERATIONS ARE TO REFRACTURE THE SOIL PROFILE THROUGH THE 12 INCH COMPACTION ZONE. SUBSTITUTE METHODS MUST BE APPROVED BY THE ENGINEER. ROTOTILLERS TYPICALLY DO NOT TILL DEEP ENOUGH TO REDUCE THE EFFECTS OF COMPACTION FROM HEAVY FOLIPMENT ROTOTILL 2 TO 3 INCHES OF SAND INTO THE BASE OF THE BIORETENION FACILITY BEFORE BACKFILLING THE REQUIRED SAND LAYER. PUMP ANY PONDED WATER BEFORE PREPARING (ROTOTILLING) BASE. WHEN BACKFILLING THE TOPSOIL OVER THE SAND LAYER, FIRST PLACE 3 TO 4 INCHES OF TOPSOIL OVER THE SAND, THEN ROTOTILL THE SAND/TOPSOIL TO CREATE A GRADATION ZONE. BACKFILL THE REMAINDER OF THE TOPSOIL TO FINAL

WHEN BACKFILLING THE BIORETENTION FACILITY, PLACE SOIL IN LIFTS 12" TO 18". DO NOT USE HEAVY EQUIPMENT DOZER/LOADER WITH MARSH TRACKS.

RECOMMENDED PLANT MATERIAL FOR BIORETENTION AREAS CAN BE FOUND IN APPENDIX A, SECTION A.2.3. OF THE 2000 MARYLAND STORMWATER DESIGN MANUAL 5. PLANT INSTALLATION

MULCH SHOULD BE PLACED TO A UNIFORM THICKNESS OF 2" TO 3". SHREDDED HARDWOOD MUCH IS THE ONLY ACCEPTED MULCH. PINE MULCH AND WOOD CHIPS WILL FLOAT AND MOVE TO THE PERIMETER OF THE BIORETENTION AREA DURING A STORM EVENT AND ARE NOT ACCEPTABLE. SHREDDED MULCH MUST BE WELL AGED (6 TO 12 MONTHS) FOR ACCEPTANCE.

ROOT STOCK OF THE PLANT MATERIAL SHALL BE KEPT MOIST DURING TRANSPORT AND ON-SITE STORAGE. THE PLANT ROOT BALL SHOULD BE PLANTED SO 1/8TH OF THE BALL IS ABOVE FINAL GRADE SURFACE. THE DIAMETER OF THE PLANTING PIT SHALL BE AT LEAST SIX INCHES LARGER THAN THE DIAMETER OF THE PLANTING BALL. SET AND TREES SHALL BE BRACED USING 2" BY 2" STAKES ONLY AS NECESSARY AND FOR THE FIRST GROWING SEASON ONLY.

STAKES ARE TO BE EQUALLY SPACED ON THE OUTSIDE OF THE TREE BALL. GRASSES AND LEGUME SEED SHOULD BE DRILLED INTO THE SOIL TO A DEPTH OF AT LEAST ONE INCH. GRASS AND LEGUME PLUGS SHALL BE PLANTED FOLLOWING THE NON-GRASS GROUND COVER PLNTING SPECIFICATIONS. THE TOPSOIL SPECIFICATIONS PROVIDE ENOUGH ORGANIC MATERIAL TO ADEQUATELY SUPPLY NUTRIENTS FROM NATURAL CYCLING. THE PRIMARY FUNCTION OF THE BIORETENTION STRUCTURE IS TO IMPROVE WATER QUALITY. ADDING FERTILIZERS DEFEATS, OR AT A MINIMUM, IMPEDES THIS GOAL. ONLY ADD FERTILIZER IF WOOD CHIPS OR MULCH ARE USED TO AMEND THE SOIL ROTOTILL UREA FERTILIZER AT A RATE OF 2 POUNDS OF NITROGEN PER 1000 SQUARE

6. UNDERDRAMS

UNDERDRAINS ARE TO BE PLACED ON A 3'-O" WIDE SECTION FILTER CLOTH. PIPE IS PLACED NEXT, FOLLOWED BY THE GRAVEL BEDDING. THE ENDS OF UNDERDRAIN PIPES NOT TERMINATING IN AN OBSERVATION WELL SHALL BE CAPPED. THE MAIN COLLECTOR PIPE FOR UNDERDRAIN SYSTEMS SHALL BE CONSTRUCTED AT A MINIMUM SLOPE OF 0.5%. OBSERVATION WELL AND/OR CLEAN-OUT PIPES MUST BE PROVIDED (ONE MINIMUM PER EVERY 1000 SQUARE FEET OF SURFACE AREA).

THE BIORETENTION FACILITY MAY NOT BE CONSTRUCTED UNTIL ALL CONTRIBUTING DRAINAGE AREA HAS BEEN STABILIZED.

UNDERGROUND SAND FILTER CONSTRUCTION SPECIFICATIONS

I. PROVIDE MANHOLE AND/OR GRATES TO ALL UNDERGROUND AND BELOW GRADE STRUCTURES. MANHOLES SHALL BE IN COMPLIANCE WITH STANDARD SPECIFICATIONS FOR EACH COUNTY, BUT DIAMETERS SHOULD BE 30" MINIMUM (TO COMPLY WITH OSHA CONFINED SPACE REQUIREMENTS). ALUMINUM AND STEEL LOUVERED DOORS ARE ALSO ACCEPTABLE. TEN INCH WIDE (MINIMUM) MANHOLE STEPS (12"O.C.) SHALL BE CAST IN PLACE OR DRILLED AND MORTARED INTO THE WALL BELOW EACH MANHOLE A 5' MINIMUM HEIGHT CLEARANCE (FROM THE TOP OF THE SAND LAYER TO THE BOTTOM OF THE UPPER/SURFACE SLAB) IS REQUIRED FOR ALL PERMANENT LINDERGROUND STRUCTURES. LIFT RINGS ARE TO BE SUPPLIED TO REMOVE/REPLACE TOP SLABS ON PRE-FABRICATED STRUCTURES. MANHOLE COVERS SHOULD ALLOW FOR PROPER

2. UNDERGROUND SANDFILTERS SHOULD BE CONSTRUCTED WITH A GATE VALVE LOCATED JUST ABOVE THE TOP OF THE FILTER BED FOR DEWATERING IN THE EVENT THAT CLOGGING OCCURS

3. UNDERGROUND SAND BEDS SHALL BE PROTECTED FROM TRASH ACCUMULATION BY A WIDE MESH GEOTEXTILE SCREEN TO BE PLACED ON THE SURFACE OF THE SAND BED; SCREEN IS TO BE ROLLED UP, REMOVED, CLEANED AND RE-INSTALLED DURING MAINTENANCE OPERATIONS.

