ROAD	CLASSIFICATION	CHART
ROAD	CLASSIFICATION	R/W WIDTH
BARNSLEY WAY	MINOR COLLECTOR	60'
	Artesta tarkir mataupita kilosoo at ilika aa daka lii keedara oo, ilika iliku siberii alkee tarkii aa kala aa aa	

	TION OFFSET	POSTED SIGN SPEED LIMIT 30 MPH	SIGN CODE
		SPEED LIMIT 30 MPH	R2-1
BARNSLEY WAY 0+			
	68 8'R	KEEP RIGHT	R4-7
BARNSLEY WAY 1+1	3 8'R	KEEP RIGHT	R4-7
BARNSLEY WAY 2+	55 23°L	ADVANCE INTERSECTION LANE CONTROL	R3-8b
BARNSLEY WAY 4+	00 23'L	SIGNAL AHEAD	W3-3(1)

		5 T	REET LIGHT CHART	
STREET NAME	STATION	OFFSET	FIXTURE/POLE TYPE	COMMENTS
***************************************	0+79	34'R		
	1+10	40'L	455 4 1 77 7004114 67 110 6 1410 6	
BARNSLEY	2+15	32'L	150-WATT "PREMIER" H.P.S. VAPOR	
WAY	3+52	23'R	FIXTURE, POST TOP FIXTURE MOUNTED ON A 14-FOOT BLACK FIBERGLASS POLE.	
	6+15	23°L	A 14-TOOT BLACK TIBERGLASS POLE.	
	7+17	24°R		4
ADAM DAVID WAY	0+52	18'R	•	

FINAL ROAD CONSTRUCTION, GRADING AND SEDIMENT CONTROL PLANS

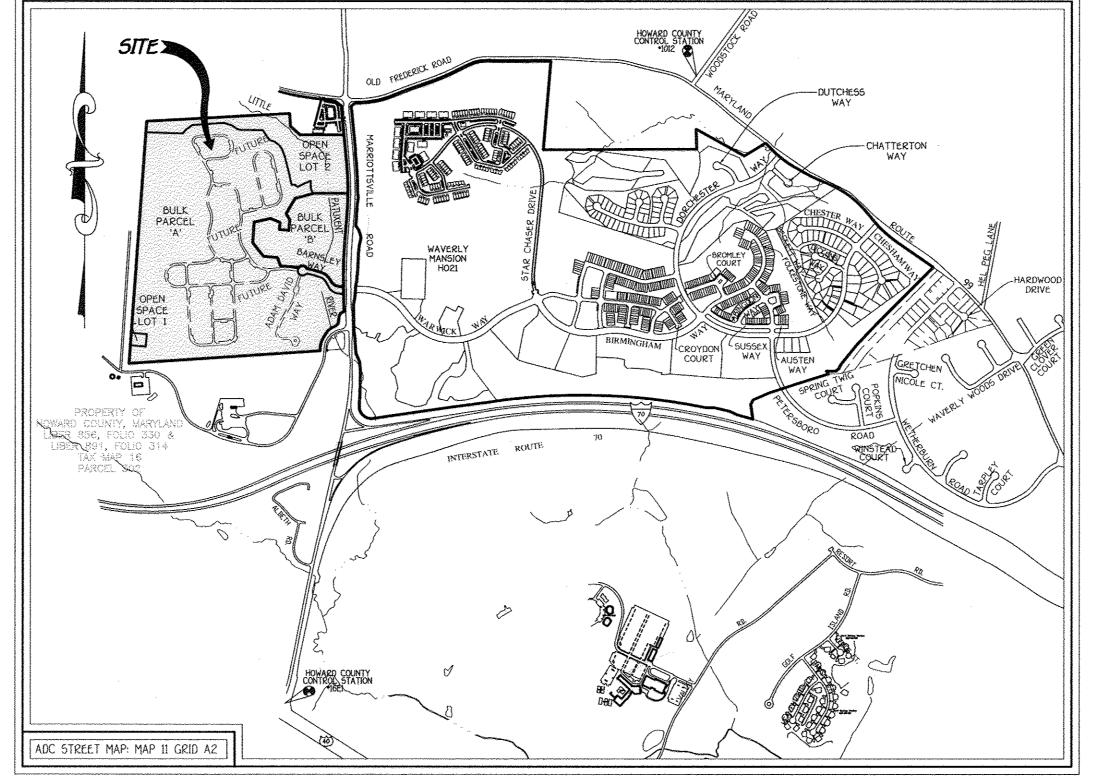
GTW'S WAVERLY WOODS

SECTION 14

BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2

Zoned: PSC & PEC

Tax Map: 16, Parcels: 120, 221 & P/O 249 Grids: 3 & 4



VICINITY MAP

THIRD ELECTION DISTRICT HOWARD COUNTY, MARYLAND

GENERAL NOTES CONTINUED

- 32. THIS PROJECT IS SUBJECT TO WAIVER PETITION WP-09-210 TO WAIVE SUBDIVISION SECTIONS: 1. 16.120(c)(2)(j), REQUEST TO NOT BE REQUIRED TO PROVIDE ANY OF THE REQUIRED MINIMUM
- "SINGLE FAMILY DETACHED" LOT FRONTAGE OF 20 FEET ON AN APPROVED PUBLIC STREET, AND: 2. 16.120(c)(4), REQUEST TO NOT BE REQUIRED TO PROVIDE ANY OF THE REQUIRED MINIMUM "SINGLE FAMILY ATTACHED" LOT FRONTAGE OF 15 FEET ON AN APPROVED PUBLIC STREET AND, BE PERMITTED TO HAVE THE "SINGLE FAMILY ATTACHED" LOTS FRONT ON A PRIVATE ROAD EXCEEDING 200 FEET IN LENGTH.
- THIS WAIVER HAS BEEN APPROVED SUBJECT TO COMPLIANCE WITH THE FOLLOWING CONDITIONS: 1. THE SUBMISSIONS OF THE PLATS TO CREATE THE INDIVIDUAL RESIDENTIAL LOTS FOR EACH PHASE OF THE PROJECT SHALL BE COORDINATED WITH THE SITE DEVELOPMENT PLANS SUBMITTED FOR EACH OF THE FIVE PHASES OF THE PROJECT.
- 2. ALL OF THE RESIDENTIAL LOTS SHALL FRONT ON AND OBTAIN ACCESS FROM THE PROPOSED PRIVATE ROADS WITHIN THE PROJECT AREA.
- 3. THE PROPOSED PRIVATE ROADS WITHIN THE PROJECT AREA SHALL BE LOCATED ON PROPERTY(IES) OWNED BY A HOMEOWNER'S ASSOCIATION (HOA) AND BE MAINTAINED BY THE SAME HOA.
- 4. THE PROPOSED PRIVATE ROADS WITHIN THE PROJECT AREA SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE DED AND DFRS.

DEVELOPER

GENERAL NOTES

- 1. ALL ASPECTS OF THE PROJECT ARE IN CONFORMANCE WITH THE LATEST HOWARD COUNTY STANDARDS UNLESS WAIVERS ARE APPROVED
- 2. THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS, DIVISION OF CONSTRUCTION INSPECTION AT 410-313-1880 AT LEAST (5) WORKING DAYS, PRIOR TO THE START OF CONSTRUCTION.

APPROVED: DEPARTMENT OF PUBLIC WORKS

2 ADD FCE"8-0.195 ACRES

DESCRIPTION

REVISIONS

REPLACE SHEETS 25 8 26, ADD SHEET 27

8-27-09

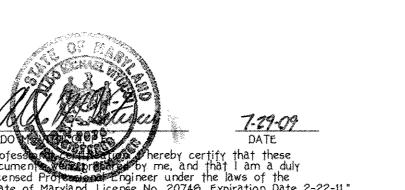
3-9-10

- 6. TOPOGRAPHY SHOWN HEREON IS FROM AERIAL MAPS FLOWN WITH 2 FOOT CONTOUR INTERVALS PREPARED BY HARFORD AERIAL SURVEYS DATED NOVEMBER 1998 AND SUPPLEMENTED WITH FIELD RUN TOPOGRAPHY BY FISHER, COLLING & CARTER, INC.

- COMPARED TO THE HYDROLOGY BASELINE STUDY PREPARED BY MILDENBERG ASSOCIATES, INC. OCTOBER, 1994. FOR CONSISTENCY WITH THE ORIGINAL, FOR CONTINUITY PURPOSES, THE SWM REPORT SUPPORTS SEVERAL PHASES OF DEVELOPMENT, FOR THIS PLAN, THE HOWARD COUNTY SOIL CONSERVATION DISTRICT HAS ONLY APPROVED POND No. 1. THE COMPUTATIONS AND DATA ASSOCIATED WITH POND Nos. 2 AND 3
- 10. THIS HORIZONTAL AND VERTICAL DATUM SHOWN ARE BASED ON THE FOLLOWING NAD '83 HOWARD COUNTY CONTROL STATIONS:
- HOWARD COUNTY MONUMENT 1012 N 601060.177 € 1345336.7580 ELEV. = 445.577. ✓ HOWARD COUNTY MONUMENT√16E1 N 593250,9322 E 1340192.7110 ELEV. = 509.924 ✓
- SUBMISSION LIMITS

- TOTAL AREA OF ROADWAY TO BE RECORDED
- TOTAL AREA OF PEC TO BE RECORDED
- PREPARED BY MARS GROUP DATED MAY, 2008 AND APPROVED UNDER P-08-010 ON 10/16/08. THE 65 DBA NOISE CONTOUR LINE DRAWN ON THIS PLAT IS ADVISORY AS REQUIRED BY THE HOWARD COUNTY DESIGN MANUAL, CHAPTER 5, REVISED FEBRUARY, 1992 AND CANNOT BE CONSIDERED TO EXACTLY LOCATE THE 65 DBA NOISE EXPOSURE, THE 65 DBA NOISE LINE WAS ESTABLISHED BY DEPARTMENT OF HOUSING AND URBAN DEVELOPMENT
- 13. TOTAL FOREST CONSERVATION OBLIGATION FOR GTW'S WAVERLY WOODS, SECTION 14: a. AFTER THE RECORDING OF PRIOR GTW'S WAVERLY WOODS RECORD PLATS INCLUDING F-08-159, PROPERTY OF GTW JOINT VENTURE. THE REMAINING FOREST OBLIGATION IS 17.46 ACRES RETENTION
- (95.7 ACRES 78.24 ACRES) AND 15.11 ACRES REFORESTATION (108.8 ACRES 93.69 ACRES) TO BE PROVIDED WITH GTW'S WAVERLY WOODS, SECTION 14.
- c. ON-SITE REFORESTATION SURETY = 4.68 x 43560 X 0.50 = \$101,930.00
- d. OFF-SITE REFORESTATION SURETY = 10.43 x 43560 X 0.50 = \$227,165.00 e. OFF-SITE REFORESTATION WILL BE PROVIDED ON THE PROPERTY OF WITCHING HOUR FARM, LLC, TAX MAP NO. 8, PARCEL NO. 249, GRID NO. 1, 4TH ELECTION DISTRICT, LIBER 10912, FOLIO 365. F. TOTAL FOREST CONSERVATION SURETY OF \$481,207.00 WILL BE PROVIDED WITH THE DEVELOPER'S AGREEMENT.
- 14. THE ORIGINAL OR BASE MODEL FLOODPLAIN STUDY FOR GTW'S WAVERLY WOODS WAS PREPARED BY MILDENBERG, BOENDER & ASSOC., DATED OCT. 1994 AND WAS APPROVED UNDER 5-94-007 IN OCTOBER, 1994. A REVISED FLOODPLAIN STUDY WAS PREPARED BY FISHER, COLLINS & CARTER, INC. DATED NOVEMBER, 2006 AND APPROVED UNDER F-07-032. A REVISED FLOODPLAIN STUDY WAS PREPARED BY FISHER, COLLINS & CARTER, INC. DATED APRIL 17, 2008 THAT MODELS THE PROPOSED CULVERTS FOR THE BARNSLEY WAY STREAM CROSSING. THIS FLOODPLAIN STUDY WAS APPROVED
- 15. THE PREVIOUS WETLANDS REPORT PREPARED BY ENVIRONMENTAL SYSTEMS ANALYSIS, INC. AND APPROVED WITH 5-94-07 ON NOVEMBER 30, 1993 HAS BEEN RE-CERTIFIED UNDER THIS PLAN BY ECO-SCIENCE PROFESSIONALS, INC. DATED APRIL, 2006 AND APPROVED WITH 5-06-013 ON JANUARY 17, 2008.
- 16. A TRAFFIC STUDY WAS PREPARED BY THE TRAFFIC GROUP AND APPROVED ON JULY 14, 1994 AS PART OF 5-94-07 AND AMENDED UNDER 5-06-13 APPROVED ON NOVEMBER 17, 2008.
- 17. SOILS INFORMATION TAKEN FROM SOIL MAP NO. 17, SOIL SURVEY, HOWARD COUNTY, MARYLAND, JULY, 1968 ISSUE. THE SOILS INVESTIGATION REPORT WAS PREPARED BY I.T.E., INC. ON JUNE 28, 1994.
- 16. FOREST STAND DELINEATION PREPARED BY ENVIRONMENTAL SYSTEM ANALYSIS, INC. AND APPROVED ON NOVEMBER 30, 1995 UNDER 5-94-07.
- 19. BOUNDARY OUTLINE IS BASED ON A FIELD MONUMENTED SURVEY PERFORMED BY FISHER, COLLINS & CARTER, INC. ON OR ABOUT AUGUST 1990.
- 20. BARNSLEY WAY IS A PUBLIC ROAD AND MAINTAINED BY HOWARD COUNTY, MARYLAND.
- 21. THE SKETCH PLAN NO. 5-94-07 WAS APPROVED ON NOVEMBER 30, 1993. THE PLANNING BOARD APPROVED, ON NOVEMBER 1, 2007, PB CASE NO. 301, GTW'S WAVERLY WOODS, SECTION 14, BULK PARCEL 'A', COMPREHENSIVE SKETCH PLAN (5-06-13) AND DEVELOPMENT CRITERIA FOR THE DEVELOPMENT OF 350 AGE-RESTRICTED ADULT HOUSING UNITS (139 SINGLE FAMILY ATTACHED UNITS AND 211
- 22. STREET LIGHT PLACEMENT AND THE TYPE OF FIXTURES AND POLES SHALL BE IN ACCORDANCE WITH THE HOWARD COUNTY DESIGN MANUAL, VOLUME III (2006), SECTION 5.5.A. A MINIMUM OF 20' SHALL BE MAINTAINED BETWEEN ANY STREET LIGHT AND ANY TREE.
- 23. PERMITS APPLICABLE FOR THIS SUBDIVISION ARE AS FOLLOWS: MDE TRACKING NUMBER 2009962/09-NT-3007
- 24. ALL HANDICAP RAMPS SHALL MEET CURRENT ADA REQUIREMENTS.
- 25. WP 95-23 WAS APPROVED ON 1/23/95 FOR DISTURBANCE TO WETLANDS, FLOODPLAIN, STREAMS, OR THEIR BUFFERS IN CERTAIN AREAS THROUGHOUT THE "WAVERLY WOODS" PROJECT. THE AREAS OF THIS SUBMISSION APPROVED UNDER THIS WAIVER FOR DISTURBANCE ARE AS FOLLOWS: a) SHEET 3 - FLOODPLAIN FOR BARNSLEY WAY ROADWAY AND STORM DRAIN CULVERT
- b) SHEET 3 WETLANDS AND BUFFER FOR BARNSLEY WAY ROADWAY AND STORM DRAIN CULVERT.
- 26. PROPERTY SUBJECT TO PRIOR DEPARTMENT OF PLANNING AND ZONING FILE Nos: 5-94-07, 5-06-013, ZB CASE NO. 1027 M, ZB CASE NO. 929-M, PB CASE NO. 301, F-01-091, F-01-093, F-01-140, F-01-147, F-08-159, P-08-010, WP-95-23 & WP-09-210.
- 27. TRAFFIC CONTROL DEVICES, MARKINGS AND SIGNING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL OF UNFORM TRAFFIC CONTROL DEVICES
- 28. ALL SIGN POSTS USED FOR TRAFFIC CONTROL SIGNS INSTALLED IN THE COUNTY RIGHT OF WAY SHALL BE MOUNTED ON A 2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE POST (14 GAUGE) INSERTED INTO 1 2-1/2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE SLEEVE (12 GAUGE) - 3' LONG. A GALVANIZED STEEL CAP SHALL BE MOUNTED ON TOP OF EACH POST.
- 29. SOIL REQUIRING 95% COMPACTION IN FILL AREAS SHALL BE IN ACCORDANCE WITH AASHTO-T-180 STANDARDS.
- 30. THE ACCESS TO OPEN SPACE LOT 1 (TO BE DEDICATED TO HOWARD COUNTY, MARYLAND) WILL BE ALONG THE EXISTING DRIVEWAY ON THE PROPERTY OF HOWARD COUNTY, MARYLAND, LIBER 856, FOLIO 330 & LIBER 891, FOLIO 314, TAX MAP No. 16, ADJACENT TM PARCELS 220 & 302.

31. LANDSCAPE OBLIGATIONS FOR BULK PARCELS 'A', 'B' AND OPEN SPACE LOTS 1 AND 2 WILL BE PROVIDED AT THE SITE DEVELOPMENT PLAN STAGE.





BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2

ZONING: PSC & PEC TAX MAP NO. 16 PARCEL Nos. 120, 221 & P/O 249 GRID Nos. 3 & 4 THIRD ELECTION DISTRICT HOWARD COUNTY, MARYLAND DATE: JULY 28, 2009 SHEET 1 OF 27

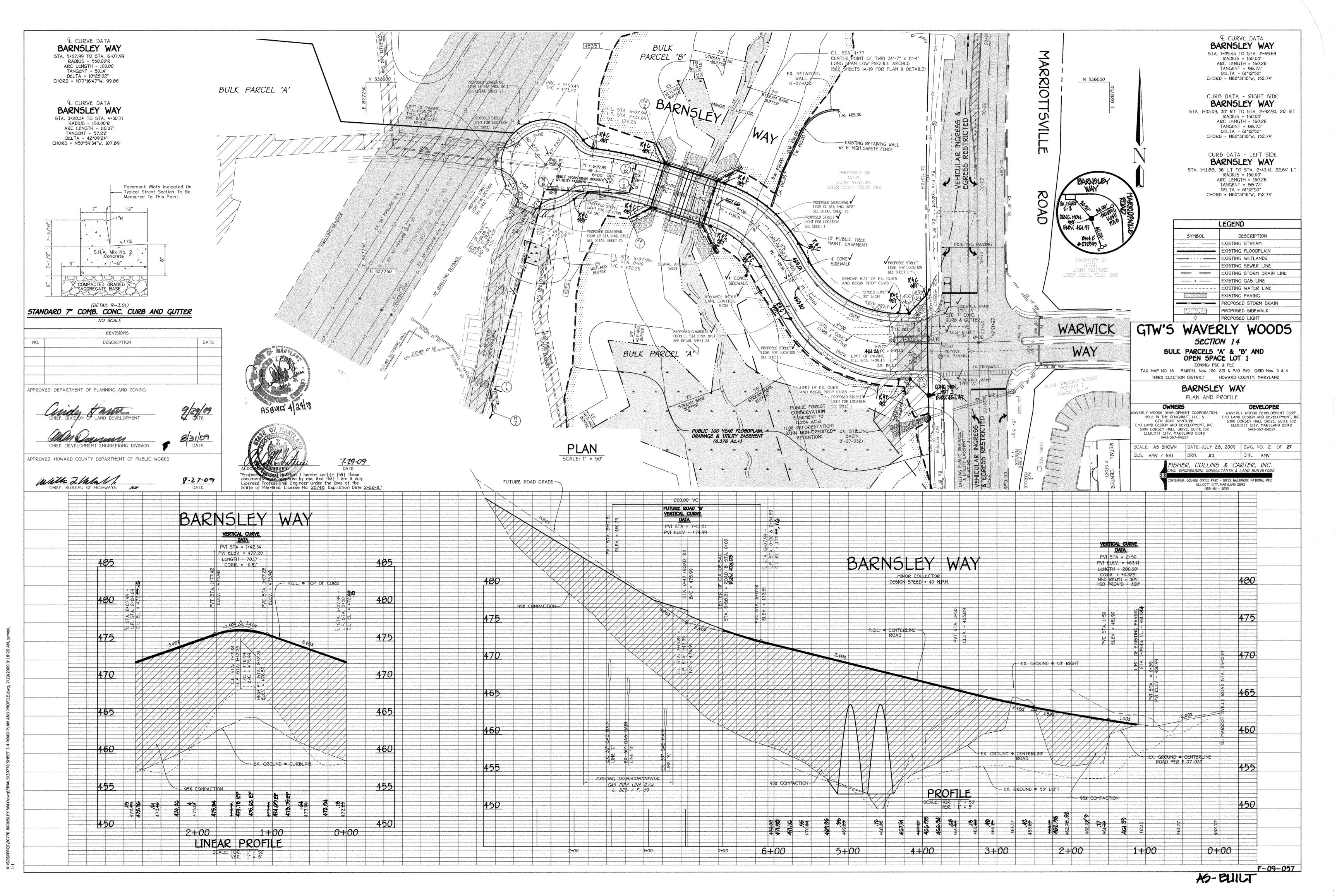
FISHER. COLLINS & CARTER. INC. FILLICOTT CITY, MARYLAND 21042

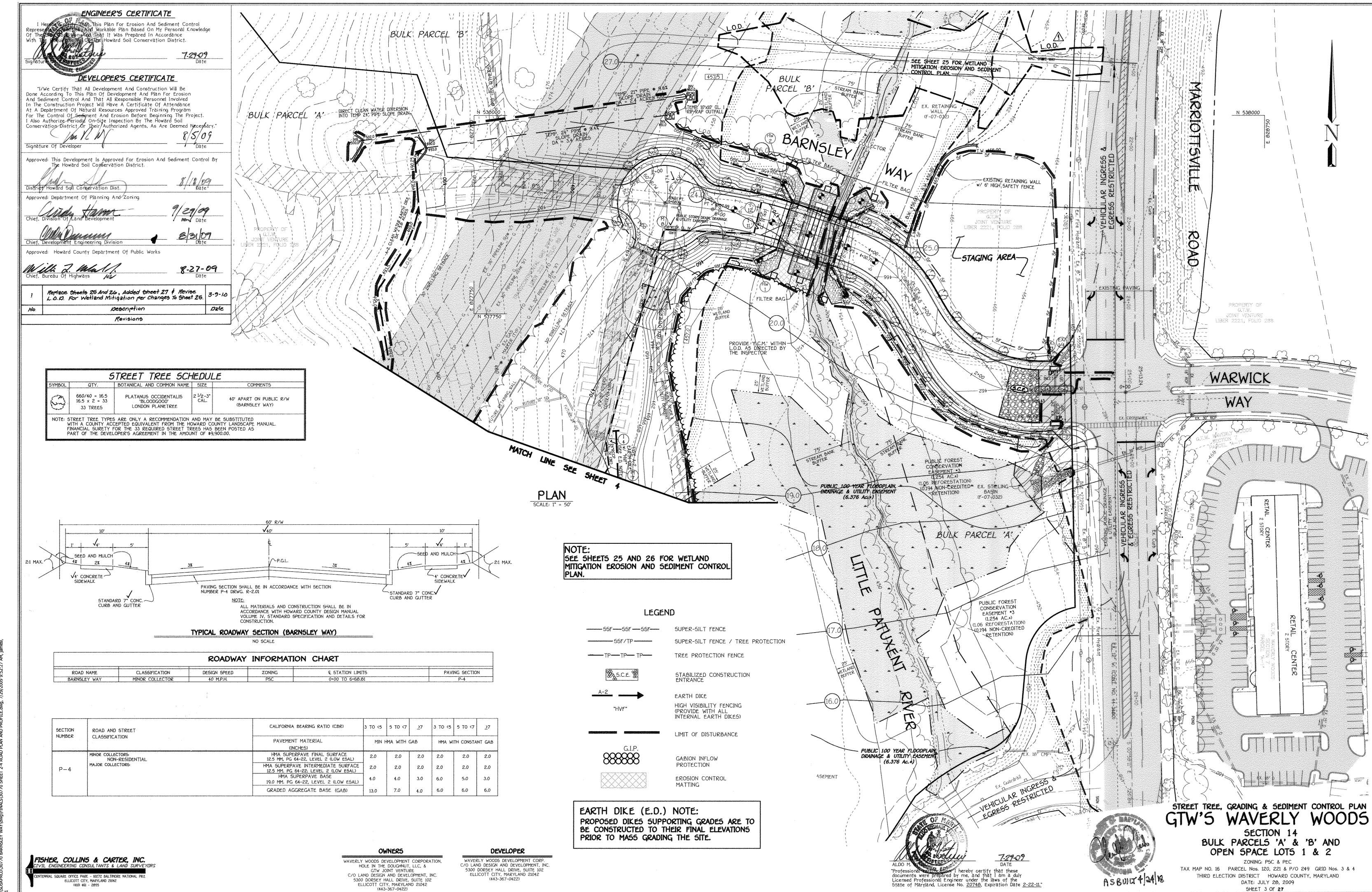
VAVERLY WOODS DEVELOPMENT CORPORATION, HOLE IN THE DOUGHNUT, LLC. & GTW JOINT VENTURE C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 ELLICOTT CITY, MARYLAND 21042

(443-367-0422)

WAVERLY WOODS DEVELOPMENT CORP. C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 ELLICOTT CITY, MARYLAND 21042

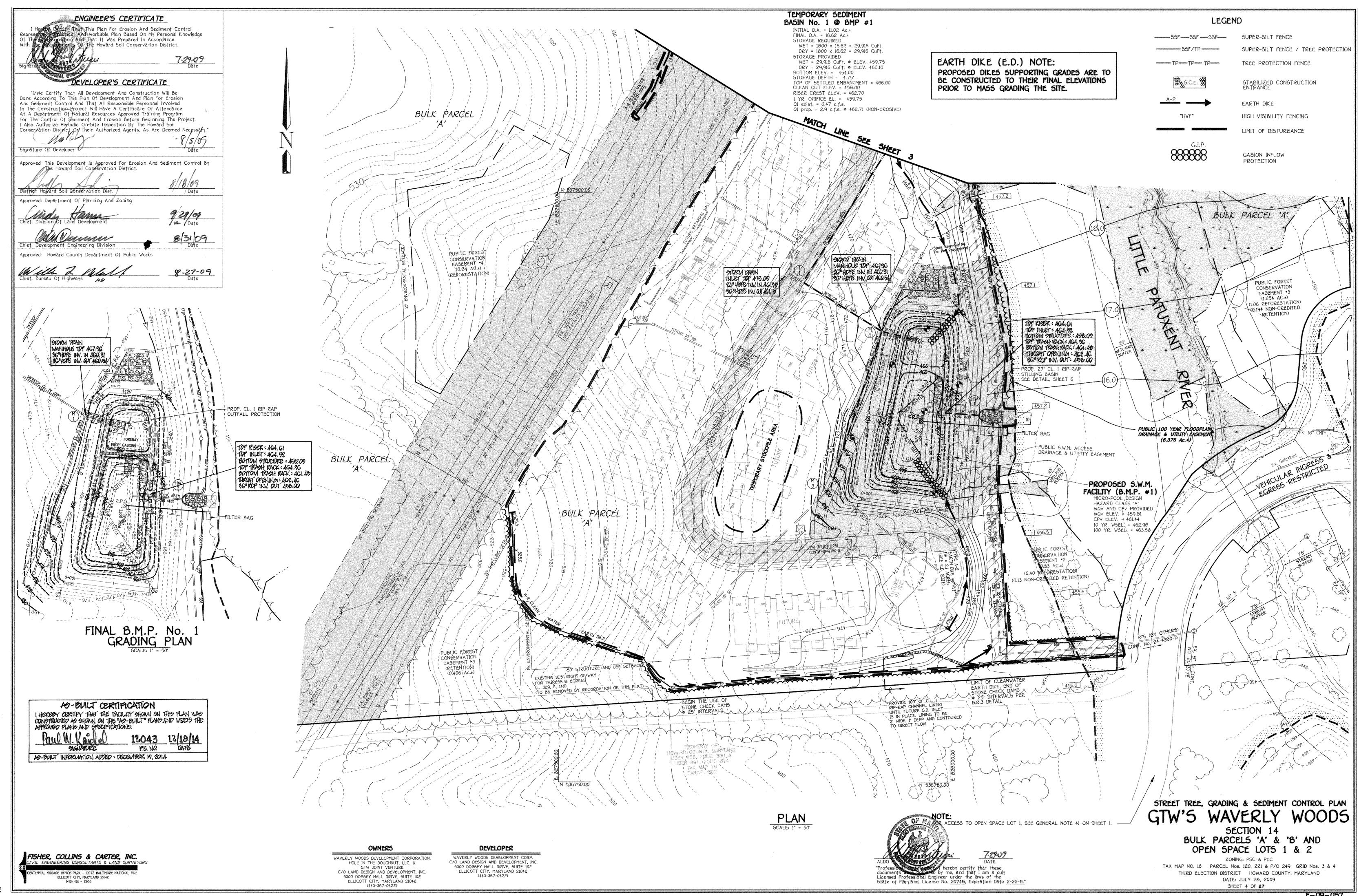
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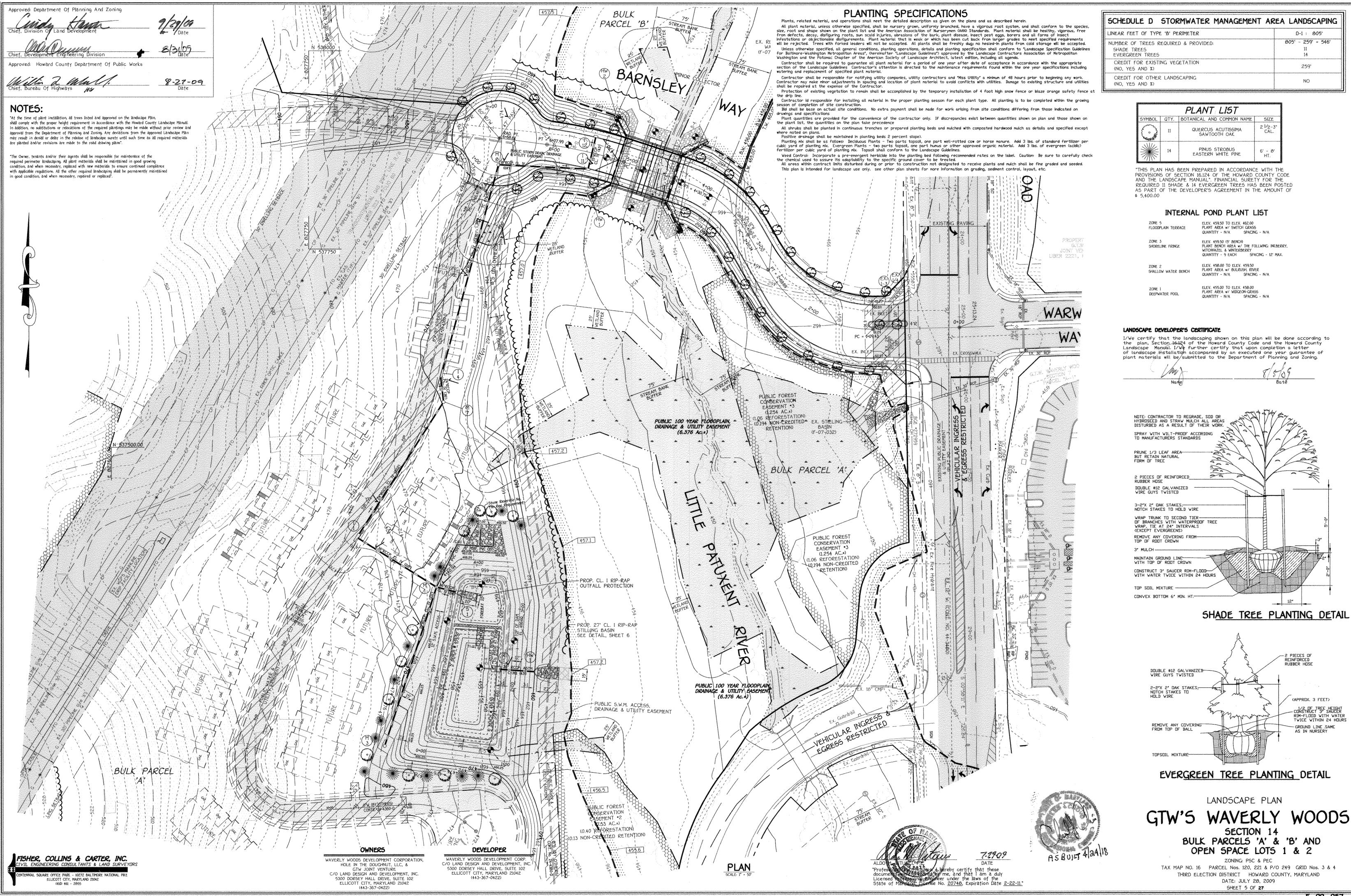


AG-BUILT F-09-057

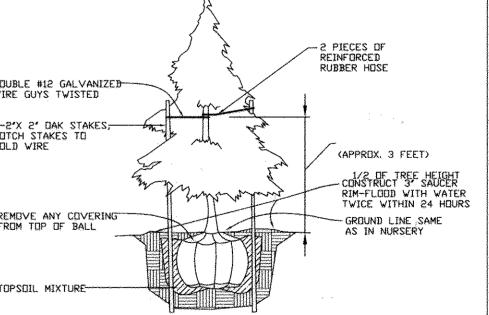
K-VSDSKDBOTTSOFT BABNSI FY WAYAMMETNA! STROTTO SHEET 2-4 BOAD BLAN AND DBOETI E AMM



F-09-057 AS-BUILT 12/19/1



SCHEDULE D STORMWATER MANAGEMENT AREA LANDSCAPING



WAVERLY WOODS

Site Preparation Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet

Areas to be covered by the reservoir will be cleared of all trees, brush, logs. fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet structure shall be cleared.

