

21.0 STANDARDS AND SPECIFICATIONS FOR TOPSOILS SEDIMENT CONTROL NOTES **DETAIL 22 - SILT FENCE** 1, A MINIMUM OF 48 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT OF PROPOSED BUILDING PROPOSED BUILDING II. FOR SITES HAVING DISTURBED AREAS UNDER 5 INSPECTION, LICENSE AND PERMITS SEDIMENT CONTROL DIVISION PRIOR TO THE START OF ANY PLACEMENT OF TOPSOIL OVER A PREPARED CONSTRUCTION (410-313-1855). 2. ALL VEGETATION AND STRUCTURAL PRACTICES ARE TO BE INSTALLED ACCORDING TO THE PROVISIONS i. PLACE TOPSOIL (IF REQUIRED) AND APPLY BROWN AND GRAY SUBSOIL PRIOR TO ESTABLISHMENT OF OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE 1994 MARYLAND STANDARDS AND MOIST SILT. PERMANENT VEGETATION. SOIL AMENDMENTS AS A MINIMUM OF 16" INTO GROUND SPECIFIED IN 20.0 VEGETATIVE
STABILIZATION - SECTION I - VEGETATIVE
STABILIZATION METHODS AND MATERIALS. AND CF SAND, TRACE TO SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL; AND REVISIONS THERETO. 13.6 LITTLE ROCK FRAGS 3. FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY 145 STABILIZATION SHALL BE COMPLETED WITHIN: (A) 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT BROWN, DARK BROWN CONTROL STRUCTURES, DIKES, PERIMETER SLOPES, AND ALL SLOPES GREATER THAN 3:1, (B) 14 DAYS AS TO PROVIDE A SUITABLE SOIL MEDIUM FOR VEGETABLE GROWTH. SOILS OF CONCERN HAVE III. FOR SITES HAVING DISTURBED AREAS OVER 5 TO ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE. MOIST SILTY CLAY, LITTLE 4. ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR LOW MOISTURE CONTENT, LOW NUTRIENT LEVELS. ACRES: PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 7, HOWARD COUNTY DESIGN MANUAL, STORM LOW PH. MATERIALS TOXIC TO PLANTS, AND/OR FRACE ROCK FRAGS (FILL) UNACCEPTABLE SOIL GRADATION. I. ON SOIL MEETING TOPSOIL BRÔWN, TAN AND 5. ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN SPECIFICATIONS, OBTAIN TEST RESULTS GREENISH GRAY MOIST SILTY CLAY, TRACE ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDING, SOD, TEMPORARY SEEDING, AND MULCHING (SEC. G). CONDITIONS WHERE PRACTICE APPLIES DICTATING FERTILIZER AND LIME AMENDMENTS REQUIRED TO BRING THE PERSPECTIVE VIEW BROWN, TAN AND MPORARY STABILIZATION WITH MULCH ALONE SHALL BE DONE WHEN RECOMMENDED SEEDING DATES I. THIS PRACTICE IS LIMITED TO AREAS HAVING 2:1 SOIL INTO COMPLIANCE WITH THE FENCE POST SECTIO DO NOT ALLOW FOR PROPER GERMINATION AND ESTABLISHMENT OF GRASSES. GREENISH GRAY OR FLATTER SLOPES WHERE: MOIST MF SAND, A. THE TEXTURE OF THE EXPOSED 6. ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE MAINTAINED IN AND SILT OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD SUBSOIL/PARENT MATERIAL IS NOT A. PH FOR TOPSOIL SHALL BE BETWEEN (DECOMPOSED **BROWN AND GREENISH** COUNTY SEDIMENT CONTROL INSPECTOR. ADEQUATE TO PRODUCE VEGETATIVE 6.0 AND 7.5. IF THE TESTED SOIL EMBED GEOTEXTILE CLASS FA DEMONSTRATES A PH OF LESS THAN 7. SITE ANALYSIS: 3" DAMP SILT, SOME OF SAND, 12.0 B. THE SOIL MATERIAL IS SO SHALLOW THAT 6.0, SUFFICIENT LIME SHALL BE 1.5 LA TRACE TO LITTLE ROCK TOTAL AREA: **GREENISH GRAY AND 1** THE ROOTING ZONE IS NOT DEEP ENOUGH PRESCRIBED TO RAISE THE PH TO 6.5 AREA DISTURBED: BROWN, TAN AND GREENISH 28.3 TO SUPPORT PLANTS OR FURNISH OR HIGHER. (DECOMPOSED ROCK) AREA TO BE ROOFED OR PAVED CROSS SECTION ROCK FRAGS, AND SILT, GRAY DAMP SILT & CLAY, GREENISH GRAY AND BLACK GRAY AND WHITE MOIST CONTINUING SUPPLIES OF MOISTURE AND AREA TO BE VEGETATIVELY STABILIZED: SILT & CLAY, AND TO SOME AND MF SAND MOIST SILTY CLAY, SOME CF PLANT NUTRIENTS. B. ORGANIC CONTENT OF TOPSOIL TO; AUGER REFUSAL AT 1.5' CF SAND (DECOMPOSED (DECOMPOSED ROCK) CF SAND (DECOMPOSED TOTAL CUT: SAND, LITTLE ROCK FRAGS 30.5 STANDARD SYMBO SHALL BE NOT LESS THAN 1.5 C. THE ORIGINAL SOIL TO BE VEGETATED TOTAL FILE PERCENT BY WEIGHT. 26.3 CONTAINS MATERIAL TOXIC TO PLANT 7.7 |5%" | TO:BROWN, TAN AND OFFSITE WASTE/BORROW AREA LOCATION ------SF ------(DECOMPOSED JOINING TWO ADJACENT SILT GROWTH AT COMPLETION, FENCE SECTIONS BROWN, TAN AND D. THE SOIL IS SO ACIDIC THAT TREATMENT C. TOPSOIL HAVING SOLUBLE SALT GRAY MOIST MF SAND, 8. ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF HOLE DRY AND CAVED AT TO; AUGER REFUSAL AT 5.0' WITH LIMESTONE IS NOT FEASIBLE. CONTENT GREATER THAN 500 PARTS UTILITIES MUST BE REPAIRED ON THE SAME DAY OF DISTURBANCE. PER MILLION SHALL NOT BE USED. SILT (DECOMPOSED ROCK 9. ADDITIONAL SEDIMENT CONTROLS MUST BE PROVIDED, IF DEEMED NECESSARY BY THE HOWARD Construction Specifications AUGER REFUSAL AT 16.0 1 DAY AFTER COMPLETION, LITTLE SILT TO; AUGER REFUSAL AT 4.5' II. FOR THE PURPOSE OF THESE STANDARDS AND 1. Fence posts shall be a minimum of 36" long, driven 16" minimum into the ground. Wood posts shall be 1 1/2" x 1 1/2" square (minimum) AT COMPLETION. COUNTY SEDIMENT CONTROL INSPECTOR HOLE DRY AND CAVED AT (DECOMPOSED ROCK) AT COMPLETION, AT COMPLETION, HOLE DRY AND CAVED AT D. NO SOD OR SEED SHALL BE PLACED SPECIFICATIONS, AREAS HAVING SLOPES 10. ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY HOLE DRY AND CAVED AT less than 1.00 pound per linear foot. HOLE DRY AND CAVED AT TO:AUGER REFUSAL AT 5.0' STEEPER THAN 2:1 REQUIRE SPECIAL ON SOIL SOIL WHICH HAS BEEN SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT Geotextile shall be fastened securely to each fence post with wire ties or staples at top and mid-section and shall meet the followin CONSIDERATION AND DESIGN FOR ADEQUATE TREATED WITH SOIL STERILANTS OR CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING, OTHER requirements for Geotextile Class F: 1 DAY AFTER COMPLETION HOLE DRY AND CAVED AT Tensile Strength Tensile Modulus Test: MSMT 509 50 lbs/in (min.) STABILIZATION, AREAS HAVING SLOPES STEEPER CHEMICALS USED FOR WEED BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL 1 DAY AFTER COMPLETION 1 DAY AFTER COMPLETION HOLE DRY AND CAVED AT THAN 2:1 SHALL HAVE THE APPROPRIATE CONTROL UNTIL SUFFICIENT TIME BY THE INSPECTION AGENCY IS MADE. HOLE DRY AND CAVED AT HOLE DRY AND CAVED AT 0.3 gal ft /minute (max.) Test: MSMT 322 STABILIZATION SHOWN ON THE PLANS. 11. TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGTHS OR THAT WHICH HAS ELAPSED (14 DAYS MIN.) TO Fittering Eggeciency 75% (min.) Test: MSMT 322
Where ends of geotextile fabric come together, they shall be overlapped, folded and stapled to prevent sediment bypass. SHALL BE BACK-FILLED AND STABILIZED WITHIN ONE WORKING DAY, WHICHEVER IS SHORTER. PERMIT DISSIPATION OF Silt Fence shall be inspected after each rainfall event and maintained when bulges occur or when sediment accumulation reaches 50% of EARTHWORK QUANTITIES ARE SOLELY FOR THE PURPOSE OF CALCULATING FEES, CONTRACTOR TO CONSTRUCTION AND MATERIAL SPECIFICATIONS PHYTO-TOXIC MATERIALS. VERIFY ALL QUANTITIES PRIOR TO THE START OF CONSTRUCTION. NOTE: TOPSOIL SUBSTITUTES OR AMENDMENTS. MARYLAND DEPARTMENT OF ENVIRONMENT ** TO BE DETERMINED BY CONTRACTOR, WITH PRE-APPROVAL OF THE SEDIMENT CONTROL INSPECTOR I. TOPSOIL SALVAGED FROM THE EXISTING SITE PAGE E - 15 - 3 WATER MANAGEMENT ADMINISTRATION MAY BE USED PROVIDED THAT IT MEETS THE AS RECOMMENDED BY A QUALIFIED AGRONOMIST WITH AN APPROVED AND ACTIVE GRADING PERMIT. STANDARDS AS SET FORTH IN THESE OR SOIL SCIENTIST AND APPROVED BY THE APPROPRIATE APPROVAL AUTHORITY, MAY BE SPECIFICATIONS. TYPICALLY, THE DEPTH OF SEQUENCE OF CONSTRUCTION **DETAIL 33 - SUPER SILT FENCE** PROPOSED TOP OF WALL USED IN LIEU OF NATURAL TOPSOIL. TOPSOIL TO BE SALVAGED FOR A GIVEN SOIL TYPE CAN BE FOUND IN THE REPRESENTATIVE 1. OBTAIN GRADING PERMIT. 150 SOIL PROFILE SECTION IN THE SOIL SURVEY ii. PLACE TOPSOIL (IF REQUIRED) AND APPLY SOIL NOTE: FENCE POST SPACING NOTIFY HOWARD COUNTY DEPARTMENT OF INSPECTIONS, LICENSE AND PERMITS AT (410) 313-1880 AT AMENDMENTS A SPECIFIED IN 20.0 VEGETATIVE STABILIZATION - SECTION I - VEGETATIVE PUBLISHED BY USDA-SCS IN COOPERATION WITH LEAST 24 HOURS BEFORE STARTING ANY WORK. MARYLAND AGRICULTURAL EXPERIMENTAL INSTALL STABILIZED CONSTRUCTON ENTRANCE, SILT FENCE AND SUPER SILT FENCE, (3 DAYS) STATION. STABILIZATION METHODS AND MATERIALS. ROUGH GRADE SITE AND INSTALL STORM DRAIN AND OTHER UTILITIES. PROVIDE INLET PROTECTION BROWN AND GREENISH AROUND ALL STORM DRAIN INLETS. (1 MONTH) II. TOPSOIL SPECIFICATIONS - SOIL TO BE USED AS V. TOPSOIL APPLICATION MOIST SILT & CLAY, LITTLE BEGIN BUILDING CONSTRUCTION. BROWN AND GRAY TOPSOIL MUST MEET THE FOLLOWING: FINISH BUILDING CONSTRUCTION AND PAVE DRIVE AND PARKING AREAS. (4 MONTHS) WHEN TOPSOILING, MAINTAIN NEEDED EROSION 20.0 12 AND CF SAND, TRACE OF FINE GRADE SITE. (1 WEEK) i. TOPSOIL SHALL BE A LOAM, SANDY LOAM, AND SEDIMENT CONTROL PRACTICES SUCH AS 145 24.1 145 145 SOME ROCK FRAGS UPON STABILIZATION OF ALL DISTURBED AREAS AND WITH THE PERMISSION OF THE SEDIMENT **DIVERSIONS, GRADE STABILIZATION** CLAY LOAM, SILT LOAM, SANDY CLAY LOAM, GRAY DAMP TO MOIST 3.0 3.0 CONTROL INSPECTOR, REMOVE ALL SEDIMENT CONTROL MEASURES AND STABILIZE ANY REMAINING LOAMY SAND, OTHER SOILS MAY BE USED STRUCTURES, EARTH DIKES, SLOPE SILT FENCE GALVANIZED OR ALUMINUM SILT & BROWN, TAN AND CLAY, SOME OF SAND TO:BROWN, GREENISH GRAY IF RECOMMENDED BY AN AGRONOMIST OR AND SEDIMENT TRAPS AND BASINS BROWN, TAN AND GRAY 18 GRAY MOIST CLAY & SILT. AND DARK GRAY MOIST 28.8 A SOIL SCIENTIST AND APPROVED BY THE -FOLLOWING INITIAL SOIL DISTURBANCE OR ANY REDISTURBANCES, PERMANENT OR MOIST SILT. LITTLE MF SAND ROCK FRAGS, SOM APPROPRIATE APPROVAL AUTHORITY. ii. GRADES ON THE AREAS TO BE TOPSOILED, TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: LITTLE CF SAND CF SAND, LITTLE SILT DECOMPOSED ROCK) WHICH HAVE BEEN PREVIOUSLY ESTABLISHED, REGARDLESS, TOPSOIL SHALL NOT BE A FILTER CLOTH . (DECOMPOSED ROCK) AT COMPLETION A. 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, AT COMPLETION. SHALL BE MAINTAINED, ALBEIT 4"-8" HIGHER IN MIXTURE OF CONTRASTING TEXTURED HOLE DRY AND CAVED AT HOLE DRY AND CAVED AT 16" MIN. 1ST LAYER OF BROWN, ORANGISH BROWN, SWALES AND ALL SLOPES GREATER THAN 3:1. SUBSOILS AND SHALL CONTAIN LESS THAN 140 22.6 20 TAN AND GREENISH GRAY MOIST SILT 8 B. 14 CALENDAR DAYS FOR ALL OTHER DISTURBED AREAS. EMBED FILTER CLOTH 8" 5% BY VOLUME OF CINDERS, STONES, -DURING GRADING AND AFTER EACH RAINFALL, CONTRACTOR WILL INSPECT AND PROVIDE ****R-1 iii. TOPSOIL SHALL BE UNIFORMLY DISTRIBUTED IN 1 DAY AFTER COMPLETION, 1 DAY AFTER COMPLETION, SLAG, COARSE FRAGMENTS, GRAVEL, CLAY. NECESSARY MAINTENANCE TO THE SEDIMENT CONTROL MEASURES ON THIS PLAN. SOME CF SAND A 4"-8" LAYER AND LIGHTLY COMPACTED TO A * IF MULTIPLE LAYERS ARE REQUIRED TO ATTAIN 42* HOLE DRY AND CAVED AT HOLE DRY AND CAVED AT STICKS, ROOTS, TRASH, OR OTHER (DECOMPOSED ROCK) MINIMUM THICKNESS OF 4", SPREADING SHALL MATERIAL LARGER THAT 1 AND 1/2" IN DIAMETER. BE PERFORMED IN SUCH A MANNER THAT CONSTRUCTION SPECIFICATIONS BROWN, GRAY AND WHITE MOIST **BROWN MOIST CLAY & SILT,** SODDING OR SEEDING CAN PROCEED WITH A 1. FENCING SHALL BE 42" IN HEIGHT AND CONSTRUCTED IN ACCORDANCE WITH THE LATEST MARYLAND STATE HIGHWAY DETAILS FOR CHAIN LINK FENCING. THE SPECIFICATION FOR A 6" FENCE SHALL BE USED, SUBSTITUTING 42" FABRIC AND MINIMUM OF ADDITIONAL SOIL PREPARATION ii. TOPSOIL MUST BE FREE OF PLANTS OR LITTLE TO SOME CF SAND CF SAND, TRACE ROCK PLANT PARTS SUCH AS BERMUDA GRASS, AND TILLAGE. ANY IRREGULARITIES IN THE 34.4 (DECOMOPOSED ROCK) SURFACE RESULTING FROM TOPSOILING OF 2. CHAIN LINK FENCE SHALL BE FASTENED SECURELY TO THE FENCE POSTS WITH WIRE TIES. THE LOWER TENSION WIRE, BRACE AND TRUSS RODS, DRIVE ANCHORS AND POST CAPS ARE NOT REQUIRED EXCEPT ON THE ENDS OF THE FENCE. QUACKGRASS, JOHNSONGRASS, NUTSEDGE, POISON IVY, THISTLE, OR OTHER OPERATIONS SHALL BE CORRECTED IN TO;BROWN, TAN AND CF SAND, TRACE ROCK 3. FILTER CLOTH SHALL BE FASTENED SECURELY TO THE CHAIN LINK FENCE WITH TIES SPACED, EVERY 24* AT THE TOP AND 18.3 OTHERS AS SPECIFIED. ORDER TO PREVENT THE FORMATION OF GREENISH GRAY MOIST SILT 8 FRAGS (FILL) 15.6 44 BROWN, TAN, GREENISI DEPRESSIONS OR WATER POCKETS. 17.0 CLAY: AND ME SAND 4 SILTER CLOTH SHALL BE EMBEDDED A MINIMUM OF 8" INTO THE GROUND iii. WHERE THE SUBSOIL IS EITHER HIGHLY 15.0 DECOMPOSED ROCK AND GRAY, DAMP TO MOIS 5. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER, THEY SHALL BE OVERLAPPED BY 6" AND FOLDED. iv. TOPSOIL SHALL NOT BE PLACE WHILE THE ACIDIC OR COMPOSED OF HEAVY CLAYS, MF SAND, LITTLE SILT 18.4 6. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND SILT BUILDUPS REMOVED WHEN "BULGES" DEVELOP IN THE SILT 11.1 TOPSOIL OR SUBSOIL IS IN A FROZEN OR MUDDY GROUND LIMESTONE SHALL BE SPREAD AT (DECOMPOSED ROCK) FENCE, OR WHEN SILT REACHES 50% OF FENCE HEIGHT HOLE DRY AND CAVED AT ROWN AND DARK GRAY MOIST SILT & CLAY, AND THE RATE OF 4-8 TONS/ACRE (200-400 CONDITION, WHEN THE SUBSOIL IS E 7. FILTER CLOTH SHALL BE FASTENED SECURELY TO EACH FENCE POST WITH WIRE TIES OR STAPLES AT TOP AND MID XCESSIVELY WET OR IN A CONDITION THAT MAY POUNDS PER 1,000 SQUARE FEET) PRIOR SECTION AND SHALL MEET THE FOLLOWING REQUIREMENTS FOR GEOTEXTILE CLASS F MOIST SILTY CLAY TRACE ROCK OTHERWISE BE DETRIMENTAL TO PROPER TO THE PLACEMENT OF TOPSOIL. LIME 1 DAY AFTER COMPLETION, LITTLE CF FRAGS(DECOMPOSED ROCK) TENSILE STRENGTH 50 LBS/IN (MIN.) AT COMPLETION. SHALL BE DISTRIBUTED UNIFORMLY OVER GRADING AND SEEDBED PREPARATION. 16.2 HOLE DRY AND CAVED AT SAND, TRACE ROCK TENSILE MODULUS 20 LBS/IN (MIN.) TEST: MSMT 50
FLOW RATE 0.3 GAUFT /MIN. (MAX.) TEST: MSMT 3 T0:BROWN, TAN AND GREENIS 8.0 FRAGS (FILL) DESIGNATED AREAS AND WORKED INTO 5%" GRAY MOIST CLAYEY SILT, AND CF THE SOIL IN CONJUNCTION WITH TILLAGE FILTERING EFFICIENCY 75% (MIN.) TEST: MSMT 32 SAND (DECOMPOSED ROCK) OPERATIONS AS DESCRIBED IN THE 1 DAY AFTER COMPLETION, AT COMPLETION, FOLLOWING PROCEDURES HOLE DRY AND CAVED AT HOLE DRY AND CAVED AT AUGER REFUSAL AT 10.5' MARYLAND DEPARTMENT OF ENVIRONMENT J.S. DEPARTMENT OF AGRICULTURE 1 DAY AFTER COMPLETION. PERMANENT SEEDING NOTES AT COMPLETION, HOLE DRY AND CAVED AT HOLE DRY AND CAVED AT BORING PROFILES APPLY TO GRADED OR CLEARED AREAS NOT SUBJECT TO IMMEDIATE FURTHER DISTURBANCE WHERE A PERMANENT LONG-LIVED VEGETATIVE COVER IS NEEDED. 1 DAY AFTER COMPLETION, NOT TO SCALE DETAIL 23C - CURB INLET PROTECTION HOLE DRY AND CAVED AT **DETAIL 23B - AT GRADE INLET PROTECTION** SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR OTHER (COG OR COS INLETS) ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED. **DETAIL 24 - STABILIZED CONSTRUCTION ENTRANCE** SOIL AMENDMENTS: IN LIEU OF SOIL TEST RECOMMENDATIONS, USE ONE OF THE FOLLOWING SCHEDULES 2' MINIMUM LENGTH 1) PREFERRED - APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS/100 SQ.FT.) AND 600 LBS OF 2" X 4" PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ.FT.) BEFORE SEEDING. HARROW OR DISC INTO UPPER THREE INCHES OF SOIL. AT THE TIME OF SEEDING, APPLY 400 LBS PER ACRE 30-0-0 UREAFORM FERTILIZER (9 LBS/1000 SQ.FT.) 2) ACCEPTABLE - APPLY 2 TONS PER ACRE DOLOMATIC LIMESTONE (92 LBS/1000 SQ.FT.) AND APPLY 2" X 4" WEIR 3/4 "-1 1/2 " STONE 1000 LBS PER ACRE 10-10-10 FERTILIZER (23 LBS/1000 SQ.FT.) BEFORE SEEDING. HARROW OR DISC - EARTH FILL GEOTEXTILE CLASS FILTER CLOTH ---- PIPE AS NECESSARY 'C' OR BETTER MINIMUM 6" OF 2"- 3" SEEDING: FOR THE PERIODS MARCH 1 THRU APRIL 30, AND AUGUST 1 THRU OCTOBER 15, SEED WITH 60 - EXISTING GROUND AND WIDTH OF STRUCTURE 6' MAXIMUM SPACIN OF 2" X 4" SPACERS LBS PER ACRE (1.4 LBS/1000 SQ.FT.) OF KENTUCKY 31 TALL FESCUE. FOR THE PERIOD MAY 1 THRU JULY 31, PROFILE SEED WITH 60 LBS KENTUCKY 31 TALL FESCUE PER ACRE AND 2 LBS PER ACRE (0.05 LBS/1000 SQ.FT.) OF WEEPING LOVEGRASS, DURING THE PERIOD OF OCTOBER 16 THRU FEBRUARY 28, PROTECT SITE BY: PLAN/CUT AWAY VIEW OPTION (1) 2 TONS PER ACRE WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING, OPTION (2) USE SOD, OPTION (3) SEED WITH 60 LBS/ACRE KENTUCKY 31 TALL FESCUE AND MULCH WITH 2 TONS/ACRE WELL ANCHORED STRAW. MULCHING: APPLY 1 1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ.FT.) OF UNROTTED SMALL GRAIN STANDARD SYMBOL STRAW IMMEDIATELY AFTER SEEDING, ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ.FT.) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ.FT.) FOR ANCHORING. 2" X 4" SPACER MAINTENANCE: INSPECT ALL SEEDED AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND SCE X WIRE MESH PLAN VIEW 2" X 4" WEIR RESEEDINGS. MAX, DRAINAGE AREA = 1/4 ACRE 6" OVERLAP CONSTRUCTION SPECIFICATION TEMPORARY SEEDING NOTES 1. LENGTH - MINIMUM OF 50' (* 30' FOR A SINGLE RESIDENCE LOT).