DEWATERING STRATEGY

Dewatering refers to the act of removing and discharging water from excavated areas on construction sites or from sediment traps or basins on construction sites. Standards and specifications for dewatering

These standards apply to removal and discharge of water from any excavated area or sediment trap or basin at any construction site. Given the unique conditions at any particular construction site, any or all of the practices may apply. Regardless of the applicability of the practices listed herein, operators are required to use acceptable procedures for maintenance and dewatering. In all cases, every effort shall be made to eliminate sediment pollution associated with dewatering

Designers shall specify the preferred procedures for dewatering on plans. In particular, designers should dentify procedures for dewatering sediment traps and basins prior to elimination of the last sediment control facility on the site or prior to conversion of sediment control facilities to stormwater management facilities. Recommended procedures shall be consistent with these standards. Atypical site conditions may require innovative dewatering designs. Dewatering measures not referenced in this standard may be used with the consent of the approval authority.

Dewatering of Excavated Areas A. Designers shall specify on plans, and in sequences of construction included on plans, practices for dewatering of excavated areas. Plan reviewers shall check to see that procedures for dewatering are included on plans.

B. In all cases, water removed from excavated areas shall be discharged such that it shall pass through a sediment control device prior to entering receiving waters. Sediment control devices include sediment traps and basins, in addition to the practices in this section.

Approved Practices for Dewatering of Excavated Areas Pumping of water to an existing sediment basin or trap in which the entire volume of water from the area to be dewatered can be contained without discharge to receiving waters. 2. Pumping of water to an existing sediment basin or trap such that the entire volume of water from the area to be dewatered can be managed without exceeding the design outflow from the sediment

control structure. 3. Removable Pumping Station ? Standards and specifications for Removable Pumping Station are

4. Use of a Sump Pit ? Standards and specifications for a sump pit are on Detail 20B. 5. Sediment Tank? Standards and specifications for a sump pit are on Detail 21. Dewatering of Sediment Traps and Basins

Designers shall specify on plans, and in sequences of construction included on plans, the practices for dewatering of traps and basins. Plan reviewers shall check to see that procedures for dewatering to be used are included on plans. In all cases, water removed from traps and basins shall be discharged so that it passes through a sediment control device prior to entering receiving waters. Approved Practices for Dewatering of Traps and Basins

Removable pumping station.

Use of a Sump Pit.

Use of a floating suction hose to pump the cleaner water from the top of the pond. As the cleaner water is pumped the suction hose will lower and eventually encounter sediment laden water. When this happens the pumping operation will cease. Provisions shall be made to filter water

OPERATION AND MAINTENANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED UNDERGROUND STORMWATER FILTRATION SYSTEM (F-2)

MATERIALS

PLANTINGS

PLANTING SOIL

(2.5' TO 4' DEEP)

PEA GRAVEL DIAPHRAGM

AND CURTAIN DRAIN

GEOTEXTILE

UNDERDRAIN GRAVEL

UNDERDRAIN PIPING

POURED IN PLACE

CONCRETE (IF REQUIRED)

(1' DEEP)

SPECIFICATIONS

SEE APPENDIX A, TABLE A.4

SAND 35-60%

SILT 30-55%

CLAY 0-5%

SHREDDED HARDWOOD

PEA GRAVEL: ASTM-D-448

ORNAMENTAL STONE: WASHED

SIZE (ASTM-D-4751), GRAB

TENSILÈ STRENGTH (ÁSTM-D-

4632), PUNCTURE RESISTANCE

(ASTM-D-4833)

F 758, TYPE PS 28 OR

AASHTO M-278

MSHA MIX NO. 3; F'C=3500

PSI@28 DAYS, NORMAL WEIGHT,

AIR-ENTRAINED:REINFORCING TO

MEET ASTM-615-60

AASHTO-M-6 OR ASTM-C-33

AASHTO M-43

CLASS

MATERIAL

UNDERDRAIN GRAVEL

COLLECTION PIPE

OBSERVATION WELL /

CLEANOUT

GEOTEXTILE FABRIC (IF

REQUIRED)

UNDERDRAIN PIPING

CONCRETE (CAST-IN

PLACE)

CONCRETE (PRE-CAST)

NON-REBAR STEEL

TOPSOIL

- APPARENT OPENING

1. THE SEDIMENT CHAMBER OUTLET DEVICES SHALL BE CLEANED AND/OR

REPAIRED WHEN DRAW-DOWN TIMES WITHIN THE CHAMBER EXCEED 36 HOURS. 2. DEBRIS AND LITTER SHALL BE REMOVED AS NECESSARY TO ENSURE PROPER OPERATION OF THE SYSTEM.

3. SEDIMENT SHALL BE CLEANED OUT OF THE SEDIMENTATION CHAMBER WHEN IT ACCUMULATES TO A DEPTH OF 6 INCHES. VEGETATION WITHIN THE SEDIMENT CHAMBER SHALL BE LIMITED TO A HEIGHT OF 18 INCHES. 4. WHEN WATER PONDS ON THE SURFACE OF THE FILTER BED FOR MORE THAN

72 HOURS, THE TOP FEW INCHES OF DISCOLORED MATERIAL SHALL BE REPLACED WITH FRESH MATERIAL. PROPER CLEANING AND DISPOSAL OF THE REMOVED MATERIALS AND LIQUID MUST BE FOLLOWED BY THE OWNER. 5. A LOG BOOK SHALL BE MAINTAINED TO DETERMINE THE RATE AT WHICH THE

6. THE MAINTENANCE LOG BOOK SHALL BE AVAILABLE TO HOWARD COUNTY FOR INSPECTION TO ENSURE COMPLIANCE WITH OPERATION AND MAINTENANCE

FACILITY DRAINS.

NEEDED.

7. ONCE THE PERFORMANCE CHARACTERISTICS OF THE INFILTRATION SYSTEM HAVE BEEN VERIFIED. THE MONITORING SCHEDULF CAN BE REDUCED TO AN ANNUAL BASIS UNLESS THE PERFORMANCE DATA INDICATES THAT A MORE FREQUENT SCHEDULE IS REQUIRED.

SYSTEMS (F-1, F-4, AND F-5)

OPERATION AND MAINTENANCE SCHEDULE FOR PRIVATELY

PER YEAR, WHEN VEGETATION REACHES 18" IN HEIGHT OR AS NEEDED.

OWNED AND MAINTAINED SURFACE STORMWATER FILTRATION

1. THE STORMWATER WETLAND FACILITY SHALL BE INSPECTED ANNUALLY AND AFTER MAJOR

STORMS. INSPECTIONS SHALL BE PERFORMED DURING WET WEATHER TO DETERMINE IF THE

2. THE TOP AND SIDE SLOPES OF THE EMBANKMENT SHALL BE MOWED A MINIMUM OF ONCE

3. FILTERS THAT HAVE A GRASS COVER SHALL BE MOWED A MINIMUM OF THREE (3) TIMES

PER GROWING SEASON TO MAINTAIN A MAXIMUM GRASS HEIGHT OF LESS THAN 12 INCHES.

4. DEBRIS AND LITTER SHALL BE REMOVED DURING REGULAR MOWING OPERATIONS AND AS

5. VISIBLE SIGNS OF EROSION IN THE FACILITY SHALL BE REPAIRED AS SOON AS IT IS

7. WHEN WATER PONDS ON THE SURFACE OF THE FILTER BED FOR MORE THAN 72 HOURS,

PROPER CLEANING AND DISPOSAL OF THE REMOVED MATERIALS AND LIQUID MUST BE

THE TOP FEW INCHES OF DISCOLORED MATERIAL SHALL BE REPLACED WITH FRESH MATERIAL.