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embarkment and other designated areas.

EARTH FILL

of the toe of the embankment.

Mâterial - The fill material shall be taken from approved designated borrow areas It shall be free of roots, stumps, wood, rubbish, stones areater than 6", frozen or other objectionable materials. Fill material for the center of the embarkment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the *200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer. Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent

Placement - Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 8-inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment.

Compaction - The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber fired or vibratory roller. Fill material shall contain sufficient moisture such that the required decree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble. Yet not be so wet that water can be squeezed out.

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within +2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

Cut Off Trench - The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at leas four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed concurrently with the outer shell of the embankment.

Structure Backfill

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi; 20 day unconfined compressive strength. Th flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2,000 ohm-cm. Material shall be placed such that a minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sides of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of he material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24" or greater over the structure or pipe. Backfill material outside the structural backfill (flowable fill) zone shall be of the type and quality conforming to the specified for the core of the embankment or other embankment materials.

Pipe Conduits All pipes shall be circular in cross section.

Corrugated Metal Pipe - All of the following criteria shall apply for corrugated

1. Materials - (Polymer Coated steel pipe) - Steel pipes with polymeric coatings shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 & M-246 with watertight coupling bands or flanges.

Materials - (Aluminum Coated Steel Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Stel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.

Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-196 or M-211 with watertight coupling banks or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be between 4 and 9. 2. Coupling bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at least 24 mils in

3. Connections- All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be

EMBANKMENT AND CUT-OFF TRENCH CONSTRUCTION

The areas of the proposed SWM facilities should be stripped of topsoil and any other unsuitable materials from the embankment or structure areas in accordance with Soil Conservation Guidelines. After stripping operations have been completed, the exposed subgrade materials should be proofrolled with a loaded dump truck or similar equipment in the presence of a geotechnical engineer or his representative. For areas that are not accessible to a dump truck, the exposed materials should be observed and tested by a geotechnical engineer or his representative utilizing a Dynamic Cone Penetrometer. Any excessively soft or loose materials identified by proofrolling or penetrometer testing should be excavated to suitable firm soil, and then grades

A representative of the Geotechnical Engineer should be present to monitor placement and compaction of fill for the embankment and cut-off trench. In accordance with NRC5-MD Code No: 378 Pond Standards/Specifications, soils considered suitable for the center of embankment and cut-off trench shall conform to Unified Soil Classification GC. SG. CH. or CL and must have at least 30% passing the *200 sieve.

It is our professional opinion that in addition to the soil materials described above a fine-grained soils including Silt (ML) with a plasticity index of 10 or more can be utilized for the center of the embankment and core tench. All fill materials must be placed and 'compacted in' accordance with NRCS-MD Code No. 378 specifications.

All connections shall use a rubber or neoprene gasket when loining pipe sections The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24-inches in diameter: flanges on both ends of the pipe with a circular 3/8 inch closed cell neoprene gasket, prepunched to the flange bolt circle, sandwiched between adjacent flanges; a 12-inch wide standard lap type band with 12-inch wide by 3/8-inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2-inch greater than the corrugation depth. Pipes 24-inches in diameter and larger shall be connected by a 24-inch long annular corrugated band using a minimum of 4 (four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12-inches on the end of each pipe. Flanged Joints with 3/8-inch closed cell gaskets the full width of the PROVIDE ASPHALT

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or a neoprene bead. 4. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide

5. Backfilling shall conform to "Structure Backfill".

6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings. Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced

1. Materials - Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C-361.

2. Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in the "Stucture Backfill" section of this standard. Gravel bedding is not permitted.

3. Laying pipe - Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser. 4. Backfilling shall conform to "Structure Backfill".

5. Other details (Anti-seep collars, valves, etc.) shall be as shown on the drawings.

The following criteria shall apply for plastic pipe: 1. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM 0-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" - 10" inch pipe shall meet the requirement of AASHTO M252 Type 5, and 12" through 24" inch shall meet the requirement of AASHTO M294 Type 5.

2. Joints and connections to anti-seep collars shall be completely watertight.

3. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide

4. Backfilling shall conform to "Structure Backfill".

5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Drainage Diaphragms - When a drainage diaphragm is used, a registered professional engineer will supervise the design and construction inspection. Concrete

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3.

Rock Riprap

Rock riprap shall meet the requirements of Maryland Department of Transportation,

Geotextile shall be placed under all riprap and shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

Care of Water during Construction

All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees, cofferdams drainage channels and stream diversions necessary to protect the areas to be occupied by the permanent works. The contractor shall also furnish. install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the excavations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall

All borrow areas shall be graded to provide proper drainage and left in a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

Erosion and Sediment Control

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and

OPERATION AND MAINTENANCE

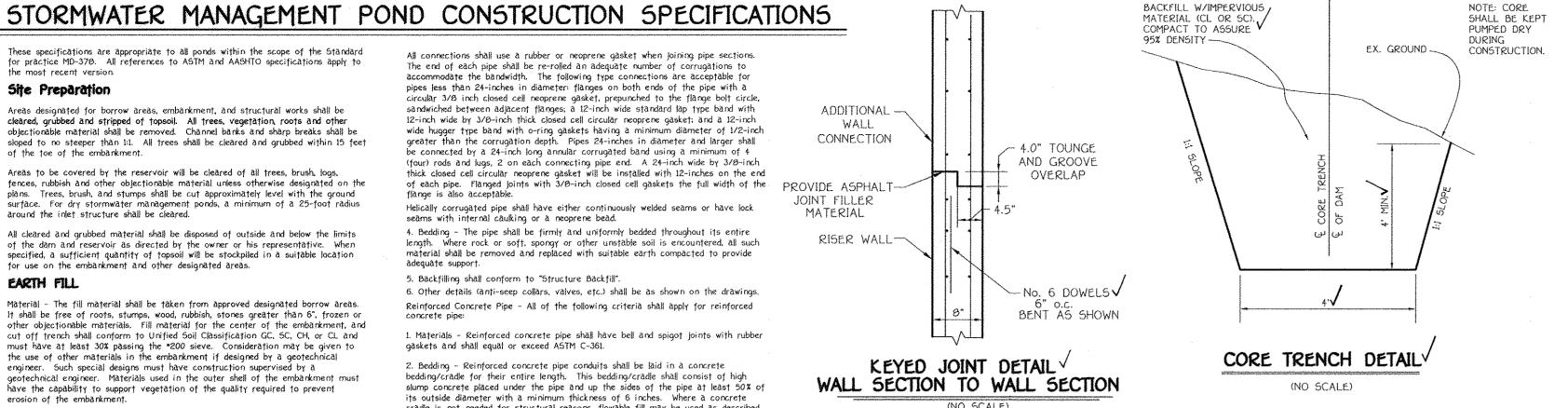
An operation and maintenance plan in accordance with Local or State Regulations will be prepared for all ponds. As a minimum, the dam inspection checklist located n Appendix A shall be included as part of the operation and maintenance plan and performed at least annually. Written records of maintenance and major repairs needs to be retained in a file. The issuance of a Maintenance and Repair Permit for any repairs or maintenance that involves the modification of the dam or spillway from its original design and specifications is required. A permit is also required for any repairs or reconstruction that involve a substantial portion of

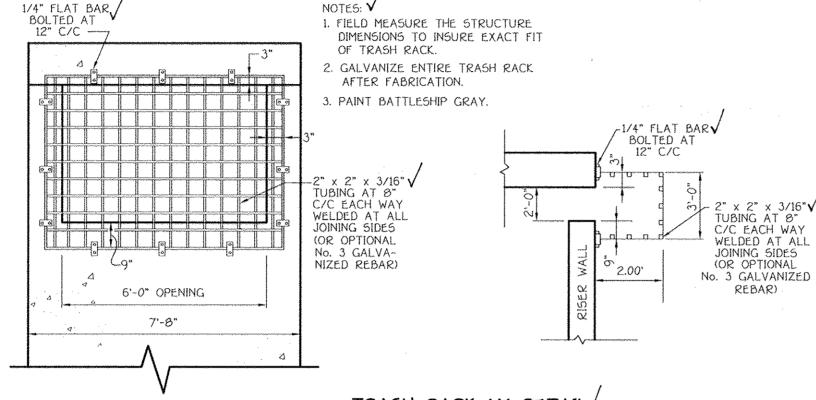
the structure. All indicated repairs are to be made as soon as practical.

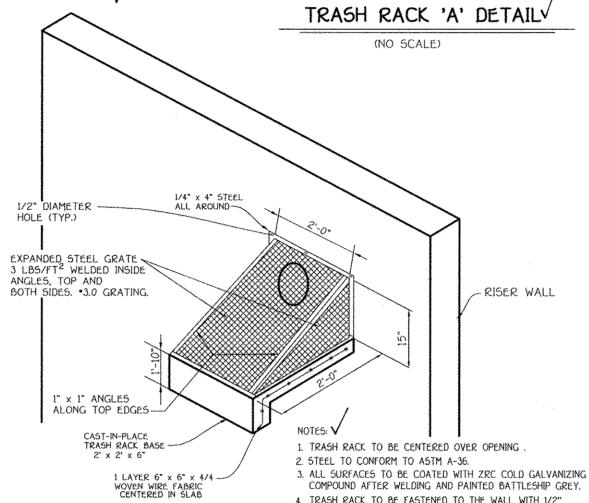
OPERATION AND MAINTENANCE SCHEDULE FOR PRIVATELY OWNED AND JOINTLY MAINTAINED STORMWATER MANAGEMENT FACILITIES

Facility shall be inspected annually and after major storms. Inspections shall be preformed

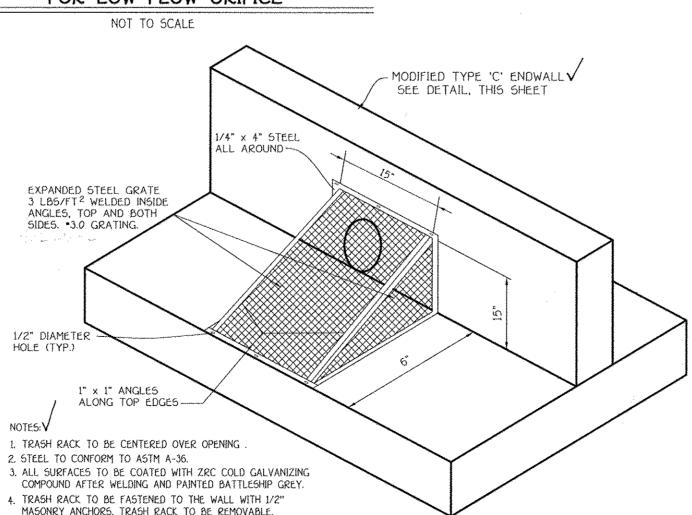
- during wet weather to determine if the pond is functioning properly.
- 2. Top and side slopes of the embankment shall be moved a minimum of two (2) times a year, once in June and once in September. Other side slopes and maintenance access should be moved as
- Debris and litter shall be removed during regular mowing
- operations and as needed.
- Visible signs of erosion in the pond as well as the rip-rap or gabion outlet area shall be repairer as soon as it is noticed.
- Structural components of the pond such as the dam, the riser, and the pipes shall be repaired upon the detection of any damage. The components shall be inspected during routine maintenance operations.
- 2. Sediment shall be removed from the pond, and forebay, no later than when the capacity of the pond or forebay, is half full of sediment, or, when deemed necessary for aesthetic reasons, upon approval from the Department of Public Works.







EXPANDED METAL TRASH RACK V FOR LOW FLOW ORIFICE



MASONRY ANCHORS, TRASH RACK TO BE REMOVABLE

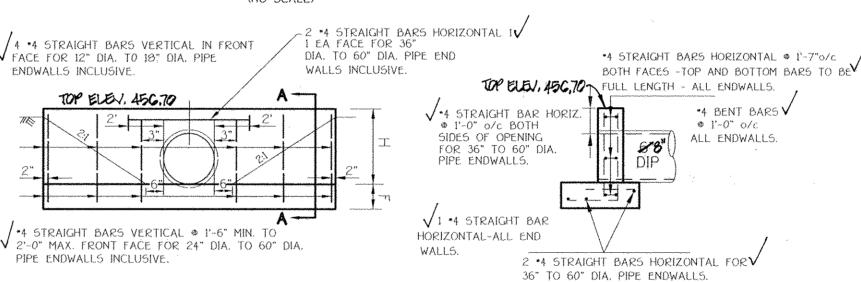
EXPANDED METAL TRASH RACK FOR DRAIN PIPE

NOT TO SCALE

WAVERLY WOODS DEVELOPMENT CORP. C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102

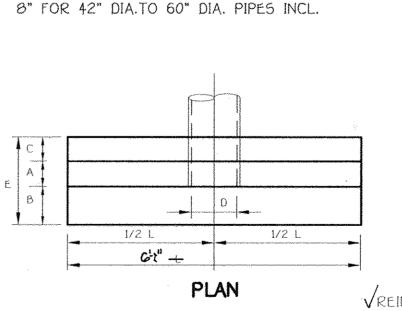
TOP OF DAM = 466.00 466 OVERFLOW CONTROLV CHANNEL-TOP OF GABION FOREBAY = 461.00 LIMIT OF FOREBAY STORAGE 46033 BOTT. OF FOREBAY 459.00 458.77 LETTER CODE L x W x D 6' x 3' x 3' 9' x 3' x 3' 12' x 3' x 3' GABION FOREBAY PROFILE V SCALE: HORIZ, 1" = 20" VERT. 1" = 2" LONGITUDINAL REINFORCEMENT Ø / ØØ

SPIGOT RING -L MASTIC JOINT SEALER RUBBER GASKET- \checkmark NOTE: PROVIDE MASTIC JOINT SEALER FROM OUTSIDE OF PIPE JOINTS PRIOR TO INSTALLING BARREL UNDERGROUND ASTM DESIGNATION C361 DIAMETERS 12 THRU 168 INCH PRESSURES TO 125 FEET OF HEAD VCONCRETE PIPE JOINT DETAIL



ELEVATION

'S' DISTANCES FROM INSIDE SURFACE OF PIPE TO VERTICAL BARS IN FRONT AND REAR FACE. 4" FOR 12" DIA. TO 18" DIA. PIPES INCL. 6" FOR 24" DIA.TO 36" DIA. PIPES INCL.



DISPOSITION OF BARS - DETAIL

SECTION A-A

 $\sqrt{\text{REINFORCING: DEFORMED STEEL BARS (1/2" DIA.)}}$ CHAMFER: ALL EXPOSED EDGES 1"x 1" OR AS DIRECTED.

CONC. SHALL BE S.H.A. A. MIX No. 2

OPENINGS VOLUME STEEL DIMENSIONS D AREA CONC 0.61 | 38 MODIFIED V

TYPE 'C' ENDWALL NO SCALE

Enameer under the laws of the No. <u>20748</u>, Expiration Date <u>2-22-11.</u>

AG-BUILT INFORMATION ADDED: NOVEMBER CC. COLA - GROUTED RIP-RAP (CL. I) 🗸 d50 = 9.5", dmax. = 15", MIN. BLANKET THICKNESS = 2.0" PLACE ATOP FILTER CLOTH HW-1 -27-24.5 PLAN r5' ● 0.00% - EL -457.25 -GROUTED RÎP-RAP (CL. 1) 🗸 d50 = 9.5", dmax. = 15", MIN. BLANKET THICKNESS = 2.0° PLACE ATOP FILTER CLOTH PROFILE STILLING BASIN OUTFALL @ POND NO.

Industry Practices.

468

By The Developer:

Signature Of Developer

Printed Name Of Developer

Approved: Department Of Public Works

Chief. Development Engineering Division

Approved: Department Of Planning And Zonin

"I/We Certify That All Development And/Or Construction Will Be Done According To These Plans, And That Any Responsible Personnel Involved In The Construction Project Will Have A

Registered Professional Engineer To Supervise Pond Construction And Provide The Howard Soil

Certificate Of Attendance At A Department Of The Environment Approved Training Program

For The Control Of Sediment And Erosion Before Beginning The Project. I Shall Engage A

Conservation District With An "As-Built" Plan Of The Pond Within 30 Days Of Completion.

"I Certify That This Plan For Pond Construction, Erosion And Sediment Control Represents A

Practical And Warkable Plan Based On My Personal Knowledge Of The Site Conditions. This Plan Was Prepared In Macrosoft with The Requirements Of The Howard Soil Conservation District. I Have Noted the Declaration That He/She Must Engage A Registered Professional Engineer To Supervision District With An

Fond Construction, Soil Erosion And Sediment Control Meet The

AS-BUILT CERTIFICATION

Hereby Certify That The Facility Shown On This Plan Was Constructed

As Shown On The " As-Built" Plans And Meets The Approved Plans And

Certify Means To State Or Declare A Professional Opinion Based Upon

Onsite Inspections And Material Tests Which Are Conducted During

Construction. The Onsite Inspections And Material Tests Are Those inspections And Tests Deemed Sufficient And Appropriate Commonly

Accepted Engineering Standards. Certify Does Not mean Or Imply A

Employment, Or Other Means, Including Meeting Commonly Accepted

Relieve Any Other Party From Meeting Requirements Imposed By Contract,

Guarantee By The Engineer Nor Does An Engineer's Certification

8-27-09

Done Within 30 Days Of Completion."

Also Authorize Periodic On Site Inspections By The Howard Soil Conservation District."

STORMWATER MANAGEMENT NOTES AND DETAILS GTW'S WAVERLY WOODS

> SECTION 14 BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2

ZONING: P5C & PEC TAX MAP NO. 16 PARCEL Nos. 120, 221 & P/O 249 GRID Nos. 3 & 4 THIRD ELECTION DISTRICT HOWARD COUNTY, MARYLAND DATE: JULY 28, 2009 SHEET 6 OF 27

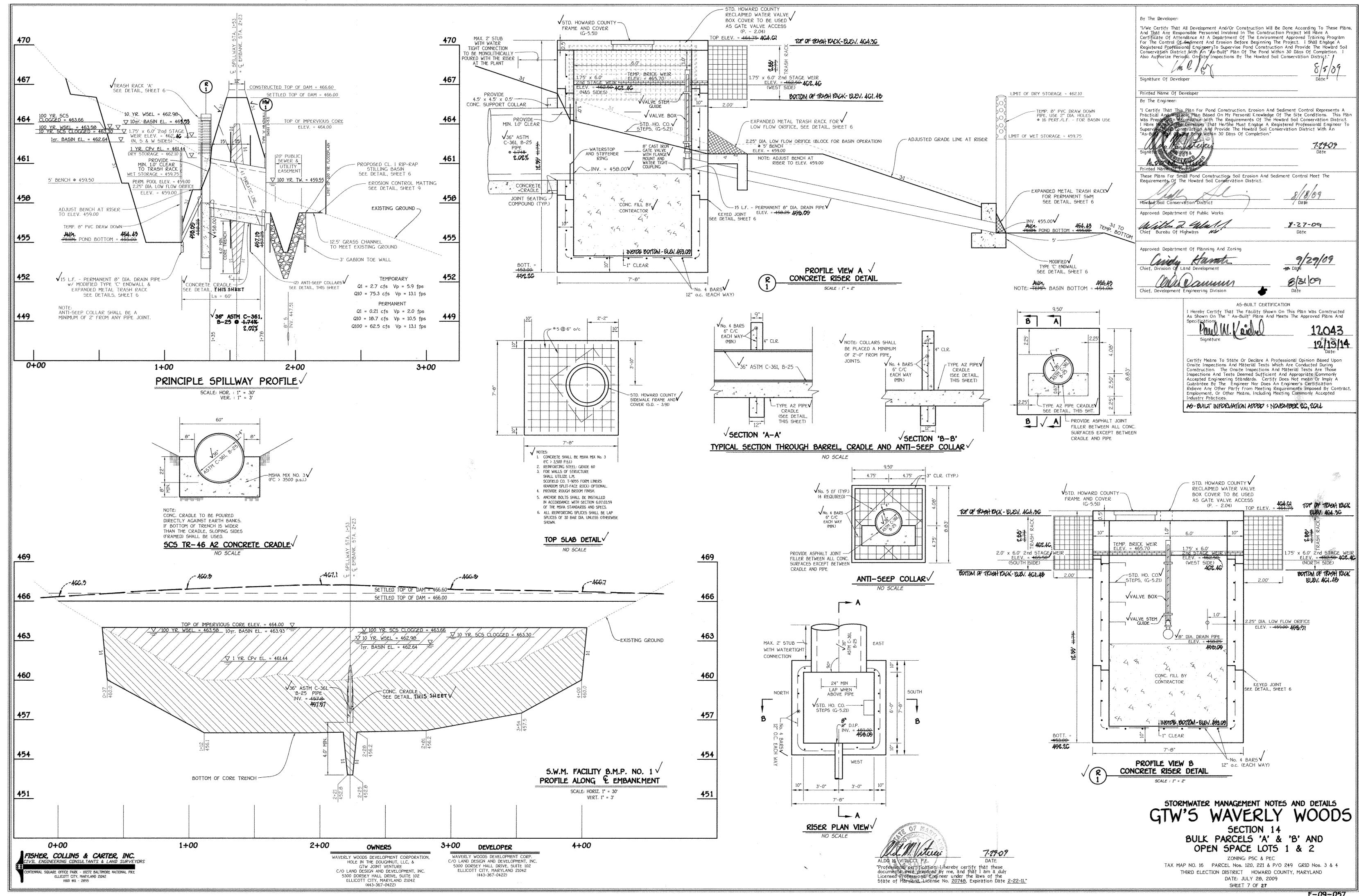
FISHER, COLLINS & CARTER, INC. ELLICOTT CITY, MARYLAND 21042 (410) 461 - 2855

re-established by backfilling with suitable soil.

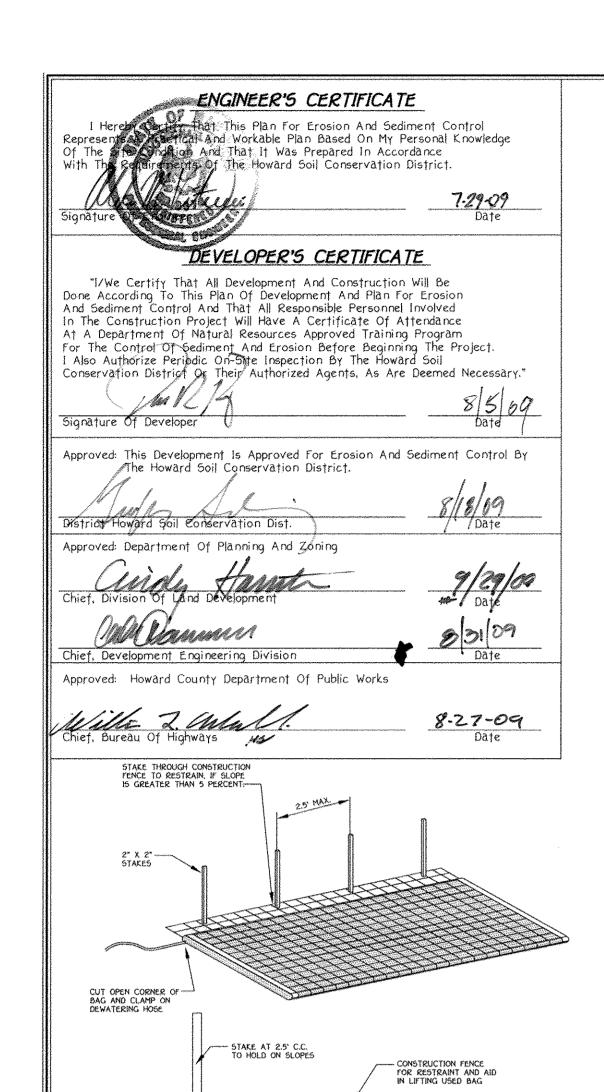
WAVERLY WOODS DEVELOPMENT CORPORATION HOLE IN THE DOUGHNUT, LLC, & GTW JOINT VENTURE C/O LAND DESIGN AND DEVELOPMENT, INC 5300 DORSEY HALL DRIVE, SUITE 102 ELLICOTT CITY, MARYLAND 21042 (443-367-0422)

OWNERS

DEVELOPER ELLICOTT CITY, MARYLAND 21042 (443-367-0422)



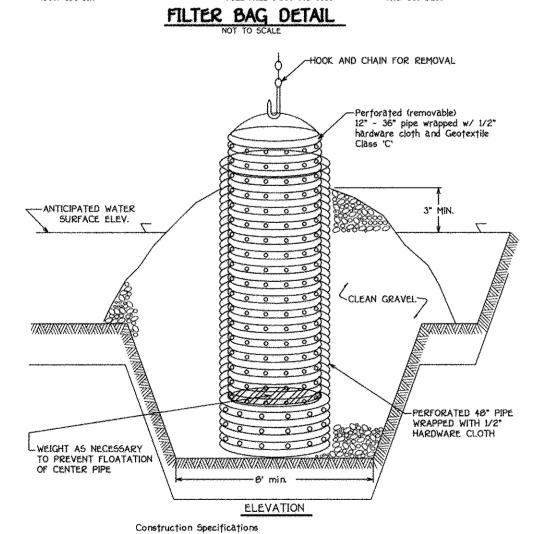
F-09-057 AG-BUILT: 11/26/14



- SUCH THAT WATER WILL FLOW AWAY FROM DEVICE AND ANY WORK AREAS
- 2. WITCH AND LENTH SHALL BE AS SHOWN IN THE TABLE.
- 3. THE FILTER BAG MUST BE STAKED IN PLACE AND SECURED TO THE PUMP DISHARGE LINE, FILTER BAG SHALL NOT BE USED FOR DISCHARGE FLOWS GREATER THAN 300 GPM.
- DEVICE SHALL BE REMOVED AND DISPOSED OF AFTER BAG IS FILLED WITH SEDIMENT. SEDIMENT FROM BAG SHALL BE SPREAD IN AN UPLAND AREA.

WATER AND— SEDIMENT

INDIAN VALLEY INDUSTRIES, INC. P.O. BOX 810 JOHNSON CITY, NEW YORK 13790 (800) 659-5111



- 1. The outer pipe should be 48" dia. or shall, in any case, be at least 4" greater in diameter than the center pipe. The outer pipe shall be wrapped with 1/2" hardware cloth to prevent backfill material from entering the perforations.
- 2. After installing the outer pipe, backfill around outer pipe with 2" aggregate or clean gravel.
- 3. The inside stand pipe (center pipe) should be constructed by perforating a corrugated or PVC pipe between 12" and 36" in diameter. The perforations shall be 1/2" X 6" slits or 1" diameter holes 6" on center The center pipe shall be wrapped with 1/2" hardware cloth first, then
- 4. The center pipe should extend 12" to 18" above the anticipated water

REMOVABLE PUMPING STATION

FISHER, COLLINS & CARTER, INC. ELLICOTT CITY, MARYLAND 21042 (410) 461 - 2855

EARTH DIKE NOT TO SCALE b 2:1 SLOPE OR FLATTER 2:1 SLOPE OR FLATTER - EXCAVATE TO PROVIDE REQUIRED FLOW WIDTH GRADE LINE AT DESIGN FLOW DEPTH DIKE B a-DIKE HEIGHT POSITIVE DRAINAGE SUFFICIENT TO DRAIN b-DIKE WIDTH d-FLOW DEPTH CUT OR FILL SLOPE PLAN VIEW

STANDARD SYMBOL

A-2 B-3

FLOW CHANNEL STABILIZATION GRADE 0.5% MIN. 10% MAX. 1. Seed and cover with straw mulch.

Construction Specifications

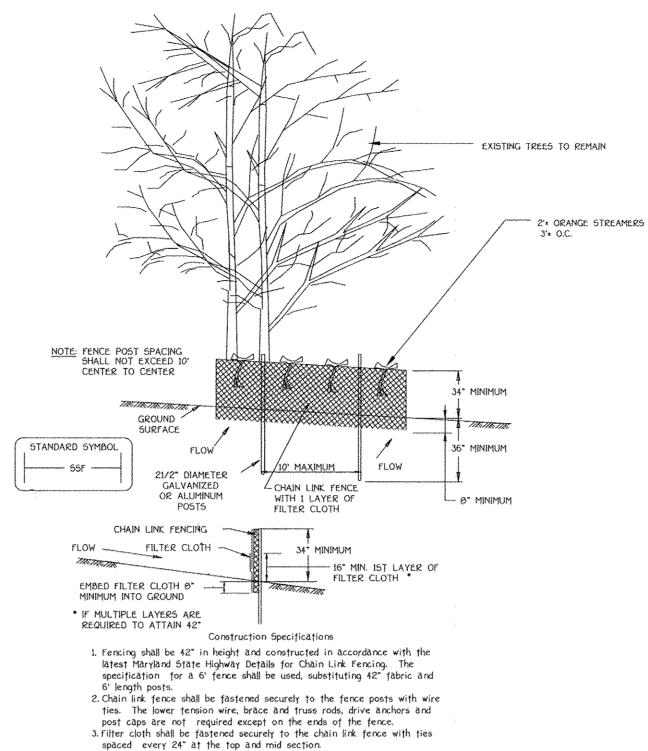
- 1. All temporary earth dikes shall have uninterrupted positive grade to an outlet. Spot elevations may be necessary for grades less than 1%.
- 2. Runoff diverted from a disturbed area shall be conveyed to a sediment trapping device.

2. Seed and cover with Erosion Control Matting or line with sod.

3. 4" - 7" stone or recycled concrete equivalent pressed into

the soil 7" minimum

- 3. Runoff diverted from an undisturbed area shall outlet directly into an undisturbed, stabilized area at a non-erosive velocity.
- All trees, brush, stumps, obstructions, and other objectionable material shall be removed and disposed of so as not to interfere with the proper functioning of the dike.
- 5. The dike shall be excavated or shaped to line, grade and cross section as required to meet the criteria specified herein and be free of bank projections or other irregularities which will impede
- 6. Fill shall be compacted by earth moving equipment.
- 7. All earth removed and not needed for construction shall be placed so that it will not interfere with the functioning of the dike.
- 8. Inspection and maintenance must be provided periodically and after each rain event.



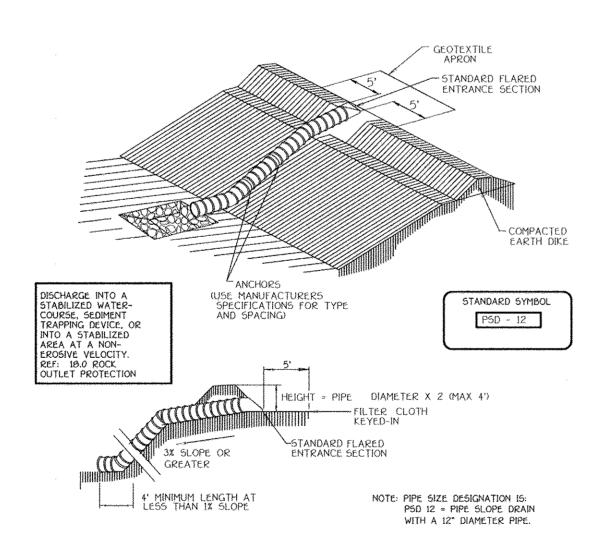
4. Filter cloth shall be embedded a minimum of 8" into the ground.