2. WIDTH - 10' MINIMUM, SHOULD BE FLARED AT THE EXISTING ROAD TO PROVIDE A TURNING RADIUS.

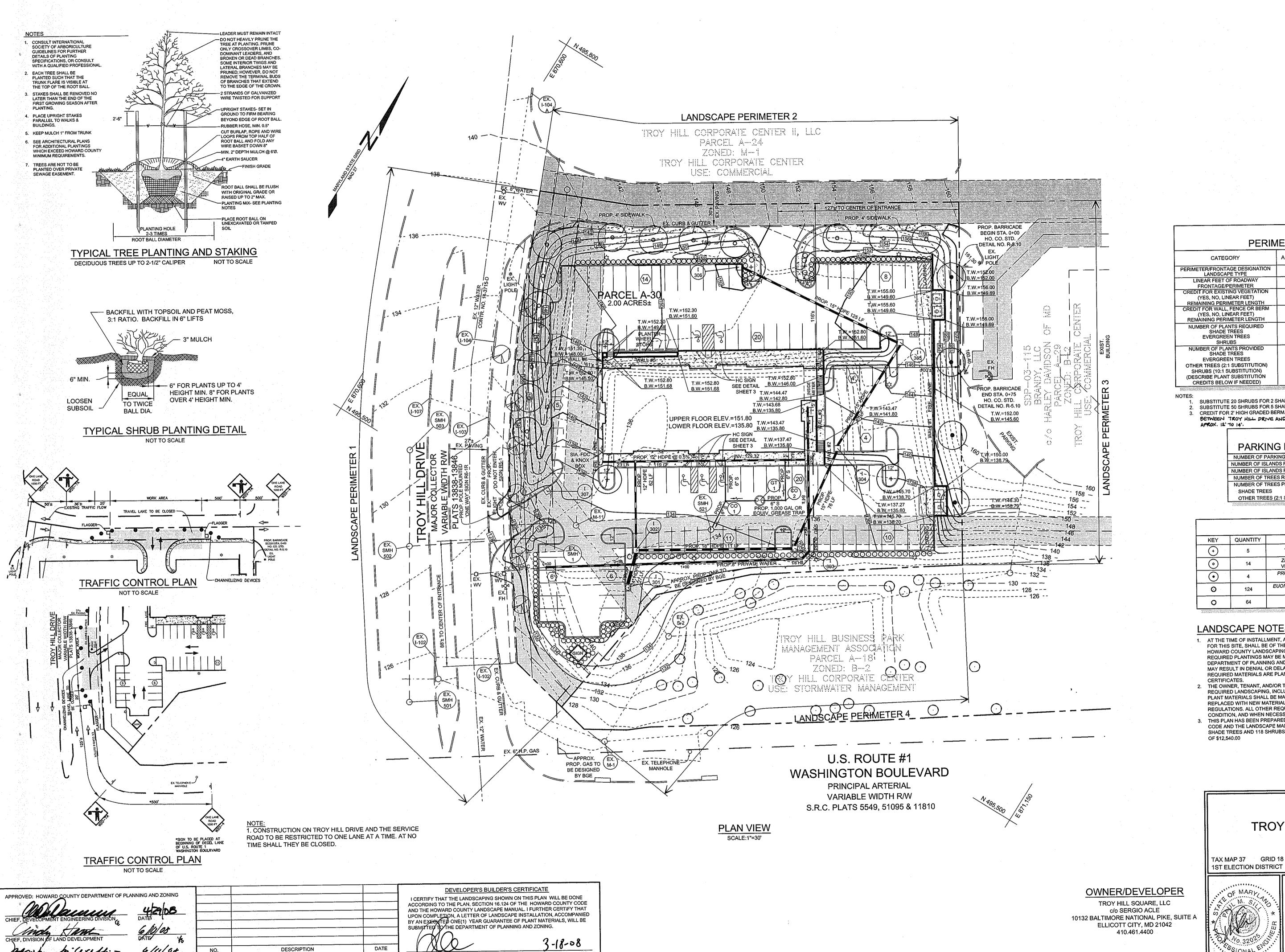
3. GEOTEXTILE FABRIC (FILTER CLOTH) SHALL BE PLACED OVER THE EXISTING GROUND PRIOR TO PLACING STONE. " THE PLAN APPROVAL AUTHORITY MAY NOT REQUIRE SINGLE FAMILY RESIDENCES TO USE GEOTEXTILE. CROSS SECTION SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED. AGIP 1. Attach a continuous piece of wire mesh (30" minimum width by throat 4") to the 2" x 4" weir (measuring throat length plus 2") APPROVAL AUTHORITY MAY NOT REQUIRE SINGLE FAMILY RESIDENCES TO USE GEOTEXTILE.

4. STONE - CRUSHED AGGREGATE (2" TO 3") OR RECLAIMED OR RECYCLED CONCRETE EQUIVALENT SHALL BE PLACED AT LEAST 6"
DEEP OVER THE LENGTH AND WIDTH OF THE ENTRANCE.

5. SURFACE WATER - ALL SURFACE WATER FLOWING TO OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED
THROUGH THE ENTRANCE, MAINTAINING POSITIVE DRAINAGE. IPIE INSTALLED THROUGH THE STABILIZED CONSTRUCTION
ENTRANCE SHALL BE PROTECTED WITH A MOUNTABLE BERM WITH 5:1 SLOPES AND A MINIMUM OF 6" OF STONE OVER THE PIPE.
PIPE HAS TO BE SIZED ACCORDING TO THE DRAINAGE. WHEN THE SCE IS LOCATED AT A HIGH SPOT AND HAS NO DRAINAGE TO
CONVEY, A PIPE WILL NOT BE NECESSARY. PIPE SHOULD BE SIZED ACCORDING TO THE AMOUNT OF RUNOFF TO BE CONVEYED. A 6"
MINIMUM WILL BE REQUIRED.