8. A LOGBOOK SHALL BE MAINTAINED TO DETERMINE THE RATE AT WHICH THE FACILITY DRAINS.

9. THE MAINTENANCE LOGBOOK SHALL BE AVAILABLE TO HOWARD COUNTY FOR INSPECTION TO

10. ONCE THE PERFORMANCE CHARACTERISTICS OF THE INFILTRATION SYSTEM HAVE-BEEN

PERFORMANCE DATA INDICATES THAT A MORE FREQUENT SCHEDULE IS REQUIRED.

VERIFIED, THE MONITORING SCHEDULE CAN BE REDUCED TO AN ANNUAL BASIS UNLESS THE

6. REMOVE SILT WHEN IT EXCEEDS FOUR (4) INCHES DEEP IN THE FOREBAY.

INSURE COMPLIANCE WITH OPERATION AND MAINTENANCE CRITERIA.

CLAY 0% OPERATION AND MAINTENANCE SCHEDULE FOR **BIO-RETENTION AREAS**

. ANNUAL MAINTENANCE OF PLANT MATERIAL, MULCH LAYER AND SOIL LAYER IS REQUIRED. MAINTENANCE OF MULCH AND SOIL IS LIMITED TO CORRECTING AREAS OF EROSION OR WASH OUT. ANY MULCH REPLACEMENT SHALL BE DONE IN THE SPRING. PLANT MATERIAL SHALL BE CHECKED FOR DISEASE AND INSECT INFESTATION AND MAINTENANCE WILL ADDRESS DEAD

TABLE B.3.2 MATERIALS SPECIFICATION FOR BIORETENTION/RAINGARDEN

SIZE

N/A

N/A

PEA GRAVEL: NO. 6

STONE: 2" TO 5"

0.375" TO 0.75"

4" TO 6" RIGID SCHEDULE

40 PVC OR SDR35

N/A

0.02" TO 0.04"

SPECIFICATIONS / TEST METHOD

CLEAN AASHTO-M-6 OR

AASHTO-M-43

ASTM-D-1785 OR ASTM-D-3034

FITTINGS SHALL MEET ASTM-D-2729

M 278 OR F 758,

ASTM-D-4833 (PUNCTURI

STRENGTH - 125 lb.)

ASTM-D-4632 (TENSILE

STRENGTH - 300 lb.)

F 758. TYPE PS 28

OR AASHTO-M-278

MSHA STANDARDS AND SPECS.

= 3500 psi, NORMAL WEIGHT,

SECTION 902, MIX NO 3, F'c

AIR-ENTRAINED: RE-INFORCING

TO MEET ASTM-615-60

PER PRE-CAST MANUFACTURER

ASTM A-36

SAND 35-60%

SILT 30-55%

MATERIAL AND PRUNING.

10 AS-BUILT INFORMATION ON THIS SHEET

TYPE PS 28

ASTM-C-33 CONCRETE SAND

TABLE B.3.1 MATERIAL SPECIFICATIONS FOR SAND FILTERS

0..02" TO 0.04"

0.375" TO 0.75"

RIGID SCHEDULE

40 OR SDR 35

6" RIGID SCHEDULE

40 OR SDR 35

0.08" THICK

QUIVALENT OPENING

SIZE OF #80 SIEVE

4"-6" RIGID

SCHEDULE 40 PVC

OR SDR35

N/A

N/A

2. SCHEDULE OF PLANT INSPECTION WILL BE TWICE A YEAR IN SPRING AND FALL. THIS INSPECTION WILL INCLUDE REMOVAL OF DEAD AND DISEASED VEGETATION CONSIDERED BEYOND TREATMENT, TREATMENT OF ALL DEFICIENT

3. MULCH SHALL BE INSPECTED EACH SPRING. REMOVE PREVIOUS MULCH LAYER BEFORE APPLYING NEW LAYER ONCE EVERY 2 TO 3 YEARS.

4. SOIL EROSION TO BE ADDRESSED ON AN AS NEEDED WITH A MINIMUM OF ONCE PER MONTH AND AFTER HEAVY STORM EVENTS.

OWNER/DEVELOPER

TSC/JMJ SNOWDEN RIVER SOUTH LLC A MD LLC 8600 SNOWDEN RIVER PKWY, SUITE 207 COLUMBIA MD 21045

410-953-0222

NOTES

PLANTINGS ARE SITE SPECIFIC

USDA SOIL TYPES LOAMY SAND, SANDY LOAM OR LOAM

AGED 6 MONTHS, MINIMUM

FOR USE BENEATH UNDERDRAINS ONLY

3/8" PERF. @ 6" ON CENTER, 4 HOLES PER ROW: MINIMUM OF 3" OF

GRAVEL OVER IPES: NOT NECESSARY UNDERNEATH PIPES

ON-SITE TESTING OF POURED-IN-PLACE CONCRETE REQUIRED:

OR PRE-CAST) NOT USING PREVIOUSLY APPROVED SATE OR LOCAL

STANDARDS REQUIRED DESIGN DRAWINGS SEALED AND APPROVED BY A

DESIGN TO INCLUDE MEETING ACI CODE 350.R/89; VERTICAL LOADING

(H-10 OR H-20); ALLOWABLE HORIZONTAL LOADING (BASED ON SOIL

PRESSURES); AND ANALYSIS OF POTENTIAL CRACKING

SAND SUBSTITUTIONS SUCH AS DIABASE AND GRAYSTONE #10 ARE NOT

ACCEPTABLE. NO CALCIUM CARBONATED OR DOLOMITIC SAND

ARE ACCEPTABLE. NO "ROCK DUST" CAN BE USED FOR SAND.

SAND SUBSTITUTIONS SUCH AS DIABASE AND GRAYSTONE #10

ARE NOT ACCEPTABLE. NO CALCIUM CARBONATED OR DOLOMITIC

USED FOR SAND

SAND SUBSTITUTIONS ARE ACCEPTABLE. NO "ROCK DUST" CAN BE

PERFORATIONS TO BE 3/8 INCH DIAMETER ROWS 6" O/C

MINIMUM 4-HOLES / ROW

PERFORATIONS TO BE 3/8 INCH DIAMETER ROWS 6" O/C

MINIMUM 4-HOLES / ROW

MUST MAINTAIN 125 gpm PER SQ. FT. FLOW RATE.

NOTE: A 4" PEA GRAVEL LAYER MAY BE SUBSTITUTED FOR

GEOTEXTILES MEANT TO "SEPARATE" SAND FILTER LAYERS.

3/8" PERF. @ 6" ON CENTER, 4 HOLES PER ROW:

MINIMUM OF 3" GRAVEL OVER PIPES; NOT NECESSARY

UNDERNEATH PIPES.

ON-SITE TESTING OF POURED-IN-PLACE CONCRETE REQUIRED:

28 DAY STRENGTH AND SLUMP TEST: ALL CONCRETE DESIGN

(CAST-IN-PLACE OR PRE-CAST) NOT USING PREVIOUSLY

APPROVED STATE OR LOCAL STANDARDS REQUIRES DESIGN

STRUCTURAL ENGINEER LICENSED IN THE STATE OF MARYLAND.