5. When two sections of filter cloth adjoin each other, they shall be overlapped by 6" and folded. 6. Maintenance shall be performed as needed and silt buildups removed when "bulges" develop in the silt fence, or when silt reaches 50% of fence

7. Filter cloth shall be fastened securely to each fence post with wire ties or staples at top and mid section and shall meet the following requirements for Geotextile Class F

	Tensile Modulus Flow Rate Filtering Efficiency		20 lbs/in (min.) 0.3 gal/ft /minute ² (max 75% (min.)	Test: MSMT 509 J Test: MSMT 322 Test: MSMT 322
			Design Criteria	1651. 15011
	Slope	Slope Steepness	Slope Length (maximum)	Silt Fence Length (maximum)
	0 - 10%	0 - 10:	Unlimited	Unlimited
	10 - 20%	10:1 - 5:	i 200 feet	1,500 feet
	20 - 33%	5:1 - 3:	1 100 feet	1,000 feet
	33 - 50%	3:1 - 2:	1 100 feet	500 feet
	50% +	2:1 +	50 feet	250 feet

SUPER SILT FENCE. TREE PROTECTION FENCE



Size	Pipe/Tubing Diameter (D) in	Maximum Drainage Area (Acres)
P5D-12	12	0.5
PSD-18	18	1.5
P5D-21	21	2.5
P5D-24	24	3.5
PSD-24 (2)	24	5.0
	PIPE SLOPE	DRAIN
	NOT TO SCALE	·

DEVELOPER

WAVERLY WOODS DEVELOPMENT CORP

C/O LAND DESIGN AND DEVELOPMENT, INC.

5300 DORSEY HALL DRIVE, SUITE 102

ELLICOTT CITY, MARYLAND 21042

(443-367-0422)

OWNERS

WAVERLY WOODS DEVELOPMENT CORPORATION,

HOLE IN THE DOUGHNUT, LLC, &

GTW JOINT VENTURE

C/O LAND DESIGN AND DEVELOPMENT, INC.

5300 DORSEY HALL DRIVE, SUITE 102

ELLICOTT CITY, MARYLAND 21042

(443-367-0422)

Construction Specifications - Pipe Slope Drain

1. The Pipe Slope Drain (PSD) shall have a slope of 3 percent or steeper.

2. The top of the earth dike over the inlet pipe shall be at least 2 times the pipe diameter measured at the invert of the

3. Flexible tubing is preferred. However, corrugated metal pipe or equivalent PVC pipe can be used. All connections shall be watertight.

4. A flared end section shall be attached to the inlet end of pipe with a watertight connection. Filter cloth shall be placed under the inlet of the pipe slope drain and shall extend out 5' from the inlet. The filter cloth shall be "keyed in" on all sides.

5. The Pipe Slope Drain shall be securely anchored to the slope by staking at the grommets provided. Spacing for anchors shall be as provided by manufacturer's specification In no case shall less than two (2) anchors be provided, equally spaced along the length of pipe. These details should be provided by pipe suppliers.

6. The soil around and under the pipe and end section shall be hand tamped in 4 inch lifts to the top of the earth dike.

8. Whenever possible where a PSD drains an unstabilized area,

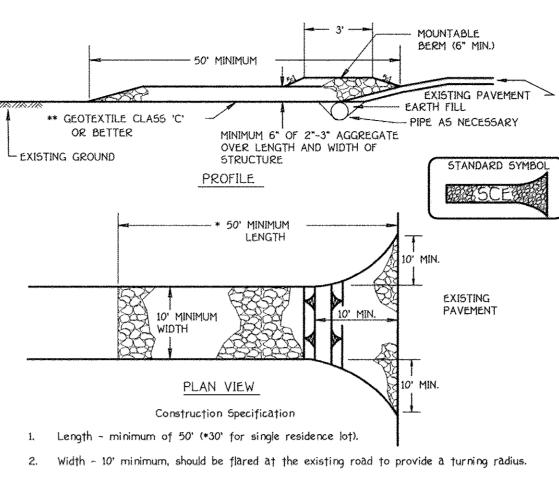
7. All pipe connections shall be watertight.

it shall outlet into a sediment trap or basin. If this is not possible then the slope drain will discharge into a stable conveyence that leads to a sediment trap or basin. When discharging into a trap or basin the PSD shall discharge at the same elevation as the wet pool elevation. The discharge from the PSD must be as far away from the sediment control outlet as possible.

9. When the drainage area is stabilized, the PSD shall discharge onto a stabilized area at a non-erosive velocity. 10. Inspection and any required maintenance shall be performed

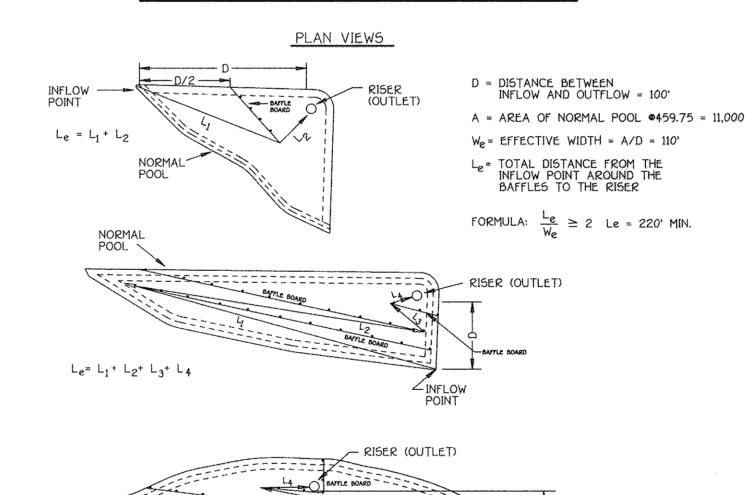
periodically and after each rain event. 11. The inlet must be kept open at all times.

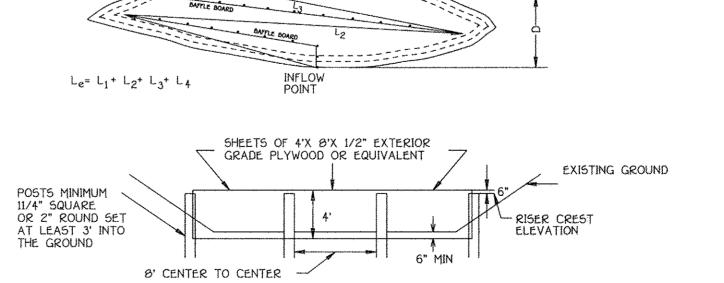
STABILIZED CONSTRUCTION ENTRANCE NOT TO SCALE



- Geotextile fabric (filter cloth) shall be placed over the existing ground prior to placing 3. stone. **The plan approval authority may not require single family residences to use
- Stone crushed aggregate (2" to 3") or reclaimed or recycled concrete equivalent shall 4. be placed at least 6" deep over the length and width of the entrance.
- Surface Water all surface water flowing to or diverted toward construction entrances shall be piped through the entrance, maintaining positive drainage. Pipe installed through the stabilized construction entrance shall be protected with a mountable berm with 5:1 slopes and a minimum of 6" of stone over the pipe. Pipe has to be sized according to the drainage. When the SCE is located at a high spot and has no drainage to convey a pipe will not be necessary. Pipe should be sized according to the amount of runoff to be conveyed. A 6" minimum will be required.
- Location A stabilized construction entrance shall be located at every point where construction traffic enters or leaves a construction site. Vehicles leaving the site must travel over the entire length of the stabilized construction entrance.

SEDIMENT BASIN BAFFLES





BAFFLE DETAIL

iffication hereby certify that these propaged by me, and that I am a duly Engineer under the laws of the State of Maryland, License No. 20748, Expiration Date 2-22-11.

SUPER SILT FENCE DETAIL

NOT TO SCALE 42" CHAIN LINK FENCE WITH I LAYER OF MIRAFI 2-1/2" DIAMETER -MCF 1212 OR EQUIVALENT — GALVANIZED OVER UPHILL SIDE OF FENCE OR ALUMINUM 10' MAXIMUM SURFACE TRIBINATION 118/18/18

PERSPECTIVE VIEW 2 1/2" DIA. GALVANIZED OR STABILIZE AREA (MIN. 36" WIDE) OR EQUIVALENT TATIATIA EMBED MIRAFI---MIN. 9" INTO GRD. LAYER MIRAFI IN TANDARD SYMBO BOTTOM OF 14" MIN. WIDE TRENCH ----55F----SECTION VIEW

CONSTRUCTION SPECIFICATIONS

1. FENCING SHALL BE 42" HIGH CHAIN CONSTRUCTED IN ACCORDANCE WITH THE LATEST MARYLAND STATE HIGHWAY ADMINISTRATION STANDARD DETAILS 690.01 AND 690.02 FOR CHAIN U FENCING. THE SPECIFICATIONS FOR A 6'-0" FENCE SHALL BE USED. SUBSTITUTING 42" FABRIC AND 8' POSTS. POSTS SHALL BE PLACED WITHOUT CONCRETE EMBEDMENT.

2. CHAIN LINK FENCE SHALL BE FASTENED SECURELY TO FENCE POSTS WITH WIRE TIES OR STAPLES. THE LOWER TENSION WIRE, BRACE AND TRUSS RODS, ANCHORS AND POST CAPS ARE NOT REQUIRED EXCEPT ON THE ENDS OF THE FENCE.

3. FILTER CLOTH TO BE FASTENED SECURELY TO CHAIN LINK FENCE WITH TIES SPACED EVERY 24" AT TOP AND MID SECTION. 4. FILTER CLOTH SHALL BE IMBEDDED A MINIMUM OF 9" INTO THE

5. WHEN TWO SECTIONS OF DIVERSION CLOTH ADJOIN EACH OTHER THEY SHALL BE OVERLAPPED BY SIX INCHES AND FOLDED. 6. MAINTENANCE SHALL BE PERFORMED AS NEEDED.

Fabric Properties	Value	Test Method
Grab Tensile Strength (lbs.)	90	ASTM D1682
Elongation at Failure (%)	50	ASTM DI68:
Mullen Burst Strength (PSI)	190	A5TM D378
Puncture Strength (lbs.)	40	ASTM D751
Slurry Flow Rate (gal/min/sf)	0.3	Virginia DOT VTM-5
Equivalent Opening Size	40-80	US 5†d Siev CW-02215
Utraviolet Radiation Stability	(z) 90	ASTM G-26
Design	Criteria	
Slope	Slope Length	Silt Fence Le

Slope	Slope Steepness	Slope Length (maximum)	Silt Fence Length (maximum)
0 - 10%	0 - 10:1	Unlimited	Unlimited
10 - 20%	10:1 - 5:1	400 feet	1,500 feet
20 - 33%	5:1 - 3:1	300 feet	1,000 feet
33 - 50%	3:1 - 2:1	200 feet	500 feet
50% +	2:1 +	100 feet	250 feet

SEDIMENT CONTROL NOTES

- 1) A MINIMUM OF 48 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT OF INSPECTIONS, LISCENSES AND PERMITS, SEDIMENT CONTROL
- DIVISION PRIOR TO THE START OF ANY CONSTRUCTION (313-1855). 2) ALL VEGETATIVE AND STRUCTURAL PRACTICES ARE TO BE INSTALLED ACCORDING TO THE PROVISIONS OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE MOST CURRENT MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL
- EROSION AND SEDIMENT CONTROL AND REVISIONS THERETO. 3) FOLLOWING INITIAL SOIL DISTURBANCE OR RE-DISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: a) 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, PERIMETER SLOPES AND ALL SLOPES STEEPER THAN 3:1, b) 14 DAYS AS TO ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE.
- 4) ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 12, OF
- THE HOWARD COUNTY DESIGN MANUAL, STORM DRAINAGE. 5) ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDING (SEC. 51), SOD (SEC. 54), TEMPORARY SEEDING (SEC. 50), AND MULCHING (SEC. 52). TEMPORARY STABILIZATION WITH MULCH ALONE CAN ONLY BE DONE WHEN RECOMMENDED SEEDING DATES DO NOT ALLOW FOR PROPER
- GERMINATION AND ESTABLISHMENT OF GRASSES. 6) ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE MAINTAINED IN OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.
- 7) SITE ANALYSIS TOTAL AREA OF SITE AREA DISTURBED ACRES AREA TO BE ROOFED OR PAVED ACRES AREA TO BE VEGETATIVELY STABILIZED 10.93 ACRES CU.YD5
- OFFSITE WASTE/BORROW AREA LOCATION ON-SITE (SDP-09-039) 8) ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF UTILITIES MUST BE REPAIRED ON THE SAME DAY OF
- DISTURBANCE. 9) ADDITIONAL SEDIMENT CONTROLS MUST BE PROVIDED, IF DEEMED NECESSARY BY THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.
- 10) ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING. OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL BY THE INSPECTION AGENCY IS MADE.
- 11) TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGHTS OR THAT WHICH SHALL BE BACK-FILLED AND STABILIZED WITHIN ONE WORKING DAY, WHICHEVER IS SHORTER.

SEDIMENT CONTROL NOTES AND DETAILS

GTW'S WAVERLY WOODS SECTION 14

BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2

ZONING: PSC & PEC TAX MAP NO. 16 PARCEL Nos. 120, 221 & P/O 249 GRID Nos. 3 & 4 THIRD ELECTION DISTRICT HOWARD COUNTY, MARYLAND DATE: JULY 28, 2009

DEVELOPER'S CERTIFICATE

"I/We Certify That All Development And Construction Will Be Done According To This Plan Of Development And Plan For Erosion And Sediment Control And That All Responsible Personnel Involved In The Construction Project Will Have A Certificate Of Attendance At A Department Of Natural Resources Approved Training Program For The Control Of Sediment And Erosion Before Beginning The Project. 1 Also Authorize Periodic On-Site Inspection By The Howard Soil Conservation District Or Their Authorized Agents, As Are Deemed Necessary.

Signature Of Developer Approved: This Development Is Approved For Erosion And Sediment Control By the Howard Soil Conservation District.

District Howard Soil Conservation Dist. Approved: Department Of Planning And Zoning

Development Engineering Divisior

Approved: Howard County Department Of Public Works

With I Mall.

20.0 STANDARDS AND SPECIFICATIONS VEGETATIVE STABILIZATION DEFINITION

Using vegetation as cover for barren soil to protect it from forces that cause erosion.

PURPOSE. Vegetätive stabilization specifications are used to promote the establishment of vegetation on exposed soil. When soil is stabilized with vegetation, the soil is less likely to erode and more likely to allow infiltration of rainfall, thereby reducing sediment loads and run-off to downstream areas, and improving wildlife habitat and visual resources.

CONDITIONS WHERE PRACTICE APPLIES This practice shall be used on denuded areas as specified on the plans and may be used on highly erodible or critically eroding areas. This specification is divided into Temporary Seeding, to quickly establish vegetative cover for short duration O(up to one year), and Permanent Seeding, for long term vegetative cover. Examples of applicable areas for Temporary Seeding are temporary Soil Stockpiles, cleared areas being left idle between construction phases, earth dikes, etc. and for Permanent Seeding

are lawns, dams, cut and fill slopes and other areas at final grade, former stockpile and staging areas, etc. EFFECTS ON WATER QUALITY AND QUANTITY Planting vegetation in disturbed areas will have an effect on the water budget, especially on volumes and rates of runoff. infiltration evaporation, transpiration, percolation, and groundwater recharge. Vegetation, over time, will increase organic matter content and improve the water holding capacity of the soil and subsequent plant growth. Vegetation will help reduce the movement of sediment, nutrients, and other chemicals carried by runoff to receiving waters. Plants will also help protect groundwater supplies by assimilating those substances present within the root zone.

Sediment control devices must remain in place during grading, seedbed preparation, seeding, mulching and vegetative establishment

SECTION 1 - VEGETATIVE STABILIZATION METHODS AND MATERIALS

to prevent large quantities of sediment and associated chemicals and nutrients from washing into surface waters

Install erosion and sediment control structures (either temporary of permanent) such as diversions, grade stabilization structures, berms, waterways, or sediment control basins. ii. Perform all grading operations at right angles to the slope. Final grading and shaping is not usually necessary for temporary seeding.

iii. Schedule required soil tests to determine soil amendment composition and application rates for sites

having disturbed area over 5 acres.

Soil Amendments (Fertilizer and Lime Specifications) Soil tests must be performed to determine the exact ratios and application rates for both lime and fertilizer on sites having disturbed areas over 5 acres. Soil analysis may be performed by the University of Maryland or a recognized commercial laboratory. Soil samples taken for engineering

purposes may also be used for chemical analyses. Fertilizers shall be uniform in composition, free flowing and suitable for accurate application by approved equipment. Manure may be substituted for fertilizer with prior approval from the appropriate approval authority. Fertilizers shall all be delivered to the site fully labeled according to the applicable state fertilizer laws and shall bear the name, trade name or trademark and warranter of the producer.

iii. Lime materials shall be ground limestone (hydrated or burnt lime may be substituted) which contains at least 50% total oxides (calcium oxide plus magnesium oxide). Limestone shall be ground to such fineness that at least 50% will pass through a •100 mesh sieve and 98-100% will pass through a •20 . Incorporate lime and fertilizer into the top 3-5" of soil by disking or other suitable means. Seedbed Preparation

a. Seedbed preparation shall consist of loosening soil to a depth of 3" to 5" by means of suitable agricultural or construction equipment, such as disc harrows or chisel plows or rippers mounted on construction equipment. After the soil is loosened it should not be rolled or dragged smooth, but left in the roughened condition. Sloped areas (greater than 3:1) should be tracked leaving the surface in an irregular condition with ridges running parallel to the contour of the slope.

Apply fertilizer and lime as prescribed on the plans.
 In corporate lime and fertilizer into the top 3-5° of soil by disking or other suitable means.
 Permanent Seeding

Minimum soil conditions required for permanent vegetative establishment:
1. Soil pH shall be between 6.0 and 7.0.

Soluble salts shall be less than 500 parts per million (ppm). The soil shall contain less than 40% clay, but enough fine grained material (>30% silt plus clay) to provide the capacity to hold a moderate amount of moisture. An exception is if lovegrass or serecia lespedezas is to be planted, then a sandy soil (30% silt plus clay) would be acceptable.

1. Soil shall contain 1.5% minimum organic matter by weight. Soil must contain sufficient pore space to permit adequate root penetration. 6. If these conditions cannot be met by soils on site, adding topsoil is required

in accordance with Section 21 Standard and Specification for Topsoil. b. Areas previously graded in conformance with the drawings shall be maintained in a true and even grade, then scarified or otherwise loosened to a depth of 3-5" to permit bonding of the topsoil to the surface area and to create horizontal erosion check slots to prevent topsoil to the surface area and to create horizontal erosion check slots to prevent topsoil from sliding down a slope.

Apply soil amendments as per soil test or as included on the plans. Mix soil amendments into the top 3-5" of topsoil by disking or other suitable means. Lawn areas should be raked to smooth the surface, remove large objects like stones and branches, and ready the area for seed and application. Where site conditions will not permit normal seedbed preparation, loosen surface soil by dragging with a heavy chain or other equipment to roughen the surface. Steep slopes (steeper than 3:1) should be tracked by a dozer leaving the soil in an irregular condition with ridges running parallel to the contour of the slope. The top 1-3" of soil should be loose and friable. Seedbed loosening may not be necessary on

newly disturbed areas.

8-27-09

 All seed must meet the requirements of the Maryland State Seed Law. All seed shall be subject to re-testing by a recognized seed laboratory. All seed used shall have been tested within the 6 months immediately preceding the date of sowing such material on this job. Note: Seed tags shall be made available to the inspector to verify type and rate of seed used. ii. Inoculant - The inoculant for freating legume seed in the seed mixtures shall be a pure culture of nitrogen-fixing bacteria prepared specifically for the species. Inoculants shall not be used later than the date indicated on the container. Add fresh inoculant as directed on package. Use four times the recommended rate when hydroseeding. Note: It is very important to keep inoculant as cool as possible until used. Temperatures above 75°-80° F. can weaken bacteria and make the inoculant less effective.

E. Methods of Seeding Hydroseeding: Apply seed uniformly with hydroseeder (slurry includes seed and fertilizer), broadcast or drop seeded, or a cultipacker seeder. a. If fertilizer is being applied at the time of seeding, the application rates amounts will not

exceed the following: nitrogen: maximum of 100 lbs. per acre total of soluble nitrogen: P205 (phosphorous): 200 lbs/ac; K20 (potassium): 200 lbs/ac. b. Lime - use only ground agricultural limestone. (Up to 3 tons per acre may be applied by hydroseeding). Normally, not more than 2 tons are applied by hydroseeding at any one

Seed and fertilizer shall be mixed on site and seeding shall be done immediately and without interruption. ii. Dry Seeding: This includes use of conventional drop or broadcast spreaders.

Seed spread dry shall be incorporated into the subsoil at the rates prescribed on the Temporary or Permanent Seeding Summaries or Tables 265 or 26. The seeded area shall then be rolled with a weighted roller to provide good seed to soil contact. Where practical, seed should be applied in two directions perpendicular to each other. Apply half the seeding rate in each direction.

iii. Drill or Cultipacker Seeding: Mechanized seeders that apply and cover seed with soil. Cultipacking seeders are required to bury the seed in such a fashion as to provide at least 1/4 inch of soil covering. Seedbed must be firm after planting. Where practical, seed should be applied in two directions perpendicular to each other.

F. Mulch Specifications (In order of preference) Straw shall consist of thoroughly threshed wheat, rye or oat straw, reasonable bright in color, and shall not be musty, moldy, caked, decayed, or excessively dusty and shall be free of noxious weed seeds as specified in the Maryland Seed Law.

i. Wood Cellulose Fiber Mulch (WCFM)
 a. WCFM shall consist of specially prepared wood cellulose processed into a uniform

b. WCFM shall be dyed green or contain a green dye in the package that will provide an appropriate color to facilitate visual inspection of the uniformly spread slurry. WCFM, including dye, shall contain no germination or growth inhibiting factors. WCFM materials shall be manufactured and processed in such a manner that the wood cellulose fiber mulch will remain in uniform suspension in water under agitatio and will blend with seed, fertilizer and other additives to form a homogeneous slurry

The mulch material shall form a blotter-like ground cover, on application having moisture absorption and percolation properties and shall cover and hold grass seed in contact with the soil without inhibiting the growth of the grass seedlings. WCFM material shall contain no elements or compounds at concentration levels that

f. WCFM must conform to the following physical requirements: fiber length to approximately 10 mm., diameter approximately 1 mm., pH range of 4.0 to 8.5, ash content of 1.6% maximum and water holding capacity of 90% minimum.

Note: Only sterile straw mulch should be used in areas where one species of grass is desired.

i. Mulchina Seeded Areas - Mulch shall be applied to all seeded areas immediately after seeding. If grading is completed outside of the seeding season, mulch along shall be applied as presc this section and maintained until the seeding season returns and seeding can be performed in accordance with these specifications.

ii. When straw mulch is used it shall be spread over all seeded areas at the rate of 2 tons/acre. Mulch shall be applied to a uniform loose depth of between 1" and 2". Mulch applied shall achieve a uniform distribution and depth so that the soil surface is not exposed. If a mulch anchoring tool is to be used, the rate should be increased to 2.5 tons/acre.

iii. Wood cellulose fiber used as a mulch shall be applied at a net dry weight of 1,500 lbs. per acre. The wood cellulose fiber shall be mixed with water, and the mixture shall contain a maximum of 50 lbs of wood cellulose fiber per 100 gallons of water.

H. Securing Straw Mulch (Mulch Anchoring): Mulch anchoring shall be performed immediately following mulch application to minimize loss by wind or water. This may be done by one of the following methods (listed by preference), depending upon size of area and erosion hazard:

i. A mulch anchoring tool is a tractor drawn implement designed to punch and anchor mulch into the soil surface a minimum of two (2) inches. This practice is most effective on large areas, but is limited to flatter slopes where equipment can operate safely. If used on sloping land, this practice should be used on the contour if possible.

ii. Wood cellulose fiber may be used for anchoring straw. The fiber binder shall be applied at a net dry weight of 750 pounds/acre. The wood cellulose fiber shall be mixed with water and the mixture shall contain a maximum of 50 pounds of wood cellulose fiber per 100 gallons

iii. Application of liquid binders should be heavier at the edges where wind catches mulch, such as in valleys and crest of banks. The remainder of area should be appear uniform after binder application. Synthetic binders - such as Acrylic DLR (Agro-Tack), DCA-70 Petroset, Terra Tax II. Terra Tack AR or other approved equal may be used at rates recommended by the manufacturer to anchor mulch.

iv. Lightweight plastic netting may be stapled over the mulch according to manufacturer's recommendations. Netting is usually available in rolls 4' to 15' feet wide and 300 to 3,000 feet long. 1. Incremental Stabilization - Cut Slopes

All cuts slopes shall be dressed, prepared, seeded and mulched as the work progresses. Slopes shall be excavated and stabilized in equal increments not to exceed 15'. ii. Construction sequence (Refer to Figure 3 below):

a. Excavate and stabilize all temporary swales, side ditches, or berms that will be used to convey runoff from the excavation.
b. Perform Phase 1 excavation, dress, and stabilize.

c. Perform Phase 2 excavation, dress and stabilize. Overseed Phase 1 areas as

necessary.
d. Perform final phase excavation, dress and stabilize. Overseed previously seeded

Note: Once excavation has begun the operation should be continuous from grubbing through the completion of grading and placement of topsoil (if required) and permanent seed and mulch. Any interruptions int he operation of completing the operation out of the seeding season will necessitate the application of temporary stabilization.

J. Incremental Stabilization of Embankments - Fill Slopes Embankments shall be constructed in lifts as prescribed on the plans.

Slopes shall be stabilized immediately when the vertical height of the multiple lifts reaches 15°, or when the grading operation ceases as prescribed in the plans.

iii. At the end of each day, temporary berns and pipe slope drains should be constructed along the top edge of the embarkment to intercept surface runoff and convey it down the slope in a non-erosive manner to

of the embankment to intercept surface runoff and convey it down the slope in a non-erosive a sediment trapping device.

Construction sequence: Refer to Figure 4 (below).

a. Excavate and stabilize all temporary swales, side ditches, or berms that will be used to divert runoff around the fill. Construct slope silt fence on low side of fill as shown in Figure 5, unless other methods shown on the plans address this area.

b. Place Phase 1 embankment, dress and stabilize.

c. Place Phase 2 embankment, dress and stabilize.

Overseed previously seeded

the operation out of the seeding season will necessitate the application of temporary stabilization.

Place final phase embankment, dress and stabilize. Overseed previously seeded areas as necessary.

Note: Once the placement of fill has begun the operation should be continuous from grubbing through the completion of and placement of topsoil (if required) grading and permanent seed and mulch. any interruptions in the operation or completing

SECTION 2 - TEMPORARY SEEDING

Vegetation - annual grass or grain used to provide cover on disturbed areas for up to 12 months. For longer duration of vegetative cover, Permanent Seeding is required. A. Seed mixtures - Temporary Seeding

i. Select one or more of the species or mixtures listed in Table 26 for the appropriate Plant Hardiness Zone (from Figure 5) and enter them in the Temporary seeding summary bel with application rates, seeding dates and seeding depths. If this summary is not put on the plans and completed, then Table 26 must be put on the plans.

ii. For sites having soil tests performed, the rates shown on this table shall be deleted and the rates recommended by the testing agency shall be written in Soil tests are not required for Temporary Seeding.

5e	ed Mixture (Hard Fron	diness Zone <u>6b</u> n Table 26	_)	Fertilizer Rate			
No.	Species	Application Rate (lb/ac)	Seeding Dates	Seeding Depths	(10-10-10)	inche de la constante de la co	
1	BARLEY OATS RYE	122 96 140	3/1 - 5/15, 8/15 - 10/15	1" - 2" 1" - 2" 1" - 2"	600 b/ac (15 b/1000sf)	2 tons/ac (100 (b/1000sf)	

SECTION 3 - PERMANENT SEEDING

Seeding grass and legumes to establish groung cover for a minimum of one year on disturbed areas

A. Seed mixtures - Permanent Seeding

. Select one or more of the species or mixtures listed in Table 25 for the appropriate Plant Hardiness Zone (from Figure 5) and enter them in the Permanent Seeding Summary below, along with application rates and seeding dates. Seeding depths can be estimated using Table 26. If this summary is not put on the construction plans and completed, then Table 25 must be put on the plans. Additional planting specifications for exceptional sites such as shorelines, streambanks, or dunes or for special purposes such as wildlife or aesthetic treatment may be found in USDA-SCS Techinical Field Office Guide, Section 342 - Critical Area Planting. For special lawn maintenance areas, see Sections IV 5od and V Turfgrass

ii. For sites having disturbed area over 5 areas, the rates shown on this table shall be deleted and the rates recommended by the soil testing agency shall be written in.

iii. For areas receiving low maintenance, apply ureaform fertilizer (46-0-0) at 3 1/2 lbs/1000 sq. ft. (150 lbs/ac), in addition to the above soil amendments shown in the table below, to be performed at

	Seed Mixture (Hardiness Zo From Table	ess Zone 6b Table 25			Fertilizer Rate (10-20-20)			Lime Rate	
No.	Species	Application Rate (lb/ac)	Seeding Dates	Seeding Depths	N	P205	K20		
3	TALL FESCUE (195%) PERENNIAL RYE GRASS (10%) KENTUCKY BLUEGRASS (5%)	125 15 10	3/1 - 5/15, 8/15 - 10/15	1" - 2"	90 lb/ac (2.0 lb/	175 b/ac	175 lb/ac (4 lb/	2 tons/ac	
10	TALL FESCUE (00%) HARD FESCUE (20%)	120 30	3/1 - 5/15, 8/15 - 10/15	1" - 2"	1000sf)	1000sf)	1000sf)	1000sf)	

Sequence of Construction

1. OBTAIN A GRADING PERMIT AND MDE/CORPS OF ENGINEERS PERMITS AS REQUIRED, PER G.N. 23.

2. NOTIFY MISS UTILITY "AT LEAST 48 HOURS BEFORE BEGINNING ANY WORK AT 1-800-257-7777. NOTIFY THE HOWARD COUNTY OFFICE OF CONSTRUCTION/INSPECTION AT 410-313-1330 24-HOURS BEFORE STARTING WORK.

3. CONTRACTOR SHALL UTILIZE THE EXISTING DRIVEWAY CROSSING FOR ACCESS TO THE WESTERN SIDE OF THE STREAM FOR CONSTRUCTION ACCESS. UPON COMPLETION OF THE TWIN ARCH BARNSLEY WAY ROADWAY REMOVAL OF THIS DRIVEWAY SHALL BE REMOVED, GRADED & STABILIZED IN ACCORDANCE WITH THE MDE PERMIT CONDITIONS OF APPROVAL AND ASSOCIATED FINAL MITIGATION PLAN. SEE S.O.C. SPECIFIC TO MITIGATION WORK ON SHEET 25.

4. CLEAR AND GRUB FOR SEDIMENT CONTROL MEASURES ONLY. INSTALL STABILIZED CONSTRUCTION ENTRANCE FOR THE STAGING AREA NEAR THE INTERSECTION OF WARWICK WAY AND MARRIOTTSVILLE ROAD. (2 weeks)

5. INSTALL THE REMAINING SEDIMENT CONTROL MEASURES. THIS WOULD INLCUDE THE SEDIMENT BASIN . TREE PROTECTION FENCE, EARTH DIKES AND SILT FENCE AS INDICATED ON THESE PLANS. (2 weeks)

6. OBTAIN PERMISSION OF THE SEDIMENT CONTROL INSPECTOR PRIOR TO PROCEEDING

7. CLEAR AND GRUB FOR THE REMAINDER OF THE STAGING AREA SITE TO BE USED FOR THE STREAM CROSSING TWIN ARCH PIPES CONSTRUCTION. THE REMAINDER OF THE SITE MAY BE GRADED PRIOR TO THE STREAM CROSSING BASED ON THE CONTRACTORS PREFERENCE. (4 weeks)

6. GRADE THE CUL-DE-SAC AREAS TO THE PROPOSED SUBGRADE AND INSTALL THE STORM DRAIN SYSTEM AND UTILITIES. STABLIZE ALL ROADWAY SLOPES IMMEDIATELY UPON COMPLETION OF GRADING AS SHOWN. SEQUENCE THE CONSTRUCTION OF STORM DRAIN FROM 5-1 TO 1-4 TO OCCUR AFTER THE MASS GRADING OF THIS AREA. (8 weeks)

9. CONSTRUCT THE PROPOSED TWIN PIPE ARCHES AND ASSOCITED SCOUR PROTECTION. GRADE THE PROPOSED BARNSLEY WAY TO THE PROPOSED SUBGRADE. INSTALL UTILITIES AS REQUIRED. (10 Weeks)

10. INSTALL BASE COURSE PAVING FOR THE PROPOSED ROADS. (2 weeks) 11. STABILIZE ALL AREAS AND OBTAIN PERMISSION FROM THE SEDIMENT CONTROL INSPECTOR PRIOR TO PROCEEDING.