6. LOCATION - A STABILIZED CONSTRUCTION ENTRANCE SHALL BE LOCATED AT EVERY POINT WHERE CONSTRUCTION TRAFFIC
ENTERS OR LEAVES A CONSTRUCTION SITE VEHICLES LEAVING THE SITE MILET TRAVEL OVER THE ENTIRE LENGTH OF THE 2. Place a continuous piece of Geotextile Class E the same dimensions as the wire mesh over the wire mesh and securely SOIL AMENDMENTS: APPLY 600 LBS PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ.FT). attach it to the 2" x 4" weir. 3. Securely nail the 2" X 4" weir to a 9" long vertical spacer to be located, between the weir and the inlet face (max. 4" apart) SEEDING: FOR PERIODS MARCH 1 THRU APRIL 30 AND FROM AUGUST 15 THRU NOVEMBER 15, SEED WITH 2 4. Place the assembly against the inlet throat and nail (minimum 2' lengths of 2" x 4" to the top of the weir at spacer locations). These 2" x 4" anchors shall extend across the inlet top and be held in place by sandbags or alternate weight. Construction Specifications 1/2 BUSHEL PER ACRE OF ANNUAL RYE (3.2 LBS/1000 SQ.FT.) FOR THE PERIOD MAY 1 THRU AUGUST 14, 5. The assembly shall be placed so that the end spacers are a minimum 1' beyond both ends of the throat opening. SEED WITH 3 LBS PER ACRE OF WEEPING LOVEGRASS (0.07 LBS/1000 SQ.FT.). FOR THE PERIOD NOVEMBER 1. Lift grate and wrap with Geotextile Class E to completely cover all openings, 6. Form the 1/2" x 1/2" wire mesh and the geotextile fabric to the concrete gutter and against the face of the curb on both sides of the inlet. Place clean 3/4" x 1 1/2" stone over the wire mesh and geotextile in such a manner to prevent water from entening SEDIMENT AND EROSION CONTROL 1 THRU FEBRUARY 28, PROTECT SITE BY APPLYING 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH ENTERS OR LEAVES A CONSTRUCTION SITE. VEHICLES LEAVING THE SITE MUST TRAVEL OVER THE ENTIRE LENGTH OF THE STABILIZED CONSTRUCTION ENTRANCE. AND SEED AS SOON AS POSSIBLE IN THE SPRING, OR USE SOD. NOTES AND DETAILS 2. Place 3/4" to 11/2" stone, 4"-6" thick on the grate to secure the fabric and 7. This type of protection must be inspected frequently and the filter cloth and stone replaced when clogged with sediment. MULCHING: APPLY 1 1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ.FT.) OF UNROTTED SMALL GRAIN TROY HILL CORPORATE CENTER STRAW IMMEDIATELY AFTER SEEDING, ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH MARYLAND DEPARTMENT OF ENVIRONMENT ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ.FT.) OF EMULSIFIED ASPHALT ON FLAT U.S. DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE MARYLAND DEPARTMENT OF ENVIRONMEN MARYLAND DEPARTMENT OF ENVIRONMENT U.S. DEPARTMENT OF AGRICULTURE WATER MANAGEMENT ADMINISTRATION PARCEL A-30 AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ.FT.) FOR ANCHORING. WATER MANAGEMENT ADMINISTRATION REFER TO THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR RATE AND METHODS NOT COVERED. **RESTAURANT AND RETAIL CENTER** TAX MAP 37 GRID 18 PARCEL 135 **1ST ELECTION DISTRICT** HOWARD COUNTY, MARYLAND DEVELOPER'S CERTIFICATE DESIGN BY: APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING ENGINEERS CERTIFICATE OWNER/DEVELOPER I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE "I CERTIFY THAT THIS PLAN FOR SEDIMENT AND EROSION CONTROL RAWN BY: ACCORDING TO THIS PLAN FOR SEDIMENT AND EROSION CONTROL, AND THAT REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT TROY HILL SQUARE, LLC Associates · LLC USDA-NATURAL RESOURCES CONSERVATION PERSONAL KNOWLEDGE OF THE SITE CONDITIONS AND THAT IT WAS DATE HECKED BY: WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE c/o SERGIO ACLE PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD APPROVED TRAINING PROGRAM FOR THE CONTROL OF SERVICE 10132 BALTIMORE NATIONAL PIKE, SUITE A ROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE SCALE: AS SHOWN SOIL CONSERVATION DISTRICT. Engineers Surveyors Planners INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT. ELLICOTT CITY, MD 21042 DATE: MARCH 18, 2008 3300 North Ridge Road, Suite 160 410.461.4400 3,19,08 Ellicott City, Maryland 21043 PROJECT #: _____06-076 NO. DESCRIPTION DATE Phone: 443.325.7682 Fax: 443.325.7685 SHEET#: 4 of 12 mail: info@saaland.com **REVISIONS**

SDP-08-029



REVISIONS

LEGEND _____382 **EXISTING CONTOUR** PROPOSED CONTOUR **EXISTING SPOT ELEVATION** PROPOSED SPOT ELEVATION DIRECTION OF FLOW **EXISTING LIGHT POLES** PROPOSED LIGHT POLES PROPOSED DUMPSTER PROPOSED LANDSCAPE TREES 0 0 PROPOSED LANDSCAPE SHRUBS EXISTING STREET TREES EXISTING LANDSCAPE TREES

		DULE A		_	
PERIN	METER LA	ANDSCA	PE EDGE		
CATEGORY	ADJACENT TO ROADWAYS		ADJACENT TO PERIMETER PROPERTIES		TOTALS
PERIMETER/FRONTAGE DESIGNATION LANDSCAPE TYPE	1 E	2 E	3 A	4 A	4 2-A, 2-E
LINEAR FEET OF ROADWAY FRONTAGE/PERIMETER	378	294	255	319	1,246
CREDIT FOR EXISTING VEGETATION (YES, NO, LINEAR FEET) REMAINING PERIMETER LENGTH	NO	NO	NO	NO	NO
CREDIT FOR WALL, FENCE OR BERM (YES, NO, LINEAR FEET) REMAINING PERIMETER LENGTH	YES 201' (3)	NO	NO	NO	NO
NUMBER OF PLANTS REQUIRED SHADE TREES EVERGREEN TREES SHRUBS	1:40 = 9 1:4 = 44	1:40 = 7 1:4 = 74	1:60 = 4	1:60 = 5	25 - 118
NUMBER OF PLANTS PROVIDED SHADE TREES EVERGREEN TREES OTHER TREES (2:1 SUBSTITUTION) SHRUBS (10:1 SUBSTITUTION) (DESCRIBE PLANT SUBSTITUTION CREDITS BELOW IF NEEDED)	7 - - 64 (1)	7 - - 74	4 -	- - 50 (2)	18 (1)(2 188 (1)(2

SUBSTITUTE 20 SHRUBS FOR 2 SHADE TREES. 2. SUBSTITUTE 50 SHRUBS FOR 5 SHADE TREES.

3. CREDIT FOR 2' HIGH GRADED BERM. TOTAL GRADE DIFFERENCE BETWEEN TROY HILL DRIVE AND PROPOSED PARKING IS

OTHER TREES (2:1 SUBSTITUTION)

SHADE TREES

SCHEDULE B PARKING LOT LANDSCAPING CHART NUMBER OF PARKING SPACES NUMBER OF ISLANDS REQUIRED NUMBER OF ISLANDS PROVIDED NUMBER OF TREES REQUIRED NUMBER OF TREES PROVIDED

		PLANT LIST		
KEY	QUANTITY	BOTANICAL NAME	SIZE	NOTE
<u>•</u>	5	ACER RUBRUM 'OCTOBER GLORY' OCTOBER GLORY RED MAPLE	2 1/2"-3" CAL.	в&в
0	14	ZELKOVA SERRATA 'VILLAGE GREEN' VILLAGE GREEN JAPANESE ZELKOVA	2 1/2"-3" CAL.	B&B
<u> </u>	4	PRUNUS CERASIFERA 'THUNDERCLOUD' THUNDERCLOUD PLUM	1 1/2"-2" CAL.	B&B
0	124	EUONYMUS KIAUTSCHOVICUS 'MANHATTAN' MANHATTAN EUONYMUS	2-1/2' - 3' HT	в&в
0	64	IILEX CORNUTA 'CARISSA' CARISSA HOLLY	2-1/2' - 3' HT	8&8

LANDSCAPE NOTES

- 1. AT THE TIME OF INSTALLMENT, ALL SHRUBS AND OTHER PLANTINGS HEREWITH LISTED AND APPROVED FOR THIS SITE, SHALL BE OF THE PROPER HEIGHT REQUIREMENTS IN ACCORDANCE WITH THE HOWARD COUNTY LANDSCAPING MANUAL. IN ADDITION, NO SUBSTITUTIONS OR RELOCATION OF REQUIRED PLANTINGS MAY BE MADE WITHOUT PRIOR REVIEW AND APPROVAL FROM THE DEPARTMENT OF PLANNING AND ZONING. ANY DEVIATION FROM THIS APPROVED LANDSCAPE PLAN MAY RESULT IN DENIAL OR DELAY IN THE RELEASE OF LANDSCAPE SURETY UNTIL SUCH TIME AS ALL
- REGULATIONS. ALL OTHER REQUIRED LANDSCAPING SHALL BE PERMANENTLY MAINTAINED IN GOO CONDITION, AND WHEN NECESSARY, REPAIRED OR REPLACED.
- THIS PLAN HAS BEEN PREPARED IN ACCORDANCE WITH SECTION 16.124 OF THE HOWARD COUNTY CODE AND THE LANDSCAPE MANUAL. FINANCIAL SURETY FOR THE REQUIRED LANDSCAPING (30 SHADE TREES AND 118 SHRUBS) WILL BE POSTED AS PART OF THE GRADING PERMIT IN THE AMOUNT

LANDSCAPING PLAN

TROY HILL CORPORATE CENTER PARCEL A-30

RESTAURANT AND RETAIL CENTER

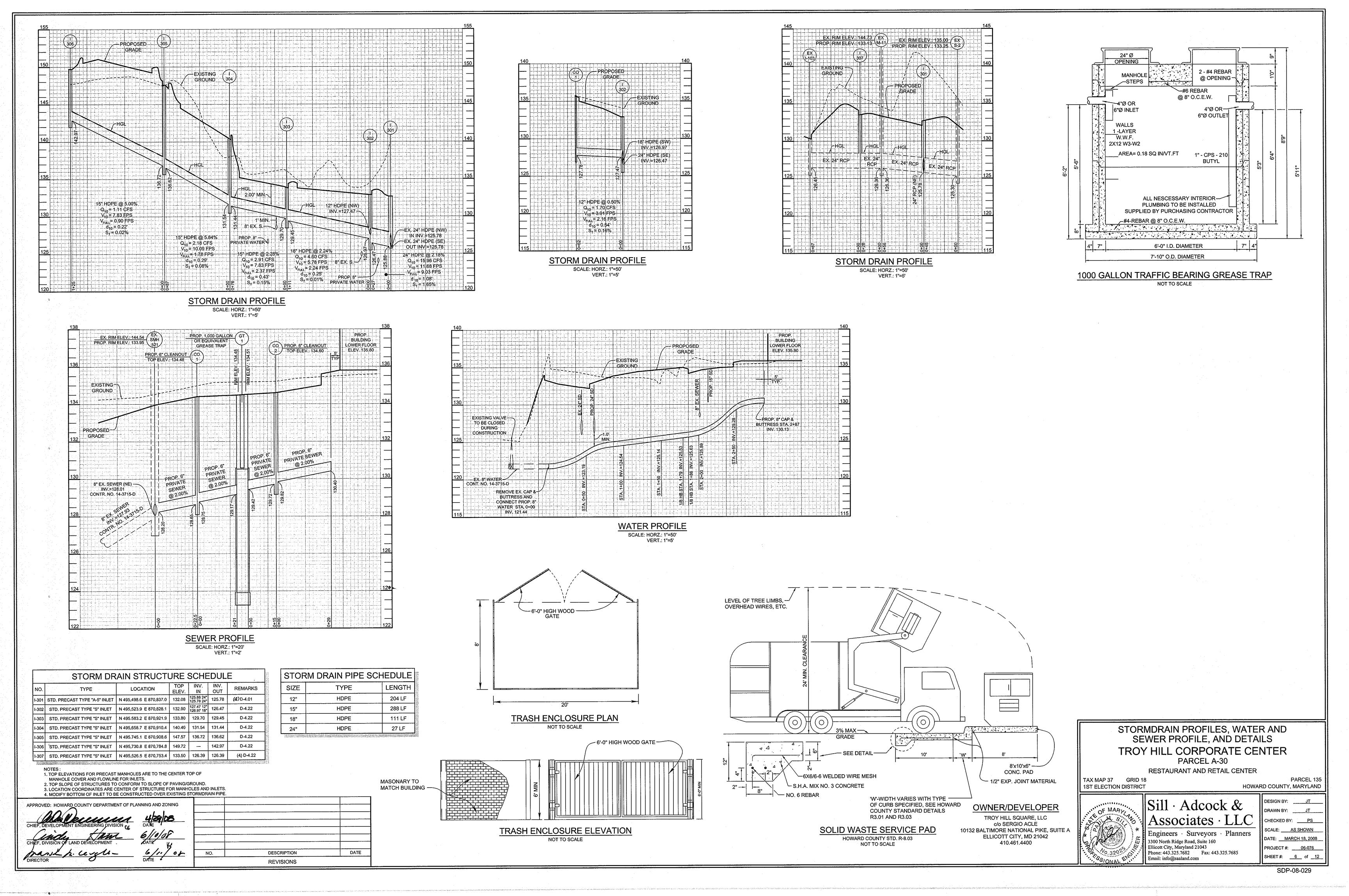
Sill Adcock & Associates · LLC

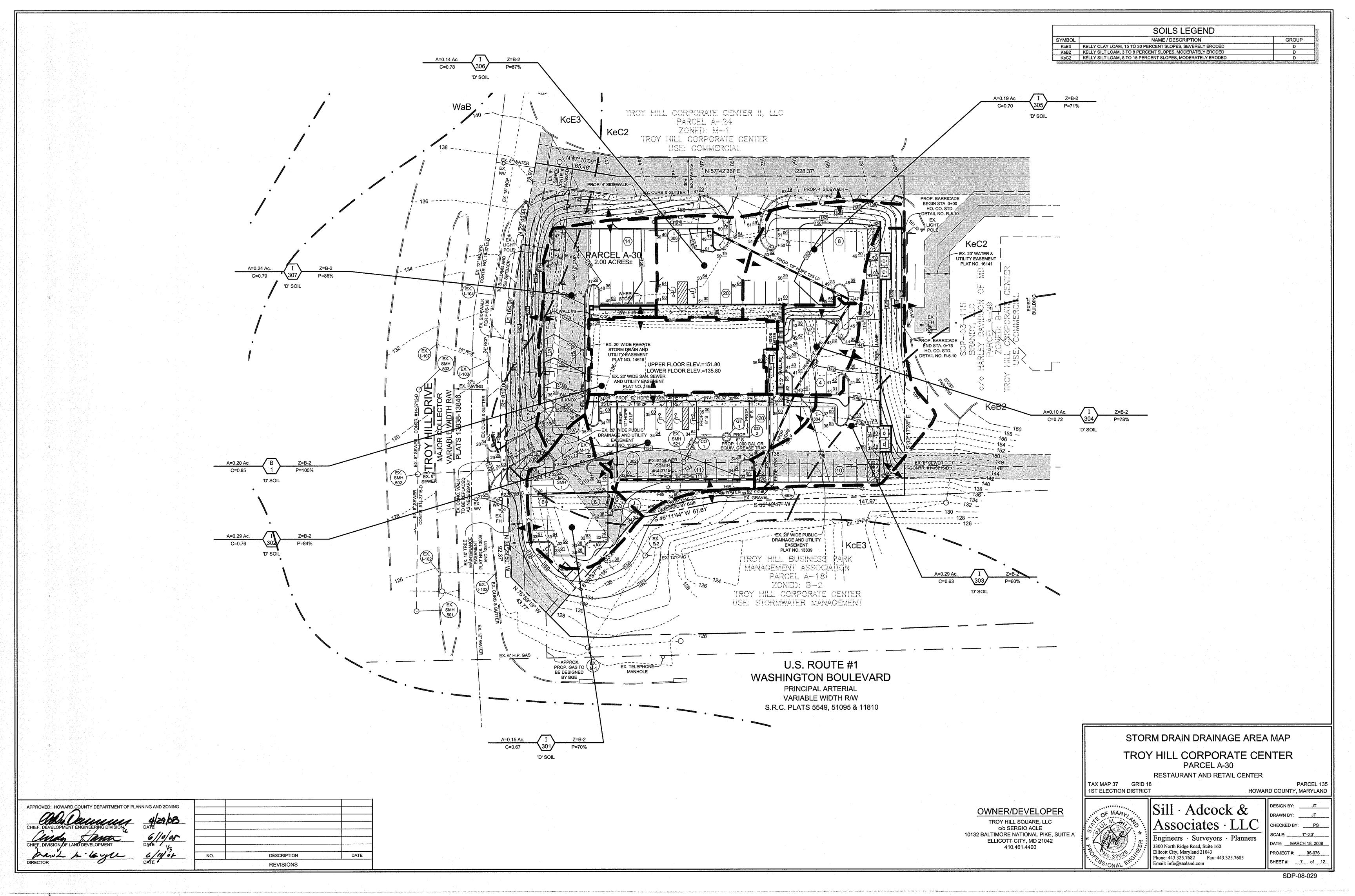
Engineers · Surveyors · Planners 3300 North Ridge Road, Suite 160 Ellicott City, Maryland 21043 Phone: 443.325.7682 Fax: 443.325.7685 Email: info@saaland.com