SEE ABOVE NOTE

STRUCTURAL STEEL TO BE HOT-DIPPED GALVANIZED ASTM-A-123

THE SOIL SHALL BE A UNIFORM MIX. FRRE ODF STONES, STUMPS

ROOTS OR OTHER SIMILAR OBJECTS LARGER THAN 1

AS-BUILT CERTIFICATION FOR PSWM

I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLANS

WAS CONSTRUCTED AS SHOWN ON THE "AS-BUILT" PLANS AND

COMPLIES WITH THE APPROVED PLANS AND SPECIFICATIONS.

I HAVE VERIFIED THAT THE CONTRIBUTING DRAINAGE AREA IS

SUFFICIENTLY STABILIZED TO PREVENT CLOGGING OF THE

UNDERGROUND SWM FACILITY.

DRAWINGS SEALED AND APPROVED BY A PROFESSIONAL

NOTES

28 DAY STRENGTH AND SLUM TEST; ALL CONCRETE DESIGN (CAST-IN-PLACE

ROFESSIONAL STRUCTURAL ENGINEER LICENSED IN THE SATE OF MARYLAND

REVISION DATE

> SITE DEVELOPMENT PLAN STORMWATER MANAGEMENT SPECIFICATIONS AND NOTES

MIDWAY BUSINESS CENTER SNOWDEN RIVER SOUTH SECTION 1, AREA 1 INDUSTRIAL / FLEX SPACE (PLAT 8795, L.10008/F.485)

TAX MAP 42 GRID 10 6TH ELECTION DISTRICT

Robert H. Vogel Engineering, Inc. ENGINEERS · SURVEYORS · PLANNERS

8407 MAIN STREET ELLICOTT CITY, MD 21043TEL: 410.461.7666
FAX: 410.461.8961

OF MAR W.O. NO.:

ROBERT H. VOGEL, PE No.1619

DRAWN BY: JAR CHECKED BY: RHV MARCH 2013

AS SHOWN

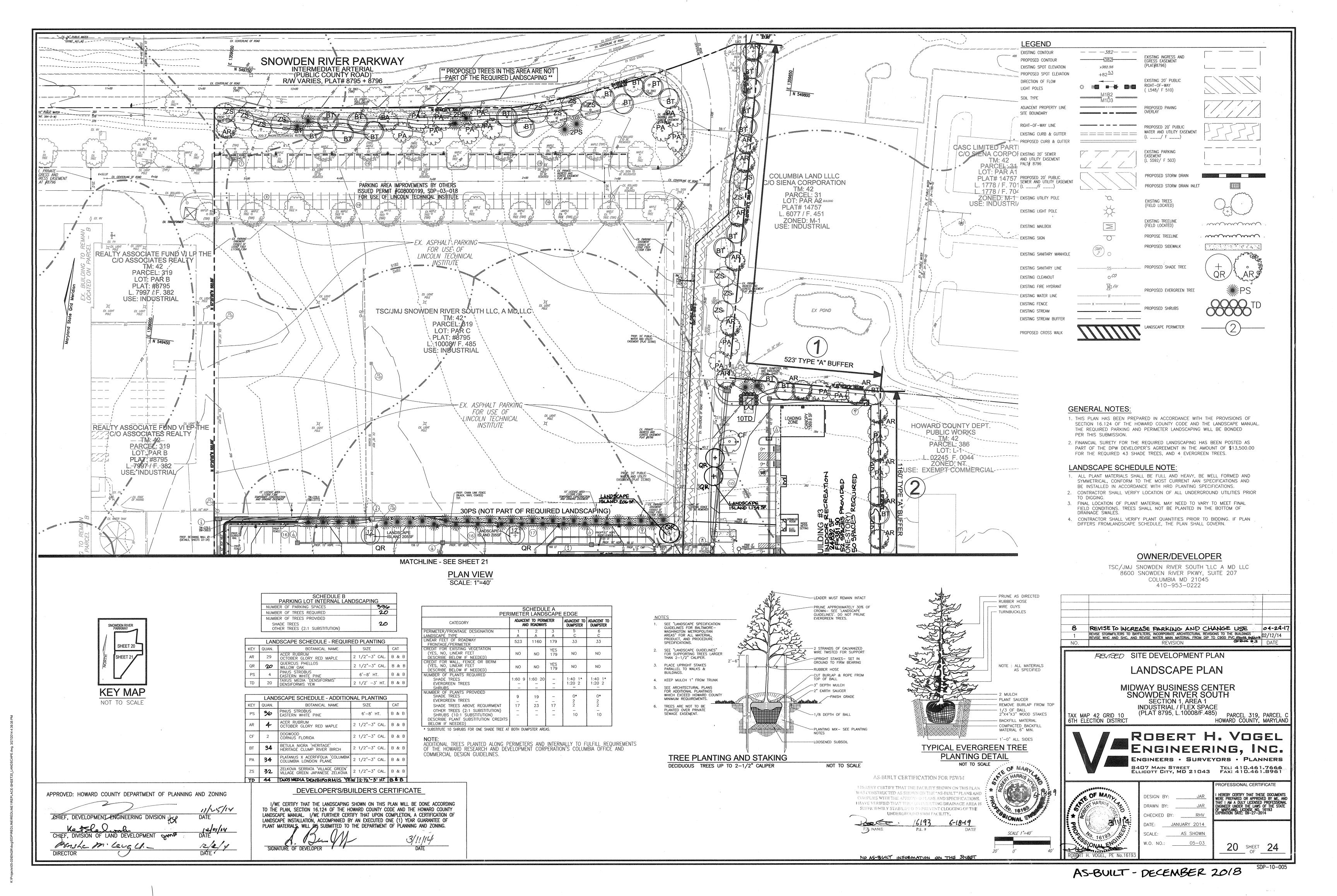
05-03

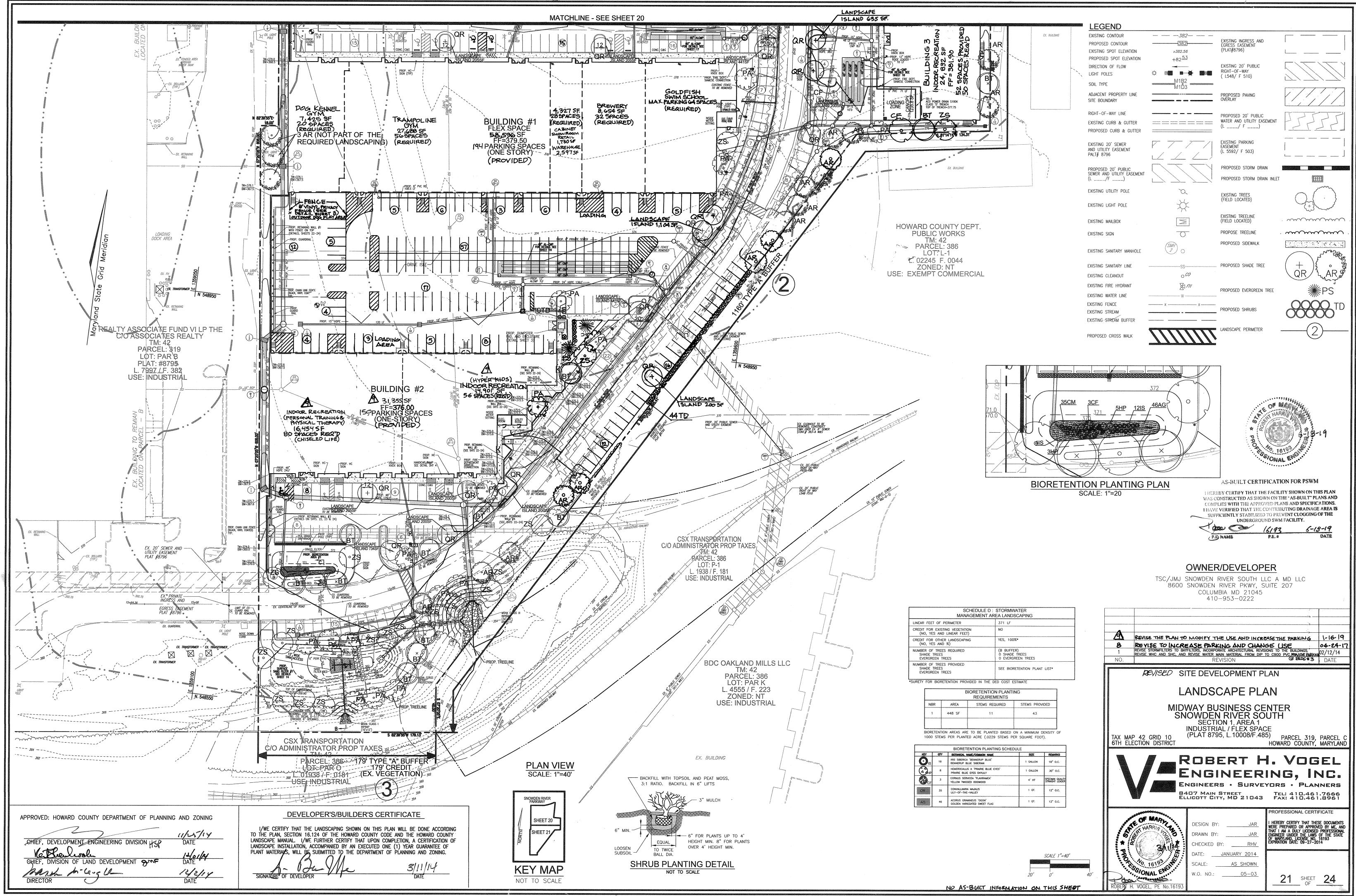
3407 MAIN STREET

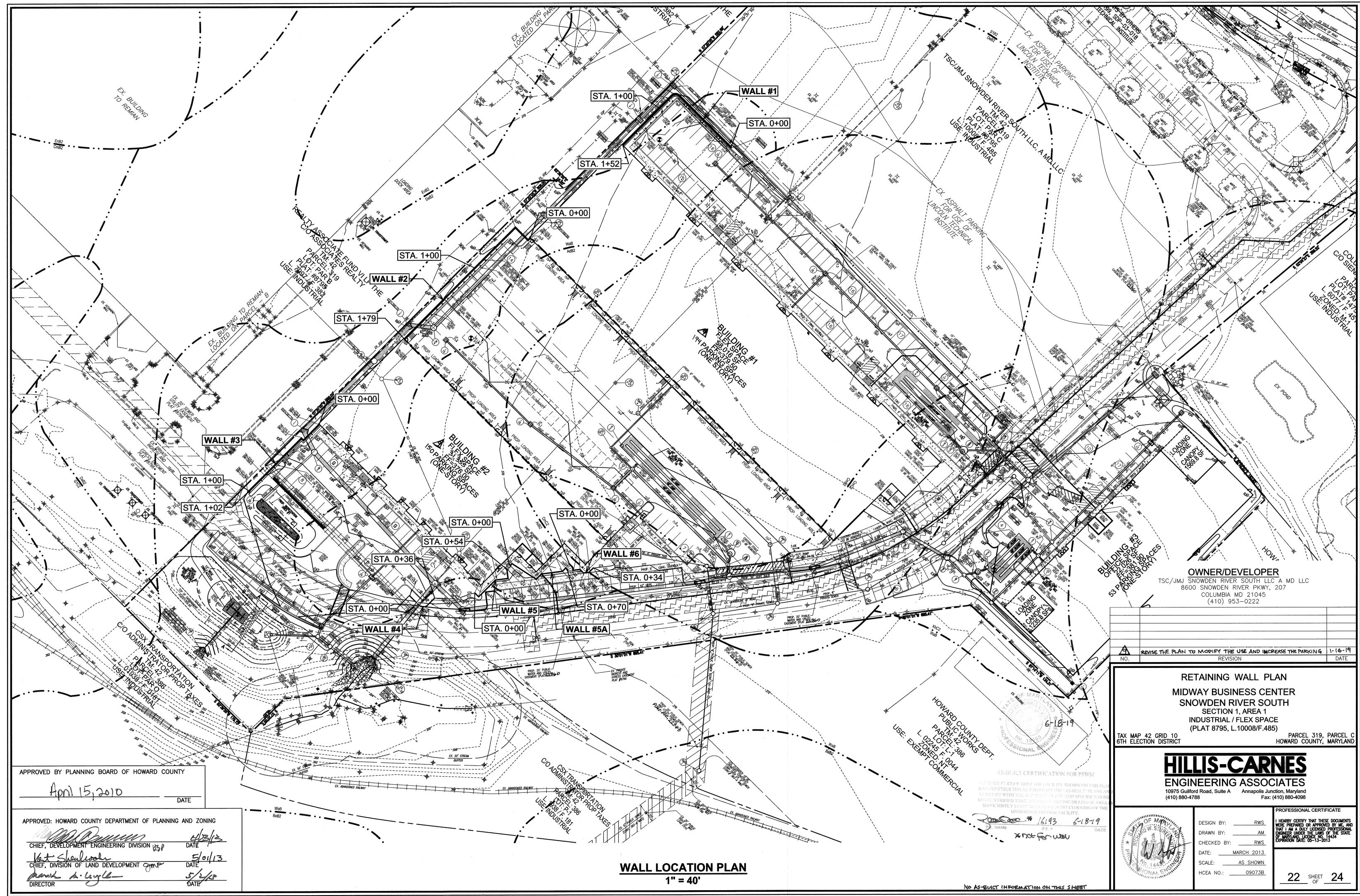
ROFESSIONAL CERTIFICATE I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 16193 EXPIRATION DATE: 09-27-2014

PARCEL 319, PARCEL

HOWARD COUNTY, MARYLAND







SPECIFICATIONS

KEYSTONE MODULAR CONCRETE BLOCK RETAINING WALL

PART 1: GENERAL

1.01 Description

- A. Work shall consist of furnishing and construction of a KEYSTONE Retaining Wall System in accordance with these specifications and in reasonably close conformity with the lines, grades, design, and dimensions shown on
- B. Work includes preparing foundation soil, furnishing and installing leveling pad, unit drainage fill and backfill to the lines and grades shown on the construction drawings. C. Work includes furnishing and installing geogrid soil reinforcement of the type, size, location, and lengths designated on the construction drawings.
- 1.02 Delivery, Storage and Handling
- A. Contractor shall check all materials upon delivery to assure that the proper type, grade, color, and certification has been received. B. Contractor shall protect all materials from damage due to
- job site conditions and in accordance with manufacturer's recommendations. Damaged materials shall not be incorporated into the work.