12. APPLY TACK COAT TO BASE COURSE AND LAY SURFACE COURSE PAVING. (1 week) 13. WHEN ALL CONTRIBUTING AREAS TO THE SEDIMENT CONTROL MEASURES HAVE BEEN STABILIZED AND WITH THE PERMISSION OF THE SEDIMENT CONTROL INSPECTOR, THE TEMOPRARY DEVICE MAY BE REMOVED, BACKFILLED OR REGRADED TO THE PROPOSED FINAL GRADES. STABILIZE ALL REMAINING AREAS WITH PERMANENT SEEDING NOTES. (4

14. NOTIFY HOWARD COUNTY OFFICE OF INSPECTIONS AND PERMITS FOR A FINAL INSPECTION OF THE COMPLETED PROJECT.

SEQUENCE NOTE: THE CONTRACTOR SHALL INSPECT AND PROVIDE NECESSARY MAINTENANCE ON ALL SEDIMENT AND EROSION CONTROL STRUCTURES SHOWN HEREON AFTER EACH RAINFALL EVENT AND ON A DAILY BASIS.

TOPSOIL NOTES

Placement of topsoil over a prepared subsoil prior to establishment of permanent vegetation.

To provide a suitable soil medium for vegetative growth. Soils of concern have low moisture content, low nutrient levels, low pH, materials toxic to plants, and/or unacceptable soil gradation. Conditions Where Practice Applies

1. This practice is limited to areas having 2:1 or flatter slopes where:

a. The texture of the exposed subsoil/parent material is not adequate to produce vegetative growth. b. The soil material is so shallow that the rooting zone is not deep enough to support plants or furnish continuing supplies of moisture and plant nutrients. c. The original soil to be vegetated contains material toxic to plant growth. d. The soil is so acidic that treatment with limestone is not feasible.

For the purpose of these Standards and Specifications, areas having slopes steeper than 2:1 require special consideration and design for adequate stabilization. Areas having slopes steeper than 2:1 shall have the appropriate stabilization shown on the plans. Construction and Material Specifications

Topsoil salvaged from the existing site may be used provided that it meets the standards as set forth in these specifications. Typically, the depth of topsoil to be salvaged for a given soil type can be found in the representative soil profile section in the Soil Survey published by USDA-SCS in peration with Maryland Agricultural Experimental Station

Topsoil Specifications - Soil to be used as topsoil must meet the following:

i. Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay loam, loamy sand. Other soils may be used if recommended by an agronomist or soil scientist and approved by the appropriate approval authority. Regardless, topsoil shall not be a mixture of contrasting textured subsoils and shall contain less than 5% by volume of cinders, stores, slag, coarse fragments, gravel, sticks, roots, trash, or other materials larger than 11/2" in diameter.

Topsoil must be free of plants or plant parts such as bermuda grass, quackgrass, Johnsongrass, nutsedge, poison ivy. thistle, or others as specified.

iii. Where the subsoil is either highly acidic or composed of heavy clays, ground limestone shall be spread at the rate of 4-8 tons/acre (200-400 pounds per 1,000 square feet) prior to the placement of topsoil. Lime shall be distributed uniformly over designated areas and worked into the soil in conjunction with tillage operations as described in the following procedures.

for sites havinc, disturbed areas under 5 acres: i. Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative

zation - Section I - Vegetative Stabilization Methods and Materials. For sites having disturbed areas over 5 acres:

i. On soil meeting Topsoil specifications, obtain test results dictating fertilizer and lime

a. pH for topsoil shall be between 6.0 and 7.5. If the tested soil demonstrates a pH of less n 6.0, sufficient lime shall be perscribed to raise the pH to 6.5 or high b. Organic content of topsoil shall be not less than 1.5 percent by weight.

c. Topsoil having soluble salt content greater than 500 parts per million shall not be used. d. No sod or seed shall be placed on soil which has been treated with soil sterilants or

chemicals used for weed control until sufficient time has elapsed (14 days min.) to permit

Note: Topsoil substitutes or amendments, as recommended by a qualified agranomist or soil scientist and approved by the appopriate approval authority, may be used in lieu of natural topsoil.

ii. Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative

i. When topsoiling, maintain needed erosion and sediment control practices such as diversions, Grades on the areas to be topsoiled, which have been previously established, shall be maintained, albeit 4" - 8" higher in elevation.

iii. Topsoil shall be uniformly distributed in a 4" - 9" layer and lightly compacted to a minimum thickness of 4". Spreading shall be performed in such a manner that sodding or seedine can proceed with a minimum of additional soil preparation and tillage. Any irregularities in the surface resulting from topsoiling or other operations shall be corrected in order to prevent the formation of depressions or water pockets.

iv. Topsoil shall not be placed while the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet or in a condition that may otherwise be detrimental to proper grading and seedbed preparation.

Alternative for Permanent Seeding - Instead of applying the full amounts of lime and commercial fertilizer, composted sludge and amendments may be applied as specified below: i. Composted Sludge Material for use as a soil conditioner for sites having disturbed areas over 5 acres shall be tested to prescribe amendments and for sites having disturbed areas under 5 acres shall conform to the following requirements: a. Composted sludge shall be supplied by, or originate from, a person or persons that are permitted (at the time of acquisition of the compost) by the Maryland Department of the Environment under COMAR 26.04.06.

b. Composted sludge shall contain at least 1 percent nitrogen, 1.5 percent phosphorus, and 0.2 percent potassium and have a Ph of 7.0 to 8.0. If compost does not meet these requirements. the appropriate constituents must be added to meet the requirements prior to use. c. Composted sludge shall be applied at a rate of 1 ton/1,000 square feet.

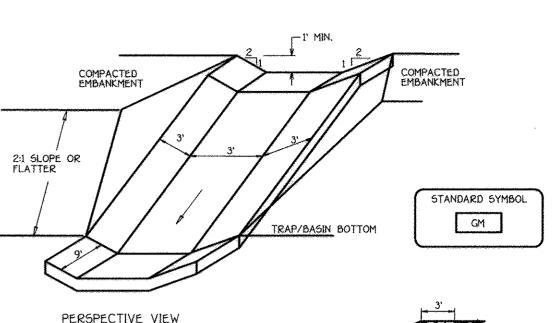
iv. Composted sludge shall be amended with a potassium fertilizer applied at the rate of 4 lb/1,000

References: Guideline Specifications, Soil Preparation and Sodding, MD-VA, Pub. *1. Cooperative Extension Service, University of Maryland and Virginia Polytechnic Institutes. Revised 1973.

State of Maryland, License No. 20748, Expiration Date 2-22-11.

square feet, and 1/3 the normal lime application rate.

GABION INFLOW PROTECTION NOT TO SCALE



PROFILE ALONG CENTERLINE Construction Specifications

1. Gabion inflow protection shall be constructed of 9' x 3' x 9" gabion baskets forming a trapezoidal cross section 1' deep, with 2:1 side

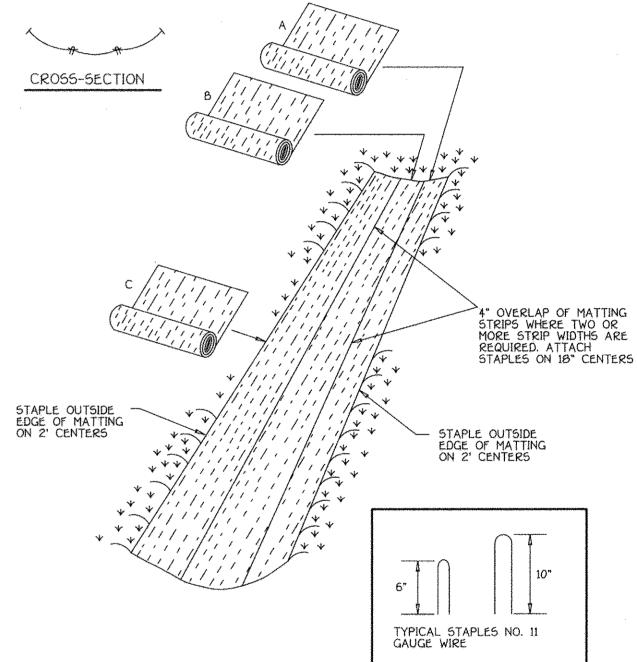
2. Geotextile Class C shall be installed under all gabion baskets.

3. The stone used to fill the gabion baskets shall be 4" - 7". 4. Gabions shall be installed in accordance with manufacturers recommendations.

5. Gabion Inflow Protection shall be used where concentrated flow is present on slopes steeper than 4:1.

EROSION CONTROL MATTING

NOT TO SCALE



Construction Specifications

1. Key-in the matting by placing the top ends of the matting in a narrow trench, 6" in depth. Backfill the trench and tamp firmly to conform to the channel cross-section. Secure with a row of staples about 4" down slope from the trench. Spacing between staples is 6".

2. Staple the 4" overlap in the channel center using an 18" spacing between staples. 3. Before stapling the outer edges of the matting, make sure the

matting is smooth and in firm contact with the soil. 4. Staples shall be placed 2' apart with 4 rows for each strip, 2

outer rows, and 2 alternating rows down the center.

5. Where one roll of matting ends and another begins, the end of the top strip shall overlap the upper end of the lower strip by 4", shiplap fashion. Reinforce the overlap with a double row of staples spaced 6" apart in a staggered pattern on either side.

6. The discharge end of the matting liner should be similarly secured with 2 double rows of staples.

> Note: If flow will enter from the edge of the matting then the area effected by the flow must be keyed-in.

> > SEDIMENT CONTROL NOTES AND DETAILS

GTW'S WAVERLY WOODS

SECTION 14 BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2

TAX MAP NO. 16 PARCEL Nos. 120, 221 & P/O 249 GRID Nos. 3 & 4 THIRD ELECTION DISTRICT HOWARD COUNTY, MARYLAND DATE: JULY 28, 2009 SHEET 9 OF 27

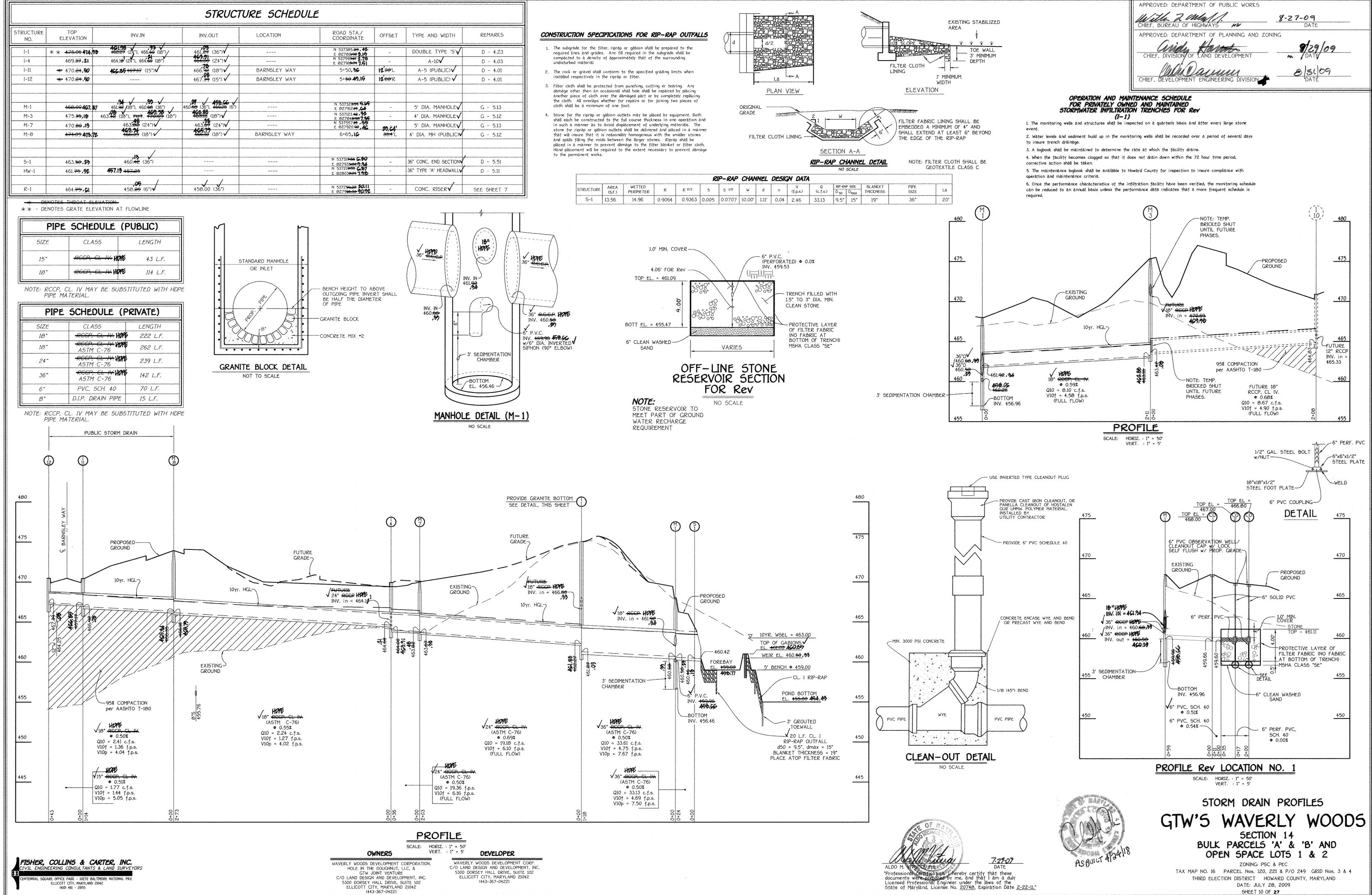
ZONING: PSC & PEC

FISHER, COLLINS & CARTER, INC. AL SQUARE OFFICE PARK - 10272 BALTIMORE NATIONAL PIK (410) 461 - 2855

OWNERS

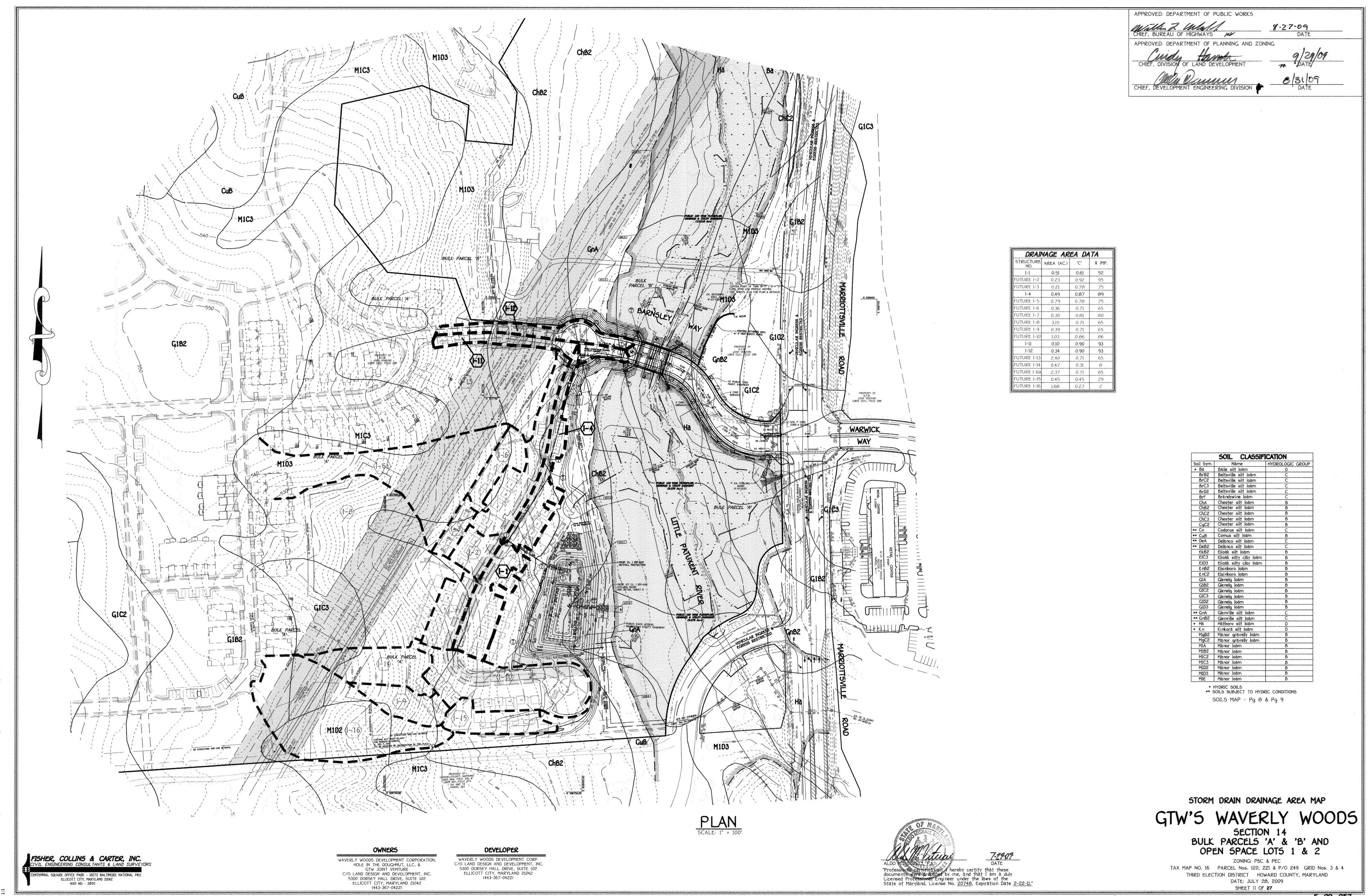
WAVERLY WOODS DEVELOPMENT CORPORATION, HOLE IN THE DOUGHNUT, LLC, & GTW JOINT VENTURE C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 FLLICOTT CITY, MARYLAND 21042

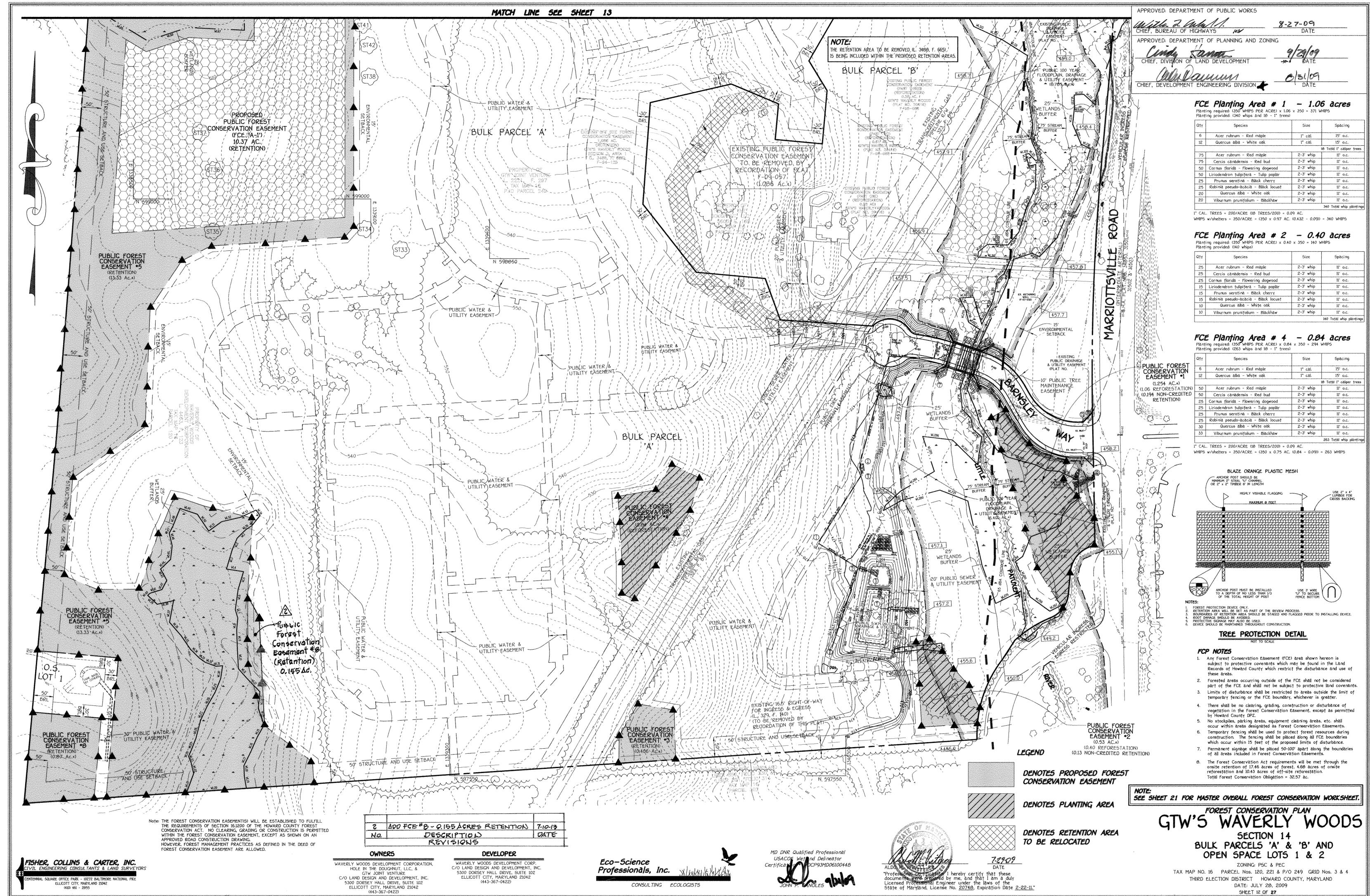
DEVELOPER (443-367-0422)

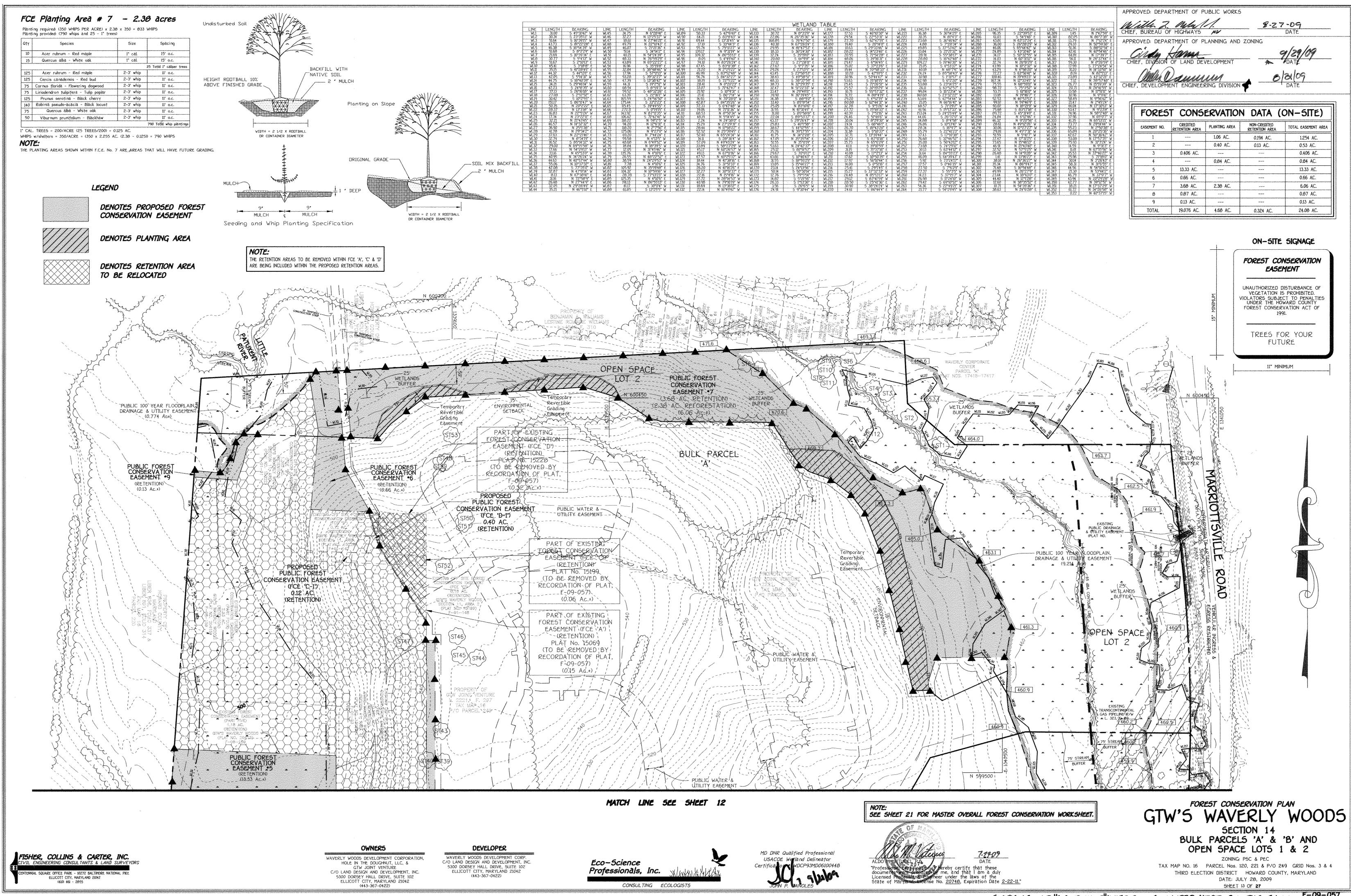


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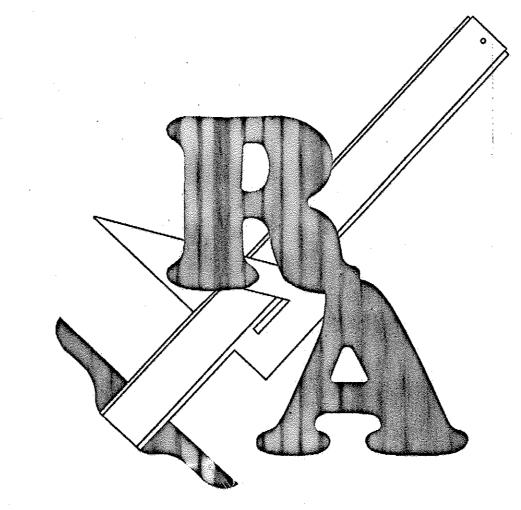




BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2 ZONING: PSC & PEC TAX MAP No. 16 PARCEL Nos. 120,221 & P/O 249 GRID Nos. 3 & 4 THIRD ELECTION DISTRICT, HOWARD COUNTY, MD

34'7"x11'4" 3ga PLATE ARCH CULVERT

STRUCTURAL DESIGN



RYAN & ASSOCIATES

A Division of WKR Consulting, Inc. CONSULTING & DESIGN ENGINEERS

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SPECIALIZING IN STRUCTURAL ENGINEERING, GEOTECHNICAL ENGINEERING AND RETAINING WALL DESIGN www.ryanandassociates.net

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WRITTEN DIMENSIONS ON THE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALE DIMENSIONS. CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THIS OFFICE MUST BE NOTIFIED OF ANY VARIATION FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THESE DRAWINGS.

Scope: The plate arch culvert engineer's (Ryan & Associates) scope consists of preparing the culvert design to enable the contractor to obtain necessary permits and properly construct the arch. The design considers the internal and local stability of the arch and is in accordance with acceptable engineering practice and these specifications. Services outside this scope such as responding to the owner's engineering firm (civil, structural, geotechnical or otherwise), provision of quality control testing & inspection, certification of arch construction, investigation of failed or non-conforming arches or any other services may be provided on a time & materials basis or for a negotiated fee. The scope of Ryan & Associates (RA) for this project does not include arch stakeout or any other civil engineering/surveying.

INSTALLATION MUST CONFORM TO THE ATTACHED "Ryan & Associates Structural Specifications and Guidelines".

PLATE ARCH MATERIALS: Structural Plate Arch to be supplied by Lane Enterprises, Inc. and to be 3 gauge steel (.249" thick). Structure type to be LA4108 low profile arch.

CONSTRUCTION REVIEW & CERTIFICATION: CONSTRUCTION INSPECTION, TESTING AND CERTIFICATION BY A STRUCTURAL/GEOTECHNICAL ENGINEER QUALIFIED IN PLANS. ACCEPTANCE OF THE USE OF THESE PLANS INDICATES AGREEMENT FOR

DRAWING INDEX

Sheet * - Cover Sheet

Sheet 2 - Head Walls, Wing Walls and Culvert Footings Plan

Sheet 3- Wall and Arch Profiles

Sheet 4 - Culvert & Footing Cross-Sections

Sheet & - Plate Arch Sections & Details

Sheet - Specifications

PROJECT INFORMATION

Project: Courtyards at Waverly Woods - West Stream Crossing

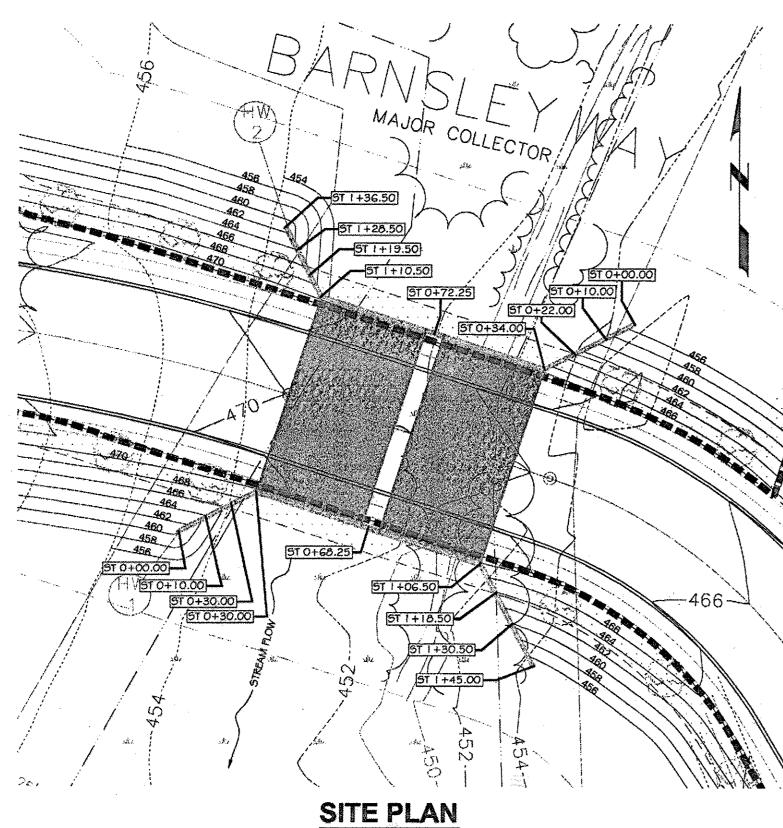
Location: Marriotsville Rd., North Side of I-70, Howard County, MD

Jurisdiction: Howard County, MD

Contractor: UNKNOWN

Site Civil Engineer: Fisher, Collins & Carter, Inc. Owner/Developer: Land Design & Development, Inc.

RA Project Manager: Chris Heyrend



CALL "MISS UTILITY"

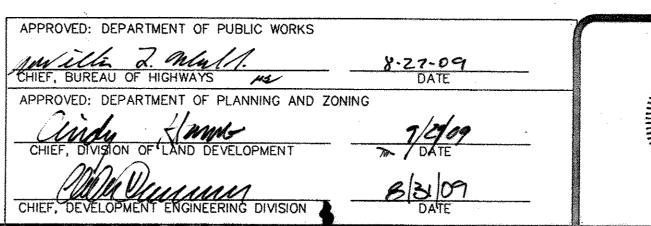
AT LEAST 48 HOURS BEFORE CONSTRUCTION.

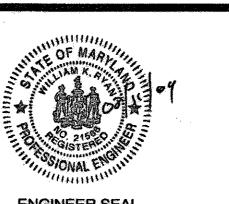
TELEPHONE 1-800-257-7777 FOR UTILITY LOCATIONS

VICINITY MAP

SITE PLAN
SCALE: 1" = 30"

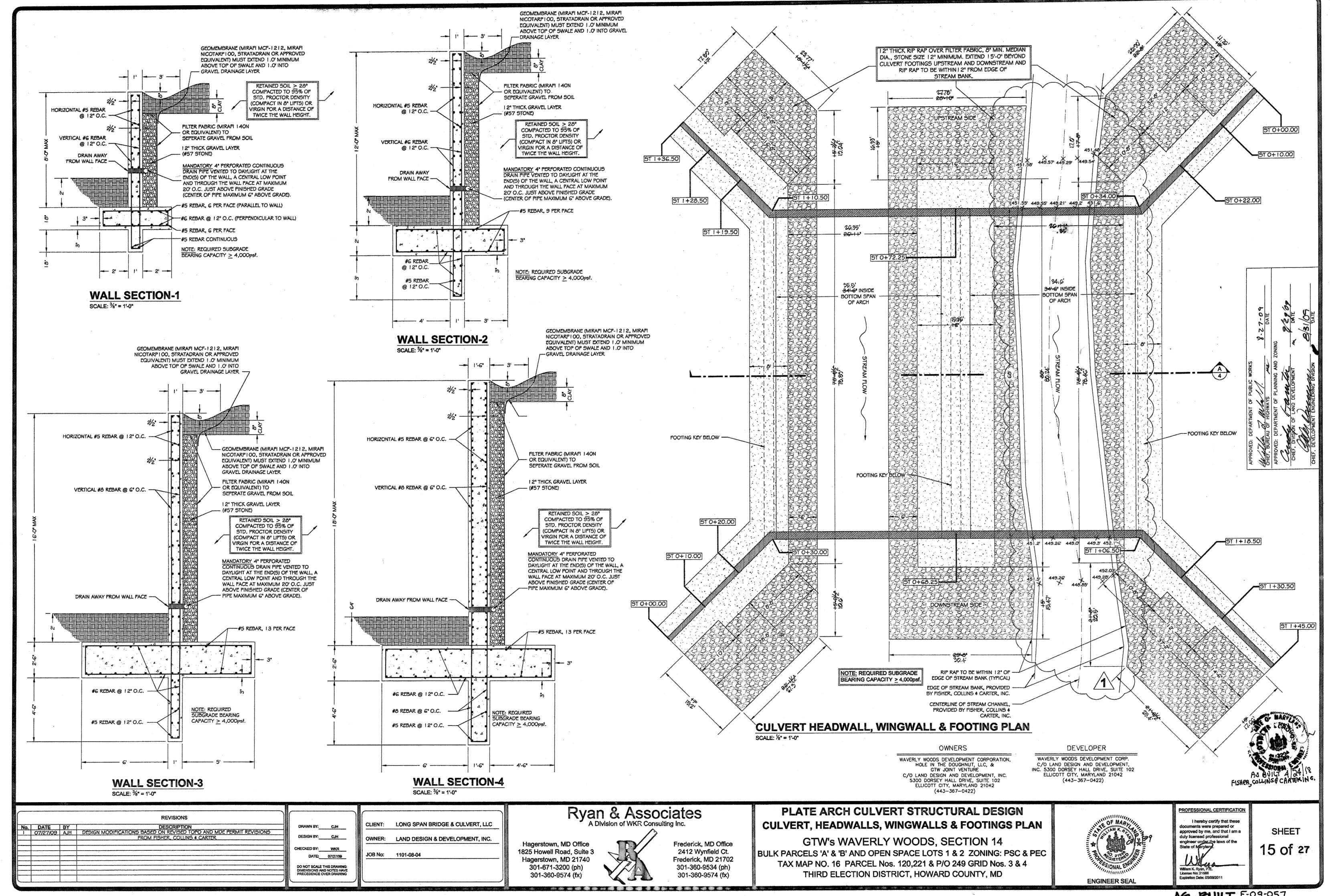
DEVELOPER

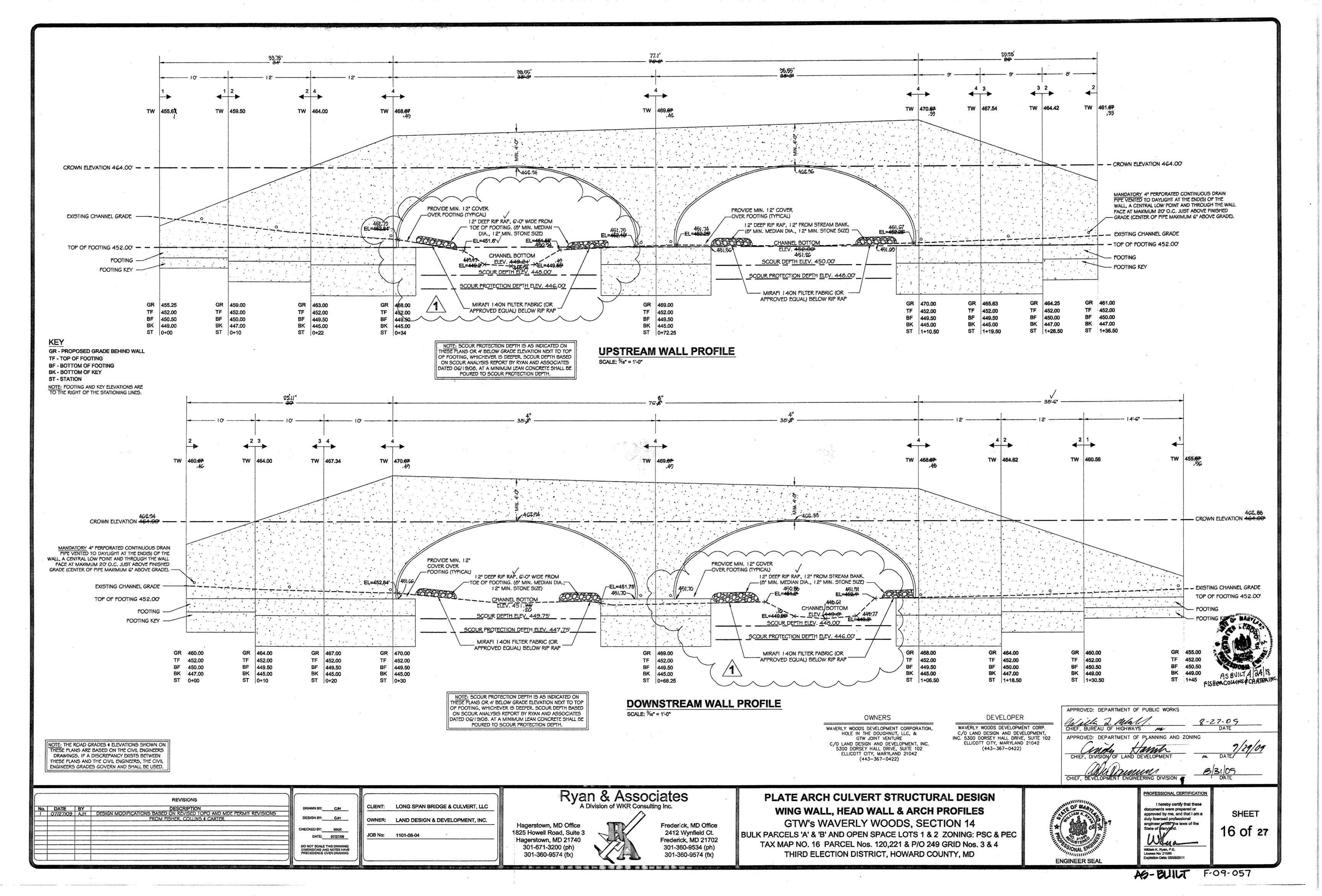


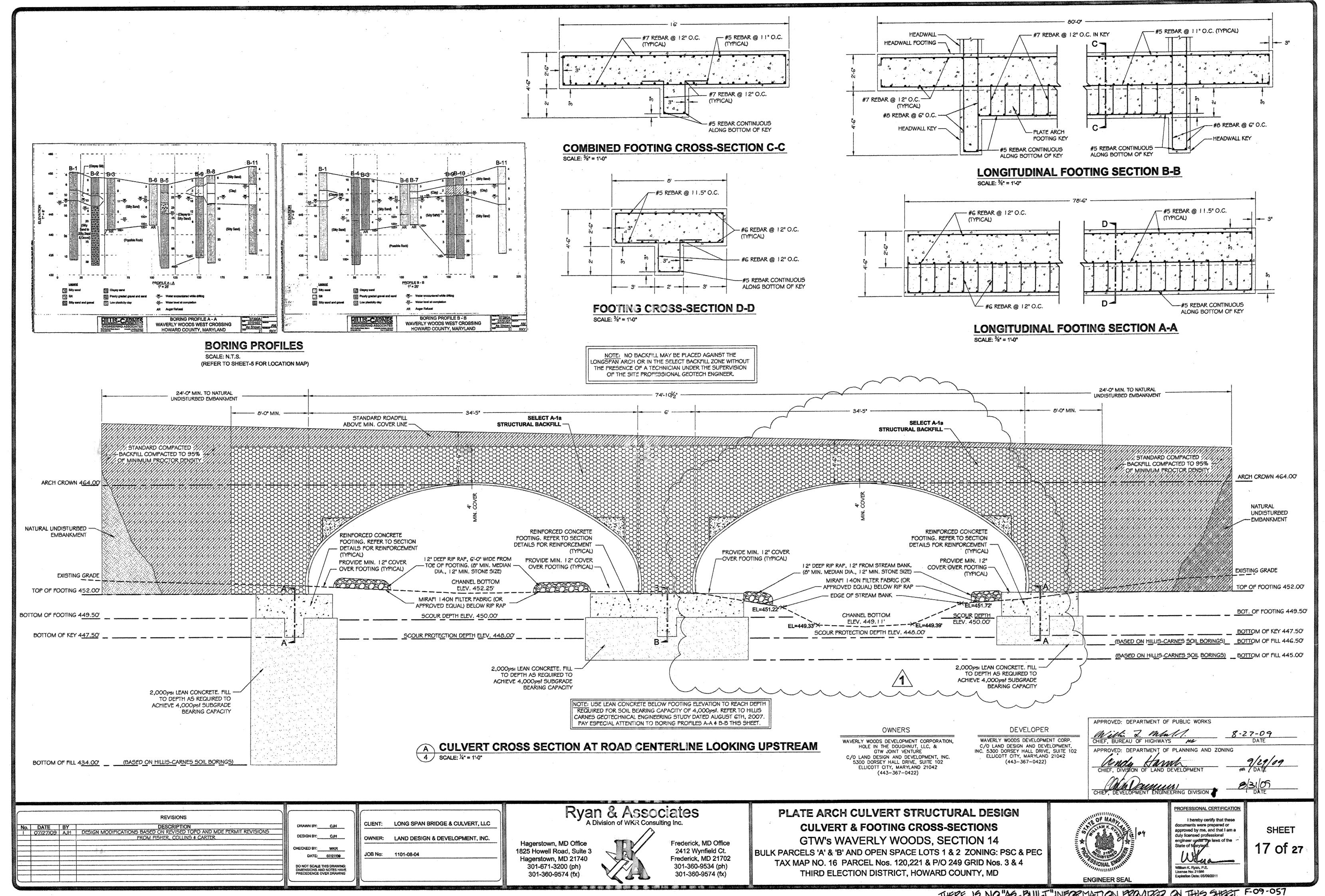


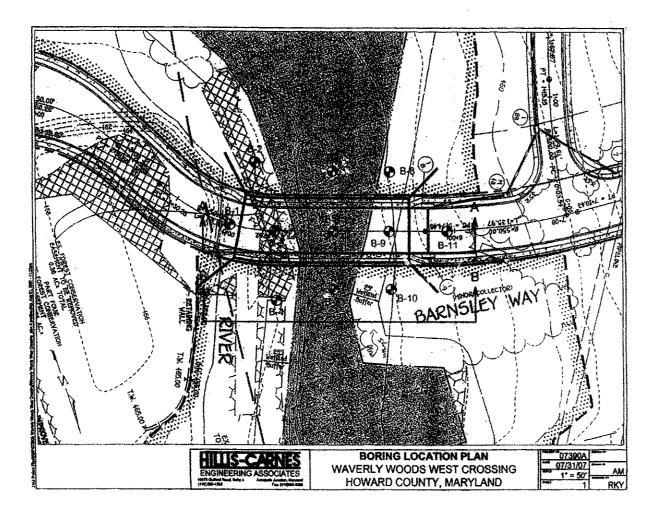
SHEET 14 of 27

THERE IS NO "AG-BLILT" INFORMATION PROVIDED ON THIS SHEET F-09-057



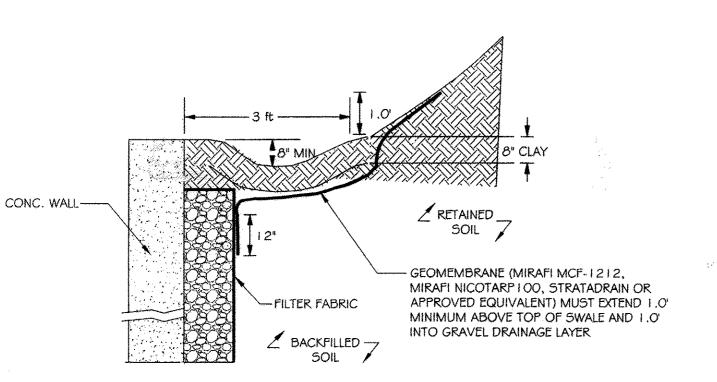






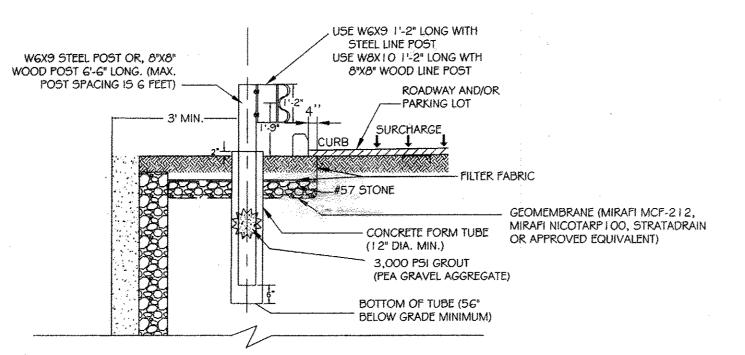
BORING LOCATION MAP

SCALE: N.T.S. (REFER TO SHEET-4 FOR BORING PROFILES)



GEOMEMBRANE LINED SWALE

N.T.S.



NOTES:

I. ENSURE PROPER SLOPE BETWEEN GUARDRAIL AND WALL SO AS TO

DIVERT WATER AWAY FROM WALL.

2. ALL STRUCTURAL STEEL SHALL CONFIRM TO ASTM A36.

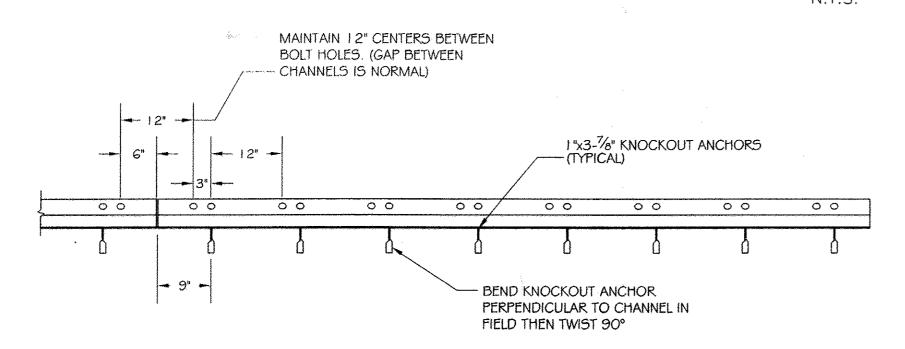
3. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE SHALL BE 3000 PSI.

3. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE SHALL BE 3000 PSI AT 28 DAYS.

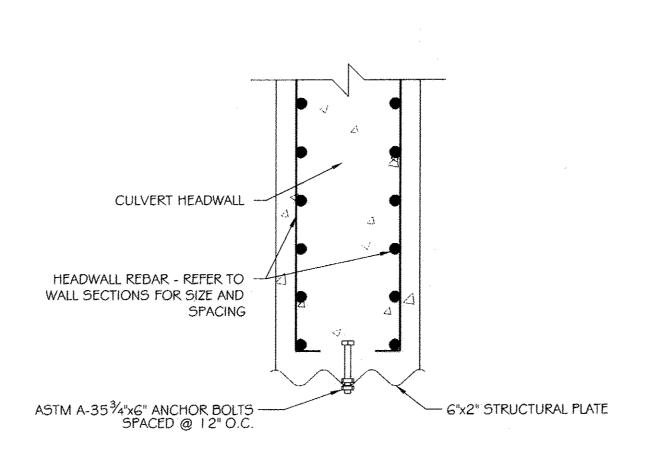
4. THE WALL INSTALLER TO PLACE THE GUARD RAIL POST SONOTUBES IN ACCORDANCE WITH THIS DETAIL FOR THE FENCE INSTALLER TO INSTALL GAURDRAIL POSTS AT A LATER DATE. THE SONOTUBES MUST BE COVERED WITH A SECURED PLASTIC SHEET TO PREVENT RAINWATER OR WATER RUN-OFF TO ENTER THE POST HOLES. IT IS HIGHLY RECOMMENDED THAT THE GUARDRAIL BE INSTALLED WITHIN A FEW DAYS OF WALL CONSTRUCTION COMPLETION.

GUARDRAIL DETAIL

GUF



BASE CHANNEL DETAIL SCALE: 3/4" = 1'-0"



HEADWALL CONNECTION DETAIL

LONGITUDINAL STIFFENER

#4 LONGITUDINAL

REBARS (CONTINUOUS)

BAR BENDING DETAIL

SCALE: N.T.S.

ANCHOR RODS

12" CENTERS

SCALE: 1" = 1'-0"

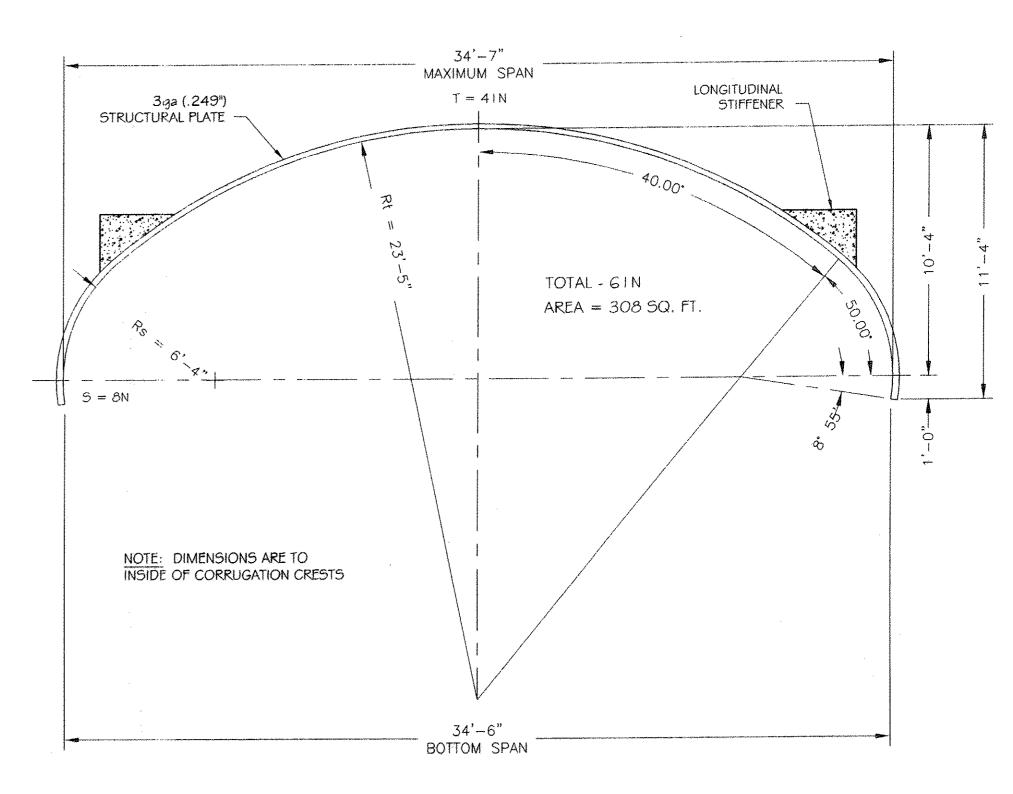
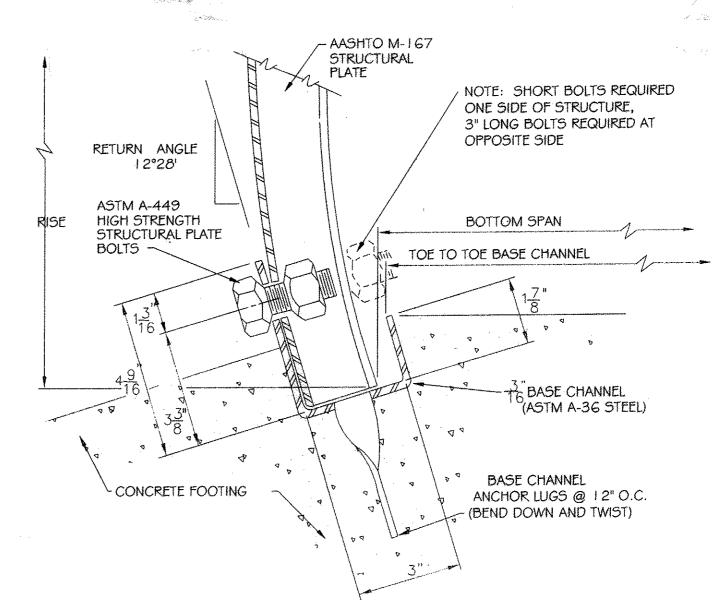


PLATE ARCH CROSS-SECTION

SCALE: 1/4" = 1'-0"



BASE CHANNEL SECTION

SCALE: N.T.S

OWNERS

WAVERLY WOODS DEVELOPMENT CORPORATION,
HOLE IN THE DOUGHNUT, LLC, &
GTW JOINT VENTURE

C/O LAND DESIGN AND DEVELOPMENT, INC.
5300 DORSEY HALL DRIVE, SUITE 102

ELLICOTT CITY, MARYLAND 21042 (443-367-0422) DEVELOPER

WAVERLY WOODS DEVELOPMENT CORP.
C/O LAND DESIGN AND DEVELOPMENT,
INC. 5300 DORSEY HALL DRIVE, SUITE 102
ELLICOTT CITY, MARYLAND 21042
(443-367-0422)

APPROVED: DEPARTMENT OF PUBLIC WORKS

27-09

CHIEF, BUREAU OF HIGHWAYS

APPROVED: DEPARTMENT OF PLANNING AND ZONING

CHIEF, DIVISION OF LAND DEVELOPMENT

CHIEF, DEVELOPMENT ENGINEERING DIVISION

DATE

REVISIONS O. DATE BY DESCRIPTION O7/27/09 AJH DESIGN MODIFICATIONS BASED ON REVISED TOPO AND MDE PERMIT REVISIONS FROM FISHER. COLLINS & CARTER DESIGN BY: CJH DESIGN BY: CJH DESIGN BY: CJH DESIGN BY: CJH DATE: 07/27/09 DO NOT SCALE THIS DRAWING: DIMENSIONS AND NOTES HAVE PRECEDENCE OVER DRAWING. DIMENSIONS AND NOTES HAVE PRECEDENCE OVER DRAWING.

Ryan & Associates A Division of WKR Consulting Inc.

Hagerstown, MD Office 1825 Howell Road, Suite 3 Hagerstown, MD 21740 301-671-3200 (ph) 301-360-9574 (fx)



Frederick, MD Office 2412 Wynfield Ct. Frederick, MD 21702 301-360-9534 (ph)

301-360-9574 (fx)

LONGITUDINAL STIFFENER DETAIL

PLATE ARCH CULVERT STRUCTURAL DESIGN PLATE ARCH SECTIONS & DETAILS

GTW's WAVERLY WOODS, SECTION 14

BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2 ZONING: PSC & PEC TAX MAP NO. 16 PARCEL Nos. 120,221 & P/O 249 GRID Nos. 3 & 4 THIRD ELECTION DISTRICT, HOWARD COUNTY, MD



I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

William K. Ryan, P.E. License No: 21586
Expiration Date: 05/09/2011

SHEET 18 of 27

STRUCTURAL NOTES & SPECIFICATIONS

- 1.01 Owner: Land Design and Development, Inc., Ellicott City, MD 21042
- 1.02 Contractor: TBD
- 1.03 Design Structural Engineer: Ryan & Associates, Hagerstown Office, MD 21740
- 1.04 Site Civil Engineer: Fisher, Collins & Carter, Ellicott City, MD 21042
- 1.05 Site Geotechnical Engineer: Hillis Carnes Engineering Associates, Inc., Annapolis Junction, MD 20701

If any of the above responsibilities change it is the owner's responsibility to notify LSBC prior to the start of the work. It is the owner's responsibility to make sure all parties listed above are aware of their roles, requirements, responsibilities and final submittals.

1.06 Reference Standards

- ASTM 3034- Specification for Polyvinyl Chloride (PVC) Plastic Pipe
- ASTM C 140- Sampling and Testing Concrete Masonry Units and related units
- ASTM D 422- Gradation of Soils
- ASTM D 698- (AASHTO T99) Standard Test Methods for Laboratory Compaction Characteristics of Soil using Standard Effort
- ASTM D 1248- Polyethelene Plastics Extrusion Materials for wire and Cable
- ASTM D 1557- (AASHTO T 180) Standard Test Method for Laboratory Compaction Characteristics of Soil using Modified Effort
- ASTM D 1586- Standard Test Method for Penetration Test and Split-Barrel Sampling of Soils
- ASTM D 2166- Unconfined Compressive Strength of Cohesive Soil
- ASTM D 2487- Standard Classification of Soils for Engineering Purposes (Unified Soil Classification
- ASTM D 3080- Direct Shear Test of Soils Under Consolidated Drained Conditions
- ASTM D 4318- Liquid Limit. Plastic Limit and Plasticity Incex of Soils
- ASTM D 2850- Unconsolidated, Undrained Compressive Strength of Cohesive Soils in Triaxial
- M. ASTM A 615- Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement
- ACI 318-02- Concrete Building Design and Construction
- ACI 315-99 "Manual of Standard Practice for Detailing Reinforcing Concrete Structures"
- AASHTO LRFD Bridge Design Specifications, 3rd Edition, 2004
- AASHTO Standard Specifications For Highway Bridges, 17th Edition, 2002
- AISI Handbook of Steel Drainage & Highway Construction Products, 1994 Edition

2: GENERAL

2.01 Contractor shall notify oversite review engineer responsible for construction certification of any discrepancies, omissions, or conflicts between the various elements of the working drawings and/or specifications before proceeding with any work involved. In all cases, unless otherwise directed by the engineer in writing, the most stringent requirements shall govern and be performed.

2.02 Contractor shall verify all conditions, dimensions and elevations, etc., at the site and shall coordinate work performed by all trades. Do not scale drawings.

- 2.03 Shop drawings shall be reviewed and approved by the structural engineer (RA) prior to fabrication.
- 2.04 Sizes, locations, loads, and anchorage of equipment shall be verified in the field with equipment manufacturers (suppliers) prior to febrication or installation of supporting structures.
- 2.05 Temporary bracing designed by a professional bracing/structural engineer shall be provided wherever necessary to take care of all loads to which the structure may be subjected, including wind. Such bracing shall be left in place as long as may be required for safety or until all the structure elements are complete, as determined by the bracing engineer.
- 2.06 During and after construction the contractor and/or owner shall keep loads on the structure within the limits of the design load until turned over to the county or final owner, then the load restrictions shall be enforced by that entity. Maximum loading is HS 25 as determined by AASHTO.
- 2.07 Contractor shall be responsible for safety and protection within and adjacent to the job site.
- 2.08 Civil engineer is responsible for erosion and sediment control design.
- 2.09 Road pavement design and its appurtenance structure are civil engineer's responsibility. Refer to the civil drawings for all pavement and roadway drainage system information and questions thereby to the civil engineer.

3: CONCRETE

3.01 All foundation concrete (footings, walls etc.) shall be normal weight concrete with a compressive strength equal to at least 4,000 psi within 28 days after casting. The water/cement ratio shall be no greater than 0.50 and slump shall be 2-4 inches.

3.02 All concrete work shall be placed, cured, stripped and protected as directed by these specifications and ACI standards and practices.

3.03 Contractor is responsible for all shoring and formwork.

3.04 Concrete design and detailing shall conform to the requirements of ACI 318-08. Contractor shall submit mix designs to the oversite review engineer accompanied by appropriate graphs and background data for approval. Mix design shall indicate 7 and 28-day strengths, cement content, air content, water-cement ratio, amount of fine and coarse aggregates and admixtures.

a. Minimum ultimate compressive strength of concrete at 28 days shall be as follows, unless noted

Footings and Walls concrete: 4000 PSI Unreinforced concrete: 2000 PSI

3.05 All exterior concrete and concrete exposed to weather shall be air-entrained. (All concrete used on this

3.06 Use of additives shall not be permitted unless specifically approved by the oversite review engineer.

3.07 The concrete subcontractor shall not reproduce any portion of the structural contract drawings for utilization as shop drawings.

3.08 Concrete shall be consolidated by means of mechanical vibration. Vibrators shall be inserted and removed vertically at regular intervals not to exceed 18" to ensure uniform consolidation. In no case shall vibrators be used to transport the concrete inside the forms.

3.09 Formwork shall follow ACI 347 "Recommended practice for concrete form work". Forms shall conform to the working drawing to shape, line and dimensions members and shall be substantially free from surface defects and sufficiently tight to prevent leakage. They shall be properly braced and tied to maintain position and shape.

3.10 Fresh concrete will be protected from rain, flowing water and mechanical injury, sun, drying winds and freezing for a period of 7 days. The temperature of the concrete must be kept above 50 degrees F for at least 7

3.11 Ground water and surface water within the subgrade excavation area must be maintained below the bottoms of the footer elevation and the bottoms of the excavation during preparation of the subgrade.

4: GEOTECHNICAL NOTES

- 4.01 Geotechnical site information provided by Hillis Carnes Engineering Associates, Inc.
- 4.02 All structural fill soils will have a minimum dry density of 105PCF unless indicated otherwise on this drawing set. Fill shall be compacted to at least 95% of the maximum dry density as determined by the standard proctor ASTM D698 (AASHTO T-99) with the exception of the top foot, which will be 100% of the maximum dry
- 4.03 All structural fill material will be placed in layers, which, before compaction, will not exceed eight inches. Each layer shall be spread to ensure conformity of materials in each layer.
- 4.04 Virgin/undisturbed soils are defined as soils with a minimum SPT "N" value of 12.

5: FOOTINGS

5.01 All footings are based on an allowable soil bearing pressure of 4,000 PSF. Any soil condition encountered during excavation that is contrary to those used for design of footings as outlined in these drawings shall be brought to the attention of the site Geotechnical Engineer for direction before proceeding.

5.02 Bottom of footings shall be a minimum of 30" below finished grade, unless a lower elevation is noted. Footing elevations noted are estimated based on available geotechnical and grading information. All footings adjacent to existing footings shall be lowered to match existing footing elevation.

5.03 All foundation subgrades shall be inspected and approved under the supervision of the registered professional site Geotechnical Engineer or their representative prior to pouring concrete. Footings may be lowered to achieve the minimum footing subgrade bearing capacity of 4,000 PSF. Undercut footing sub-grade as neccesary to achieve 4,000psf bearing capacity and fill with unreinforced concrete.

6: REINFORCING STEEL

6.01 Reinforcing bars shall be deformed billet steel conforming to ASTM A615, grade 60. All welded wire fabric shall conform to ASTM A185. Bars shall be branded by the manufacture with bar size and grade of steel and certified mill reports shall be submitted to oversite review engineer for approval and record. Reinforcing steel shall be detailed in accordance with the ACI 315-99 "Manual of Standard Practice for Detailing Reinforced Concrete Structures", latest edition. Provide corner bars at junctions of concrete walls and wall footings and lap 48 x bar diameters.

6.02 With wall reinforcing as shown in typical details, size and spacing of corner bars to be same as horizontal wall reinforcing, unless shown otherwise. Where continuous bars are called for, they shall run continuously around corners and lapped as necessary min. 48 x bar diameters. Provide standard hooks at discontinuous ends. Tension and compression lap splices shall not be less than the splice lengths as given in ACI 318. Generally lap top bars at mid span and bottom bars at supports. Provide placing accessories in accordance with ACI recommendations.

6.03 Provide the following minimum concrete cover for reinforcement:

- a. Concrete cast against and permanently exposed to earth ...3"
- b. Concrete exposed to earth or weather
- No. 6 through No. 18 bars ... 2" No. 5 bar, W31 or D31 wire, and smaller ... 1 1/2"
- c. Concrete slabs, walls and joists not exposed to the earth or weather:
- No. 14 and No. 18 bars ... 1 1/2" No. 11 bar and smaller ...3/"
- d. Beams, columns: Primary reinforcement, ties, stirrups and spirals ... 1 1/2"

7: EARTHWORK SPECIFICATIONS

- 7.01 The contractor shall furnish all labor, material and equipment for the earthwork. The contractor shall perform all work and services except those set out and furnished by Long Span Bridge & Culvert, LLC. (LSBC)
- 7.02 This work shall consist of all clearing and grading, preparation of the land to be filled, filling of the land, spreading and compaction of the fill, and all subsidiary work necessary to complete the grading of the cut and fill areas to conform with the project lines, grades, slopes and specifications.
- 7.03 This work is to be accomplished under the observation of the oversite review engineer or their representative. Placement of the backfill material will not be permitted unless the Engineer or their representative is on site

7.04 Prior to bidding the work, the contractor shall examine, investigate and inspect the construction site as to the nature and location of the work and local conditions at the construction site including, without limitation, the character of the surface or subsurface conditions and obstacles to be encountered on and around the construction site; and shall make such additional investigation as they may deem necessary for the planning and proper execution of the work.

7.05 The professional Site Geotechnical Engineer or their representative in the field shall verify the subgrade soil condition, gravel, and the rock quality. All stone subgrade shall be compacted with a vibratory plate compactor in no more than 8" lifts and verified by the professional Site Geotechnical Engineer or their representative.

7.06 If conditions other than those indicated by the confirmatory subsurface boring program are encountered by the contractor, Long Span Bridge & Culvert, LLC (LSBC) should be notified immediately. The material, which the contractor believes to be a changed condition, should not be disturbed so that LSBC and/or their designated representative can investigate the condition.

7.07 The work for clearing and grubbing includes furnishing all labor, materials, transportation, supervision, tools and construction machinery, which may be necessary to accomplish the clearing and grubbing for this project area.

7.09 All stumps, vegetation, brush, debris or deleterious materials shall be removed from the limits of the fill or other

7.08 All trees, bushes, etc., shall be removed from the limits of the proposed areas to receive fill or other engineered structures. The areas may be extended outside the actual lines of construction only to the distance required to provide the contractor with sufficient space to perform the work.

7.10 The work for stripping includes furnishing all labor, materials, transportation, supervision, tools and construction machinery, which may be necessary to be provided by the contractor.

7.11 When the construction/operation sequence requires, the area of fill or other engineered structures shall be properly stripped. This stripping shall include topsoil and other deleterious materials. Topsoil shall be removed to its full cepth and stockpiled for use in the final cover. Any rubbish, organic and objectionable soils and other deleterious material shall be properly disposed of at a site approved by owner or LSBC.

7.12 The lines and grades shall be established by using control benchmarks provided by licensed surveyors.

7.13 Soft or spongy cohesive or silty materials encountered at the base of the excavation shall be removed at the direction of the Site Geotechnical Engineer or their representative. The excavation for the footing wall foundations shall be observed and subgrade-bearing capacity certified by the Site Geotechnical Engineer upon completion of this task. At the direction of the Site Geotechnical Engineer or their representative, soft material will be removed to a depth directed by the Site Geotechnical Engineer or their representative, and replaced with granular backfill compacted at least 100% of the maximum dry unit weight density at a moisture content within 2% of optimum as determined by AASHTO T-99 method / ASTM D698.

7.14 If a shape control technician is supplied by LSBC, no select granular backfill may be placed, without being observed by LSBC's shape control technician.

7.15 Ground water and surface water within the subgrade excavation area must be maintained at least 3 feet below the footer elevation during preparation of the subgrade if additional excavation is required to remove unsuitable materials, the water must be maintained 3 feet below the deepest excavation elevation.

7.16 The subgrade shall be compacted with a soil vibratory compactor or equivalent with a dynamic force of 50,000 pounds (min.). The top 1 foot of the subgrade soil shall be compacted to at least 100% of the maximum dry unit weight at a moisture content within 2% of optimum as determined by AASHTO T-99 method (standard proctor). All compaction and subgrade bearing capacity to be verified by the site geotechnical engineer or representative.

7.17 All select granular backfill material around the culvert and above the footing shall consist of AASHTO M 145 A-1-a. Recycled concrete material shall not be allowed. The select backfill material shall have fines (pass no. 200 sieve material) maximum 15% by weight. See typical select backfill chart this sheet.

7.18 The select granular backfill material and site soil backfill for the adjoining embankment material shall be tested in the laboratory for grain size distribution (AASHTO T-27 for granular material; AASHTO T-88 for soil material) and moisture-density relationship (AASHTO T-99). The testing described above is for purposes of verification of site soil backfill parameters and is in addition to the general project specifications for the embankment backfill, but does not supersede project specifications that may be more stringent.

7.19 All backfill operations shall place the material evenly on both sides of the plate arch and each lift shall extend for the entire length of the plate arch prior to placement of the next sequential lift. Fill placement shall begin in the middle of the plate arch length and extend equally on both sides in the upstream and downstream directions.

7.20 The select granular backfill shall be placed in horizontal layers not to exceed 8" loose depth. The lift thickness may be reduced by the Site Geotechnical Engineer or their representative to obtain the required compaction, fill all the voids. achieve the proper seating of the backfill material and achieve the stability of the backfill material and the plate arch. The granular backfill shall be compacted to 95% of the maximum dry unit weight as determined by the standard proctor test (AASHTO T-99). Greater emphasis shall be given to a uniform degree of compaction throughout each lift than to achieving a degree of compaction greater than the minimum specified criteria. Site Geotechnical Engineer shall do testing of select granular backfill.

7.21 All granular material shall be compacted using mechanical devices, vibrating plates or other equipment approved by the Site Geotechnical Engineer. Compaction equipment weighing more than 24,000 pounds shall not be used within 2.5' of the corrugated metal structure. The compaction equipment shall be capable of compacting the material under the haunch of the plate arch (I.E.; below the spring line of the plate arch).

7.22 The soil backfill (compacted normal backfill) within 32'-0" or to natural undisturbed embankment backfill on each side shall be placed in layers not to exceed 8" loose depth. The lift thickness may be reduced by the Site Geotechnical Engineer to obtain the required compaction. The soil backfill shall be compacted to a minimum of 95% of the maximum dry unit weight as determined by the standard proctor test (AASHTO T-99) and to a moisture content within 2% of the optimum moisture content as determined by the same test. Field nuclear density test shall be performed at a minimum frequency of four tests per every other lift and every 25' on the soil backfill on each side of the structure. The testing described above is in addition to the general project specifications for the embankment backfill, but does not supersede project specifications that may be more stringent than those requirements. The Site Geotechnical Engineer is responsible for testing and recording measurements of the soil backfill.

7.23 If at any time longitudinal cracks develop in the backfill surrounding the pipe to a distance of 30' from the spring line of the plate arch, these features must be brought to the immediate attention of the field QA/QC personnel and the Site Geotechnical Engineer.

7.24 While compacting granular backfill material with a vibrator compactor and adjacent to the plate arch, the opposite side of the plate arch should be observed to note if vibrations are loosening the granular material on that side. This may be more prevalent at higher elevations of the backfill with respect to the plate arch. If this condition occurs, the field QA/QC technician and Site Geotechnical Engineer should be notified prior to placement of a sequential lift on either side.

7.25 The structure should not be crossed with equipment heavier than a D4 dozer. No other equipment or highway (HS25) loading shall be allowed to cross the structure until the asphalt pavement is placed unless there is a minimum of 12" of soil cover or span/8 inches of soil cover whichever is greater, covering the plate arch. Top filling should begin at the middle of the structure (lengthwise) with the backfill being pushed up and over the structure with a D4 or preferably smaller type dozer. The fill should be pushed over the structure in a manner 45 to 90 degrees to the axis of the structure. Field nuclear density test shall be performed at a minimum frequency of four tests per every lift on the soil backs on each side of the structure. The testing described above is in addition to the general project specifications for the embankment backfill, but does not supersede project specifications that may be more stringent than those requirements. The contractor shall submit to the owner samples of all proposed soil backfill material for laboratory testing to verify moisture and density relationships (AASHTO T-99/ASTM D698) and grain size relationships (AASHTO T-27/ASTM

7.26 All construction to be certified at the end of the job by a Professional Structural/Geotechnical Engineer (oversite review engineer) qualified in the design and construction of plate arch culverts (minimum 10 years experience) that all work performed by contractor meets these design requirements and specifications. Certification to be submitted to LSBC, RA and the local jurisdiction for record file.

8: REQUIRED SUBMITTALS

- 8.01 The contractor must submit the following items to the oversite review engineer for approval in writing at least 2 weeks prior to use:
 - a.Manufacturer certification for yield strength of reinforcing steel.
 - b. Manufacturer certification for concrete design. c. Shop drawings of all concrete work.
 - d.Plate arch shop drawings.

9: DEWATERING REQUIREMENTS

9.01 Dewater footing excavations using sump pumps or well points as required. Footing excavation must be dewatered and maintained that way for a minimum of seven days or concrete strength of 3,000psi has been reached, whichever is greater

10: CONSTRUCTION OVERSIGHT CERTIFICATIONS

9.01 The plate arch construction requires engineering oversight and inspection. The oversite review, Civil, Structural and Geotechnical Engineers must provide LSBC certification reports of all footings and retaining wall/headwalls reinforcing placement and the following items:

- a. Subgrade bearing capacity and backfill (select granular and compacted normal backfill) compaction testing, field reports, testing results, testing locations, and registered professional engineer's certification.
- b. Field reports of concrete placement review, laboratory test results of concrete cylinder breaks at 7 and 28 days and certified by a Registered Professional Structural Engineer.
- c.Final report of construction certification that the construction was performed in accordance with the design and the material testing and inspection verifying the same, stamped by a Registered Professional Structural/Geotechnical Engineer.

11: ENVIRONMENTAL PERMITTING

10.01 These plans do not address environmental permitting requirements, which must be addressed and applied for with the state and Army Corp of Engineers, as required.

12: SAFETY

11.01 All contractors (and owners), their representatives and their crew must be qualified/certified to perform all works within their scope. They must adhere to OSHA's health and safety laws. The General Contractor is solely responsible for all site safety.

13: RA'S RESPONSIBILITY

12.01 RA's scope of work for this project are design of plate arch (minimum steel thickness stress analysis), footings, headwalls and wingwalls and RA is responsible for that only. Acceptance of the plan drawings by our client & the owner means they agree to our scope and responsibilities.

14: LONG SPAN BRIDGE & CULVERT, LLC SCOPE OF WORK

13.01 Long Span Bridge & Culvert, LLC (LSBC) will deliver, furnish and assemble the Long Span low profile arch on footings designed by Ryan & Associates and prepared by Site Contractor. The base channel will be furnished by LSBC

13.03 LSBC will provide a shape control technician to monitor structure's shape and observe the proper placement and compaction of the select fill material, unless provided otherwise and approved in writing by LSSC.

13.04 LSBC will require the Site Contractor to unload the structure plates and base channel. LSBC will require the Site Contractor to provide access to the structure for a rubber-tired crane. Parallel access roads shall be within 30' of the centerline of the structure on each side.

BACKFILL CHART

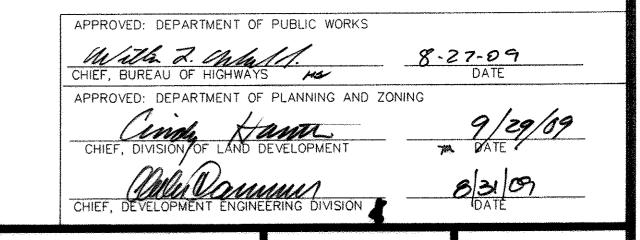
AASHT	OM 145- T	ABLE 2 (M	IODIFIED*)		
GROUP CLASSIFICATION	-	\-1	A-2 (MODIFIED)		
SIEVE ANALYSIS, PERCENT PASSING	A-1-a	A-1-b	A-2-4	A-2-5	
NO. 10 (2.00 mm)	50 max		nd ye by be	***	
NO. 40 (.425 mm)	30 max	50 max	**-*		
NO. 100 (.150 mm)			50 max	50 max	
NO. 200 (.075 mm)	15 max	25 max	20 max	20 max	
Characteristics of fraction passir	ng No. 40 (0.425 m	m)	<u></u>		
Liquid Limit			40 max	41 max	
Plasticity Index	6 max	6 max	10 max	10 max	
Usual Material Types	Stone Fragr	ments	Silty or Cla	зуеу	
	Gravel and	Sand	Gravel and	d Sand	

*Modified to be more select than M-145

OWNERS WAVERLY WOODS DEVELOPMENT CORPORATION. HOLE IN THE DOUGHNUT, LLC, & GTW JOINT VENTURE C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 ELLICOTT CITY, MARYLAND 21042

(443 - 367 - 0422)

DEVELOPER WAVERLY WOODS DEVELOPMENT CORP. C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 ELLICOTT CITY, MARYLAND 21042 (443 - 367 - 0422)



REVISIONS DATE BY DESCRIPTION
07/27/09 AJH DESIGN MODIFICATIONS BASED ON REVISED TOPO AND MDE PERMIT REVISIONS FROM FISHER, COLLINS & CARTER

LONG SPAN BRIDGE & CULVERT, LLC DRAWN BY: CJH DESIGN BY: CJH LAND DESIGN & DEVELOPMENT, INC. HECKED BY: WKR JOB No: 1101-08-04 DATE: 07/27/09 DO NOT SCALE THIS DRAWING: DIMENSIONS AND MOTES HOW PRECEDENCE OVER DRAWING

engineered structures.

Ryan & Associates
A Division of WKR Consulting Inc.

Hagerstown, MD Office

1825 Howell Road, Suite 3

Hagerstown, MD 21740

301-671-3200 (ph)

301-360-9574 (fx)

Frederick, MD Office 2412 Wynfield Ct. Frederick, MD 21702 301-360-9534 (ph) 301-360-9574 (fx)

PLATE ARCH CULVERT STRUCTURAL DESIGN **SPECIFICATIONS**

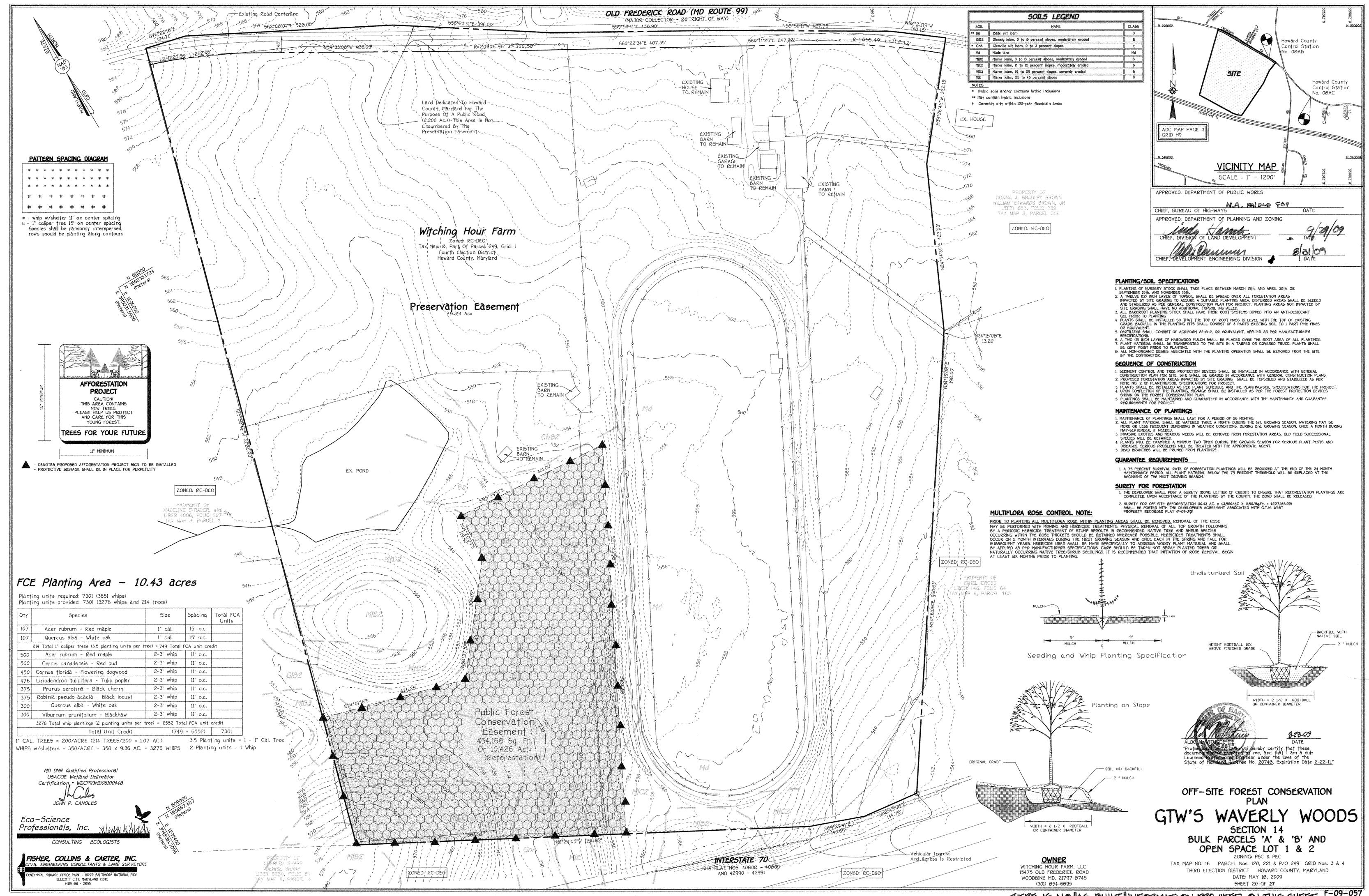
GTW's WAVERLY WOODS, SECTION 14 BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2 ZONING: PSC & PEC TAX MAP NO. 16 PARCEL Nos. 120,221 & P/O 249 GRID Nos. 3 & 4 THIRD ELECTION DISTRICT, HOWARD COUNTY, MD



PROFESSIONAL CERTIFICATION locuments were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. Willam K. Ryan, P.E License No: 21586

I hereby certify that these SHEET 19 of 27

and installed in the concrete foundations by the Site Contractor in accordance with the plans. Structural plate for this job to be 3ga (.249" thick) steel. 13.02 LSBC will conduct a pre-construction meeting prior to foundation preparation and arch assembly. Attendance at the pre-construction meeting is mandatory for the owner or the owner's representative (e.g. Site Civil Engineer, Site Contractor and Concrete Contractor) and the oversite review Structural/Geotechnical Engineer. It is the owner's responsibility to have each party in attendance. If a party is not in attendance it is the owner's responsibility to inform that entity of its responsibilities and duties prior to the start of work.



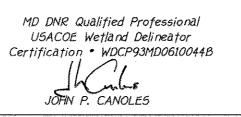
	Master Overall Forest Conservation — GTW's Waverly Woods							
SECTION/AREA DESIGNATION LEGEND	HOWARD COUNTY DPZ FILE No.	FOREST RECORDING REFERENCE	AREA OF FOREST RETENTION REQUIRED	AREA OF CREDITED FOREST RETENTION PROVIDED	AREA OF FOREST PLANTING REQUIRED	AREA OF CREDITED PLANTING PROVIDED	AREA OF SURPLUS FOREST RETENTION	AREA OF SURPLUS FOREST PLANTING
SECTION 4/AREA 1 SECTION 4/AREA 2	F-95-173 F-95-174	PLAT No. 12249 PLAT No. 12250	4.09 AC.±	O.S. LOT 19 = 3.10 AC.* O.S. LOT 10 = 0.98 AC.* OFF-SITE = 0.18 AC.* TOTAL = 4.26 AC.*	8.11 AC.±	WEST FRIENDSHIP ESTATES PRES. PARCEL 'B' TOTAL = 8.13 AC.±	+0.17 AC.±	+0.02 AC.±
SECTION 5	F-96-179	PLAT No. 12717 PLAT No. 12718	2.58 AC.*	O.S. LOT 55 = 1.97 AC.± O.S. LOT 6 = 0.62 AC.± TOTAL = 2.59 AC.±	2.55 AC.±	O.S. LOT 55 = 1.77 AC.* O.S. LOT 6 = 0.70 AC.* TOTAL = 2.55 AC.*	+0.01 AC.*	+0.00 AC.±
SECTION 6	F-98-88 F-98-88 F-98-88 F-98-88 F-98-88 F-98-88	PLAT No. 13513 PLAT No. 13514 PLAT No. 13515 PLAT No. 13516 PLAT No. 13516 PLAT F-90-00-J-2 (Part No. 1) PLAT F-90-00-J-2 (Part No. 2) PLAT F-90-00-J-2	2.01 AC.±	0.42 AC.± 1.83 AC.± 0.00 AC. 0.00 AC. 0.34 AC.± 0.00 AC. 0.00 AC.	4.77 AC.±	0.00 AC.± 0.00 AC.± 0.26 AC.± 0.02 AC.± 0.67 AC.± 0.28 AC.± 0.81 AC.± 0.62 AC.±	+0.58 AC.±	+0.46 AC.*
	F-98-88 F-98-88 F-98-88	(Part No. 3) PLAT F-90-08-J-2 (Part No. 4) PLAT F-98-88-J-1 (Part No. 1) PLAT F-98-88-J-1 (Part No. 2)		0.00 AC. 0.00 AC. TOTAL = 2.59 AC.±		0.02 AC.* 1.46 AC.* 0.27 AC.* (0.5, LOT 19, 54, A1) 0.84 AC.* (0.5, LOT 19, 54, A1) TOTAL = 5.23 AC.*		:
SECTION 7	F-97-180 F-00-133	PLAT No. 13439 & 13440 PLAT No. 14271	13.14 AC.	0.5. LOT 1 & PARCEL B = 7.019 Ac O. S. LOT 1 = 8.944 Ac	0.00 AC.	0.00 AC.±	2.823 AC.	0.00 AC.±
SECTION 10	F-00-06 F-00-06	PLAT No. 14140 PLAT No. 14119	0.00 AC.	0.00 AC. 0.00 AC. TOTAL = 0.00 AC.	0.62 AC.	0.5. LOT 34 = 0.71 AC.± 0.5. LOT 34 (SECT. 6) = 0.84 AC.± TOTAL = 1.55 AC.±	+0.00 AC.	+0.93 AC±
SECTION 11, AREA 1	F-01-91	PLAT No. 15022	1.30 AC.	WEST SIDE MARRIOTTSVILLE ROAD 1.30 AC. (SEE SHEET 10 OF 11 ROAD PLANS)	0.00 AC.	0.00 AC.	+0.00 AC.	0.00 AC.
SECTION 11, AREA 2	F-01-148	PLAT No. 15199	0.18 AC.	0.18 AC	0.00 AC.	0.00 AC.	+0.00 AC.	+0.00 AC.
SECTION 11, AREA 3	F-01-147	PLAT No. 15228 PLAT No. 15223	1.13 AC.	WEST SIDE MARRIOTTSVILLE ROAD 0.72 AC. (SEE ROAD SHEET 8) 0.5. LOT 11 = 0.41 Ac	0.00 AC.	0.00 AC.	+0.00 AC.	+0.00 AC.
SECTION 11, AREA 4	F-01-93	PLAT No. 15069 PLAT No. 15060 THRU 15063	14.59 AC.	TOTAL = 1.13 Ac WEST SIDE MARRIOTTSVILLE ROAD 10.52 AC. (SEE ROAD SHEET 12) 0.5. LOT 22 = 74, 75= 4.07 Ac TOTAL = 14.59 Ac	0.00 AC.	0.00 AC.	+0.00 AC.	+0.00 AC.
SECTION 12	F-01-31 F-01-31 F-01-31	PLAT No. 14792 PLAT No. 14790 PLAT No. 14791	0.00 AC.	0.00 AC. TOTAL = 0.00 AC.	1.86 AC.	0.5. LOT 0 = 0.40 AC.± 0.5. LOT 40 = 0.31 AC.± 0.5. LOT 97 = 0.26 AC.± TOTAL = 0.97 AC.±	+0.00 AC.	(-)0.89 AC.
SECTION 13	F-04-58 F-04-58	PLAT NO. 16941-16962 PLAT NO. 16963	23.99 AC. 0.00 AC.	23.99 AC.± 0.00 AC	30.90 AC. 0.00 AC.	O.S. LOT 3 = 4.13 AC* HOWARD HUNT PROPERTIES 41.00 AC* TOTAL = 45.13 AC*	+0.00 AC.	+14.23 AC.
AMENDED PLATS SECTION 4, AREA 1; SECTION 5; SECTION 7; SECTION 11, AREA 2; AND SECTION 11, AREA 3	F-04-105	PLAT NO. 17240-17264	0.00 AC.	11.283 AC.±	0.00 AC.	7.766 Ac.	11.283 AC.	7.766 Ac.
GAITHER HUNT SECTION 1 AREA 1 NON-BUILDABLE PRESERVATION PARCEL 'B'	F-05-100	PLAT NO. 17243-17247	0.00 AC.	0.00 AC.±	0.00 AC.	6.14 Ac.	0.00 AC.	6.14 Ac.
AMENDED PLATS GTW'S WAVERLY WOODS SECTION 13, OPEN SPACE LOT 5	F-06-175	PLAT NO. 18625-18627	0.00 AC.	0.36 AC.±	0.00 AC.	0.00 Ac.	0.36 AC.	0.00 Ac.
AMENDED PLATS HOWARD COUNTY CONSERVANCY, INC	F-07-33	PLAT NO. 18611-18613	0.00 AC.	0.00 AC.±	0.00 AC.	16.22 Ac.	0.00 AC.	16.22 Ac.
AMENDED PLAT GTW'S WAVERLY WOODS 5ECTION 13, OPEN SPACE LOTS 2 AND 7	F-08-	PLAT NO.	0.00 AC.	0.00 AC.± (SEE NOTE 2)	0.00 AC.	0.00 AC.± (SEE NOTE 2)	0.00 AC.	0.00 Ac.
REVISION PLAT OPEN SPACE LOT 19 SECTION 4, AREA 1	F-08-202	PLAT NO.	0.00 AC.	0.00 AC.± (SEE NOTE 1)	0.00 AC.	0.00 AC.± (SEE NOTE 1)	0.00 AC.	0.00 Ac.
PROPERTY OF GTW JOINT VENTURE	F-08-159	PLAT NO.	0.00 AC.	0.00 AC.± (SEE NOTE 3)	0.00 AC.	0.00 AC.* (SEE NOTE 3)	0.00 AC.	0.00 Ac.
SECTION 14	F-09-057	PLAT NO.	17.46 AC.±	FCE NOS. 1 THRU 7 = 19.00 AC. ABANDONED FCE AREA = 1.62 AC.	15.11 AC.*	FCE NOS. 1 THRU 7 = 4.60 AC. OFF-51TE WITCHING HOUR FARM PROPERTY= 10.43 AC.	0.00 AC.	0.00 Ac.
TOTAL5			80.47 AC.±	95.70 AC.±	63.92 AC.±	108.80 AC.	15.23 AC.±	44.88 AC.

A Secretary
9.2209
•
FISHER, COLLINS & CARTER, INC.

CENTENNIAL SQUARE OFFICE PARK - 10272 BALTIMORE NATIONAL PIKE

ELLICOTT CITY, MARYLAND 21042 (410) 461 - 2055





OWNERS

WAVERLY WOODS DEVELOPMENT CORPORATION,
HOLE IN THE DOUGHNUT, LLC, &
GTW JOINT VENTURE
C/O LAND DESIGN AND DEVELOPMENT, INC.
5300 DORSEY HALL DRIVE, SUITE 102
ELLICOTT CITY, MARYLAND 21042
(443-367-0422)

DEVELOPER

WAVERLY WOODS DEVELOPMENT CORP.
C/O LAND DESIGN AND DEVELOPMENT,
INC. 5300 DORSEY HALL DRIVE, SUITE 102
ELLICOTT CITY, MARYLAND 21042
(443-367-0422)

Commercial And Golf Use FOREST CONSERVATION WORKSHEET VERSION 1.0

AEKOIOIA 1.0		VERSION 1.0			
ASIC SITE DATA: ROSS SITE AREA		BASIC SITE DATA: GROSS SITE AREA			
T TRACT AREA (NTA):	487.2	NET TRACT AREA (NTA):			
FORMATION FOR CALCULATIONS: ET TRACT AREA (NTA):	487.2 97.4 73.1	INFORMATION FOR CALCULATIONS: NET TRACT AREA (NTA):	170.3 25.5 25.5		
USTING FOREST ON NTA:= USTING FOREST ABOVE CONSERVATION THRESHOLD= ZEAK EVEN POINT: (IF APPLICABLE) (96.2x0.2) + 97.4	193.6 96.2 116.6	EXISTING FOREST ON NTA:* EXISTING FOREST ABOVE CONSERVATION THRESHOLD* BREAK EVEN POINT: (IF APPLICABLE) (05.4X0.2) + 25.5	110.9 85.4 42.6		
REST TO BE CLEARED: PREST TO BE RETAINED	135.3 50.3	FOREST TO BE CLEARED: FOREST TO BE RETAINED	73.5 37.4		
EFORESTATION CALCULATIONS		REFORESTATION CALCULATIONS			
LEARING ABOVE THRESHOLD LEARED ABOVE THRESHOLD X1/4: ACRES DREST RETAINED ABOVE THRESHOLD: ACREA EFORESTATION REQUIRED (a-b)		CLEARING ABOVE THRESHOLD CLEARED ABOVE THRESHOLD 73.5 X1/4: 18.4 ACRES FOREST RETAINED ABOVE THRESHOLD: 11.9 ACREA REFORESTATION REQUIRED (a-b)	6.5		
LEARING BELOW THRESHOLD FOREST CLEARED ABOVE THRESHOLD - 96.2 x 1/4: 24.1 ACRES FOREST CLEARED BELOW THRESHOLD - 39.1 x 2: 78.2 ACRES		CLEARING BELOW THRESHOLD a. FOREST CLEARED ABOVE THRESHOLD b. FOREST CLEARED BELOW THRESHOLD			
FORESTATION REQUIRED (a-b)	102.3	REFORESTATION REQUIRED (a-b)			

NOTE NO. 1: THIS REVISION PLAT DOES NOT CREATE ANY NEW FOREST CONSERVATION EASEMENT.

The Purpose Of This Plat Is To Identify 'Part One' 0.260 Acres And 'Part Two', 0.037 Acres To Establish New Forest Conservation Easements Within Open Space Lot 19, GTW's Waverly Woods, Section 4, Area 1 - Plat No. 12246 Thru 12251, As Shown On Final Road Plans, GTW'S Waverly Woods, Section 6 (F-98-00) To Be Associated With A New Forest Conservation Developer's Agreement As A Replacement For The Abandonment Of 1.105 Acres Of FCE Reforestation As Shown On F-90-000, Plat No. 13512 Thru 13517.

NOTE NO. 2: THIS REVISION PLAT DOES NOT CREATE ANY NEW FOREST CONSERVATION EASEMENT. The Purpose Of This Plat Is To Abandon 0.125 Acres Forest Retention In Existing Forest Conservation Easement No. 9A And To Abandon 0.464 Acres Forest Retention In Existing Forest Conservation Easement No. 11 On GTW'S Waverly Woods, Section 13 Plat Nos. 16950 And 16952. The Total 0.59 Acres Forest Retention Is Relocated On GTW'S Waverly Woods Section 14 (F-07-159).

NOTE NO. 3: THIS REVISION PLAT DOES NOT CREATE ANY NEW FOREST CONSERVATION EASEMENT. The Purpose Of This Plat Is To:

The Purpose Of this Plat Is to:
(1) Create Forest Conservation Easement, Part One, Containing 0.28 Acres Reforestation Approved As Area 'G' Under

GTW's Waverly Woods, Section 6, F-98-88 Road Plans,

(2) Create Forest Conservation Easement, Part Two, Containing 0.61 Acres Reforestation Approved As Area 'E' Under GTW's Waverly Woods, Section 6, F-90-80 Road Plans,

(3) Create Forest Conservation Easement, Part Three, Containing 0.50 Acres Reforestation Approved As Area 'D' Under

GTW's Waverly Woods, Section 6, F-90-88 Road Plans,

(4) Create Forest Conservation Easement, Part Four, Containing 2.93 Acres Forest Retention To Replace Forest

Conservation Easement, Area 'G' Forest Planting Containing 1.46 Acres For GTW's Waverly Woods, Section 6, F-98-88,

(5) Create Forest Conservation Easement, Part Five, Containing 1.18 Acres Forest Retention To Replace 0.59 Acres
 Forest Planting Removed From GTW's Waverly Woods, Section 13, Plat Nos. 17217 Thru 17223, And
 (6) Create Three (3) Public Drainage And Utility Easements For Marriottsville Road Mitigation Plan, F-07-032.

MASTER OVERALL FOREST CONSERVATION CHART

GTW'S WAVERLY WOODS SECTION 14

BULK PARCELS 'A' & 'B' AND OPEN SPACE LOT 1 & 2

(A SUBDIVISION OF THE PROPERTY OF WAVERLY WOODS DEVELOPMENT

CORPORATION, LIBER 4879, FOLIO 307)

ZONING: P5C & PEC

TAX MAP NO. 16 PARCEL Nos. 120, 221 & P/O 249 GRID Nos. 3 & 4

THIRD ELECTION DISTRICT HOWARD COUNTY, MARYLAND

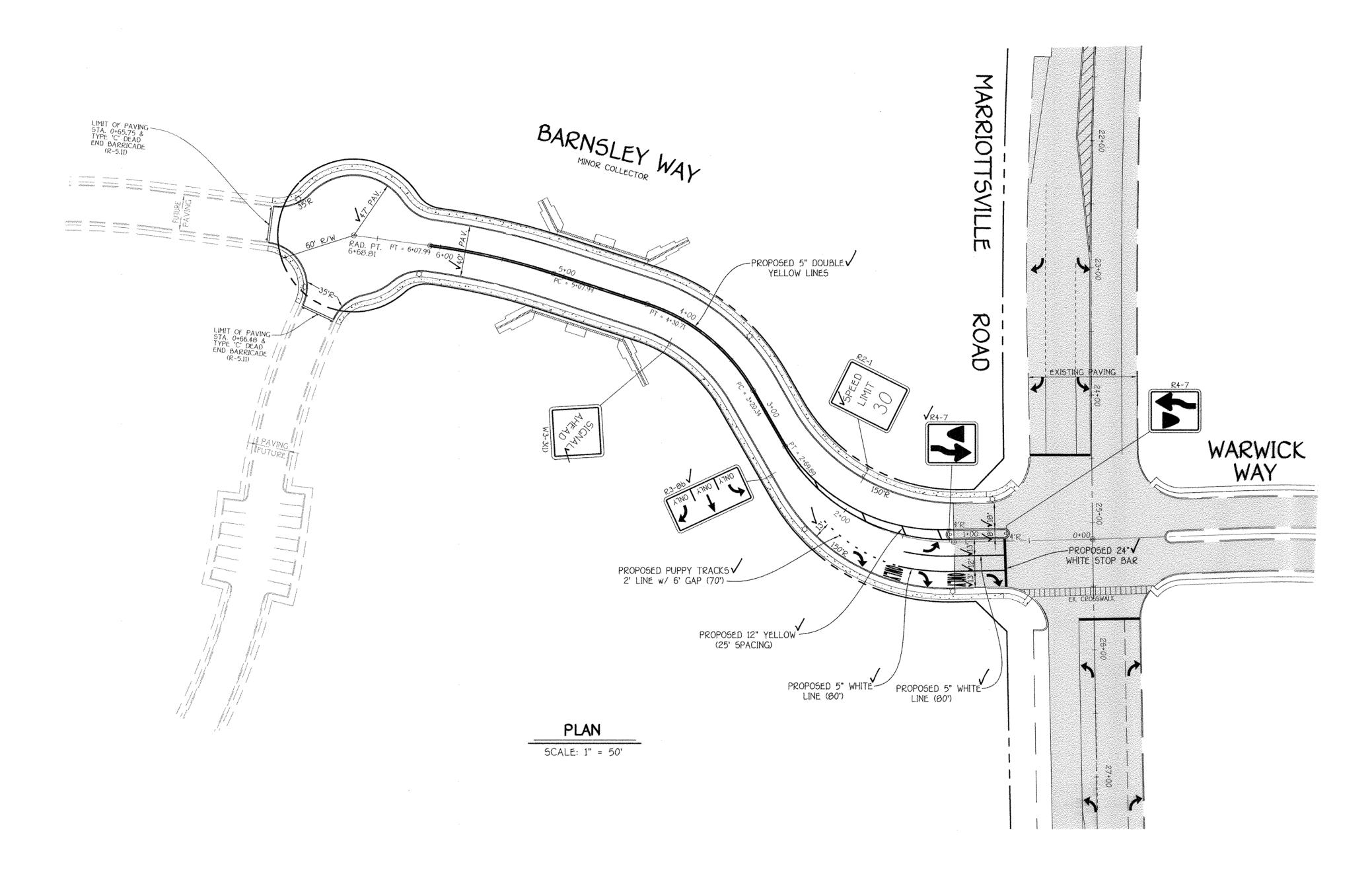
DATE: JULY 28, 2009

1. ALL LANE DESIGNATION TO BE THERMOPLASTIC LINE STRIPING.

2. ALL PAVEMENT MARKINGS SHALL BE 5" WIDE UNLESS NOTED OTHERWISE.

3. ANY RELOCATED SIGNS ARE TO BE INSTALLED ON NEW 2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE POSTS (14 GAUGE) INSERTED INTO A 2-1/2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE SLEEVES (12 GAUGE) 3' LONG WITH A GALVANIZED STEEL CAP ON THE TOP OF POST.

APPROVED: DEPARTMENT OF PUBLIC WORKS CHIEF, BUREAU OF HIGHWAYS MS 8-27-09 APPROVED: DEPARTMENT OF PLANNING AND ZONING



NOTE: CONTRACTOR SHALL CONTACT PARRIS ZIRKENBACH AT (410) 313-2430 HOWARD COUNTY TRAFFIC PRIOR TO STARTING ANY PAVEMENT MARKINGS.



BARNSLEY WAY - STRIPING PLAN

GTW'S WAVERLY WOODS SECTION 14

BULK PARCELS 'A' & 'B' AND OPEN SPACE LOT 1 & 2 (A SUBDIVISION OF THE PROPERTY OF WAVERLY WOODS DEVELOPMENT

ZONING: PSC & PEC

TAX MAP NO. 16 PARCEL Nos. 120, 221 & P/O 249 GRID Nos. 3 & 4

THIRD ELECTION DISTRICT HOWARD COUNTY, MARYLAND

DATE: JULY 20, 2009

SHEET 22 OF 27

CORPORATION, LIBER 4079, FOLIO 307)

FISHER, COLLINS & CARTER, INC.
CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS ELLICOTT CITY, MARYLAND 21042 (410) 461 - 2855

OWNERS WAVERLY WOODS DEVELOPMENT CORPORATION, HOLE IN THE DOUGHNUT, LLC, &
GTW JOINT VENTURE
C/O LAND DESIGN AND DEVELOPMENT, INC.
5300 DORSEY HALL DRIVE, SUITE 102
ELLICOTT CITY, MARYLAND 21042
(443-367-0422)

DEVELOPER WAVERLY WOODS DEVELOPMENT CORP. C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 ELLICOTT CITY, MARYLAND 21042 (443-367-0422)

TYPE K TRAFFIC BARRIER END TREATMENT

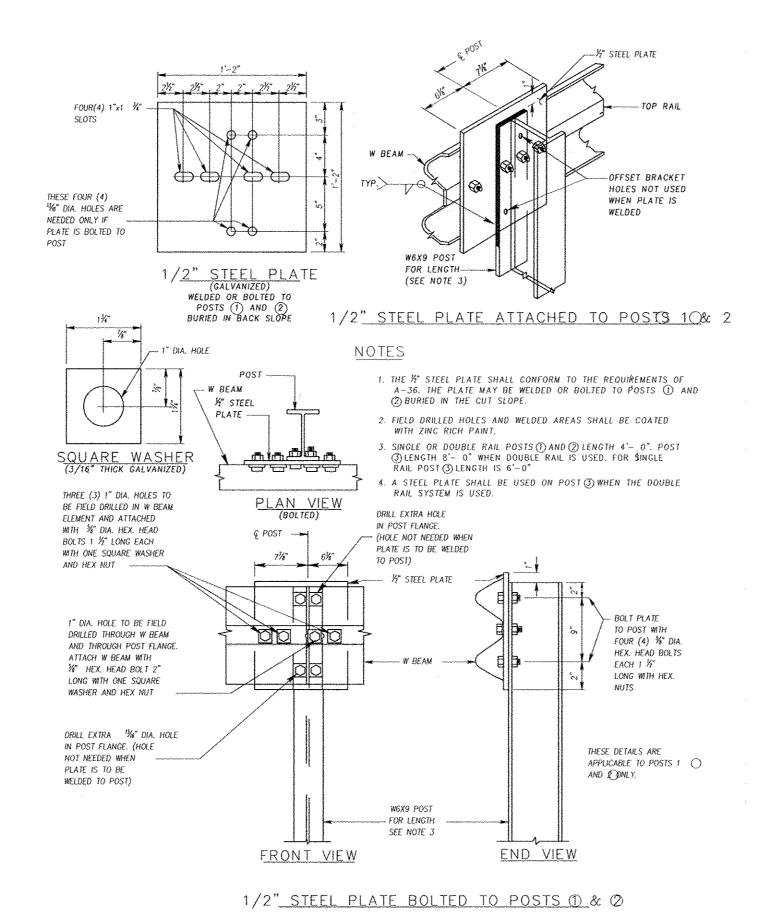
OPTION 2 & 3 ANCHORAGE STANDARD NO. MD 605.10-01

2. IF THE FOUNDATION TUBE AND SOIL PLATE (OPTION 2) ARE DRIVEN INTO THE SOIL, PROPER CARE SHOULD BE TAKEN TO ENSURE THAT THE SOIL PLATE FASTENERS ARE NOT BROKEN DURING THE DRIVING PROCESS.

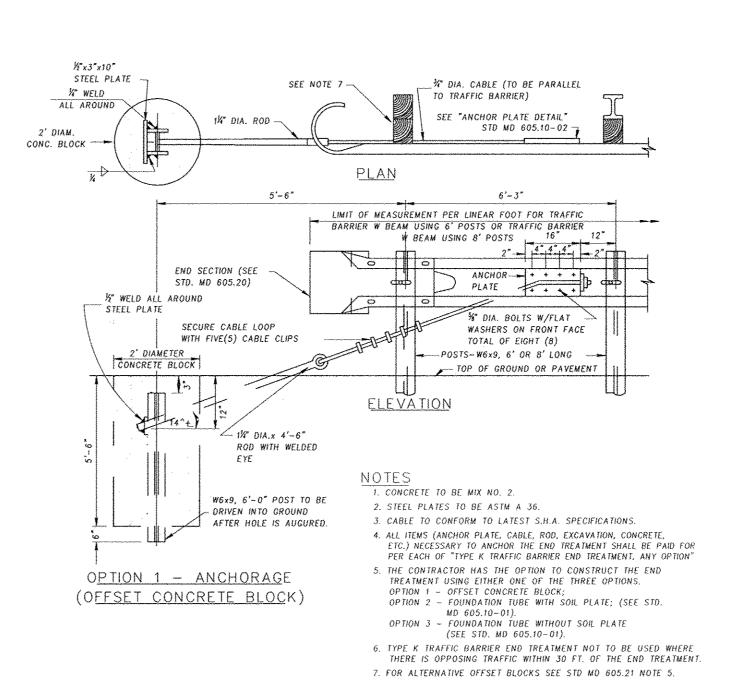
3. SECURE BEARING PLATE WITH 16 PENNY GALVANIZED NAIL TO PREVENT ROTATION OF PLATE.

1. NOTES ON STO. MD 605.10 APPLY TO OPTION 2 & 3.

4. SAME AS MD 605.01 NOTE 6

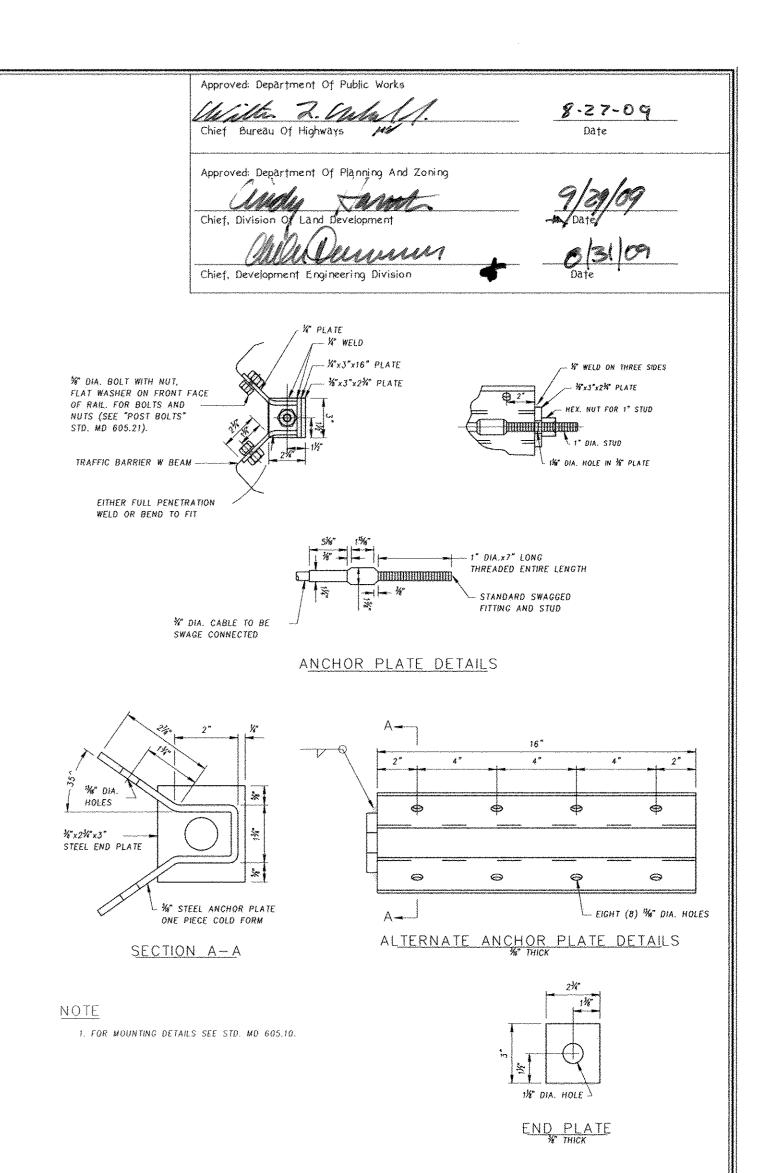


TRAFFIC BARRIER W BEAM WITH TYPE A END ANCHORAGE (OPTION 1)



TYPE K TRAFFIC BARRIER END TREATMENT OPTION 1 ANCHORAGE

STANDARD NO. MD 605.10



TYPE K TRAFFIC BARRIER END TREATMENT ANCHORAGE DETAILS

State of Maryland, License No. 20748, Expiration Date 2-22-11."

GUARDRAIL DETAILS GTW'S WAVERLY WOODS

SECTION 14
BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2

TAX MAP NO. 16 PARCEL Nos. 120, 221 & P/O 249 GRID Nos. 3 & 4 THIRD ELECTION DISTRICT HOWARD COUNTY, MARYLAND DATE: JULY 28, 2009

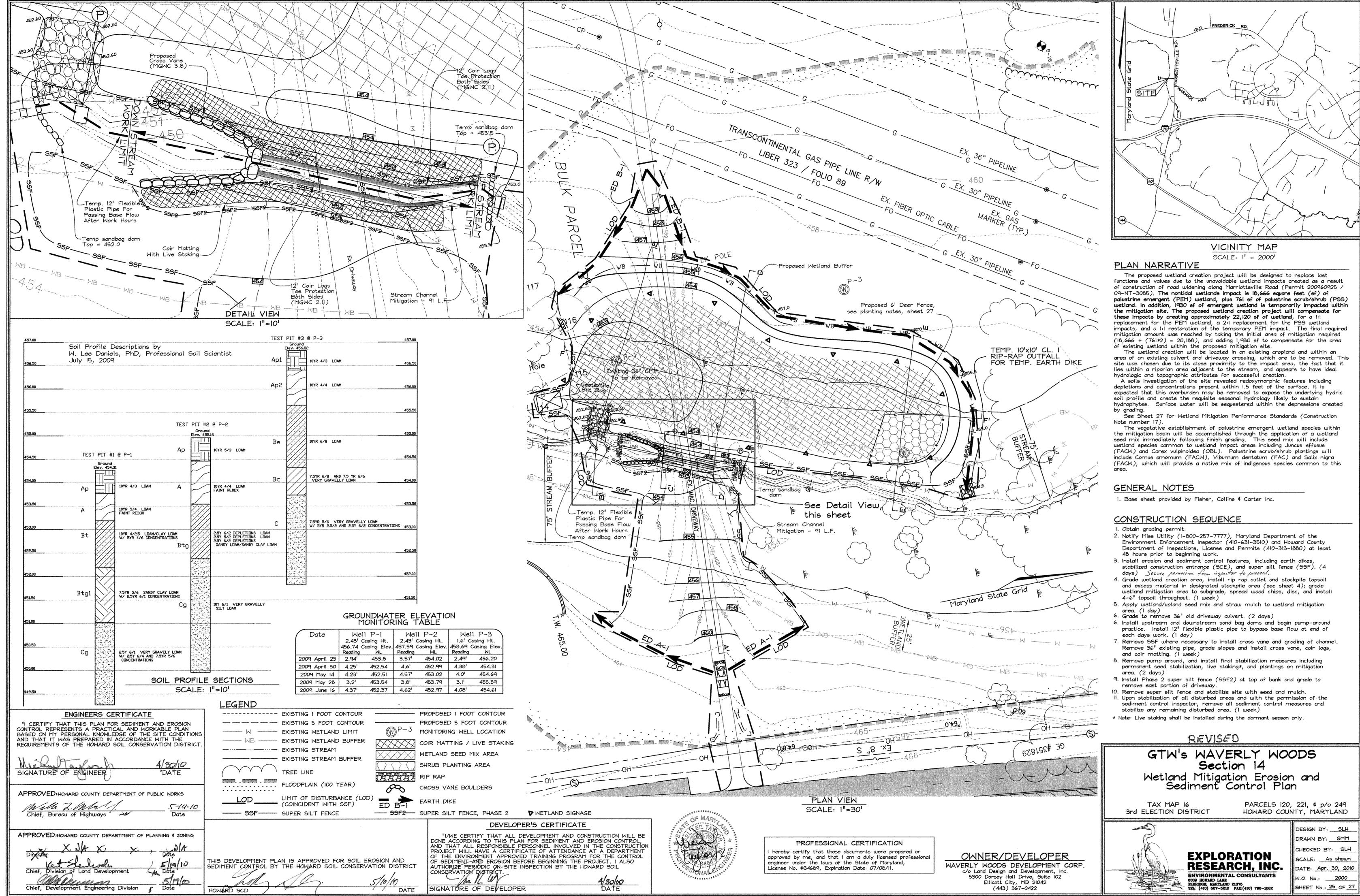
FISHER, COLLINS & CARTER, INC.
CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS ENTENNIAL SQUARE OFFICE PARK - 10272 BALTIMORE NATIONAL PIKE ELLICOTT CITY, MARYLAND 21042

OWNERS WAVERLY WOODS DEVELOPMENT CORPORATION, HOLE IN THE DOUGHNUT, LLC, & C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 ELLICOTT CITY, MARYLAND 21042 (443-367-0422)

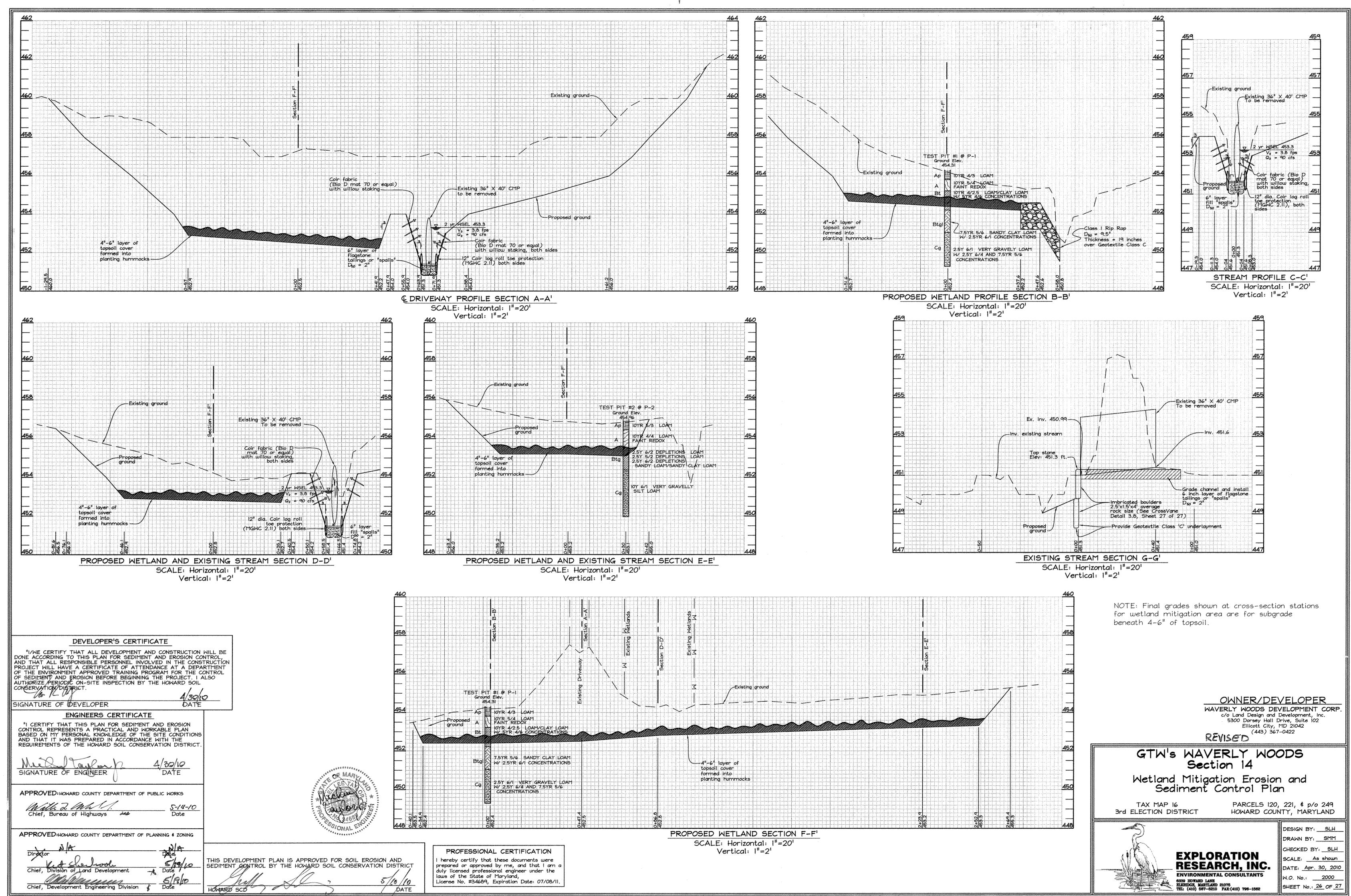
DEVELOPER C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 ELLICOTT CITY, MARYLAND 21042 (443-367-0422)

THERE IS NO "AS-BUILT" INFORMATION PROVIDED ON THIS SHEET F-09-057

HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waterly Woods West Grossing Borry No. 8-1	HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waverly Woods West Crossing Boring No. B-2	HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waverly Woods West Stream Crossing Boring No. 8-3	HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waverly Woods West Stream Crossing Boring No. B-4 Location Boring County Mandatod	#ILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waverly Woods West Crossing 80/60 30 30 Location 100 8 073800	Approved: Department Of Public Works Millin Z. Mill. 8.27-09 Chief Bureau Of Highways Market Date
SARPLER Coston: 140-95 Pt. Harmer Drop 30 in. Rock Core Chamelor Sarple Brown, moist, soft to creation silk. Topposit-67 processor services serv	Sample S	Determ	Delice Hercenter Country Manyland July Grand Country Manyland July Grand Country Manyland July Grand Gra	Datum Harmoner VII. 140 Sh. Hole Damenter E* Foreman 5. Smith Butt Elev. 480.88 Ft. Harmoner Drop 30 in. Ropx Core Districts Started 11/24/07 Pice Size 2 in Boring Method HSA District Completed 11/24/07 Elevation Started 11/24/07 Pice Size 2 in Boring Method HSA District Completed 11/24/07 Elevation Started 11/24/07 Pice Size 2 in Boring Method HSA District Completed 11/24/07 Elevation Started 11/24/07 Pice Size 2 in Boring Method HSA District Completed 11/24/07 Elevation Started 11/24/07 Pice Size 2 in Boring Method HSA District Completed 11/24/07 Elevation Indian Started Indi	Approved: Department Of Planning And Zoning Chief, Division Of Rand Development Chief, Development Engineering Division Chief, Development Engineering Division Date
AT PRESENT SPECIAL TUBES 1- MEAST AFTER WHITE. Day is 4.5 is ceal contributors resent ancems CA- COMPRESENT ANCEMS V- UPOSTUMED AFTER WES. is is us described cased RC-MICK COME. I - LOST. MCC-MICK WO	PT - PRESSED SHELBY TUSE 1. MTACT AFTER 24HRS. Dry 1t. 5.5 1t. CPA - CONTRACOUS FLIGHT ALDERS. GA - CONTRACOUS FLIGHT ALDER 11 - LINUS TURBED AFTER HRS. 1t. 1t. CC - DRYWING CASING RO - HOCK CORE 11 - LINUS TURBED AFTER HRS. 1t. 1t. CC - DRYWING CASING RO - HOCK CORE 11 - LOST	CA - CONTINUOUS FLICHT AUGER U - UNDISTURBED AFTER HSS. R. S. G ORNAMO CASSHO RC - ROCK CORE L - LOST MD - MUD BRELING	SAMPLE CONORICAS GROUND GROU	SAMPLE TYPE SAMPLE CONCUTIONS DRAFER SPLIT SPOON UNLESS OTHERWISE D. DESTREGATED AT COMPLETION DIV 8 6.0 0 HILL HOLLOW STEM ALKERS PT . PRESSED SHELBY TURE 1 . SET OF AFTER SHESS DIV 9 6.0 0 CFA . COMPRESSED RESERVE ALKERS RC . POCK COME RC . POCK COME 1 . LOST STANDARD FENETRATION TEST OFFRING 2 O.D. SAMPLER Y WITH MOST HAMBER FALLING 30" COLSIT MACE ALL OF CHERWALE.	
HILLIS - CARNES FINGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waverly Woods West Stream Crossing Storing No. 8-1	HILLIS - CARNES ENGINFERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Butter Waverly Woods West Stream Grossing Sering No. 8-2	STANDARD PENETRATION TEST-DEPOND 2" O.D. SAMPLER IT WITH 1408 HOMMER FALLING 50" COUNT MADE AT 0" INTERVALS HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waverly Woods West Stream Crossing Boring No. 8-3	HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waverly Woods West Stream Crossing Bodg No. 8-4 Lecation Howard County, Maryland Job # 07390A	HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waverly Woods West Stream Crossing Science Stocking No. B.S.	HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name Waverly Woods West Stream Creating Bosing No. G-6 Location Howard County, Maryland Joh 9 973898
Description	Location		Dates Manner VI. 140	Discount Hardward Constity, Manyland SAAPPLER Discount Hardward M. 140 SAAPPLER S. 0." Forenain Latroort Smith Latroort Smith Discount Disc	Section Harmon VI 160 In Hose Galaries 160 Femans Lagrand Simple
HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Proact Nation Waverly Woods West Stream Crossing Series No. 8-7	STANDARD PENETRATION TEST-DRIVING T O.D. SAMPLER I WITH HOS HAMMER FALLING MY COUNT MADE AT S' BREMVALS. HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Wasserts Windle Manual Streets Cressing Bodies No. B-8	HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Weverly Woods West Stream Crossing Boring No. 8-9	HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Wayerly Woods West Stream Crossing Bosing No. B-40	HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Waverly Woods West Street Consider Service Se	STANDARD PROSETRATION TOOL SAMPLER 1' WITH IND HAMBER FALLING OF COUNT MALE AS STANDARDS.
Proport Name **Through Michael Strategy Constitution **Formatic County** Market Strategy Constitution **Schaffle English Market County** Market Strategy County** **Schaffle English Market County** Market Strategy County** **Schaffle English Market County** Market Strategy County** **Schaffle English Market County** Market M	Project Name	Project Name	Project Name	Project Name Manager Wiley Weet Streams Crossing Boning No B-11	SOIL BORING PROFILES GTW'S WAVERLY WOODS SECTION 14 BULK PARCELS 'A' & 'B' AND OPEN SPACE LOTS 1 & 2
FISHER, COLLINS & CARTER, INC. CIVIL ENGINEERING CONSULTANTS & LAND SURVEYORS CENTENNIAL SQUARE OFFICE PARK - 10272 BALTIMORE NATIONAL PIKE ELLICOTT CIVIN ABEYLAND 21042	WAVERLY WOODS DEVELOPMENT CORPORATION, HOLE IN THE DOUGHNUT, LLC, & GTW JOINT VENTURE C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 FILICOTT CITY MARYLAND 21042	WAVERLY WOODS DEVELOPMENT CORP. C/O LAND DESIGN AND DEVELOPMENT, INC. 5300 DORSEY HALL DRIVE, SUITE 102 ELLICOTT CITY, MARYLAND 21042 (443-367-0422)		ALDO M. VILICAL PRODUCTION TO DATE "Professional certification I hereby certify that these documents were prepared by me, and that I am a duly Licensed Professional Engineer under the laws of the State of Maryland License No. 20748. Expiration Date 2-22-11"	ZONING: PSC & PEC TAX MAP NO. 16 PARCEL Nos. 120, 221 & P/O 249 GRID Nos. 3 & 4 THIRD ELECTION DISTRICT HOWARD COUNTY, MARYLAND DATE: JULY 20, 2009



N:\GTW-Waverly\dwg\Percon\Wet_mitigation\Phase2\MitigationPlan_SCD_all_s1.dwg, 4/30/2010 9:04:41 AM. smarmon, 1:1



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BEST MANAGEMENT PRACTICES

WATERWAYS, OR THE 100-YEAR FLOOD PLAIN.

ORIGINALLY AUTHORIZED STRUCTURE OR FILL.

FOR WORKING IN NONTIDAL WETLANDS, WETLAND

- BUFFERS, WATERWAYS, AND 100-YEAR FLOODPLAINS I. NO EXCESS FILL, CONSTRUCTION MATERIAL, OR DEBRIS SHALL BE STOCKPILES OR STORED IN NONTIDAL WETLANDS, NON-
- TIDAL WETLAND BUFFERS, WATERWAYS, OR 100 YEAR FLOODPLAIN. 2. PLACE MATERIALS IN A LOCATION AND MANNER WHICH DOES NOT ADVERSELY IMPACT SURFACE OR SUBSURFACE WATER FLOW INTO OR OUT OF NONTIDAL WETLANDS, NONTIDAL WETLAND BUFFERS,
- 3. DO NOT USE THE EXCAVATED MATERIAL AS BACKFILL IF IT CONTAINS WASTE METAL PRODUCTS, UNSIGHTLY DEBRIS, TOXIC MATERIAL, OR ANY OTHER DELETERIOUS SUBSTANCE. IF ADDITIONAL BACKFILL IS REQUIRED, USE CLEAN MATERIAL FREE OF WASTE METAL PRODUCTS, UNSIGHTLY DEBRIS, TOXIC MATERIAL, OR ANY OTHER DELETERIOUS SUBSTANCE.
- 4. PLACE HEAVY EQUIPMENT ON MATS OR SUITABLY OPERATE THE EQUIPMENT TO PREVENT DAMAGE TO NONTIDAL WETLANDS, NONTIDAL WETLAND BUFFERS, WATERWAYS, OR THE 100-YEAR FLOODPLAIN. 5. REPAIR AND MAINTAIN ANY SERVICEABLE STRUCTURE OR FILL SO THERE IS NO PERMANENT LOSS OF NONTIDAL WETLANDS, NONTIDAL WETLAND BUFFERS, OR WATERWAYS, OR PERMANENT MODIFICATION

OF THE 100-YEAR FLOODPLAIN IN EXCESS OF THAT LOST UNDER THE

- 6. RECTIFY ANY NONTIDAL WETLANDS, WETLAND BUFFERS, WATERWAYS OR 100-YEAR FLOODPLAIN TEMPORARILY IMPACTED BY ANY
- CONSTRUCTION. 7. ALL STABILIZATION IN THE NONTIDAL WETLAND AND NONTIDAL WETLAND BUFFER SHALL CONSIST OF THE FOLLOWING SPECIES: ANNUAL RYE GRASS (LOLIUM MULTIFLORUM)
- MILLET (SETARIA ITALICA) BARLEY (HORDEUM SPECIES)

RYE (SECALE CEREALE) THESE SPECIES WILL ALLOW FOR THE STABILIZATION OF THE SITE WHILE ALSO ALLOWING FOR THE VOLUNTARY REVEGETATION OF NATURAL WETLAND SPECIES. OTHER NON-PERSISTENT VEGETATION MAY BE ACCEPTABLE, BUT MUST BE APPROVED BY THE NONTIDAL WETLANDS AND WATERWAYS DIVISION, KENTUCKY 31 FESCUE SHALL NOT BE UTILIZED IN WETLAND OR BUFFER AREAS. THE AREA

- SHOULD BE SEEDED AND MULCHED TO REDUCE EROSION AFTER CONSTRUCTION ACTIVITIES HAVE BEEN COMPLETED. 8. AFTER INSTALLATION HAS BEEN COMPLETED, MAKE POST-CONSTRUCTION GRADES AND ELEVATIONS THE SAME AS THE
- 9. TO PROTECT AQUATIC SPECIES, IN-STREAM WORK IS PROHIBITED AS DETERMINED BY THE CLASSIFICATION OF THE STREAM

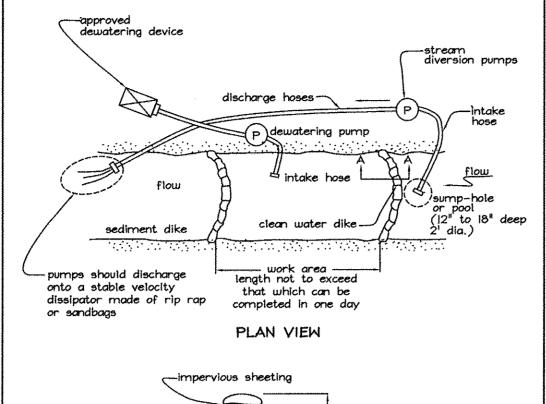
ORIGINAL GRADES AND ELEVATIONS IN TEMPORARILY IMPACTED

- USE I WATERS: IN STREAM WORK SHALL BE CONDUCTED DURING THE PERIOD MARCH I THROUGH JUNE 15, INCLUSIVE,
- 10. STORMWATER RUNOFF FROM IMPERVIOUS SURFACES SHALL BE CONTROLLED TO PREVENT THE WASHING OF DEBRIS INTO THE
- II. CULVERTS SHALL BE CONSTRUCTED AND ANY RIPRAP PLACED SO AS NOT TO OBSTRUCT THE MOVEMENT OF AQUATIC SPECIES, UNLESS THE PURPOSE OF THE ACTIVITY IS TO IMPOUND WATER.

PLANTING NOTES

- . Riparian areas may be planted as soon as reasonable to do so. Late vary from year to year but planting may generally begin as soon as the ground is no longer frozen. Alternate planting dates may be considered as condition warrants.
- . Soil amendments and fertilization recommendations will be made based upon the results of soil analysis for nitrogen, phosphorus, potassium, organic matter content and pH. If required, fertilizer will be provided using a slow release, soluble 16-8-16 analysis designed to last 5-8 years contained in polyethylene perforated bags such as manufactured
- 3. Plant materials will be planted in accordance with the Plantina Distribution Diagram, Planting Details and plant schedule. . Plant material shall be nursery grown and inspected prior to planting. Plants not conforming to the American Standard for Nursery Stock specifications for size, form, vigor, or roots, or due to trunk wounds,
- breakage, desiccation, insect or disease must be replaced. . Planting stock must be protected from desiccation at all times prior to planting. Materials held for planting shall be moistened and placed in cool shaded areas until ready for placement.
- 6. Newly planted trees may require watering at least once per week during the first growing season depending on rainfall in order to get established. The initial planting operation should allow for watering during installation to completely soak backfill material.
- . Planting holes should be excavated to a minimum diameter of 2.5 to 3 times the diameter of the root ball or container. Mechanical augering is preferred with scarification of the sides of each hole. 8. Mulch shall be applied in accordance with the diagram provided and
- shall consist of composted, shredded hardwood bark mulch, free of wood alcohol. 3. Eighty-five per cent (85%) survival of mitigation plantings shall be
- guaranteed for one (1) year. Replacement plantings shall be provided after first year's growing season.
- 10. Planting contractor must install 6' deer fence, Deerbusters item DEE-8106 or approved equivalent, and implement a deer protection management plan to protect plant materials during the warranty period.

DETAIL 1.2: PUMP-AROUND PRACTICE



cross section of sandbaa dike

Maryland Department of the Environment Mater Marynament Administration

SECTION A-A

MONITORING PROTOCOL FOR MITIGATION PROJECTS

PAGE 1.2 - 3

LESS THAN OR EQUAL TO 0.5 ACRES A. Compensatory mitigation projects required as a condition of a State Nontidal Wetlands and Waterways Permit for wetland impacts should achieve the goals and objectives established in the mitigation plan. Mitigation projects of less than or equal to one-half acres should conform to the following criteria, unless otherwise agreed to by the regulatory agencies.

1. After five (5) years, greater than 85% of the site shall be vegetated (either by planted or naturally revegetated plants) by native wetland species similar to those found in the nontidal wetland lost or by a species composition acceptable to the Nontidal Wetlands and Waterways Division; and 2. The entire site should have wetland hydrology.

B. An as-built design plan shall be submitted to the Nontidal Wetlands and Waterways Division within 120 days of the completion (this includes grading, planting and/or vegetative stabilization) of the mitigation project. If the project was built as planned, a notification stating that can substitute for the "as-built" plan.

C. The permittee will be responsible for submitting annual monitoring reports to the Nontidal Wetlands and Waterways Division, for a period of five (5) consecutive years from the completion of the construction of the mitiaation site. If the U.S. Army Corps of Engineers requires a longer monitoring period, reports sent to them should also be sent to the Nontidal Wetlands and Waterways Division. In the case of longer monitoring periods, monitoring reports may not be required every year, but will be determined by the MDE and the Corps. Annual monitoring reports winter-early spring plantings are preferred. Earliest planting dates will must be submitted to MDE by December 31 of each monitoring year. The first monitoring report is due the year the mitigation planting occurs, unless planting occurs after April 15, in which case the first monitoring report will not be due until the end of the next year. For each monitoring report, at least one monitoring visit should be conducted during the growing season for the vegetative monitoring (between May I and September 3) for forested/scrub-shrub systems and between June 15 and September 31 for emergent systems). These site visits should preferably be during a period with normal precipitation and aroundwater levels. The following information should be included with the annual monitoring

Date of site inspection.

- 2. A brief paragraph describing the purpose of the approved project, acreage and type of quatic resources impacted, and mitigation acreage and type of aquatic resources authorized to compensate for the aquatic impacts. Include the dates the mitigation construction was started and the planting was completed. 3. A narrative description of the mitigation site addressing its position in the
- 4. A narrative description of how the mitigation site has achieved the goals objectives and project standards established for the project. 5. Estimate the percent cover by dominant plant species (including volunteer plants) and any invasive plant species. Estimate percent cover by plants with a wetland indicator status of FAC or wetter.

landscape, adjacent waterbodies, and adjacent land use.

- 6. Estimate percent survival of woody planted material and estimate woody stem density per acre for trees/shrubs taller than 10 inches (including volunteers). 7. One set of photographs of the mitigation site taken any time during the months of May through September of each monitoring year. Photo location points should be identified on the appropriate maps and labeled with the direction in which
- the photo was taken 8. Where invasive species (ex. Phragmites australis) become a problem, it may be necessary to undertake eradication measures. If eradication is warranted, the permittee should submit a ps impacted, and mitigation acreage and type of aquatic resources authorized to compensate for the aquatic impacts. Include the dates the mitigation construction was started and the planting was completed. A narrative description of the mitigation site addressing its position in the landscape, adjacent waterbodies, and adjacent land use.
- A narrative descr??Ian to the regulatory agency detailing the method to be used. 9. Groundwater monitoring wells may be required as part of the monitoring plan. Water levels within each well should be measured, at a minimum, on a bi-monthly basis during the months of March through May and monthly from June through October. List these data (i.e., water levels and date measured) in the monitoring
- 10. Estimate percent cover of the site that is inundated or saturated to the surface on the dates of the site visits. 11. Stressors to the site (ex. deer browse, beaver, ATVs, etc.)

12. A description of any modifications which have been made or need to be made to implement the mitigation plan so as to meet the project standards.

D. Remedial measures proposed by the permittee are subject to review and approval by the regulatory agencies prior to implementation. In the event that remedial measures are implemented, the monitoring period may be extended on a case-by-case basis, but will not be extended for more than a three-year period. The treatment of non-native invasive plant species does not need the approval of the MDE Wetlands and Waterways Division, but should be completed at the correct time of year by someone with a current pesticide applicator certification and the required toxic materials permit.

DEVELOPER'S CERTIFICATE

"I/WE CERTIFY THAT ALL DEVELOPMENT AND CONSTRUCTION WILL BE DONE ACCORDING TO THIS PLAN FOR SEDIMENT AND EROSION CONTROL, AND THAT ALL RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I ALSO AUTHORIZE PERIODIC ON SITE INSPECTION BY THE HOWARD SOIL CONSERVATION DISTRICT.

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS Matte 2. May 1. Chief, Bureau of Highways 42 5-14-10 SIGNATURE OF DEVELOPER Date

4/20/10

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING & ZONING

ENGINEERS CERTIFICATE

"I CERTIFY THAT THIS PLAN FOR SEDIMENT AND EROSION

BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS

REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT

CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN

AND THAT IT WAS PREPARED IN ACCORDANCE WITH THE

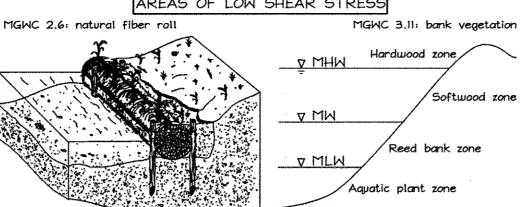
SIGNATURE OF ENGINEER!

N:\CTW-Waverly\dwg\Percon\Wet_mitigation\Phase2\MitigationPlan_SCD_all_s3.dwg, 4/30/2010 8:55:44 AM, smarrton, 1:1

Division of Land Development

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT GONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT HOWARD SCD / DATE AREAS OF LOW SHEAR STRESS

DETAIL 2.11(a): TOE PROTECTION



MATERIAL SPECIFICATIONS

Plant species including woody varieties should be chosen by a plant specialist according to location within the riparian bank zone and adaptability to sitespecific conditions and objectives. Refer to MGWC 2.4 (Live Stakes) for material and Detail 2.11 for placement and usage. The use of non-native plantings may result in reduced natural biodiversity.

Rock toe protection should be composed of angular stones sized to resist the near-bed channel velocities resulting from the design storm event according to MGWC 2.1 Riprap. The minimum toe trench depth should be sufficent to resist scour or at a minimum of 1.5 times the maximum riprap diameter. INSTALLATION GUIDELINES

The stream should be redirected by an approved temporary stream diversion (See Section I: Temporary Instream Construction Measures, Maryland's Guidelines to Waterway Construction), the construction area should be dewatered, and any disturbed banks should be stabilized. 2. The appropriateness of toe stabilization measures should be based primarily upon the magnitude of the imposed shear stress at the reach of interest among other considerations as shown in Detail 2.11. Installation will vary according to material used. Vegetated Toe Protection Measures:

Vegetation: Refer to MGWC 2.6: Natural Fiber Rolls Live Fascines: Refer to MGWC 2.5: Live Fascines Crib Walls: Refer to MGWC 2.9: Live Crib Walls Rigid Toe Protection Measures:

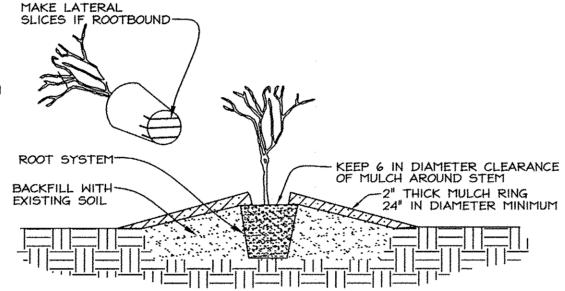
Riprap: a rock toe designed to withstand the near-bed velocities of the design storm event can be used to increase the effectiveness of toe protection measures in moderate to high shear stress areas. Rocks should be sized and filter layers designed according to MGMC 2.1: Riprap and Figure 2.1. Imbricated Riprap: Refer to MGWC 2.2: Imbricated Riprap Gabion: Refer to MGWC 2.3: Gabion

3. The use of rock vanes (MGWC 3.3: Rock Vanes) should be considered to oreak up high velocities at the toe of any embankment where bank stabilization measures are to be employed. Additionally, grade control structures such as weirs and step pool sequences should be used to enhance channel bed

stability in reaches that are actively incising or may be subjected to upstream migrating instabilities. 4. Once construction is completed, the diversion should be removed from upstream to downstream. Sediment control devices, including perimeter erosion controls, are to remain in place until all disturbed areas are stabilized in accordance with an approved sediment and erosion control plan and the

inspection authority approves their removal

MARYLAND DEPARTMENT OF THE ENVIRONMEN WATER MANAGEMENT ADMINISTRATION



CONTAINER PLANTING NOT TO SCALE

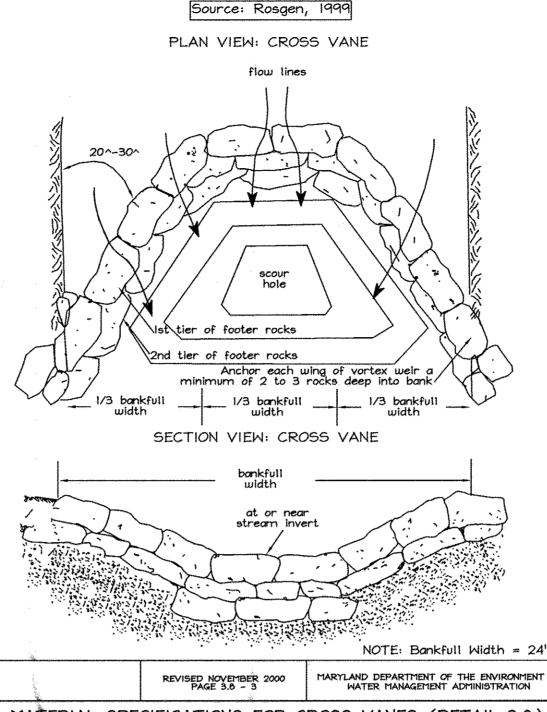
PLANTING PROCEDURE FOR CONTAINER GROWN PLANTS REMOVE THE PLANT EITHER BY CUTTING OR INVERTING THE CONTAINER USE A KNIFE TO CUT THROUGH BOTTOM HALF OF THE ROOT BALL. PLANT TREES AND SHRUBS ON FORMED UP MOUNDS 12" ABOVE THE

4. PLANTING HOLE TO BE 2-3 TIMES THE DIAMETER OF THE CONTIANER. 5. BACKFILL 2/3 OF THE ROOT BALL AND WATER.

6. AFTER WATER PERCOLATES, BACKFILL HOLE TO TOP OF ROOT BALL AND GENTLY TAMP SOIL TO FIRM CONTACT WITH PLANT.

7. APPLY MULCH RING AROUND PLANT KEEPING A 6 IN CLEARANCE FROM STEM

DETAIL 3.8(a): CROSS VANES



MATERIAL SPECIFICATIONS FOR CROSS VANES (DETAIL 3.8) Rock and boulder material for the construction of cross vanes should meet the following requirements: Footer Rocks Vortex rocks should be large enough to achieve the desired height

when partially buried in the stream bed and should be sized to resist movement from shear stresses expected for the design flow. Footer rocks should be long and Riprap: Riprap for added stability, bank armoring, and toe protection should be capable of withstanding bankfull flow velocities according to MGWC 2.1: Riprap.

INSTALLATION GUIDELINES FOR CROSS VANES (DETAIL 3.8) All erosion and sediment control devices, including dewatering basins, should be implemented as the first order of business according to a plan approved by the WMA. The recommended construction procedure for both cross vanes and weirs

should proceed as follows (refer to Detail 3.8): 1. The stream should be diverted according to a WMA recommended measure, and the construction area should be dewatered. 2. Cross vanes are typically designed with a "U" shape such that the apex of the

structure points upstream. The angle the arms make with the upstream bank should be approximately 20 to 30 degrees so that flows are directed away from the banks and deeper pool areas are created directly downstream of the vane or weir. All rocks should touch adjacent rocks to form a tight fit. Vane rocks shall be placed on top of footer rocks so that each vane rock rests upon two halves of each faoter rock below, and so that the vane rock is offset in the upstream direction. Vane rocks shall be shingled upstream. On unstable bed substrates, two tiers of or cross vane from being undermined. The top elevation of the center rock(s), at the apex of the weir or vane, should be at or near bed level to permit fish passage at low flows, and the end rocks on either bank should be at bankfull level. Once the excavated portion of the bank has been backfilled, it should be armored with appropriately sized riprap, sod mats, or willow transplants.

3. Adjacent cross vanes should be spaced sufficiently far apart to allow for proper

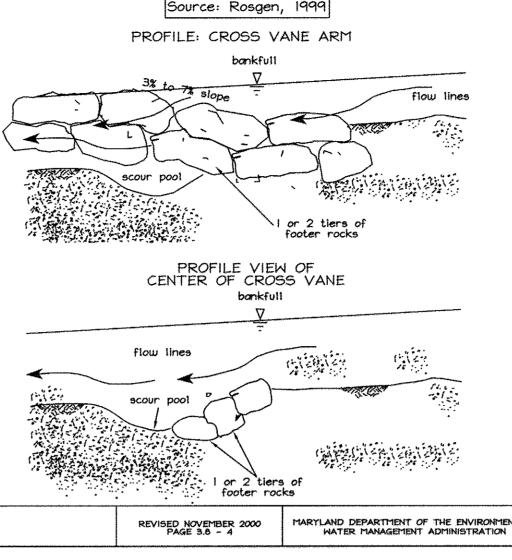
riffle or pool development according to step-pool and pool-riffle configurations as provided in MGWC 3.9: Step Pools. Additionally, it has been recommended that the overall maximum drop controlled by a set of weirs should be less than 2 feet (0.6 meters) for stability reasons. 4. All disturbed sections of the channel, including the banks and streambed, should be stabilized with methods approved by the WMA.

5. All cross vanes should be monitored to determine if: * their orientation and geometry (e.g., the height of the drop) hinder fish * their performance is adversely affected by deposited sediment, and * their placement causes bank instabilities and undesirable lateral stream overment

especially in the vicinity of the plunge pools.

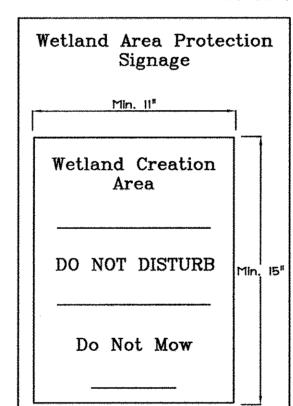
Source: Rosgen, 1999

DETAIL 3.8(b): CROSS VANES



* SPECIES A SPECIES 2 A SPECIES 3

PLANTING DISTRIBUTION DIAGRAM



5 lbs. / Acre 5 lbs. / Acre

NETLAND INDICATOR | APPLICATION RATE 2 lbs. / Ac. 20 lbs. / Ac.

WOODY PLANTING LIST - 0.04 Ac. (350 TPA) SCIENTIFIC NAME QUANITY SPACING SIZE COMMON NAME 24"-36' Gallon Cornus amomum Random /ibumum dentatum arrow wood 12' to Salix nigra Container 231 O.C. Grown 14 2-3' O.C. Live Stake black willow Salix nigra

GTW'S WAVERLY WOODS Section 14

TAX MAP 16 3rd ELECTION DISTRICT

PARCELS 120, 221, \$ p/o 249 HOWARD COUNTY, MARYLAND



3. The cross-section shall be excavated to the neat lines and arades as shown on the plans. Over-excavated areas shall be backfilled with moist soil compacted to the density of the surrounding material.

4. No abrupt deviations from the design grade or horizontal alignment shall be permitted unless authorized by the ERI Stream Restoration

CONSTRUCTION NOTES/SPECIFICATIONS

1. The contractor shall install appropriate sediment and erosion control

2. The foundation area shall be cleared of trees, stumps, roots, sod,

the stream restoration specialist and these drawings

loose rock, or other objectionable material.

devices before project. All work to be performed at the direction of

5. Filter, bedding, and rock rip-rap shall be placed to line and grade in the manner specified. 6. Construction operations shall be done in such a manner that erosion,

air, and water pollution will be minimized and held within lead limits. The completed job shall present a workmanlike appearance. All disturbed areas shall be vegetated or otherwise protected against soil

7. Filter cloth shall be placed beneath rip-rap where indicated. The filter cloth shall consist of either woven or non-woven monofilament fiber and shall conform to the ASTM D 1777, ASTM D 1682, having a thickness of 20-60 Mils, and a grab strength of 90-120 LBS.

8. All boulders shall be well graded selected rip-rap boulders, natural in color and pre-approved by the Stream Restoration Specialist. 9. The subgrade for the filter, rip-rap, or gabion shall be prepared to the required lines and grades. Any fill required in the subgrade shall

be compacted to a density of approximately that of the surrounding undisturbed material. 10. The rock or gravel shall conform to the specified grading limits when

installed. 11. Geotextile shall be protected from punching, cutting, or tearing. Any damage other than an occasional small hole shall be repaired by placing another piece of geotextile over the damaged part or by completely

replacing the geotextile. All overlaps whether for repairs or for

joining two pieces of geotextile shall be a minimum of one foot.

12. Stone and boulders for the rip-rap may be placed by equipment. It shall be constructed to the full course thickness in one operation and in such a manner as to avoid displacement of underlying materials. The stone for rip-rap or gabion outlets shall be delivered and placed in a manner that will ensure that it is reasonably homogeneous with the small stones and spalls filling the voids between the larger stones. Rip-rap shall be placed in a manner to prevent damage to the filter blanket or geotextile. Hand placement will be required to the extent necessary to prevent damage to the permanent works. Exact placement will be required as directed by the ERI Stream Specialist in

13. The stone shall be placed so that it blends in with the existing grade. If the stone is placed too high, then the flow will be forced out of the channel and scour adjacent to the stone will occur.

14. The addition of a minimum 30 cubic yards of wood chips is required within the wetland mitigation area.

15. Wood chips must be spread evenly over the subgrade then 4"-6" topsoil overlayed. Disc or chisel plow the mitigation area to a minimum depth of 8" immediately after placement of wood chips and topsoil.

16. Add a minimum of one and 1/2 dump truck loads (three (3) dump truck loads per acre) of large (>/= 12" diameter) woody debris to the mitigation site.

17. PERFORMANCE STANDARDS: Maryland Department of the Environment (MDE) requires a five (5) year monitoring schedule establishing responsibility for the removal of exotic and nuisance vegetation, and permanent establishment of the nontidal wetland and its component parts. Monitoring shall be conducted according to MDE's Monitoring Protocol. If tree protection tubes or states are used, they must be removed prior to monitoring termination. After five (5) years from the completion of the mitigation project, greater than 85% of the mitigation site shall be vegetated (either by planted or naturally revegetated plants) by native wetland species similar to those found in the nontidal wetland lost or by a species composition acceptable to the Nontidal Metlands and Materways Division. Vegetative communities not acceptable to the Division would include communities dominated by nuisance vegetation or facultative upland species. Additionally for the project to be deemed successful, and the applicant to satisfy the mitigation responsibility, the entire wetland must have wetland

18. Exploration Research, Inc shall be responsible for preparing and submitting the annual monitoring reports for this project.

19. Documentation that the selected protection mechanism has been executed must be submitted to MDE within sixty (60) days of the completion of construction of the mitigation project.

SEEDMIX SPECIFICATIONS

. WETLAND SEED MIX APPLIED TO ALL DISTURBED AREAS IN BOTTOM OF WETLAND CREATION

II. UPLAND SEED MIX TO BE APPLIED TO ALL OTHER DISTURBED AREAS.

OWNER/DEVELOPER WAVERLY WOODS DEVELOPMENT CORP c/o Land Design and Development, Inc. 5300 Dorsey Hall Drive, Suite 102 Ellicott City, MD 21042

(443) 367-0422

REVISED

Wetland Mitigation Erosion and Sediment Control Plan

DESIGN BY: SLH

DRAWN BY: SMM CHECKED BY: SLH **EXPLORATION** SCALE: As shown RESEARCH, INC. DATE: Apr. 30, 2010 **ENVIRONMENTAL CONSULTANTS** W.O. No.: ____2000 6332 HOWARD LANE ELECHDIGE WARYLAND 21075 TEL: (410) 567-5210 FAX:(410) 796-1562 SHEET No.: 27 OF 27

duly licensed professional engineer under the laws of the State of Maryland, License No. #34689, Expiration Date: 07/08/11

the dormant season. 2. Basal end of stake should be cut on an angle with the top cut

in 18in. to 3 feet long stakes. 4. Keep cuttings moist at all times.

. Install stakes with buds pointing upwards.

PLACE LIVE STAKES PERPENDICULAR TO BANK SLOPE-ANGLED DOWN

SEEDING-END COIR FABRIC FEET ABOVE THE MOST UPSLOPE ROW OF LIVE BANK FULL WATER ELEVATION BASE FLOW *RoLanka Bio D Mat 70 ANCHOR COIR IVE STAKES (2-3 FT LENGTH)
1/2 TO 1 1/2 INCHES
IN DIAMETER

TYPICAL LIVE STAKE AND COIR MATTING DETAIL

12" COIR LOG TOE PROTECTION UPSTREAM OF CROSS VANE

PROFESSIONAL CERTIFICATION

hereby certify that these documents were

prepared or approved by me, and that I am a

CROSS-SECTION VIEW (NOT TO SCALE) MATERIAL SPECIFICATIONS AND INSTALLATION GUIDELINES FOR LIVE STAKING

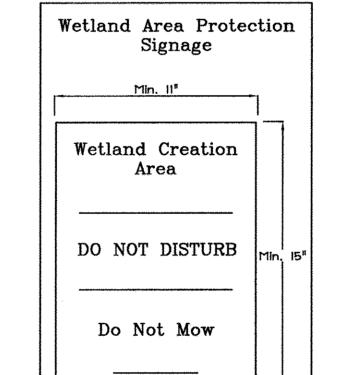
> 1. All cuttings shall be freshly cut from live woody plants of the species indicated, such as willow, alder, and shrub dogwood, during

3. Prepare cuttings from dormant .5 in. to 2 in. diameter stock cut 5. Install stakes with deadblow hammer, angled downstream, on 3.0 ft.

. Replace live stakes that have split or become mushroomed.

1. Mix shrubs in staging area. 2. Spacing should be random, between 12' and 23' apart.

NOT TO SCALE



MGWC 1.2: Pump-Around Practice The work shall consist of installing a temporary pump around and supporting measures to divert flow around in-stream construction sites

Implementation Sequence Sediment control measures, pump arounds, and associated channel and bank construction shall be completed in the following sequence: 1. Construction activities including the installation of erosion and sediment control measures shall not

shall be marked in the field prior to construction. The contractor is responsible for any damage to existing utilities that may result from construction and shall repair the damage at his/her own expense to the county's satisfaction. 2. The contractor shall notify the Maryland Department of the Environment or WMA sediment control inspector at least 5 days before beginning construction. Additionally, the contractor shall inform the local environmental protection and resource management inspection and enforcement division and the provider of local utilities a minimum of 48 hours before starting construction. 3. The contractor shall conduct a pre-construction meeting on site with the WMA sediment control

begin until all necessary easements and/or right-of-ways have been acquired. All existing utilities

inspector, the county project manager, and the engineer to review limits of disturbance, erosion and sediment control requirements, and the sequence of construction. (The contractor shall stake out all limits of disturbance prior to the pre-construction meeting.) The participants will also designate the contractor's staging areas and flag all trees within the limit of disturbance which will be removed for construction access. Trees shall not be removed within the limit of disturbance without approval from the WMA or local authority. 4. Construction shall not begin until all sediment and erosion control measures have been installed and

approved by the engineer and the sediment control inspector. The contractor shall stay within the limits of the disturbance as shown on the plans and minimize disturbance within the work area whenever possible. 5. Upon installation of all sediment control measures and approval by the sediment control inspector and the local environmental protection and resource management inspection and enforcement division, the contractor shall begin work at the upstream section and proceed downstream beginning with the establishment of stabilized construction entrances. The sequence of construction must be followed unless the contractor gets written approval for deviations from the WMA or local authority. The contractor shall only begin work in an area which can be completed by the end of the day (including grading adjacent to the channel). At the end of each work day, the work area must be stabilized

and the pump around removed from the channel. Work shall not be conducted in the channel during 6. Sandbag dikes shall be situated at the upstream and downstream ends of the work area as shown on the plans, and stream flow shall be pumped around the work area. The pump shall discharge onto a stable velocity dissipater made of riprap or sandbags. 7. Water from the work area shall be pumped to sediment filtering measure such as a sediment bag. The measure shall be located such that the water drains back into the channel below the downstream

8. Traversing a channel reach with equipment where no work is proposed should be avoided. If equipment has to traverse such a reach for access to another area, then timber mats or similar measures shall be used to minimize disturbance to the channel. Temporary stream crossings shall be used only when necessary and only where noted on the plans or specified by the engineer, 9. All steam restoration measures shall be installed as indicated by the plans and all banks graded in accordance with the grading plans and typical cross-sections. All grading must be permanently stabilized at the end of each day with seed and mulch or seed and matting as specified on the plans. 10. After an area is completed and stabilized, sandbag diversions, the water pump, and sediment filtering measure shall be moved to the next work area. This shall be accomplished by first moving the downstream sandbag dike to the new upstream pump around location and then by relocating the upstream sandbag dike, velocity dissipater, and sediment filter to the new downstream location.

11. A pump around must be installed on any tributary or storm drain outfall which contributes

base-flow to the work area. This should be accomplished by locating a sandbag dike at the

12. If a tributary is to be restored, construction should take place on the tributary before work on the main stem reaches the tributary confluence. Construction in the tributary, including pump around practices, shall follow the same sequence as for the main stem of the river or stream. When construction on the tributary is completed, work on the main stem shall resume. Water from the tributary shall continue to be pumped around the work area in the main stem. 13. The contractor is responsible for providing access to and maintaining all erosion and sediment control devices until the sediment control inspector approved their removal.

14. After construction, all disturbed areas shall be regraded and revegetated as per the planting plan.

downstream end of the tributary or storm drain outfall and pumping the steam flow around the work

area. This water should discharge onto the same velocity dissipater used for the main stem pump