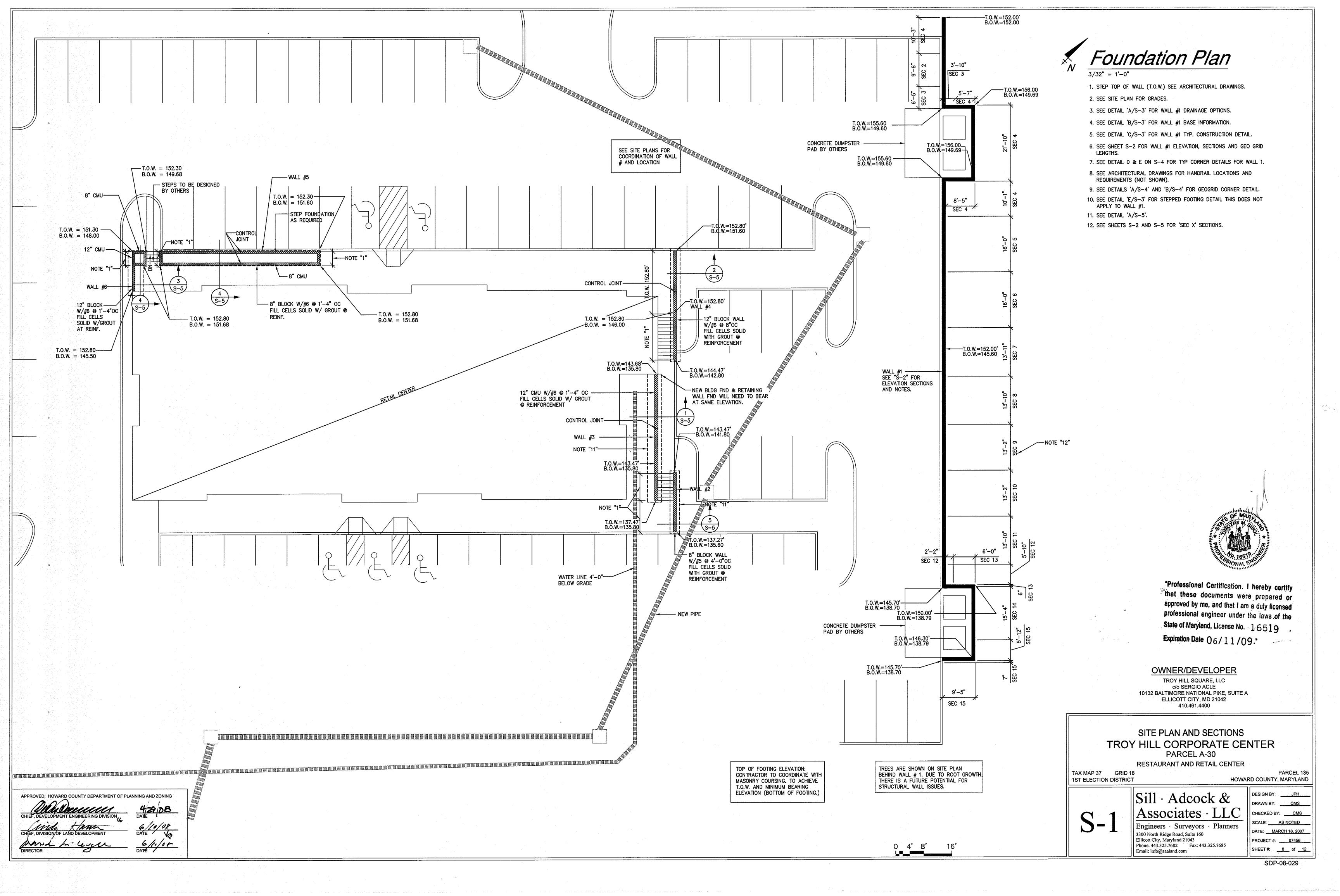
DATE: MARCH 18, 2008 PROJECT#: 06-076 SHEET#: <u>5</u> of <u>12</u>

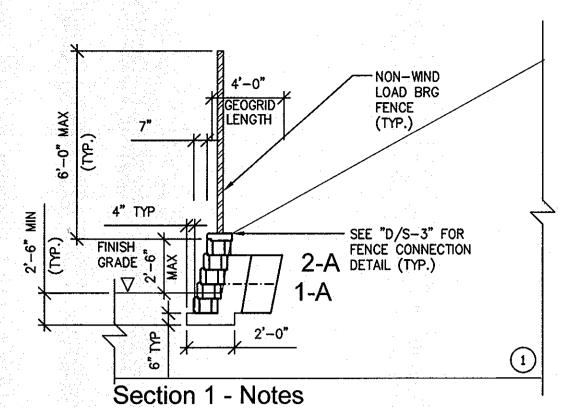
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HOWARD COUNTY, MARYLAND

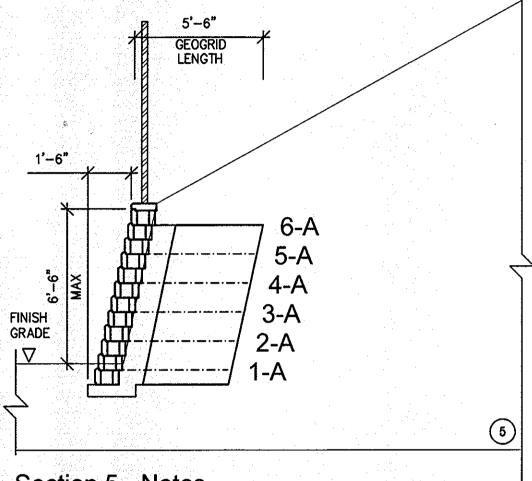




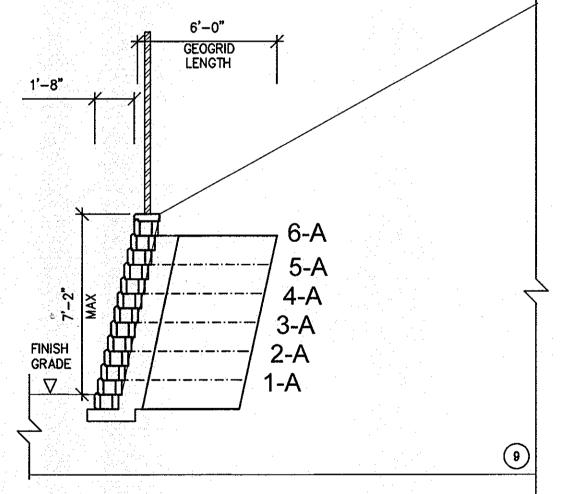




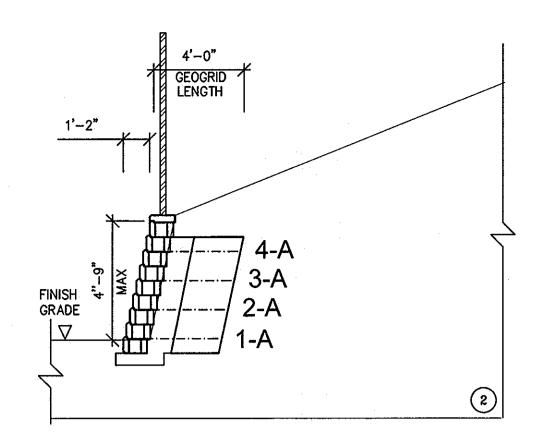
*Setback distance is approximate and does not consider curves, radii or corners.



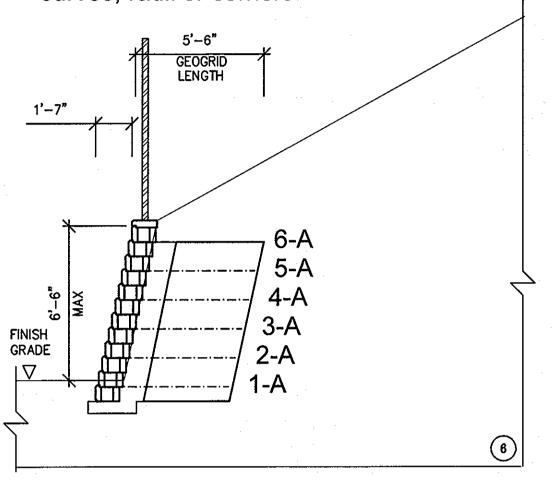
Section 5 - Notes *Setback distance is approximate and does not consider curves, radii or corners.



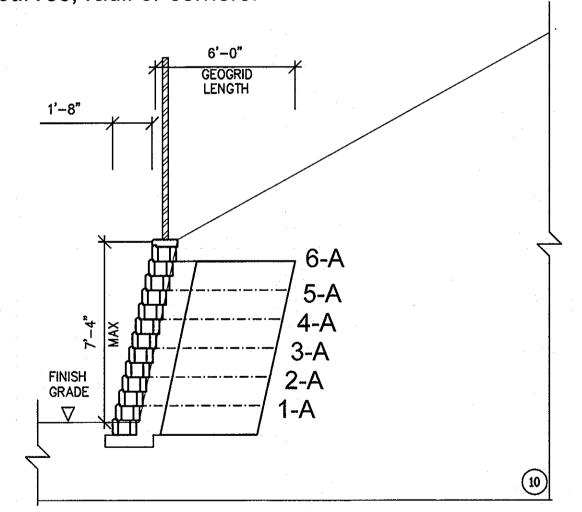
Section 9 - Notes *Setback distance is approximate and does not consider curves, radii or corners.



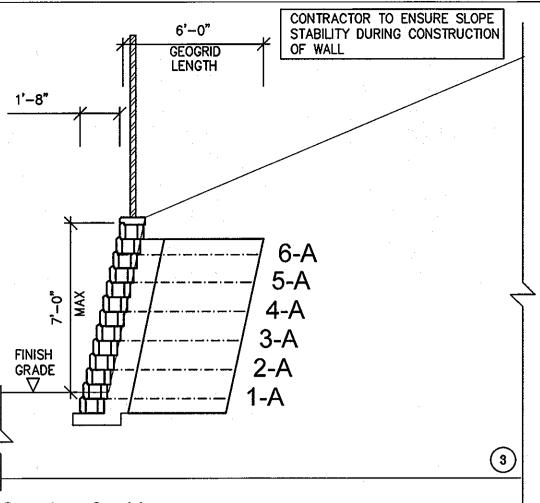
Section 2 - Notes *Setback distance is approximate and does not consider curves, radii or corners.



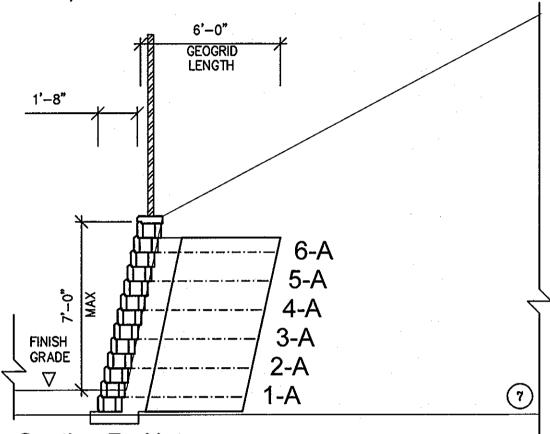
Section 6 - Notes *Setback distance is approximate and does not consider curves, radii or corners.



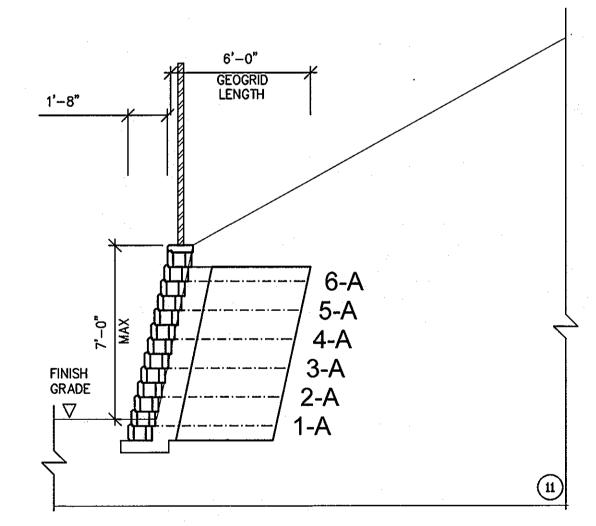
Section 10 - Notes *Setback distance is approximate and does not consider curves, radii or corners.



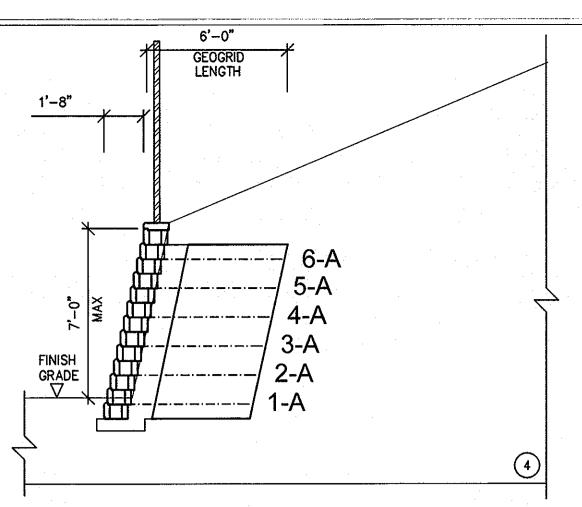
Section 3 - Notes
*Setback distance is approximate and does not consider curves, radii or corners.



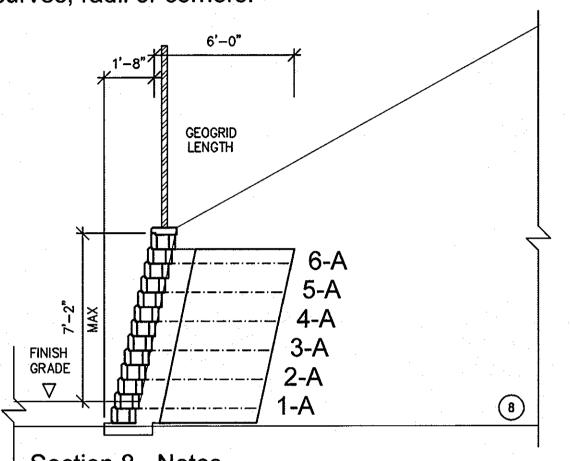
Section 7 - Notes *Setback distance is approximate and does not consider curves, radii or corners.



Section 11 - Notes *Setback distance is approximate and does not consider curves, radii or corners.



Section 4 - Notes *Setback distance is approximate and does not consider curves, radii or corners.