PART 2: PRODUCTS

- 2.01 Modular Concrete Retaining Wall Units
- A. Modular concrete units shall conform to the following architectural requirements: face color - concrete gray - standard manufacturers color may be specified by the Owner. face finish - sculptured rock face in angular tri-planer configuration. Other face finishes will not be allowed without written approval of Owner. bond configuration - running with bonds nominally located at midpoint vertically adjacent units, in both straight and
- curved alignments. exposed surfaces of units shall be free of chips, cracks or other imperfections when viewed from a distance of 10
- feet under diffused lighting. B. Modular concrete materials shall conform to the requirements of ASTM C1372 - Standard Specifications
- for Segmental Retaining Wall Units. C. Modular concrete units shall conform to the following structural and geometric requirements measured in accordance with appropriate references: compressive strength = 3000 psi minimum;
- absorption = 8 % maximum (6% in northern states) for standard weight aggregates; dimensional tolerances = $\pm 1/8$ " from nominal unit dimensions not including rough split face, ±1/16" unit
- height top and bottom planes; unit size - 8" (H) x 18" (W) x 12" (D) minimum; unit weight - 75 lbs/unit minimum for standard weight

- inter-unit shear strength 1000 plf minimum at 2 psi normal pressure; geogrid/unit peak connection strength - 1000 plf minimum
- at 2 psi normal force. D. Modular concrete units shall conform to the following constructability requirements: vertical setback = 1/8"± per course (near vertical) or 1"+ per course per the design; alignment and grid positioning mechanism - fiberglass pins, two per unit minimum; maximum horizontal gap between erected units shall be -
- 2.02 Shear Connectors

1/2 inch.

- A. Shear connectors shall be 1/2 inch diameter thermoset isopthalic polyester resin-protruded fiberglass reinforcement rods or equivalent to provide connection between vertically and horizontally adjacent units. Strength of shear connectors between vertical adjacent units shall be applicable over a design temperature of 10 degrees F to + 100 degrees F.
- **B.** Shear connectors shall be capable of holding the geogrid in the proper design position during grid pre-tensioning and backfilling.

2.03 Base Leveling Pad Material A. Material shall consist of a compacted #57 crushed stone

- base as shown on the construction drawings.
- 2.04 Unit Drainage Fill A. Unit drainage fill shall consist of #57crushed stone

B. Material can be site excavated soils where the above

requirements can be met. Unsuitable soils for backfill (high

plastic clays or organic soils) shall not be used in the

2.05 Reinforced Backfill

3/4 inch

No. 40

No. 200

reinforced soil mass.

2.06 Geogrid Soil Reinforcement

A. Reinforced backfill shall type SM, be free of debris and meet the following gradation tested in accordance with ASTM D-422 and meet other properties shown on the Percent Passing Sieve Size 2 inch

100-75

- Plasticity Index (PI) <10 and Liquid Limit <40 per ASTM

 - A. Geogrid shall be oriented with the highest strength axis perpendicular to the wall alignment. B. Geogrid reinforcement shall be placed at the strengths, lengths, and elevations shown on the construction design
 - C. The geogrid shall be laid horizontally on compacted backfill and attached to the modular wall units. Place the next course of modular concrete units over the geogrid. The geogrid shall be pulled taut, and anchored prior to

#57 CRUSHED

STONE BASE

A. Geosynthetic reinforcement shall consist of geogrids manufactured specifically for soil reinforcement applications and shall be manufactured from high tenacity

2.07 Drainage Pipe A. The drainage pipe shall be perforated corrugated HDPE pipe manufactured in accordance with ASTM D-1248.

PART 3 EXECUTION

- 3.01 Excavation A. Contractor shall excavate to the lines and grades shown on the construction drawings. Owner's representative shall be responsible for inspecting and approving the excavation prior to placement of leveling material or fill
- 3.02 Base Leveling Pad A. Leveling pad material shall be placed to the lines and grades shown on the construction drawings, to a minimum
- thickness of 6 inches and extend laterally a minimum of 6" in front and behind the modular wall unit. B. Leveling pad shall be prepared to insure full contact to the
- base surface of the concrete units. 3.03 Modular Unit Installation
- A. First course of units shall be placed on the leveling pad at the appropriate line and grade. Alignment and level shall be checked in all directions and insure that all units are in full contact with the base and properly seated.
- B. Place the front of units side-by-side. Do not leave gaps between adjacent units. Layout of corners and curves shall be in accordance with manufacturer's recommendations.
- C. Install shear/connecting devices per manufacturer's recommendations. D. Place and compact drainage fill within and behind wall
- units. Place and compact backfill soil behind drainage fill. Follow wall erection and drainage fill closely with structure E. Maximum stacked vertical height of wall units, prior to unit
- drainage fill and backfill placement and compaction, shall not exceed three courses
- 3.04 Structural Geogrid Installation
- drawings or as directed by the Engineer.

- backfill placement on the geogrid. **D.** Geogrid reinforcements shall be continuous throughout their embedment lengths and placed side-by-side to provide 100% coverage at each level. Spliced connections between shorter pieces of geogrid or gaps between adjacent pieces of geogrid are not permitted.
- 3.05 Reinforced Backfill Placement A. Reinforced backfill shall be placed, spread, and
- compacted in such a manner that minimizes the development of slack in the geogrid and installation
- B. Reinforced backfill shall be placed and compacted in lifts not to exceed 6 inches where hand compaction is used, or 8 - 10 inches where heavy compaction equipment is used. Lift thickness shall be decreased to achieve the required density as required. C. Reinforced backfill shall be compacted to 95% of the
- maximum density as determined by ASTM D698. The moisture content of the backfill material prior to and during compaction shall be uniformly distributed throughout each layer and shall be + 3% to - 3% of optimum.
- D. Only lightweight hand-operated equipment shall be allowed within 3 feet from the tail of the modular concrete
- E. Tracked construction equipment shall not be operated directly upon the geogrid reinforcement. A minimum fill thickness of 6 inches is required prior to operation of tracked vehicles over the geogrid. Tracked vehicle turning should be kept to a minimum to prevent tracks from displacing the fill and damaging the geogrid. F. Rubber tired equipment may pass over geogrid
- braking and sharp turning shall be avoided. G. At the end of each day's operation, the Contractor shall slope the last lift of reinforced backfill away from the wall units to direct runoff away from wall face. The Contractor shall not allow surface runoff from adjacent areas to enter

reinforcement at slow speeds, less than 10 MPH. Sudden

3.06 Cap Installation A. Cap units shall be glued to underlying units with an all-weather adhesive recommended by the manufacturer.

the wall construction site.

- 3.07 Field Quality Control A. The Owner shall engage inspection and testing services, including independent laboratories, to provide quality
- assurance and testing services during construction. B. As a minimum, quality assurance testing should include foundation soil inspection, soil and backfill testing, verification of design parameters, and observation of construction for general compliance with design drawings and specifications.

CONTINUOUS FILTER

FABRIC OVER #57 STONE

- PAVEMENT

- MIRAGRID

GEOGRID

ABOVE TOP GEOGRID

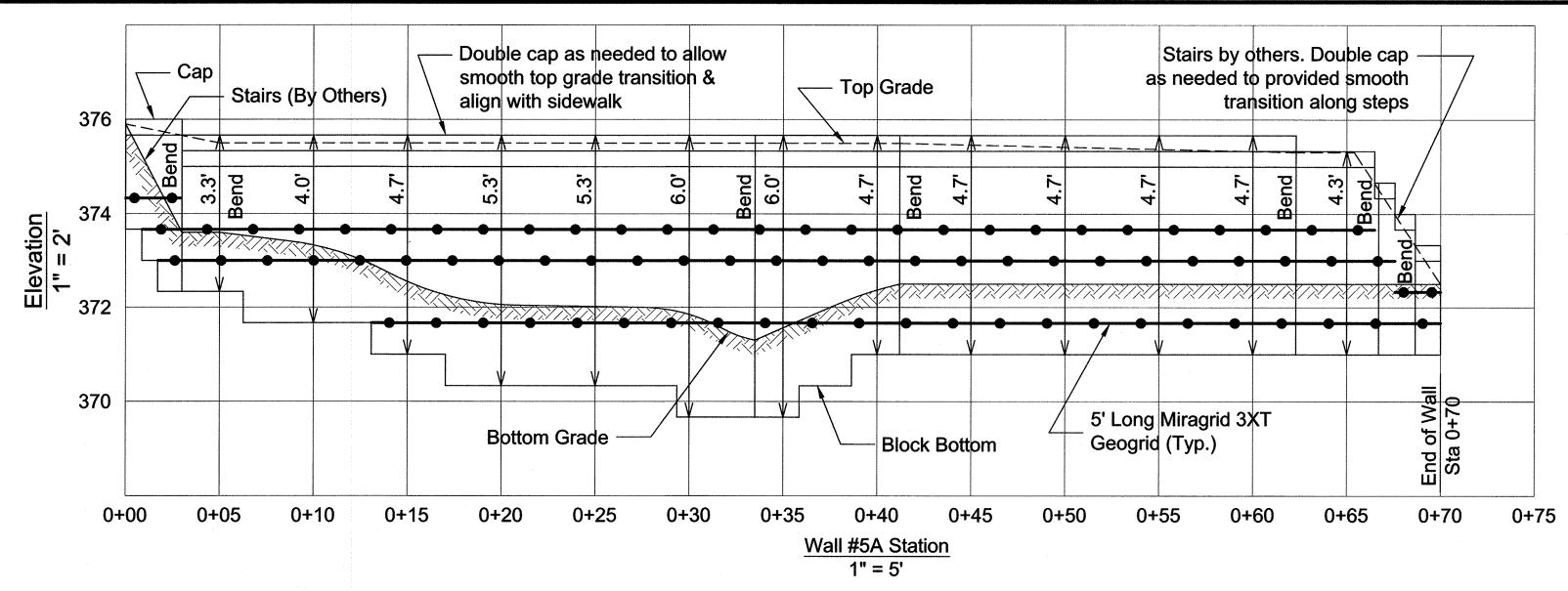
LAYER

TYPE SM

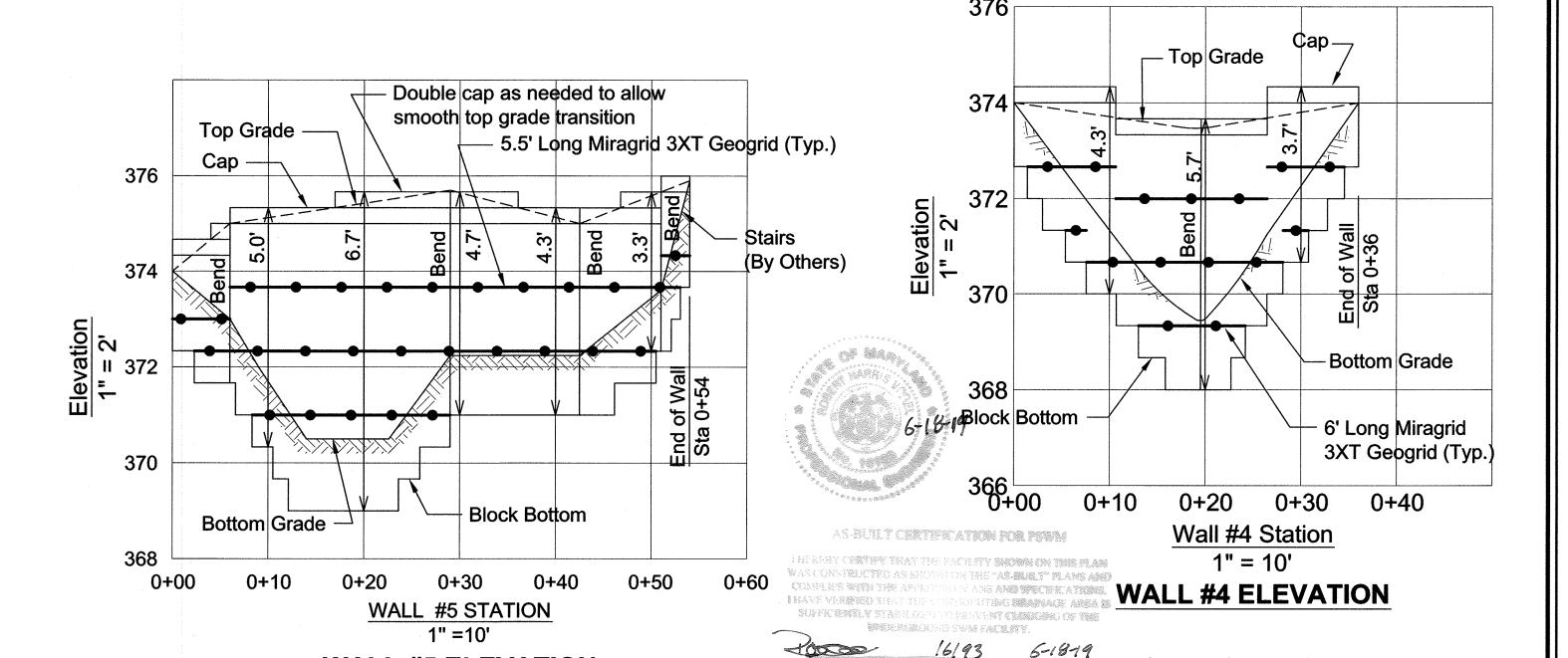
 γ = 125 PCF

 $Ø = 28^{\circ} MIN$

- STD CURB

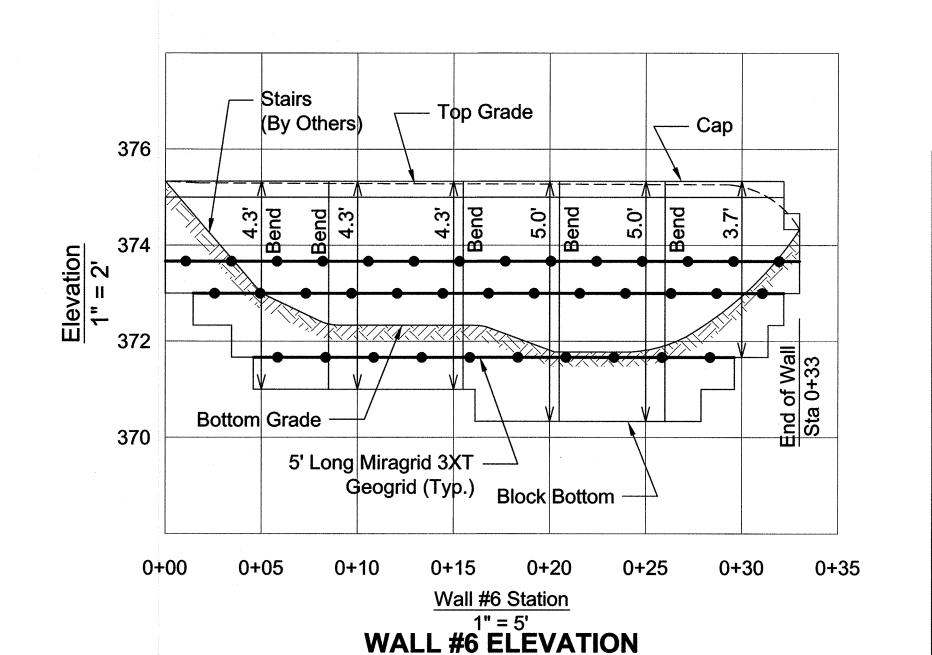


WALL #5A ELEVATION



NO AS-BUILT INFORMATION ON THIS SHEET

* nother wow



WALL #5 ELEVATION