Section 8 - Notes *Setback distance is approximate and does not consider curves, radii or corners.

1-A DENOTES GEOGRID LEVEL TYPICAL. GEOGRID HAS BEEN DESIGNED USING THE SOIL DATA PROVIDED IN THE GEOTECHNICAL REPORT BY HERBST/BENSON ASSOCIATES DATED 8/31/2007. IF SITE CONDITIONS VARY, LENGTH OF GEOGRID MAY NEED TO BE INCREASED AT THE DIRECTION OF THE GEOTECHNICAL ENGINEER. SEE DETAIL 'C/S-3' FOR MINIMUM BEARING ELEVATION COORDINATE WITH ALLAN BLOCK REQUIREMENTS

SEE DETAIL 'A/S-3' FOR DRAINAGE.



Professional Certification. Lehereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 16519 ____

Expiration Date 06/11/09.*

OWNER/DEVELOPER

TROY HILL SQUARE, LLC c/o SERGIO ACLE

10132 BALTIMORE NATIONAL PIKE, SUITE A

ELLICOTT CITY, MD 21042

410.461.4400

SECTIONS AND ELEVATION TROY HILL CORPORATE CENTER PARCEL A-30

RESTAURANT AND RETAIL CENTER

TAX MAP 37 GRID 18 1ST ELECTION DISTRICT

PARCEL 135 HOWARD COUNTY, MARYLAND

Sill · Adcock & Associates · LLC Engineers Surveyors Planners

CHECKED BY: CMS SCALE: AS NOTED DATE: MARCH 18, 2007 3300 North Ridge Road, Suite 160
Ellicott City, Maryland 21043
Phone: 443.325.7682 Fax: 443.325.7685
Email: info@saaland.com PROJECT#: 07456 SHEET#: 9 of 12

WALL #1 ELEVATION N.T.S.

1. SEE S-3 FOR TYPICAL DETAILS "A-E" AND GENERAL NOTES.

*CONTINUE SECTIONS ON S-5

NOTES:

1. DESIGN SHOWN IS FOR ALLAN BLOCK AB STONES WITH MIRAGRID 3xT FABRIC. IF A DIFFERENT SYSTEM IS USED DESIGN SHOWN MAY NOT APPLY.

2. SEE 'D/S-3' FOR FENCE DETAILS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

General Notes



DESIGN LOADS

- A. THE STRUCTURE WAS DESIGNED FOR THE LIVE LOADS SHOWN BELOW AND DEAD LOADS AS REQUIRED BY CONSTRUCTION IN ACCORDANCE WITH IBC 2003.
- B. LIVE LOADS SHOWN BELOW ARE IN POUNDS PER SQUARE FOOT. * PARKING SURCHARGE: 250 PSF STAIRS SURCHARGE: 100 PSF



SUBMITTALS

- A. BEFORE SUBMISSION OF SHOP DRAWINGS. CONTRACTOR SHALL HAVE DETERMINED AND VERIFIED QUANTITIES, DIMENSIONS, SPECIFIED PERFORMANCE CRITERIA, INSTALLATION REQUIREMENTS, MATERIALS, CATALOG NUMBERS AND SIMILAR DATA WITH RESPECT THERETO AND REVIEWED OR COORDINATED EACH SHOP DRAWING WITH OTHER SHOP DRAWINGS AND SAMPLES AND WITH THE REQUIREMENTS OF THE WORK AND THE CONTRACT DOCUMENTS.
- B. AFTER CHECKING AND VERIFYING COMPLIANCE WITH CONTRACT DOCUMENTS AND ACTUAL FIELD CONDITIONS, CONTRACTOR SHALL SUBMIT, FOR REVIEW, SHOP DRAWINGS REFERENCED IN THE INDIVIDUAL MATERIALS SECTIONS. CONTRACTOR SHALL STAMP OR PROVIDE A SIMILAR WRITTEN INDICATION THAT CONTRACTOR HAS REVIEWED THE SUBMISSION AND IS SATISFIED THAT MATERIALS SHOWN ARE IN COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- C. A REVIEW PERIOD OF 5 WORKING DAYS WILL BE REQUIRED FOR SHOP DRAWING REVIEW, OF EACH UNIT TYPE. SHOP DRAWING SUBMISSION OF MULTIPLE COMPONENT TYPES WILL REQUIRE ADDITIONAL REVIEW TIME. SHOP DRAWINGS WILL BE FORWARDED TO ARCHITECT OR CLIENT FOR THEIR REVIEW BEFORE RETURNING TO THE CONTRACTOR



FOUNDATIONS

- A. A SOIL BEARING CAPACITY OF 4000 P.S.F. WAS USED FOR FOOTING DESIGN. IF SOIL OF THIS CAPACITY IS NOT ENCOUNTERED AT ELEVATIONS INDICATED. INCREASE FOOTING SIZE OR LOWER AS DIRECTED BY THE GEOTECHNICAL ENGINEER.
- B. INSTALL EXTERIOR FOOTING BOTTOMS 2'-6" MINIMUM BELOW FINISH GRADE. C. COMPACT FILL AND BACKFILL TO 95% OF A.S.T.M D-698 (1557). PERFORM FILL AND BACKFILL OPERATIONS UNDER THE DIRECT SUPERVISION OF THE
 - D. PRIOR TO POURING CONCRETE, ENGAGE THE SERVICES OF A PROFESSIONAL GEOTECHNICAL ENGINEER (REGISTERED IN MARYLAND). TO PERFORM TESTS. BORINGS, ETC. REQUIRED TO CERTIFY THAT THE SOIL BEARING CAPACITY MEETS OR EXCEEDS THAT SHOWN IN THE GENERAL NOTES ABOVE. GEOTECHNICAL ENGINEER SHALL VERIFY SUBGRADE CAPACITIES PRIOR TO INSTALLATION OF DRAINAGE FILL AND MOISTURE BARRIER.

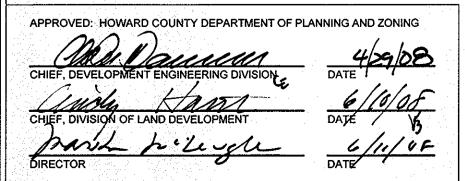


CONCRETE

- A. CONCRETE WORK INCLUDING FORMING, MIXING, PLACING AND CURING SHALL BE IN ACCORDANCE WITH A.C.I. 301.
- B. SUBMIT COMPLETE SHOP AND ERECTION DRAWINGS FOR REVIEW PRIOR TO FABRICATION OR ERECTION. REPRINTS OF CONTRACT DRAWINGS ARE NOT ACCEPTABLE. SUBMIT DESIGN MIXES FOR EACH CLASS OF CONCRETE PRIOR TO
- CONCRETE REINFORCING: A.S.T.M. A-615, GRADE 60.
- PORTLAND CEMENT: A.S.T.M. C-150, TYPE I

F. FLY ASH: A.S.T.M. C-618, TYPE F MAX 25%.

- BLENDED HYDRALIC CEMENT: A.S.T.M. C-595.
- G. AGGREGATE: A.S.T.M. C-33. 1" MAXIMUM FOR FOOTINGS, WALLS AND SLABS ON GRADE, K" MAXIMUM FOR THIN SLABS AND 3/8" FOR WALL FILL
- H. WATER CEMENT RATIO NOT TO EXCEED .45 FOR AIR ENTRAINED CONCRETE
- REINFORCING FOR FOOTINGS AND OTHER CONCRETE USING EARTH FORMS SHALL HAVE 3" CONCRETE COVER. REINFORCING FOR CONCRETE EXPOSED TO GROUND OR WEATHER AFTER REMOVAL OF FORMS SHALL HAVE 2" CONCRETE COVER. REINFORCING SHALL HAVE 3/4" CONCRETE COVER FOR SLABS AND WALLS AND 1 1/2" COVER FOR BEAMS, GIRDERS & COLUMNS



- J. LAP CONTINUOUS FOOTING REINFORCING 36 BAR DIAMETERS AT SPLICES.
 - K. USE A WATER REDUCING ADMIXTURE IN ALL CONCRETE.
 - USE A MINIMUM OF 5 1/2 BAGS OF CEMENT AND A MAXIMUM OF 6 1/2 GALLONS OF WATER PER BAG FOR EACH CUBIC YARD OF CONCRETE.
 - M. SLUMP AS REQUIRED BY ACI (211.1). SHOULD EXTRA WATER BE REQUIRED BEFORE DEPOSITING CONCRETE AND WATER/CEMENT RATIO OF ACCEPTED MIX DESIGN HAS NOT BEEN EXCEEDED, GENERAL CONTRACTOR'S SUPERINTENDENT SHALL HAVE SOLE AUTHORITY TO AUTHORIZE ADDITION OF WATER. ANY ADDITIONAL WATER ADDED TO MIX AFTER LEAVING BATCH PLANT SHALL BE INDICATED ON THE TRUCK TICKET AND SIGNED BY PERSON RESPONSIBLE. SUBMIT COPY OF TRUCK TICKET FOR REVIEW.
 - N. NO CALCIUM CHLORIDE WILL BE PERMITTED IN CONCRETE.
 - O. ENGAGE THE SERVICES OF A TESTING AGENCY APPROVED BY THE ARCHITECT TO PERFORM TESTS OF CONCRETE. TAKE A MINIMUM OF 5 CYLINDERS FOR EACH CLASS OF CONCRETE POURED IN ANY ONE DAY. PERFORM 1 SLUMP TEST PER TRUCK LOAD OF CONCRETE.
 - PROVIDE TWO COMPRESSION TESTS AT 7 DAYS, TWO AT 28 DAYS, AND RETAIN ONE TEST FOR ADDITIONAL TESTING AS REQUIRED.

(4.1)

MASONRY

- MANUFACTURE AND INSTALL MASONRY IN ACCORDANCE WITH (ACI 530/ASCE 5/TMS 402), (ACI 530.1/ASCE 6/TMS 602). BLOCK: CONCRETE MASONRY UNITS: 1,900 PSI COMPRESSIVE STRENGTH (AVERAGE OF THREE UNITS). DESIGNED F'M: 1500 PSI. A.S.T.M. C-90 WITH MINIMUM DENSITY OF 125 LBS. PER CU. FT. FOR NORMAL WEIGHT UNITS.
- MORTAR: A.S.T.M. C-270 TYPE S.
- GROUT: A.S.T.M. C-476 (NON SHRINK, NON METALLIC).
- D. REINFORCING: A.S.T.M. A-615, GRADE 60.
- E. BLOCK SHALL HAVE GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCING AT 16" O/C MAXIMUM WITH PREFABRICATED CORNER AND "T" PIECES UNLESS NOTED. PARAPET WALLS SHALL HAVE HORIZONTAL JOINT REINFORCING AT 8" O/C. LAP SPLICES 6" MIN. STOP HORIZONTAL JOINT REINFORCING EACH SIDE OF CONTROL AND EXPANSION JOINTS.
- HORIZONTAL JOINT REINFORCING SHALL BE IN ACCORDANCE WITH ASTM A951 AND SHALL BE MANUFACTURED FROM 9 GAGE (Ø.148) MIN. COLD DRAWN STEEL WIRE CONFORMING TO ASTM A-82 AND SHALL CONSIST OF TWO DEFORMED LONGITUDINAL SIDE RODS WELDED AT 16" PLUS OR MINUS INTERVALS TO A PERPENDICULAR CROSS ROD FORMING A LADDER DESIGN. CROSS ROD AND SIDE RODS SHALL BE LOCATED IN THE SAME PLANE AS THE LONGITUDINAL RODS. OUT TO OUT SPACING OF SIDE RODS SHALL BE APPROXIMATELY 2" LESS THAN THE NOMINAL WALL THICKNESS.
- JOINT REINFORCEMENT IN EXTERIOR WALLS TO BE HOT DIPPED GALVANIZED. AFTER FABRICATION, IN ACCORDANCE WITH ASTM A-153 CLASS B2 (1.80 OZ./SQ. FT.).
- BLOCK SHALL BE LAID IN FULL BED OF MORTAR INCLUDING CROSSWEBS. WALLS NOTED AS FILLED SOLID SHALL HAVE CORES OF BLOCK FILLED WITH
- PEA GRAVEL CONCRETE IN SIX COURSE MAXIMUM LIFTS. PROVIDE CONTROL JOINTS AT 30' MAXIMUM ON CENTER IN MASONRY WALLS.
- K. LAP SPLICES IN REINFORCING 48 BAR DIAMETER MINIMUM. UNLESS NOTED OTHERWISE VERTICAL REINFORCING TO BE FULL HEIGHT OF WALL AND DOWELED INTO FOOTINGS.
- FILL CELLS OF BLOCK CONTAINING VERTICAL REINFORCING WITH PEA GRAVEL CONCRETE IN SIX COURSE MAX. LIFTS. CONCRETE TO CONTAIN AN APPROVED WATER REDUCER

〈 5.9)

STEEL GUARDRAILS/HANDRAILS

- MANUFACTURER SHALL DESIGN GUARDRAILS / HANDRAILS AND CONNECTIONS TO STRUCTURE AT BALCONIES, STAIRWELLS, RAMPS AND FLOOR OPENINGS TO SUPPORT THE FOLLOWING DESIGN LOADS: 50 POUNDS PER LINEAL FOOT OR 2000 POUND CONCENTRATED LOAD, WHICHEVER IS GREATER, APPLIED AT ANY POINT AND IN ANY DIRECTION TO TOP OF RAIL, AND 50 POUND CONCENTRATED LOAD APPLIED ON A 1-SQUARE-FOOT AREA AT ANY POINT FOR REMAINING GUARDRAIL INFILL COMPONENTS. SUBMIT SHOP AND ERECTION DRAWINGS INDICATING SIZES AND CONNECTIONS OF GUARDRAIL AND HANDRAIL COMPONENTS.
- SUBMIT MANUFACTURER'S SHOP DRAWINGS CONTAINING A CERTIFICATION SEALED BY A PROFESSIONAL ENGINEER (REGISTERED IN MARYLAND) STATING THAT THE GUARDRAIL AND HANDRAIL COMPONENTS HAVE BEEN DESIGNED TO SUPPORT THE SPECIFIED LOADS.

PAF POWER ACTIVATED FASTENER R PLATE EWB EACH WAY BOT EX EXISTING EXP EXPANSION PSF POUNDS PER SQUARE FOOT AB ANCHOR BOLTS EXT EXTERIOR PSI POUNDS PER SQUARE INCH FIN FINISHED FF FINISHED FLOOR FND FOUNDATION PSL PARALLEL STRAND LUMBER PLWD PLYWOOD PT PRESSURE TREATED RCP REINFORCED CONCRETE PIPE REINF REINFORCING BLKG BLOCKING GA GAUGE GALV GALVANIZED GC GENERAL CONTRACTOR GEN GENERAL REQ'D REQUIRED BOTTOM CHORD SOG SLAB ON GRAD GLB GLUE LAMINATED BEAM SPF SPRUCE PINE FIR HGR HANGER STIFF STIFFENER TC TOP CHORD HT HEIGHT HORIZ HORIZONTAL T&B TOP AND BOTTOM HSS TUBE STEEL/PIPE COLUMN T&G TOUNGUE AND GROOVE T&B TOP AND BOTTOM /FTG TOP OF FOOTING CONN | CONNECTION TOS TOP OF STEEL. THRU THROUGH OORD COORDINATE TYP TYPICAL UNO UNLESS NOTED OTHERWISE VERT VERTICAL LLH LONG LEG HORIZONTAL LLV LONG LEG VERTICAL LLV LONG LEG VERTICAL WF VERIFY IN FIELD LVL LAMINATED VENEER LUMBER W WIDE FLANGE BEAM LG LONG LT GA LIGHT GAUGE W/O MITH OUT DF DOUGLAS FIR MATL MATERIAL WWF WELDED WIRE FABRIC MAX MAXIMUM MIDHT MID HEIGTH MIN MINIMUM

MNFR MANUFACTURER
NIC NOT IN CONTRACT
NTS NOT TO SCALE
OC ON CENTER
OPNG OPENING

EJ EXPANSION JOINT

STRUCTURAL ABBREVIATIONS

MARK

ES EACH SIDE

SYMBOLS

l all typical details, sections, and notes are general in nature

---- TYPICAL DETAIL LETTER

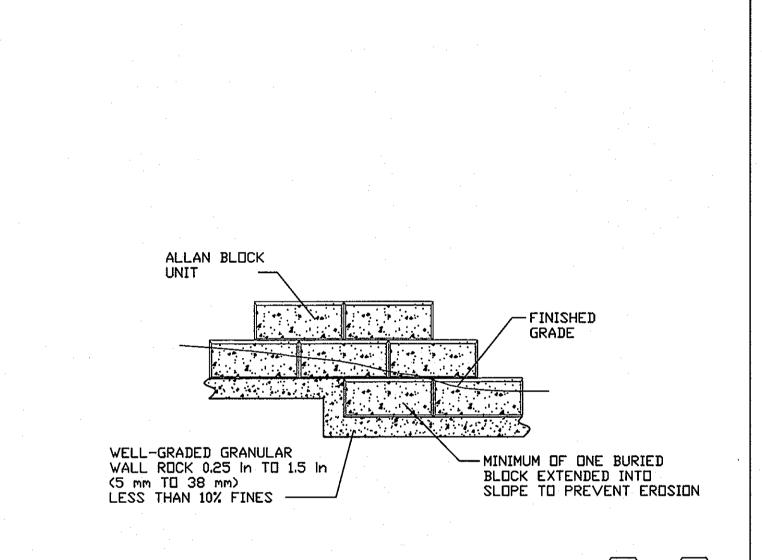
--- SHEET WHERE DETAIL IS DRAWN

sheet where section is drawn

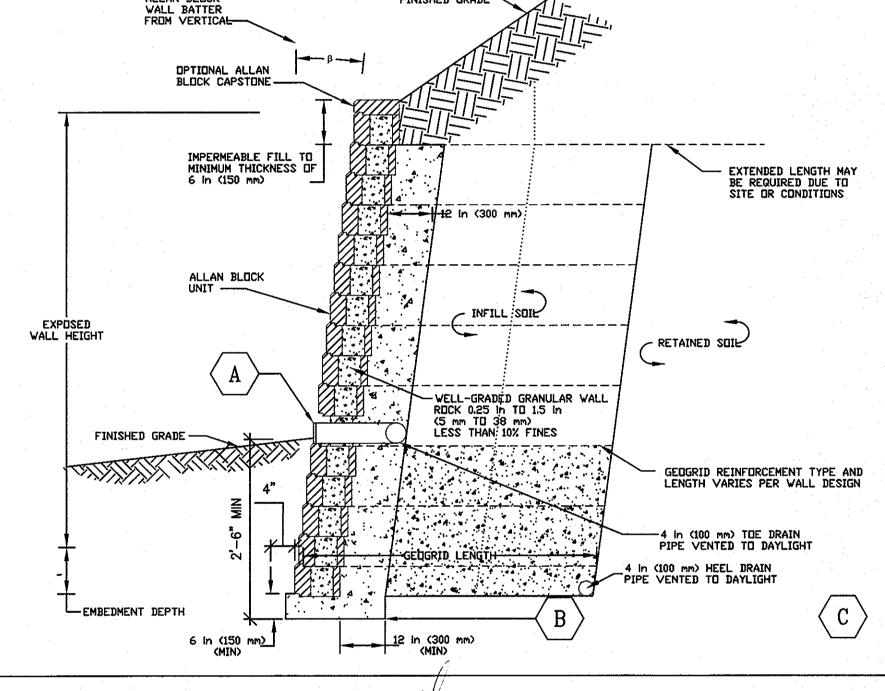
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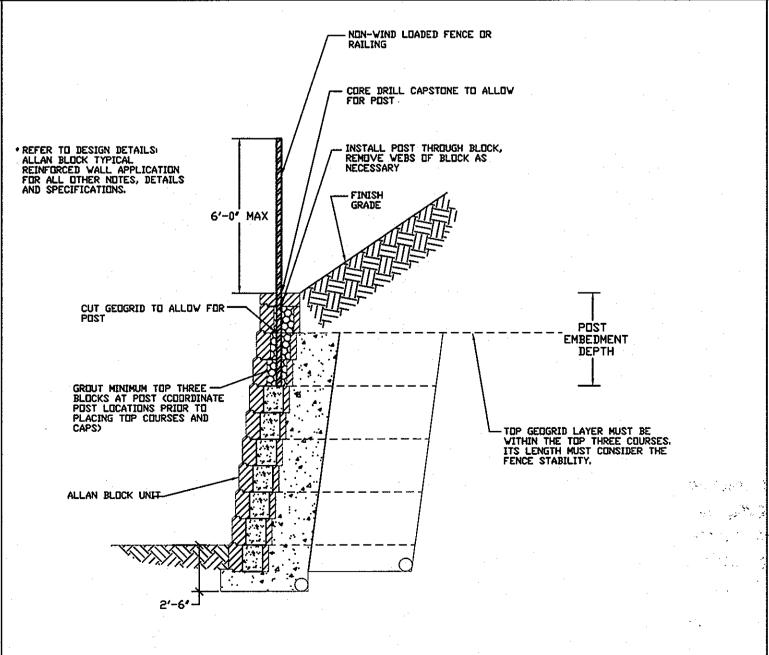
AND USAGE IS NOT LIMITED TO WHERE SPECIFICALLY NOTED.

* REFER TO DESIGN DETAILS: ALLAN BLOCK TYPICAL REINFORCED WALL APPLICATION FOR ALL OTHER NOTES, DETAILS AND SPECIFICATIONS. ALLAN BLOCK CUT NOTCH IN ALLAN BLOCK TO ALLOW FOR DRAIN PIPE. RODENT SCREEN AS REQUIRED. — WELL-GRADED GRANULAR WALL ROCK 0.25 In to 1.5 In (5 mm to 38 mm) LESS THAN 10% FINES In (100 mm) TOE DRAIN PIPE VENTED THROUGH THE FACE OF THE WALL AT THE LOWEST FINISHED GRADE TO POSSIBLE ELEVATION ON 10 ft SLOPE AWAY FROM CENTERS MAXIMUM 2'-6**'**— THE WALL FACE ______ IMPERVIOUS SOIL-OR SURFACE LOW PERMEABILITY GRANULAR MATERIAL LEVEL GRADE BELOW PLACED IN CORES OF THE ALLAN BLOCK AS WELL AS IN THE INFILL SOIL-TYPICAL DRAINAGE DETAIL



ALLAN BLOCK TYPICAL BASE DETAIL (D)







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"Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 16519 Expiration Date 06/11/09.

OWNER/DEVELOPER

TROY HILL SQUARE, LLC c/o SERGIO ACLE 10132 BALTIMORE NATIONAL PIKE, SUITE A ELLICOTT CITY, MD 21042 410.461.4400

GENERAL NOTES & DETAILS TROY HILL CORPORATE CENTER PARCEL A-30

TAX MAP 37 GRID 18 **1ST ELECTION DISTRICT**

RESTAURANT AND RETAIL CENTER HOWARD COUNTY, MARYLAND

|Sill Adcock & Associates · LLC

Engineers · Surveyors · Planners 3300 North Ridge Road, Suite 160 Ellicott City, Maryland 21043 Phone: 443.325.7682 Fax: 443.325.7685 Email: info@saaland.com

CHECKED BY: CMS N.T.S SCALE: ___ DATE: MARCH 18, 2007 PROJECT #: ____07456 SHEET#: 10 of 12

PARCEL 135

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SDP-08-029

DESIGN BY:

ALLAN BLOCK SPECIFICATIONS

PART 1: GENERAL

Work includes furnishing and installing modular concrete block retaining wall units to the lines and grades designated on the construction drawings and as specified herein.

1.2 Applicable Sections of Related Work

Geogrid Wall Reinforcement (see section 2)

1.3 Reference Standards Unless over wise noted most recent standard should be used

ASTM C1372 Standard Specification for Segmental Retaining Wall Units.

ASTM 1262 Evaluating the Freeze thaw Durability of Manufactured CNU's and Related concrete Units ASTM D698 Moisture Density Relationship for Soils, Standard Method

ASTM D422 Gradation of Soils

ASTM C140 Sample and Testing concrete Masonry Units

1.4 Delivery, Storage, and Handling

A. Contractor shall check the materials upon delivery to assure proper material has been received.

- B. Contractor shall prevent excessive mud, wet cement, and like construction debris from coming in contact
- C. Contractor shall protect the materials from damage. Damaged material shall not be incorporated in the project (ASTM C1372)

PART 2: MATERIALS

2.1 Modular Wall Units

A. Wall units shall be produced by a licensed manufacturer.

- 8. Wall units shall have minimum 28 day compressive strength of 3000 psi (20.7 MPa) in accordance with ASTM C1372. The concrete units shall have adequate freeze—thaw protection in accordance with ASTM C1372 or an average absorption rate of 7.5 lb/ft3 (120 kg/m3) for northern climates and 10 lb/ft3 (160 kg/m3) for southern climates.
- C. Exterior dimensions shall be uniform and consistent. Maximum dimensional deviations on the height of any two units shall be 0.125 in. (3 mm).
- D. Wall units shall provide a minimum of 110 lbs total weight per square foot of wall face area (555 kg/m2). Fill contained within the units may be considered 80% effective weight.

E. Exterior face shall be textured. Color as specified by owner.

2.3 Infil Soil

- A. Material must be well-graded compactable aggregate, 0.25 in. to 1.5 in., (6 mm 38 mm) with no more than 10% possing the \$200 sieve. (ASTM D422)
- B. Material behind and within the blocks may be the same material.

A. Infill material shall be site excavated soils when approved by the on-site soils engineer unless otherwise specified in the drawings. Unsuitable soils for backfill (heavy clays or organic soils) shall not be used in the

reinforced soil mass. Fine grained cohesive soils (f<31°) may be used in wall construction, but additional

Ackfilling, compaction and water management efforts are required. Poorly graded sands, expansive clays and/or soils with a plasticity index (PI) >20 or a liquid limit (LL) >40 should not be used in wall

B. The infill soil used must meet or exceed the designed friction angle and description noted on the design cross sections, and must be free of debris and consist of one of the following inorganic USCS soil types: GP, GW, SW, SP, SM, SN-SC meeting the following gradation as determined in accordance with ASTN D422.

Sieve Size	Percent Passing	
4 inch	100 75	
No. 4	100 20	
No. 40	0 - 60	
No. 200	0 - 35	

C. Where additional fill is required, contractor shall submit sample and specifications to the wall design engineer or the on-site soils engineer for approval and the approving engineer must certify that the soils proposed for use has properties meeting or exceeding original design standards.

PART 3: WALL CONSTRUCTION

3.1 Excavation

- A. Contractor shall excavate to the lines and grades shown on the construction drawings. Contractor shall use caution not to over-excavate beyond the lines shown, or to disturb the base elevations beyond those shown.
- B. Contractor shall verify locations of existing structures and utilities prior to excavation. Contractor shall ensure all surrounding structures are protected from the effects of wall excavation.
- 3.2 Foundation Soil Preparation
 - A. Foundation soil shall be defined as any soils located beneath a wall.
 - B. Foundation soil shall be excavated as dimensioned on the plans and compacted to a minimum of 95% of Standard Proctor (ASTM D698) prior to placement of the base material.
 - C. Foundation soil shall be examined by the on-site soils engineer to ensure that the actual foundation soil strength meets or exceeds assumed design strength. Soil not meeting the required strength shall be removed and replaced with acceptable material.

A. Base material shall be placed as shown on the construction drawing. Top of base shall be located to allow bottom wall units to be buried to proper depths as per wall heights and specifications.

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- B. Base material shall be installed on undisturbed native soils or suitable replacement fills compacted to a minimum of 95% Standard Proctor (ASTN D698).
- C. Base shall be compacted at 95% Standard Proctor (ASTM D698) to provide a level hard surface on which to coating specifically fabricated for use as a soil reinforcement material. place the first course of blocks. The base shall be constructed to ensure proper wall embedment and the final elevation shown on the plans. Well-graded sand can be used to smooth the top 1/2 in. (13 mm) on the base
- D. Base material shall be a 4 in. (100 mm) minimum depth for walls under 4 ft (1.2 m) and a 6 in. (150 mm) minimum depth for walls over 4 ft (1.2 m).

- A. The first course of wall units shall be placed on the prepared base with the raised lip facing up and out and the front edges tight together. The units shall be checked for level and alignment as they are placed.
- B. Ensure that units are in full contact with base. Proper care shall be taken to develop straight lines and smooth
- C. Fill all cores and cavities and a minimum of 12 in. (300 mm) behind the base course with wall rock. Use approved soils to backfill behind the wall rock and in front of the base course to firmly lock in place. Check again for level and alignment. Use a plate compactor to consolidate the area behind the base course. All excess material shall be swept from top of units.
- D. Install next course of wall units on top of base row. Position blocks to be offset from seams of blocks below. Perfect "running bond" is not essential, but a 3 in. (75 mm) minimum offset is recommended. Check each block for proper alignment and level. Fill all cavities in and around wall units and to a minimum of 12 in. (300 mm) depth behind block with wall rock. For tailer wall application the depth of wall rock behind the block should be increased; walls from 15ft (4.57m) to 25ft (7.62m) should have a minimum of 2ft (0.61m) and walls above 25ft (7.62m) should have a minimum of 3ft (0.91m). Spread backfill in uniform lifts not exceeding 8 in. (200 mm) in uncompacted thickness and compact to 95% of Standard Proctor (ASTM D698) behind the consolidation zone.
- E. The consolidation zone shall be defined as 3 ft (1 m) behind the wall. Compaction within the consolidation zone shall be accomplished by using a hand operated plate compactor and shall begin by running the plate compactor directly on the block and then compacting in parallel paths from the wall face until the entire consolidation zone has been compacted. A minimum of two passes of the plate compactor are required with maximum lifts of 8 in. (200 mm). Expansive or fine-grained soils may require additional compaction passes and/or specific compaction equipment such as a sheepsfoot roller. Maximum lifts of 4 inches (100 mm) may be required to achieve adequate compaction within the consolidation zone. Employ methods using lightweight compaction equipment that will not disrupt the stability or batter of the wall. Final compaction requirements in the consolidation zone shall be established by the engineer of record.
- F. Install each subsequent course in like manner. Repeat procedure to the extent of wall height.
- G. As with any construction work, some deviation from construction drawing alignments will occur. Variability in construction of SRWs is approximately equal to that of cast-in-place concrete retaining walls. As opposed to cast-in-place concrete walls, alignment of SRWs can be simply corrected or modified during construction. Based upon examination of numerous completed SRWs, the following recommended minimum tolerances can be achieved with good construction techniques.