REVISION RETAINING WALL DETAILS AND ELEVATIONS MIDWAY BUSINESS CENTER SNOWDEN RIVER SOUTH **SECTION 1, AREA 1** INDUSTRIAL / FLEX SPACE (PLAT 8795, L.10008/F.485) TAX MAP 42 GRID 10 6TH ELECTION DISTRICT PARCEL 319, PARCEL C HOWARD COUNTY, MARYLAND **ENGINEERING ASSOCIATES** 10975 Guilford Road, Suite A Annapolis Junction, Maryland (410) 880-4788

Fax: (410) 880-4098 ROFESSIONAL CERTIFICATE CHECKED BY: RWS DATE: MARCH 2013 SCALE: AS SHOWN

OWNER/DEVELOPER

TSC/JMJ SNOWDEN RIVER SOUTH LLC A MD LLC

8600 SNOWDEN RIVER PKWY, 207

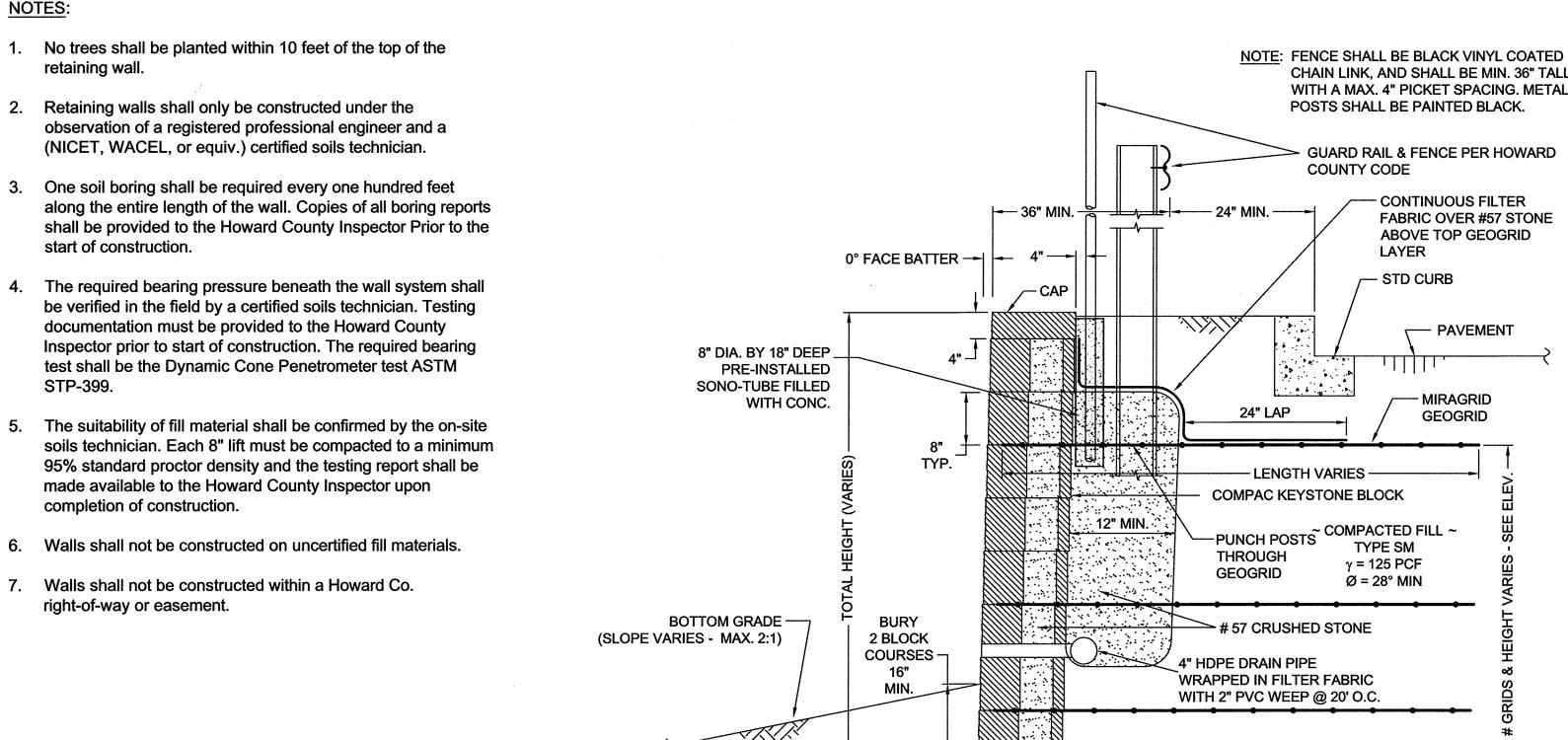
COLUMBIA MD 21045

(410) 953-0222

DATE

23 SHEET 24 AS-BUILT - DECEMBER 2018

HCEA NO.: 09073B



TYPICAL WALL SECTION N.T.S.

~ SUBGRADE APPROVED ~

FOR 2000 PSF BEARING

APPROVED BY PLANNING BOARD OF HOWARD COUNTY April 15, 2010

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