Vertical Control - fi1.25 in. (32 mm) max. over 10 ft (3 m) distance. Horizontal Location Control - straight lines fi1.25 in. (32 mm) over a 10 ft (3 m) distance. Rotation - from established plan wall batter: 2.0

Bulging - 1.0 in. (25 mm) over a 10 ft (3.0 m) distance

3.5 Additional Construction Notes

- A. Filter fabric use is not suggested for use with cohesive soils. Clagging of such fabric creates unacceptable hydrostatic pressures in soil reinforced structures. When filtration is deemed necessary in cohesive soils, use a three dimensional filtration system of clean sand or filtration aggregate.
- C. Water management is of extreme concern during and after construction. Steps must be taken to ensure that drain pipes are properly installed and vented to daylight and a grading plan has been developed that routes water away from the retaining wall location. Site water management is required both during construction of the wall and after completion of construction.

F.Alian Block Geogrid Specs Page of

Specification Guidelines: Geogrid Reinforcement Systems

SECTION 2 PART 1: GENERAL

1.1 Scope

Work includes furnishings and installing geogrid reinforcement, wall fill, and backfill to the lines and grades

designated on the construction drawings and as specified herein. designated on the construction drawings and as specified herein.

1.2 Applicable Section of Related Work

Section 1: Modular Retaining Wall Systems.

1.3 Reference Standards

See specific geogrid manufacturers reference standards.

- A. ASTM D4595 Tensile Properties of Geotextiles by the Wide-Width Strip Method
- B. ASTM D5262 Test Method for Evaluating the Unconfined Creep Behavior of Geogrids
- C. ASTM D6638 Grid Connection Strength (SRW-U1)
- D. ASTM D6916 Grid Shear Strength (SRW-U2)
- E. GRI-GG4 Grid Long Term Allowable Design Strength (LTDS)
- F. ASTN D6706 Test Method for Grid Pullout

1.4 Delivery, Storage, and Handling

- A Contractor shall check the geogrid upon delivery to assure that the proper material has been received.
- B. Geogrid shall be stored above -10 F (-23 C).
- C. Contractor shall prevent excessive mud, wet cement, or other foreign materials from coming in contact with the geogrid material.

PART 2: MATERIALS

2.1 Definitions

- A. Geogrid products shall be of high density polyethylene or polyester yarns encapsulated in a protective
- B. Concrete retaining wall units are as detailed on the drawings and shall be Allon Block Retaining Wall Units.
- C. Drainage material is free draining granular material as defined in Section 1, 2.2 Wall Rock D. Backfill is the soil used as fill for the reinforced soil mass.
- E. Foundation soil is the in-situ soil.

2.2 Products

Geogrid shall be the type as shown on the drawings having the property requirements as described within the manufacturers specifications.

2.3 Acceptable Manufacturers

A manufacturer's product shall be approved by the wall design engineer.

PART 3: WALL CONSTRUCTION

3.1 Foundation Soil Preparation

- A. Foundation soil shall be excavated to the lines and grades as shown on the construction drawings, or as directed by the on-site soils engineer.
- B. Foundation soil shall be examined by the on-site soils engineer to assure that the actual foundation soil strength meets or exceeds assumed design strength.
- C. Over-excavated areas shall be filled with compacted backfill material approved by on-site soils engineer. D. Contractor shall verify locations of existing structures and utilities prior to excavation. Contractor shall

ensure all surrounding structures are protected from the effects of wall excavation. 3.2 Wall Construction

Wall construction shall be as specified under Section 1, Part 3, Wall Construction.

3.3 Geogrid Installation

- A. Install wall to designated height of first geogrid layer. Backfill and compact in layers not to exceed 8 in. (200 mm) lifts behind wall to depth equal to designed grid length before grid is installed.
- B. Out geogrid to designed embedment length and place on top of block to back edge of lip. Extend away from wall approximately 3% above horizontal on compacted backfill.
- C. Lay geogrid at the proper elevation and orientations shown on the construction drawings or as directed by the
- D. Correct orientation of the geogrid shall be verified by the contractor and on-site soils engineer. Strength direction is typically perpendicular to wall face. E. Follow manufacturers guidelines for overlap requirements. In curves and corners, layout shall be as
- specified in Design Details 9-12, see Page 15 of the AB Spec Book. F. Place next course of block on top of grid and fill block cores with wall rock to lock in place. Remove slack
- and folds in grid and stake to hold in place. G. Adjacent sheets of geogrid shall be butted against each other at the wall face to achieve 100 percent
- H. Geogrid lengths shall be continuous. Splicing parallel to the wall face is not allowed.
- 3.4 Fill Placement and Backfill Placement B. Backfill shall be placed, spread and compacted in such a manner that minimizes the development of slock or 1.5 Heel Drain
- movement of the geogrid. C. Only hand-operated compaction equipment shall be allowed within 3 ft (1 m) behind the wall. This area shall be defined as the consolidation zone. Compaction in this zone shall begin by running the plate compactor directly on the block and then compacting in parallel paths to the wall face until the entire consolidation zone has been compacted. A minimum of two passes of the plate compactor are required with
- D. When fill is placed and compaction cannot be defined in terms of Standard Proctor Density, then compaction shall be performed using ordinary compaction process and compacted so that no deformation is observed
- from the compaction equipment or to the satisfaction of the engineer of record or the site soils engineer. E. Tracked construction equipment shall not be operated directly on the geogrid. A minimum backfill thickness of 6 in. (150 mm) is required prior to operation of tracked vehicles over the geogrid. Turning of tracked vehicles should be kept to a minimum to prevent tracks from displacing the fill and damaging the geogrid.
- F. Rubber-tired equipment may pass over the geogrid reinforcement at slow speeds, less than 10 mph (16 Km/h). Sudden braking and sharp turning shall be avoided. G. The infill shall be compacted to achieve 95% Standard Proctor (ASTM D698). Compaction tests shall be taken at 3 ft (1 m) behind the block and at the back of the reinforced zone and frequency shall be as
- determined by the on-site soils engineer or as specified on the plan. Soil tests of the backfill material shall be submitted to the on-site soils engineer for review and approval prior to the placement of any backfill. The contractor is responsible for achieving the specified compaction requirements. The on-site soils engineer may direct the contractor to remove, correct or amend any soil found not in compliance with these written
- A.Alian Block Water Management Specs Page of Specification Guidelines: Water Management

maximum lifts of 8 in. (200 mm).

PART 1: GENERAL DRAINAGE

retaining wall system.

1.1 Surface Drainage

- A. At the end of each day's construction and at final completion, grade the backfill to avoid water accumulation behind the wall or in the reinforced zone.
- B. Surface water must not be allowed to pond or be trapped in the area above the wall or at the toe of
- C. Existing slopes adjacent to retaining wall or slopes created during the grading process shall include drainage details so that surface water will not be allowed to drain over the top of the slope face and/or wall. This may require a combination of berms and surface drainage ditches.

D. Irrigation activities at the site shall be done in a controlled and reasonable manner. If an irrigation

system is employed, the design engineer or irrigation manufacture shall provide details and specification for

required equipment to ensure against over irrigation which could damage the structural integrity of the

- D. Irrigation activities at the site shall be done in a controlled and reasonable manner. If an irrigation system is employed, the design engineer or irrigation manufacture shall provide details and specification for required equipment to ensure against over irrigation which could damage the structural integrity of the retaining wall system.
- E. Surface water that cannot be diverted from the wall must be collected with surface drainage swales and drained laterally in order to disperse the water around the wall structure. Construction of a typical swale system shall be in accordance with Design Detail 5: Swales, on page 14 of the Allan Block Spec Book.

C. Existing slopes adjacent to retaining wall or slopes created during the grading process shall include drainage details so that surface water will not be allowed to drain over the top of the slope face and/or wall. This

- A. Establish final grade with a positive gradient away from the wall structure. Concentrations of surface water runoff shall be managed by providing necessary structures, such as paved ditches, drainage swales, catch basins, etc.
- B. Grading designs must divert sources of concentrated surface flow, such as parking lots, away from

1.3 Drainage System

- A. All walls will be constructed with a minimum of 12 in. (300 mm) of wall rock directly behind the wall facing. The material shall meet or exceed the specification for wall rock outlined in Section 1, 2.2 Wall Rock.
- B. The drainage collection pipe, drain pipe, shall be a 4 in. (100 mm) perforated or slotted PVC, or

corrugated HDPE pipe as approved by engineer of record.

may require a combination of berms and surface drainage ditches.

- C. All walls will be constructed with a 4 in. (100 mm) diameter drain pipe placed at the lowest possible elevation within the 12 in. (300 mm) of wall rock. This drain pipe is referred to as a toe drain, Section 3,
- D. Geogrid Reinforced Walls shall be constructed with an additional 4 in. (100 mm) drain pipe at the back bottom of the reinforced soil mass. This drain pipe is referred to as a heel drain, Section 3, 1.5 Heel Drain.
 - A. For site configurations with bottoms of the base on a level plane it is recommended that a minimum one percent gradient be maintained on the placement of the pipe with outlets on 50 ft (15) m) centers, or 100 ft (30 m) centers if pipe is crowned between the outlets. This would provide for a maximum height above the bottom of the base in a flat configuration of no more than 6 in. (150
 - B. For rigid drain pipes with drain holes the pipes should be positioned with the holes located down. Allan Block does not require that toe drain pipes be wrapped when installed into base rock complying with the specified wall rock material.

C. Pipes shall be routed to storm drains where appropriate or through or under the wall at low points

- when the job site grading and site layout allows for routing. Appropriate details shall be included to prevent pipes from being crushed, plugged, or infested with rodents. D. On sites where the natural drop in grade exceeds the one percent minimum, drain pipe outlets shall be on 100 foot (30 m) centers, maximum. This will provide outlets in the event that excessive
- water flow exceeds the capacity of pipe over long stretches. E. When the drain pipe must be raised to accommodate outlets through the wall face, refer to the

Design Detail 4: Alternate Drain, on page 14 of the Allan Block Spec Book.

1.6 Ground Water

- A. The piping used at the back of the reinforced mass shall have a one percent minimum gradient over the length, but it is not critical for it to be positioned at the very bottom of the cut. Additionally the entire length of the pipe may be vented at one point and should not be tied into
- B. The pipe may be a rigid pipe with holes at the bottom with an integral sock encasing the pipe or a corrugated perforated flexible pipe with a sock to filter out fines when required based on soil conditions. For infill soils with a high percentage of sand and/or gravel the heel drain pipe does not need to be surrounded by drainage rock. When working with soils containing more than fifty percent clay, one cubic foot of drainage rock is required for each foot of pipe.
- A. If water is encountered in the area of the wall during excavation or construction, a drainage system (chimney, composite or blanket) must be installed as directed by the wall design engineer.
- B. Standard retaining wall designs do not include hydrostatic forces associated with the presence of ground water. If adequate drainage is not provided the retaining wall design must consider the presence of the water may not be adequate for the additional hydrostatic pressure.
- C. When non-free draining soils are used in the retained zone, the incorporation of a chimney and blanket drain should be added to minimize the water penetration into the reinforced mass. Refer to

1.7 Concentrated Water Sources

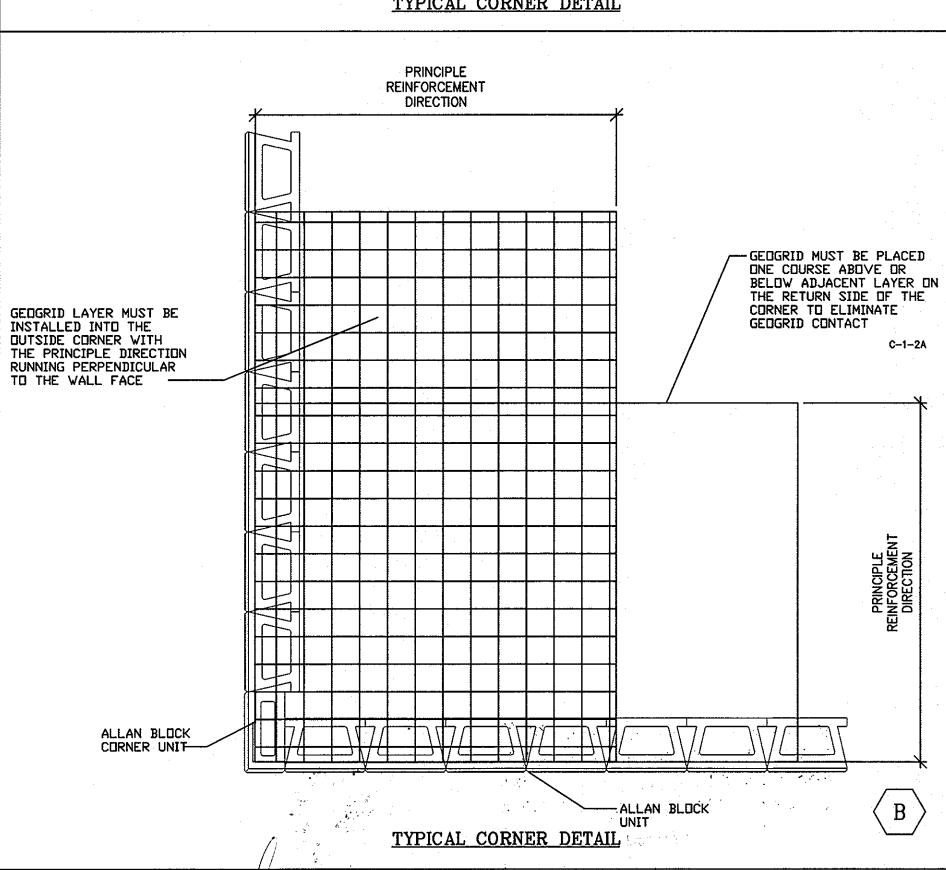
away from the retaining wall structure.

- A. All roof downspouts of nearby structures shall be sized with adequate capacity to carry storm water from the roof away from the wall area. They shall be connected to a drainage system in closed pipe and routed around the retaining wall area.
- B. Site layout must take into account locations of retaining wall structures and all site drainage paths. Drainage paths should always be away from retaining wall structures.
- C. Storm sewers and catch basins shall be located away from retaining wall structures and designed so as not to introduce any incidental water into the reinforced soil mass.

D. A path to route storm sewer overflow must be incorporated into the site layout to direct water

Expiration Date 06/11/09:

SUBSEQUENT GEOGRID LAYERS SHOULD EXTEND ONE QUARTER OF THE WALL HEIGHT PAST THE CORNER LOCATION IN ALTERNATE DIRECTIONS TALLAN BLOCK PRINCPLE REINFORCEMENT DIRECTION TYPICAL CORNER DETAIL





"Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 16519

OWNER/DEVELOPER TROY HILL SQUARE, LLC

c/o SERGIO ACLE 10132 BALTIMORE NATIONAL PIKE, SUITE A ELLICOTT CITY, MD 21042

Allan Block Specifications & Details TROY HILL CORPORATE CENTER PARCEL A-30

RESTAURANT AND RETAIL CENTER

TAX MAP 37 GRID 18 1ST ELECTION DISTRICT

> Sill · Adcock & Associates · LLC Engineers · Surveyors · Planners

3300 North Ridge Road, Suite 160 Ellicott City, Maryland 21043 Phone: 443.325.7682 Fax: 443.325.7685

SHEET#: 11 of 12

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DATE: MARCH 18, 2007

PROJECT#: 07456

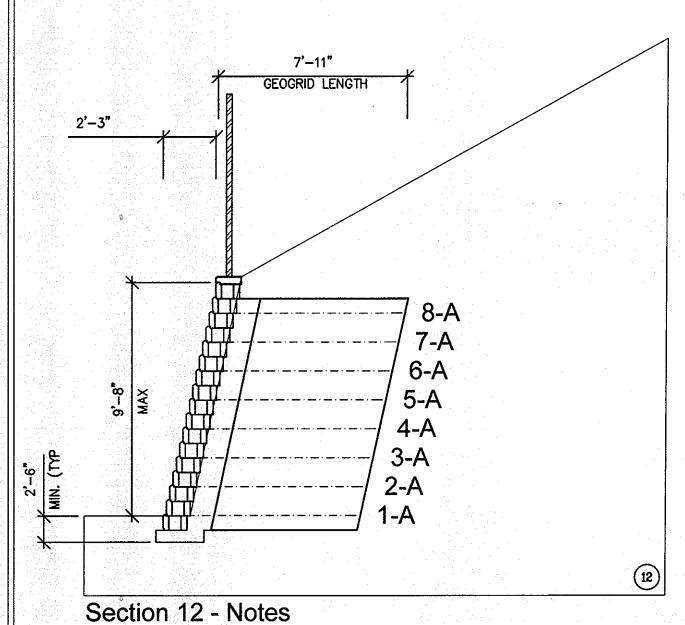
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HOWARD COUNTY, MARYLAND

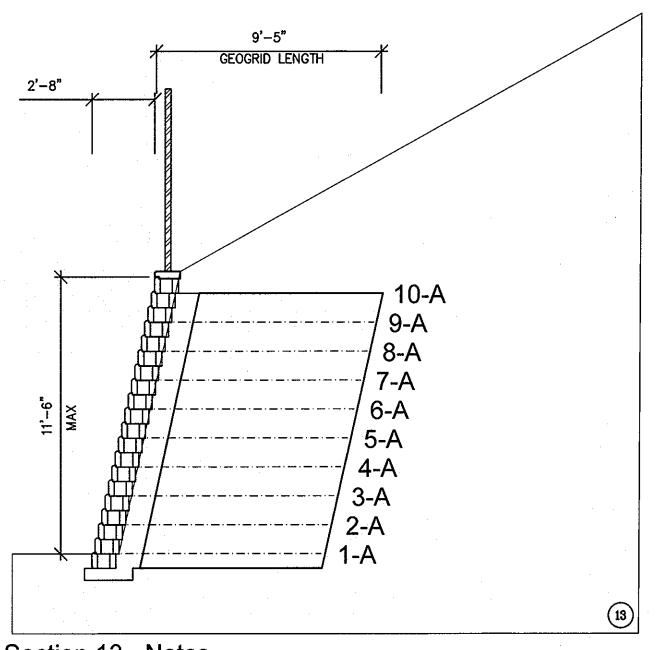
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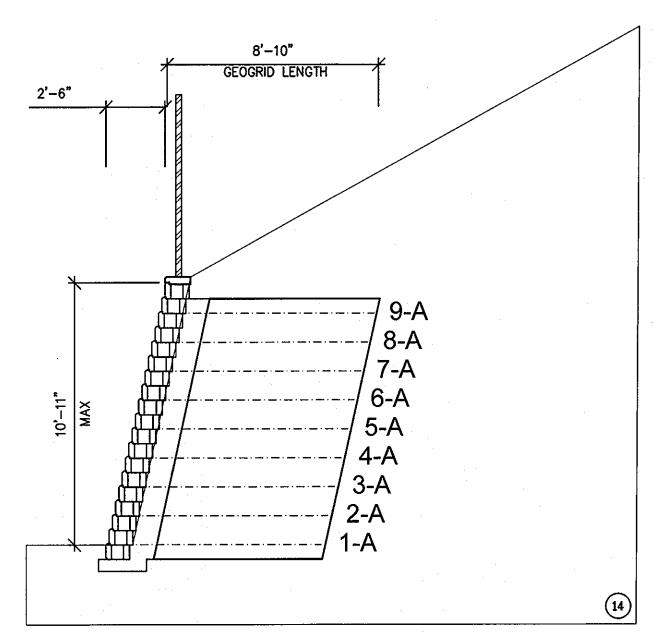
PARCEL 135



*Setback distance is approximate and does not consider curves, radii or corners.



Section 13 - Notes *Setback distance is approximate and does not consider curves, radii or corners.

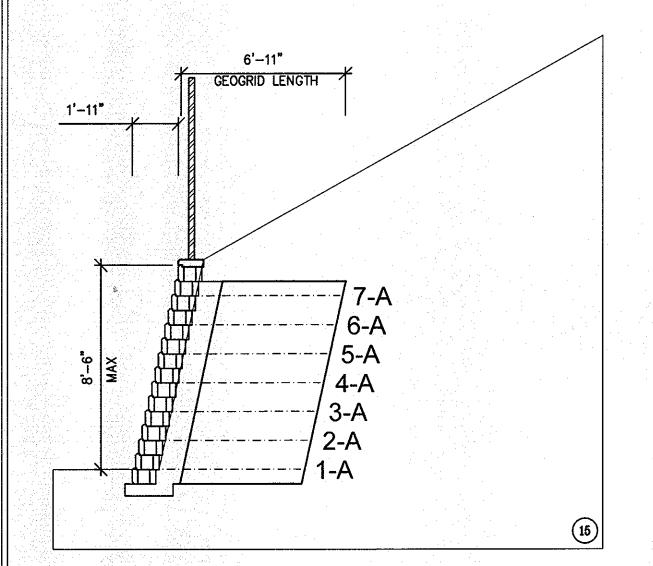


Section 14 - Notes
*Setback distance is approximate and does not consider curves, radii or corners.

- FINISH GRADE

(151.80')

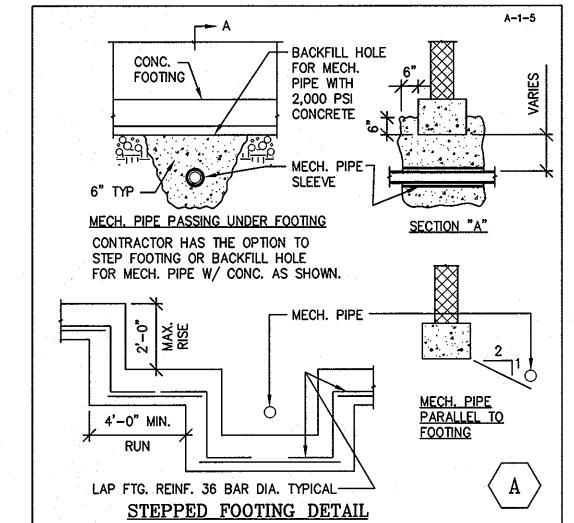
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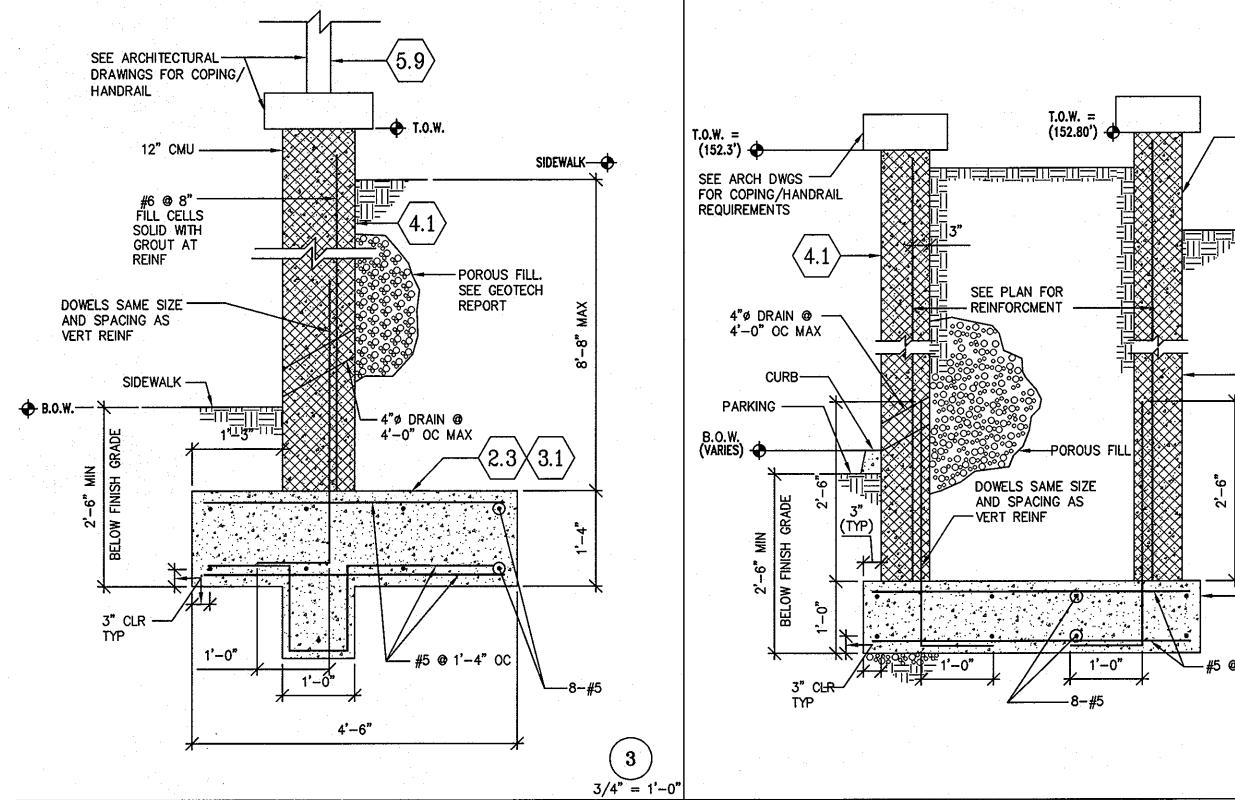


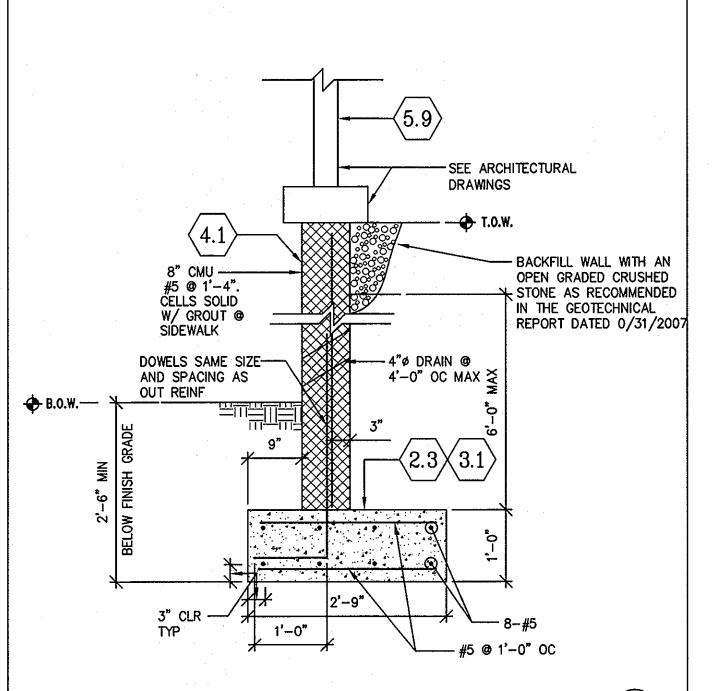
Section 15 - Notes *Setback distance is approximate and does not consider curves, radii or corners.

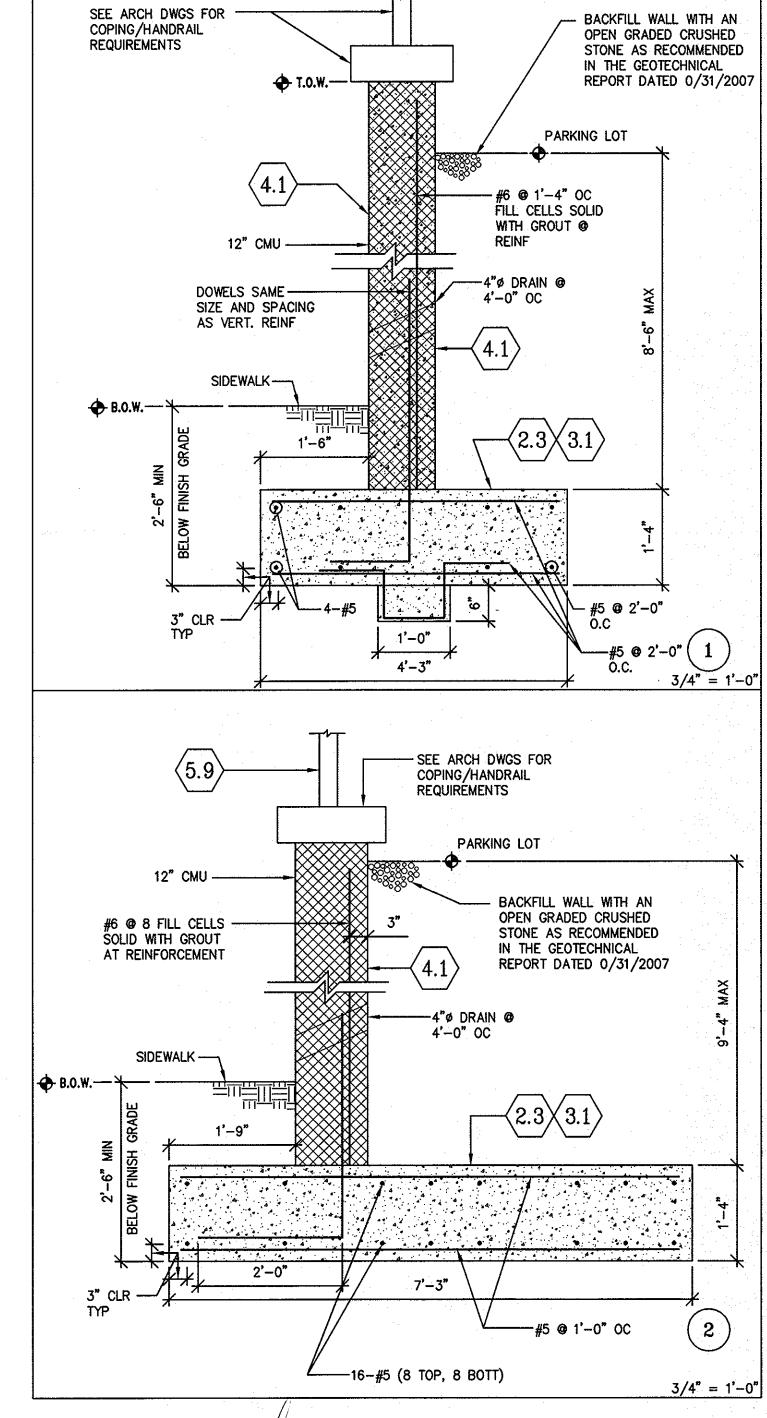
X - A REPRESENTS THE GEO GRID LEVEL

SEE DETAILS 'A/S-3' AND 'C/S-3' FOR DRAINAGE AND FOUNDATION INFORMATION.









- BACKFILL WALL WITH AN



*Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 16519

Expiration Date 06/11/09.

OWNER/DEVELOPER

TROY HILL SQUARE, LLC c/o SERGIO ACLE 10132 BALTIMORE NATIONAL PIKE, SUITE A ELLICOTT CITY, MD 21042 410.461.4400

SECTIONS TROY HILL CORPORATE CENTER PARCEL A-30

RESTAURANT AND RETAIL CENTER TAX MAP 37 GRID 18 HOWARD COUNTY, MARYLAND 1ST ELECTION DISTRICT

> Sill · Adcock & |Associates · LLC

Engineers · Surveyors · Planners 3300 North Ridge Road, Suite 160 Ellicott City, Maryland 21043 Phone: 443.325.7682 Fax: 443.325.7685 Email: info@saaland.com

CHECKED BY: CMS SCALE: AS NOTED DATE: MARCH 18, 2007 PROJECT#: 07456

PARCEL 135

SDP-08-029

3/4" = 1'-0"

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING