GENERAL NOTES

1.) THE PROJECT IS IN CONFORMANCE WITH THE LATEST HOWARD COUNTY STANDARDS UNLESS WAIVERS HAVE BEEN APPROVED.

2.) BOUNDARY IS BASED ON A FIELD RUN MONUMENTED SUBURBAN BOUNDARY SURVEY PERFORMED BY JOHN A. MILDENBERG IN MARCH. 2006.

3.) THE EXISTING TOPOGRAPHY SHOWN ONSITE IS BASED ON AN AERIAL TOPOGRAPHIC SURVEY PERFORMED BY WINGS AERIAL MAPPING CO., INC. FLOWN ON OR ABOUT JANUARY, 2006

COORDINATES SHOWN HEREON ARE BASED UPON THE HOWARD COUNTY GEODETIC CONTROL WHICH IS BASED UPON THE MARYLAND STATE PLANE COORDINATE SYSTEM. HOWARD COUNTY MONUMENT NOS. 16E1 AND 0012 WERE USED FOR THIS PROJECT.

5.) WATER IS PUBLIC. THE CONTRACT NUMBER IS 24-4549-D . THE DRAINAGE AREA IS LITTLE PATUXENT. 6.) SEWER IS PUBLIC, THE CONTRACT NUMBER IS 24-4549-D. THE DRAINAGE AREA IS LITTLE PATUXENT.

7.) STORMWATER MANAGEMENT QUALITY AND QUANTITY CONTROL IS PROVIDED WITHIN SWMF #3 & #4 (P-1 MICROPOOL ED PONDS), THE OFFLINE RECHARGE CHAMBER AT SWMF#4, SWMF#5 (P-5 POCKET POND), SWMF#6 (F-1 SURFACE SAND FILTER WITH DRY DETENTION POND) AND SHEETFLOW TO BUFFER CREDITS. SWMF #3, #4 & #5 SHALL BE PRIVATELY OWNED AND JOINTLY MAINTAINED. THE RECHARGE CHAMBER AT 4 SHALL BE PRIVATELY OWNED AND PRIVATELY MAINTAINED. FOR SWMF #6, THE SURFACE SAND FILTER PRIVATELY OWNED AND PRIVATELY MAINTAINED WHILE THE DRY DETENTION POND SHALL BE

PRIVATELY OWNED AND JOINTLY MAINTAINED. 8.) EXISTING UTILITIES SHOWN ARE BASED ON CONTRACT DRAWINGS, AERIAL AND FIELD SURVEYED LOCATIONS 9.) 100-YEAR FLOODPLAIN STUDY AND REPORT WAS PREPARED BY BENCHMARK ENGINEERING, INC. IN MAY 2008 FOR THE FLOODPLAIN LOCATED WITHIN OPEN SPACE LOT 204 AND GOLF SPACE LOTS 77 & 209. THE

FLOODPLAIN LOCATED ON PARCEL 'B' IS BASED ON THE STUDY PERFORMED BY BERNARD JOHNSON IN MAY, 10.) WETLANDS LOCATIONS SHOWN ARE BASED ON APPROVED STUDIES AS SHOWN ON COMPREHENSIVE SKETCH PLAN OF TURF VALLEY. WETLANDS ARE BASED ON A STUDY CONDUCTED BY EXPLORATION RESEARCH FOR

S-86-13 AND VERIFIED BY ECO-SCIENCE PROFESSIONALS, INC. IN JUNE 2002 AND APRIL 2004 (FOR DEVELOPMENT IN AND AROUND PODS I, K, L, M, N, O, P, Q, & S). 11.) NOISE STUDY WAS PREPARED BY POLYSONICS DATED NOVEMBER, 2007 AND REVISED IN MAY, 2008. THE 65 dba noise contour line drawn on this subdivision plan is advisory as required by the howard Y DESIGN MANUAL, CHAPTER 5, REVISED FEBRUARY, 1992 AND CANNOT BE CONSIDERED TO EXACTLY LOCATE THE 65 dBA NOISE EXPOSURE. THE 65 dBA NOISE LINE WAS ESTABLISHED BY HOWARD COUNTY TO

ALERT DEVELOPERS, BUILDERS AND FUTURE RESIDENTS THAT AREAS BEYOND THIS THRESHOLD MAY EXCEED GENERALLY ACCEPTED NOISE LEVELS ESTABLISHED BY THE U.S. DEPARTMENT OF HOUSING AND URBAN DEVELOPMEN

12.) THE GEOTECHNICAL REPORT WAS PREPARED BY HILLIS CARNES ENGINEERING ASSOCIATES, INC. IN MARCH, 2006 AND SUPPLEMENTED IN AUGUST, 2007. 13.) THE SUBJECT PROPERTY IS ZONED PGCC PER THE 2-2-2004 COMPREHENSIVE ZONING PLAN AND THE

"COMP LITE" ZONING AMENDMENTS EFFECTIVE 7-28-2006.

14.) TO THE BEST OF OUR KNOWLEDGE, THERE ARE NO BURIAL GROUNDS, CEMETERIES OR HISTORIC STRUCTURES LOCATED ON THIS SITE.

15.) NO GRADING, REMOVAL OF VEGETATIVE COVER OR TREES, PAVING AND NEW STRUCTURES SHALL PERMITTED WITHIN THE LIMITS OF WETLANDS, STREAMS, OR THEIR REQUIRED BUFFERS, 100-YEAR FLOODPLAIN OR 25% OR GREATER STEEP SLOPES THAT ARE AT LEAST 20,000 S.F. OF CONTIGUOUS AREA EXCEPT FOR THE DISTURBANCES SHOWN ASSOCIATED WITH THE 3 STREAM CROSSINGS (STATIONS 55+50, 78+50 AND 82+00 ALONG RESORT ROAD, MDE PERMIT #02-NT-0009 / 200261454 EFFECTIVE MAY 16, 2006 WITH AN EXPIRATION OF DECEMBER 31, 2011, THE DISTURBANCE OF NON-TIDAL WETLANDS ALLOWED IN THIS PERMIT REQUIRES 73,745 S.F. OF WETLAND MITIGATION WHICH SHALL OCCUR ONSITE (WITHIN TURF VALLEY). NONE OF THAT MITIGATION IS PROPOSED WITHIN THE LIMITS OF THE VILLAGES AT TURF VALLEY.

16.) THIS PROJECT IS LOCATED WITHIN THE METROPOLITAN DISTRICT.

17.) THE LOTS SHOWN HEREON COMPLY WITH THE MINIMUM OWNERSHIP WIDTH AND LOT AREA AS REQUIRED BY THÉ MARYLAND STATE DEPARTMENT OF THE ENVIRONMENT.

18.) DRIVEWAYS SHALL BE PROVIDED PRIOR TO ISSUANCE OF A USE AND OCCUPANCY PERMIT FOR ANY NEW DWELLINGS FOR FIRE AND EMERGENCY VEHICLES PER THE FOLLOWINGMINIMUM REQUIREMENTS:

A) WIDTH - 12' (16' SERVING MORE THAN ONE RESIDENCE).

- B) SURFACE 6" OF COMPACT CRUSHER RUN BASE WITH TAR AND CHIP COATING (1.5" MIN).) GEOMETRY - MAX. 15% GRADE, MAX. 10% GRADE CHANGE & MIN. 45' TURNING RADIUS.
- D) STRUCTURES(CULVERTS/BRIDGES) CAPABLE OF SUPPORTING 25 GROSS TONS (H25 LOAD)) DRAINAGE ELEMENTS - CAPABLE OF SAFELY PASSING 100 YEAR FLOODPLAIN WITH NO MORE THAN FOOT DEPTH OVER DRIVEWAY.
- F) STRUCTURE CLEARANCES MINIMUM 12 FEET. G) MAINTENANCE - SUFFICIENT TO INSURE ALL WEATHER USE.

19.) LANDSCAPING FOR THIS SUBDIVISION IS PROVIDED IN ACCORDANCE WITH A CERTIFIED LANDSCAPE PLAN INCLUDED WITH THIS ROAD CONSTRUCTION PLAN SET IN ACCORDANCE WITH SECTION 16.124 OF THE HOWARD COUNTY CODE AND THE LANDSCAPE MANUAL, FINANCIAL SURETY FOR THE REQUIRED LANDSCAPING HAS BEEN POSTED AS PART OF THE DPW DEVELOPER'S AGREEMENT IN THE AMOUNT OF \$18,450.00 (\$11,400.00 FOR 38 SHADE TREES, \$7,050.00 FOR 47 EVERGREENS).

20. VILLAGES AT TURF VALLEY, PHASE 2

TOTAL FOREST CONSERVATION OBLIGATION AMOUNT OF 4.48 ACRES SHALL BE MET BY THE RETENTION OF 2.03 AC. OF NET TRACT AREA FOREST WITHIN A FOREST CONSERVATION EASEMENT (#6-8) ON PARCEL 401 AND BY THE OFFSITE RETENTION OF 4.90 AC. OF FOREST WITHIN FOREST CONSERVATION EASEMENTS (#9-13) OF WHICH ONLY 2.45 AC. IS CREDITED (SINCE THIS RETENTION IS OFFSITE IT IS CREDITED AT A 2:1 RATIO), FINANCIAL SURETY FOR THE REQUIRED FOREST CONSERVATION HAS BEEN POSTED AS PART OF THE DPW DEVELOPERS AGREEMENT IN THE AMOUNT OF \$42,689.00 (\$0.20 PER SQUARE FOOT).

PARCEL 8 & 394: THIS PORTION OF THE PROJECT IS EXEMPT FROM HOWARD COUNTY FOREST CONSERVATION REQUIREMENTS UNDER SECTION 16.1202(b) OF THE COUNTY CODE SINCE IT IS A PLANNED UNIT DEVELOPMENT UNDER S-86-13

FAIRWAYS AT TURF VALLEY, PHASE 2 AND 3:

OF EASEMENT IN THE LAND RECORDS OF HOWARD COUNTY.

THE TOTAL FOREST CONSERVATION OBLIGATION OF 1.12 ACRES (PHASE 2, F-10-084 AND 0.22 ACRES PHASE 3, F-10-086) HAS BEEN MET BY THE OFFSITE RETENTION OF 1.0 ACRES OF FOREST WITHIN FOREST CONSERVATION EASEMENT #14 OF WHICH ONLY 0,54 ACRES IS CREDITED (SINCE THIS RETENTION IS OFFSITE IT IS CREDITED AT A 2:1 RATIO) AND BY THE PLANTING OF O. BOACRES OF FOREST WITHIN FOREST CONSERVATION FASEMENT #14 FINANCIAL SURFTY FOR THE REQUIRED FOREST CONSERVATION HAS BEEN POSTED AS PART OF THE DPW DEVELOPERS AGREEMENT IN THE AMOUNT OF \$22,608.00 FOR PHASE 2, F-10-084 AND \$3,833.00 FOR PHASE 3, F-10-086. THE FOREST CONSERVATION EASEMENTS FOR FAIRWAYS AT TURF VALLEY, PHASES 2 AND 3 AND SHOWN ON THESE PLANS SATISFY FOREST CONSERVATION OBLIGATIONS BASED ON PRELIMINARY FOREST CONSERVATION PLANS FOR THOSE SUBDIVISIONS. THE EASEMENTS ARE SUBJECT TO CHANGE BASED ON DEVELOPMENT OF THE FINAL FOREST CONSERVATION PLANS FOR FAIRWAYS AT TURF VALLEY, PHASES 2 AND 3 DURING THEIR FINAL SUBDIVISION PLAN STAGE.

THE FOREST CONSERVATION EASEMENTS HAVE BEEN ESTABLISHED TO FULFILL THE REQUIREMENTS OF SECTION 16.1200 OF THE HOWARD COUNTY CODE, FOREST CONSERVATION ACT. NO CLEARING, GRADING, OR CONSTRUCTION IS PERMITTED WITHIN THE FOREST CONSERVATION EASEMENTS: HOWEVER, FOREST MANAGEMENT PRACTICES AS DEFINED IN THE DEED OF FOREST CONSERVATION EASEMENT ARE ALLOWED

21.) <u>RESERVATION OF PUBLIC UTILITY AND FOREST CONSERVATION EASEMENTS</u> DEVELOPER RESERVES UNTO ITSELF, ITS SUCCESSORS AND ASSIGNS, ALL EASEMENTS SHOWN ON THIS PLAN FOR WATER, SEWER, STORM DRAINAGE, OTHER PUBLIC UTILITIES, LOCATED IN, ON, OVER AND THROUGH LOT 203, OPEN SPACE LOTS 204-207, GOLF SPACE LOTS 77, 78, 208 & 209, PARCEL 'A' AND PARCEL 'B'. ANY CONVEYANCES OF THE AFORESAID LOTS/PARCELS SHALL BE SUBJECT TO THE EASEMENTS HEREIN RESERVED. WHETHER OR NOT EXPRESSLY STATED IN THE DEED(S) CONVEYING SAID LOT(S)/PARCELS. DEVELOPER SHALL EXECUTE AND DELIVER DEEDS FOR THE EASEMENTS HEREIN RESERVED TO HOWARD COUNTY WITH A METES AND BOUNDS DESCRIPTION OF THE FOREST CONSERVATION AREA. UPON COMPLETION OF THE PUBLIC UTILITIES AND THEIR ACCEPTANCE BY HOWARD COUNTY, AND IN THE CASE OF THE FOREST CONSERVATION EASEMENT(S), UPON COMPLETION OF THE DEVELOPER'S OBLIGATION UNDER THE FOREST CONSERVATION INSTALLATION AND MAINTENANCE AGREEMENT EXECUTED BY THE DEVELOPER AND THE COUNTY, AND THE RELEASE OF DEVELOPER'S SURETY POSTED WITH SAID AGREEMENT, THE COUNTY SHALL ACCEPT THE EASEMENTS AND RECORD THE DEED(S)

22). THIS SUBDIVISION IS SUBJECT TO SECTION 18.122B OF THE HOWARD COUNTY CODE. PUBLIC WATER AND/OR SEWER SERVICE HAS BEEN GRANTED UNDER THE TERMS AND PROVISIONS, THEREOF, EFFECTIVE T.B.D., ON WHICH DATE DEVELOPER AGREEMENT #24-4549-D WAS FILED AND ACCEPTED.

23.) THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS/BUREAU OF ENGINEERING/CONSTRUCTION INSPECTION DIVISION AT 410-313-1880 AT LEAST FIVE (5) WORKING DAYS PRIOR

24.) THE CONTRACTOR SHALL NOTIFY "MISS UTILITY" AT 1-800-257-7777 AT LEAST 48 HOURS PRIOR TO ANY EXCAVATION WORK BEING DONE. 25.) STREET LIGHT PLACEMENT AND TYPE OF FIXTURE AND POLE SHALL BE IN ACCORDANCE WITH THE HOWARD

COUNTY DESIGN MANUAL, VOLUME III (1993) AND AS MODIFIED BY "GUIDELINES FOR STREET LIGHTS IN RESIDENTIAL DEVELOPMENTS (JUNE 1993)." A MINIMUM SPACING OF 20 FEET SHALL BE MAINTAINED BETWEEN ANY STREETLIGHT AND ANY TREE.

26.) TRAFFIC CONTROL DEVICES, MARKINGS AND SIGNING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). ALL STREET AND REGULATORY SIGNS SHALL BE IN PLACE PRIOR TO PLACEMENT OF ANY ASPHALT. 27.) ALL SIGN POSTS USED FOR TRAFFIC CONTROL SIGNS INSTALLED IN THE COUNTY RIGHT-OF-WAY SHALL BE

MOUNTED ON A 2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE POST (14 GAUGE) INSERTED INTO A 2-1/2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE SLEEVE (12 GAUGE)-3' LONG. A GALVANIZED STEEL POLE CAP SHALL BE MOUNTED ON TOP OF EACH POST. 28.) THIS PROJECT IS SUBJECT TO THE AMENDED FIFTH EDITION OF THE SUBDIVISION AND LAND DEVELOPMENT

REGULATIONS AND THE ZONING REGULATIONS EFFECTIVE APRIL 13, 2004. PER SECTION 126(H)(1), PLANNING BOARD APPROVAL IS REQUIRED FOR THE SITE DEVELOPMENT PLAN FOR THIS PROJECT.

APPROVED: DEPARTMENT OF PUBLIC WORKS	
CHIEF, BUREAU OF HIGHWAYS 45	1-9-14 DATE
APPROVED: DEPARTMENT OF PLANNING AND Z	ONING
Hetsleibert	1/23/14 37373 DATE
Chief, Development Engineering Division	/•/5·14/ @ DATE

P:\1915\dwg\Phase 2 Section 1\Major Redline 2\7009_s01.dwg, 12/3/2013 1:52:32 PM

	F WAY ELEV	ATION
R/W PT NO	PESCRIPTION	ELEVATION
110	REBAR & CAP	499.98
308	REBAR & CAP	458.88
309	REBARE CAP	465.00'
30	REBAR & CAP	465.20
318	REBAR & CAP	485.31
319	REBARECAP	465.34'
320	REBAR & CAP	455.48'
321	REBAR & CAP	452.85
322	REBARÉCAP	458.99
400	REBAR & CAP	500.10
401	REBAR & CAP	506.07
402	REBAR & CAP	480.78'
412	CONC. MON	475.96
413	REBARECAP	47462'
414	REBAR & CAP	461.84'
415	REBARECAP	465.62*
428	REBARECAP	4.82.91'
420	REBARECAP	485,19
4.30	REBARECAP	465.96'
43!	GRD RAIL POST	458,44'
432	REBAR & CAP	\$53,22'
453	REBAR & CAP	482.01'
454	REBAR & CAP	16514
465	REBAR & CAP	467.50
456	REBARECAP	124.40'
457	REBAR & CAP	480 PD1'
458	REBARECAP	504.01'
450	REBAR & CAP	503.04'
460	REBARSCAP	505,02'
461	CONC. MON.	506.50'
462	REBAR & CAP	50565'
416	CONC. MON	463.42'
417	REBARECAP	463.26'
426	REBARECAP	463,75'
1 427	REBAR & CAP	464.13
403	REBARS CAP	470.81'
404	REBAR & CAP	479.02

* SEE SHEET NO.5 FOR RECOVERY SKETCHES

AS-BUILT NOTES:

1.) HORIZONITAL DATUM FOR THIS AS-BUT IS BASED ON THE MARYLAND STATE REFERENCE SYSTEM WAD 28 / ADJ 07 AS PROJECTED FROM HO. CO. GEODETIC CONTROL STATIONS 16E1 AND CO12. VERTICAL DATUM FOR THIS AS-BUILT 15 NORTH AMERICAN VERTICAL DATUM NGVD 20 AS PROJECTED FROM THE ABOVE MENTIONED HOWARD COUNTY GEODETIC CONTROL STATIONS.

2) THE INSTRUMENTS USED IN PERFORMING THE AS - BUILT WERE A 5" TOTAL STATION AND PRISM AND RIK GPS.

3.) THIS AS. BUILT WAS PERFORMED BY BENCHMARK ENGINEERING INC.

S AT TURF	VALLEY PHAS	SII
S.F.A.	S.F.D.	
0	0	
41	21	
0	0	
0	58	
0	0	
23	0	
0	0	1
0	0	
123	53	
	S.F.A. 0 41 0 0 0 23 0 0 0	0 0 41 21 0 0 0 58 0 0 23 0 0 0 0 0 0 0

P/O PARCEL 8

MANGIONE ENTERPRISES OF TURF VALLEY, LP

29.) WP-05-074 A REQUEST TO WAIVE SECTION 16.116(a)(2)(II) TO ALLOW GRADING, REMOVAL OF VEGETATIVE COVER AND TREES, PAVING AND NEW STRUCTURES WITHIN 75 FEET STREAM BUFFER FOR THE PURPOSE OF A RETAINING WALL FOR RESORT ROAD AT STATION 39+00 WAS DENIED ON MARCH 10, 2005. THE REMAINING DISTURBANCES IDENTIFIED ON THESE PLANS BY THEIR ENVIRONMENTAL IMPACT ID NUMBER WERE DEEMED AS NECESSARY BY DPZ DURING THE REVIEW OF S-03-01.

30.) THIS PROJECT IS SUBJECT TO THE TRAFFIC STUDY PREPARED BY THE TRAFFIC GROUP, INC. UNDER S-86-13, AND UPDATED IN MARCH 2004.

31.) THE VILLAGES AT TURF VALLEY SUBDIVISION (PHASES 1-4) CONSTITUTED 241 TOTAL UNITS, WHICH MET THE SKETCH PLAN MILESTONE DATE OF JANUARY 1, 2001 THROUGH JUNE 30, 2002 FOR BOTH PHASE IVA (131 UNITS) & IVB (110 UNITS) AS ESTABLISHED BY THE REVISED PHASING PLAN DATED JUNE 21, 2000. UNDER P-06-013, 42 CONDOMINIUM UNITS THAT WERE APPROVED WERE USED FOR OAKMONT AT TURF VALLEY (F-02-082). THESE 42 CONDOMINIUM UNITS WERE NOT PREVIOUSLY INCLUDED WITH THE OAKMONT AT TURF VALLEY (F-02-82) PLANS, IN ORDER TO RECEIVE BUILDING ALLOCATIONS, THESE 42 CONDOMINIUM UNITS WERE SHOWN AND APPROVED ON THE PRELIMINARY PLAN FOR THE VILLAGES AT TURF VALLEY (P-06-013). THE SECOND AMENDMENT TO THE TURF VALLEY MULTI-USE FINAL DEVELOPMENT PLAN WAS RECORDED ON NOVEMBER 2007, INCREASING THE PROJECTED UNITS IN THE OAKMONT AT TURF VALLEY AREA FROM 150 TO 200. AS A THOSE 42 UNITS ARE NO LONGER A PART OF THE VILLAGES AT TURF VALLEY WHICH LEAVES UNIT TOTAL AT 199. HOWEVER, WITH THE APPROVAL OF WP-08-009 AN ADDITIONAL 21 UNITS WERE ADDED TO THE VILLAGES AT TURF VALLEY. THE FINAL UNIT TOTAL FOR THIS SUBDIVISION COMES TO 220.

32.) WP-08-009, A WAIVER PETITION TO SECTION 16.145(a) AND 16.146 (a) WHICH REQUIRES A SUBMISSION OF A SKETCH PLAN AND PRELIMINARY PLAN. RESPECTIVELY TO ALLOW THE ADDITION OF 21 UNITS TO THIS PROJECT WAS APPROVED ON 12-12-2007 WITH THE FOLLOWING CONDITIONS:

1. PETITIONER SHALL COMPLY WITH ALL RELEVANT PARKING REGULATIONS AT THE TIME OF SITE DEVELOPMENT PLAN SUBMISSION FOR ALL DEVELOPMENT PROPOSED ON LOT 203 OF "VILLAGES AT TURF VALLEY, PHASE 2" (F-08-084)

2. PETITIONER SHALL COMPLY WITH ALL RELEVANT STORMWATER MANAGEMENT REGULATIONS AT THE TIME OF SITE DEVELOPMENT PLAN SUBMISSION FOR PROPOSED DEVELOPMENT OF LOT 203 OF "VILLAGES AT TURF VALLEY, PHASE 2" (F-08-084).

33.) OPEN SPACE DEDICATION FOR LOTS 204-207:

THE OPEN SPACE SHOWN HEREON IS HEREBY DEDICATED TO A PROPERTY OWNERS ASSOCIATION FOR THE RESIDENTS OF THIS SUBDIVISION AND RECORDING REFERENCES OF THE ARTICLES OF INCORPORATION AND RESTRICTIONS ARE SHOWN HEREON. 34.) PRIOR TO GRADING PERMIT APPLICATION. THE PROJECT SHALL COMPLY WITH THE REQUIREMENTS OF

SECTION 16.129 OF THE HOWARD COUNTY CODE. 35.) NOISE WALLS SHALL BE PRIVATELY OWNED AND MAINTAINED.

36.) THE OWNER/DEVELOPER, PRIOR TO GRADING PERMIT APPLICATION, SHALL OBTAIN A LETTER FROM COLUMBIA GAS AUTHORIZING ACTIVITIES PROPOSED WITHIN THEIR EASEMENT. A COPY OF THIS LETTER SHALL BE PROVIDED TO THE DEPARTMENT OF PLANNING AND ZONING AND DEPARTMENT OF INSPECTIONS, LICENSES AND PERMITS.



PARCEL 15

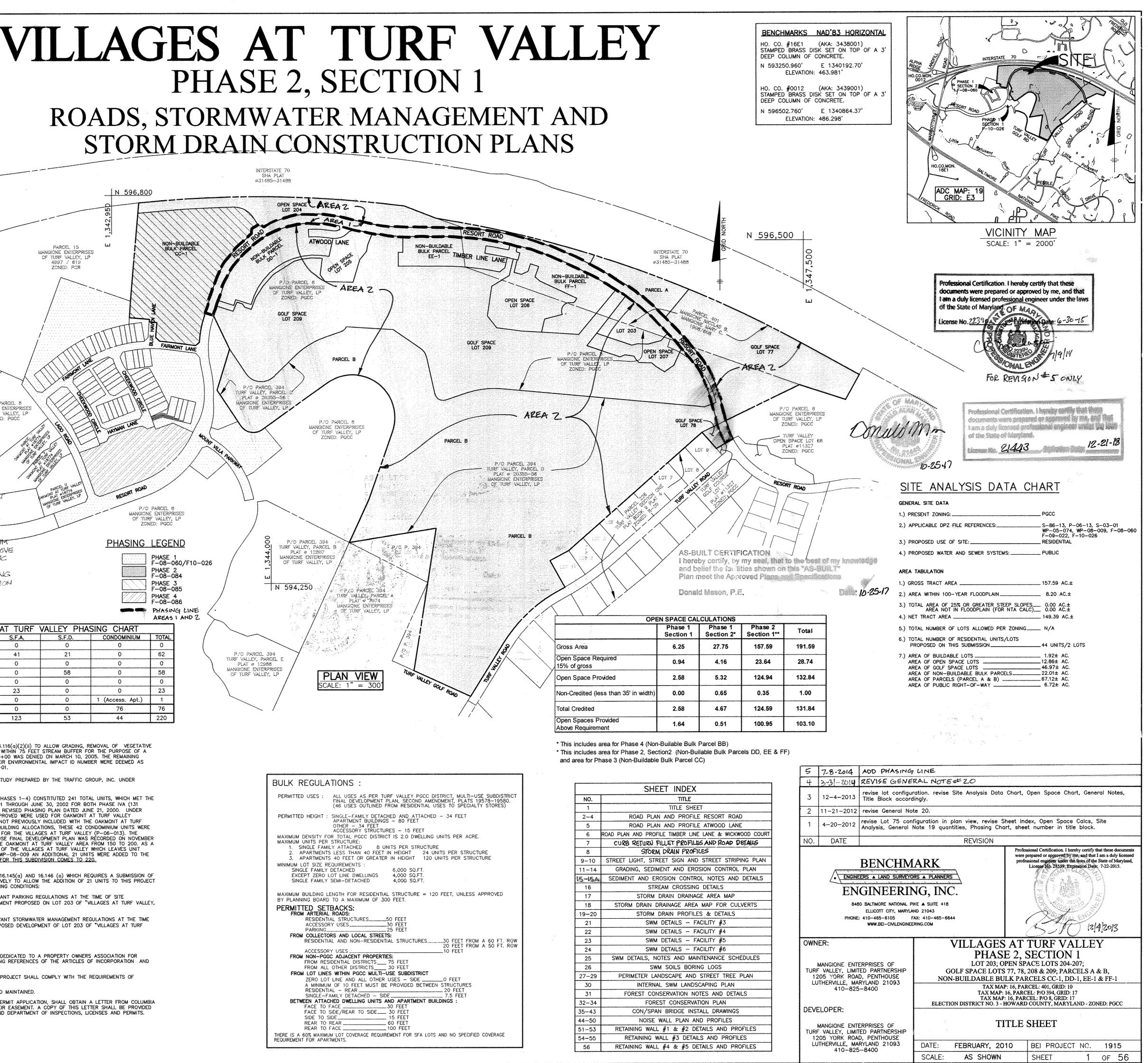
OF TURF VALLEY, LP

4997 / 619

ZONED: POR

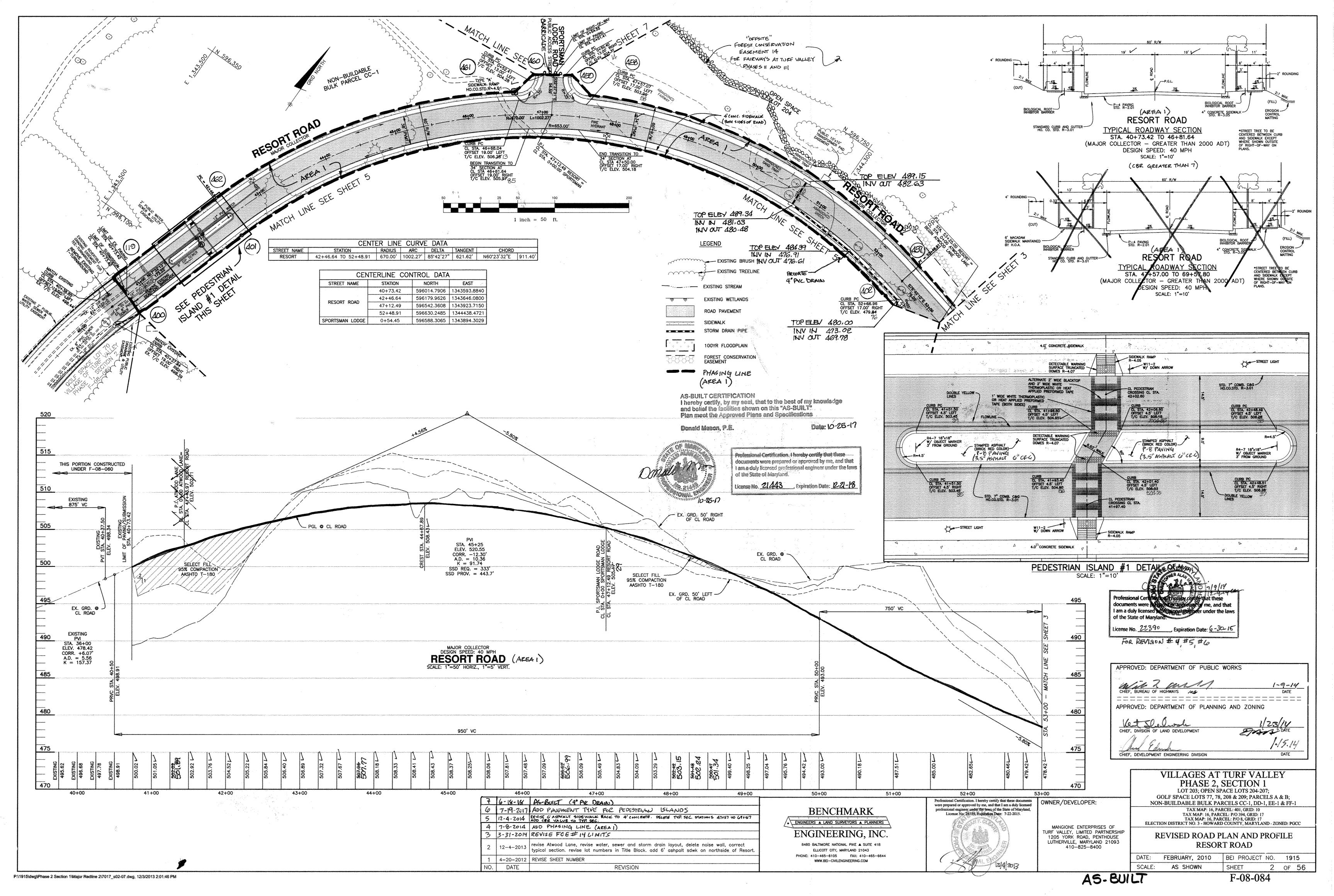
ANGIONE ENTERPRISES

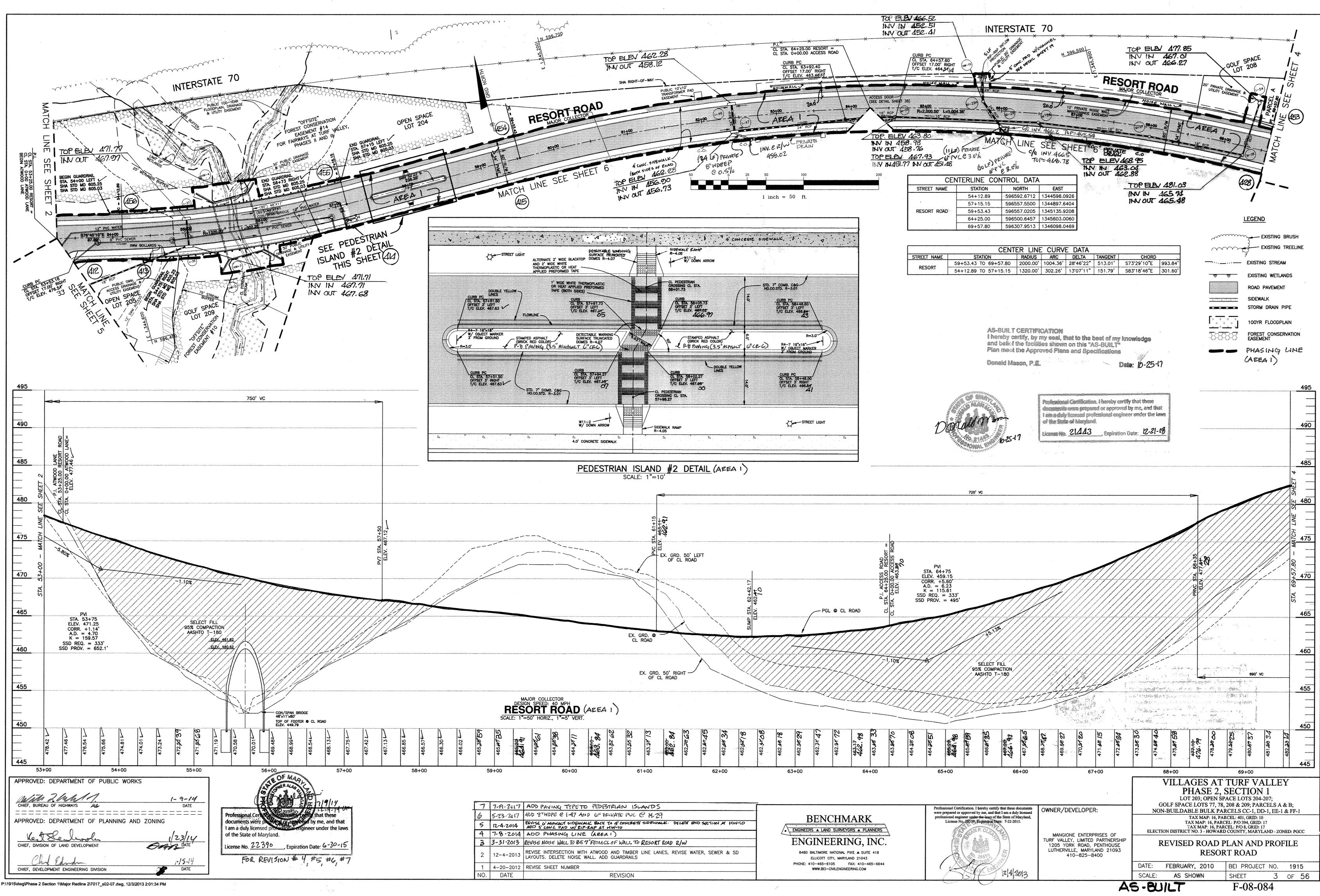
PHASE 2, SECTION 1



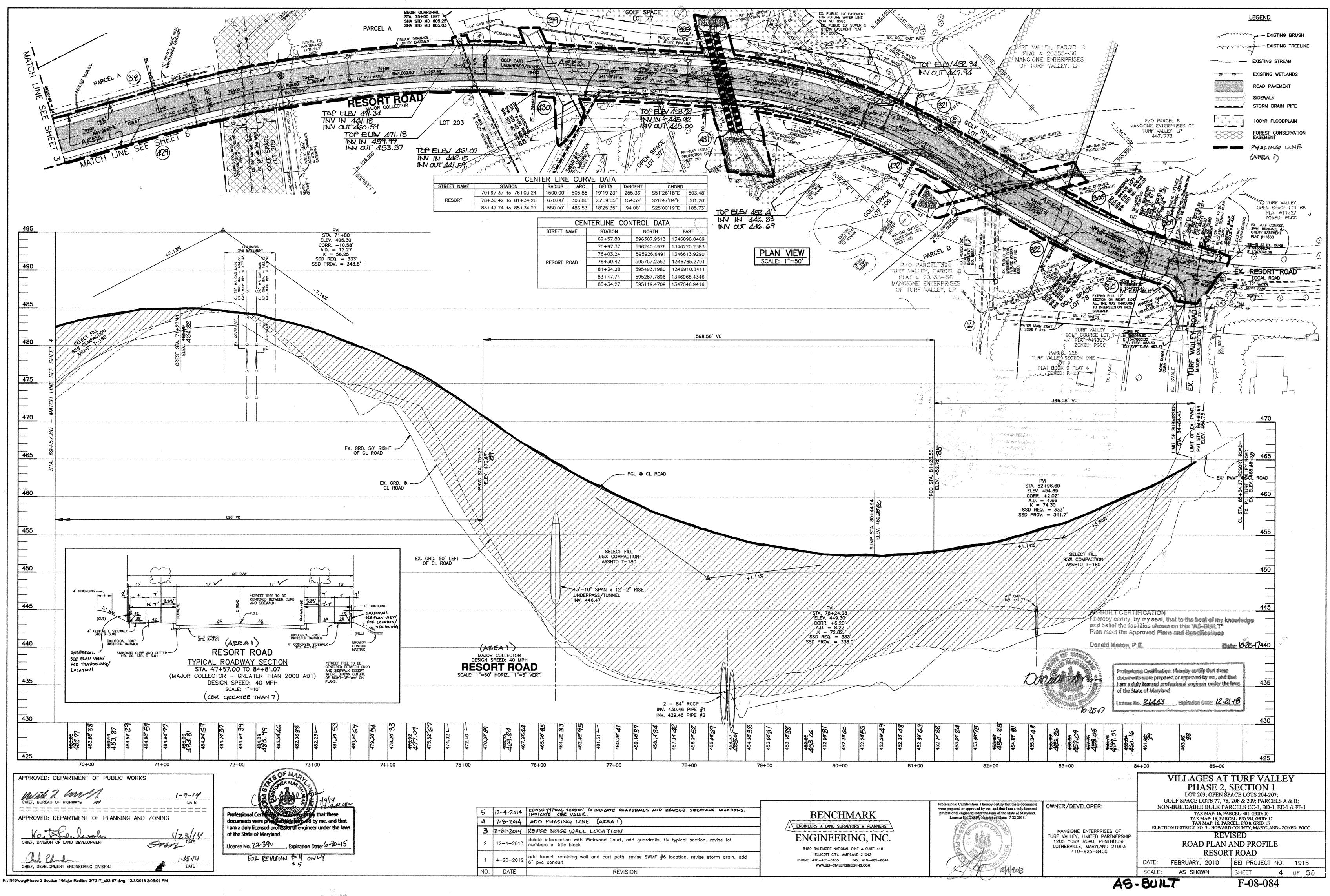
AS-BUILT

F-08-084



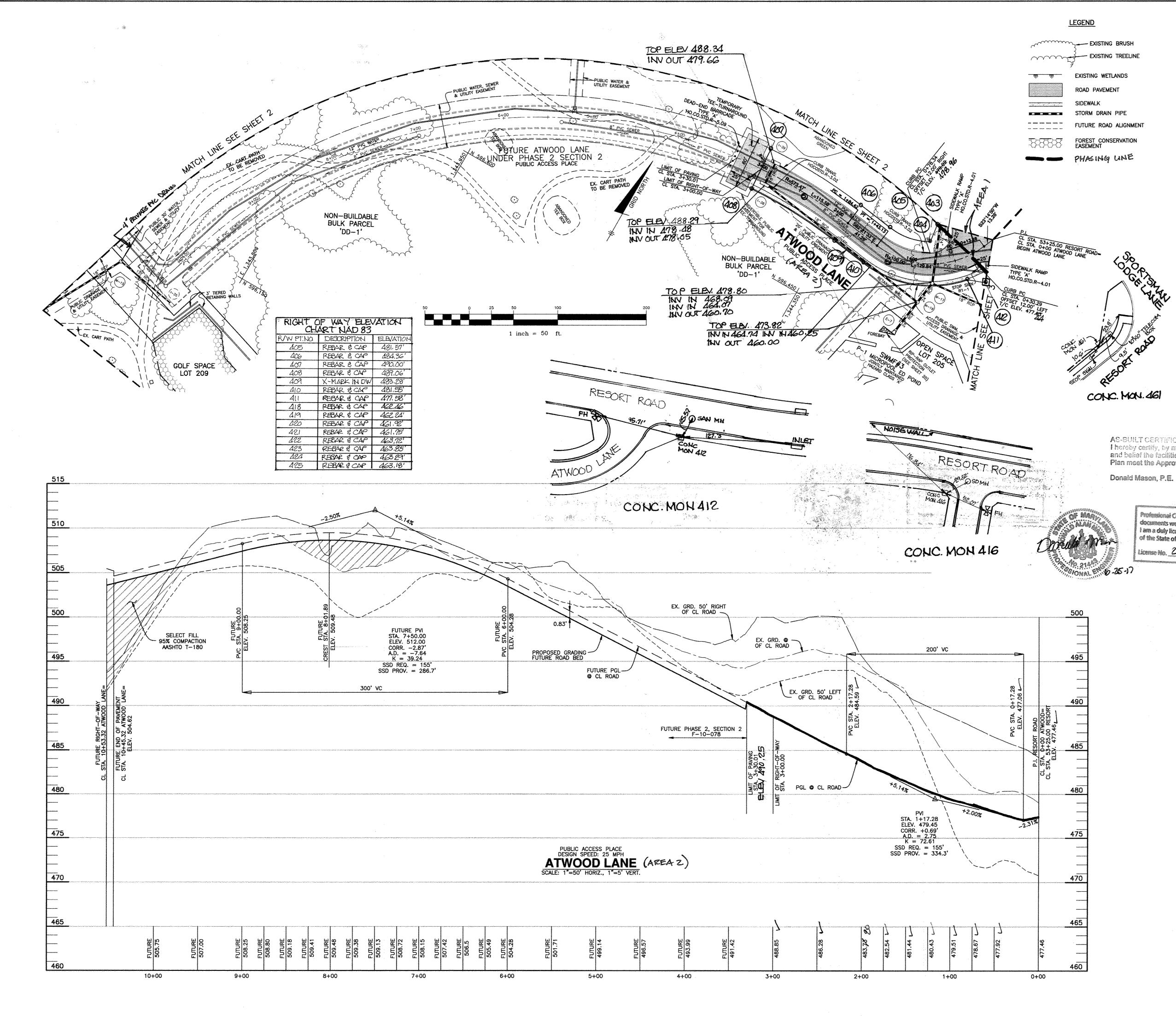


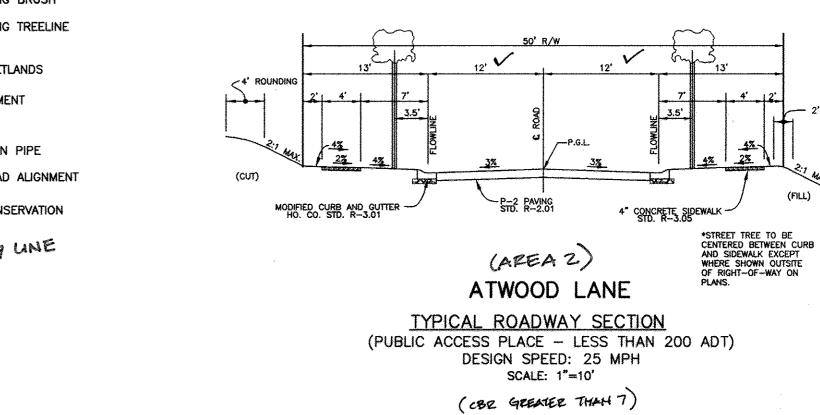
17	7-19-2017	ADD PAVING TYPETO PEDESTRIAN ISLANDS	1999 - Anno 1999 - Anno 1997
6	5-23-2017	ADD 8"HOPE @ 1.47 AND G" PRIVATE PUC @ M-29	BENCHMARK
5	12-4-2014	REVISE & ASPHALT SIDEWALL BACK TO A CONCRETE SIDEWALK. DELETE END SECTION AT HW-10 ADD 5' LONG PAD W/ RIP-RAP AT HW-10	Vauahandrimahaanhanhanhandrimahandrimahand
4	7-8-2014	ADD PHASING LINE (AREA I)	ENGINEERS & LAND SURVEYORS & PLANNERS
З	3-31-2013	REVISE NOISE WALL TO BE 7'FROM CLOF WALL TO RESORT ROAD R/W	ENGINEERING, INC.
2	12-4-2013	REVISE INTERSECTION WITH ATWOOD AND TIMBER LINE LANES, REVISE WATER, SEWER & SD LAYOUTS. DELETE NOISE WALL. ADD GUARDRAILS	8480 BALTIMORE NATIONAL PIKE ▲ SUITE 418 ELLICOTT CITY, MARYLAND 21043
1	4-20-2012	REVISE SHEET NUMBER	PHONE: 410-465-6105 FAX: 410-465-6644 WWW.BEI-CMLENGINEERING.COM
NO.	DATE	REVISION	



te se set

5	12-4-2014	REVISE TYPICAL SECTION TO INDICATE GUARDRAILS AND REVISED SIDEWALK LOCATIONS. INDICATE COR VALUE. ADD PHASING LINE (AREA 1)	BENCHMARK
3	3-31-2014	REVISE NOISE WALL LOCATION	- ENGINEERS A LAND SURVEYORS A PLANNERS
2	12-4-2013	delete intersection with Wickwood Court, add guardrails, fix typical section. revise lot numbers in title block	ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE A SUITE 418
1	4-20-2012	add tunnel, retaining wall and cart path. revise SWMF #6 location, revise storm drain. add 6" pvc conduit	ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644 WWW.BEI-CIMLENGINEERING.COM
NO.	DATE	REVISION	





		ER LINE	· · · · · · · · · · · · · · · · · · ·				
STREET NAME	STATION	RADIUS	ARC	DELTA	TANGENT	CHORD	
	0+13.28 TO 1+43.13	100.00'	129.84'	74'23'37"	75.90'	S60'26'38"W	120.91
ATWOOD LANE	1+84.42 TO 3+00.00	573.47'	115.58'	11'23'52"	57.99'	N88'07'59"W	115.39

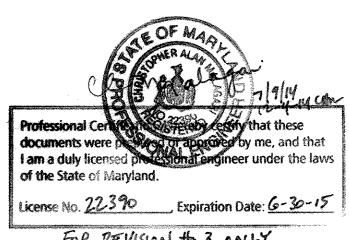
CENTERLINE CONTROL DATA							
STREET NAME	STATION	NORTH	EAST				
	0+00	596612.8120	1344512.5386				
ATWOOD LANE	0+13.28	596600.6066	1344507.2954				
ATTOOD LANE	1+43.13	596540.9644	1344402.1180				
	1+84.42	596546.4549	1344361.1907				
	3+00.00	596550.2142	1344245.8669				

AS-BUILT CERTIFICATION

I hereby certify, by my seal, that to the best of my knowledge and belief the facilities shown on this "AS-BUILT" Plan meet the Approved Plans and Specifications

Dete: 10-25-17

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the lates of the State of Maryland. License No. 21443 Expiration Date: 12-21-18



2' ROUNDING

(FILL)

			FOR REVISIO	44 Leo #	3 ONLY 4	
	APPR	OVED: DEI	PARTMENT OF PU	IBLIC 1	WORKS	
	CHIEF	BUREAU OF	2 Unit	1.	1- 9	DATE
		OVED: DEF	PARTMENT OF PL		G AND ZONING	
	CHIEF	bitslon of	Land DEVELOPMENT	*****	1/2	3/14 DATE
	CHIEF	DEVELOPMEN	IT ENGINEERING DIVISION	ł	eren :	1.15.14 DATE
5 6-18-18	BS-BULT (
4 12-4-2014	ADD CBR YALL					
3 7-8-2014	ADD PHASING	LINE	r man a an ainm Thosacha an ann acan mac an Idrichean Anna Anna Anna Anna Anna Anna Anna A		*******	en sign for en die er fan Hang af de Halg de fal de Serve de fan ei yn oegener waarde af an fan serve
2 12-4-2013	revise Atwood drain layout. r			le. rev	ise water, sewer	and storm
1 4-20-2012	REVISE SHEET	NUMBER				
NO. DATE			REVISION		עריין איז	ŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢ
EN 84	BENCHM EERS A LAND SURVEYO GINEERI 30 BALTIMORE NATIONAL P ELLICOTT CITY, MARYLA 410-465-6105 F WWW.BEI-CIMILENGINEE	NG, IN KE & SUITE 416 ND 21043 AX: 410-465-6	C .	were p	sional Certification. I hereby certi repared or approved by me, and t ssional engineer under the laws of License No. 28559. Expiration I	hat I am a duly licensed the State of Maryland,
OWNER:			PHASI LOT 203; OI	E 2, S PEN SPA	TURF VALL SECTION 1 ACE LOTS 204-207;	
TURF VALLEY, LIMI 1205 YORK ROA LUTHERVILLE, MA 410-825	D, PENTHOUSE RYLAND 21093		N-BUILDABLE BU TAX MAP TAX MAP:	LK PAR 16, PAR	208 & 209; PARCELS CELS CC-1, DD-1, EE CEL: 401, GRID: 10 EL: P/O 394, GRID: 17 CEL: P/O 8, GRID: 17	· ·
DEVELOPER:			TION DISTRICT NO. 3 - H	IOWARD	COUNTY, MARYLAND - 2	· · · · · · · · · · · · · · · · · · ·
MANGIONE ENT TURF VALLEY, LIMI 1205 YORK ROA	ED PARTNERSHIP	I			LAN AND PRO	FILE
LUTHERVILLE, MA	RYLAND 21093	DATE:	SEPTEMBER, 20	13	BEI PROJECT NO.	1915
410-020		SCALE.	AS SHOWN			OF 56

AS SHOWN

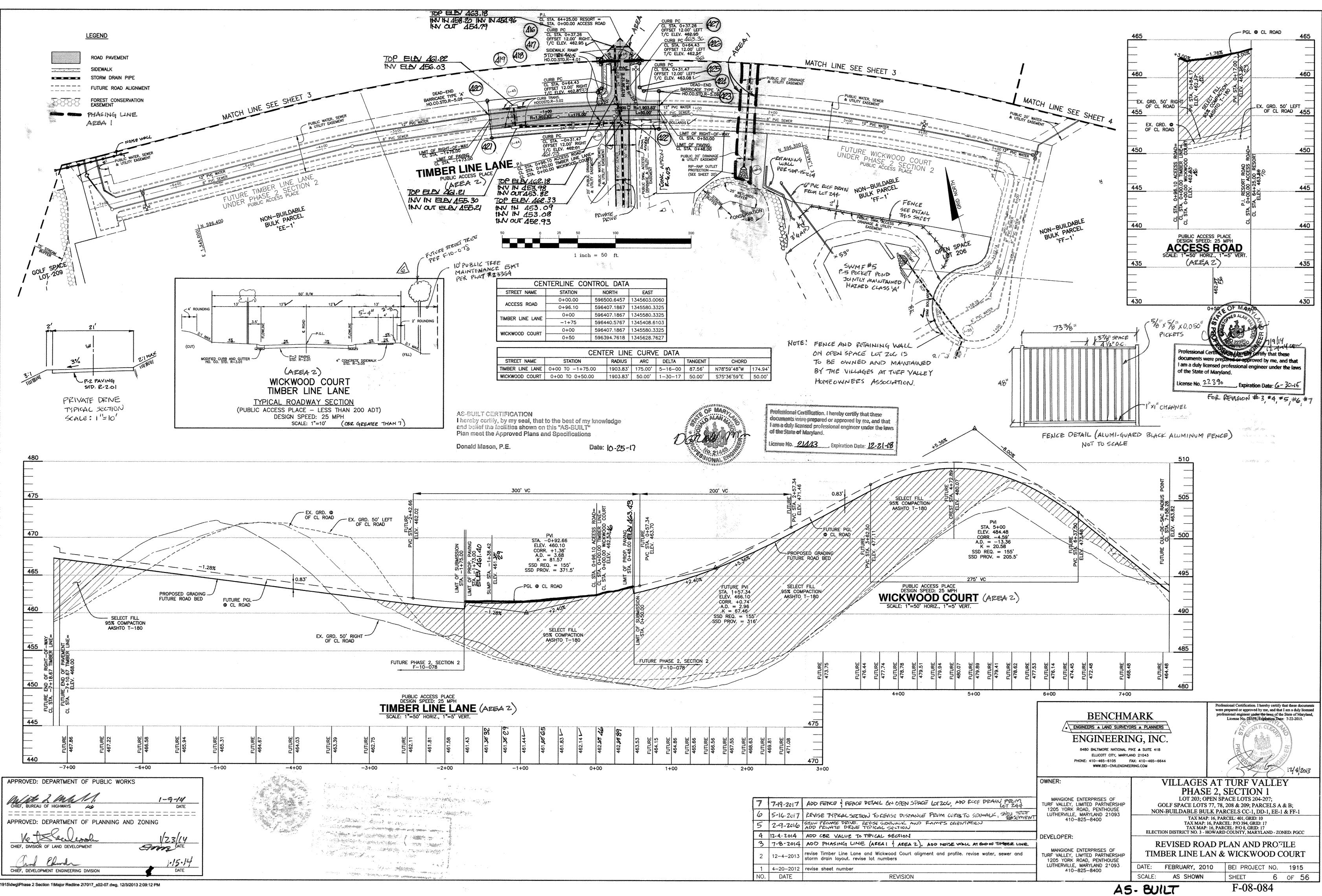
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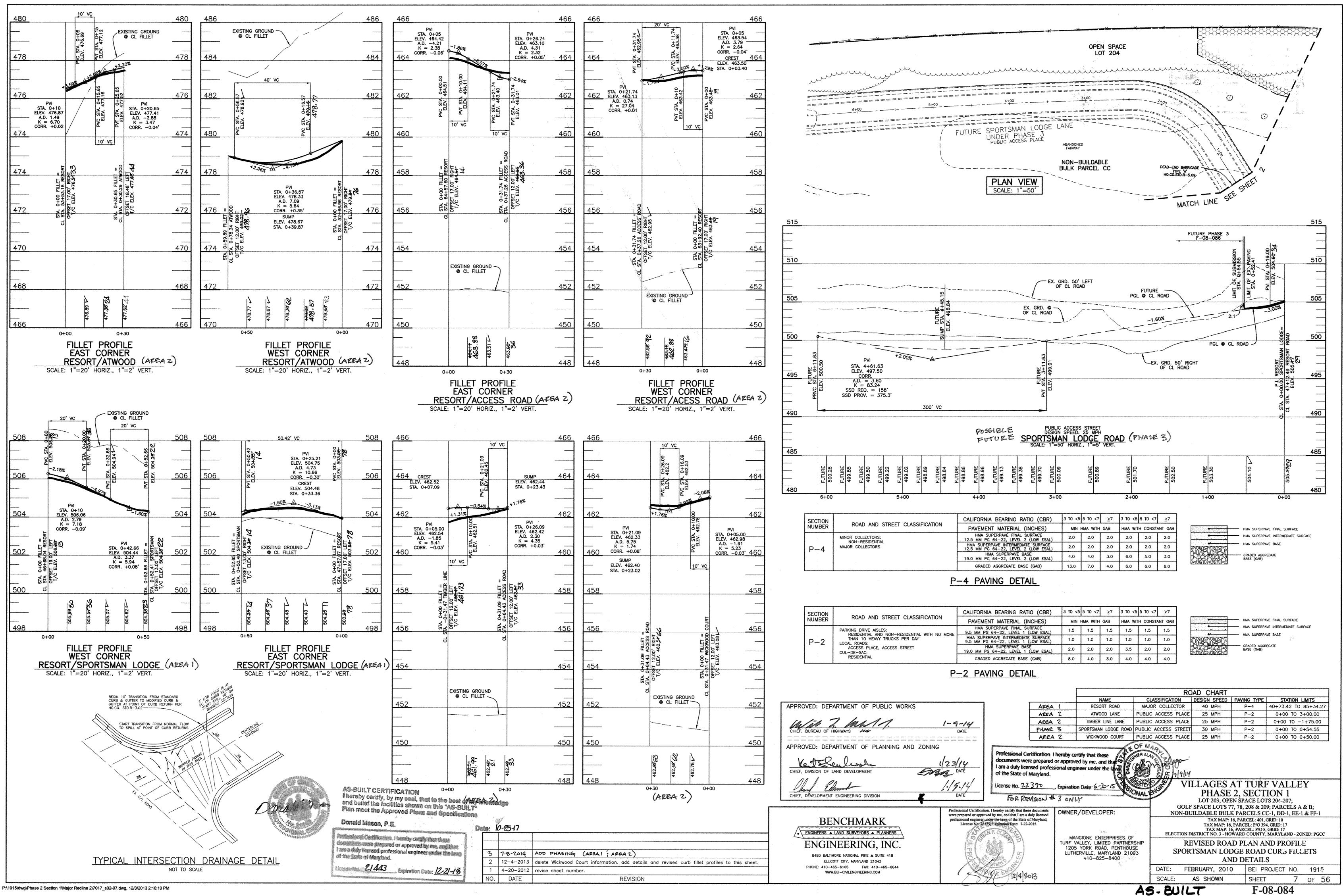
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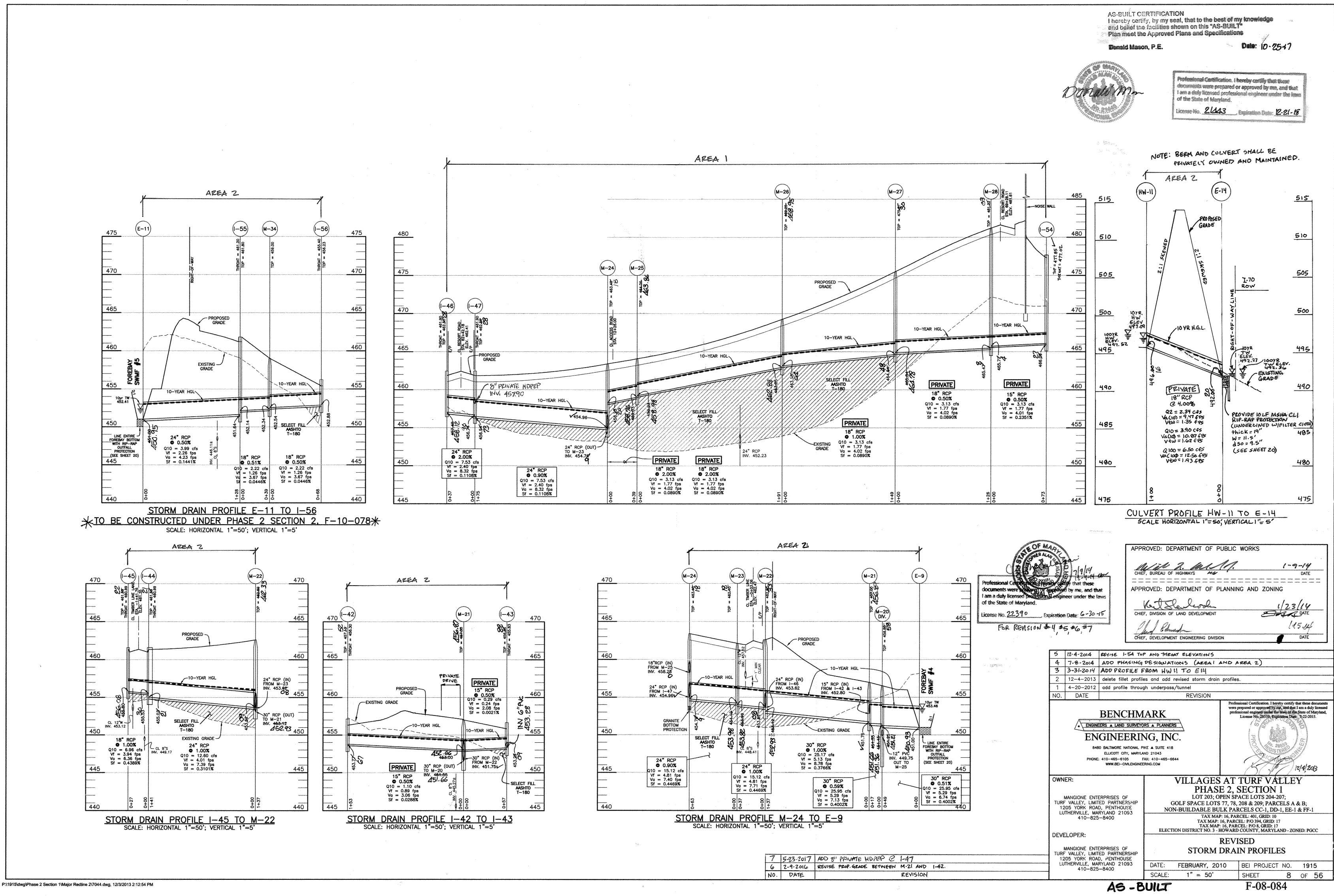
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5 OF 56

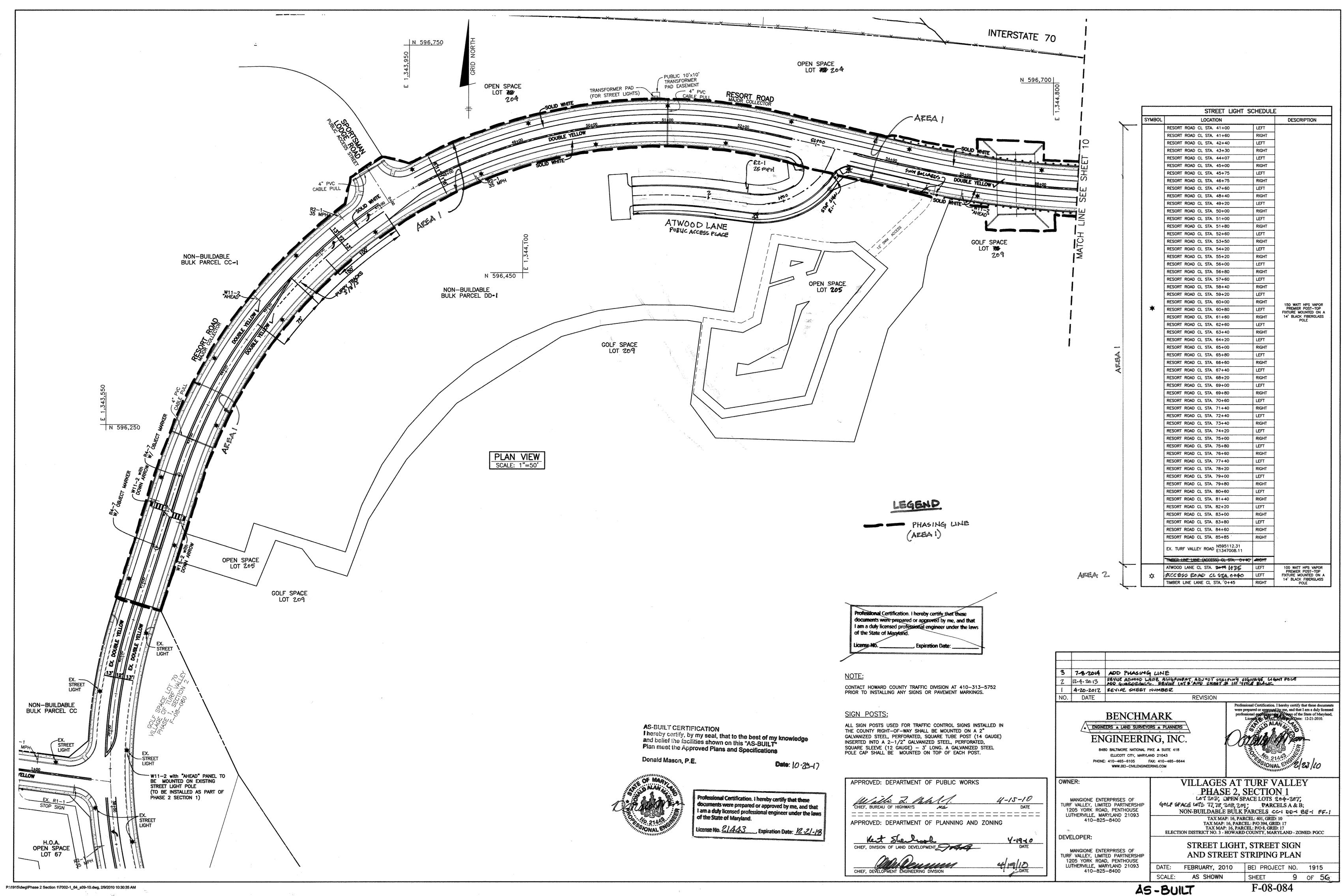


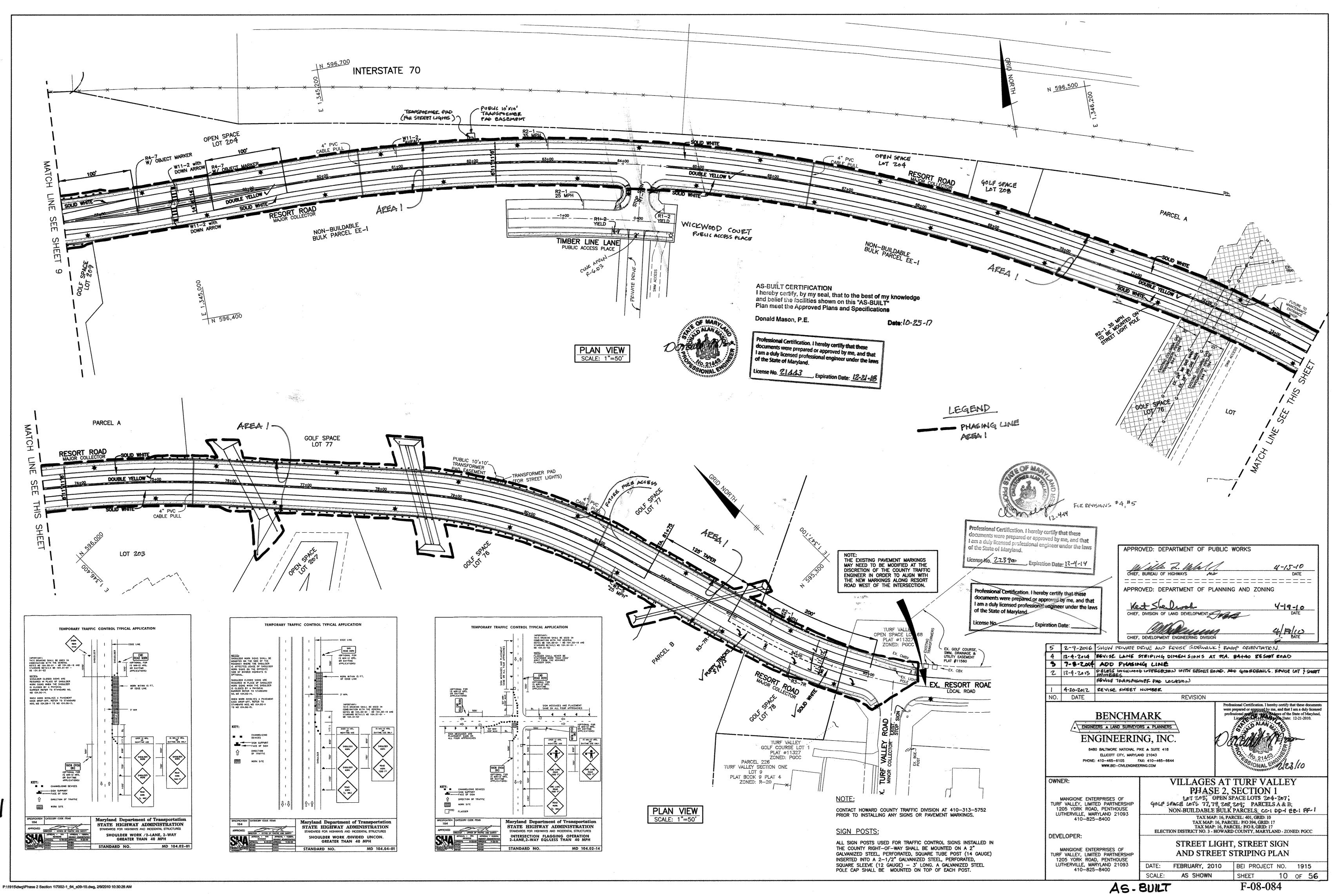
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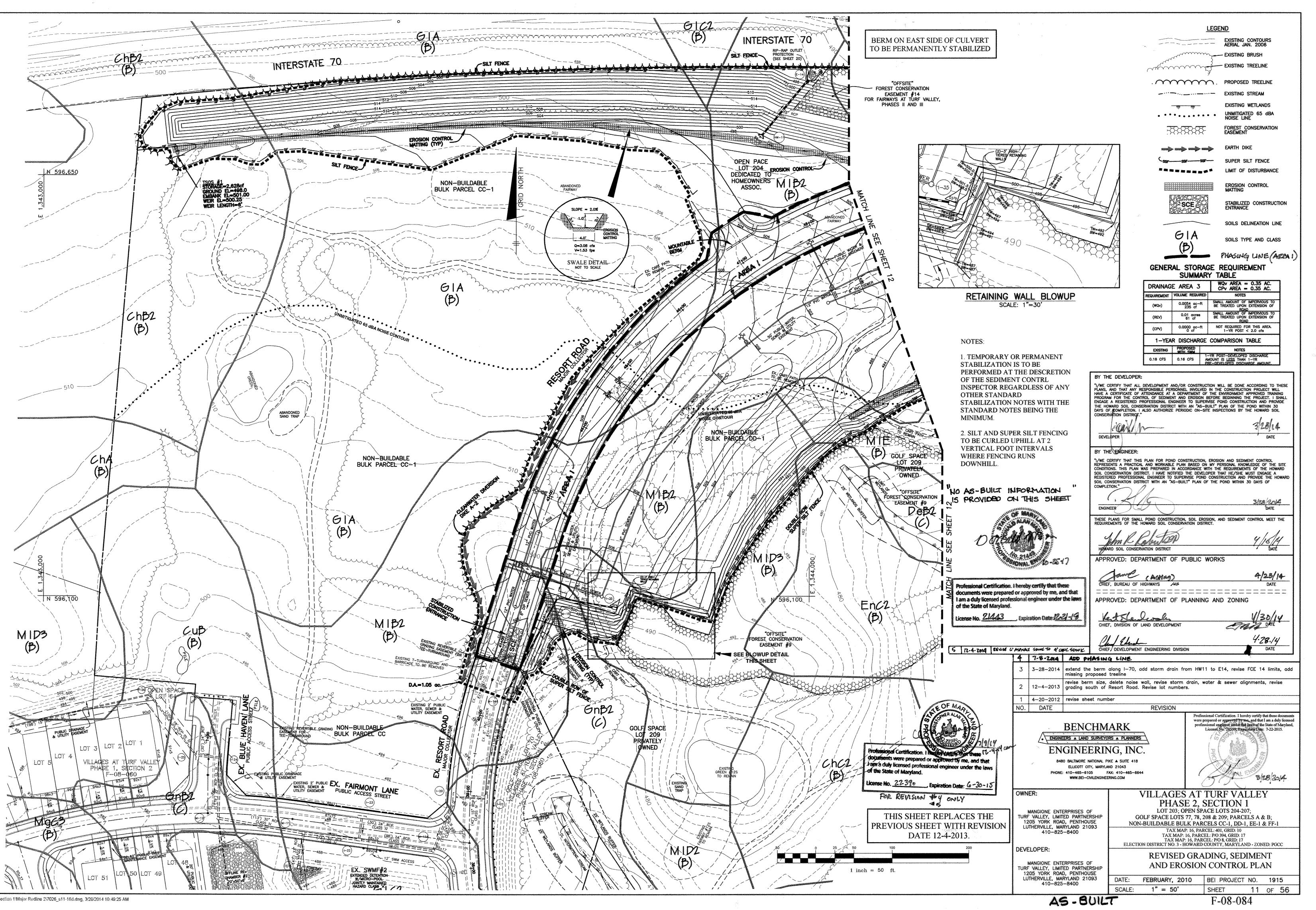


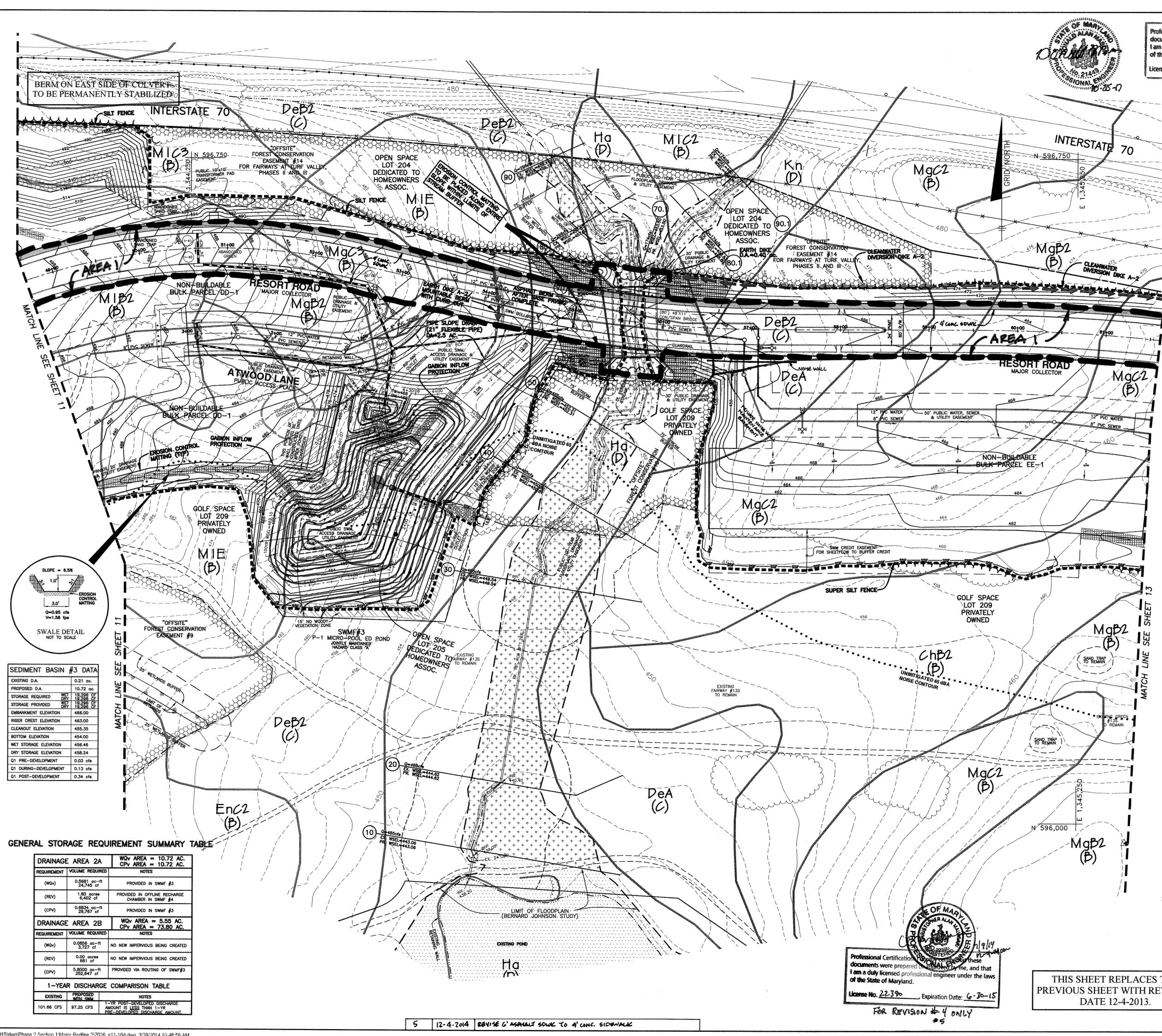


NO.	DATE		REVISION	
6	2-9-2016	REVISE PROP. GRADE BETWEEN	M-21 AND	1-
7		ADD 8" PRIVATE HDREP C		



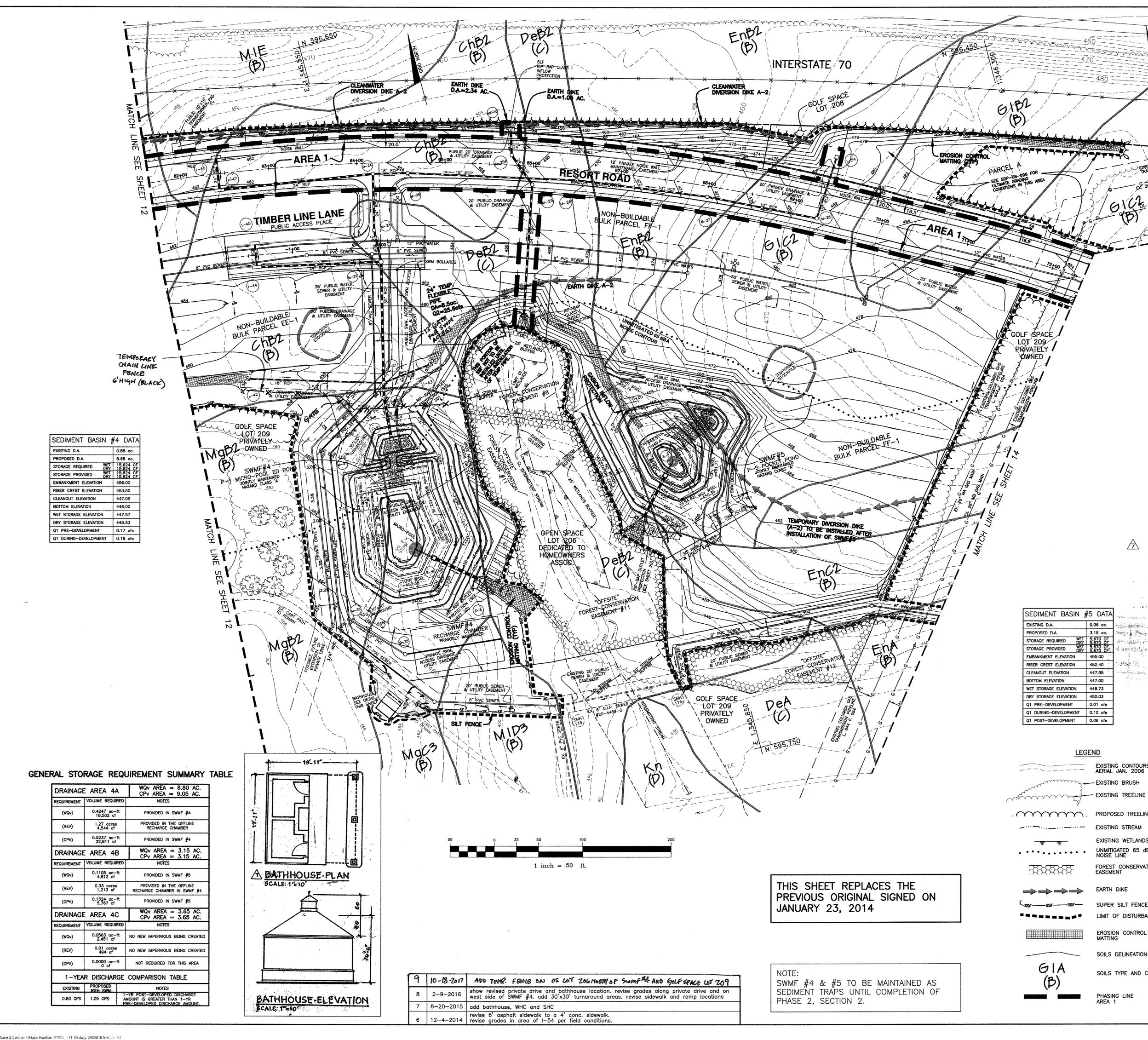






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rofessional Certification. I heroby certify that these LEGEND focuments were prepared or approved by me, and that I am a duly licensed professional engineer under the laws EXISTING CONTOURS AERIAL JAN, 2006 and were and show and a start and of the State of Maryland. License No. 21443 Expiration Date: 12-21-19 ---- EXISTING BRUSH . ______ PROPOSED TREELINE EXISTING STREAM EXISTING WETLANDS UNMITIGATED 65 dBA NOISE LINE 70 FOREST CONSERVATION EASEMENT -773-773-EARTH DIKE SUPER SILT FENCE LIMIT OF DISTURBANCE EROSION CONTROL MATTING SOILS DELINEATION LINE GIA SOILS TYPE AND CLASS (B) PHASING LINE (AREA 1) AS-BUILT CERTIFICATION I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN WAS CONSTRUCTED AS SHOWN ON THE "AS-BUILT" PLANS AND MEETS THE APPROVED PLANS AND SPECIFICATIONS. PE NO. 2143 Donald Man Λαζ DATE 10-25-17 CERTIFY MEANS TO STATE OR DECLARE A PROFESSIONAL OPINION BASED UPON ONSITE INSPECTIONS AND MATERIAL TESTS WHICH ARE CONDUCTED DURING CONSTRUCTION. THE ONSITE INSPECTIONS AND MATERIAL TESTS ARE THOSE INSPECTIONS AND TESTS DEEMED SUFFICIENT AND APPROPRIATE BY COMMONLY ACCEPTED ENGINEERING STANDARDS. CERTIFY DOES NOT MEAN OR IMPLY A GUARANTEE BY THE ENGINEER NOR DOES AN ENGINEER'S CERTIFICATION RELIEVE ANY OTHER PARTY FROM MEETING REQUIREMENTS IMPOSED BY CONTRACT, EMPLOYMENT, OR OTHER MEANS, INCLUDING MEETING COMMONLY ACCEPTED INDUSTRY PRACTICES. 2" PVC WATER 8" PVC SEWER NOTES: BY THE DEVELOPER: "I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE ACCORDING TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE . TEMPORARY OR PERMANENT STABILIZATION IS TO BE PERFORMED AT THE DESCRETION THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTIONS BY THE HOWARD SOIL OF THE SEDIMENT CONTRL CONSERVATION DISTRICT." INSPECTOR REGARDLESS OF ANY OTHER STANDARD 3/28/14 100x10 STABILIZATION NOTES WITH THE DEVELOPER DATE STANDARD NOTES BEING THE MINIMUM. BY THE ENGINEER: "I/WE CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD 2. SILT AND SUPER SILT FENCING TO BE CURLED UPHILL AT 2 SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD VERTICAL FOOT INTERVALS SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF WHERE FENCING RUNS COMPLETION." DOWNHILL. 1 ment 3/28/2014 ENGINEER -*ADATE* THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION, AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. 4/18/14 DATE MgB2 HOWARD SOIL CONSERVATION DISTRICT (\mathbb{B}) APPROVED: DEPARTMENT OF PUBLIC WORKS SAND TRAP 4/28/H (ACHina) CHIEF, BUREAU OF HIGHWAYS APPROVED: DEPARTMENT OF PLANNING AND ZONING EK 4/30/1Y Ching DATE CHIEF, DIVISION OF LAND DEVELOPMENT · 1 Ledand 4.28.14 CHIEF, DEVELOPMENT ENGINEERING DIVISION DATE -----4 7-8-2014 ADD NOISE WALL AT STATIONS 56+50 TO 57450 ALONG RESOLT ROAD. BREAK OUT RESOLT ROAD RIGHT-OF-WAT AND CALL IT AREA 1 3-28-2014 extend berm along 1-70, revise FCE 14 limits. revise Atwood Lane, water, sewer storm drain layout. delete noise wall, add retaining walls by 12-4-2013 SWMF #3. revise grading. revise lot numbers. 4-20-2012 revise sheet number DATE NO. REVISION rofessional Certification. I hereby certify that these document were prepared or approved by me, and that I am a duly license professional engineer under the laws of the State of Maryland License No. 28539, Expiration Date: 7-22-2015. BENCHMARK MaB2 ENGINEERS & LAND SURVEYORS & PLANNERS (**B**) ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE & SUITE 418 226 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644 WW.BEI-CIVILENGINEERING.COM Y MALS! 328/2014 **OWNER:** VILLAGES AT TURF VALLEY PHASE 2, SECTION MANGIONE ENTERPRISES OF TURF VALLEY, LIMITED PARTNERSHIP 1205 YORK ROAD, PENTHOUSE LOT 203; OPEN SPACE LOTS 204-207; GOLF SPACE LOTS 77, 78, 208 & 209; PARCELS A & B; NON-BUILDABLE BULK PARCELS CC-1, DD-1, EE-1 & FF-1 LUTHERVILLE, MARYLAND 21093 TAX MAP: 16, PARCEL: 401, GRID: 10 410-825-8400 TAX MAP: 16, PARCEL: P/O 394, GRID: 17 TAX MAP: 16, PARCEL: P/O 8, GRID; 17 ELECTION DISTRICT NO. 3 - HOWARD COUNTY, MARYLAND - ZONED: PGCC THIS SHEET REPLACES THE **DEVELOPER:** PREVIOUS SHEET WITH REVISION GRADING, SEDIMENT AND MANGIONE ENTERPRISES OF TURF VALLEY, UMITED PARTNERSHIP EROSION CONTROL PLAN DATE 12-4-2013. 1205 YORK ROAD, PENTHOUSE LUTHERVILLE, MARYLAND 21093 FEBRUARY, 2010 DATE: BEI PROJECT NO. 1915 410-825-8400 1" = 50' SCALE: SHEET 12 OF 56 AS-BUILT F-08-084



31915/dwgiPhase 2 Section 11Major Redline 207035 - 11-16.dwg, 2/8/2016 9:40 35 (14-

EnR TERSTATE /7C MATCH LINE SEE SHEET 14 Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 21443 _____ Expiration Date: 12-21-18 AS-BUILT CERTIFICATION I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN WAS CONSTRUCTED AS SHOWN ON THE "AS-BUILT" PLANS AND MEETS THE APPROVED PLANS AND SPECIFICATIONS. PE NO. 21443 Sonald Mm DATE 10-25-17 ENGINEER CERTIFY MEANS TO STATE OR DECLARE A PROFESSIONAL OPINION BASED UPON ONSITE INSPECTIONS AND MATERIAL TESTS WHICH ARE CONDUCTED DURING CONSTRUCTION. THE ONSITE INSPECTIONS AND MATERIAL TESTS ARE THOSE INSPECTIONS AND TESTS DEEMED SUFFICIENT AND APPROPRIATE BY COMMONLY ACCEPTED ENGINEERING STANDARDS. CERTIFY DOES NOT MEAN OR IMPLY A GUARANTEE NOTES: BY THE ENGINEER NOR DOES AN ENGINEER'S CERTIFICATION RELIEVE ANY OTHER PARTY FROM MEETING REQUIREMENTS IMPOSED BY CONTRACT, EMPLOYMENT, OR OTHER MEANS, INCLUDING MEETING COMMONLY ACCEPTED INDUSTRY PRACTICES. 1. TEMPORARY OR PERMANENT STABILIZATION IS TO BE BY THE DEVELOPER: PERFORMED AT THE DESCRETION "I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE ACCORDING TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF, COMPLETION. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTIONS BY THE HOWARD SOIL CONSERVATION DISTRICT." OF THE SEDIMENT CONTRL INSPECTOR REGARDLESS OF ANY OTHER STANDARD STABILIZATION NOTES WITH THE STANDARD NOTES BEING THE 2/9/16 MINIMUM. WW DEVELOPER DATE 2. SILT AND SUPER SILT FENCING TO BE CURLED UPHILL AT 2 BY THE ENGINEER: VERTICAL FOOT INTERVALS 1/WE CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWAR WHERE FENCING RUNS DOWNHILL SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF 3. THE OWNER/DEVELOPER SHALL COMPLETION." SUBMIT APPLICATION TO DILP 2-9-16 a malazari FOR A BUILDING PERMIT FOR ENGINEER - CHRIS MALAGARI, P.E. # 22390 DATE PROPOSED BATHHOUSE WITHIN ONE YEAR FROM THE DATE OF THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION, AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. APPROVAL OF THIS REDLINE **REVISION.** 4. NO PARKING IS REQUIRED FOR THE PROPOSED BATHHOUSE. 0.06 ac. PPROVED: DEPARTMENT OF PUBLIC WORKS 5. THE CONTRACTOR SHALL 3.15 ac. 2/24/2016 TRUme CONTACT MISS UTILITY CHIEF, BUREAU OF HIGHWAYS DATE 455.00 (1-800-257-7777) AT LEAST 12 452.40 HOURS BEFORE START OF 447.95 APPROVED: DEPARTMENT OF PLANNING AND ZONING CONSTRUCTION OF THE 447.00 BATHHOUSE. 448.73 Vetthelist 3-7-16 450.03 CHIEF, DIVISION OF LAND DEVELOPMENT 0.01 cfs 2.29.16 CHIEF, DEVELOPMENT ENGINEERING DIVISION DATE 5 7-8-2014 add phasing line LEGEND 4 3-31-2014 revise dimension of noise wall to curb to be 20 feet. EXISTING CONTOURS AERIAL JAN. 2006 shift Timber Line Lane, delete intersection of Wickwood Court and Resort Road. revise mass 3 12-4-2013 grading, revise lot numbers, revise storm drain, water & sewer alignments. delete noise wall EXISTING BRUSH 2 8-22-2013 add retaining wall #3. 1 4-20-2012 revise match line location with sheet 14. revise sheet number NO. DATE REVISION PROPOSED TREELINE Professional Certification. I hereby certify that these document were prepared or approved by me, and that I am a duly licensed EXISTING STREAM BENCHMARK professional engineer under the laws of the State of Maryland License No. 22490 P ion Date: 6-30-2017 EXISTING WETLANDS ENGINEERS & LAND SURVEYORS & PLANNERS UNMITIGATED 65 dBA NOISE LINE ENGINEERING, INC. FOREST CONSERVATION EASEMENT 8480 BALTIMORE NATIONAL PIKE A SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644 WWW.BEI-CIVILENGINEERING.COM EARTH DIKE SUPER SILT FENCE OWNER: VILLAGES AT TURF VALLEY PHASE 2, SECTION 1 LIMIT OF DISTURBANCE LOT 203; OPEN SPACE LOTS 204-207: MANGIONE ENTERPRISES OF EROSION CONTROL TURF VALLEY, LIMITED PARTNERSHIP GOLF SPACE LOTS 77, 78, 208 & 209; PARCELS A & B; MATTING 1205 YORK ROAD, PENTHOUSE NON-BUILDABLE BULK PARCELS CC-1, DD-1, EE-1 & FF-1 LUTHERVILLE, MARYLAND 21093 410-825-8400 TAX MAP: 16, PARCEL: 401, GRID: 10 TAX MAP: 16, PARCEL: P/O 394, GRID: 17 SOILS DELINEATION LINE TAX MAP: 16, PARCEL: P/O 8, GRID: 17 ELECTION DISTRICT NO. 3 - HOWARD COUNTY, MARYLAND - ZONED: PGCC DEVELOPER: SOILS TYPE AND CLASS REVISED GRADING, SEDIMENT AND MANGIONE ENTERPRISES OF TURF VALLEY, LIMITED PARTNERSHIP EROSION CONTROL PLAN PHASING LINE 1205 YORK ROAD, PENTHOUSE AREA LUTHERVILLE, MARYLAND 21093 DATE: FEBRUARY, 2010 BEI PROJECT NO. 1915 410-825-8400 SCALE: 1" = 50' SHEET 13 OF 56 AS-BUILT F-08-084

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			anner weiter with the
			Harring Contraction
			FOREST CONSERVATION
	NOTE: SUPER SILT FENCE ALONG	MATCH LINE SEE SHEET 13	FASEMENT #6
	THE SWM ACCESS IS TO BE "CURLED" INTO THE LIMIT OF	MATCH LINE SEE	2
	DISTURBANCE AT APPROX. 35' INTERVALS.	MATCH LINE PARCEL A FOR prophis SEE 50P-00-096 FOR prophis SEE 50P-00-096 FOR prophis SEE 50P-00-096 FOR prophis	GOLF SPACE W
			PRIVATEL PRIVATEL OWNED 310 210 210 210 210 210 210 210 2
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		LIZ' EMBANEMENT GOLF SPACE	LOT 207
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	1	L (B) 1N 595.65	
	ENSTUS: AS FAIRING TAS	9 19 19 19 19 19 19 19 19 19 19 19 19 19	
		The second	
	SEDIMENT BASIN #6 DATA EXISTING D.A. 0.41 ac.		AD 9=613/18
	PROPOSED D.A. 5.55 ac. STORAGE REQUIRED WET 9.990 CF STORAGE PROVIDED WET 9.990 CF STORAGE PROVIDED WET 9.990 CF		
	EMBANKMENT ELEVATION445.00RISER CREST ELEVATION442.50CLEANOUT ELEVATION437.37		(30) 9+61 6fs EXI WEL=40.85 PR. WEL=40.85 INV
	BOTTOM ELEVATION435.00WET STORAGE ELEVATION438.61		
	DRY STORAGE ELEVATION440.13Q1 PRE-DEVELOPMENT0.04 cfsQ1 DURING-DEVELOPMENT0.08 cfs	GENERAL STORAGE REQUIREMENT SUMMARY	TABLE
	Q1 POST-DEVELOPMENT 0.23 cfs	DRAINAGE AREA 5A WQv AREA = 5.39 AC. CPv AREA = 5.55 AC. REQUIREMENT VOLUME REQUIRED NOTES	
	NOTES: 1. TEMPORARY OR PERMANENT	(WQv) 0.3495 ac-ft 15,224 cf PROVIDED IN SWMF #6 SURFACE SAND FILTER (REV) 0.99 acres 3,441 cf PROVIDED IN STONE CHAMBER BELOW SURFACE SAND FILTER	E OF MAD
	STABILIZATION IS TO BE PERFORMED AT THE DESCRETION	(CPV) 0.4878 ac-ft PROVIDED IN SWMF #6 DETENTION 21,247 cf WQV AREA = 3.12 AC.	AN COMERALAW ILLE
	OF THE SEDIMENT CONTRL INSPECTOR REGARDLESS OF ANY OTHER STANDARD	CPv AREA = 3.12 AC. REQUIREMENT VOLUME REQUIRED NOTES	
	STABILIZATION NOTES WITH THE STANDARD NOTES BEING THE	(REV) 0.00 acres NO NEW IMPERVIOUS BEING CREATED	Professional Certification. I hereby certify that these documents were propagated or approximate the second secon
	MINIMUM.	(CPV) 0.0000 ac-ft NOT REQUIRED FOR THIS AREA	documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

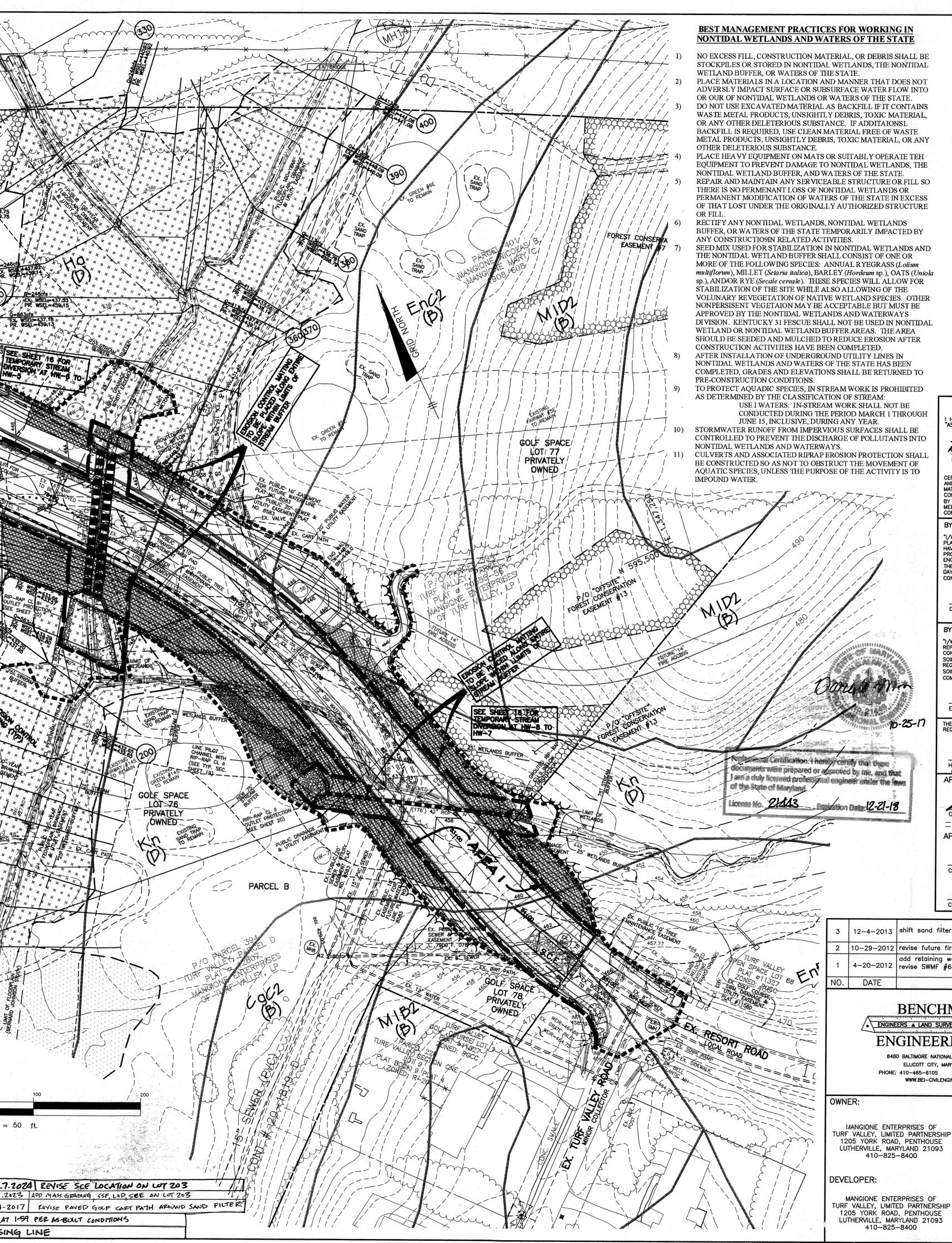
2. SILT AND SUPER SILT FENCING TO BE CURLED UPHILL AT 2 VERTICAL FOOT INTERVALS WHERE FENCING RUNS DOWNHILL.

DRAINAG	E AREA 5A	WQv AREA = 5.39 AC. CPv AREA = 5.55 AC.		
REQUIREMENT	VOLUME REQUIRED	NOTES		
(WQv)	0.3495 ac-ft 15,224 cf	PROVIDED IN SWMF #6 SURFACE SAND FILTER		
(REV)	0.99 acres 3,441 cf	PROVIDED IN STONE CHAMBER BELOW SURFACE SAND FILTER		
(CPV)	(CPV) 0.4878 ac-ft PROVIDED IN SWMF #6 DETEI 21,247 cf POND			
DRAINAGE	E AREA 5B	WQV AREA = 3.12 AC. CPV AREA = 3.12 AC.		
REQUIREMENT	VOLUME REQUIRED	NOTES		
(WQv)	(WQv) 0.0481 ac-ft NO NEW IMPERVIOUS BEING CRE			
(REV)	0.00 acres 306 cf	NO NEW IMPERVIOUS BEING CREATED		
(CPV)	0.0000 ac-ft 0 cf	NOT REQUIRED FOR THIS AREA		
1-YEA	R DISCHARGE	COMPARISON TABLE		
EXISTING	PROPOSED WITH SWM	NOTES		
1.72 CFS	1.69 CFS	-YR POST-DEVELOPED DISCHARGE MOUNT IS LESS THAN 1-YR PRE-DEVELOPED DISCHARGE AMOUNT.		

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	and a second second Second second	4	7-8-201	A		ADD	

Reference -WAR BARRY

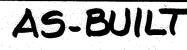
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1205 YORK ROAD, PENTHOUSE LUTHERVILLE, MARYLAND 21093

1205 YORK ROAD, PENTHOUSE LUTHERVILLE, MARYLAND 21093

LEGEND EXISTING CONTOURS AERIAL JAN. 2006 _____ ---- EXISTING BRUSH EXISTING TREELINE . PROPOSED TREELINE EXISTING STREAM EXISTING WETLANDS 7110 7110 UNMITIGATED 65 dBA NOISE LINE • • • • • • • • • • • FOREST CONSERVATION EASEMENT EARTH DIKE SUPER SILT FENCE *** LIMIT OF DISTURBANCE EROSION CONTROL MATTING STABILIZED CONSTRUCTION SCE ENTRANCE SOILS DELINEATION LINE GIA SOILS TYPE AND CLASS (B) PHASING LINE AREA ! AS-BUILT CERTIFICATION I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN WAS CONSTRUCTED AS SHOWN ON THE "AS-BUILT" PLANS AND MEETS THE APPROVED PLANS AND SPECIFICATIONS. PE NO. 21443 11/10 DATE 10-25-17 ENGINEER CERTIFY MEANS TO STATE OR DECLARE A PROFESSIONAL OPINION BASED UPON ONSITE INSPECTIONS AND MATERIAL TESTS WHICH ARE CONDUCTED DURING CONSTRUCTION. THE ONSITE INSPECTIONS AND MATERIAL TESTS ARE THOSE INSPECTIONS AND TESTS DEEMED SUFFICIENT AND APPROPRIATE BY COMMONLY ACCEPTED ENGINEERING STANDARDS. CERTIFY DOES NOT MEAN OR IMPLY A GUARANTEE BY THE ENGINEER NOR DOES AN ENGINEER'S CERTIFICATION RELIEVE ANY OTHER PARTY FROM MEETING REQUIREMENTS IMPOSED BY CONTRACT, EMPLOYMENT, OR OTHER MEANS, INCLUDING MEETING COMMONLY ACCEPTED INDUSTRY PRACTICES. BY THE DEVELOPER: "I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE ACCORDING TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD. SOUL CONSERVATION DISTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTIONS BY THE HOWARD SOIL ONSERVATION DISTRICT lowin 12 5 13 DEVELOPER DATE BY THE ENGINEER: "I/WE CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER TH REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION." 12/4/2013 ENGINEER - DONALD A. WASON, P.E. # 21443 DATE THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION, AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. Pobutan APPROVED: DEPARTMENT OF PUBLIC WORKS Will 2. Mall 1-9-14 CHIEF, BUREAU OF HIGHWAYS DATE -----APPROVED: DEPARTMENT OF PLANNING AND ZONING Ket Sherloore 1/23/14 CHIEF, DIVISION OF LAND DEVELOPMENT DATE Ensis Ment Educate 1.15.14 CHIEF, DEVELOPMENT ENGINEERING DIVISION DATE 12-4-2013 shift sand filter location and access for SWMF #6. 2 10-29-2012 revise future fire access and FCE #13 add retaining wall and cart path north of Resort Road, add tunnel, add 6" pvc conduit, 4-20-2012 revise SWMF #6 location and related storm drain REVISION Professional Certification. I hereby certify that these document were prepared or approved by me, and that I am a duly license BENCHMARK professional engineer under the laws of the State of Maryland License No. 28559 Expiration Date: 7-22-2015 ENGINEERS & LAND SURVEYORS & PLANNERS ENGINEERING, INC 8480 BALTIMORE NATIONAL PIKE A SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644 WWW.BEI-CIVILENGINEERING.COM 2015 12/4/2015 VILLAGES AT TURF VALLEY PHASE 2, SECTION 1 LOT 203; OPEN SPACE LOTS 204-207; GOLF SPACE LOTS 77, 78, 208 & 209; PARCELS A & B; NON-BUILDABLE BULK PARCELS CC-1, DD-1, EE-1 & FF-1 TAX MAP: 16, PARCEL: 401, GRID: 10 TAX MAP: 16, PARCEL: P/O 394, GRID: 17 TAX MAP: 16, PARCEL: P/O 8, GRID: 17 ELECTION DISTRICT NO. 3 - HOWARD COUNTY, MARYLAND - ZONED: PGCC REVISED GRADING, SEDIMENT AND EROSION CONTROL PLAN DATE: FEBRUARY, 2010 BEI PROJECT NO. 1915



SCALE:

1" = 50'

SHEET

F-08-084

14 OF 56

B-4 STANDARDS AND SPECIFICATIONS	B-4-2 STANDARDS AND SPECIFICATIONS	
FOR	FOR	
VEGETATIVE STABILIZATION Definition	SOIL PREPARATION, TOPSOILING, AND SOIL AMENDMENTS Definition	
Using vegetation as cover to protect exposed soil from erosion.	The process of preparing the solls to sustain adequate vegetative stabilization.	The application of s
Purpose To promote the establishment of vegetation on exposed soil.	Purpose To provide a suitable soil medium for vegetative growth.	To protect disturbe
Conditions Where Practice Applies On all disturbed areas not stabilized by other methods. This specification is divided into sections on	Conditions Where Practice Applies Where vegetative stabilization is to be established.	To the surface of al
incremental stabilization; soil preparation, soil amendments and topsoiling; seeding and mulching; temporary	A. Soil Preparation	
stabilization;	1. Temporary Stabilization	A. Seeding 1. Specificati
and permanent stabilization. Effects on Water Quality and Quantity Stabilization practices are used to promote the establishment of vegetation on exposed soil. When soil is	a. Seedbed preparation consists of loosening soil to a depth of 3 to 5 inches by means of suitable agricultural or construction equipment, such as disc harrows or chisel plows or rippers mounted on construction equipment. After the soil is loosened, it must not be	a. All s
stabilized with vegetation, the soil is less likely to erode and more likely to allow infiltration of rainfall, thereby reducing sediment loads and runoff to downstream areas.	rolled or dragged smooth but left in the roughened condition. Slopes 3:1 or flatter are to be tracked with ridges running parallel to the contour of the slope. b. Apply fertilizer and lime as prescribed on the plans.	
Planting vegetation in disturbed areas will have an effect on the water budget, especially on volumes and	c. Incorporate lime and fertilizer into the top 3 to 5 inches of soil by disking or other	b. Mul
rates of runoff, infiltration, evaporation, transpiration, percolation, and groundwater recharge. Over time, vegetation		c, Inoc
will increase organic matter content and improve the water holding capacity of the soil and subsequent plant growth.	 A soil test is required for any earth disturbance of 5 acres or more. The minimum soil conditions required for permanent vegetative establishment are: I. Soil pH between 6.0 and 7.0. 	
Vegetation will help reduce the movement of sediment, nutrients, and other chemicals carried by runoff to	II. Soluble saits less than 500 parts per million (ppm).	
receiving waters. Plants will also help protect groundwater supplies by assimilating those substances present	iii. Soll contains less than 40 percent clay but enough fine grained material (greater than 30 percent silt plus clay) to provide the capacity to hold a moderate amount of moisture.	d. Sod
within the root zone. Sediment control practices must remain in place during grading, seedbed preparation, seeding, mulching,	An exception: if lovegrass will be planted, then a sandy soil (less than 30 percent silt plus clay) would be acceptable.	
and vegetative establishment.	iv. Soil contains 1.5 percent minimum organic matter by weight.	2. Application
Adequate Vegetative Establishment inspect seeded areas for vegetative establishment and make necessary repairs, replacements, and	 v. Soil contains sufficient pore space to permit adequate root penetration. b. Application of amendments or topsoil is required if on-site soils do not meet the above 	a. Dry
reseedings within the planting season.	conditions. c. Graded areas must be maintained in a true and even grade as specified on the	
 Adequate vegetative stabilization requires 95 percent groundcover. If an area has less than 40 percent groundcover, restabilize following the original recommendations 	approved plan, then scarified or otherwise loosened to a depth of 3 to 5 inches. d. Apply soil amendments as specified on the approved plan or as indicated by the results	
for lime, fertilizer, seedbed preparation, and seeding.	of a soll test.	b. Drill
3. If an area has between 40 and 94 percent groundcover, over-seed and fertilize using half of the rates originally specified.	 Mix soil amendments into the top 3 to 5 inches of soil by disking or other suitable means. Rake lawn areas to smooth the surface, remove large objects like stones and 	U. 148
4. Maintenance fertilizer rates for permanent seeding are shown in Table B.6.	branches, and ready the area for seed application. Loosen surface soll by dragging with a heavy chain or other equipment to roughen the surface where site conditions will not	
B-4-1 STANDARDS AND SPECIFICATIONS	permit normal seedbed preparation. Track slopes 3:1 or flatter with tracked equipment	
FOR	leaving the soil in an irregular condition with ridges running parallel to the contour of the slope. Leave the top 1 to 3 inches of soil loose and friable. Seedbed loosening may be	c. Hyd
INCREMENTAL STABILIZATION Definition	unnecessary on newly disturbed areas. B. Topsoiling	
Establishment of vegetative cover on cut and fill slopes.	 Topsoiling Topsoil is placed over prepared subsoil prior to establishment of permanent vegetation. The 	
Purpose To provide timely vegetative cover on cut and fill slopes as work progresses.	purpose is to provide a suitable soil medium for vegetative growth. Soils of concern have low moisture content, low nutrient levels, low pH, materials toxic to plants, and/or unacceptable soil	
Conditions Where Practice Applies Any cut or fill slope greater than 15 feet in height. This practice also applies to stockpiles.	gradation.	
Criteria A. Incremental Stabilization - Cut Slopes	Topsoil salvaged from an existing site may be used provided it meets the standards as set forth in these specifications. Typically, the depth of topsoil to be salvaged for a given soil type	
1. Excavate and stabilize cut slopes in increments not to exceed 15 feet in height. Prepare seedbed	can be found in the representative soil profile section in the Soil Survey published by USDA-NRCS.	
and apply seed and mulch on all cut slopes as the work progresses. 2. Construction sequence example (Refer to Figure B.1):	3. Topsoiling is limited to areas having 2:1 or flatter slopes where:	B. Mulching
a. Construct and stabilize all temporary swales or dikes that will be used to convey runoff	 The texture of the exposed subsoil/parent material is not adequate to produce vegetative growth. 	1. Mulch Mat
around the excavation. b. Perform Phase 1 excavation, prepare seedbed, and stabilize.	b. The soil material is so shallow that the rooting zone is not deep enough to support plants or furnish continuing supplies of moisture and plant nutrients.	.8. 348
c. Perform Phase 2 excavation, prepare seedbed, and stabilize. Overseed Phase 1 areas as necessary.	c. The original soil to be vegetated contains material toxic to plant growth.	
d. Perform final phase excavation, prepare seedbed, and stabilize. Overseed previously	 d. The soil is so acidic that treatment with limestone is not feasible. 4. Areas having slopes steeper than 2:1 require special consideration and design. 	b. Woo
seeded areas as necessary. Note: Once excavation has begun the operation should be continuous from grubbing through the	 Topsoil Specifications: Soil to be used as topsoil must meet the following criteria: a. Topsoil must be a loam, sandy loam, clay loam, silt loam, sandy clay loam, or loamy 	
completion of grading and placement of topsoil (if required) and permanent seed and mulch. Any interruptions in the operation or completing the operation out of the seeding season will necessitate	sand. Other soils may be used if recommended by an agronomist or soil scientist and	
the application of temporary stabilization.	approved by the appropriate approval authority. Topsoil must not be a mixture of contrasting textured subsoils and must contain less than 5 percent by volume of cinders,	
B. Incremental Stabilization - Fill Slopes 1. Construct and stabilize fill slopes in increments not to exceed 15 feet in height. Prepare seedbed	stones, slag, coarse fragments, gravel, sticks, roots, trash, or other materials larger than	
and apply seed and mulch on all slopes as the work progresses.	 1½ inches in diameter. b. Topsoil must be free of noxious plants or plant parts such as Bermuda grass, quack 	
 Stabilize slopes immediately when the vertical height of a lift reaches 15 feet, or when the grading operation ceases as prescribed in the plans. 	grass, Johnson grass, nut sedge, poison ivy, thistle, or others as specified.	
3. At the end of each day, install temporary water conveyance practice(s), as necessary, to intercept	 Topsoil substitutes or amendments, as recommended by a qualified agronomist or soil scientist and approved by the appropriate approval authority, may be used in lieu of 	
surface runoff and convey it down the slope in a non-erosive manner. 4. Construction sequence example (Refer to Figure B.2):	natural topsoil. 6. Topsoil Application	
 Construct and stabilize all temporary swales or dikes that will be used to divert runoff around the fill. Construct silt fence on low side of fill unless other methods shown on the plans 	 a. Erosion and sediment control practices must be maintained when applying topsoil. b. Uniformly distribute topsoil in a 5 to 8 inch layer and lightly compact to a minimum 	
address this area.	thickness of 4 inches. Spreading is to be performed in such a manner that sodding or	
b. At the end of each day, install temporary water conveyance practice(s), as necessary, to intercept surface runoff and convey it down the slope in a non-erosive manner.	seeding can proceed with a minimum of additional soil preparation and tillage. Any irregularities in the surface resulting from topsoiling or other operations must be	
c. Place Phase 1 fill, prepare seedbed, and stabilize. d. Place Phase 2 fill, prepare seedbed, and stabilize.	corrected in order to prevent the formation of depressions or water pockets.	
e. Place final phase fill, prepare seedbed, and stabilize. Overseed previously seeded areas as	c. Topsoil must not be placed if the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet or in a condition that may otherwise be detrimental	2. Application
necessary. Note: Once the placement of fill has begun the operation should be continuous from grubbing through the	to proper grading and seedbed preparation. C. Soil Amendments (Fertilizer and Lime Specifications)	a. App b. Whe
completion of grading and placement of topsoil (if required) and permanent seed and mulch. Any	1. Soil tests must be performed to determine the exact ratios and application rates for both lime	
interruptions in the operation or completing the operation out of the seeding season will necessitate the application of temporary stabilization.	and fertilizer on sites having disturbed areas of 5 acres or more. Soil analysis may be performed by a recognized private or commercial laboratory. Soil samples taken for	
Figure B.	engineering purposes may also be used for chemical analyses.	c. Woo
	Fertilizers must be uniform in composition, free flowing and suitable for accurate application by appropriate equipment. Manure may be substituted for fertilizer with prior approval from the	2 Anchesian

- appropriate equipment. Manure may be substituted for fertilizer with prior approval from the appropriate approval authority. Fertilizers must all be delivered to the site fully labeled according to the applicable laws and must bear the name, trade name or trademark and warranty of the producer
- 3. Line materials must be ground limestone (hydrated or burnt lime may be substituted except when hydroseeding) which contains at least 50 percent total oxides (calcium oxide plus magnesium oxide). Limestone must be ground to such fineness that at least 50 percent will pass through a #100 mesh sieve and 98 to 100 percent will pass through a #20 mesh sieve. Lime and fertilizer are to be evenly distributed and incorporated into the top 3 to 5 inches of soil by disking or other suitable means.
- Where the subsoil is either highly acidic or composed of heavy clays, spread ground limestone at the rate of 4 to 8 tons/acre (200-400 pounds per 1,000 square feet) prior to the placement of

NOTIFY SEDIMENT CONTROL DIVISION 48 HOURS PRIOR TO START OF CONSTRUCTION PHASE 1

SEQUENCE OF CONSTRUCTION

1). OBTAIN GRADING PERMIT. MDE PERMIT NUMBER 02-NT-0009 / 200261454 EFF. 5-10-2006. (DAY 1)

2). INSTALL STABILIZED CONSTRUCTION ENTRANCES. (DAY 2)

3). INSTALL STREAM DIVERSION PIPES/CHANNELS. SEE DETAILS ON SHEET 16. (DAY 3) 4). INSTALL THE CON/SPAN BRIDGE (HW-3 TO HW-4), THE 2-84" RCP CULVERTS (HW-5 TO HW-6) AND THE 42" RCP CULVERT (HW-7 TO HW-8) FOR THE STREAM CROSSINGS OF RESORT ROAD. INSTALL THE 24" CULVERT (HW-9 TO HW-10). CONSTRUCT AND

STABILIZE RELOCATED STREAM CHANNEL BELOW HW-7. STREAM CLOSURE DATES: MARCH 1st THRU JUNE 15th. COVER PIPES AND ARCH WITH ADEQUATE FILL, L.O.D. FOR THE

STREAM CROSSINGS (PHASE 1) SHALL BE LIMITED TO 50' ON EACH SIDE OF THE STREAM AND 45' LAONG EACH SIDE OF PROPOSED RESORT ROAD. (DAY 4-21)

PHASE 2

5). INSTALL SILT FENCES, SUPER SILT FENCES AND CLEANWATER DIVERSION DIKES. (DAY 22-30)

6). INSTALL TEMPORARY SEDIMENT BASINS. (DAY 31-40)

7). AFTER INSTALLATION OF BASIN #5, INSTALL EARTH DIKES AND TEMPORARY PIPE AT BASIN #4. INSTALL SANDBAGS AND TEMP PIPES AT STREAM CROSSINGS (I.E. AT THE 2-84" CULVERTS AND THE 42" CULVERT) (DAY 42)

8). UPON APPROVAL, OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR BRING ROAD BEDS TO SUBGRADE INSTALL TUNNEL AND RETAINING WALL ALONG RESORT ROAD. STABILIZE SLOPES IN ACCORDANCE WITH TEMPORARY SEED BED NOTES. UTILIZE DUST CONTROL METHODS. NO MORE THAN 20 AC MAY BE UNSTABILIZED AT ANYTIME. (DAY 43-60)

9). UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, INSTALL STORM DRAINS, (IN CLUDING THE CULVERT FROM HW-11 TO E-14) WATER MAINS, SEWER MAINS AND NOISE WALL. (DAY 61-90)

10). INSTALL CURB AND GUTTER AND PAVE ROADWAYS. (DAY 91-110) AND CART PATH.

11). COMPLETE CLEARING AND MASS GRADING OF SITE INCLUDING THE BERM IN OPEN SPACE LOT 204 AND STABILIZE DISTURBED AREAS IN ACCORDANCE WITH THE PERMANENT SEEDBED NOTES. ADD TSOS AND TGOS TO THE SWALE AT BOTTOM OF THE BERM ALONG 1-70. (DAY 111-130)

12). UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, CONVERT TEMPORARY BASIN #6 TO A PERMANENT SWM FACILITY. SHAPE FACILITY PER FINAL GRADES SHOWN ON THE PLANS AND STABILIZE DISTURBED AREAS IN ACCORDANCE WITH THE PERMANENT SEEDBED NOTES. CONTRACTOR SHALL REMOVE ALL OLD AND NEW TRASH, JUNK AND DEBRIS FROM POND. BASINS #3, #4 AND #5 SHALL NOT BE CONVERTED UNTIL FUTURE PHASE 3 (BASIN #3), AND PHASE 2, SECTION 2 (BASINS #4 & #5) HAVE BEEN STABILIZED AS A THESE PHASES RUNOFF ARE TREATED BY THE INDICATED BASIN(S). (DAY 131-145)

13). UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, REMOVE REMAINING SEDIMENT CONTROL DEVICES, AND STABILIZED DISTURBED AREAS IN ACCORDANCE WITH THE PERMANENT SEEDBED NOTES. (DAY 146-147)

B-4-3 STANDARDS AND SPECIFICATIONS

FOR SEEDING AND MULCHING

Definition of seed and mulch to establish vegetative cover.

Purpose bed soils from erosion during and at the end of construction.

Conditions Where Practice Applies all perimeter controls, slopes, and any disturbed area not under active grading.

All seed must meet the requirements of the Maryland State Seed Law. All seed must be subject to re-testing by a recognized seed laboratory. All seed used must have been tested within the 6 months immediately preceding the date of sowing such material on any project. Refer to Table B.4 regarding the quality of seed. Seed tags must be available upon request to the inspector to verify type of seed and seeding rate.

- fulch alone may be applied between the fall and spring seeding dates only if the ground is frozen. The appropriate seeding mixture must be applied when the ground thaws. oculants: The inoculant for treating legume seed in the seed mixtures must be a pure culture of nitrogen fixing bacteria prepared specifically for the species. Inoculants must not be used later than the date indicated on the container. Add fresh inoculants as directed on the package. Use four times the recommended rate when hydroseeding. Note: It is very important to keep inoculant as cool as possible until used. Temperatures above 75 to 80 degrees Fahrenheit can weaken bacteria and make the inoculant less
- effective. od or seed must not be placed on soil which has been treated with soil sterilants or chemicals used for weed control until sufficient time has elapsed (14 days min.) to permit dissipation of phyto-toxic materials.
- Dry Seeding: This includes use of conventional drop or broadcast spreaders. i. Incorporate seed into the subsoil at the rates prescribed on Temporary Seeding Table B.1, Permanent Seeding Table B.3, or site-specific seeding summaries II. Apply seed in two directions, perpendicular to each other. Apply half the seeding rate in each direction. Roll the seeded area with a weighted roller to provide good
- seed to soil contact. rill or Cultipacker Seeding: Mechanized seeders that apply and cover seed with soil. i. Cultipacking seeders are required to bury the seed in such a fashion as to provide at least 1/4 inch of soil covering. Seedbed must be firm after
- ii. Apply seed in two directions, perpendicular to each other. Apply half the seeding rate in each direction.
- lydroseeding: Apply seed uniformly with hydroseeder (slurry includes seed and
- i. If fertilizer is being applied at the time of seeding, the application rates should not exceed the following: nitrogen, 100 pounds per acre total of soluble nitrogen; P2O5 (phosphorous), 200 pounds per acre; K2O (potassium), 200 pounds per acre.
- ii. Lime: Use only ground agricultural limestone (up to 3 tons per acre may be applied by hydroseeding). Normally, not more than 2 tons are applied by hydroseeding at any one time. Do not use burnt or hydrated lime when
- iii. Mix seed and fertilizer on site and seed immediately and without interruption. iv. When hydroseeding do not incorporate seed into the soil.

faterials (in order of preference)

Anchoring

traw consisting of thoroughly threshed wheat, rye, oat, or barley and reasonably bright in color. Straw is to be free of noxious weed seeds as specified in the Maryland Seed Law and not musty, moldy, caked, decayed, or excessively dusty, Note: Use only sterile straw mulch in areas where one species of grass is desired Vood Cellulose Fiber Mulch (WCFM) consisting of specially prepared wood cellulose processed into a uniform fibrous physical state.

- I. WCFM is to be dyed green or contain a green dye in the package that will provide an appropriate color to facilitate visual inspection of the uniformly spread slurry.
- ii. WCFM, Including dye, must contain no germination or growth inhibiting factors. iii. WCFM materials are to be manufactured and processed in such a
- manner that the wood cellulose fiber mulch will remain in uniform suspension in water under agitation and will blend with seed, fertilizer and other additives to form a homogeneous slurry. The mulch material must form a blotter-like ground cover, on application, having moisture absorption and percolation properties and must cover and hold grass seed in contact with the soil without inhibiting
- the growth of the grass seedlings. IV. WCFM material must not contain elements or compounds at
- concentration levels that will be phyto-toxic. v. WCFM must conform to the following physical requirements: fiber length
- of approximately 10 millimeters, diameter approximately 1 millimeter, pH range of 4.0 to 8.5, ash content of 1.6 percent maximum and water holding capacity of 90 percent minimum

pply mulch to all seeded areas immediately after seeding.

hen straw mulch is used, spread it over all seeded areas at the rate of 2 tons per acre to a uniform loose depth of 1 to 2 inches. Apply mulch to achieve a uniform distribution and depth so that the soil surface is not exposed. When using a mulch anchoring tool, increase the pplication rate to 2.5 tons per acre

lood cellulose fiber used as mulch must be applied at a net dry weight of 1500 pounds per acre. Mix the wood cellulose fiber with water to attain a mixture with a maximum of 50 pounds of wood cellulose fiber per 100 gallons of water.

a. Perform mulch anchoring immediately following application of mulch to minimize loss by wind or water. This may be done by one of the following methods (listed by preference), depending upon the size of the area and erosion hazard

- i. A mulch anchoring tool is a tractor drawn implement designed to punch and anchor mulch into the soil surface a minimum of 2 inches. This practice is most effective on large areas, but is limited to flatter slopes where equipment can operate safely. If used on sloping land, this practice should follow the contour. li. Wood cellulose fiber may be used for anchoring straw. Apply the fiber binder at a net
- dry weight of 750 pounds per acre. Mix the wood cellulose fiber with water at a maximum of 50 pounds of wood cellulose fiber per 100 gallons of water. iii. Synthetic binders such as Acrylic DLR (Agro-Tack), DCA-70, Petroset, Terra Tax II,
- Terra Tack AR or other approved equal may be used. Follow application rates as specified by the manufacturer. Application of liquid binders needs to be heavier at the edges where wind catches mulch, such as in valleys and on crests of banks. Use of asphalt binders is strictly prohibited. iv. Lightweight plastic netting may be stapled over the mulch according to manufacturer
- recommendations. Netting is usually available in rolls 4 to 15 feet wide and 300 to 3,000 feet long.

documents were prepared on approved by me, and that I am a duly licensed professional engineer under the law of the State of Maryland. License No. 22390 , Expiration Date: 6-30-15

FOR REVISION #4 ONLY

B-4-5 STANDARDS AND SPECIFICATIONS

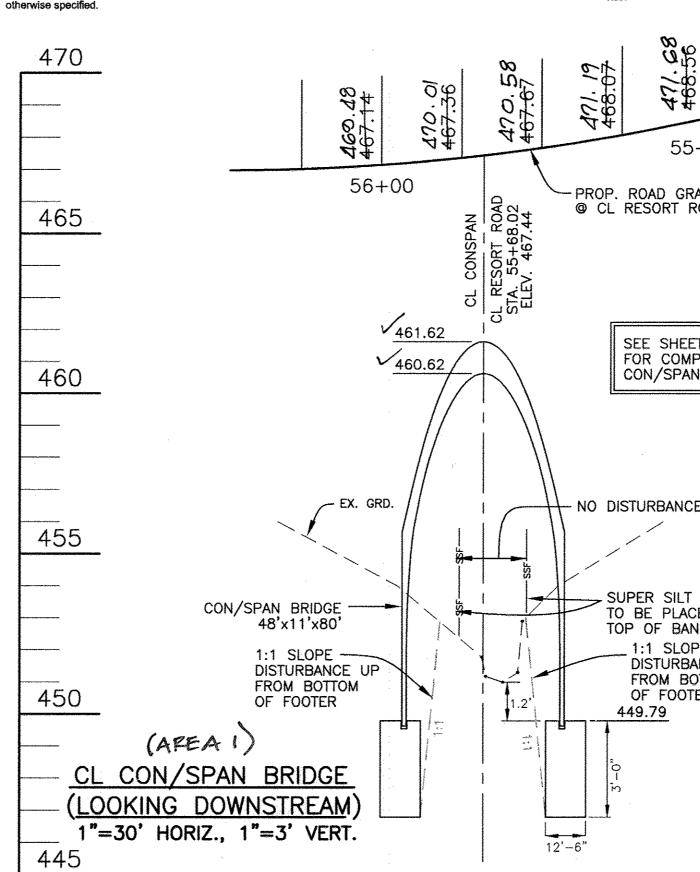
PERMANENT STABILIZATION

To stabilize disturbed soils with permanent vegetation. Purpose

To use long-lived perennial grasses and legumes to establish permanent ground cover on disturbed soils. Conditions Where Practice Applies Exposed soils where ground cover is needed for 6 months or more.

A. Seed Mixtures 1. General Use

- a Select one or more of the species or mixtures listed in Table B.3 for the appropriate Plant Hardiness Zone (from Figure B.3) and based on the site condition or purpose found on Table B.2. Enter selected mixture(s), application rates, and seeding dates in the Permanent Seeding Summary. The Summary is to be placed on the plan. b Additional planting specifications for exceptional sites such as shorelines, stream banks, or dunes or
- for special purposes such as wildlife or aesthetic treatment may be found in USDA-NRCS Technical Field Office Guild, Section 342 - Critical Area Planting. c For sites having disturbed areas over 5 acres, use and show the rates recommended by the soil testing agency.
- d For areas receiving low maintenance, apply urea form fertilizer (46-0-0) at 3 ½ pounds per 1000 square feet (150 pounds per acre) at the time of seeding in addition to the soil amendments shown in the Permanent Seeding Summary.
- 2. Turfgrass Mixtures a. Areas where turfgrass may be desired include lawns, parks, playgrounds, and commercial sites which will receive a medium to high level of maintenance.
- b. Select one or more of the species or mixtures listed below based on the site conditions or purpose. Enter selected mixture(s), application rates, and seeding dates in the Permanent Seeding Summary. The summary is to be placed on the plan i. Kentucky Bluegrass: Full sun Mixture: For use in areas that receive intensive management.
- Irrigation required in the areas of central Maryland and Eastern Shore. Recommended Certified Kentucky Bluegrass Cultivars Seeding Rate: 1.5 to 2.0 pounds per 1000 square feet. Choose a minimum of three Kentucky Bluegrass Cultivars with each ranging from 10 to 35 percent of the total mixture by weight ii. Kentucky Bluegrass/Perennial Rye: Full Sun Mixture: For use in full sun areas where rapid
- establishment is necessary and when turf will receive medium to intensive management. Certified Perennial Ryegrass Cultivars/Certified Kentucky Bluegrass Seeding Rate: 2 pounds mixture per 1000 square feet. Choose a minimum of three Kentucky Bluegrass Cultivars with each ranging from 10 to 35 percent of the total mixture by weigh iii. Tall Fescue/Kentucky Bluegrass: Full Sun Mixture: For use in drought prone areas and/or for areas
- receiving low to medium management in full sun to medium shade. Recommended mixture includes; Certified Tall Fescue Cultivars 95 to 100 percent, Certified Kentucky Bluegrass Cultivars 0 to 5 percent. Seeding Rate: 5 to 8 pounds per 1000 square feet. One or more cultivars may be blended iv.Kentucky Bluegrass/Fine Fescue: Shade Mixture: For use in areas with shade in Bluegrass lawns. For establishment in high quality, intensively managed turf area. Mixture includes Certified Kentucky Bluegrass Cultivars 30 to 40 percent and Certified Fine Fescue and 60 to 70 percent. Seeding Rate:
- 1 ½ to 3 pounds per 1000 square feet. Notes:Select turfgrass varieties from those listed in the most current University of Maryland Publication, Agronomy Memo #77, "Turfgrass Cultivar Recommendations for Maryland" Choose certified material. Certified material is the best guarantee of cultivar purity. The certification program of the Maryland Department of Agriculture. Turf and Seed Section, provides a reliable means of consumer protection and assures a pure genetic line.
- c. Ideal Times of Seeding for Turf Grass Mixtures Western MD: March 15 to June 1, August 1 to October 1 (Hardiness Zones: 5b, 6a) Central MD:March 1 to May 15, August 15 to October 15 (Hardiness Zone: 6b) Southern MD, Eastern Shore: March 1 to May 15, August 15 to October 15
- (Hardiness Zones: 7a, 7b) d. Till areas to receive seed by disking or other approved methods to a depth of 2 to 4 inches, level and rake the areas to prepare a proper seedbed. Remove stones and debris over 1 ½ inches in diameter. The resulting seedbed must be in such condition that future mowing of grasses will pose
- no difficulty. e. If soil moisture is deficient, supply new seedings with adequate water for plant growth (½ to 1 inch every 3 to 4 days depending on soil texture) until they are firmly established. This is not especially true when seedings are made late in the planting season, in abnormally dry or hot seasons, or on adverse sites
- B. Sod: to provide quick cover on disturbed areas (2:1 grade or flatter). 1. General Specifications
- a. Class of turfgrass must be Maryland State Certified. Sod labels must be made available to the job foreman and inspector. b. Sod must be machine cut at a uniform soil thickness of ¼ inch, plus or minus ¼ inch, at the time of
- cutting. Measurement for thickness must exclude top growth and thatch. Broken pads and or uneven ends will not be acceptable. c. Standard size sections of sod must be strong enough to support their own weight and retain their
- size and shape when suspended vertically with a firm grasp on the upper 10 percent of the section. d. Sod must not be harvested or transplanted when moisture content (excessively dry or wet) may adversely affect its survival
- e. Sod must be harvested, delivered, and installed within a period of 36 hours. Sod not transplanted within this period must be approved by an agronomist or soil scientist prior to its installation. 2. Sod Installation a. During periods of excessively high temperature or in areas having dry subsoil, lightly irrigate the
- subsoil immediately prior to laying the sod. b. Lay the first row of sod in a straight line with subsequent rows placed parallel to it and tightly wedged against each other. Stagger lateral joints to promote more uniform growth and strength. Ensure that sod is not stretched or overlapped and that all joints are butted tight in order to prevent
- voids which would cause air drying of the roots. c. Wherever possible, lay sod with the long edges parallel to the contour and with staggering joints. Roll and tamp, peg or otherwise secure the sod to prevent slippage on slopes. Ensure solid contact
- exists between sod roots and the underlying soil surface. d. Water the sod immediately following rolling and tamping until the underside of the new sod pad and soil surface below the sod are thoroughly wet. Complete the operations of laying, tamping and irrigating for any piece of sod within eight hours.
- 3. Sod Maintenance a. In the absence of adequate rainfall, water daily during the first week or as often and sufficiently as necessary to maintain moist soil to a depth of 4 inches. Water sod during the heat of the day to prevent wilting.
- b. After the first week, sod watering is required as necessary to maintain adequate moisture content. c. Do not mow until the sod is firmly rooted. No more than 1/3 of the grass leaf must be removed by the initial cutting or subsequent cuttings. Maintain a grass height of at least 3 inches unless otherwise specified.



- To stabilize disturbed soils with vegetatic To use fast growing vegetation that provi
- Exposed soils where ground cover is nee permanent stabilization practices are req
- Select one or more of the species Hardiness Zone (from Figure B.3), with application rates, seeding date completed, then Table B.1 plus fer
- For sites having soil tests performed Soil tests are not required for Temp 3. When stabilization is required outsid
- alone as prescribed in Section B-4-

B-4-8 STAN

A mound or pile of soil protected by approp To provide a designated location for the ten sedimentation, and changes to drainage pa

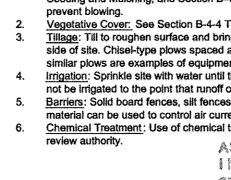
- Stockpile areas are utilized when it is neces 1. The stockpile location and all relat
- erosion and sediment control 2. The footprint of the stockpile musi
- and based on a side slope rat accordance with Section B-3 I
- Runoff from the stockpile area mu 4. Access the stockpile area from the 5. Clear water runoff into the stockpi
- an earth dike, temporary swal concentrated flow in a non-error
- Where runoff concentrates along control practice must be used
- 7. Stockpiles must be stabilized in ac Standard B-4-1 Incremental S
- 8. If the stockpile is located on an im to facilitate cleanup. Stockpiles
- impermeable sheeting.

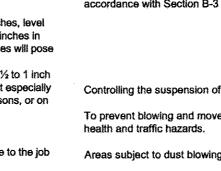
The stockpile area must continuously mee accordance with Section B-4 Vegetative St 2:1 ratio. The stockpile area must be kept f feet for 2:1 slopes, 30 feet for 3:1 slopes, o accordance with Section B-3 Land Grading

Controlling the suspension of dust particles To prevent blowing and movement of dust f

Areas subject to dust blowing and moveme

- Mulches: See Section B-4-2 Soil Pre
- Seeding and Mulching, and Section prevent blowing.
- review authority.
- tom





B-4-4 STANDARDS AND SPE	-CIFICATIONS				DNSERVATION DISTRICT MENT CONTROL NOTES
<u>FOR</u> TEMPORARY STABLIZ			1. A M	INIMUM OF 48 HOURS NOTICE MUST	BE GIVEN TO THE HOWARD COUNTY DEPARTMENT
Definition h vegetation for up to 6 months.				Y CONSTRUCTION (313-1855).	EDIMENT CONTROL DIVISION PRIOR TO THE START
that provides cover on disturbe			PROVIS	SIONS OF THIS PLAN AND ARE TO BE	TICES ARE TO BE INSTALLED ACCORDING TO THE IN CONFORMANCE WITH THE MOST CURRENT
Conditions Where Practic over is needed for a period of 6 as are required.	reaction in the second	n of time,		AND STANDARDS AND SPECIFICATIONS DNS THERETO.	FOR SOIL EROSION AND SEDIMENT CONTROL AND
Criteria	in Table B.1 for the appropriate Pl	ant	STABIL	IZATION SHALL BE COMPLETED WITHIN	RE-DISTURBANCE, PERMANENT OR TEMPORARY A) 3 CALENDAR DAYS FOR ALL PERIMETER
eding dates and seeding depths.	Temporary Seeding Summary bek . If this Summary is not put on the				RIMETER SLOPES AND ALL SLOPES GREATER THAN ED OR GRADED AREAS ON THE PROJECT SITE.
	ust be put on the plan. ecommended rates by the testing a	agency.	ACCOR	DANCE WITH THE 2011 MARYLAND ST	ZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ANDARDS AND SPECIFICATIONS FOR SOIL EROSION
	n, apply seed and mulch or straw	mulch	8-4-4) AND MULCHING (SEC. B-4-3). TEN	EEDING (SEC. B-4-5), TEMPORARY SEEDING (SEC. PORARY STABILIZATION WITH MULCH ALONE CAN IG DATES DO NOT ALLOW FOR PROPER
ction B-4-3.A.1.b and maintain u	until the next seeding season.		GERMIN	IATION AND ESTABLISHMENT OF GRAS	SES.
			MAINT		E TO REMAIN IN PLACE AND ARE TO BE PERMISSION FOR THEIR REMOVAL HAS BEEN MENT CONTROL INSPECTOR.
-4-8 STANDARDS AND SPECI FOR	FICATIONS		<i>n</i> = <i>n</i>	ITE ANALYSIS:	
STOCKPILE AREA Definition			T	OTAL AREA OF SITE:	157.59 ACRES
	on and sediment control measures	i.		REA DISTURBED:	<u>33.23</u> ACRES
4	I that controls the potential for eros	ion,		REA TO BE ROOFED OR PAVED: REA TO BE VEGETATIVELY STABILIZED	9.30 ACRES 23.93 ACRES
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	ractices must be clearly indicated of	on the	T	OTAL FILL:	239,015 CY TE WITH APPROVED SDP AND
nt control plan. kpile must be sized to accommo ∋ slope ratio no steeper than 2:1.	date the anticipated volume of ma	terial	0	FFISTE WASTE/BORROW LOCATION:	ACTIVE GRADING PERMIT
ction B-3 Land Grading. e area must drain to a suitable s					IS DISTURBED BY GRADING ACTIVITY FOR ON THE SAME DAY OF DISTURBANCE.
a from the upgrade side. The stockpile area must be minimi	ized by use of a diversion device s			ITIONAL SEDIMENT CONTROL MUST BE D COUNTY SEDIMENT CONTROL INSPE	PROVIDED, IF DEEMED NECESSARY BY THE CTOR.
a non-erosive manner.	Provisions must be made for discha				EXCESS OF 2 ACRES, APPROVAL OF THE UPON COMPLETION OF INSTALLATION OF PERIMETER
t be used to intercept the discha			EROSIC	IN AND SEDIMENT CONTROLS, BUT BE BANCE OR GRADING. OTHER BUILDING	FORE PROCEEDING WITH ANY OTHER EARTH OR GRADING INSPECTION APPROVALS MAY NOT BE
emental Stabilization and Standa	day stabilization requirement as w ard B-4-4 Temporary Stabilization. her should be provided below the side of				BY THE INSPECTION AGENCY IS MADE.
	ated material must be covered with		THAT 1		TABILIZED BY THE END OF EACH WORKDAY,
Maintenance usly meet the requirements for A	Adequate Vegetative Establishmer				QUENCE OF CONSTRUCTION MUST BE REVIEWED
etative Stabilization. Side slopes t be kept free of erosion. If the ve	s must be maintained at no steepe ertical height of a stockpile exceed	r than a	CONST	RUCTION	nen – 1655 – 1745 – 1844 – 1844 – 1844 – 1844 – 1845 – 1845 – 1845 – 1845 – 1845 – 1845 – 1845 – 1846 – 1846 – Na serie – 1855 – 1856 – 1856 – 1856 – 1856 – 1856 – 1856 – 1856 – 1856 – 1856 – 1856 – 1856 – 1856 – 1856 – 18
slopes, or 40 feet for 4:1 slopes d Grading.	s, benching must be provided in		UNIT (I	MAXIMUM ACREAGE OF 20 ACRES PER	AT GRADING ACTIVITIES BEGIN ON ONE GRADING R GRADING UNIT) AT A TIME. WORK MAY PROCEED LEAST 50 PERCENT OF THE DISTURBED AREA IN
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Definition t particles from construction acti	<u>n</u>				a series and a series of the series of th Series and the series of the
Purpose nt of dust from exposed soil surf	aces to reduce on and off-site dan	nage including			
Conditions Where Pra		4. """ "." "z	BY TH	E DEVELOPER:	
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ows spaced about 12 inches apa s of equipment that may produce	art, spring-toothed harrows, and		CONSER	ATION DISTRICT."	
	. Repeat as needed. The site must				12/5/13
ntrol air currents and soil blowing			DEVEL	OFER	DATE
of chemical treatment requires a AS-BUILT CER			BY TH	E ENGINEER:	
hareby certify	. by my seal, that to the	best of my knowledge	REPRES	ENTS A PRACTICAL AND WORKABLE PLAN	TRUCTION, EROSION AND SEDIMENT CONTROL BASED ON MY PERSONAL KNOWLEDGE OF THE SITE
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				nclude wall & tunnel installation.	
	Prophysical Systems and	NO. DATE		REVISION	
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		BENC	untunudur	unhumhununh	professional engineer under the laws of the State of Maryland, License No. 28559, Expiration Date: 7-22-2015.
RBANCE			manning	hanna the second	15 Jan Carlos
	455	ENGINE	ERΠ	NG, INC.	
		8480 BALTIMORE M		I	(ä) (š) (s)
R SILT FENCE		PHONE: 410-465-61 WWW.BEI-C		AX: 410-465-6644 RING.COM	C. H.L.S.
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7 <u>9</u>		TURF VALLEY, LIMITED PARTNE 1205 YORK ROAD, PENTHOL	JSE		DTS 208-209; PARCELS A & B; LK PARCELS CC-1 DD-1, EE-1 & FF-1
		LUTHERVILLE, MARYLAND 210 410-825-8400	193	ТАХ МАР	: 16, PARCEL: 401, GRID: 10 6, PARCEL: P/O 394, GRID: 17
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FEBRUARY, 2010

REVISED SEDIMENT AND EROSION

CONTROL NOTES AND DETAILS

SHEET

BEI PROJECT NO. 1915

F-08-084

15 OF 56

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445

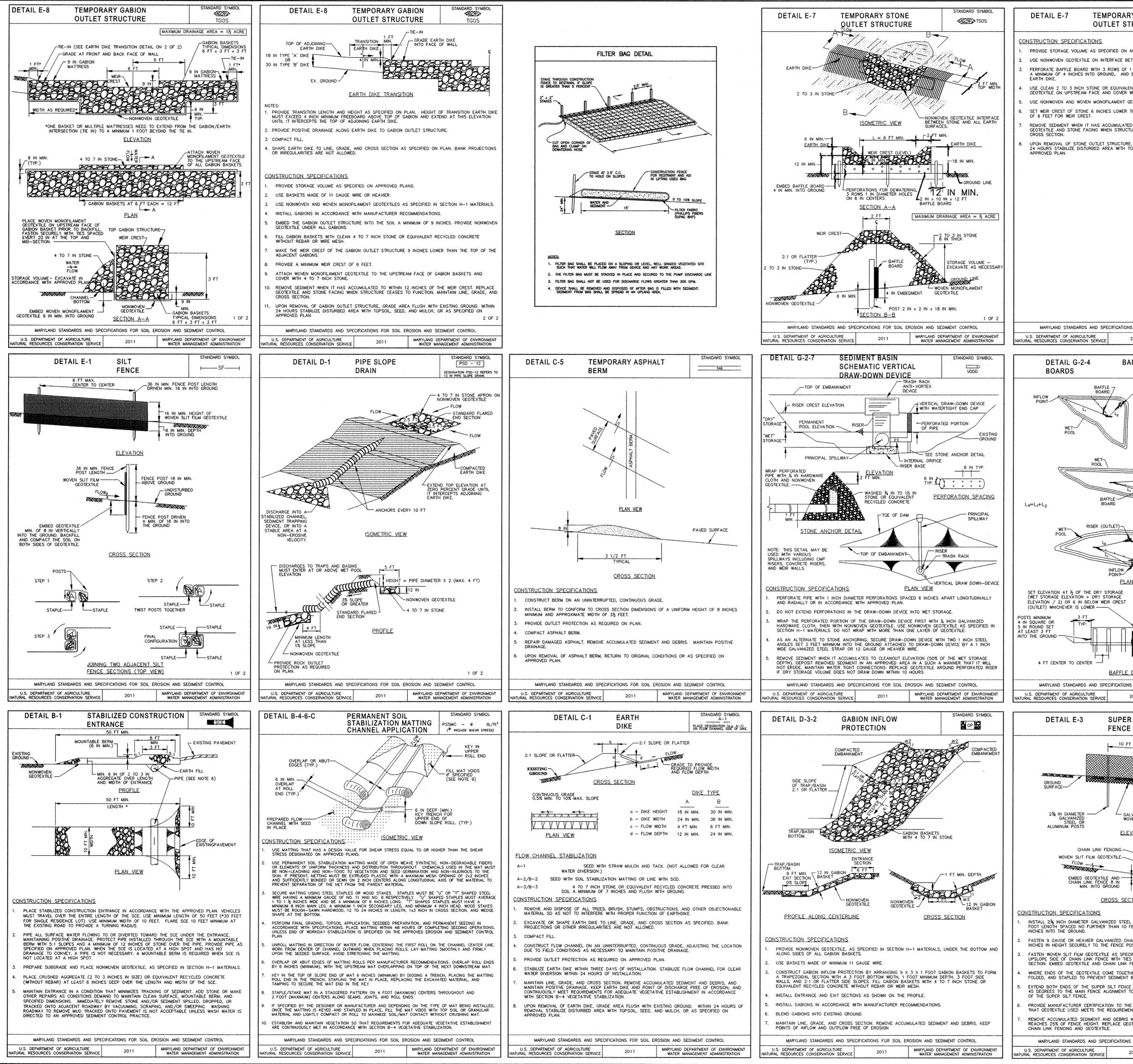
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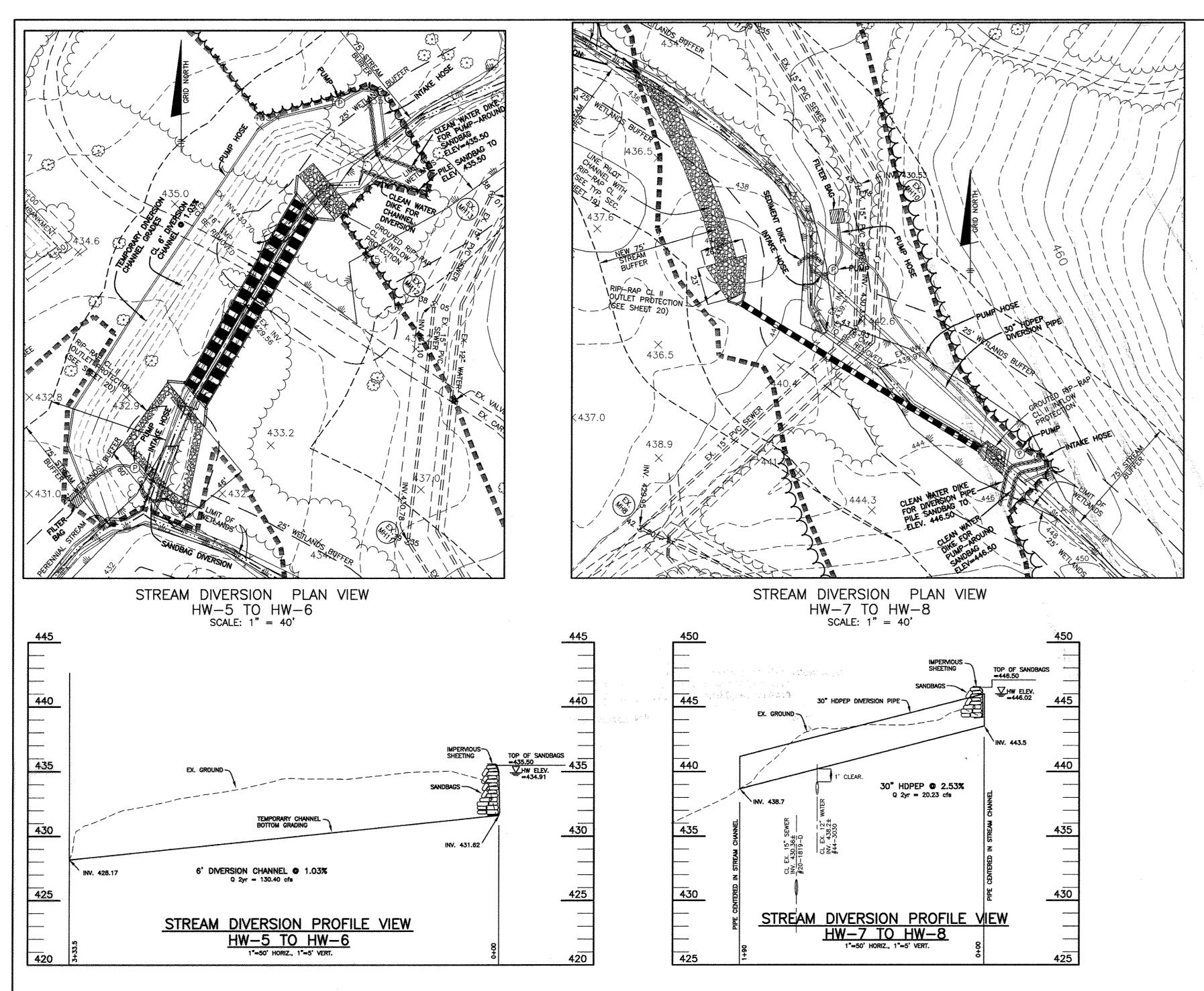
LUTHERVILLE, MARYLAND 21093

410-825-8400



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mporary measure for dewatering in

channel construction sites

DESCRIPTION

The work should consist of installing fabric-based diversion channels for the purpose of erosion control when construction activities occur within the stream channel.

EFFECTIVE USES & LIMITATIONS

Diversions are used to divert flow during construction of in-stream projects. Diversions which have an insufficient Now capacity can fail and severely crode the disturbed channel section under construction. Therefore, in-channel construction activities should occur only during periods of low rainfall.

MATERIAL SPECIFICATIONS

- Materials for fabric-based channel diversions should meet the following requirements:
- Riprop: Class I riprap should be used with fabric-based channel diversions. Filter Cloth: Filter cloth should be a woven or non-woven fabric consisting only of continuous chain polymeric filaments or yarns of polyester. The fabric should be inert to commonly encountered chemicals, hydro-carbons,

and mildew and should be ret resistant. Anchor Plns: Hold down pins should have a minimum length of 18 inches (0.45 meters), and accompanying

- washers should have a minimum diameter of 1 inch (2.5 centimeters). Sandbags: Sandbags should consist of materials which are resistant to ultra-violet radiation, tearing, and
- puncture and should be woven tightly enough to prevent leakage of fill material (i.e., sand, fine gravel, etc.). Sheeting: Sheeting should consist of polyethylene or other material which is impervious and resistant to puncture and tearing.

INSTALLATION GUIDELINES

All crosion and sediment control devices, including mandatory dewatering basins, should be installed as the first

order of business according to a plan approved by the WMA or local authority. Installation should proceed from upstream to downstream during periods of low flow. Construction of fabric-based channel diversions involves channel excavation, placement of geotextile fabric, and installation of flow diverters for both the main channel and all tributaries contributing flow to the work area (refer to

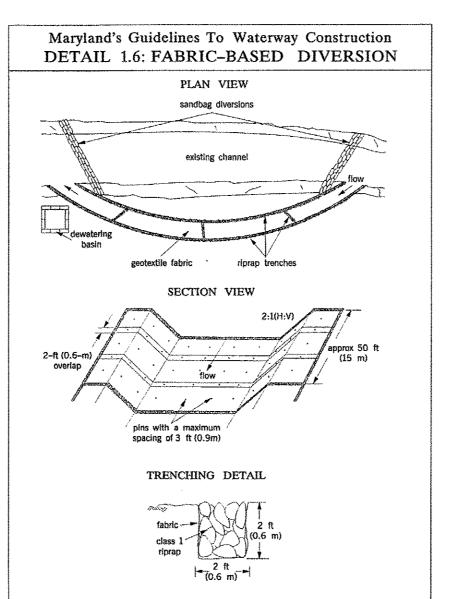
Detail 1.6). Channel Excavation

- }. All disturbances resulting from construction of the channel should be contained by appropriate sediment control
- 2. Excavation of the channel should begin at the downstream end and proceed upstream. The channel should have Encorrection of the channel should begin at the dominator the and proceed spacetime. The channel should a minimum capacity sufficient to convey the stream's base flow for projects with duration of 2 weeks or 1 For projects of longer duration, channels should have a capacity sufficient to convey bankfull flow. All excavated materials should be stockgiled outside of the 100 year flood plain and temporarily stabilized to
- TEMPORARY INSTREAM CONSTRUCTION MEASURES MARYLAND DEPARTMENT OF THE ENVIRONMENT WATERWAY CONSTRUCTION GUIDELINES REVISED NOVEMBER 2000
 - Page 1.6 1

MGWC 1.6: FABRIC-BASED CHANNEL DIVERSION prevent re-entry into the stream channel,

- 3. The process of excavation and stabilization with fabric should be a continuous and uninterrupted operation. All materials should be on-site prior to channel construction
- The downstream and upstream connection to the natural channel should be constructed under dry conditions.
- The stream should be contained by sandbags along the opposing bank during the process of cutting the diversion channel into the natural stream channel. Excavation and stabilization should be a continuous and uninterrupted
- 5. All debris such as rocks, sticks, etc. should be removed and the channel surfaces made smooth so that the fabric will rest flush with the channel at all sides and bottom.
- Stabilization with Geotextile Fabric 1. The fabric should have a minimum width such that it is keyed in and anchored at the top of stream bank.
- 2. Fabric should be placed so that it rests flush with the channel at all points of contact.
- 3. Fabric should be placed such that one piece will line the entire channel. If this is not possible, fabric should be placed so that transverse overlapping occurs in accordance with the detail. Longitudinal overlaps should not be allowed. Upstream sections should overlap downstream sections. Overlap width should equal 2 feet (0.6 meters) minimun
- 4. The fabric should be keyed into 2 by 2-foot (0.6 by 0.6-meter) trenches located at the upstream edge and at 50foot (15.25-meter) intervals with the overlap placed nearest to each 50 feet increment. The key-in should be from top of channel to top of channel. Class I riprap should be carefully placed into the trench with zero drop
- heigt 5. The fabric sections should be secured with hold down pins and washers. Overlaps should be pinned along transverse and longitudinal axes with spacing equal to 3 feet (0.9 meters) maximum.
- 6. Sediment from surrounding areas of disturbance should not be allowed to enter the diversion channel. Alternate Methods of Placing the Fabric
- 1. The above design may be modified to allow sewing of the geotextile fabric. Sewing of the geotextile fabric, rather then overlapping, should eliminate the requirement for transverse placement of the fabric. Either transverse or longitudinal placement should work equally well.
- The spacing of the pins could be either larger or smaller depending on the anticipated velocities and thickness and type of geotextile fabric.
- . The entire bottom of the channel could be riprapped if high velocities are anticipated. When the area is riprapped, it is not required that the geotextile fabric underneath the riprap be pinned.
- **Removal of Diversion** Water should not be allowed through the natural stream until all construction is completed.
- 2. After redirecting the flow through the natural channel, all fabric should be removed from the temporary diversion. The diversion should then be backfilled and stabilized. Points of tie-in to the natural channel should be protected with riprap according to the riprap guidelines.
- **FEMPORARY INSTREAM CONSTRUCTION MEASURES** MARYLAND DEPARTMENT OF THE ENVIRONMEN WATERWAY CONSTRUCTION GUIDELINES REVISED NOVEMBER 2009

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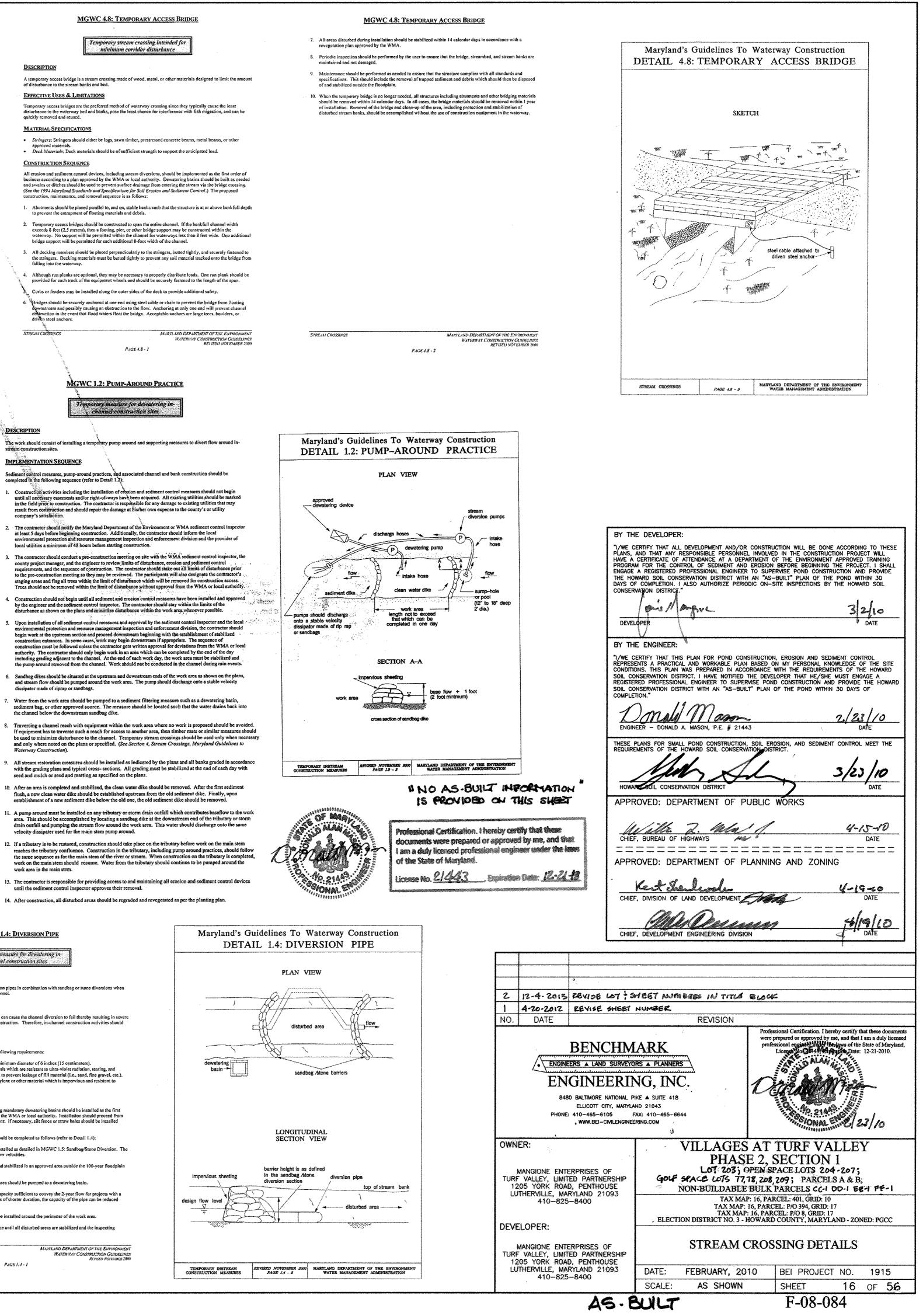
MARYLAND DEPARTMENT OF THE ENVIRONMENT WATER MANAGEMENT ADMINISTRATION



emporary stream crossing intended fo minimum corridor disturbance DESCRIPTION A temporary access bridge is a stream crossing made of wood, metal, or other materials designed to limit the amount **EFFECTIVE USES & LIMITATIONS** Comporary access bridges are the preferred method of waterway crossing since they typically cause the least disturbance to the waterway bed and banks, pose the least chance for interference with fish migration, and can be quickly removed and reused. MATERIAL SPECIFICATIONS · Stringers: Stringers should either be logs, sawn timber, prestressed concrete beams, metal beams, or other Deck Materials: Deck materials should be of sufficient strength to support the anticipated load. CONSTRUCTION SEQUENCE All erosion and sediment control devices, including stream diversions, should be implemented as the first order of business according to a plan approved by the WMA or local authority. Dewatering basins should be built as needed and swales or ditches should be used to prevent surface drainage from entering the stream via the bridge crossing. (See the 1994 Maryland Standards and Specifications for Soil Erosion and Sediment Control.) The proposed struction, maintenance, and removal sequence is as follow: 1. Abutments should be placed parallel to, and on, stable banks such that the structure is at or above bankfull depth o prevent the entrapment of floating materials and debris. Temporary access bridges should be constructed to span the entire channel. If the bankfull channel width exceeds 8 feet (2.5 meters), then a footing, pier, or other bridge support may be constructed within the waterway. No support will be permitted within the channel for waterways less than 8 feet wide. One additional bridge support will be permitted for each additional 8-foot width of the channel. All decking members should be placed perpendicularly to the stringers, butted tightly, and securely fustened to the stringers. Decking materials must be butted tightly to prevent any soil material tracked onto the bridge from falling into the waterway 4. Although run planks are optional, they may be necessary to properly distribute loads. One run plank should be ovided for each track of the equipment wheels and should be securely fastened to the length of the spar Curbs or fenders may be installed along the outer sides of the deck to provide additional safety Bridges should be securely anchored at one end using steel cable or chain to prevent the bridge from floating downstream and possibly causing an obstruction to the flow. Anchoring at only one end will prevent channel obstruction in the event that flood waters float the bridge. Acceptable anchors are large trees, boulders, or driven steel anchors. STREAM CROSS MARYLAND DEPARTMENT OF THE ENVIRONMEN REVISED NOVEMBER 200 PAGE 4.8 - 1 MGWC 1.2: PUMP-AROUND PRACTICE DESCRIPTION The work should consist of installing a temporary pump around and supporting measures to divert flow around in-MPLEMENTATION SEQUENCE

- Sediment control measures, pump-around practices, and associated channel and bank construction should be completed in the following sequence (refer to Detail 1.2)
- Construction activities including the installation of erosion and sediment control measures should not begin until all necessary easements and/or right-of-ways have been acquired. All existing utilities should be marked in the field prior to construction. The contractor is responsible for any damage to existing utilities that may result from construction and should repair the damage at his/her own expense to the county's or utility company's satisfaction
- The contractor should notify the Maryland Department of the Environment or WMA sediment control inspector at least 5 days before beginning construction. Additionally, the contractor should inform the local nental protection and resource management inspection and enforcement division and the provider of local utilities a minimum of 48 hours before starting construction 3. The contractor should conduct a pre-construction meeting on site with the WMA sediment control inspector, the
- county project manager, and the engineer to review limits of disturbance, erosion and sediment control requirements, and the sequence of construction. The contractor should stake out all limits of disturbance prior to the pre-construction meeting so they may be reviewed. The participants will also designate the contractor's staging areas and flag all trees within the limit of disturbance which will be removed for construction access. Trees should not be removed within the limit of disturbance without approval from the WMA or local authority
- 4. Construction should not begin until all sediment and erosion control measures have been installed and approved. by the engineer and the sediment control inspector. The contractor should stay within the limits of the disturbance as shown on the plans and minimize disturbance within the work area whenever possible
- mental protection and resource management inspection and enforcement division, the contractor should begin work at the upstream section and proceed downstream beginning with the establishment of stabilized construction entrances. In some cases, work may begin downstream if appropriate. The sequence of construction must be followed unless the contractor gets written approval for deviations from the WMA or local authority. The contractor should only begin work in an area which can be completed by the end of the day including grading adjacent to the channel. At the end of each work day, the work area must be stabilized and
- 6. Sandbag dikes should be situated at the upstream and downstream ends of the work area as shown on the plans and stream flow should be pumped around the work area. The pump should discharge onto a stable velocity dissipater made of riprap or sandbags.
- Water from the work area should be pumped to a sediment filtering measure such as a dewatering basin, sediment bag, or other approved source. The measure should be located such that the water drains back into the channel below the downstream sandbag dike.
- 8. Traversing a channel reach with equipment within the work area where no work is proposed should be avoided. If equipment has to traverse such a reach for access to another area, then timber mats or similar measures should be used to minimize disturbance to the channel. Temporary stream crossings should be used only when necessary and only where noted on the plans or specified. (See Section 4, Stream Crossings, Maryland Guidelines to
- 9. All stream restoration measures should be installed as indicated by the plans and all banks graded in accordance with the grading plans and typical cross- sections. All grading must be stabilized at the end of each day with seed and mulch or seed and matting as specified on the plans.
- 10. After an area is completed and stabilized, the clean water dike should be removed. After the first sediment flush, a new clean water dike should be established upstream from the old sediment dike. Finally, upon establishment of a new sediment dike below the old one, the old sediment dike should be removed.
- 11. A pump around must be installed on any tributary or storm drain outfall which contributes baseflow to the work area. This should be accomplished by locating a sandbag dike at the downstream end of the tributary or storm drain outfall and pumping the stream flow around the work area. This water should discharge onto the same velocity dissipater used for the main stem pump around.
- 12. If a tributary is to be restored, construction should take place on the tributary before work on the main stem reaches the tributary confluence. Construction in the tributary, including pump around practices, should follow the same sequence as for the main stem of the river or stream. When construction on the tributary is completed, work on the main stem should resume. Water from the tributary should continue to be pumped around the work area in the main stem.
- until the sediment control inspector approves their removal.





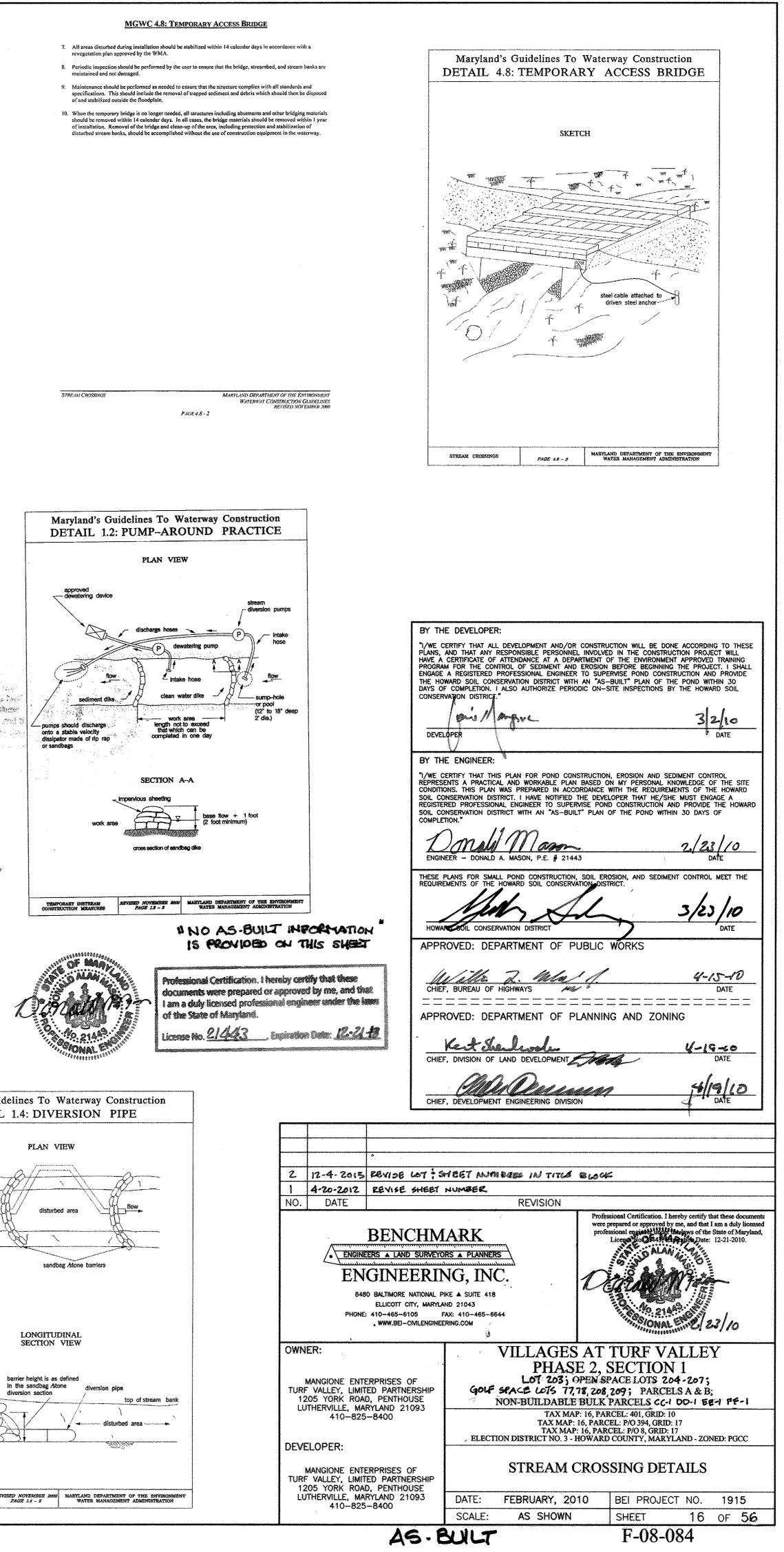
MGWC 1.4: DIVERSION PIPE

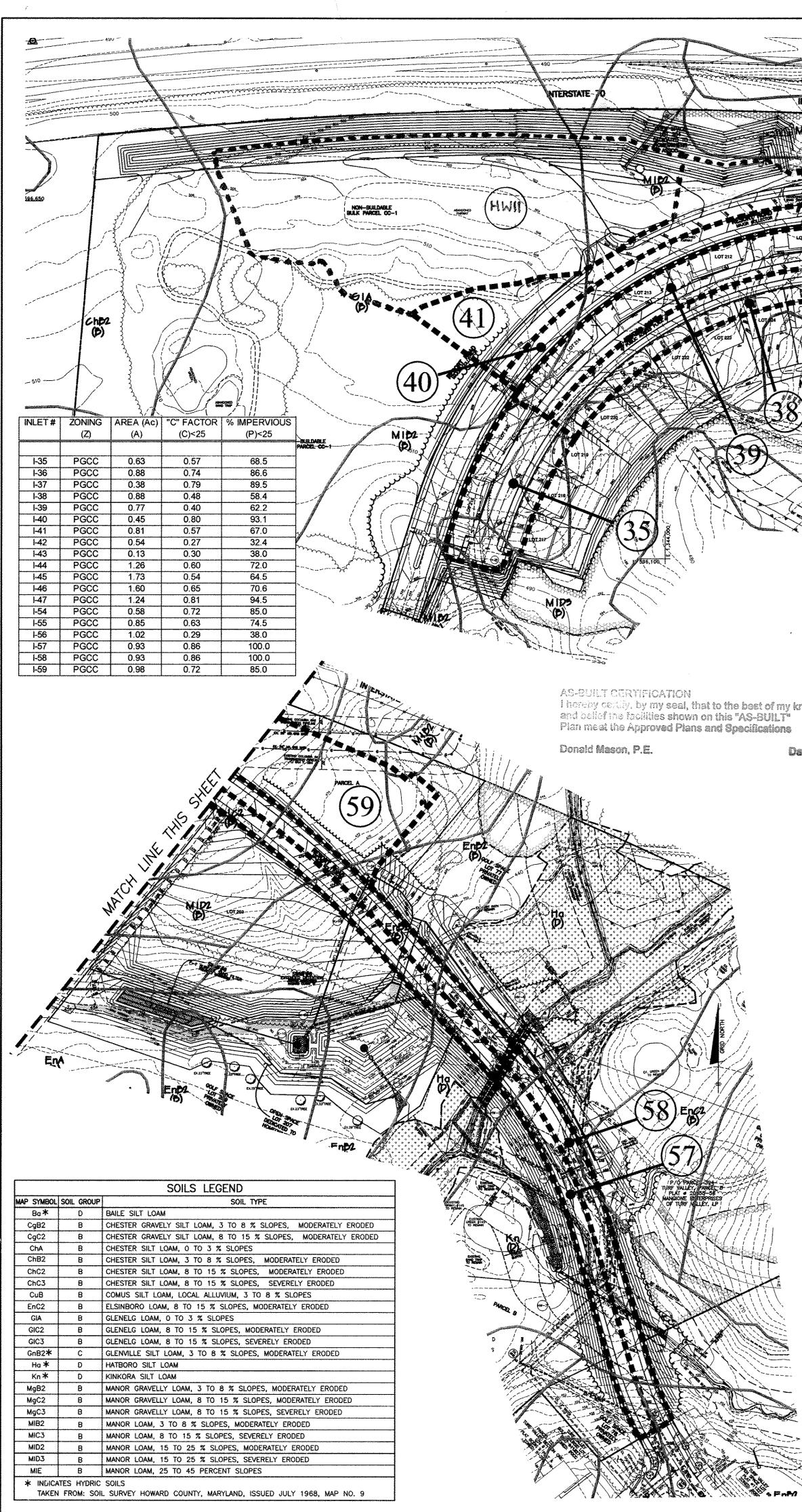
Temporary measure for dewatering in channel construction sites DESCRIPTION

- The work should consist of installing flow diversion pipes in combination with sandbag or stone diversions when construction activities occur within the stream channel. **EFFECTIVE USES & LIMITATIONS**
- Diversion pipes with an insufficient flow capacity can cause the channel diversion to fail thereby resulting in severe

erosion of the disturbed channel section under construction. Therefore, in-channel construction activities should occur only during periods of low flow.

- MATERIAL SPECIFICATIONS Materials for stream diversions should meet the following requirements:
- Riprap: Stone should be washed and have a minimum diameter of 6 inches (15 centimeters). . Sundhags: Sandbags should consist of materials which are resistant to ultra-violet radiation, tearing, and
- puncture and should be woven tightly enough to prevent leakage of fill material (i.e., sand, fine gravel, etc.), · Sheeting: Sheeting should consist of polycthylene or other material which is impervious and resistant to puncture and learing.
- INSTALLATION GUIDELINES All crosion and sediment control devices including mandatory dewatering basins should be installed as the first order of business according to a plan approved by the WMA or local authority. Installation should proceed from
- pstream to downstream during low flow conditions. If necessary, silt fence or straw bales should be installed around the perimeter of the work area. Diversion pipes with sandbag or stone barriers should be completed as follows (refer to Detail 1.4):
- 1. Sandbag/stone barriers should be sized and installed as detailed in MGWC 1.5: Sandbag/Stone Diversion. The materials should be sized to withstand baseflow velocities.
- 2. All excavated material should be deposited and stabilized in an approved area outside the 100-year floodplain unless otherwise authorized by the WMA. 3. Sediment-laden water from the construction area should be pumped to a dewatering basin.
- 4. The diversion pipe should have a minimum capacity sufficient to convey the 2-year flow for projects with a duration of two weeks or greater. For projects of shorter duration, the capacity of the pipe can be reduced
- accordingiy 5. If necessary, silt fence or straw bales should be installed around the perimeter of the work area.
- 6. Sediment control devices are to remain in place until all disturbed areas are stabilized and the inspecting uthority approves their removal
- TEMPORARY INSTREAM CONSTRUCTION MEASURES MARYLAND DEPARTMENT OF THE ENVIRONMENT WATERIFAT CONSTRUCTION GUIDELINES Revised Northmen 2000 PAGE 1.4 - 1

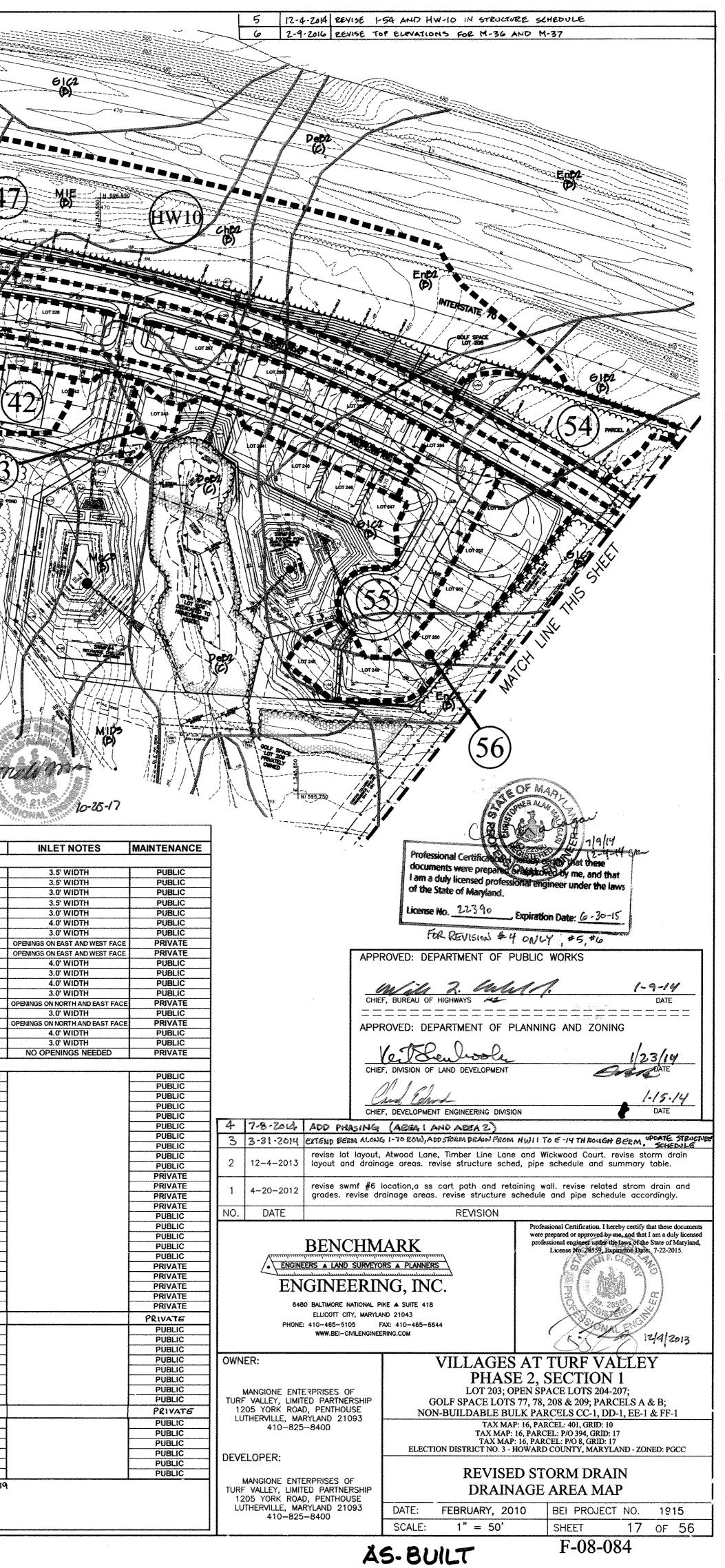


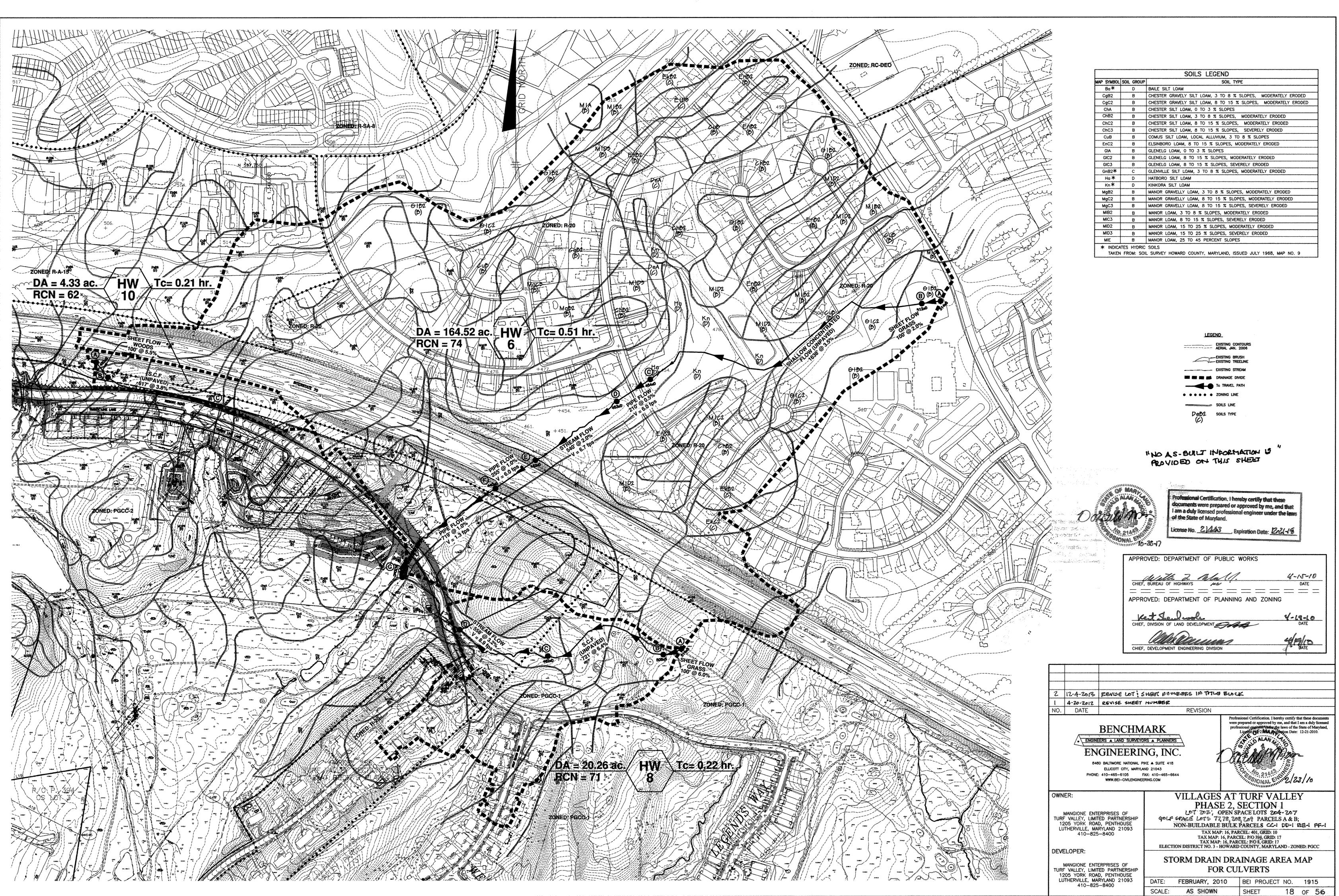


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STRUCTURE LOCATION FOR INLETS IS AT THE CENTER OF THE INLET FACE FOR A-5 AND A-10 INLETS AND AT THE TOP CENTER OF D-INLETS. STRUCTURE LOCATION FOR THE END-SECTIONS IS AT THE MIDPOINT OF THE END OF THE STRUCTURE.

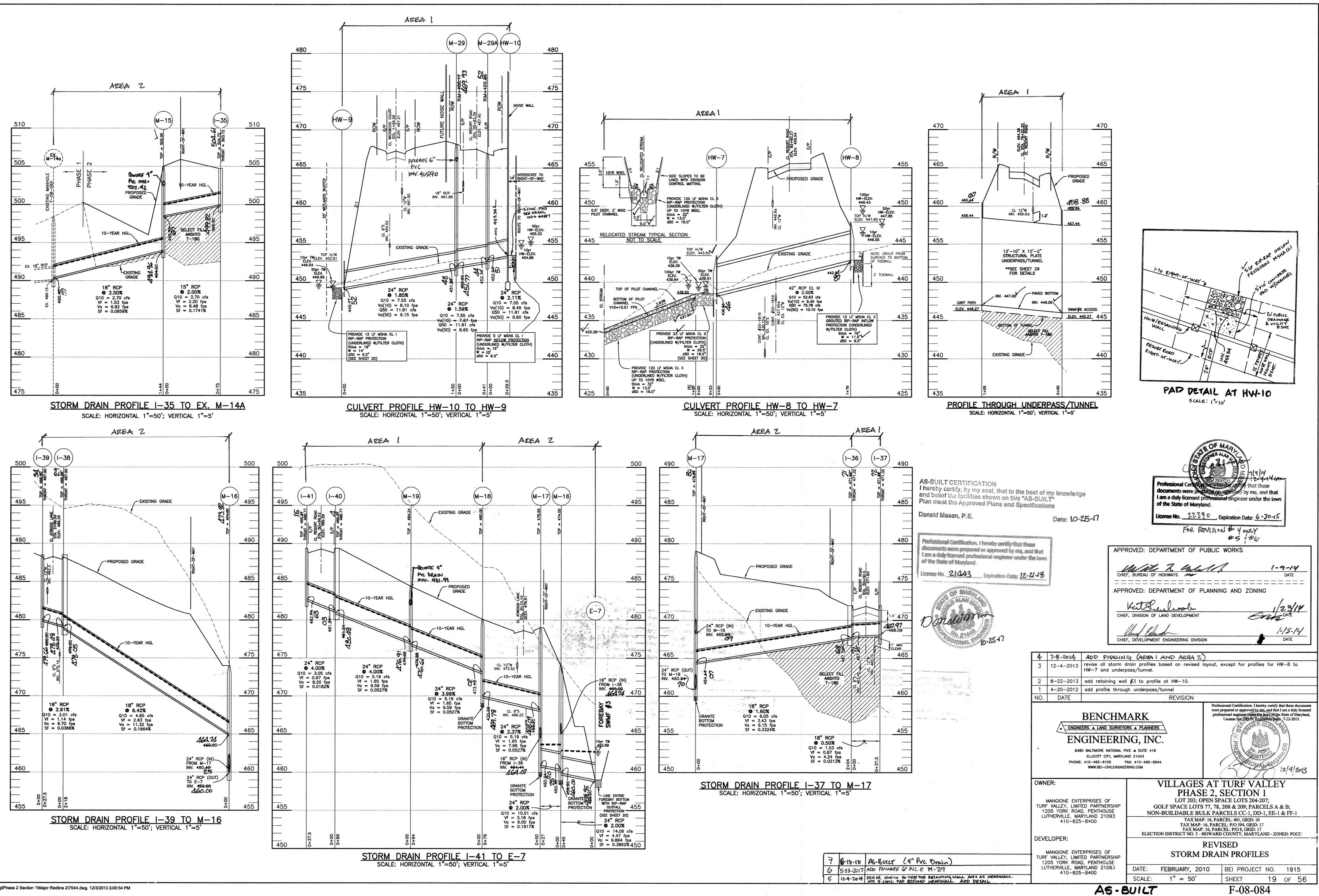
PRECAST STRUCTURES MEETING HS-20 LOADING MAY BE USED.

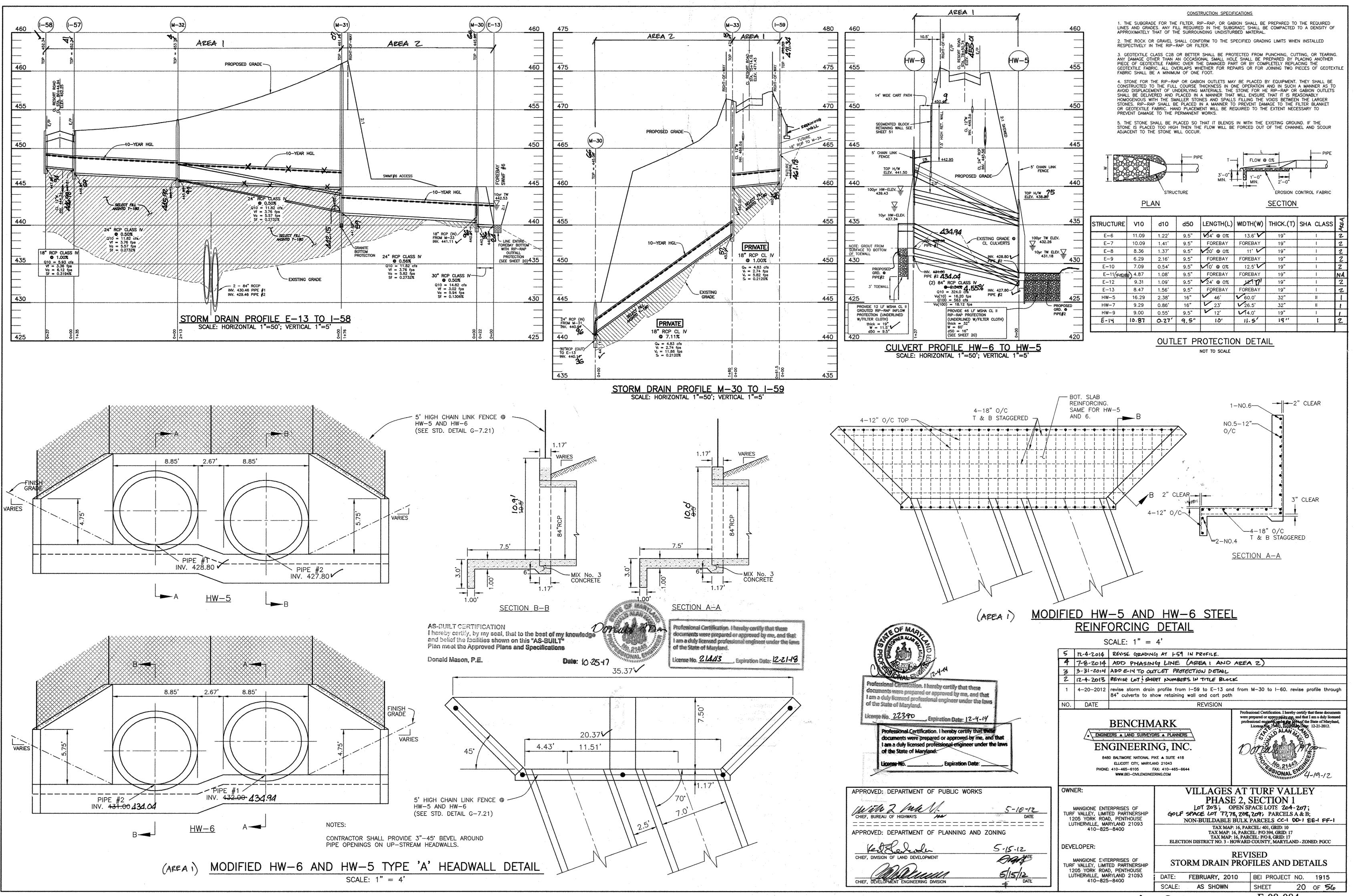




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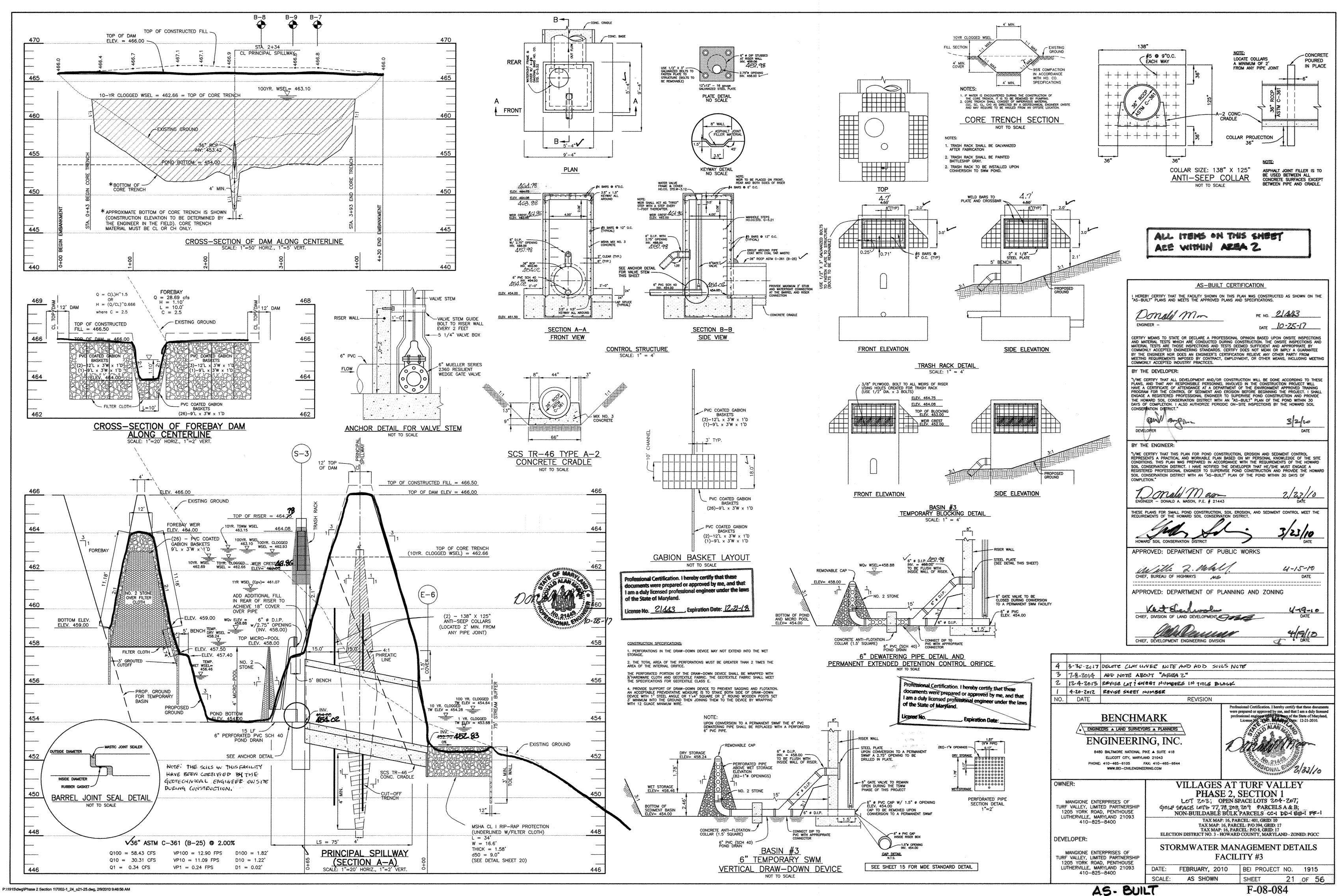


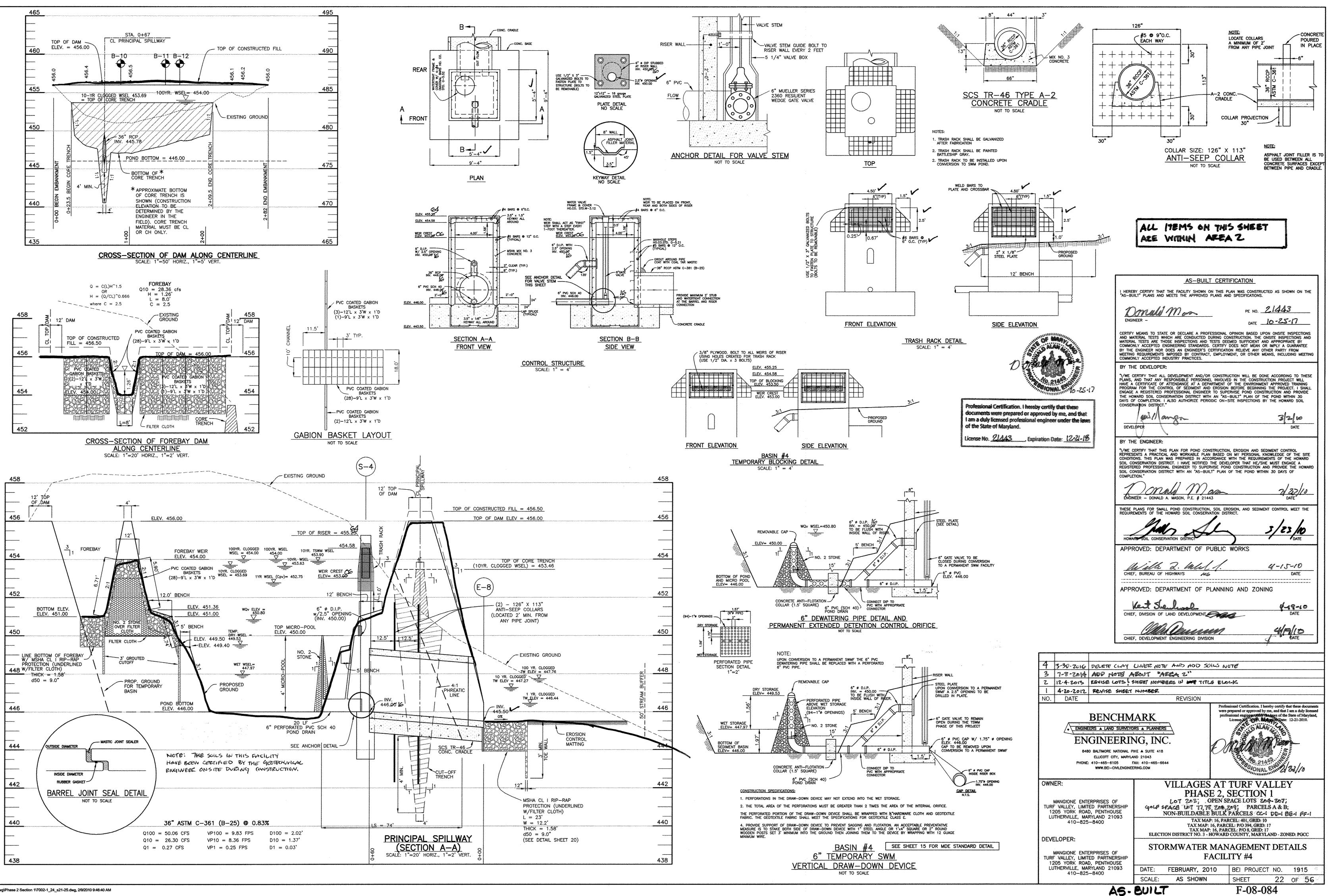


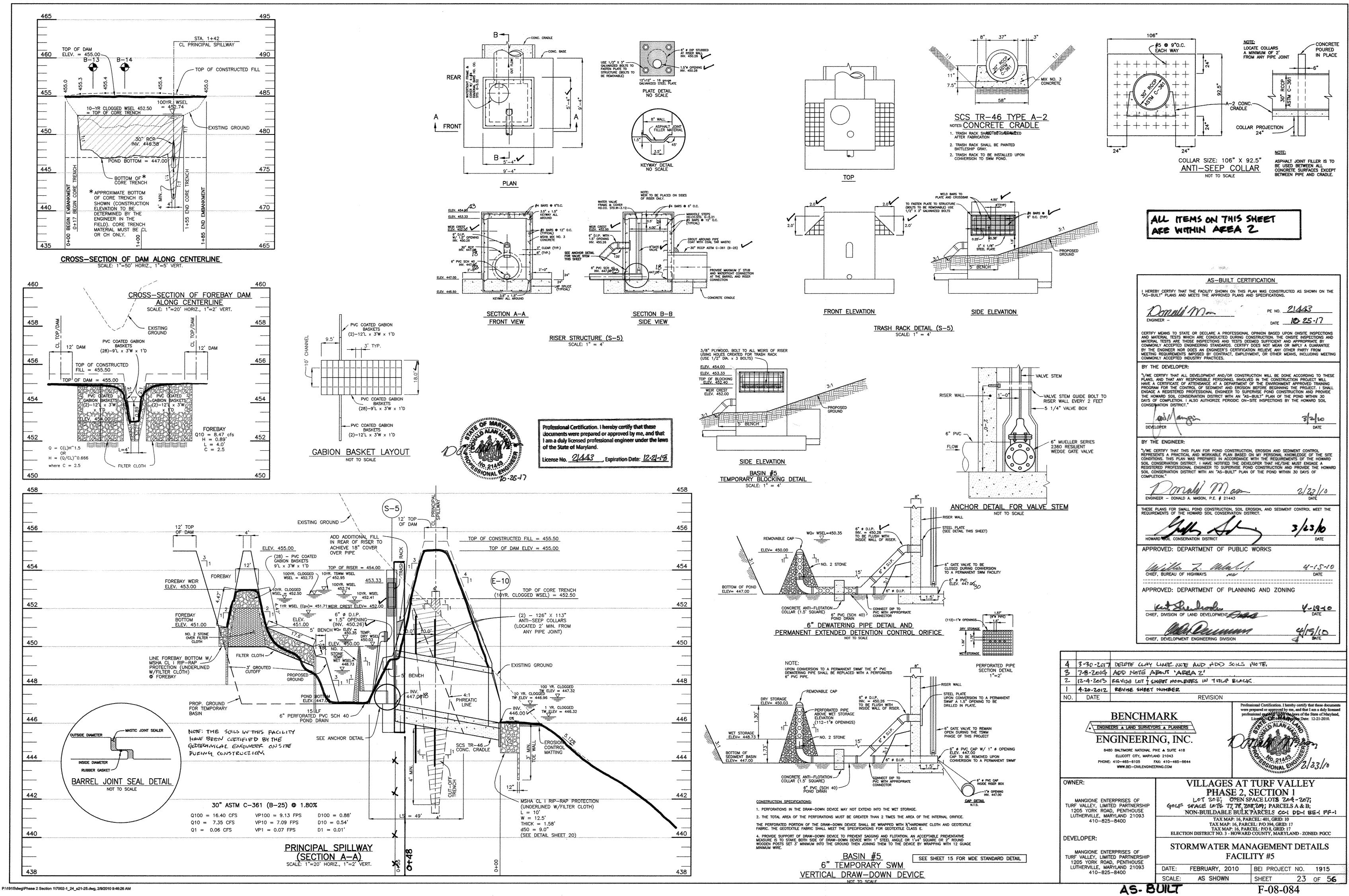
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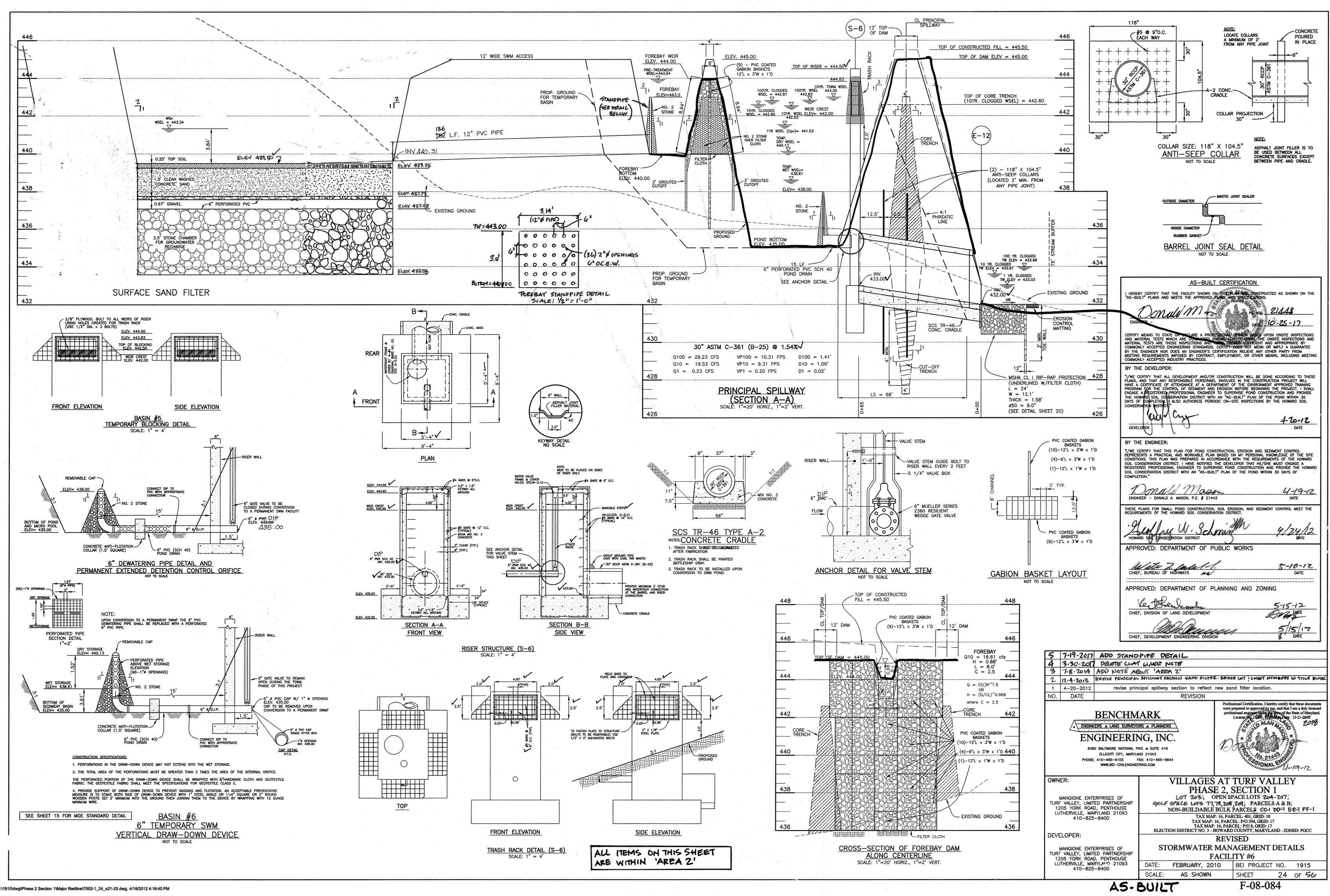
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shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the toe of the embankment.

dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

<u>Material — The fill material shall be taken from approved designated borrow areas. If shall</u> be free of roots, stumps, wood, rubbish, stones greater than 6", frozen or other objectionable material. Fill material for the center of the embankment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have

be placed in the downstream portions of the embankment. The principal spillway must be

controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, subber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with teh

95% of maximum dry density with a moisture content within \pm 2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified Method T-99 (Standard Proctor).

parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be a least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be and minimum permeability.

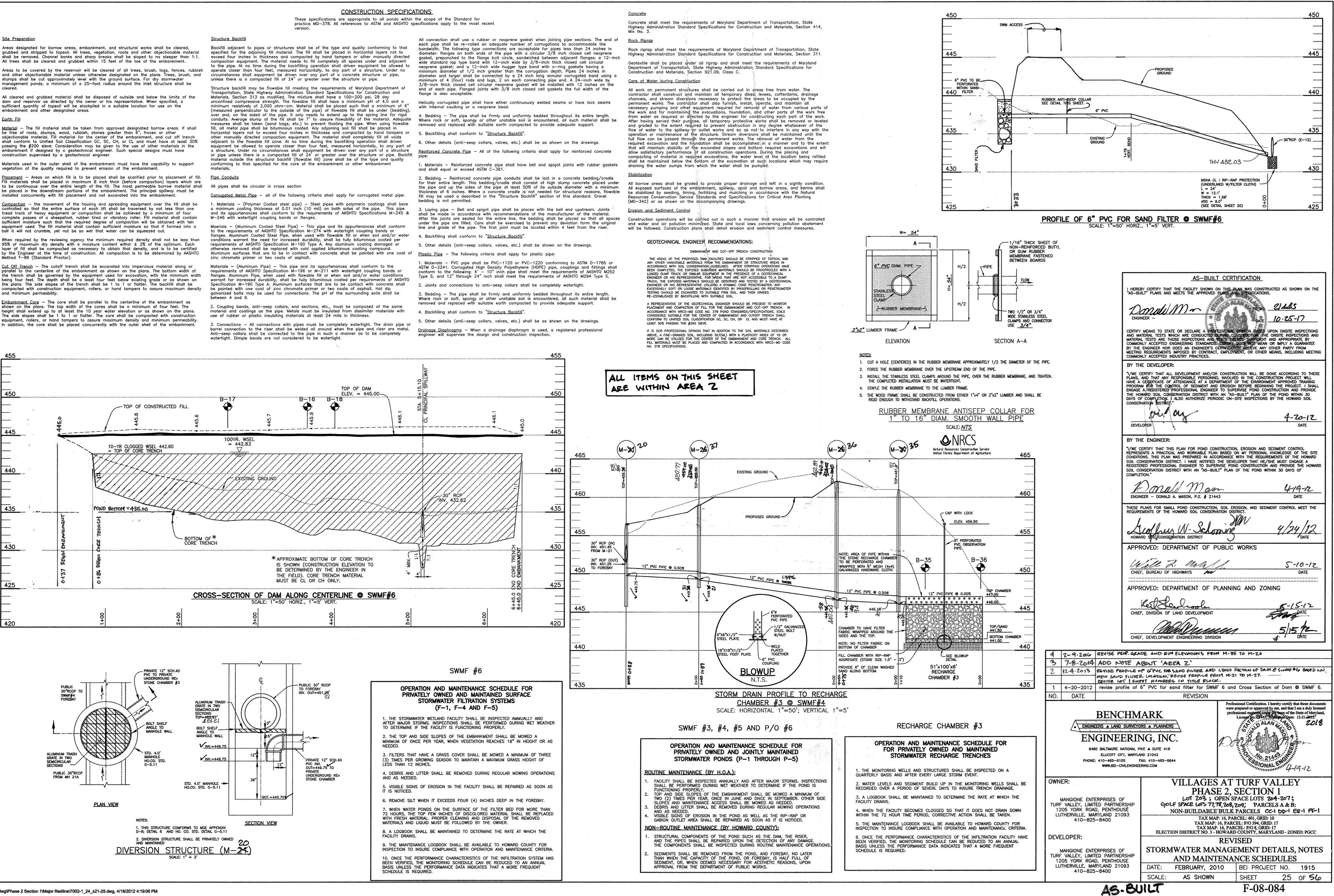
Embankment Core — The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the cores shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability In addition, the core shall be placed concurrently with the outer shell of the embankment.

each pipe shall be re-rolled an adequate number of corrugations to accommodate the diameter: flanges on both ends of the pipe with a circular 3/8 inch closed cell neoprene wide standard lap type band with 12-inch wide by 3/8-inch thick closed cell circular minimum of 4 (four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8—inch thick closed cell circular neoprene gasket will be installed with 12 inches on the

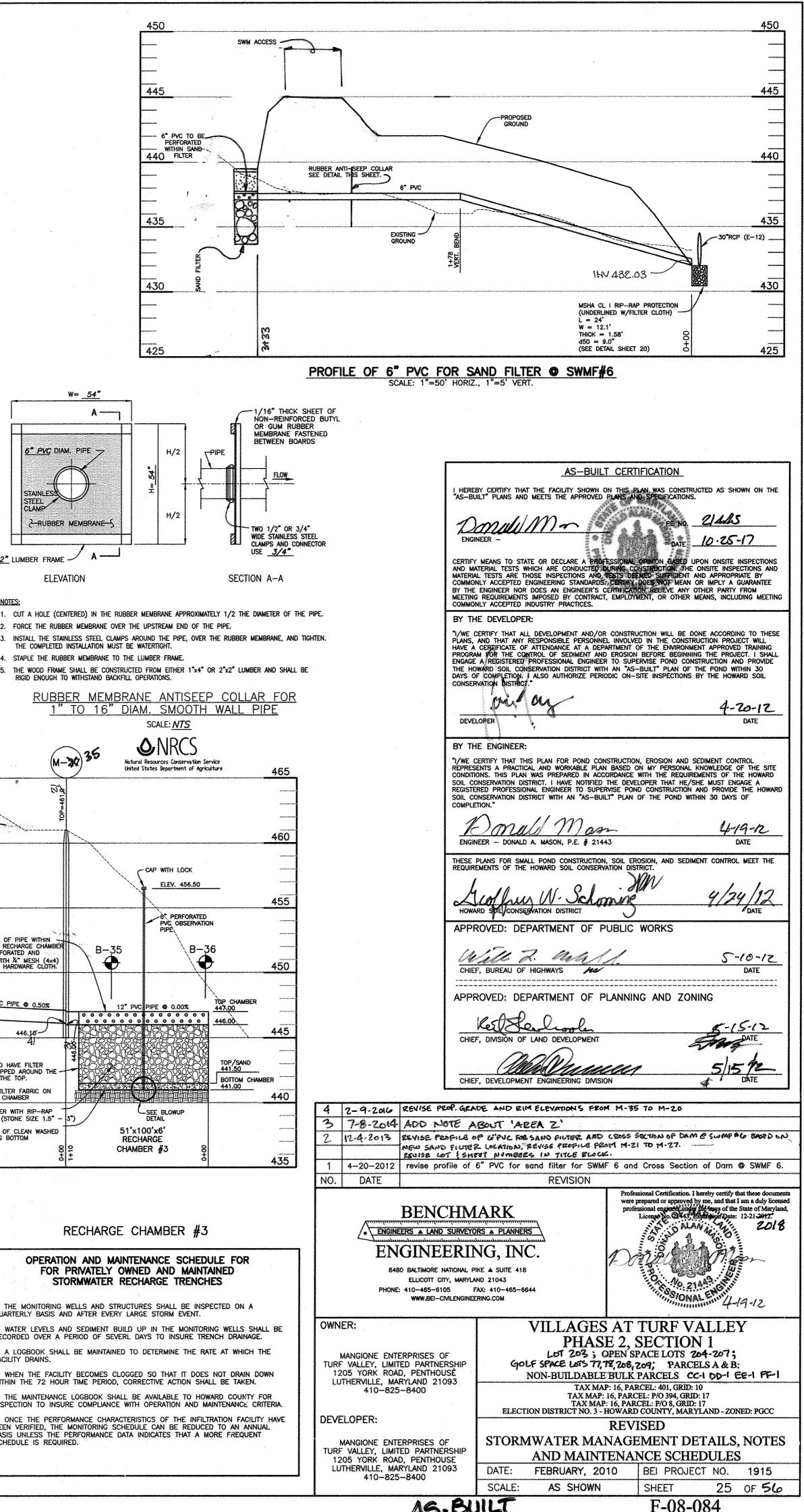
6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

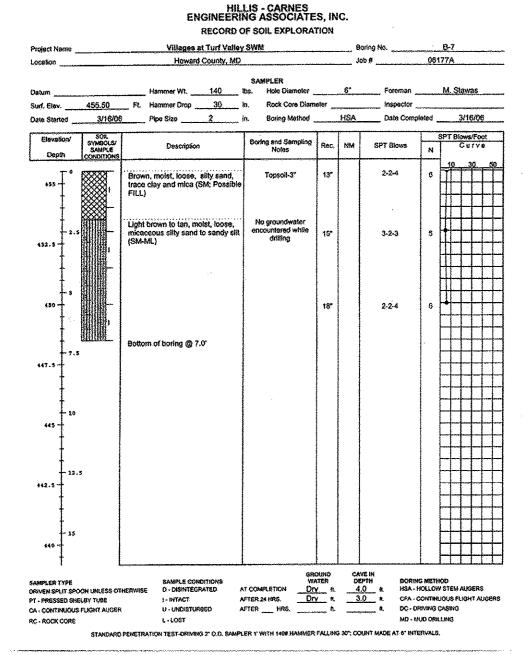
. Bedding — Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle

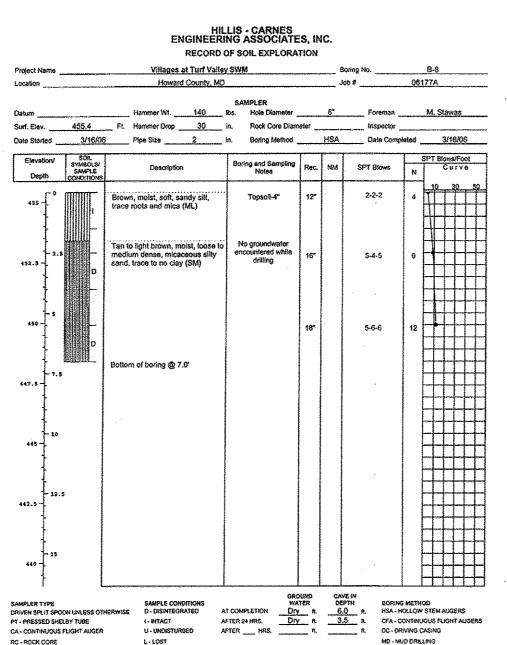
5. Other details (anti-seep collars, valves, etc.) shall be shown on the drawings.











STANDARD PENETRATION TEST-DRIVING 2" O.D. SAMPLER 1" WITH 140# HAMMER FALLING 30"; COUNT MADE AT 5" INTERVALS.

457.5

447.5-

RC - ROCK CORE

Location

452.5

SAMPLER TYPE

RC - ROCK CORE

L-LOST

and the second second second

STANDARD PENETRATION TEST-DRIVING 2" O.D. SAMPLER 1' WITH 1408 HAMMER FALLING 30"; COUNT MADE AT 6" INTERVALS.

NO - MUO DRELINO

RC - ROCK CORE

L-LOST

STANDARD PENETRATION TEST-DRIVING 2" O.D. SAMPLER 1" WITH 1408 HAMMER FALLING 30". COUNT MADE AT 6" INTERVALS.

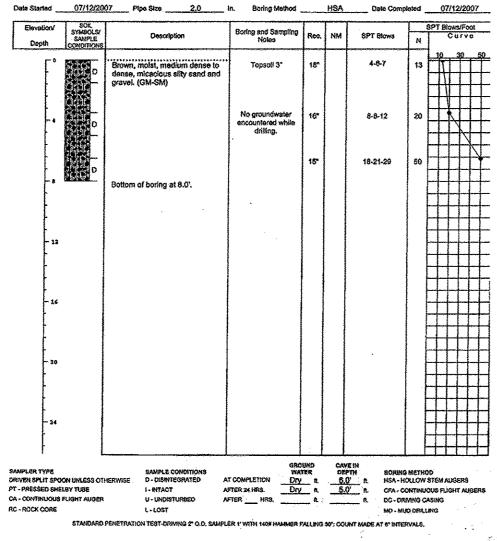
HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION Project Name _____ Villages at Turf Valley SWM _____ Boring No, _____ B-14 Location ______ Job # ______ Job # ______ 06177A SAMPLER Hammer Wi. 140 Bs. Hole Diameter 6" Foreman M. Stawas Surf. Elev. 453.80 FL Hammer Drop 30 in. Rock Core Diameter ______ Inspector ______ Date Started 3/16/06 Pipe Size 2. in. Boring Method HSA Date Completed 3/16/06 Elevation/ SOIL SYNBOLS/ Description BAMPLE Depth CONDITIONS Boring and Sampling Rec. NM SPT Blows N Curve

Depiti	CONDITIONS	[1	Í			4		
ſ		Light brown, moist, medium stilf, micaceous sandy silt, trace roots	Topso#-4"	15"		4-3-4	7		Î	Ţ
4 52.5 -		(ML)								
- 2.9	-	Light brown, moist, medium dense, micaceous silty sand to sandy silt (SM-ML)	No groundwater encountered while drilling	16*		4-9-13	22			
450	D 									-
				18"		9-11-14	25			
**************************************	- 1					*				
		Gray, moist, medium dense, micaceous silty sand (SM)								
445	- -	tilicaceoos sity sano (ow)		14"		9-7-7	14	-4		+
10				15*		8-9-13	22			
452.5 -	1	Bottom of boring @ 11.5'								
- 12.	5	7.1°								-
9. č.						-				╞
***										╞
- 13										
437.5 - SAMPLER TYPE DRIVEN SPLIT SPO PT - PRESSED SHE CA - CONTINUOUS	LBY TUBE	I - INTACT	AT COMPLETION Dry AFTER 24 HRS. 4.5 AFTER HRS. 4.5		CAV DE <u>6.</u>		ollow Ontinu	STEM /		
RC - ROCK CORE		L-LOST				加口・私(AD ORIU	LING		

RC - ROCK CORE 1 - LOST STANDARD PENETRATION YEST-DRIVING 2" O.D. SAMPLER 1" WITH 146# HAMMER FALLING 30", COUNT MADE AT 8" INTERVALS.

HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION

Villages at Turf Valley SWM Boring No. ______B-36 Howard County, Maryland Job # _____ 06177A_____ Hammer WL 140 ibs. Hole Diameter 2.25 Foreman T. Carroli Surf. Elev. 449.00 Ft. Haramer Drop 30 In. Rock Core Diameter Inspector Elevation/



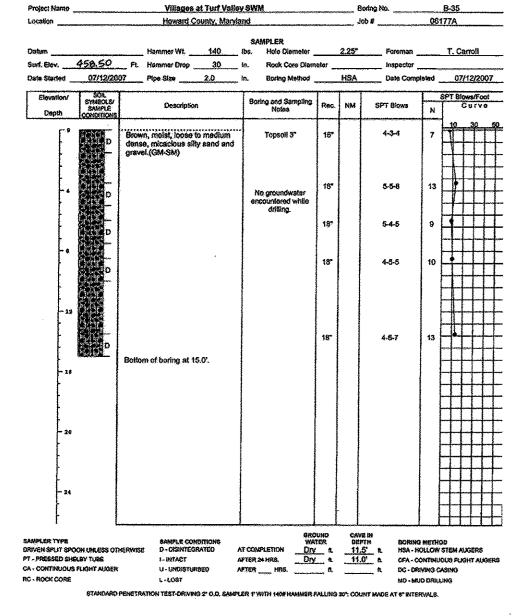
IVEN SPLIT SPOON UNLESS OTHERWISE D-DISINTEGRATED AT COMPLETION <u>Dry n. 7.0 n.</u> HSA-HOLLOW STEM AUGERS - PRESSED SHELBY TUBE I- INITACY AFTER 24 HRS. <u>4.0 n. 5.5 n.</u> CFA-CONTINUOUS FLIGHT AUGERS - CONTINUOUS FLIGHT AUGER U-UNDISTURGED AFTER HRS. <u>n.</u> n. DC-DRIVING CASING	Location			Howar	t County, MD				loo #	061	77A			
urf. Elev				1.10.00.00.00.00.00.00	140	SAMPLER	ator	£*	Encom		M 51	01425		
Instruct Open Size 2 Instruct Boring Mellod HSA Date Completed Stiff Blows/Foot Elevation/ Depin SMELLS SWELLS Description Boring and Sampling Notes Re. NM SPT Blows SPT Blows/Foot 1 Status Description Boring and Sampling Notes Re. NM SPT Blows SPT Blows/Foot 1 Status Description Boring and Sampling Notes Re. NM SPT Blows SPT Blows/Foot 1 Status Description Boring and Sampling Re. NM SPT Blows SPT Blows/Foot 1 Call Sampling Light brown, molal, each to medium on clary (ML) Topolk-57 15* 2-2-3 5 Interpretained SPT Blows/Foot 447.3 -				LIBUUUDEL AAFT	140	TOS. FUNCTION								
Cardwards Deput Several Samplero Note Description Description Description Rec. NM SPY Blows N CUrve F 1 Curve Commission Light brown, molel, each to medium in clay (ML) Topsol-5" 15" 2-2-3 5 10 10 30 60 1 Curve Commission Light brown, molel, each to medium in clay (ML) Topsol-5" 15" 2-2-3 5 10 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>6</td><td></td></td<>													6	
Dep0 CONNITIONES Constitutions Constitutions Topsoll.5" 15" 2.2-3 5 0	Elevation/	SYMBOLS/		Descriptio		Boring and Sam	ipting Rec	NM	SPT Blow					
450 2.3 stiff, micaceous sandy sill, trace to no clay (ML) No groundwater encountered with drilling 16° 2.3-5 8 447.3 -	Depin	CONDITIONS				60005		_						
447.5 -	C		still, m	ticaceous sand	oft to medium ly slit, trace to	Topsoil-5"	15*		2-2-3	5				
447.5 -	ł					No groundwa encountered w	ا ہم			•			$\frac{1}{1}$	
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437.5 -12.5 437.5 -13 MPLER TYPE SAMPLE CONDITIONS AT COMPLETION MER ALL ST AT COMPLETION AT COMPLETIO	{												11	
437.5 437.5 415 HINTACY ROCK CORE I- ROCK CORE K-ROCK K-ROCK K-ROCK K-ROCK K-ROCK K-ROCK K-ROCK K-ROCK K-ROCK K-ROCK	440-			•									╄╋	
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A15 A15 MPLER TYPE A55 MPLER TYPE SAMPLE CONDITIONS PRESSED SHEENY TUBE - PRESSED SHEENY	1- 12.	5				1			[┝╍╋╍	╋╋	[
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NPLER TYPE SAMPLE CONDITIONS GROUND WATER CAVE IN WATER BORING METHOD IVEN SPLIT SPOON UNLESS OTHERWISE D - DISINTEGRATED AT COMPLETION Dry n. 7.0 n. HSA - HOLLOW STEM AUGERS - PRESSED SHELEY TUBE I - INITACT AFTER 24 HRS. 4.0 n. 5.5 n. CFA - CONTINUOUS FLIGHT AUGERS - CONTINUOUS FLIGHT AUGER U - UNDISTURGED AFTER 24 HRS. 4.0 n. DC - DRIVING CASING - CONTINUOUS FLIGHT AUGER U - UNDISTURGED AFTER HRS.	ł											T	Ħ	1
NUMER TYPE SAMPLE CONDITIONS WATER DEPTH BORING METHOD NUMEN SPLIT SPOON UNLESS OTHERWISE D - DISINTEGRATED AT COMPLETION Dry n. 7.0 n. HSA - HOLLOW STEM AUGERS - PR\$SSED SHELEY TUBE I- NITACT A FER 24 HRS. 4.0 n. 5.5 n. CFA - CONTINUOUS FLIGHT AUGERS - CONTINUOUS FLIGHT AUGER U- UNDISTURBED AFER HBS. n. DC - DRIVING CASING ROCK CORE L - LOST MD - MUD DRILLING MD - MUD DRILLING	435					L		1	L				Ҵ	J
- ROCK CORE L-LOST NO-MUD DRILLING	- PRESSED SHE	LOY TUBE		D - DISINTEG	RATED	AFTER 24 HRS.	WATER Dry 1 4.0	- 7 7	ертн в 1 <u>0</u> а. н 1 <u>5</u> а. с	sa - Hollow Fa - Conting	STEM /			燽
	C - ROCK CORE					- structure	`	*****						
		STANDARD	PENETRA		13 2" O.D. SAMPI	LER 1' WITH 140# MA	MMER FALLP	46 30°; C4	OUNT MADE AT 6	INTERVALS.				

HILLIS - CARNES ENGINEERING ASSOCIATES, INC.

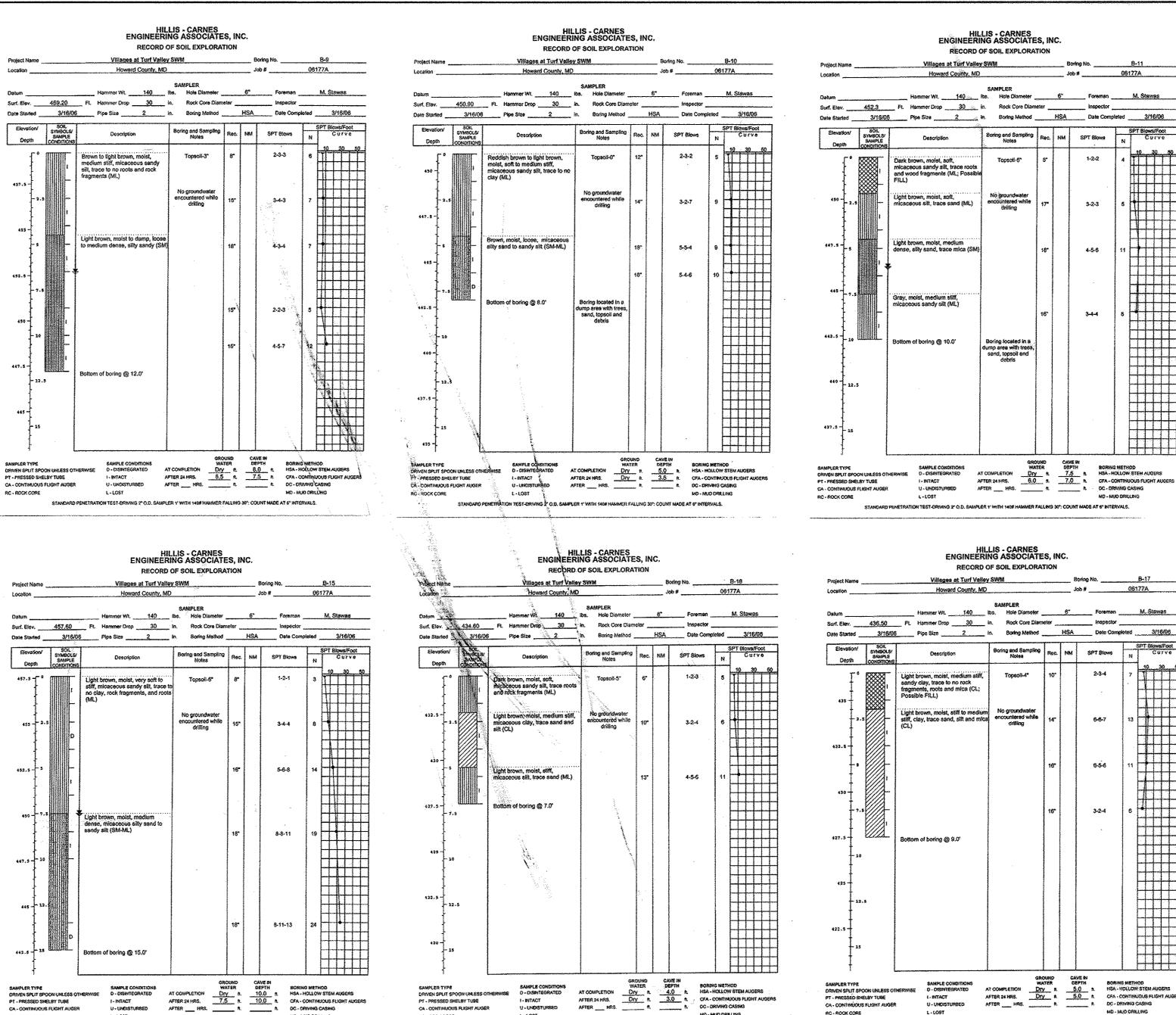
RECORD OF SOIL EXPLORATION

Villages at Turf Valley SWM Boring No. B-13

HILLIS - CARNES ENGINEERING ASSOCIATES, INC. RECORD OF SOIL EXPLORATION



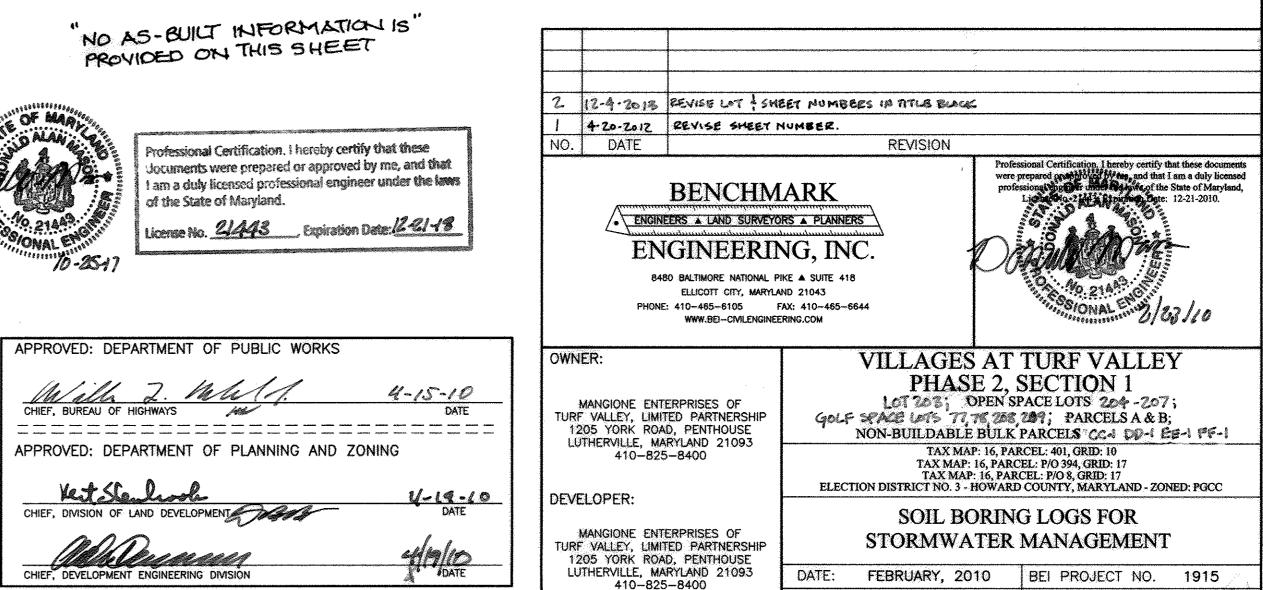
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RC - ROCK CORE



MO - MUD DRILLING

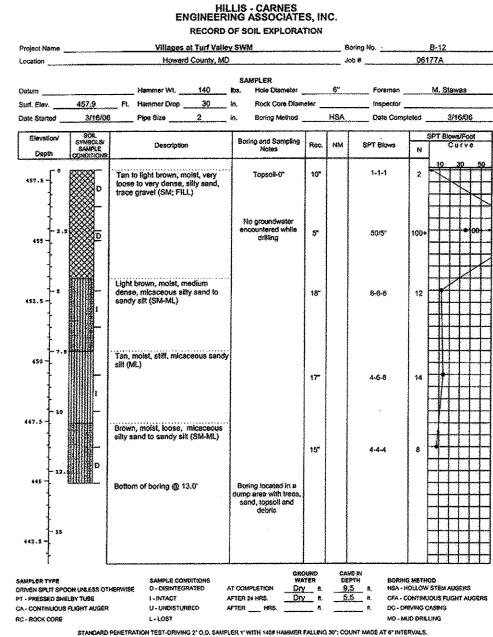


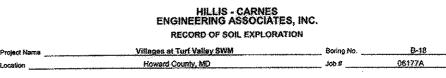
SCALE:

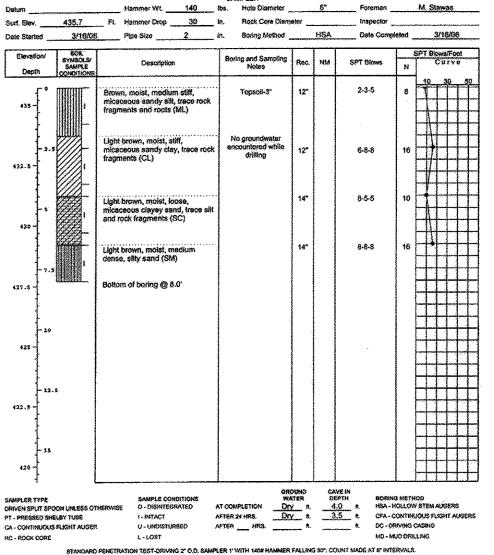
AS. BUILT

AS SHOWN

35.50		Howard	County, MC)			ab#	06	177A
36.50			Howard County, MD						
\$6.50				SAMPLER					
¥6.50		Hammer WL				.,			M. Stawas
		Hammer Drop					inspec		
3/16/06		Pipe Size	2	in. Boring Meth	×*	HSA	Date C	ompioted	3/16/06
SOIL				Boring and Samp	500				SPT Blows/Foot
MBOLS/		Description		Notes	Rec.	NM	SPT Blow	• N	Curve
OTTIONS)				+		-			10 30 50
88	Light	brown, moist, m	edium stiff,	Topsoll-4"	10"		2-3-4	7	
X		clay, trace to ni ients, roots and i						1	
XI	Possi	ble FILL)	onae fani		1				
8 I			, . ,						
7L I	Light	brown, moist, sti	If to median	No groundwate encountered wh	8 a 1			13	
12	(CL)	ay, trace sand,	SIC BID THO	dritting	^{ee} 14*		6-6-7	1 13	
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		SAMPLE CON		AT COMPLETION	WATER	05	PTH B	DRING METH	iod V stem Augens
nless off Ure	ERWISE	D - DISINTEGR		AT SOMPLETION	Dry a Dry a				UOUS FLIGHT AUGERS
nt alkoer		U-UNDISTUR		AFTER HRS				C - DRIVING	Contraction of the second s
		L-LOST					м	D - MUD ORI	LING





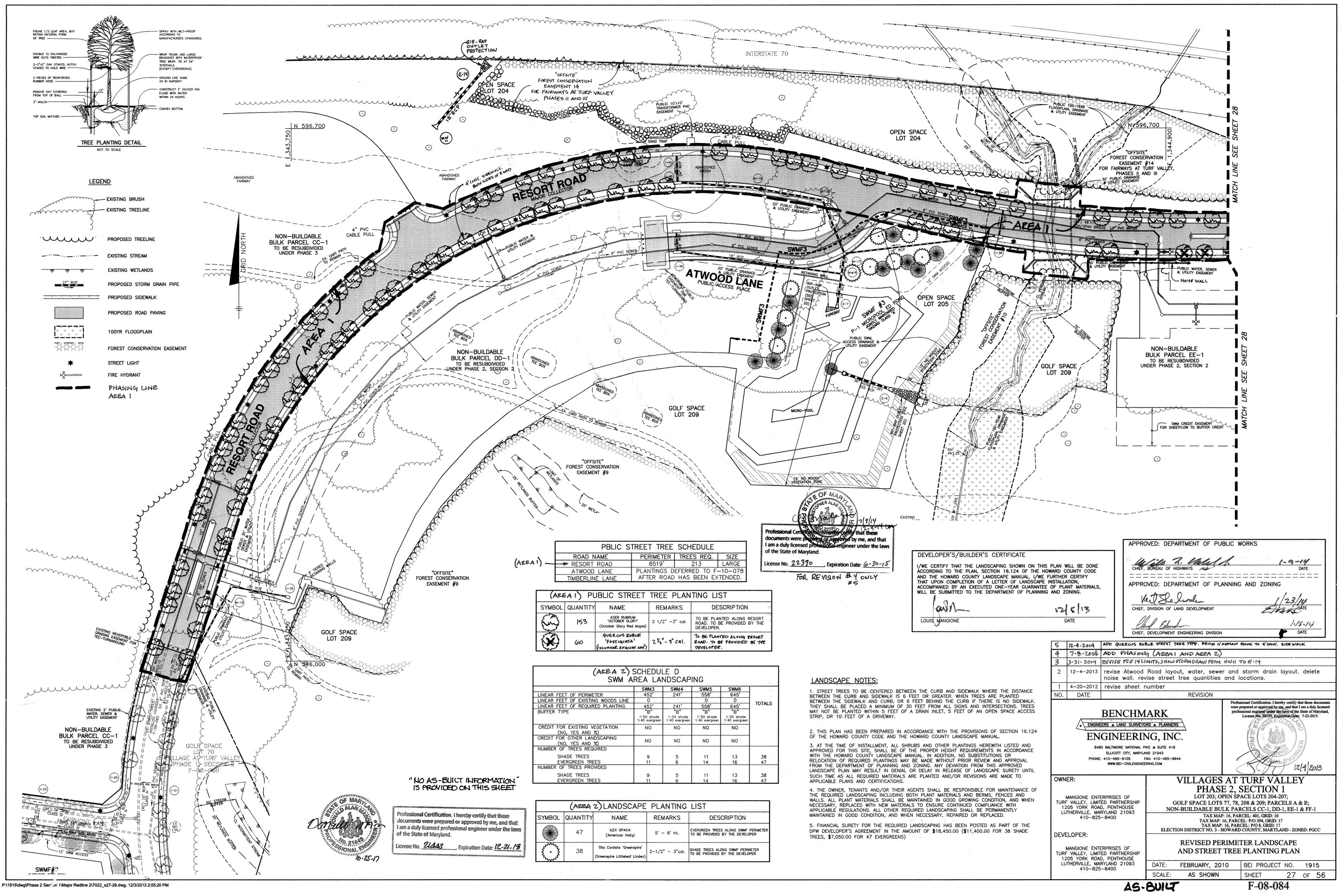


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SHEET

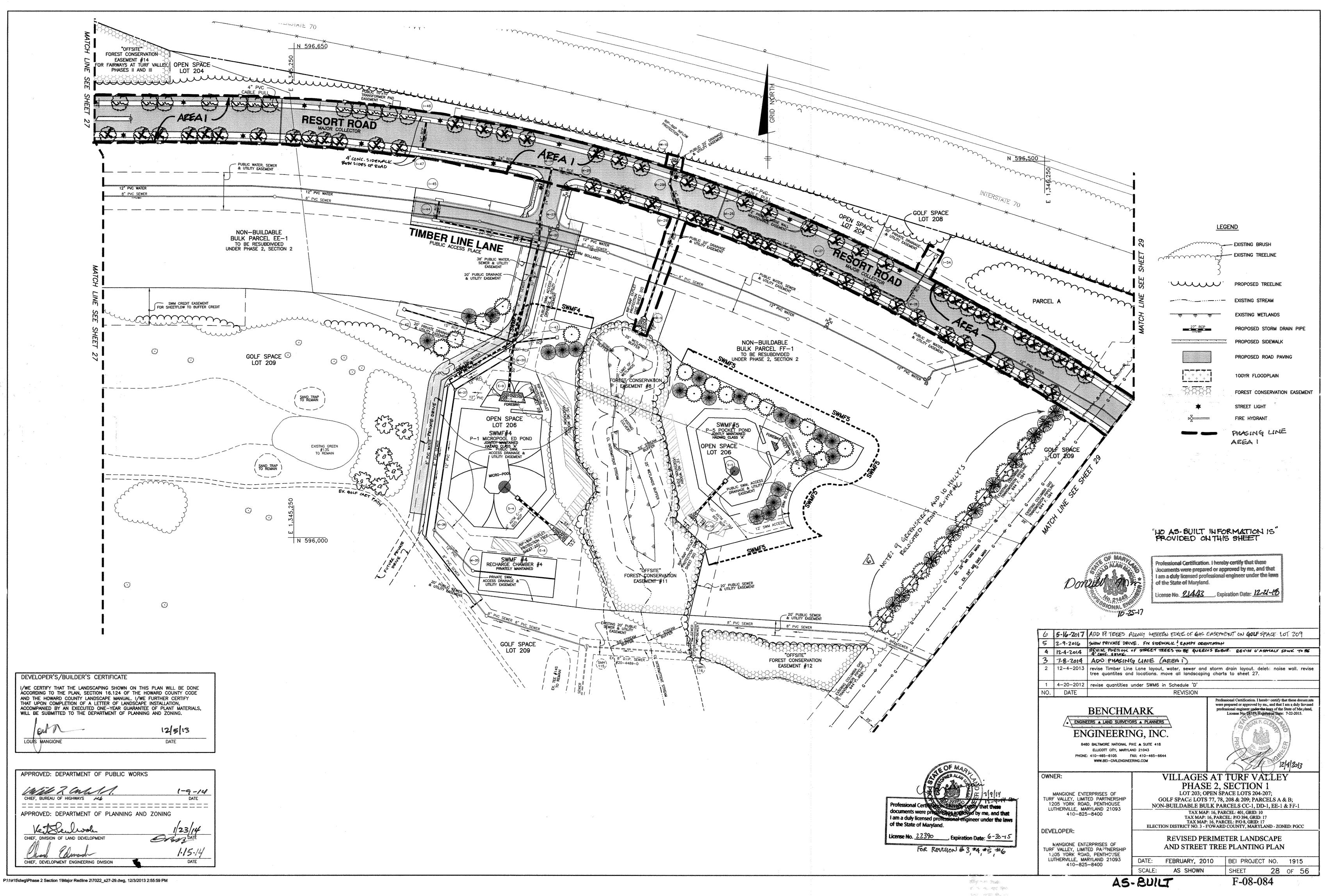
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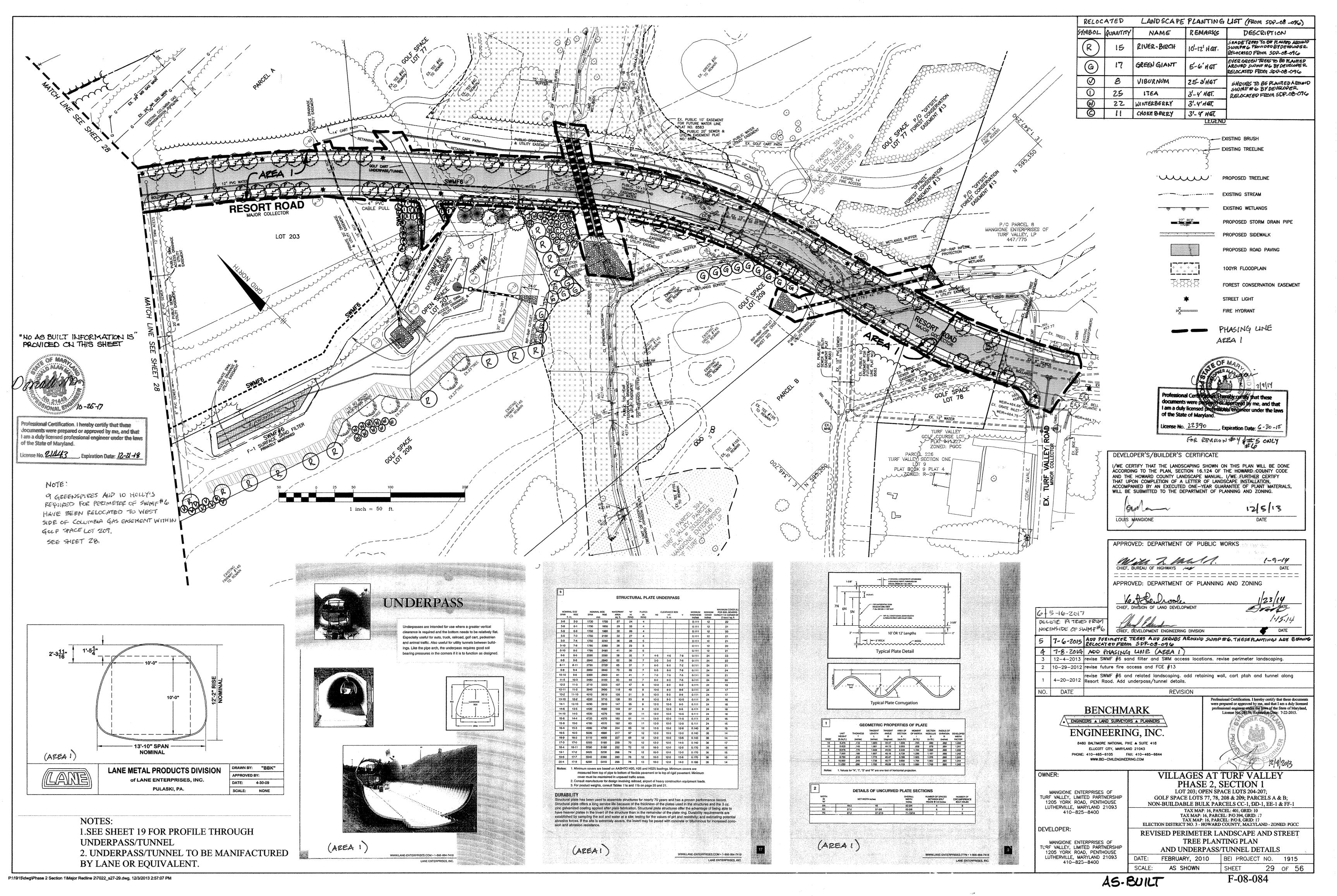
SAMPLER

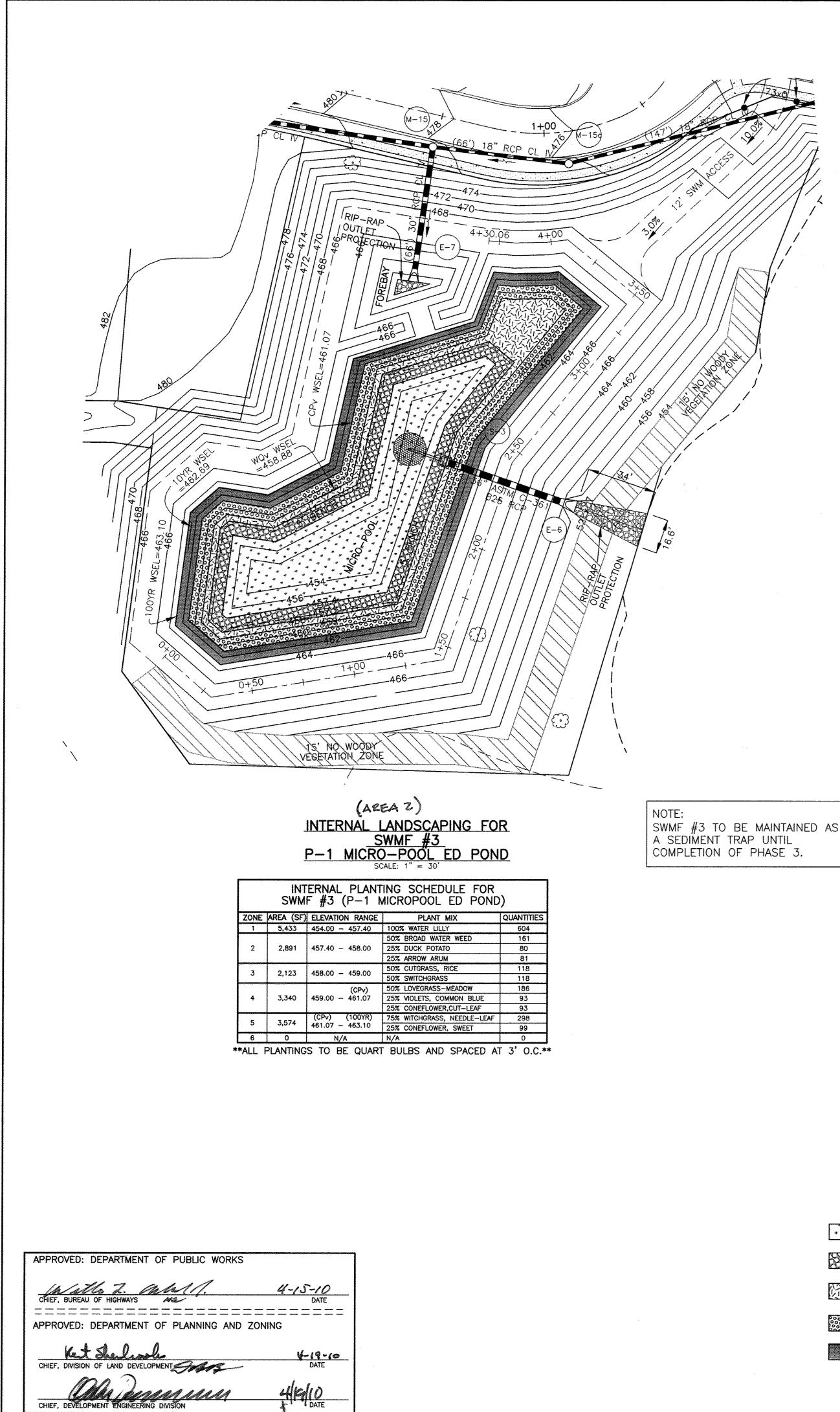


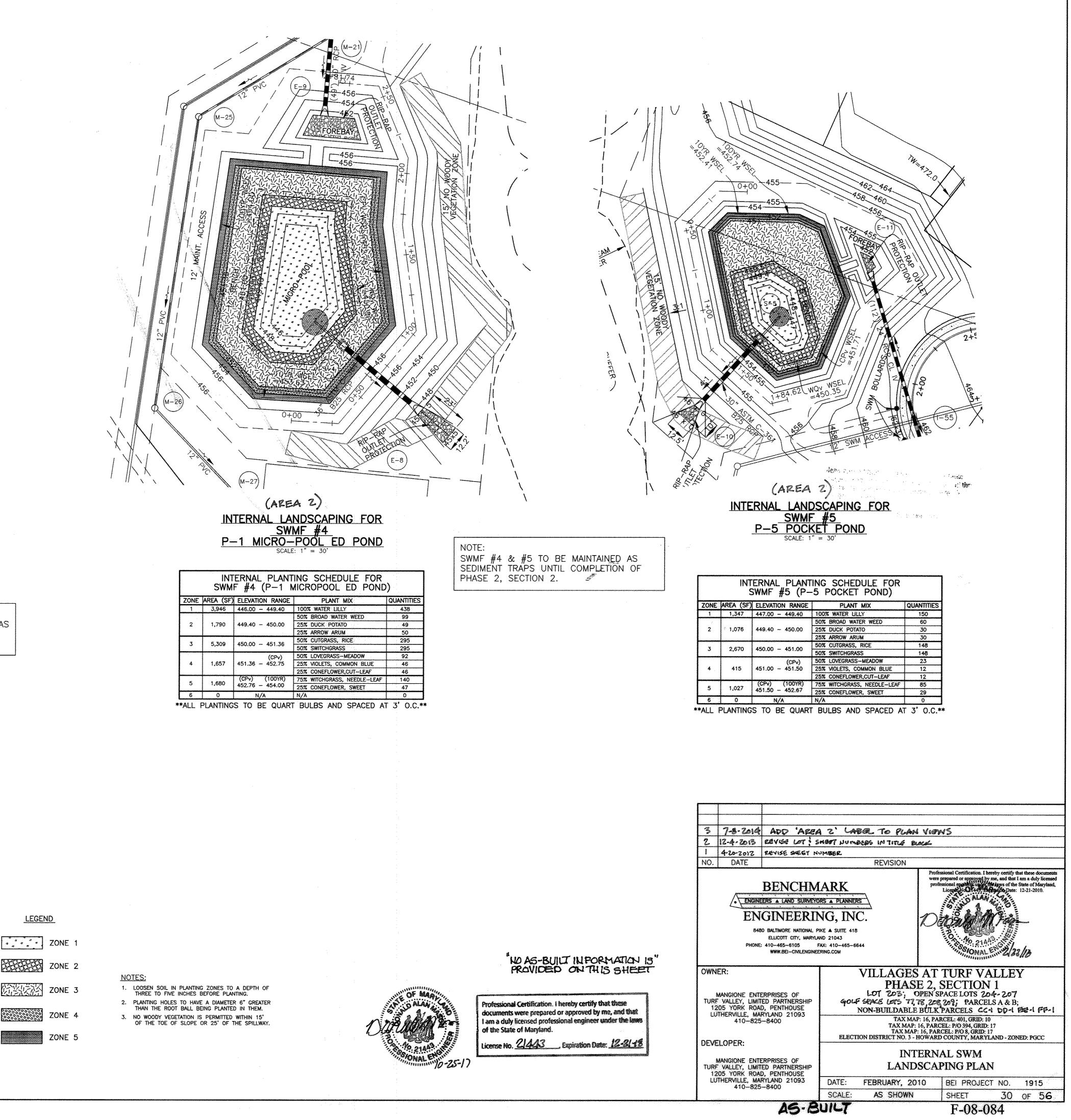
(AREA Z) SCHEDULE D SWM AREA LANDSCAPING							
, <u>, , , , , , , , , , , , , , , , , , </u>	SWM3	SWM4	SWM5	SWM6			
LINEAR FEET OF PERIMETER	452'	241'	558'	645'	.2		
LINEAR FEET OF EXISTING WOODS LINE	0		0	0	TOTALS		
LINEAR FEET OF REQUIRED PLANTING	452'	241'	558'	645'	IVIALS		
BUFFER TYPE	"B"	*B*	"B"	"B"			
	1:50 shade 1:40 evergreen	1:50 shade 1:40 evergreen	1:50 shode 1:40 evergreen	1:50 shade 1:40 evergreen			
CREDIT FOR EXISTING VEGETATION (NO, YES AND %)	NO	NO	NO	NO			
CREDIT FOR OTHER LANDSCAPING (NO, YES AND %)	NO	NO	NO	NO			
NUMBER OF TREES REQUIRED							
SHADE TREES	9	5	11	13	38		
EVERGREEN TREES	11	6	14	16	47		
NUMBER OF TREES PROVIDED							
SHADE TREES	9	5	11	13	38		
EVERGREEN TREES	11	6	14	16	47		

	(AREA	2)LANDSCAPE	PLANTING	LIST
SYMBOL	QUANTITY	NAME	REMARKS	DESCRIPTION
	47	ILEX OPACA (American Holly)	5' – 6' ht.	EVERGREEN TREES ALONG SWMF PERIMETER TO BE PROVIDED BY THE DEVELOPER
*****	38	Tilla Cordata 'Greenspire' (Greenspire Littleleaf Linden)	2—1/2" — 3"cal.	SHADE TREES ALONG SWMF PERIMETER TO BE PROVIDED BY THE DEVELOPER





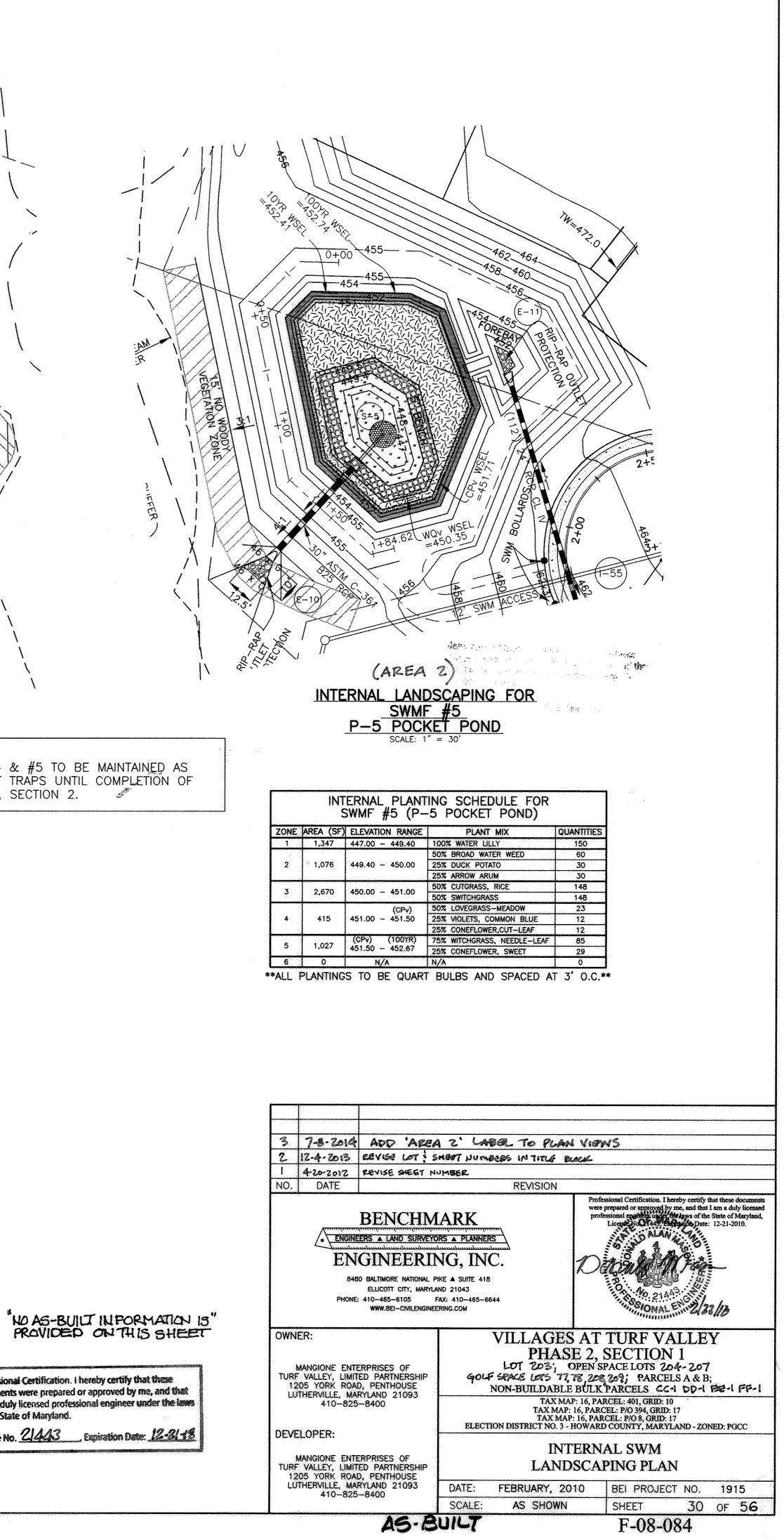




			ING SCHEDULE FOR))
ZONE	AREA (SF)	ELEVATION RANGE	PLANT MIX	QUANTITIES
1	3,946	446,00 - 449.40	100% WATER LILLY	438
			50% BROAD WATER WEED	99
2	1,790	449.40 - 450.00	25% DUCK POTATO	49
			25% ARROW ARUM	50
3	5,309	0 450.00 451.70	50% CUTGRASS, RICE	295
3	5,509	450.00 - 451.36	50% SWITCHGRASS	295
		(CPv)	50% LOVEGRASS-MEADOW	92
4	1,657	451.36 - 452.75	25% VIOLETS, COMMON BLUE	46
			25% CONEFLOWER, CUT-LEAF	46
5	1 690	(CPv) (100YR)	75% WITCHGRASS, NEEDLE-LEAF	140
Э	1,680	452.76 - 454.00	25% CONEFLOWER, SWEET	47
6	0	N/A	N/A	0

ZONE 2





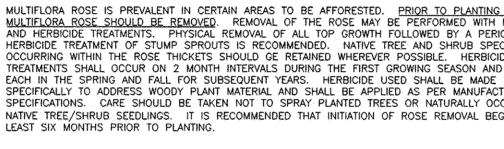
		SOILS
MAP SYMBOL	SOIL GROUP	
Ba*	D	BAILE SILT LOAM
CgB2	B	CHESTER GRAVELY SILT LO
CgC2	B	CHESTER GRAVELY SILT LO
ChA	В	CHESTER SILT LOAM, O TO
ChB2	8	CHESTER SILT LOAM, 3 TO
ChC2	В	CHESTER SILT LOAM, 8 TO
ChC3	B	CHESTER SILT LOAM, 8 TO
CuB	B	COMUS SILT LOAM, LOCAL
EnC2	8	ELSINBORO LOAM, 8 TO 1
GIA	В	GLENELG LOAM, O TO 3 9
GIC2	В	GLENELG LOAM, 8 TO 15
GIC3	В	GLENELG LOAM, 8 TO 15
GnB2*	С	GLENVILLE SILT LOAM, 3
Ha \star	D	HATBORO SILT LOAM
Kn*	D	KINKORA SILT LOAM
MgB2	В	MANOR GRAVELLY LOAM, 3
MgC2	8	MANOR GRAVELLY LOAM, 8
MgC3	В	MANOR GRAVELLY LOAM, 8
MIB2	B	MANOR LOAM, 3 TO 8 %
MIC3	B	MANOR LOAM, 8 TO 15 %
MID2	В	MANOR LOAM, 15 TO 25
MID3	В	MANOR LOAM, 15 TO 25
MIE	B	MANOR LOAM, 25 TO 45
	TES HYDRIC FROM: SOIL	SOILS SURVEY HOWARD COUNTY

PLANTING NOTES:

1. MULTIFLORA ROSE CONTROL MAY BE REQUIRED AS PART OF THIS PLANTING PLAN. 2. BAREROOT PLANT MATERIAL MAY BE USED TO OFFSET THE COST OF MULTIFLORA ROSE REMOVAL AND MAINTENANCE. IF BAREROOT MATERIAL IS USED IT MUST BE PLANTED IN MARCH - APRIL AND AN ANTI-DESICCANT GEL SHOULD BE UTILIZED TO PROTECT ROOT SYSTEMS. CONTAINER GROWN STOCK IS RECOMMENDED.

SHOULD ALSO BE PLANTED IN A GRID PATTERN TO FACILITATE MAINTENANCE AND REMOVAL OF INVASIVE AND EXOTIC SPECIES.

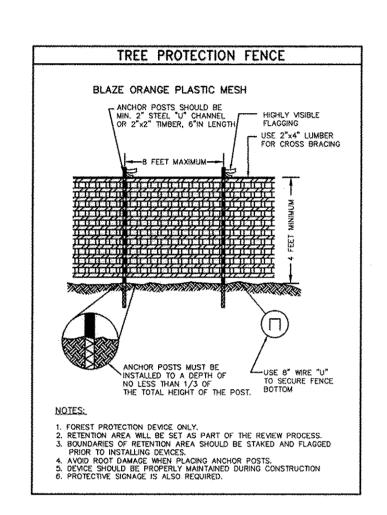
MULTIFLORA ROSE CONTROL NOTE

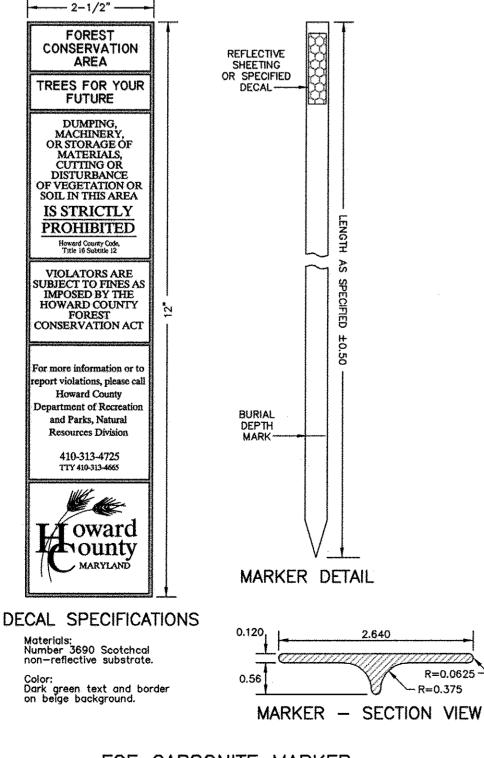


PLANTING/SOIL SPECIFICATIONS

- SEQUENCE OF CONSTRUCTION
- 1. PLANTS SHALL BE INSTALLED AS PER PLANT SCHEDULE AND PLANTING/SOIL SPECIFICATIONS FOR THE PROJECT.
- PROJECT. 2. UPON COMPLETION OF THE PLANTING, SIGNAGE SHALL BE INSTALLED AS PER THE FOREST RETENTION AREA PROTECTION DEVICES SHOWN ON THE FOREST CONSERVATION PLAN, 3. PLANTINGS SHALL BE MAINTAINED AND GUARANTEED IN ACCORDANCE WITH THE MAINTENANCE AND GUARANTEE REQUIREMENTS FOR PROJECT.
- MAINTENANCE OF PLANTINGS
- 4. 5. DEAD BRANCHES WILL BE PRUNED FROM PLANTINGS.
- GUARANTEE REQUIREMENTS
- AFTER ONE GROWING SEASON, PLANT MATERIAL SHALL BE MAINTAINED AT 90% SURVIVAL THRESHOLD. A 75% SURVIVAL RATE OF FORESTATION PLANTINGS WILL BE REQUIRED AT THE END OF THE 24 MONTH MAINTENANCE PERIOD. ALL PLANT MATERIAL BELOW THE 75% THRESHOLD WILL BE REPLACED AT THE BEGINNING OF THE NEXT GROWING SEASON. THE CONTRACTOR WILL NOT BE LIABLE FOR PLANT LOSS DUE TO THEFT OR VANDALISM. HOWEVER, DEVELOPER SHALL BE RESPONSIBLE UNTIL RELEASE FROM THE FOREST SURETY OBLIGATION. SURETY FOR REFORESTATION
- THE DEVELOPER SHALL POST A SURETY (BOND, LETTER OF CREDIT) TO ENSURE THAT REFORESTATION PLANTINGS ARE COMPLETED. UPON ACCEPTANCE OF THE PLANTINGS BY THE COUNTY, THE BOND SHALL BE RELEASED.
 - 2" CALIPER TREES = 7 PLANTING UNITS 1" CALIPER TREES = 3.5 PLANTING UNITS WHIPS W/SHELTERS = 2 PLANTING UNITS SEEDLINGS = 1 PLANTING UNIT

PLANTING SHALL BE INSTALLED IN A CURVILINEAR FASHION ALONG A CONTOUR TO AVOID A GRID APPEARANCE BUT SHOULD BE SPACED TO FACILITATE MAINTENANCE. LARGER STOCK TREES (1" AND 2" CALIPER) SHOULD BE STAGGERED ALONG THE OUTER PERIMETER OF THE PLANTING AREAS TO SERVE AS A DEMARCATION OF THE FCE.





FCE CARSONITE MARKER NOT TO SCALE

LEGEND
SOIL TYPE
AM, 3 TO 8 % SLOPES, MODERATELY ERODED
AM, 8 TO 15 % SLOPES, MODERATELY ERODED
3 % SLOPES
8 % SLOPES, MODERATELY ERODED
15 % SLOPES, MODERATELY ERODED
15 % SLOPES, SEVERELY ERODED
ALLUVIUM, 3 TO 8 % SLOPES
5 % SLOPES, MODERATELY ERODED
SLOPES
% SLOPES, MODERATELY ERODED
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0 8 % SLOPES, MODERATELY ERODED
TO 8 % SLOPES, MODERATELY ERODED
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SLOPES, MODERATELY ERODED
SLOPES, SEVERELY ERODED
ERCENT SLOPES

TAKEN FROM: SOIL SURVEY HOWARD COUNTY, MARYLAND, ISSUED JULY 1968, MAP NO. 9

3. PLANTS SHOULD BE FLAGGED TO AID ON LOCATION DURING MAINTENANCE. PLANTINGS

MULTIFLORA ROSE IS PREVALENT IN CERTAIN AREAS TO BE AFFORESTED. <u>PRIOR TO PLANTING ALL</u> MULTIFLORA ROSE SHOULD BE <u>REMOVED</u>. REMOVAL OF THE ROSE MAY BE PERFORMED WITH MOWING AND HERBICIDE TREATMENTS. PHYSICAL REMOVAL OF ALL TOP GROWTH FOLLOWED BY A PERIODIC HERBICIDE TREATMENT OF STUMP SPROUTS IS RECOMMENDED. NATIVE TREE AND SHRUB SPECIES OCCURRING WITHIN THE ROSE THICKETS SHOULD GE RETAINED WHEREVER POSSIBLE. HERBICIDE TREATMENTS SHALL OCCUR ON 2 MONTH INTERVALS DURING THE FIRST GROWING SEASON AND ONCE SPECIFICALLY TO ADDRESS WOODY PLANT MATERIAL AND SHALL BE APPLIED AS PER MANUFACTURERS SPECIFICATIONS. CARE SHOULD BE TAKEN NOT TO SPRAY PLANTED TREES OR NATURALLY OCCURRING NATIVE TREE/SHRUB SEEDLINGS. IT IS RECOMMENDED THAT INITIATION OF ROSE REMOVAL BEGIN AT

 PLANTING OF NURSERY STOCK SHALL TAKE PLACE BETWEEN MARCH 15TH AND APRIL 30TH. CONTAINER STOCK MAY BE PLANTED BETWEEN SEPTEMBER 1ST AND OCTOBER 30TH.
 A TWELVE (12) INCH LAYER OF TOPSOIL SHALL BE SPREAD OVER ALL REFORESTATION AREAS IMPACTED BY SITE GRADING TO ASSURE A SUITABLE PLANTING AREA. DISTURBED AREAS SHALL BE SEEDED AND STABILIZED AS PER GENERAL CONSTRUCTION PLAN FOR PROJECT. PLANTING AREAS NOT IMPACTED BY SITE GRADING SHALL HAVE TO ADDITIONAL TOPSOIL INSTALLED.
 ALL BARENOT PLANTING STOCK SHALL HAVE THEIR ROOT SYSTEMS DIPPED INTO AN ANTI-DESICCANT GEL PRIOR TO PLANTING.
 PLANTS SHALL BISTALLED SO THAT THE TOP OF ROOT MASS IS LEVEL WITH THE TOP OF EXISTING GRADE. BACKFILL IN THE PLANTING PITS SHALL CONSIST OF 3 PARTS EXISTING SOIL TO 1 PART PINE FINES OR EQUIVALENT.
 FERTILIZER SHALL CONSIST OF AGRIFORM 22-8-2, OR EQUIVALENT, APPLIED AS PER MANUFACTURER'S SPECIFICATIONS.
 A TWO (2) INCH LAYER OF HARDWOOD MULCH SHALL BE PLACED OVER THE ROOT AREA OF ALL PLANTINGS.
 INCH LAYER OF HARDWOOD MULCH SHALL BE PLACED OVER THE ROOT AREA OF ALL PLANTINGS. PLANTINGS. 7. PLANT MATERIAL SHALL BE TRANSPORTED TO THE SITE IN A TARPED OR COVERED TRUCK. PLANTS SHALL BE KEPT MOIST PRIOR TO PLANTING. 8. ALL NON-ORGANIC DEBRIS ASSOCIATED WITH THE PLANTING OPERATION SHALL BE REMOVED FROM THE SITE BY THE CONTRACTOR.

MAINTENANCE OF ALL PLANTINGS SHALL LAST FOR A PERIOD OF 24 MONTHS. ALL PLANT MATERIAL SHALL BE WATERED TWICE A MONTH DURING THE 1ST GROWING SEASON. WATERING MAY BE MORE OR LESS FREQUENT DEPENDING ON WEATHER CONDITIONS. DURING SECOND GROWING SEASON, ONCE A MONTH DURING MAY-SEPTEMBER, IF NEEDED. INVASIVE EXOTICS AND NOXIOUS WEEDS WILL BE REMOVED FROM REFORESTATION AREAS. OLD FIELD SUCCESSIONAL SPECIES WILL BE RETAINED. PLANTS WILL BE EXAMINED A MINIMUM OF TWO TIMES DURING THE GROWING SEASON FOR SERIOUS PLANT PESTS AND DISEASES. SERIOUS PROBLEMS WILL BE TREATED WITH THE APPROPRIATE AGENT. DEAD REMOVEDS WILL BE PRINTED FROM PLANTINGS.

FOREST CONSERVATION NOTES: (AREA 2)

1. VILLAGES AT TURF VALLEY, PHASE 2:

PARCEL 401: THE TOTAL FOREST CONSERVATION OBLIGATION AMOUNT OF 4.48 ACRES SHALL BE MET BY THE RETENTION OF 2.03 AC. OF NET TRACT AREA FOREST WITHIN A FOREST CONSERVATION EASEMENT (#6-8) ON PARCEL 401 AND BY THE OFFSITE RETENTION OF 4.90 AC. OF FOREST WITHIN FOREST CONSERVATION EASEMENTS (#9-14) OF WHICH ONLY 2.45 AC. IS CREDITED (SINCE THIS RETENTION IS OFFSITE IT IS CREDITED AT A 2:1 RATIO). FINANCIAL SURETY FOR THE REQUIRED FOREST CONSERVATION HAS BEEN POSTED AS PART OF THE DPW DEVELOPERS AGREEMENT IN THE AMOUNT OF \$42,689.00 (\$0.20 PER SQUARE FOOT ROUNDED UP TO NEAREST DOLLAR).

PARCEL 8 & 394: THIS PORTION OF THE PROJECT IS EXEMPT FROM HOWARD COUNTY FOREST CONSERVATION REQUIREMENTS UNDER SECTION 16.1202(b) OF THE COUNTY CODE SINCE IT IS A PLANNED UNIT DEVELOPMENT UNDER S-86-13. FAIRWAYS AT TURF VALLEY, PHASE 2 AND 3:

THE TOTAL FOREST CONSERVATION OBLIGATION OF 1.12 ACRES (PHASE 2, F-10-084) AND 0.22 ACRES (PHASE 3, F-10-086) (HASE 3) HAS BEEN MET BY THE OFFSITE RETENTION OF 1.08 ACRES OF FOREST WITHIN FOREST CONSERVATION EASEMENT #14 OF WHICH ONLY OF ACRES OF FOREST WITHIN FOREST CONSERVATION EASEMENT #14 OF WHICH ONLY OF ACRES IS CREDITED (SINCE THIS RETENTION IS OFFSITE IT IS CREDITED AT A 2:1 RATIO) AND BY THE PLANTING OF 0.80 ACRES OF FOREST WITHIN FOREST CONSERVATION EASEMENT #14. FINANCIAL SURETY FOR THE REQUIRED FOREST CONSERVATION HAS BEEN POSTED AS PART OF THE DPW DEVELOPERS AGREEMENT IN THE AMOUNT OF 1 22,000,00 FOR PHASE 2, F-10-084 AND 3,833.00 FOR PHASE 3, F-10-086. THE FOREST CONSERVATION EASEMENTS FOR FAIRWAYS AT TURF VALLEY, PHASES 2 AND 3 AND SHOWN ON THESE PLANS SATISFY FOREST CONSERVATION OBLIGATIONS BASED ON PRELIMINARY FOREST CONSERVATION PLANS FOR THOSE SUBDIVISIONS. THE EASEMENTS ARE SUBJECT TO CHANGE BASED ON DEVELOPMENT OF THE FINAL FOREST CONSERVATION PLANS FOR FAIRWAYS AT TURF VALLEY, PHASES 2 AND 3 DURING THEIR FINAL SUBDIVISION PLAN STAGE.

2. ANY FOREST CONSERVATION EASEMENT (FCE) AREA SHOWN HEREON IS SUBJECT TO PROTECTIVE COVENANTS WHICH MAY BE FOUND IN THE LAND RECORDS OF HOWARD COUNTY WHICH RESTRICT THE DISTURBANCE AND USE OF THESE AREAS.

3 THE FOREST CONSERVATION EASEMENTS HAVE BEEN ESTABLISHED TO FULFILL THE REQUIREMENTS OF SECTION 16.1200 OF THE HOWARD COUNTY CODE, FOREST CONSERVATION ACT. NO CLEARING, GRADING, OR CONSTRUCTION IS PERMITTED WITHIN THE FOREST CONSERVATION EASEMENTS; HOWEVER, FOREST MANAGEMENT PRACTICES AS DEFINED IN THE DEED OF FOREST CONSERVATION EASEMENT ARE ALLOWED

4. LIMITS OF DISTURBANCE SHALL BE RESTRICTED TO AREAS OUTSIDE THE LIMIT OF TEMPORARY FENCING OR THE FCE BOUNDARY, WHICHEVER IS GREATER. 5. THERE SHALL BE NO CLEARING, GRADING, CONSTRUCTION OR DISTURBANCE

OF VEGETATION IN THE FOREST CONSERVATION EASEMENT, EXCEPT AS PERMITTED BY HOWARD COUNTY DPZ. 6. NO STOCKPILES, PARKING AREAS, EQUIPMENT CLEANING AREAS, ETC. SHALL OCCUR WITHIN AREAS DESIGNATED AS FOREST CONSERVATION

EASEMENTS.

7. TEMPORARY FENCING SHALL BE USED TO PROTECT FOREST RESOURCES DURING CONSTRUCTION. THE FENCING SHALL BE PLACED ALONG ALL FCE RETENTION BOUNDARIES WHICH OCCUR WITHIN 15 FEET OF THE PROPOSED LIMITS OF DISTURBANCE.

8. PERMANENT SIGNAGE SHALL BE PLACED 50-100' APART ALONG THE BOUNDARIES OF ALL AREAS INCLUDED IN FOREST CONSERVATION EASEMENTS. 9. PORTIONS OF THE SITE OCCURRING WITHIN THE 100-YEAR FLOODPLAIN ARE NOT INCLUDED AS PART OF THE NET TRACT AREA OF THE SITE. AREAS OF FLOODPLAIN FOREST OCCURRING WITHIN THE LIMITS OF A FOREST CONSERVATION EASEMENT WILL BE PROTECTED BY THE EASEMENT RESTRICTIONS BUT HAVE NOT BEEN CREDITED TOWARD THE PROJECTS FCA OBLIGATIONS.

10. THE FOREST CONSERVATION WATERSHED FOR THIS PROJECT IS THE LITTLE PATUXENT RIVER (UPPER) #2131105A.

11. THERE ARE NO RARE, THREATENED OR ENDANGERED SPECIES LOCATED ON THIS SITE. THERE ARE NO SPECIMEN OR CHAMPION TREES LOCATED ON THIS SITE. TO THE BEST OF OUR KNOWLEDGE, THERE ARE NO HISTORIC STRUCTURES LOCATED ON THIS SITE.

12. THE PROTECTIVE SIGNAGE SHALL STAY ON-SITE IN PERPETUITY.

		Forest C	Conservation	Summary	Table	AREA	2)
		Parcel 4	01, Villages at Tu	rf Valley, Ph	ase 2		
Easement #		Retention	Retention credited toward obligation	Non-credited retention	Planting	Priority Areas	Total Easement Area
6	<u> </u>	0.23	0.23	0	0	0.10	0.23
7	Onsite credit at	1.54	1.54	0.08	0	0	1.62
8	1:1	0.26	0.26	0	0	0.24	0.26
9		2.52	1.26	0	0	0.42	2.52
10	Offsite credit at	0.56	0.28	0.26	0	0.76	0.82
11	2:1	0.80	0.40	0	0	0.71	0.80
12	2.1	0.23	0.115	0	0	0	0.23
13		0.55	0.275	0	0	0.12	0.55
14		0.2.4	0.12	10°1	0	0.	0.24
TOTALS		6.93	448	0.34	0	2.35	7.27
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		Forest C	conservation s	Summary	Table		
		Fairwa	ys at Turf Valley,	Phases 2 ai	nd 3		
			Retention credited	Non-credited		Priority	Total Easement
Easement #		Retention	toward obligation	retention	Planting	Areas	Area
	Offsite credit at		· · · · · · · · · · · · · · · · · · ·				
14	2:1	1.08	0. 54	0.21	0,90	0.84	2.09
	*All areas are in a	cres	י. 		an (in) a an air an air an a fair is fair i fair an a' an air		

209 AC. FOR FAIRWAYS, PHASES 2 AND 3 0.24 AC. FOR VILLAGES, PHASE 2

Certification #

0.24	AC	POL	VILLACIE	⇒,	PHASE	•

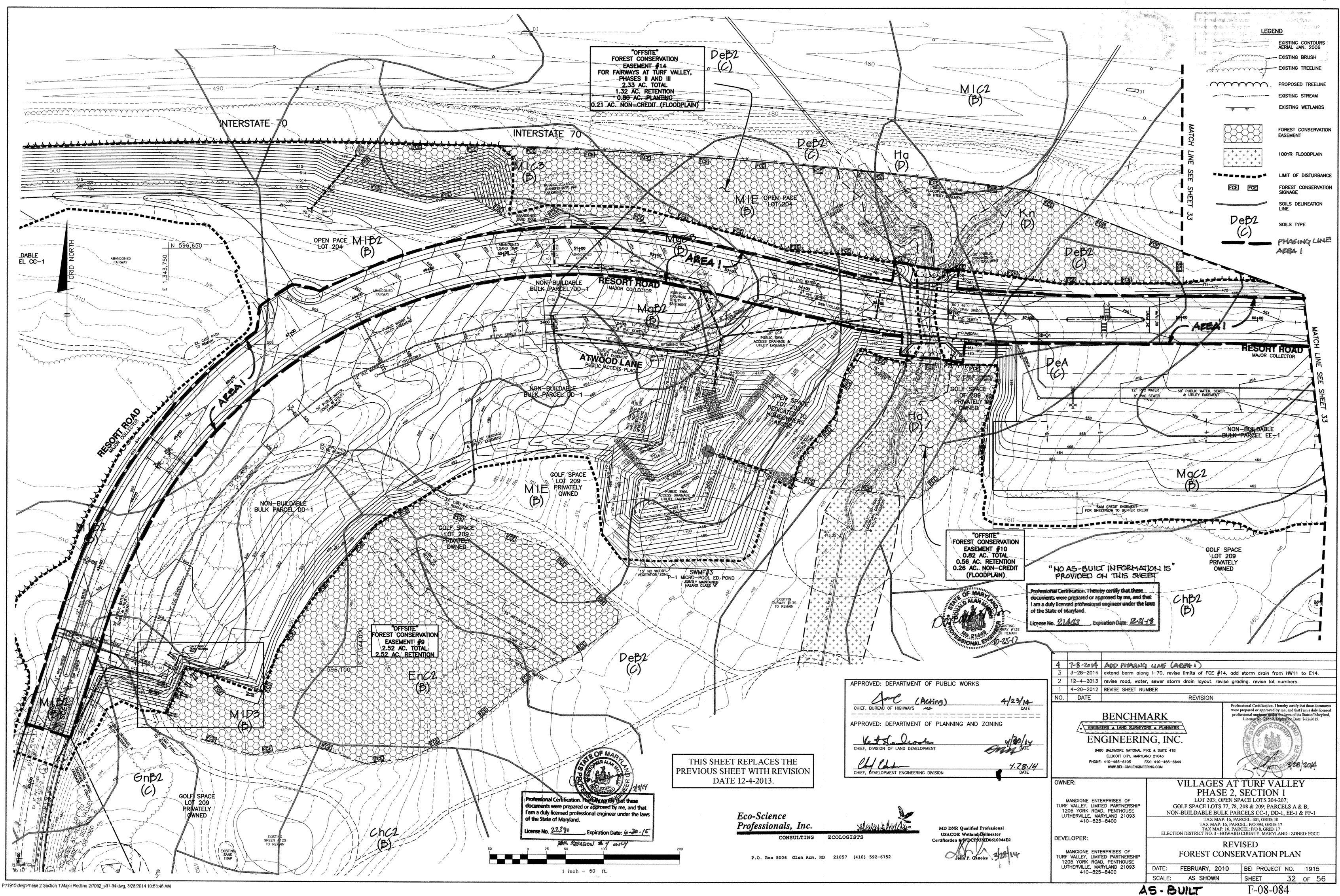
e e .	NET RETENTION AREA.	RETENTION CREDITED TOWARD OBLIGATION
TOTAL FOE #14	1.32 AC	0.66 AC
CEEDITED TO FAIRWAYS 2	0.64 AC	0.32AC
CREDITED TO FALEWAYS 3	0.44 AC	0.22 AC
CREDITED TO VILLAGES 2	0.24 AC	0.12 AC

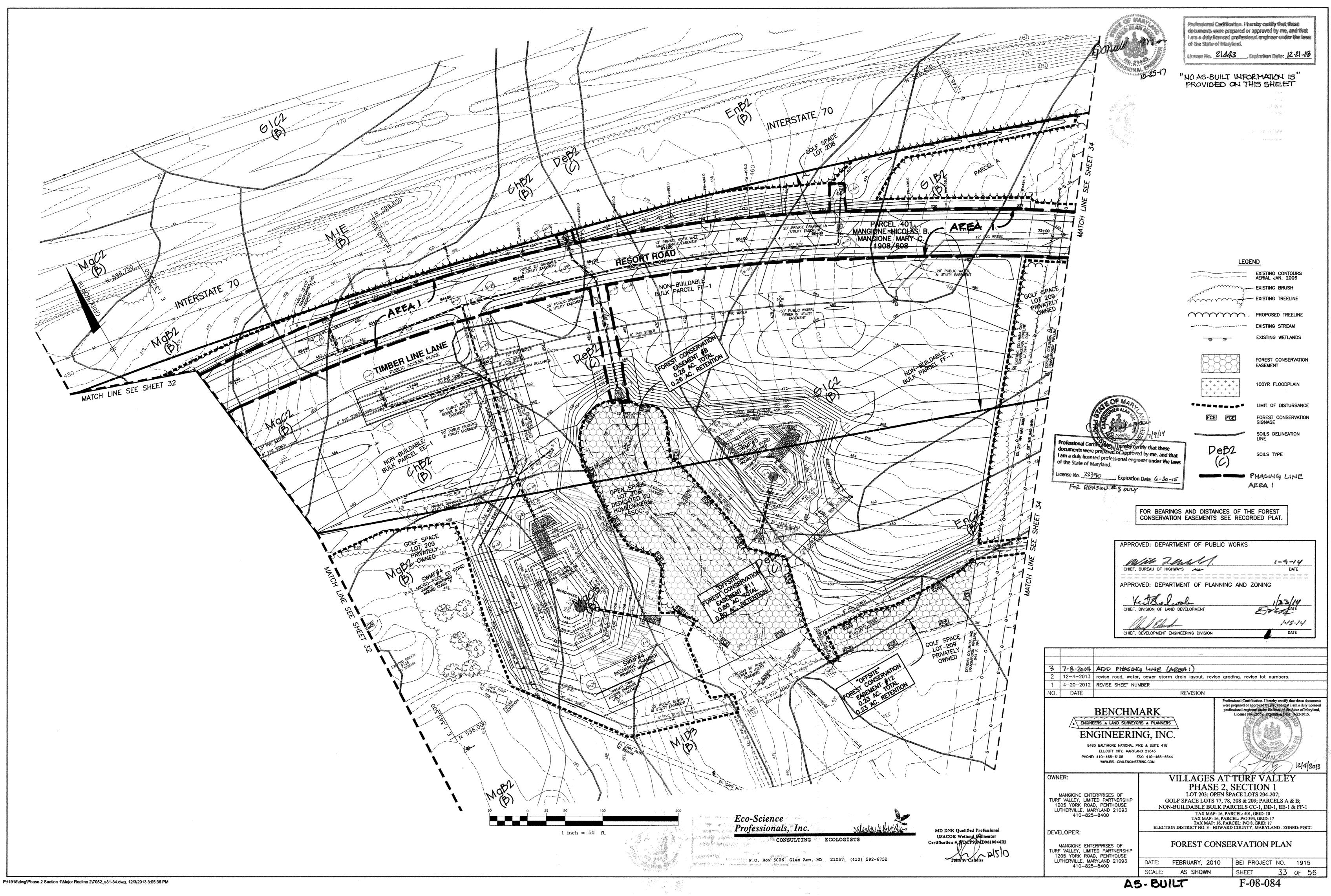
Eco-Science Professionals, Inc. WALL & MANA ECOLOGISTS CONSULTING

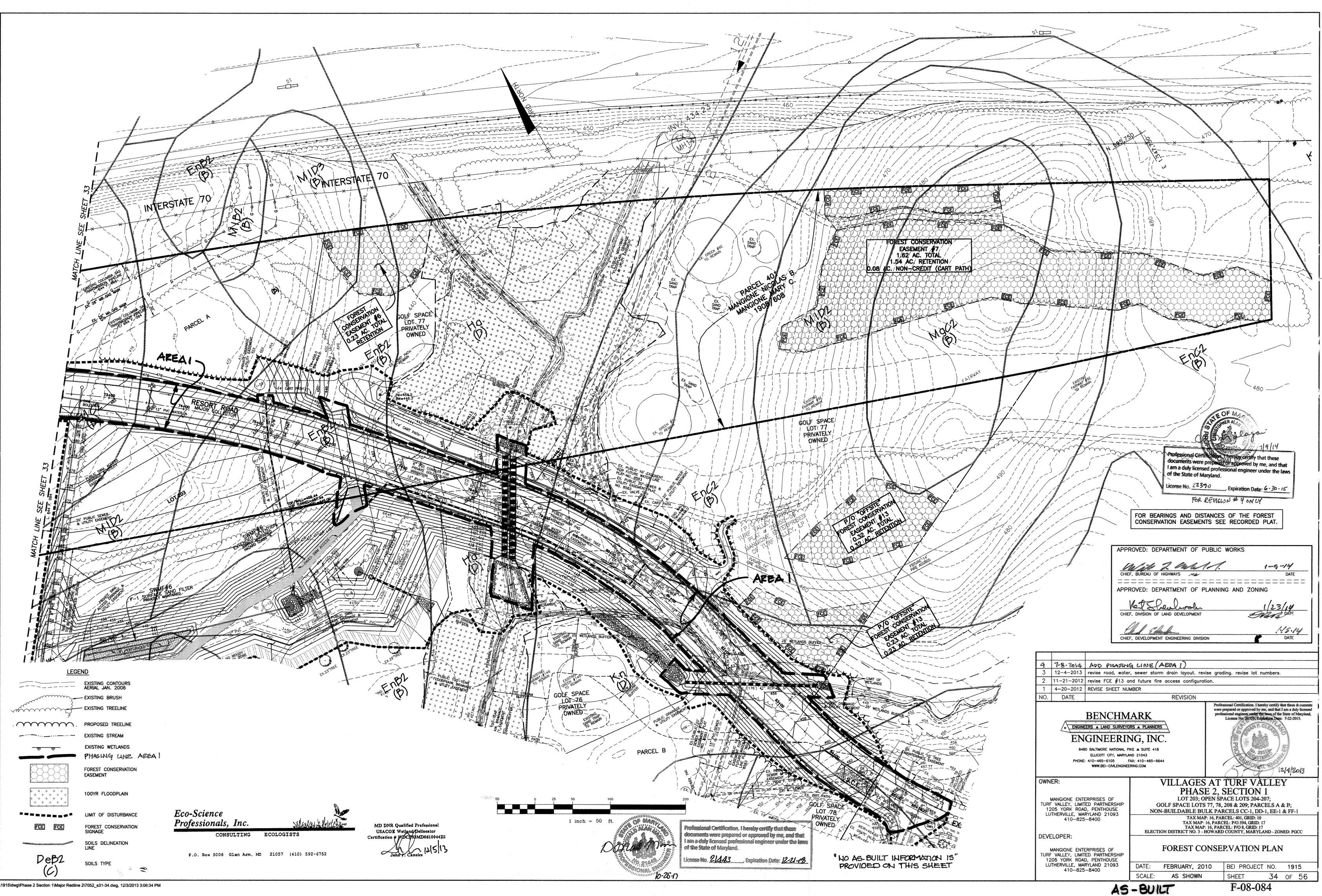
P.O. Box 5006 Glen Arm, MD 21057 (410) 592-6752

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	Installation Drawings Sheet Index
35	TITLE SHEET, LOCATION PLAN & GENERAL NOTES
36	BRIDGE PLAN & DETAIL
37	FOUNDATION PLAN
38	FOOTING SECTIONS & DETAILS
39	ELEVATIONS
40	SECTION & DETAILS
41	DETAILS
42	SPECIFICATIONS
43	SPECIFICATIONS

NOTES GENERAL NOTES:

- 1. This bridge has been designed for general site conditions. The project engineer shall be responsible for the structure's suitability to the existing site conditions and for the hydraulic evaluation -including scour and confirmation of soil conditions.
- 2. Prior to construction, contractor must verify all elevations shown through the engineer.
- 3. Only CONTECH Bridge Solutions Inc. the CON/SPAN® approved precaster in Maryland may provide the structure designed in accordance with these plans.
- 4. The use of another precast structure with the design assumptions used for the CON/SPAN® structure may lead to serious design errors. Use of any other precast structure with this design and drawings voids any certification of this design and warranty. CONTECH Bridge Solutions Inc. assumes no liability for design of any alternate or similar type structures.
- 5. Alternate structures may be considered, provided that signed and sealed design drawings (and calculations) are submitted to the engineer 2 weeks prior to the bid date for review and approval.
- 6. Proposed alternates to a CON/SPAN® Bridge System must submit at least two (2) independently verified full scale load tests that confirm the proposed design methodology of the three sided/arch structure(s). The proposed alternate, upon satisfactory confirmation of design methodology, may be considered an acceptable alternate.

DESIGN DATA

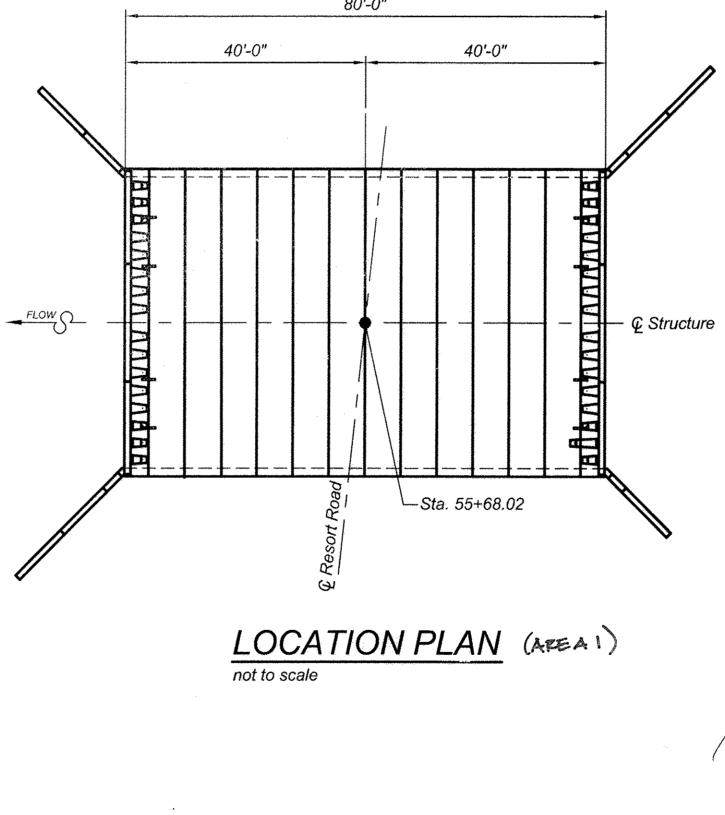
Design Loading: Bridge Units: HS25-44 + Maryland Legal Loads Headwalls: Earth Pressure Only Wingwalls: Earth Pressure Only Design Fill Height: 2'-0" min. to 9-0" max. from top of crown to top of pavement. Design Method: Load factor per AASHTO Specification Assumed Bearing: 4000 PSF *

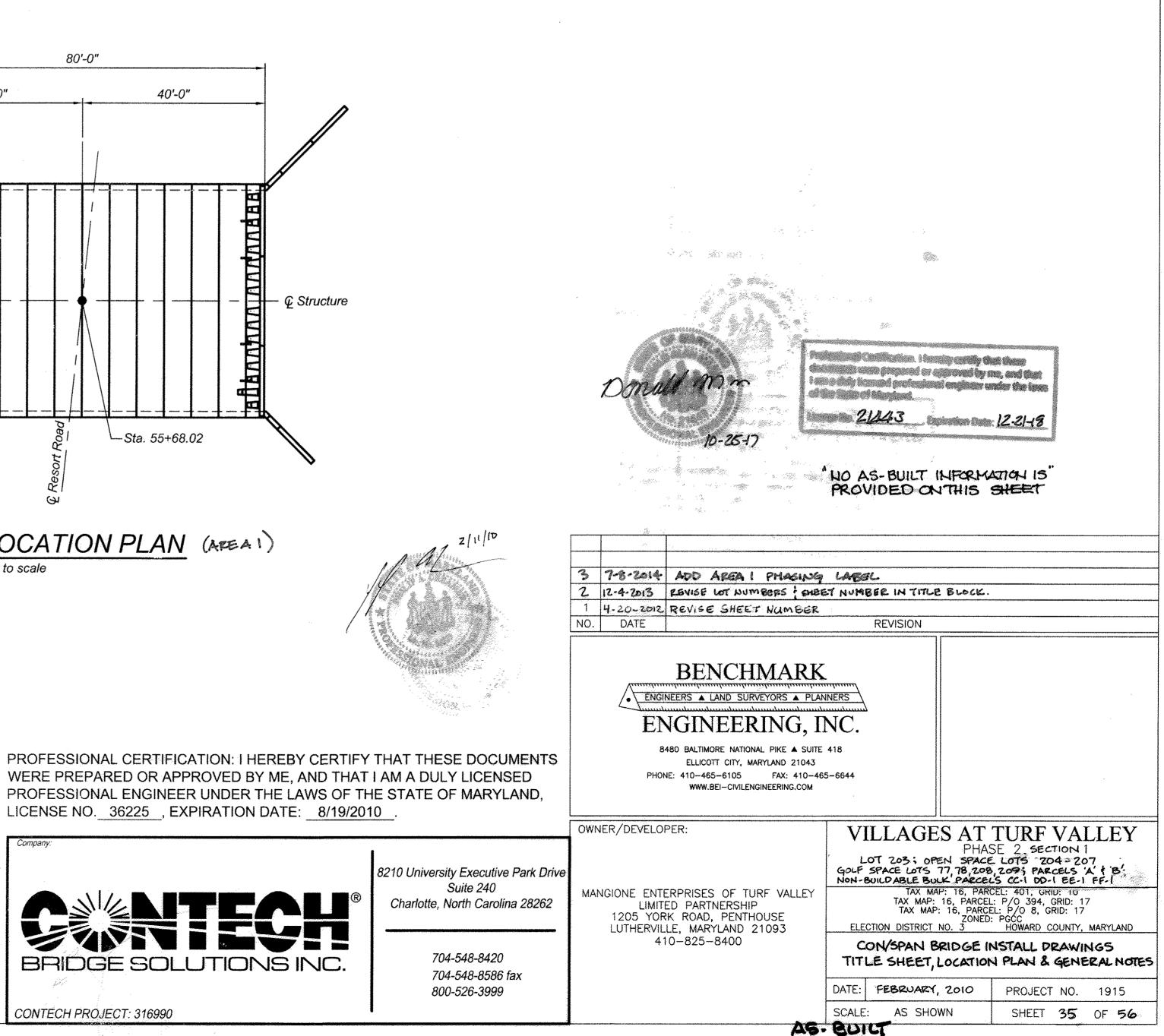
*At the time of design, a geotechnical report for the project site was not available. It is the project engineer's, owner's and/or the contractor's responsibility to verify that the actual site conditions at the time of construction are consistent with the assumed allowable soil bearing pressure with a geotechnical investigation from a qualified geotechnical engineer.

MATERIALS

Precast units shall be constructed and installed in accordance with CON/SPAN® Specifications. Concrete for Footings shall have a minimum compressive strength of 4000 psi. Reinforcing steel for footings shall conform to ASTM A615 or A996-Grade 60.

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80'-0" 12 - 48'-0" Span x 11'-0" Rise x 5'-11³/₄" Long 2 - 48'-0" Span x 11'-0" Rise x 4'-0" Long Precast Concrete Bridge Units Plus 13 Joints @ $\frac{1}{4}$ "± per Joint ______ _______ _____ • ······ . лунны малам наруун ланала, муруун здалуш ануну ----where were work where where --------------7 ____ ---- []] 3a 49 corners with 2'-0" of Filter Fabric BRIDGE PLAN (AREAD) ,

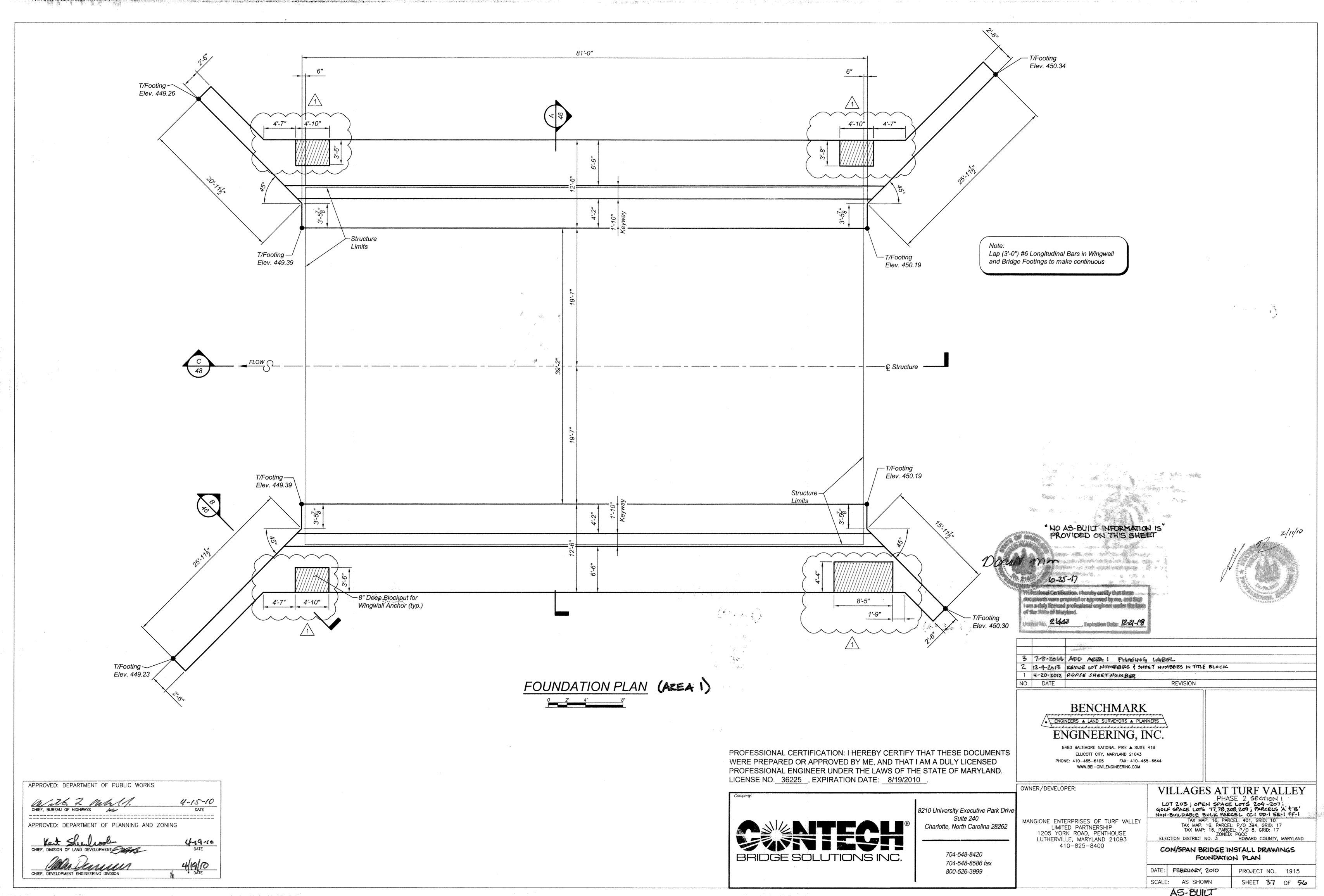
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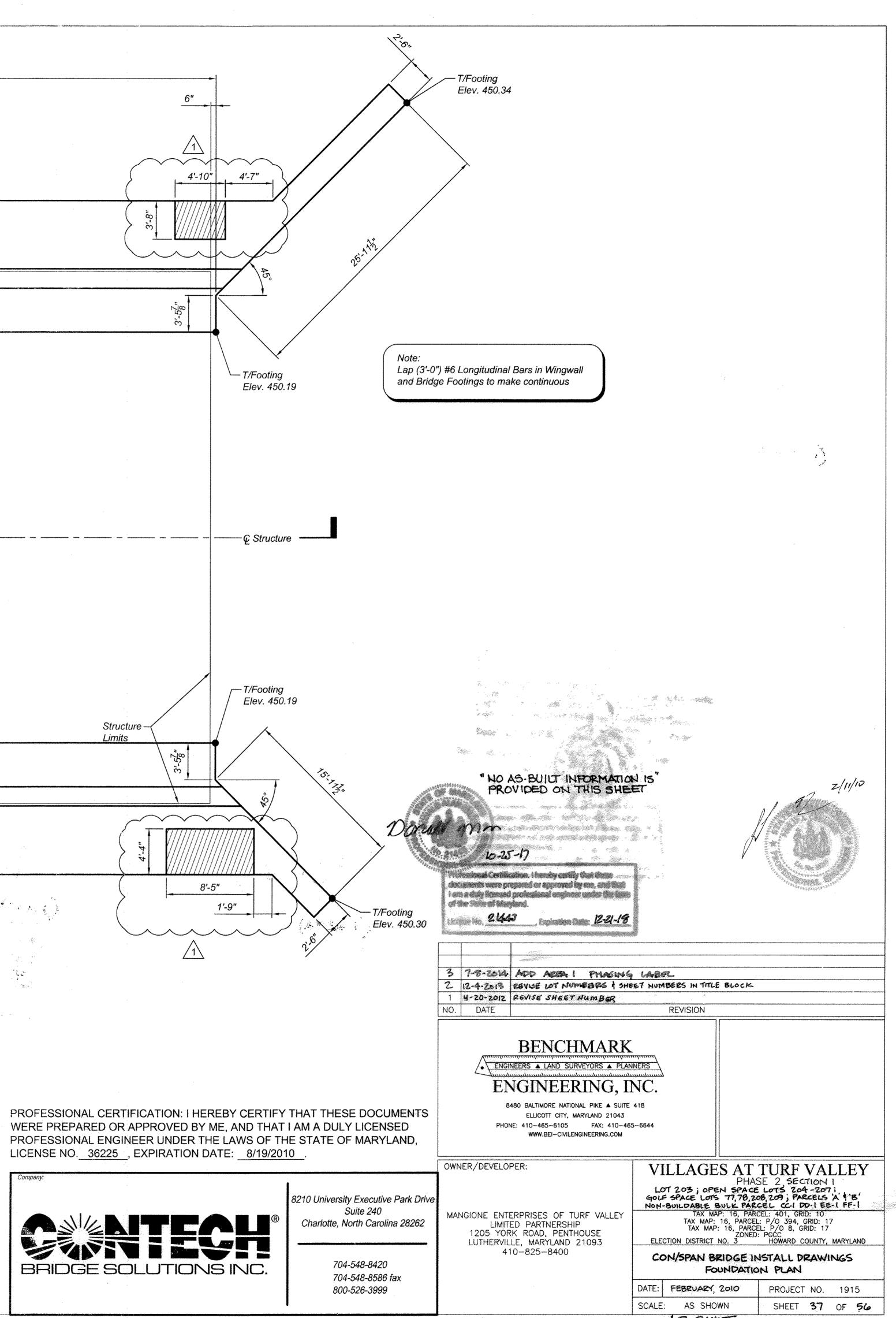
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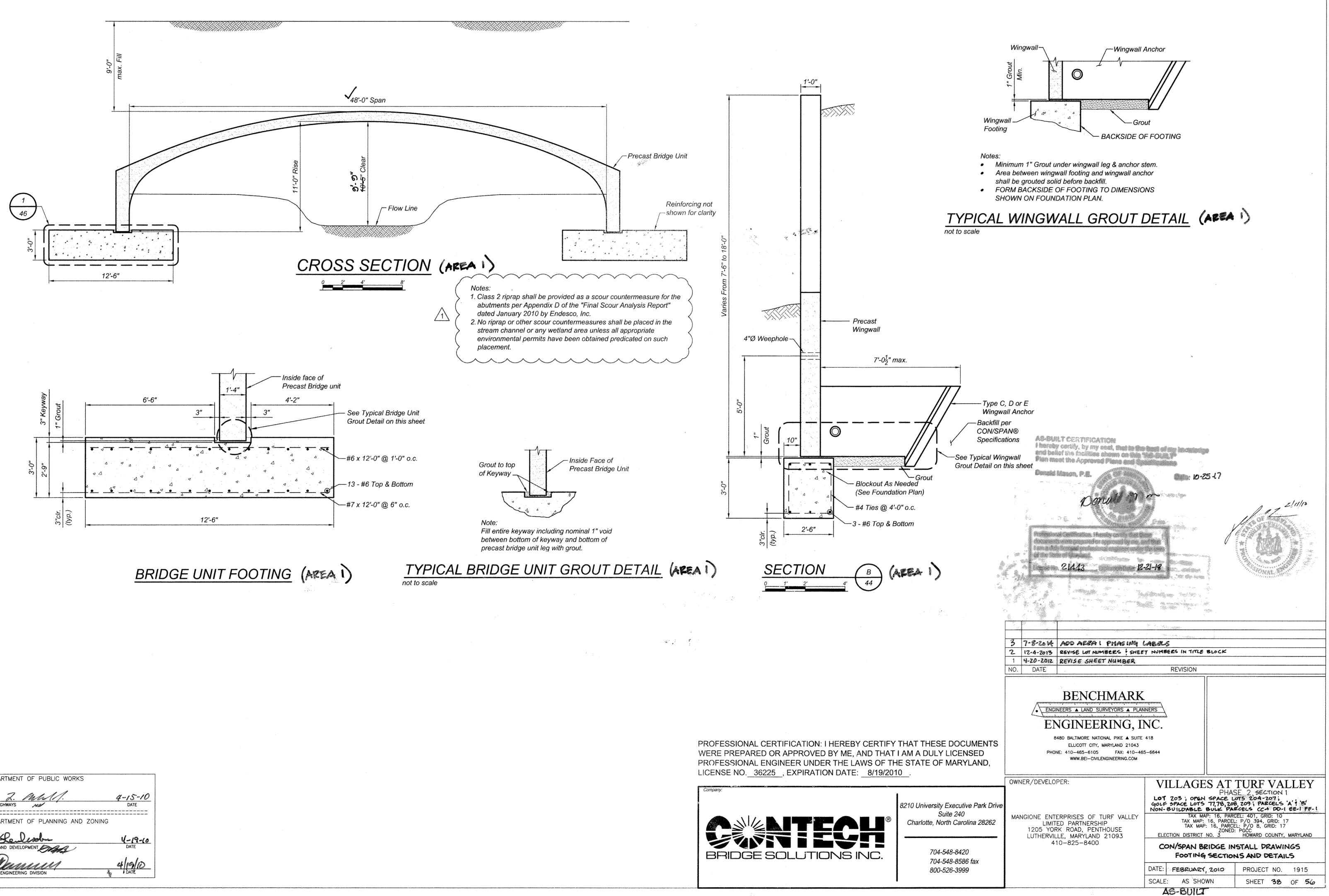
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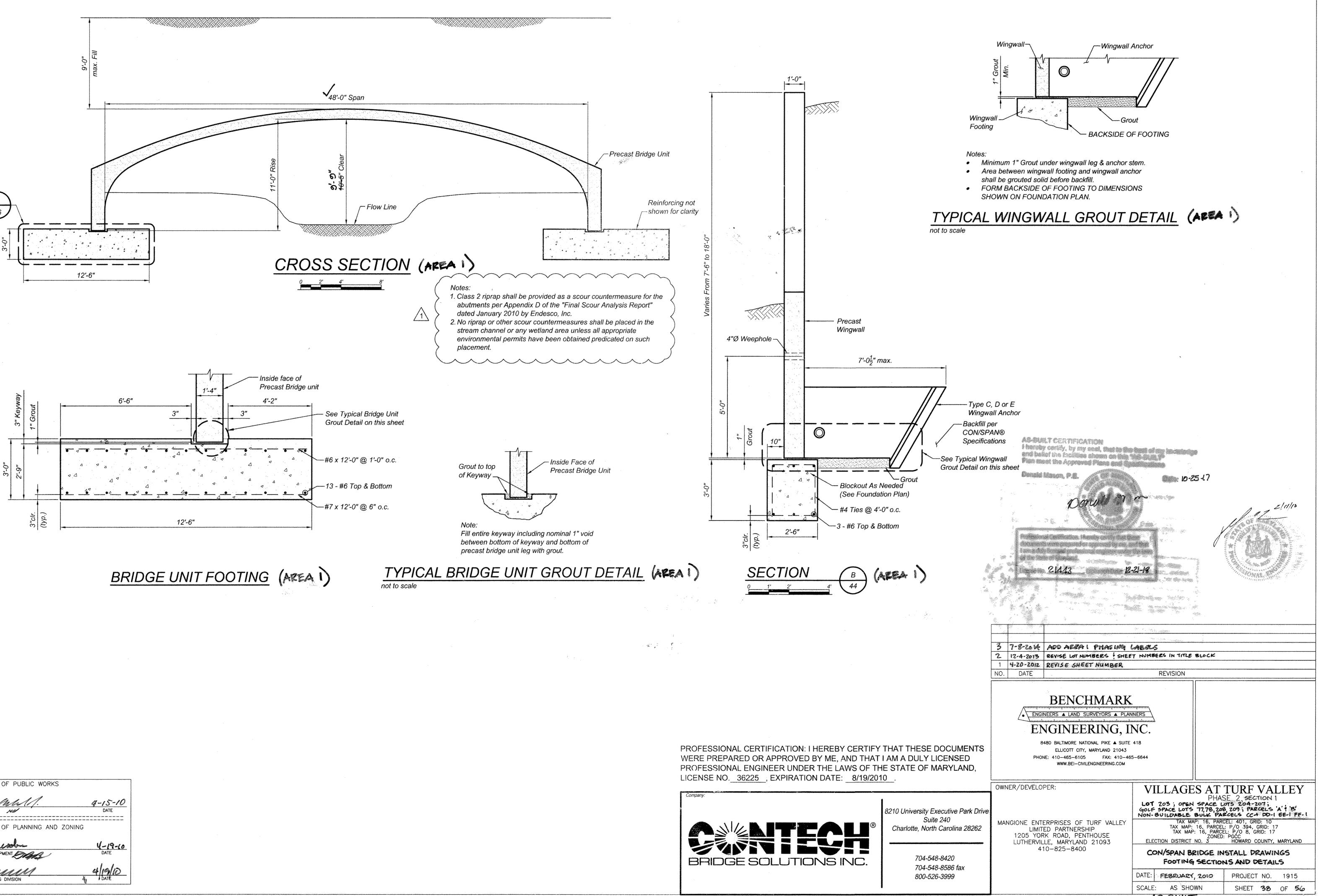
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210 University Executive Park Drive Suite 240 Charlotte, North Carolina 28262	MANGIONE ENTERPRISES OF TURF VALLEY LIMITED PARTNERSHIP 1205 YORK ROAD, PENTHOUSE LUTHERVILLE, MARYLAND 21093 410-825-8400	PHASE 2, SECTION 1 LOT ZO3; OPEN SPACE LOTS 204-207; GOLF SPACE LOTS 77,78,208,209; PARCELS A' +'B' NON-BUILDABLE BULK PARCELS CC-1 DD-1 EE-1 FF-1 TAX MAP: 16, PARCEL: 4UT, GRID: 10 TAX MAP: 16, PARCEL: P/O 394, GRID: 17 TAX MAP: 16, PARCEL: P/O 394, GRID: 17 TAX MAP: 16, PARCEL: P/O 8, GRID: 17 ZONED: PGCC ELECTION DISTRICT NO. 3 HOWARD COUNTY, MARYLAND CON/SPAN BRIDGE INSTALL DRAWINGS
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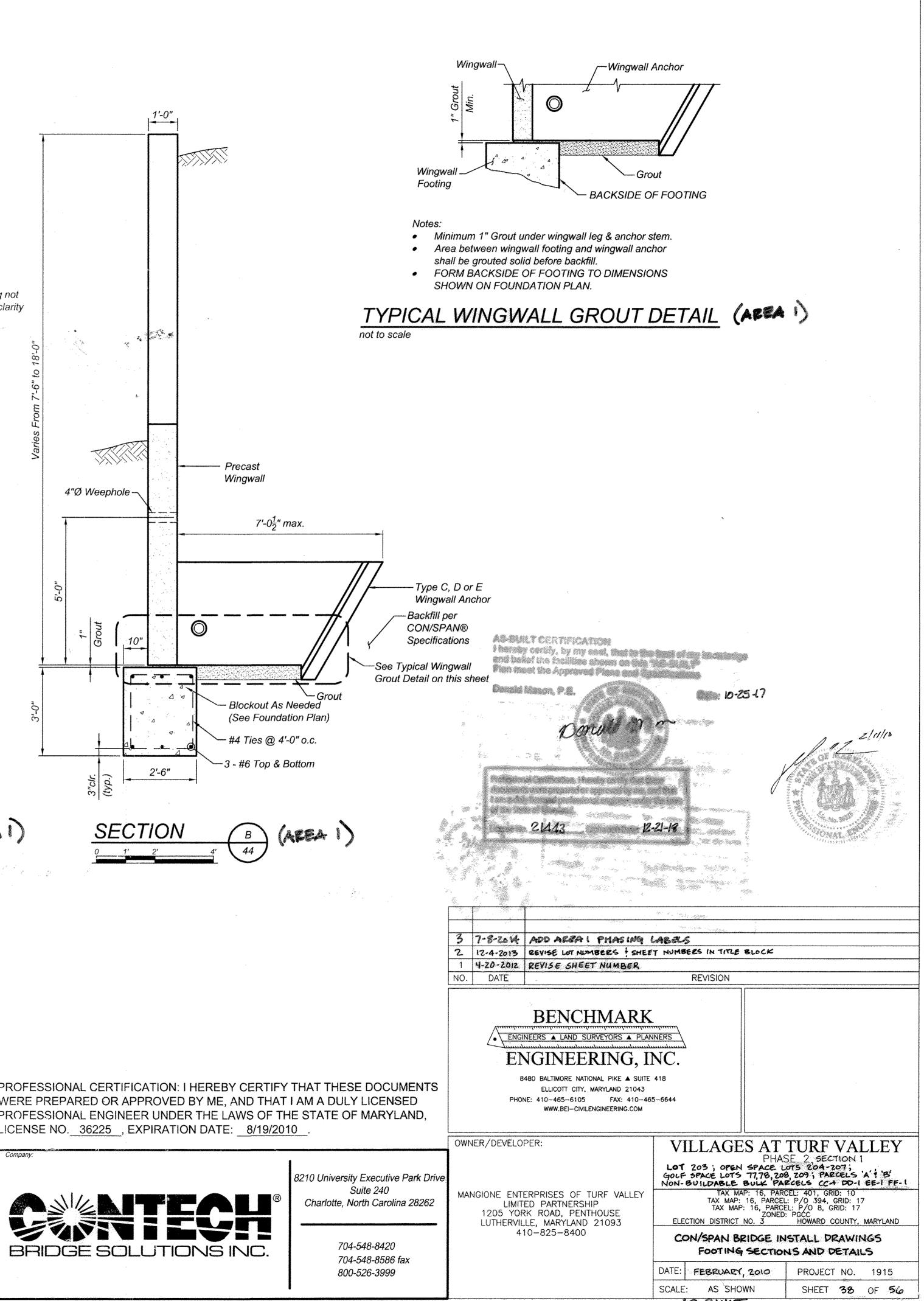


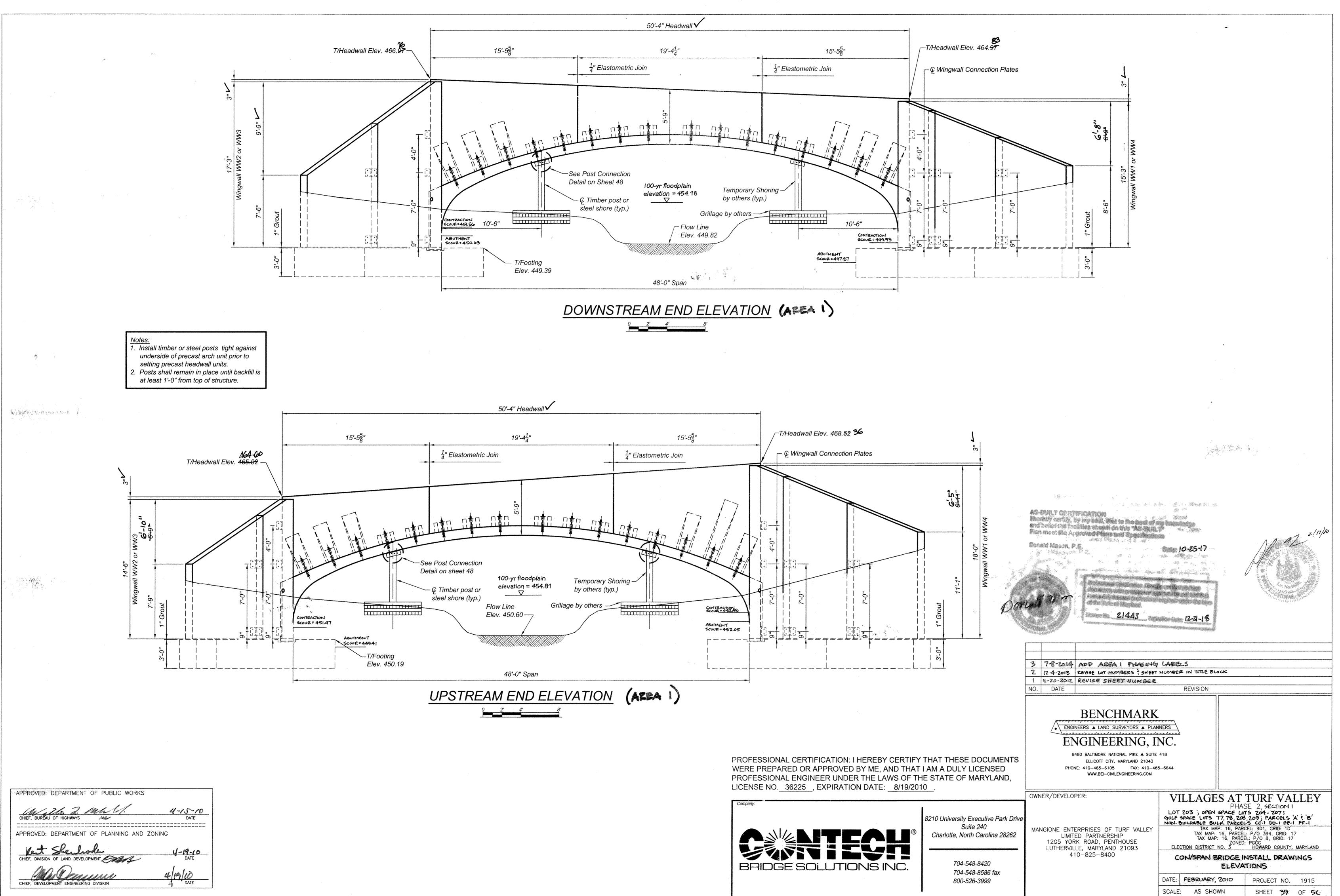


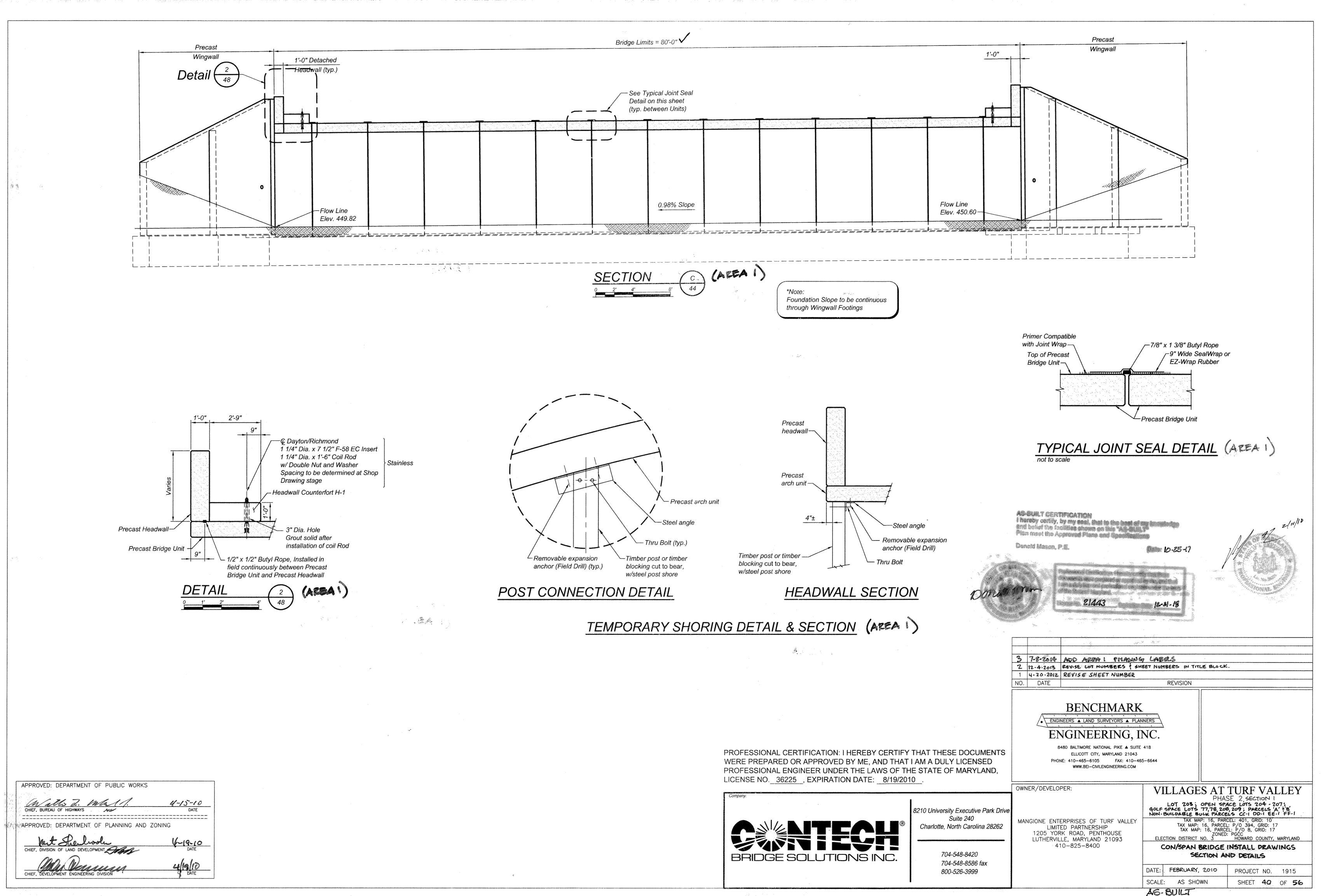
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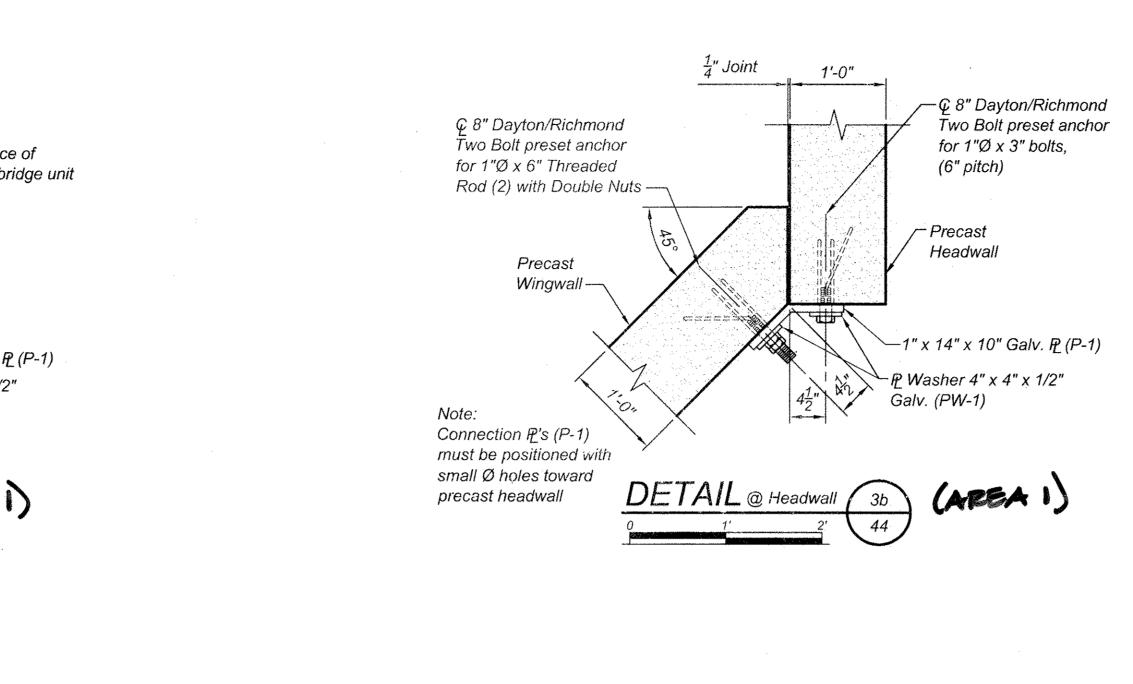




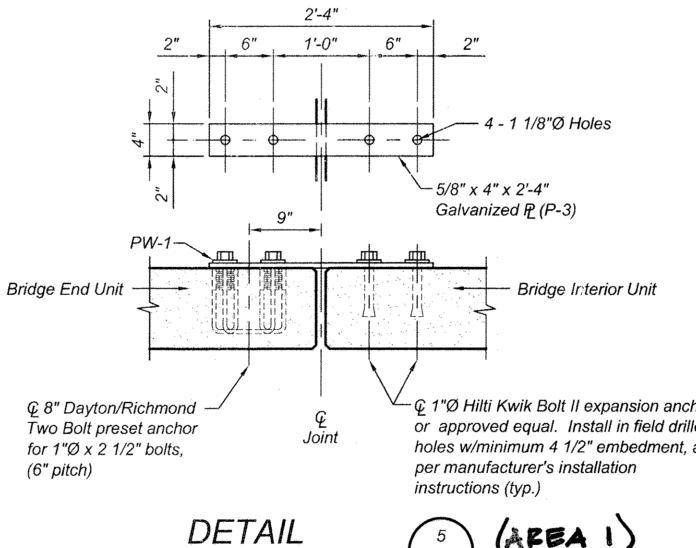
┌─♀ 8" Dayton/Richmond . Two Bolt preset anchor $\frac{1}{4}$ " Joint for 1"Ø x 3" bolts, € 8" Dayton/Richmond (6" pitch) Two Bolt preset anchor Inside face of for 1"Ø x 6" Threaded precast bridge unit Rod (2) with Double Nuts Precast Wingwall — -----└─1" x 14" x 10" Galv. ₽ (P-1) - ₽ Washer 4" x 4" x 1/2" Galv. (PW-1) XX2/ 4<u>1</u>" Note: Connection P's (P-1) must be positioned with small Ø holes toward $\frac{\text{DETAIL} @ Unit Leg}{\frac{3a}{44}} \text{ (AREA I)}$ precast bridge unit 1992 1 1000 Precast Wingwall 10, € 8" Dayton/Richmond -Two Bolt preset anchor for 1"Ø x 6" Threaded Rod (2) with Double Nuts -1" x 14" x 10" Galv. P2 (P-2) IP Washer 4" x 4" x 1/2" Precast -Galv. (PW-1) Wingwall (AREA I) DETAIL 4

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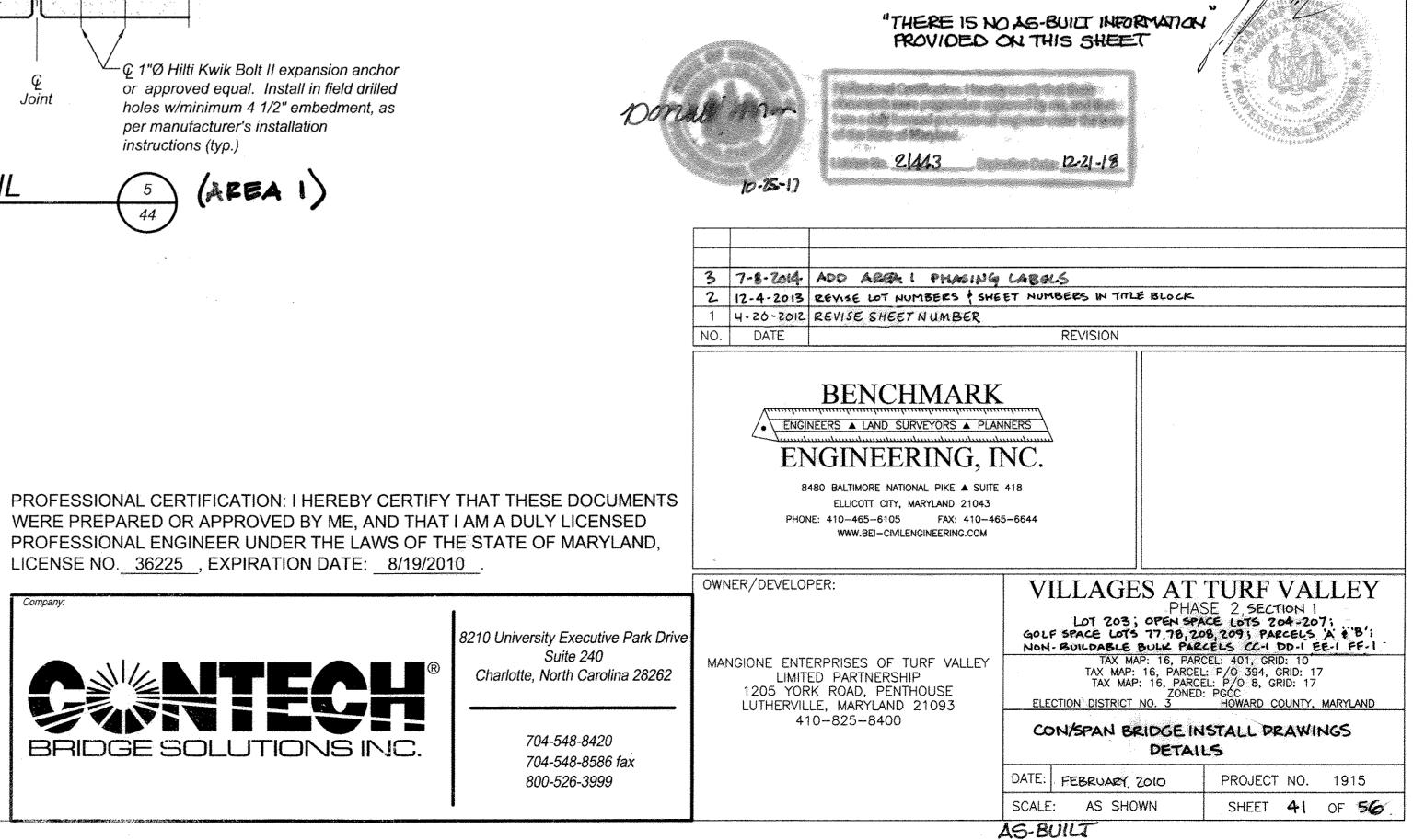
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SPECIFICATIONS FOR MANUFACTURE AND INSTALLATION OF CON/SPAN® BRIDGE SYSTEMS (AFEA I)

1. Description

- 1.1. Type This work shall consist of furnishing and constructing a CON/SPAN® bridge system in accordance with these specifications and in reasonably close conformity with the lines, grades, design and dimensions shown on the plans or as established by the Engineer. In situations where two or more specifications apply to this work, the most stringent requirements shall govern.
- 1.2. Designation Precast reinforced concrete CON/SPAN® bridge units manufactured in accordance with this specification shall be designated by span and rise. Precast reinforced concrete wingwalls and headwalls manufactured in accordance with this specification shall be designated by length, height, and deflection angle.

2. Design

2.1. Specifications The precast elements are designed in accordance with the "Standard Specifications for Highway Bridges" 17th Edition, adopted by the American Association of State Highway and Transportation Officials, 2002. A minimum of one foot of cover above the crown of the bridge units is required in the installed condition. (Unless noted otherwise on the shop drawings and designed accordingly.)

3. Materials

- 3.1. Concrete The concrete for the precast elements shall be air-entrained when installed in areas subject to freeze-thaw conditions, composed of Portland cement, fine and coarse aggregates, admixtures and water. Air-entrained concrete shall contain 6 ± 2 percent air. The airentraining admixture shall conform to AASHT0 M154. The minimum concrete compressive strength shall be as shown on the shop drawings.
- 3.1.1 . Portland Cement Shall conform to the requirements of ASTM Specifications C150-Type I, Type II, or Type III cement. 3.1.2. Coarse Aggregate - Shall consist of stone having a maximum
- size of 1 inch. Aggregate shall meet requirements for ASTM C33.
- 3.1.3. Water Reducing Admixture The manufacturer may submit, for approval by the Engineer, a water-reducing admixture for the purpose of increasing workability and reducing the water requirement for the concrete.
- 3.1.4. Calcium Chloride The addition to the mix of calcium chloride or admixtures containing calcium chloride will not be permitted.
- 3.1.5. Mixture The aggregates, cement and water shall be proportioned and mixed in a batch mixer to produce a homogeneous concrete meeting the strength requirements of this specification. The proportion of Portland cement in the mixture shall not be less than 564 pounds (6 sacks) per cubic vard of concrete.
- 3.2. Steel Reinforcement
- 3.2.1. The minimum steel yield strength shall be 60,000 psi, unless otherwise noted on the shop drawings.
- 3.2.2. All reinforcing steel for the precast elements shall be fabricated and placed in accordance with the detailed shop drawings submitted by the manufacturer.
- 3.2.3. Reinforcement shall consist of welded wire fabric conforming to ASTM Specification A 185 or A 497, or deformed billet steel bars conforming to ASTM Specification A 615, Grade 60. Longitudinal distribution reinforcement may consist of welded wire fabric or deformed billet-steel bars.

3.3. Steel Hardware

- 3.3.1. Bolts and threaded rods for wingwall connections shall conform to ASTM A 307. Nuts shall conform to AASHTO M292 (ASTM A194) Grade 2H. All bolts, threaded rods and nuts used in wingwall connections shall be mechanically zinc coated in accordance with ASTM B695 Class 50.
- 3.3.2. Structural Steel for wingwall connection plates and plate washers shall conform to AASHTO M 270 (ASTM A 709) Grade 36 and shall be hot dip galvanized as per AASHTO M111 (ASTM A123).
- 3.3.3. Inserts for wingwalls shall be 1" diameter Two-Bolt Preset Wingwall Anchors as manufactured by Dayton/Richmond Concrete Accessories, Miamisburg, Ohio, (800) 745-3700.
- 3.3.4. Ferrule Loop Inserts shall be F-64 Ferrule Loop Inserts as manufactured by Dayton/Richmond Concrete Accessories, Miamisburg, Ohio, (800) 745-3700.
- 3.3.5. Hook Bolts used in attached headwall connections shall be ASTM A307.
- 3.3.6. Inserts for detached headwall connections shall be AISI Type 304 stainless steel, F-58 Expanded Coil inserts as manufactured by Dayton/Richmond Concrete Accessories, Miamisburg, Ohio, (800) 745-3700. Coil rods and nuts used in headwall connections shall be AISI Type 304 stainless steel. Washers used in headwall connections shall be either AISI Type 304 stainless steel plate washers or AASHTO M270 (ASTM A709) Grade 36 plate washers hot dip galvanized as per AASHTO M111 (ASTM A123).
- 3.3.7. Reinforcing bar splices shall be made using the Dowel Bar Splicer System as manufactured by Dayton/Richmond Concrete Accessories, Miamisburg, Ohio, (800) 745-3700, and shall consist of the Dowel Bar Splicer (DB-SAE) and Dowel-In (DI).

- 4. Manufacture of Precast Elements Subject to the provisions of Section 5, below, the precast element dimension and reinforcement details shall be as prescribed in the plan and shop drawings provided by the manufacturer.
- 4.1. Forms The forms used in manufacture shall be sufficiently rigid and accurate to maintain the required precast element dimensions within the permissible variations given in Section 5 of these specifications. All casting surfaces shall be of a smooth material. 4.2. Placement of Reinforcement
- 4.2.1. Placement of Reinforcement in Precast Bridge Units The cover of concrete over the outside circumferential reinforcement shall be 2 inches minimum. The cover of concrete over the inside circumferential reinforcement shall be 1 1/2 inches minimum. unless otherwise noted on the shop drawings. The clear distance of the end circumferential wires shall not be less than one inch nor more than two inches from the ends of each section. Reinforcement shall be assembled utilizing single or multiple layers of welded wire fabric (not to exceed 3 layers), supplemented with a single layer of deformed billet-steel bars, when necessary. Welded wire fabric shall be composed of circumferential and longitudinal wires meeting the spacing requirements of 4.3, below, and shall contain sufficient longitudinal wires extending through the bridge unit to maintain the shape and position of the reinforcement. Longitudinal distribution reinforcement may be welded wire fabric or deformed billet-steel bars and shall meet the spacing requirements of 4.3, below. The ends of the longitudinal distribution reinforcement shall be not more than 3 inches and not less than 1 1/2 inches from the ends of the bridge unit.
- 4.2.2. Bending of Reinforcement for Precast Bridge Units The outside and inside circumferential reinforcing steel for the corners of the bridge shall be bent to such an angle that is approximately equal to the configuration of the bridge's outside corner.
- 4.2.3. Placement of Reinforcement for Precast Wingwalls and Headwalls - The cover of concrete over the longitudinal and t ransverse reinforcement shall be 2 inches minimum. The clear distance from the end of each precast element to the end of reinforcing steel shall not be less than 1/2 inch nor more than 3 inches. Reinforcement shall be assembled utilizing a single layer of welded wire fabric, or a single layer of deformed billet-steel bars. Welded wire fabric shall be composed of transverse and longitudinal wires meeting the spacing requirements of 4.3, below, and shall contain sufficient longitudinal wires extending through the element to maintain the shape and position of the reinforcement. Longitudinal reinforcement may be welded wire fabric or deformed billet-steel bars and shall meet the spacing requirements of 4.3, below.

4.3. Laps, Welds, Spacing

- 4.3.1. Laps, Welds, and Spacing for Precast Bridge Units Tension splices in the circumferential reinforcement shall be made by lapping. Laps may be tack welded together for assembly purposes. For smooth welded wire fabric, the overlap shall meet the requirements of AASHTO 8.30.2 and 8.32.6. For deformed welded wire fabric, the overlap shall meet the requirements of AASHTO 8.30.1 and 8.32.5. The overlap of welded wire fabric shall be measured between the outer-most longitudinal wires of each fabric sheet. For deformed billet-steel bars, the overlap shall meet the requirements of AASHTO 8.25. For splices other than tension splices, the overlap shall be a minimum of 12" for welded wire fabric or deformed billet-steel bars. The spacing center to center of the circumferential wires in a wire fabric sheet shall be not less than 2 inches nor more than 4 inches. The spacing center to center of the longitudinal wires shall not be more than 8 inches. The spacing center to center of the longitudinal distribution steel for either line of reinforcing in the top slab shall be not more than 16 inches.
- 4.3.2. Laps, Welds, and Spacing for Precast Wingwalls and Headwalls - Splices in the reinforcement shall be made by lapping. Laps may be tack welded together for assembly purposes. For smooth welded wire fabric, the overlap shall meet the requirements of AASHTO 8.30.2 and 8.32.6. For deformed welded wire fabric, the overlap shall meet the requirements of AASHTO 8.30.1 and 8.32.5. For deformed billet-steel bars, the overlap shall meet the requirements of AASHTO 8.25. The spacing center-to-center of the wires in a wire fabric sheet shall be not less than 2 inches nor more than 8 inches.
- 4.4. Curing The precast concrete elements shall be cured for a sufficient length of time so that the concrete will develop the specified compressive strength in 28 days or less. Any one of the following methods of curing or combinations thereof shall be used: 4.4.1. Steam Curing - The precast elements may be low-pressure
- steam cured by a system that will maintain a moist atmosphere. 4.4.2. Water Curing - The precast elements may be water cured by
- any method that will keep the sections moist. 4.4.3. Membrane Curing - A sealing membrane conforming to the requirements of ASTM Specification C309 may be applied and shall be left intact until the required concrete compressive strength is attained. The concrete temperature at the time of

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CHIEF, DIVISION OF LAND DEVELOPMENT	V-19-10 DATE
CHIEF, DEVELOPMENT ENGINEERING DIVISION	4/19/10 DATE

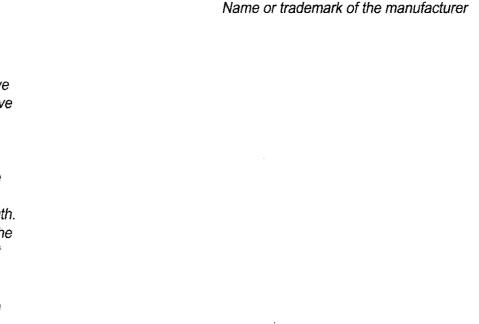
strength is attained. The concrete temperature at the time of reinforcement shall conform to the tolerances prescribed in the application shall be within +/- 10 degrees F of the atmospheric ASTM Specification for that type of reinforcement. temperature. All surfaces shall be kept moist prior to the 5.2. Wingwalls & Headwalls application of the compounds and shall be damp when the 5.2.1. Wall Thickness - The wall thickness shall not vary from that compound is applied. shown in the design by more than 1/2 inch. 4.5. Storage, Handling & Delivery 5.2.2. Length/ Height of Wall sections - The length and height of the 4.5.1. Storage wall shall not vary from that shown in the design by more than Precast concrete bridge elements shall be lifted and stored in 1/2 inch. 5.2.3. Position of Reinforcement - The maximum variation in the "as-cast" position. Precast concrete headwall and wingwall units are cast, stored and position of the reinforcement shall be $\pm 1/2$ inch. In no case shipped in a flat position. shall the cover over the reinforcement be less than 1 1/2 inches The precast elements shall be stored in such a manner to 5.2.4. Size of Reinforcement - The permissible variation in diameter of prevent cracking or damage. Store elements using timber any reinforcing shall conform to the tolerances prescribed in the ASTM Specification for that type of reinforcing. Steel area supports as appropriate. The units shall not be moved until the concrete compressive strength has reached a minimum of 2500 greater than that required shall not be cause for rejection. psi, and they shall not be stored in an upright position. 6. Testing/Inspection 4.5.2 Handling 6.1. Testing Handling devices shall be permitted in each precast element for 6.1.1. Type of Test Specimen - Concrete compressive strength shall the purpose of handling and setting. be determined from compression tests made on cylinders or Spreader beams may be required for the lifting of precast cores. For cylinder testing, a minimum of 3 cylinders shall be concrete bridge elements to preclude damage from bending or taken for each lot of bridge elements. (A lot is defined as the torsion forces. precast elements made using the same concrete mix during a 4.4.3. Delivery single day's production.) For core testing, one core shall be cut Precast concrete elements must not be shipped until the concrete from each of 3 precast elements selected at random from each has attained the specified design compressive strength, or as group of 15 or fewer elements made using a single concrete mix directed by the design Engineer. in the same day's production. Each lot shall be considered Precast concrete elements may be unloaded and placed on the separately for the purpose of testing and acceptance. ground at the site until installed. Store elements using timber 6.1.2. Compression Testing - Cylinders shall be made and tested as supports as appropriate. prescribed by the ASTM C 39 Specification. Cores shall be 4.6. Quality Assurance The Precaster shall demonstrate adherence to the obtained and tested for compressive strength in accordance with standards set forth in the NPCA Quality Control Manual. The the provisions of the ASTM C42 Specification. Precaster shall meet either Section 4.7.1 or 4.7.2 6.1.3. Acceptability of Cylinder Tests - When the average compressive 4.6.1. Certification: The Precaster shall be certified by the strength of all cylinders tested is equal to or greater than the Precast/Prestressed Concrete Institute Plant Certification Program design compressive strength, and not more than 10% of the or the National Precast Concrete Association's Plant Certification cylinders tested have a compressive strength less than the Program prior to and during production of the products covered design concrete strength, and no cylinder tested has a by this specification. compressive strength less than 80% of the design compressive 4.6.2. Qualifications, Testing and Inspection strength, then the lot shall be accepted. When the compressive 4.6.2.1. The Precaster shall have been in the business of strength of the cylinders tested does not conform to these producing precast concrete products similar to those acceptance criteria, the acceptability of the lot may be specified for a minimum of three years. He shall determined as described in section 6.1.4, below. maintain a permanent quality control department or retain 6.1.4. Acceptability of Core Tests - The compressive strength of the an independent testing agency on a continuing basis. concrete in a lot is acceptable when the average core test The agency shall issue a report, certified by a licensed strength is equal to or greater than the design concrete strength. engineer, detailing the ability of the Precaster to produce When the compressive strength of a core tested is less than the quality products consistent with industry standards. design concrete strength, the precast element from which that 4.6.2.2. The Precaster shall show that the following tests are core was taken may be re-cored. When the compressive performed in accordance with the ASTM standards strength of the re-core is equal to or greater than the design indicated. Tests shall be performed as indicated in concrete strength, the compressive strength of the concrete in Section 6 of these specifications. that lot is acceptable. 4.6.2.2.1. Air Content: C231 or C173 6.1.4.1. When the compressive strength of any recore is less 4.6.2.2.2. Compressive Strength: C31, C39, C497 than the design concrete strength, the precast element 4.6.2.3. The Precaster shall provide documentation demonstrating from which that core was taken shall be rejected. Two compliance with this section to CONTECH® Bridge precast elements from the remainder of the lot shall be Solutions at regular intervals or upon request. selected at random and one core shall be taken from 4.6.2.4. The Owner may place an inspector in the plant when each. If the compressive strength of both cores is equal the products covered by this specification are being to or greater than the design concrete strength, the manufactured. compressive strength of the remainder of that group is 4.6.3. Documentation - The Precaster shall submit Precast Production acceptable. If the compressive strength of either of the Reports to CONTECH® Bridge Solutions as required. two cores tested is less than the design concrete 5. Permissible Variations strength, the remainder of the group shall be rejected or, 5.1. Bridge Units at the option of the manufacturer, each precast element 5.1.1. Internal Dimensions - The internal dimension shall vary not more of the remainder of the group shall be cored and than 1% from the design dimensions nor more than 1-1/2 inches accepted individually, and any of these elements that have cores with less than the design concrete strength whichever is less. 5.1.2. Slab and Wall Thickness - The slab and wall thickness shall not shall be rejected. Plugging Core Holes - The core holes be less than that shown in the design by more than 1/4 inch. A shall be plugged and sealed by the manufacturer in a thickness more than that required in the design shall not be manner such that the elements will meet all of the test cause for rejection. requirements of this specification. Precast elements so 5.1.3. Length of Opposite Surfaces - Variations in laying lengths of two sealed shall be considered satisfactory for use. opposite surfaces of the bridge unit shall not be more than 1/2 6.1.4.2. Test Equipment - Every manufacturer furnishing precast elements under this specification shall furnish all facilities inch in any section, except where beveled ends for laying of curves are specified by the purchaser. and personnel necessary to carry out the test required. 5.1.4. Length of Section - The underrun in length of a section shall not 6.2. Inspection The quality of materials, the process of manufacture, and be more than 1/2 inch in any bridge unit. the finished precast elements shall be subject to inspection by the 5.1.5. Position of Reinforcement - The maximum variation in position of - purchaser. the reinforcement shall be \pm 1/2 inch. In no case shall the cover 7. Joints over the reinforcement be less than 1 1/2 inches for the outside The bridge units shall be produced with flat butt ends. The ends of the circumferential steel or be less than 1 inch for the inside bridge units shall be such that when the sections are laid together they will circumferential steel as measured to the external or internal make a continuous line with a smooth interior free of appreciable surface of the bridge. These tolerances or cover requirements irregularities, all compatible with the permissible variations in section 5, do not apply to mating surfaces of the joints. above. The joint width between adjacent precast units shall not exceed 3/4 5.1.6. Area of Reinfcrcement - The areas of steel reinforcement shall inches. be the design steel areas as shown in the manufacturer's shop

drawings. Steel areas greater than those required shall not be

cause for rejection. The permissible variation in diameter of any

PROFESSIONAL CERTIFICATION: I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND, LICENSE NO. 36225 , EXPIRATION DATE: 8/19/2010





No.

8. Workmanship/ Finish

finish

9. Repairs

10. Rejection

11. Marking

because of any of the following:

10.3. Honeycombed or open texture.

satisfactory joint.

The bridge units, wingwalls, and headwalls shall be substantially free of

fractures. The ends of the bridge units shall be normal to the walls and

centerline of the bridge section, within the limits of the variations given in

section 5, above, except where beveled ends are specified. The faces of

shall be a smooth steel form or troweled surface. Trapped air pockets

the wingwalls and headwalls shall be parallel to each other, within the limits

of variations given in section 5, above. The surface of the precast elements

causing surface defects shall be considered as part of a smooth, steel form

Precast elements may be repaired, if necessary, because of imperfections in

manufacture or handling damage and will be acceptable if, in the opinion of

the purchaser, the repairs are sound, properly finished and cured, and the

The precast elements shall be subject to rejection on account of any of the

specification requirements. Individual precast elements may be rejected

10.1. Fractures or cracks passing through the wall, except for a single end

crack that does not exceed one half the thickness of the wall.

Each bridge unit shall be clearly marked by waterproof paint. The following

10.2. Defects that indicate proportioning, mixing, and molding not in

10.4. Damaged ends, where such damage would prevent making a

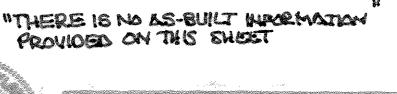
shall be shown on the inside of the vertical leg of the bridge section:

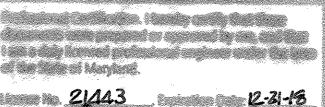
Bridge Span X Bridge Rise

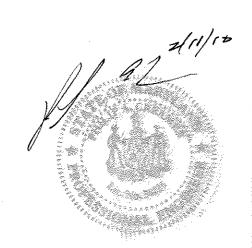
Date of Manufacture

compliance with section 4 of these specifications.

repaired section conforms to the requirements of this specification.







3	7-8-2014	APD AREAI PHASING LABEL			
2	12-4-2013	REVISE LOT NUMBERS & SHEET NUMBERS IN TITLE BLOCK			
1	4-20-2012	EVISE SHEET NUMBER			
NO.	DATE	REVISION			
					

ENGINEERING, INC

8480 BALTIMORE NATIONAL PIKE A SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644 WWW.BEI-CIVILENGINEERING.COM

OWNER/DEVELOPER:

MANGIONE ENTERPRISES OF TURF VALLEY LIMITED PARTNERSHIP 1205 YORK ROAD, PENTHOUSE LUTHERVILLE, MARYLAND 21093 410-825-8400

VILLAGES AT TURF VALLEY PHASE 2 SECTION 1 LOTS 203; OPEN SPACE LOTS 204-207; GOLF SPACE LOTS 77,78,208,209; PARCELS A & B NON-BUILDABLE BULK PARCELS CC-1 DD-1 EE-1 FF-1 TAX MAP: 16, PARCEL: 401, GRID: 10 TAX MAP: 16, PARCEL: P/O 394, GRID: 17 TAX MAP: 16, PARCEL: P/O 8, GRID: 17 TAX MAP: 16, PARCEL: P/O 8, GRID: 17 ZONED: PGCC ELECTION DISTRICT NO. 3 HOWARD COUNTY, MARYLAND

CON/SPAN BRIDGE INSTALL DRAWINGS Anne and same as 15

SPECIF	CATIONS				
DATE: FEBRUARY, 2010	PROJECT	NO.	19	15	
SCALE: AS SHOWN	SHEET	42	OF	56	ante
AS-BUILT					****

704-548-8420 704-548-8586 fax 800-526-3999

Suite 240

SPECIFICATIONS FOR MANUFACTURE AND INSTALLATION OF CON/SPAN® B

12. Installation Preparation

To ensure correct installation of the precast concrete bridge system, care and caution must be exercised in forming the support areas for bridge units, headwall, and wingwall elements. Exercising special care will facilitate the rapid installation of the precast components.

12.1. Footings

Do not over excavate foundations unless directed by site soil engineer to remove unsuitable soil.

The site soils engineer shall certify that the bearing capacity meets or exceeds the footing design requirements, prior to the contractor pouring of the footings. A copy of the report shall be submitted to CONTECH® Bridge Solutions prior to shipment of precast concrete elements.

The bridge units and wingwalls shall be installed on either precast or cast-in-place concrete footings. The size and elevation of the footings shall be as designed by the Engineer. A keyway shall be formed in the top surface of the bridge footing as specified on the plans. No keyway is required in the wingwall footings, unless otherwise specified on the plans.

The footings shall be given a smooth float finish and shall reach a compressive strength of 2,000 psi before placement of the bridge and wingwall elements. Backfilling shall not begin until the footing has reached the full design compressive strength without written approval from CONTECH® Bridge Solutions.

The footing surface shall be constructed in accordance with grades shown on the plans. When tested with a 10-foot straight edge, the surface shall not vary more than 1/4 inch in 10 feet.

If a precast concrete footing is used, the contractor shall prepare a 4-inch thick base layer of compacted granular material the full width of the footing prior to placing the precast footing.

The foundations for precast concrete bridge elements and wingwalls must be connected by reinforcement to form one monolithic body. Expansion joints shall not be used.

The contractor shall be responsible for the construction of the foundations per the plans and specifications.

13. Installation

13.1. General The installation of the precast concrete elements shall be as explained in the publication CON/SPAN Bridge Systems Installation Handbook.

13.1.1. Lifting It is the responsibility of the contractor to ensure that a crane of the correct lifting capacity is available to handle the precast concrete units. This can be accomplished by using the weights given for the precast concrete components and by determining the lifting reach for each crane unit. Site conditions must be checked well in advance of shipping to ensure proper crane location and to avoid any lifting restrictions. The lift anchors or holes provided in each unit are the only means to be used to lift the elements. The precast concrete elements must not be supported or raised by other means than those given in the manuals and drawings without written approval from CONTECH® Bridge Solutions.

13.1.2. Construction equipment weight restrictions: In no case shall equipment operating in excess of the design load (HS20 or HS25) be permitted over the bridge units unless approved by CONTECH® Bridge Solutions.

13.1.2.1 In the immediate area of the bridge units, the following restrictions for the use of heavy construction

machinery during backfilling operations apply: No construction equipment shall cross the bare

- precast concrete bridge unit. • After the compacted fill level has reached a
- minimum of 4 inches over the crown of the bridge, construction equipment with a weight of less than 10 tons may cross the bridge.
- After the compacted fill level has reached a minimum of 1 foot over the crown of the bridge, construction equipment with a weight of less than 30 tons may cross the bridge.
- After the compacted fill level has reached the design cover, or 2 feet minimum, over the crown of the precast concrete bridge, construction equipment within the design load limits for the road may cross the precast concrete bridge.

13.1. Leveling Pad/ Shims The bridge units and wingwalls shall be set on masonite or steel shims measuring 6" x 6", minimum, unless shown otherwise on the plans. A minimum gap of 1/2 inch shall be provided between the footing and the bottom of the bridge's vertical legs or the bottom of the wingwall.

13.3. Placement of Bridge Units

The bridge units shall be placed as show drawings. Special care shall be taken in true line and grade. The joint width between shall not exceed 3/4 inches.

It is imperative that any lateral spreading avoided during and after their placement. ties are shipped in the larger bridge elem If, due to site restrictions, these ties must placement of the bridge element, the con wedges on site. These hardwood wedge outside the legs of the bridge elements, wedges are added before complete relea the crane. Also, a supply of 1/4, 1/2 inch ar shims for various shimming purposes sho 13.2.

13.4. Placement of Wingwalls & Headwalls

The wingwalls and headwalls shall be pla drawings. Special care shall be taken in true line and grade.

- 13.5. Waterproofing/ Joint protection and Subs 13.5.1. External Protection of Joints - The adjoining bridge units shall be con preformed bituminous joint sealar wide joint wrap. The surface sha applying the joint material. A prin wrap to be used shall be applied inches on each side of the joint. either EZ-WRAP RUBBER by PF CORPORATION, SEAL WRAP b MANUFACTURING CO. INC. or a be covered continuously from the leg, across the top of the bridge a section leg. Any laps that result
 - minimum of six inches long with t 13.5.2. In addition to the joints between the end bridge unit and the heady described above. If precast wing between the end bridge unit and with a 2'-0" strip of filter fabric. Al the bridge units, they shall be prin 9" square of joint wrap.
- 13.5.3. During the backfilling operation, joint wrap in its proper location of 13.5.4. Subsoil drainage shall be as direct 13.6. Grouting
 - 13.6.1. Grouting shall not be performed expected to go below 35° for a pe Fill the bridge-foundation keyway with cer and water or cement mortar com sand and water) with a minimum of 3000 psi. Vibrate as required around the bridge element is con
 - elements have been set with tem grout must attain a minimum com before ties may be removed. 13.6.2. All grout shall have a maximum a
- 13.6.3. Lifting and erection anchor recess 13.7. Backfill
 - 13.7.1. Do not perform backfilling during 13.7.2. No backfill shall be placed agains they have been approved by the Engineer.
 - 13.7.3. Backfill shall be considered as all replaced excavation and new embankment adjacent to the precast concrete elements. The project construction and material specifications, which include the specifications for excavation for structures and roadway excavation and embankment construction, shall apply except as modified in this section.
 - 13.7.4. Backfill Zones In-situ soil
 - Zone A: constructed embankment or overfill.
 - Zone B: fill that is directly associated with precast concrete
 - bridge installation.
 - Zone C: road structure.
 - 13.7.5. Required Backfill Properties

13.7.5.1. In-situ soil

- effective support to the precast concrete bridge units. As a guide, the existing natural ground should be of
- similar quality and density to Zone B material for minimum lateral dimension of one bridge span outside
- of the bridge footing. 13.7.5.2. Zone A
 - Zone A requires fill material with specifications and compacting procedures equal to that for normal road embankments.

APPROVED: DEPARTMENT OF PUBLIC WORKS	- <u>Ales</u>
CHIEF, BUREAU OF HIGHWAYS	<u>4-15-10</u> Date
APPROVED: DEPARTMENT OF PLANNING AND ZONIN	G
CHIEF. DIVISION OF LAND DEVELOPMENT	U-19-10 DATE
CHIEF, DEVELOPMENT ENGINEERING DIVISION	4/19/10

	13.7.5.3. Zone B					
own on the Engineer's plan	Generally, soils shall be reasonably free of organic					
in setting the elements to the	matter, and, near concrete surfaces, free of stones		· · · · · · · · · · · · · · · · · · ·			
tween adjacent precast units	larger than 3 inches in diameter. See charts for					
	detailed descriptions of acceptable soils.		Typical USCS	3	1	1
• • • • • • • • •	13.7.5.4. Zone C		Materials	Group	SubGroup	
ng of the bridge elements be	Zone C is the road section of gravel, asphalt or					
ent. Generally, horizontal cable	concrete built in compliance with local engineering					⊢
ements to prevent this spreading.	practices.		GW, GP, SP		A-1a	
ust be removed prior to				A1		
ontractor must provide hardwood	13.7.6. Placing and Compacting Backfill		GM, SW, SP,		A-1b	
lges are placed in the keyway	Dumping for backfilling is not allowed any nearer than 3 ft from		SM			
s, and smaller shims and	the bridge leg.		·····			┡
lease of the bridge element from			GM, SM, ML,		A-2-4	
and 1/8-thick steel or masonite	The fill must be placed and compacted in layers not exceeding		SP, GP	A2		
should be on site, per section	8 inches. The maximum difference in the surface levels of the				100	
	fill on opposite sides of the bridge must not exceed 2 feet.		SC, GC, GM		A-2-5	
				 		⊢
placed as shown on the plan	The fill behind wingwalls must be placed at the same time as		SP, SM, SW	A3		
in setting the elements to the	that of the bridge fill. It must be placed in progressively placed		, ,			
	horizontal layers not exceeding 8 inches per layer.		ML, SM, SC	Δ.4		
bsurface Drainage			IVIL, SIVI, SC	A4		L
The butt joint made by two	The backfill of Zone B shall be compacted to a minimum					
covered with a 7/8" x 1 3/8"	density of 95% of the Standard Proctor, as required by					
lant and a minimum of a 9-inch	AASHTO T-99.					
hall be free of dirt before						
primer compatible with the joint	Soil within 1 foot of concrete surfaces should be		<u> </u>	~		
ed for a minimum width of nine	hand-compacted. Elsewhere, use of rollers is acceptable. If		والمراجع	С	and the state of the	*****
t. The external wrap shall be	vibrating roller-compactors are used, they should not be started					
PRESS-SEAL GASKET	or stopped within Zone B and the vibration frequency should be					
P by MAR MAC	at least 30 revolutions per second.		Γ			
or approved equal. The joint shall			(200	ိမ္မိုိင်	
he bottom of one bridge section	The backfill material and compacting behind wingwalls should	AL		၀ိ မိ ိ	000	
e and to the opposite bridge	satisfy the criteria for the bridge backfill, Zone B.	ΞΨλ	A {			
ult in the joint wrap shall be a		11 H	, (В		
h the overlap running downhill.	Backfill against a waterproofed surface shall be placed carefully			00		
n bridge units, the joint between	to avoid damage to the waterproofing material.	-4	<u>⊥</u> n (°*	\$°°°	/	
adwall shall also be sealed as	13.7.7. Bridge Units	1				
ngwalls are used, the joint	For fill heights over 12 feet, no backfilling may begin until a	\wedge	1 承	ိမ္မ်ိဳး	s /	
id the wingwall shall be sealed	backfill compaction testing plan has been coordinated with and	└─_In-sit	u			
Also, if lift holes are formed in	approved by CONTECH® Bridge Solutions. Cost of the backfill	Soil -				
primed and covered with a 9" x	compaction testing shall be included in the cost of the precast		/ 背)			
	units. This included cost applies only to projects with fill heights		く "副い!	0		
, care shall be taken to keep the	over 12 feet (as measured from top crown of bridge to finished		U- X-		T	
over the joint.	grade).				ليستجمعها	
rected by the engineer.	13.7.8. Wingwalls					Γ
	Backfill in front of wingwalls shall be carried to ground lines					
d when temperatures are	shown in the plans.					F
period of 72 hours.	13.8. Monitoring					1
cement grout (Portland cement	The contractor shall check settlements and horizontal displacement of					
mposed of Portland cement,	foundation to ensure that they are within the allowable limit provided by					
m 28-day compressive strength	the engineer. These measurements should give an indication of the					
d to ensure that the entire key	settlements and deformations along the length of the foundations.					
ompletely filled. If bridge	bolaomonto ana bolonnasione along the tong the total additione.					
emporary ties (cables, bars, etc.)	The first measurement row should take place after the erection of all					
ompressive strength of 1500 psi	precast bridge system elements, a second after completion of					
omprosente su engui en 1000 pol	backfilling, and a third before opening of the bridge to traffic. Further					
n aggregate size of ¼ inch.	measurements may be made according to local conditions.					
esses shall be filled with grout.	mousaremente may se made according to total conditions.			by Anchor	Туре	
	The maximum difference in vertical displacements 'v' should not exceed			A=3'-2"		
ng wet or freezing weather.	1 inch along the length of one foundation.			B=4'-1"	ALO	#
inst any structural elements until	i mon along the rengin of one roundation.			<u>C=5'-1"</u>	1'-0'	
ne Engineer.		Finished G	1	D=6'-1"	Min.	And the second se
io Enginoer.			X	E-7' 1"	1 1 1 1 1 1 1 1	1

Natural ground is to be sufficiently stable to allow

WALL BACKFILL REQUIREM

E=7'-1"

PROFESSIONAL CERTIFICATION: I HEREBY CERTIFY TH WERE PREPARED OR APPROVED BY ME, AND THAT I AM PROFESSIONAL ENGINEER UNDER THE LAWS OF THE S LICENSE NO. 36225 , EXPIRATION DATE: 8/19/2010

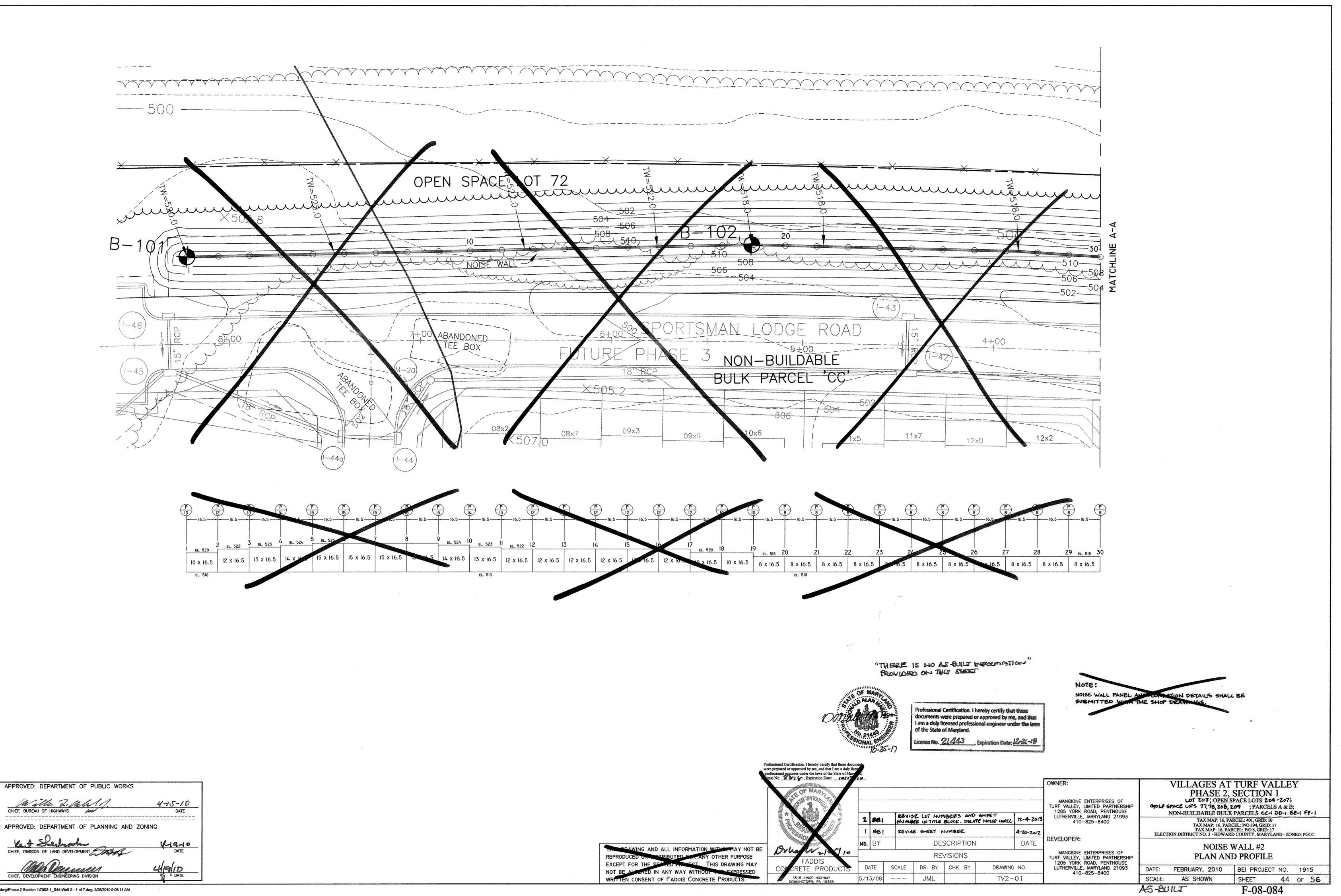
Compacted Material

Precast wingwall ·

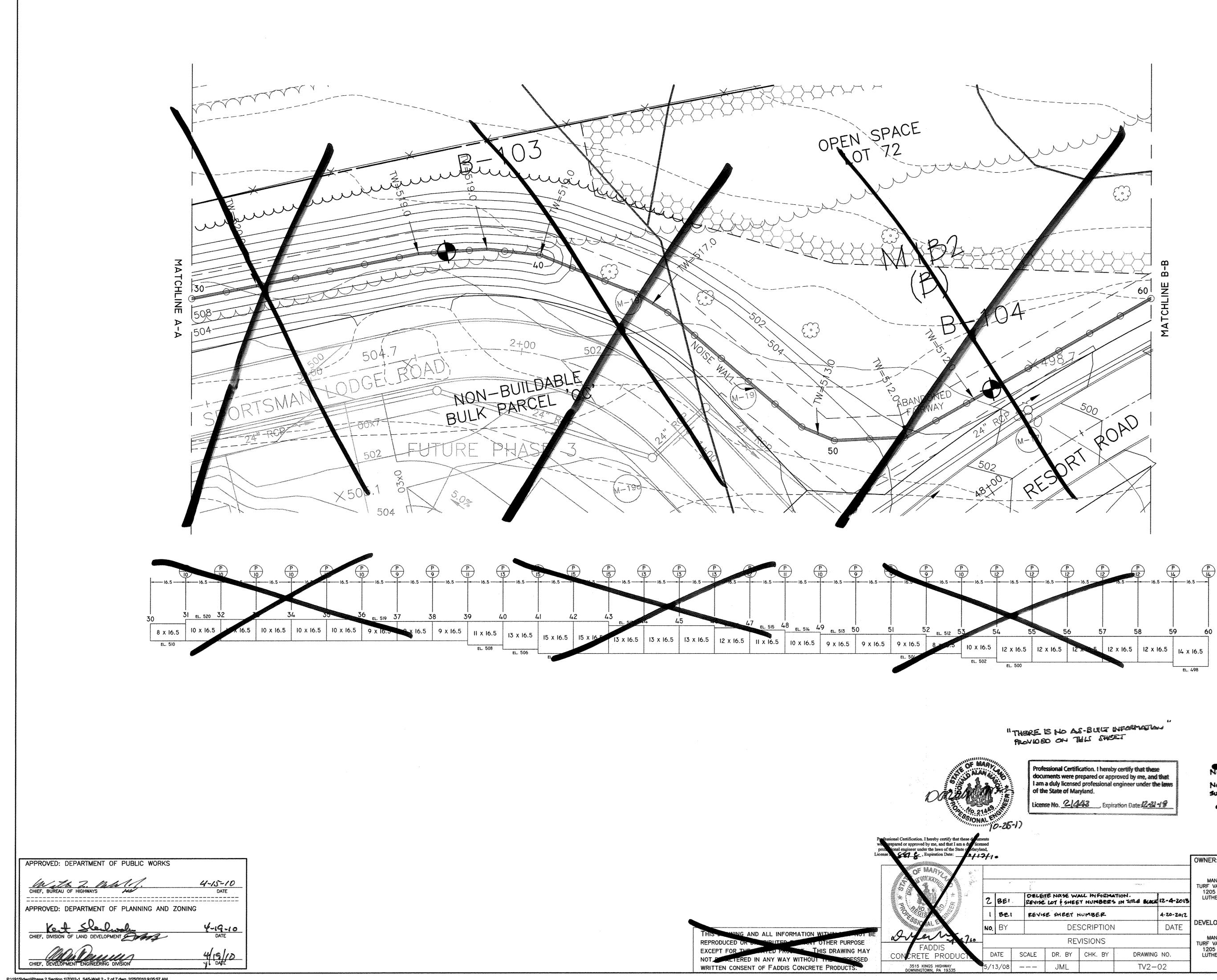
(Same as unit backfill) ----



RIDGE S	YSTE	MS (CC	NT'D)	(AREA)
Acceptable Soils for use in 2	Zone B Backfill			
Percent passing US Sieve No.	Character of Fraction passing No. 40 Sieve			
#10 #40 #200	Liquid Plasticity	- Soil Desription		
50 max 30 max 15 max	Limit Index 6 max	Largely gravel but can include	sand	
50 max 25 max	6 max	and fines Gravelly sand or graded sand,		
35 max	40 max 10 max	Sands, gravels with low-plastic		
35 max	41 max 10 max	silt fines Sands, gravels with plastic silt	2	
51 min 10 max	non-plastic	fines Fine sands		
36 min	40 max 10 max	Low-compressibility silts	· · · · ·	
	inished Grade	••••••••••••••••••••••••••••••••••••••		
jut	ngyaang jega maanin kargat an astat mgan anti-nagyas siya kayas dagaa sa ng aga kana Malaka	С		
Fill Height		***	Limits of Critical Back	kfill
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			ATT.	
L To Roadway	/ Base / e or 2'-0" min.	0*****	In-situ	
		4'-0" min	Soil J	
	_		`	
SPAN FILL HEIGHT INSIL	DE ZONE B			
≤ 24'-0" < 12'-0" A1,	A2, A3, A4			
<u>BACKFILL REQUIRE</u>	A1, A3	AI		
	(,			
Limits of Excavation Zone A	"THERE PROVID	E IS NO AS-BUILT IN NO ON THIS SHE	PORTATION"	21:1/10
Dores	10 mm	21443	E-21-18	
Limits of Critical Backfill Zone	×			1997
(C.B.Z.) Situ	3 7-8-2014 AL	D AREA I PHASING	LABRS	
il	2 12-4-2013 RE	VISE LOT NUMBERST SHEE	den in it der einen sich der der sich der der der der der der der der der der	.oc¥
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MENTS (AREA I)	E	BENCHMARK		
	- Junuluum	RS A LAND SURVEYORS A PLAT		
$i \gamma_{2}$		GINEERING, I		
AT THESE DOCUMENTS		BALTIMORE NATIONAL PIKE ▲ SUITE ELLICOTT CITY, MARYLAND 21043 10-465-6105 FAX: 410-46		
TATE OF MARYLAND,	Service	WWW.BEI-CIVILENGINEERING.COM		
•	OWNER/DEVELOPER		VILLAGES	S AT TURF VALLEY
10 University Executive Park Drive			LOT 203 ; GOLF SPACE LOTS T	PHASE 2 SECTION 1 OPEN SPACE LOTS 204-207; 17.78,208,209; PARCELS A 18;
Suite 240 Suite Charlotte, North Carolina 28262	MANGIONE ENTERP	RISES OF TURF VALLEY PARTNERSHIP	NON-BUILDABLE BU	: 16. PARCELS CC-1 DD-1 EE-1 FF-1
	1205 YORK I LUTHERVILLE,	ROAD, PENTHOUSE MARYLAND 21093	ELECTION DISTRICT N	
704-548-8420	410-	825-8400		RIDGE INSTALL DRAWINGS ECIFICATIONS
704-548-8586 fax 800-526-3999			DATE: FEBRUARY, 2	PROJECT NO. 1915
			SCALE: AS SHOW	N SHEET 43 OF 56



APPROVED:	DEPARTMENT	OF PLANNING	AND ZONIN	G
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CHIEF, DIVISIO	N OF LAND DEVELO	DPMENT	L	DATE
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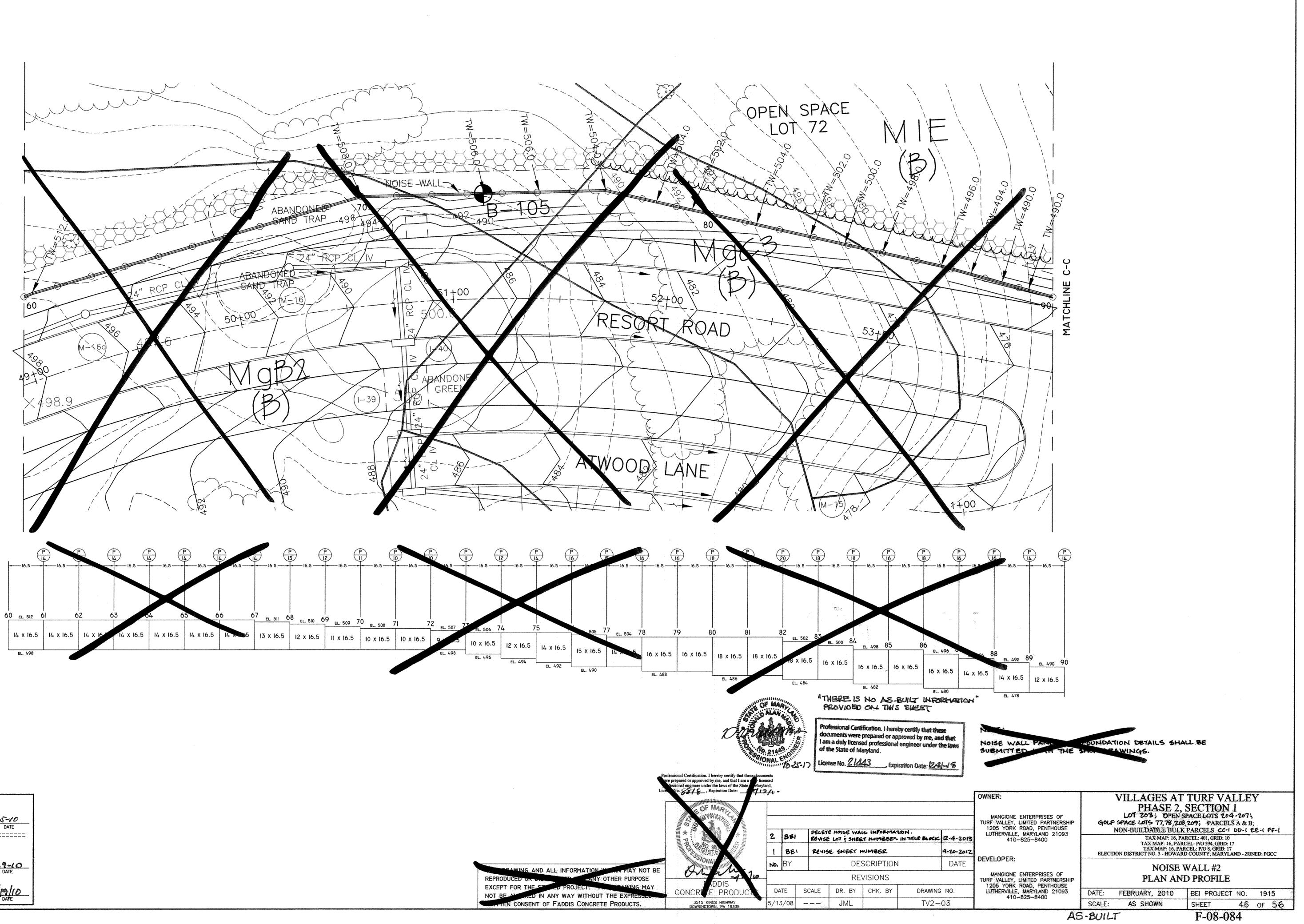


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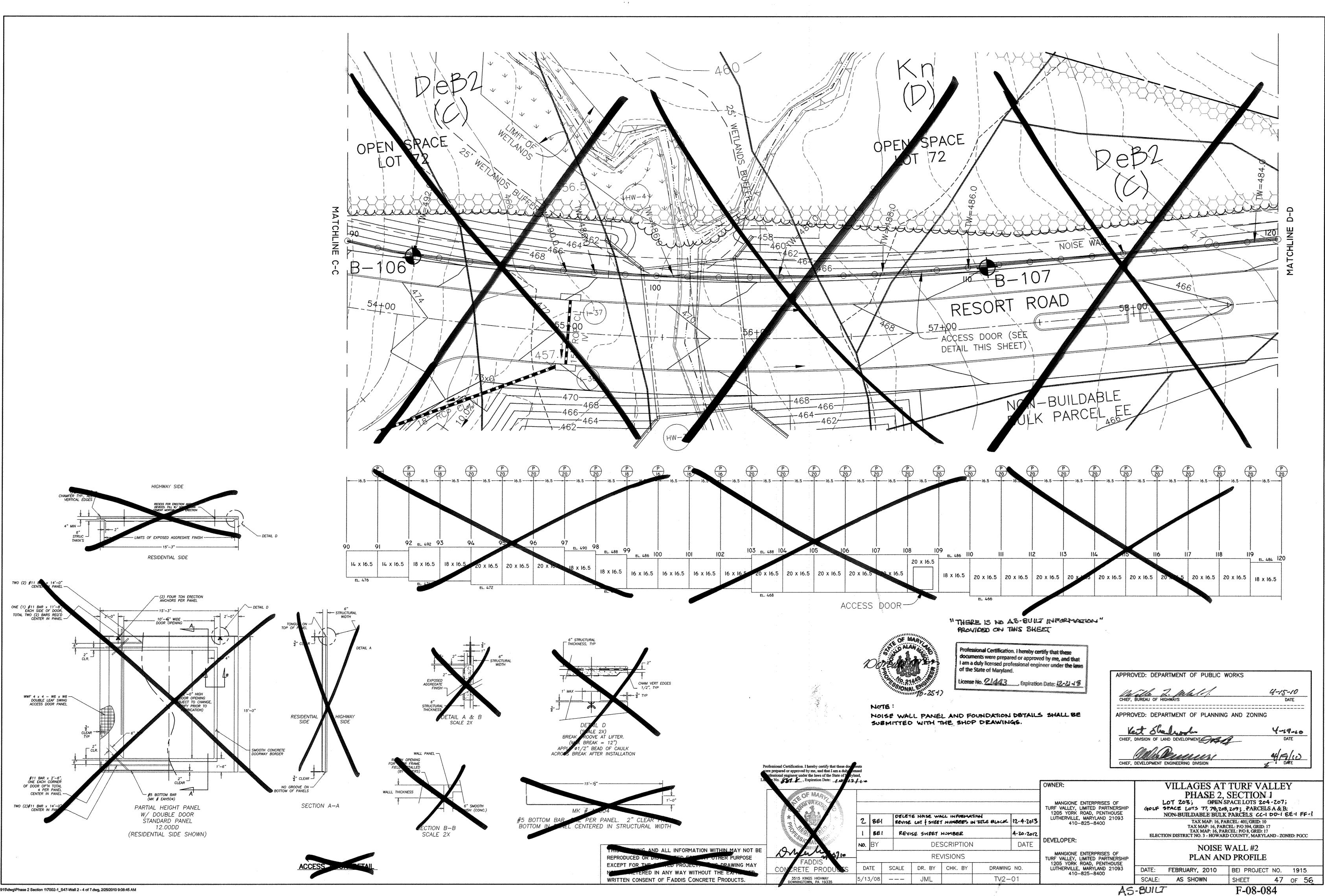
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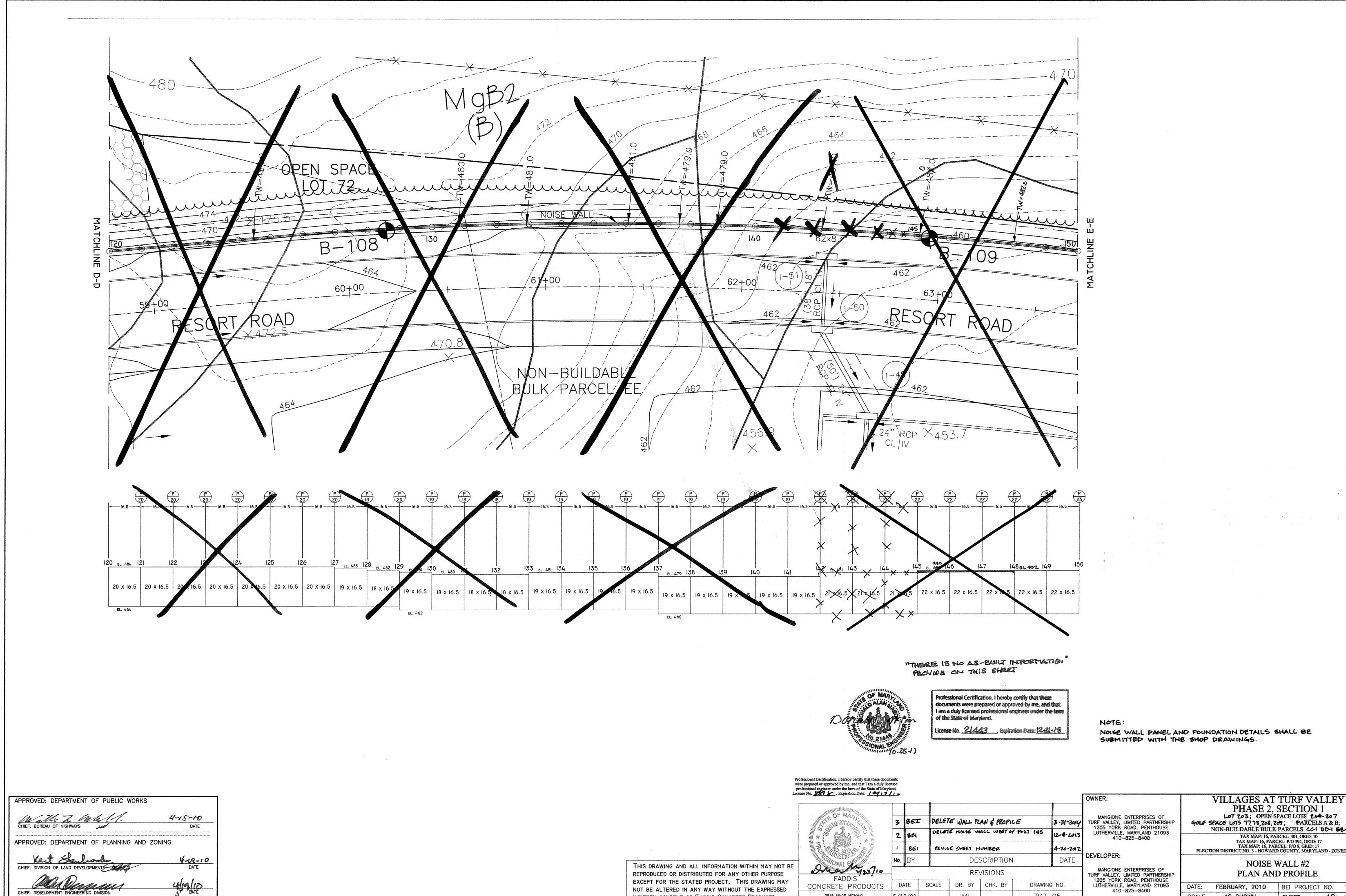
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APPROVED: DEPARTMENT OF PUBLIC WORKS	
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APPROVED: DEPARTMENT OF PLANNING AND	ZONING
CHIEF, DIVISION OF LAND DEVELOPMENT	419-10 DATE
CHIEF, DEVELOPMENT ENGINEERING DIVISION	4/19/10 DATE





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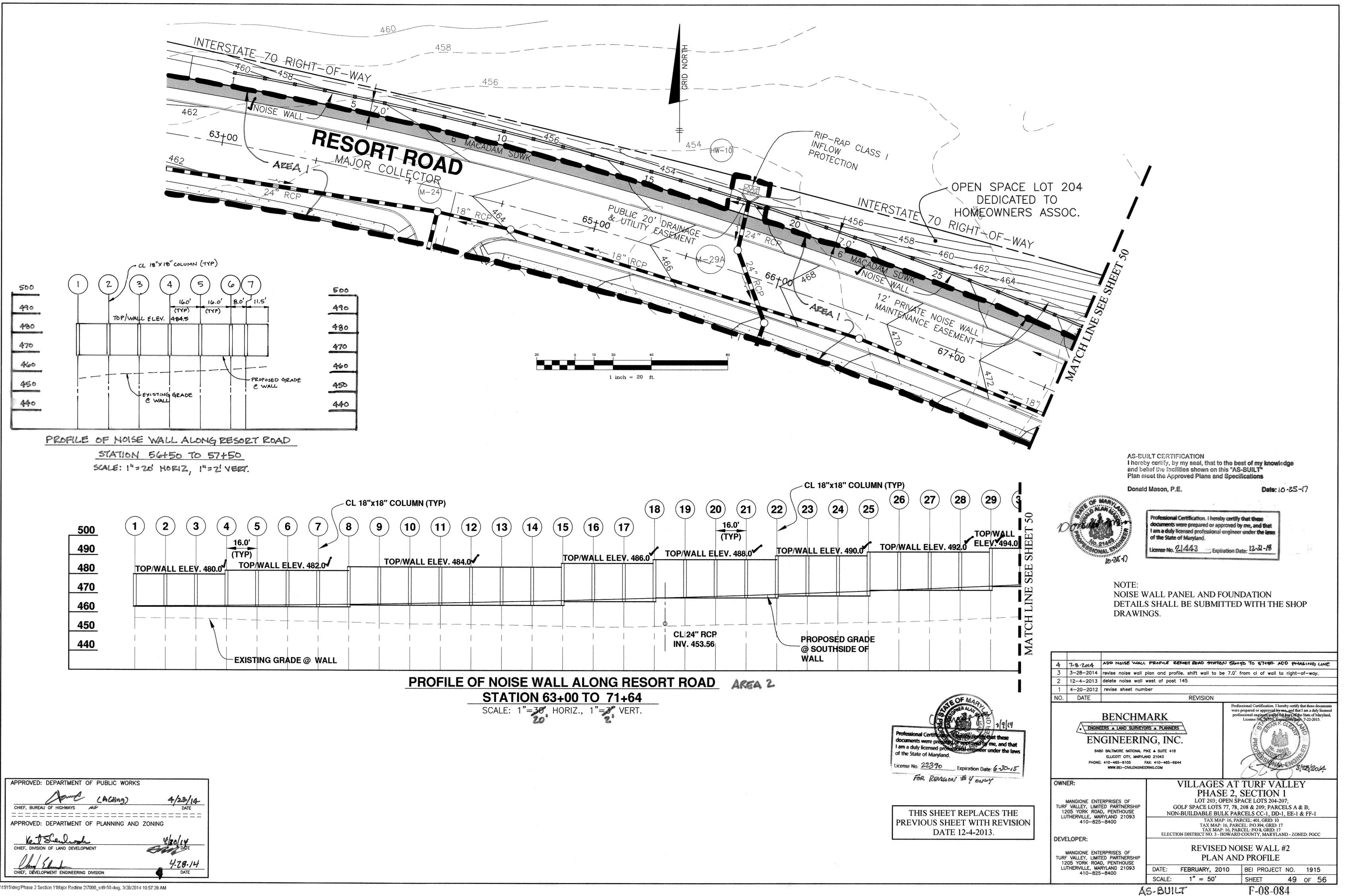
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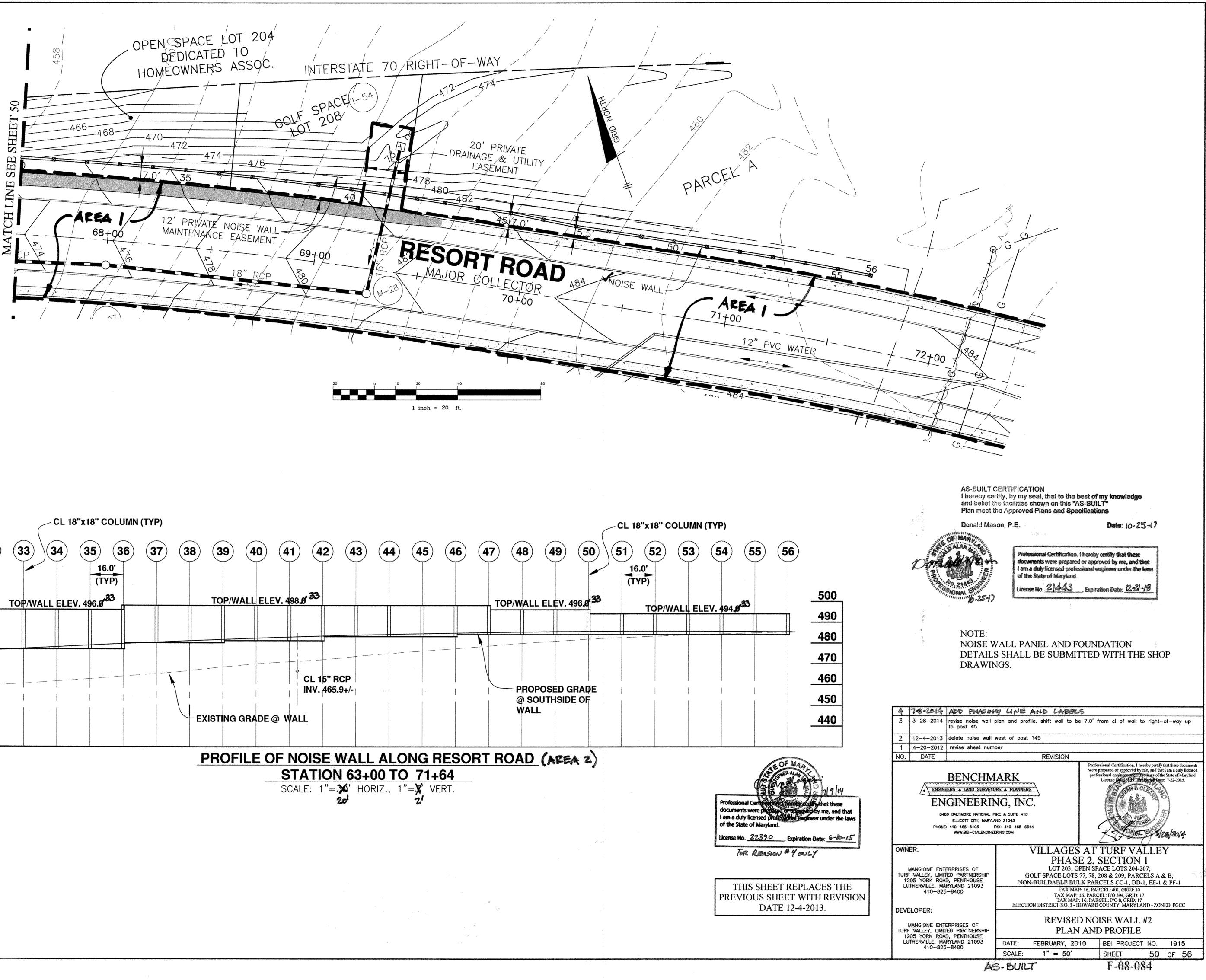
NOT BE ALTERED IN ANY WAY WITHOUT THE EXPRESSED WRITTEN CONSENT OF FADDIS CONCRETE PRODUCTS.

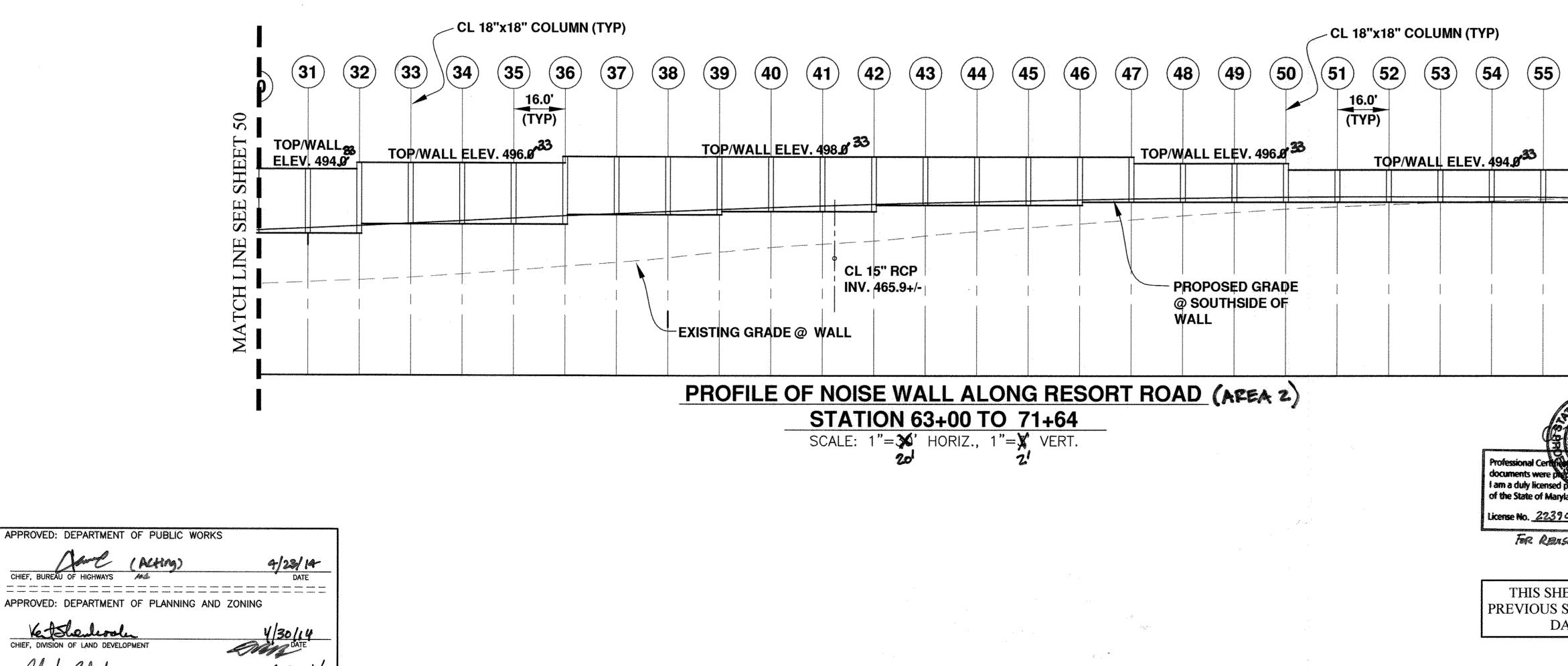
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LUTHERVILLE, MARYLAND 21093 410-825-8400	DATE: FEBRUA	RY, 2010	BEI PROJECT	NO. 191	5
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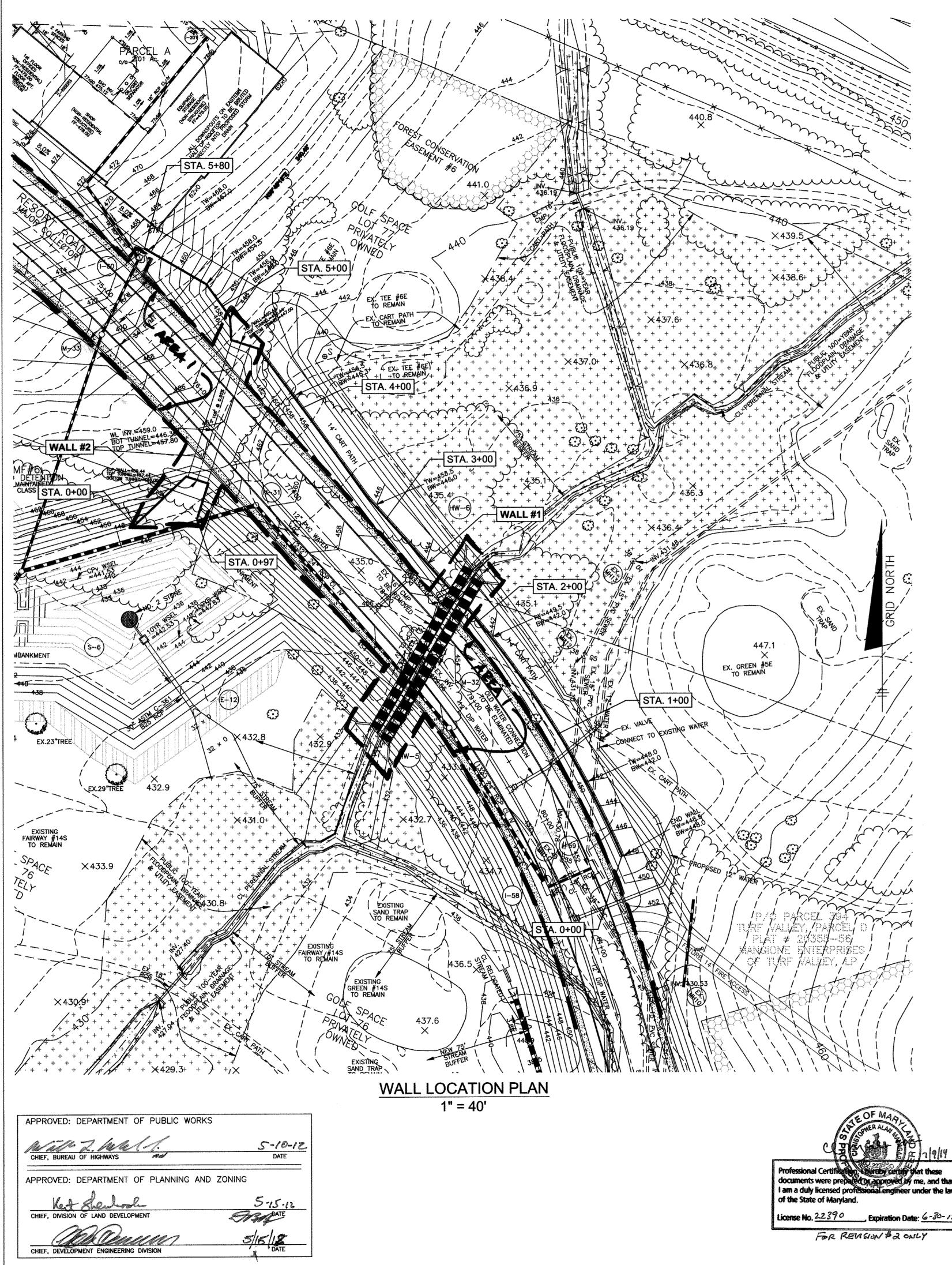
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4.28.14 DATE

CHIEF, DIVISION OF LAND DEVELOPMENT

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CHIEF, DEVELOPMENT ENGINEERING DIVISION



__ Expiration Date: 6-30-15

SPECIFICATIONS

MODULAR CONCRETE BLOCK RETAINING WALL

PART 1: GENERAL

1.01 Description

- A. Work shall consist of furnishing and construction of a Modular Retaining Wall System in accordance with these specifications and in reasonably close conformity with the lines, grades, design, and dimensions shown on the plans.
- B. Work includes preparing foundation soil, furnishing and installing leveling pad, unit drainage fill and backfill to the lines and grades shown on the construction drawings.
- C. Work includes furnishing and installing geogrid soil reinforcement of the type, size, location, and lengths
- designated on the construction drawings. 1.02 Delivery, Storage and Handling
- A. Contractor shall check all materials upon delivery to assure that the proper type, grade, color, and certification has been received.
- B. Contractor shall protect all materials from damage due to job site conditions and in accordance with manufacturer's recommendations. Damaged materials shall not be incorporated into the work.

PART 2: PRODUCTS

- face finish sculptured rock face in angular tri-planer or flat configuration. Other face finishes will not be allowed without written approval of Owner.
- bond configuration running with bonds nominally located at midpoint vertically adjacent units, in both straight and curved alignments.
- or other imperfections when viewed from a distance of 10 feet under diffused lighting.
- B. Modular concrete materials shall conform to the requirements of ASTM C1372 - Standard
- C. Modular concrete units shall conform to the following structural and geometric requirements measured in accordance with appropriate references:
- compressive strength = 3000 psi minimum; absorption = 8% maximum (6% in northern states) for standard weight aggregates;
- dimensional tolerances = ±1/8" from nominal unit dimensions not including rough split face, ±1/16"
- unit height top and bottom planes; unit size 8" (H) x 18" (W) x 12" (D) minimum;
- unit weight 75 lbs/unit minimum for standard weight

aggregates;

- inter-unit shear strength 1000 plf minimum at 2 psi normal pressure; at 2 psi normal force.
- geogrid/unit peak connection strength 1000 plf minimum
- D. Modular concrete units shall conform to the following constructability requirements: (if applicable)
- vertical setback = 1/8"± per course (near vertical) or 1"+ per course per the design; alignment and grid positioning mechanism - fiberglass
- pins, two per unit minimum; maximum horizontal gap between erected units shall
- be 1/2 inch.

2.02 Shear Connectors (if applicable)

- A. Shear connectors shall be 1/2 inch diameter thermoset isopthalic polyester resin-protruded fiberglass reinforcement rods or equivalent to provide connection between vertically and horizontally adjacent units. Strength of shear connectors between vertical adjacent units shall be applicable over a design temperature of 10 degrees F to + 100 degrees F. B.
- Shear connectors shall be capable of holding the geogrid in the proper design position during grid pre-tensioning and backfilling. 2.03 Base Leveling Pad Material

- stone base as shown on the construction drawings.
- 2.04 Unit Drainage Fill

A. Unit drainage fill shall consist of #57crushed stone

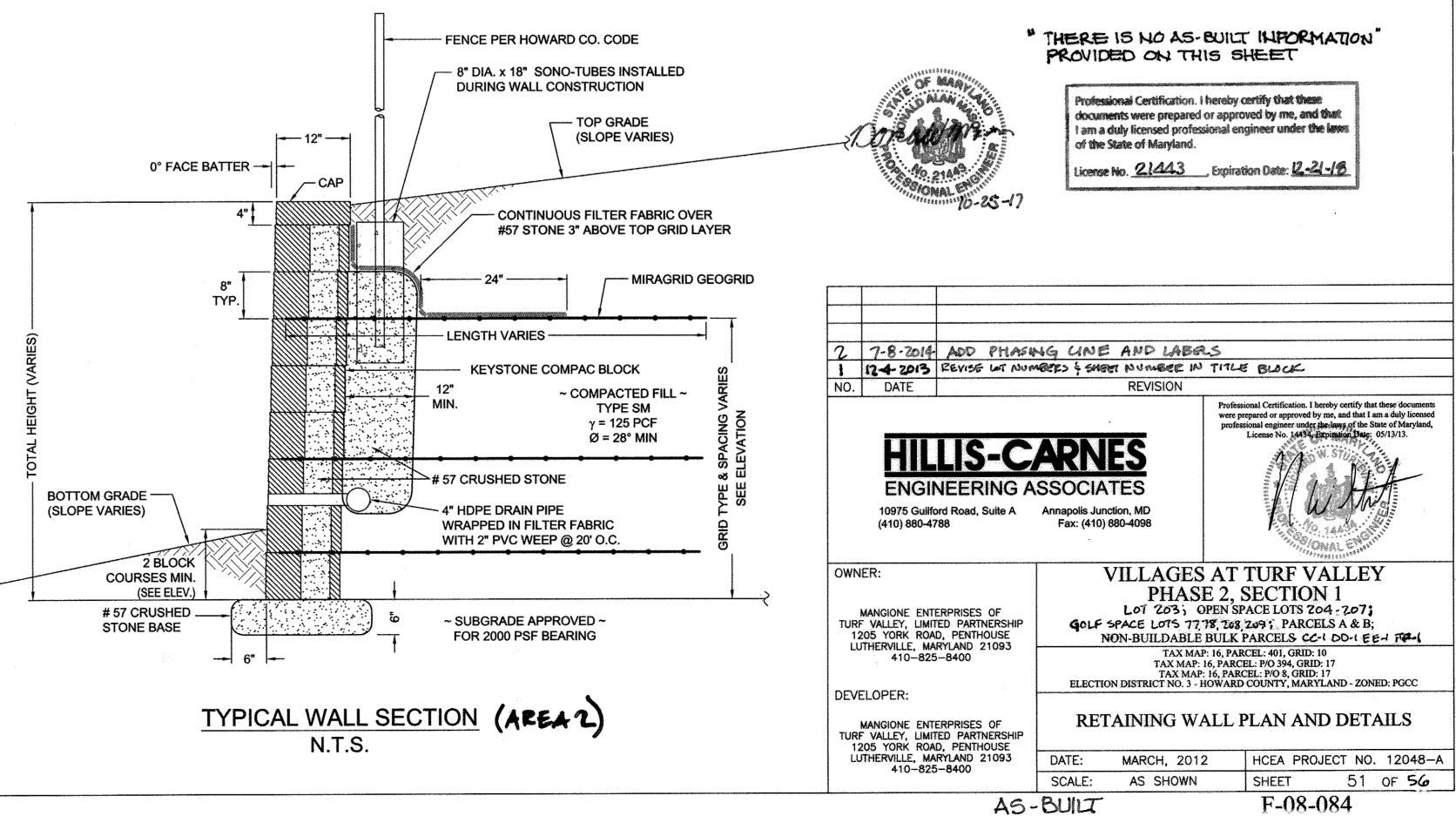
- 2.05 Reinforced Backfill
- meet the following gradation tested in accordance with ASTM D-422 and meet other properties shown on the plan:

Sieve Size	Percent P
2 inch	100-75
3/4 inch	100-75
No. 40	0-60
No. 200	0-40
atiaity Inday (DI) <10 a	nd Liquid Limit ~

- Plasticity Index (PI) <10 and Liquid Limit <35 per ASTM D-4318. B. Material can be site excavated soils where the above
- requirements can be met. Unsuitable soils for backfill (high plastic clays or organic soils) shall not be used in the reinforced soil mass.

2.06 Geogrid Soil Reinforcement

A. Geosynthetic reinforcement shall consist of geogrids manufactured specifically for soil reinforcement



2.01 Modular Concrete Retaining Wall Units

- A. Modular concrete units shall conform to the following architectural requirements: face color - color may be
- specified by the Owner.
- exposed surfaces of units shall be free of chips, cracks
- Specifications for Segmental Retaining Wall Units.

A. Material shall consist of a compacted #57 crushed

A. Reinforced backfill shall type SM, be free of debris and

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applications and shall be manufactured from high tenacity polyester yarn.

2.07 Drainage Pipe

A. The drainage pipe shall be perforated corrugated HDPE pipe manufactured in accordance with ASTM D-1248.

PART 3 EXECUTION

- 3.01 Excavation
- A. Contractor shall excavate to the lines and grades shown on the construction drawings. Owner's representative shall be responsible for inspecting and approving the excavation prior to placement of leveling material or fill soils.

3.02 Base Leveling Pad

- A. Leveling pad material shall be placed to the lines and grades shown on the construction drawings, to a minimum thickness of 6 inches and extend laterally a minimum of 6" in front and behind the modular wall unit.
- B. Leveling pad shall be prepared to insure full contact to the base surface of the concrete units.
- 3.03 Modular Unit Installation
- A. First course of units shall be placed on the leveling pad at the appropriate line and grade. Alignment and level shall be checked in all directions and insure that all units are in full contact with the base and properly seated
- B. Place the front of units side-by-side. Do not leave gaps between adjacent units. Layout of corners and curves shall be in accordance with manufacturer's recommendations.
- C. Install shear/connecting devices per manufacturer's recommendations.
- D. Place and compact drainage fill within and behind wall units. Place and compact backfill soil behind drainage fill. Follow wall erection and drainage fill closely with structure backfill.
- E. Maximum stacked vertical height of wall units, prior to unit drainage fill and backfill placement and compaction, shall not exceed three courses

3.04 Structural Geogrid Installation

- A. Geogrid shall be oriented with the highest strength axis perpendicular to the wall alignment.
- B. Geogrid reinforcement shall be placed at the strengths, lengths, and elevations shown on the construction design drawings or as directed by the Engineer.
- C. The geogrid shall be laid horizontally on compacted backfill and attached to the modular wall units. Place the next course of modular concrete units over the geogrid. The geogrid shall be pulled taut, and anchored prior to backfill placement on the geogrid.

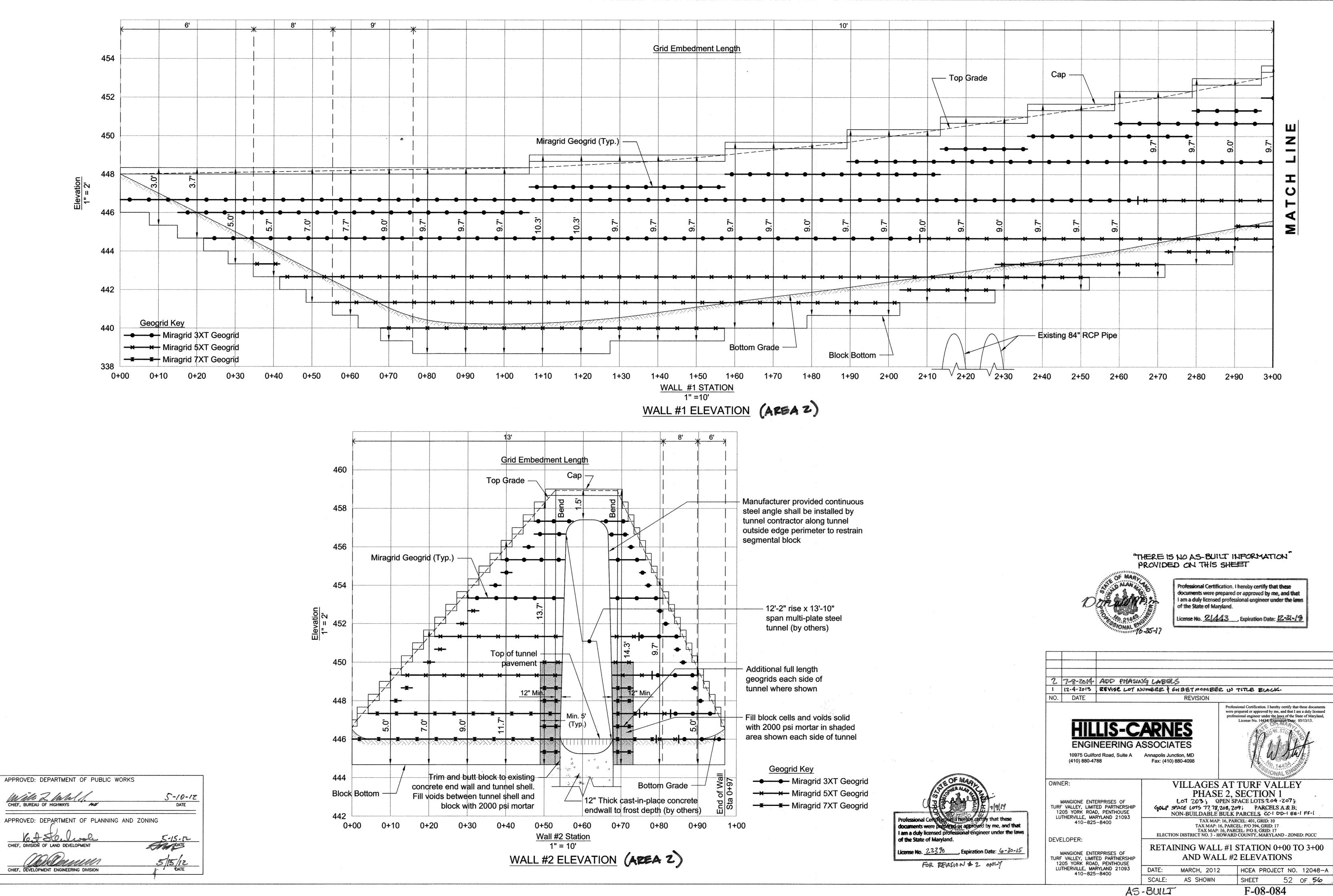
- D. Geogrid reinforcements shall be continuous throughout their embedment lengths and placed side-by-side to provide 100% coverage at each level. Spliced connections between shorter pieces of geogrid or gaps between adjacent pieces of geogrid are not permitted.
- 3.05 Reinforced Backfill Placement
- A. Reinforced backfill shall be placed, spread, and compacted in such a manner that minimizes the development of slack in the geogrid and installation damage.
- B. Reinforced backfill shall be placed and compacted in lifts not to exceed 6 inches where hand compaction is used, or 8 - 10 inches where heavy compaction equipment is used. Lift thickness shall be decreased to achieve the required density as required.
- C. Reinforced backfill shall be compacted to 95% of the maximum density as determined by ASTM D698. The moisture content of the backfill material prior to and during compaction shall be uniformly distributed throughout each layer and shall be + 3% to - 3% of optimum.
- D. Only lightweight hand-operated equipment shall be allowed within 3 feet from the tail of the modular concrete unit.
- E. Tracked construction equipment shall not be operated directly upon the geogrid reinforcement. A minimum fill thickness of 6 inches is required prior to operation of tracked vehicles over the geogrid. Tracked vehicle turning should be kept to a minimum to prevent tracks from displacing the fill and damaging the geogrid.
- F. Rubber tired equipment may pass over geogrid reinforcement at slow speeds, less than 10 MPH. Sudden braking and sharp turning shall be avoided.
- G. At the end of each day's operation, the Contractor shall slope the last lift of reinforced backfill away from the wall units to direct runoff away from wall face. The Contractor shall not allow surface runoff from adjacent areas to enter the wall construction site.

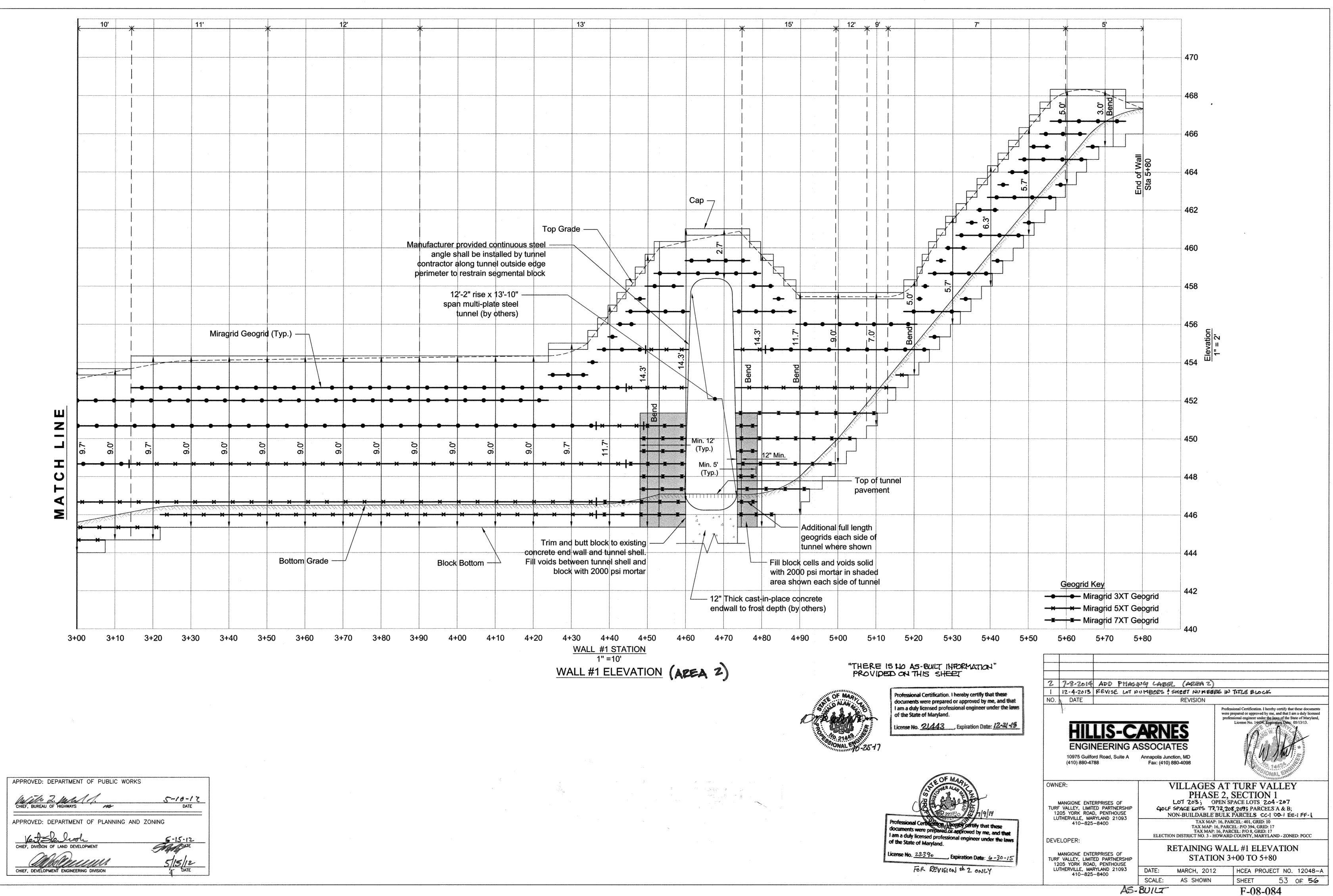
3.06 Cap Installation

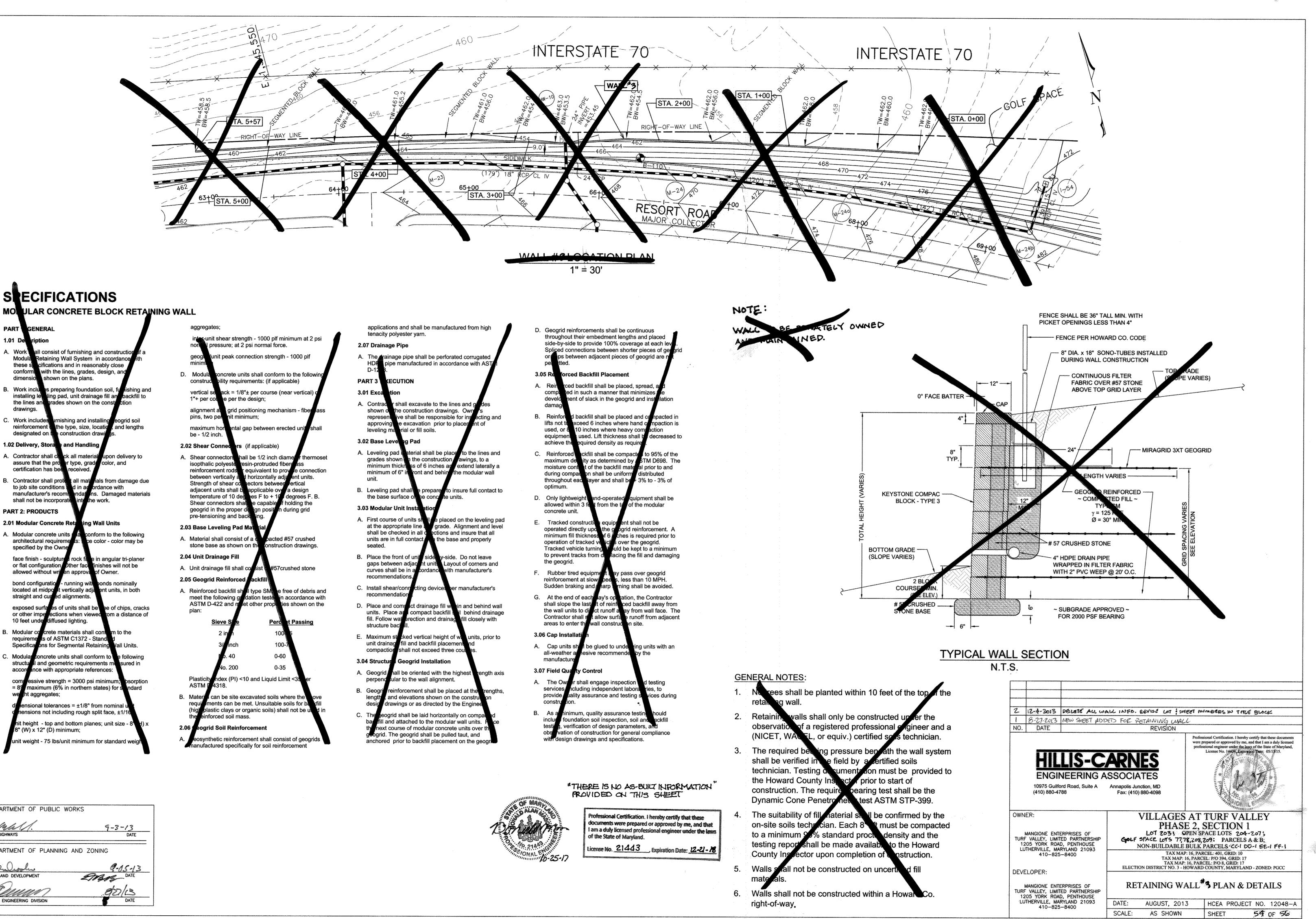
A. Cap units shall be glued to underlying units with an all-weather adhesive recommended by the manufacturer.

3.07 Field Quality Control

- A. The Owner shall engage inspection and testing services, including independent laboratories, to provide quality assurance and testing services during construction.
- B. As a minimum, quality assurance testing should include foundation soil inspection, soil and backfill testing, verification of design parameters, and observation of construction for general compliance with design drawings and specifications.







SECIFICATIONS

PART GENERAL 1.01 De

- A. Work all consist of furnishing and construction Retaining Wall System in accordance Modulà these s cifications and in reasonably close conform with the lines, grades, design, and dimensid shown on the plans.
- B. Work incluins preparing foundation soil, funishing and installing leading pad, unit drainage fill an obackfill to the lines and grades shown on the construction drawings.
- C. Work includes urnishing and installing reogrid soil reinforcement the type, size, locatic, and lengths designated on the construction drawi
- 1.02 Delivery, Stora and Handling
- A. Contractor shall check all material upon delivery to assure that the proper type, grade color, and certification has be received
- B. Contractor shall protect all maturials from damage due to job site condition lance with manufacturer's recommendations. Damaged materials shall not be incorporate wint the work. shall not be incorporat

PART 2: PRODUCTS

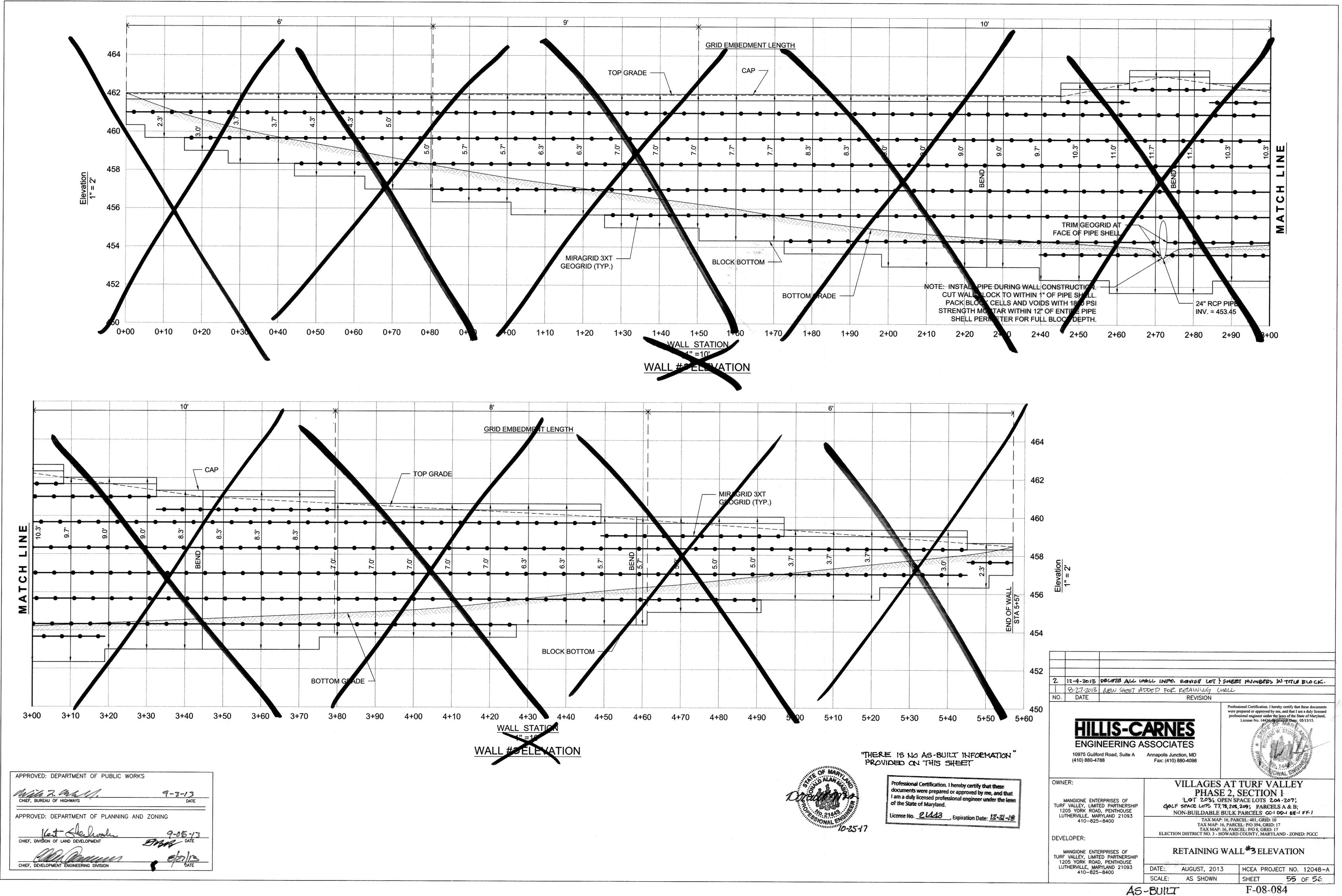
- 2.01 Modular Concrete Retaing Wall Units
- A. Modular concrete units plan conform to the following architectural requirements: the color color may be specified by the Owne
- face finish sculptured rock face in angular tri-planer or flat configuration. Other face finishes will not be allowed without written approva of Owner.
- bond configuration running with bonds nominally located at midpolit vertically adjagent units, in both straight and cur ed alignments.
- exposed surfaces of units shall be use of chips, cracks or other imperiections when viewed from a distance of 10 feet unde diffused lighting.
- B. Modular concrete materials shall continue to the ts of ASTM C1372 - Stand requireme Specifications for Segmental Retaining Vall Units.
- C. Modular concrete units shall conform to be following structural and geometric requirements masured in accordince with appropriate references:
- essive strength = 3000 psi minimum, bsorption maximum (6% in northern states) for sundard nt aggregates;
- ensional tolerances = ±1/8" from nominal u hensions not including rough split face, ±1/16 hit height - top and bottom planes; unit size - 8
- 8" (W) x 12" (D) minimum; unit weight - 75 lbs/unit minimum for standard weigh

ieve Site	Percent Passing
2 in h	100, 5
3/ inch	100-7
) 0 . 40	0-60
No. 200	0-35

APPROVED: DEPARTMENT OF PUBLIC WOR	KS
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APPROVED: DEPARTMENT OF PLANNING AI	9-05-13
CHIEF, DIVISION OF LAND DEVELOPMENT	DATE
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SPECIFICATIONS

MODULAR CONCRETE BLOCK RETAINING WALL

PART 1: GENERAL

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1.01 Description

- A. Work shall consist of furnishing and construction of a Modular Retaining Wall System in accordance with these specifications and in reasonably close conformity with the lines, grades, design, and dimensions shown on the plans.
- B. Work includes preparing foundation soil, furnishing and installing leveling pad, unit drainage fill and backfill to the lines and grades shown on the construction drawings.
- C. Work includes furnishing and installing geogrid soil reinforcement of the type, size, location, and lengths designated on the construction drawings.
- 1.02 Delivery, Storage and Handling
- A. Contractor shall check all materials upon delivery to assure that the proper type, grade, color, and certification has been received.
- B. Contractor shall protect all materials from damage due to job site conditions and in accordance with manufacturer's recommendations. Damaged materials shall not be incorporated into the work.

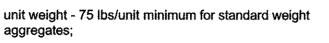
PART 2: PRODUCTS

- 2.01 Modular Concrete Retaining Wall Units
- A. Modular concrete units shall conform to the following architectural requirements: face color - color may be specified by the Owner.
- face finish sculptured rock face in angular tri-planer or flat configuration. Other face finishes will not be allowed without written approval of Owner.
- bond configuration running with bonds nominally located at midpoint vertically adjacent units, in both straight and curved alignments.
- exposed surfaces of units shall be free of chips, cracks or other imperfections when viewed from a distance of 10 feet under diffused lighting.
- B. Modular concrete materials shall conform to the requirements of ASTM C1372 - Standard Specifications for Segmental Retaining Wall Units.
- C. Modular concrete units shall conform to the following structural and geometric requirements measured in accordance with appropriate references:

compressive strength = 3000 psi minimum; absorption = 8% maximum (6% in northern states) for standard weight aggregates;

dimensional tolerances = $\pm 1/8$ " from nominal unit dimensions not including rough split face, ±1/16"

unit height - top and bottom planes; unit size - 8" (H) x 18" (W) x 12" (D) minimum;



- inter-unit shear strength 1000 plf minimum at 2 psi normal pressure; at 2 psi normal force.
- geogrid/unit peak connection strength 1000 plf minimum D. Modular concrete units shall conform to the following constructability requirements: (if applicable)
- vertical setback = 1/8"± per course (near vertical) or 1"+ per course per the design;
- alignment and grid positioning mechanism fiberglass pins, two per unit minimum;

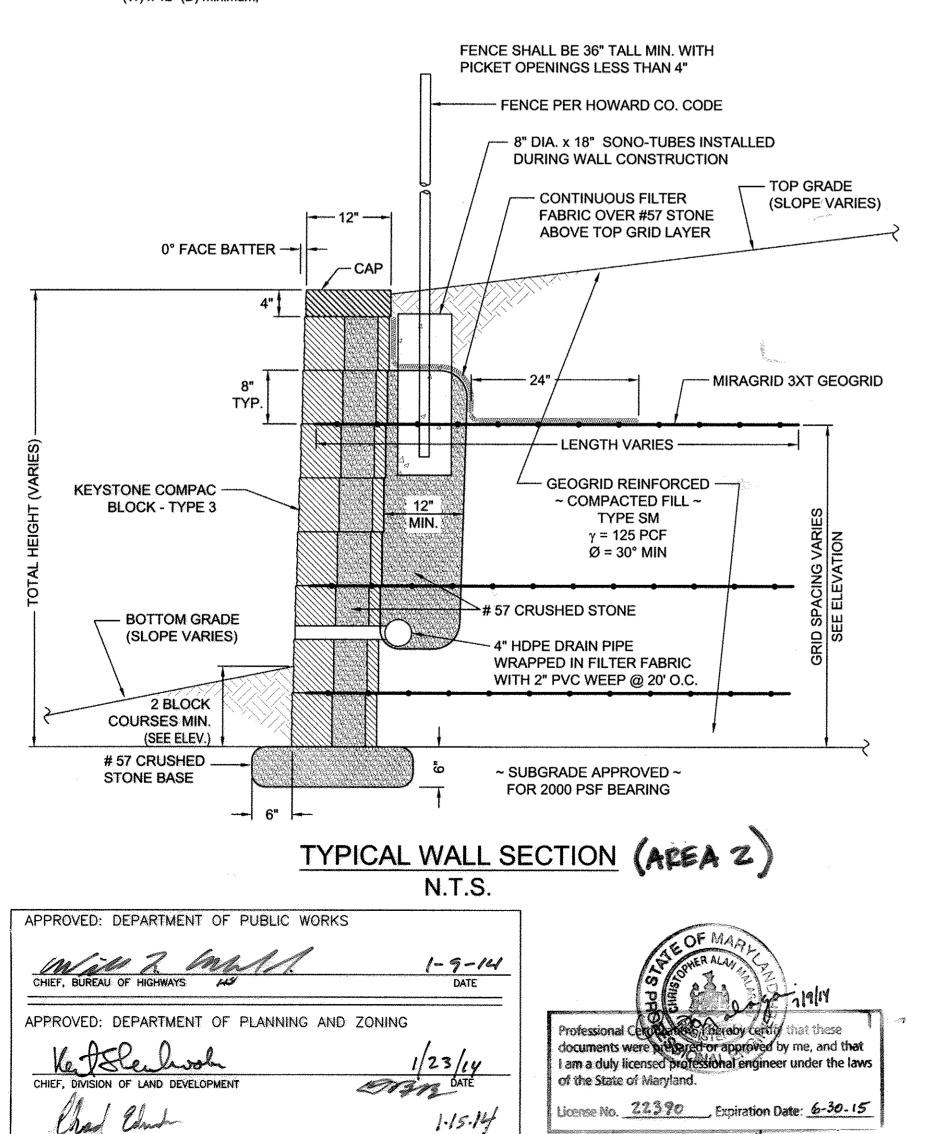
maximum horizontal gap between erected units shall be -1/2 inch.

2.02 Shear Connectors (if applicable)

- A. Shear connectors shall be 1/2 inch diameter thermoset isopthalic polyester resin-protruded fiberglass reinforcement rods or equivalent to provide connection between vertically and horizontally adjacent units. Strength of shear connectors between vertical adjacent units shall be applicable over a design temperature of 10 degrees F to + 100 degrees F. B. Shear connectors shall be capable of holding the geogrid in the proper design position during grid pre-tensioning and backfilling.
- 2.03 Base Leveling Pad Material
- A. Material shall consist of a compacted #57 crushed stone base as shown on the construction drawings.
- 2.04 Unit Drainage Fill
- A. Unit drainage fill shall consist of #57crushed stone
- 2.05 Geogrid Reinforced Backfill
- A. Reinforced backfill shall type SM, be free of debris and meet the following gradation tested in accordance with ASTM D-422 and meet other properties shown on the plan:

Sieve Size	Percent Passing
2 inch	100-75
3/4 inch	100-75
No. 40	0-60
No. 200	0-35

- Plasticity Index (PI) <10 and Liquid Limit <35 per ASTM D-4318.
- B. Material can be site excavated soils where the above requirements can be met. Unsuitable soils for backfill (high plastic clays or organic soils) shall not be used in the reinforced soil mass.



DATE

CHIEF, DEVELOPMENT ENGINEERING DIVISION

2.06 Geogrid Soil Reinforcement

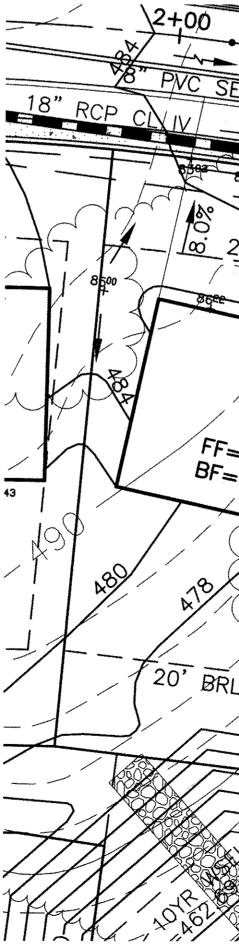
- A. Geosynthetic reinforcement shall consist of geogrids yarn.
- 2.07 Drainage Pipe

PART 3 EXECUTION 3.01 Excavation

prior to placement of leveling material or fill soils.

3.02 Base Leveling Pad

- A. Leveling pad material shall be placed to the lines and in front and behind the modular wall unit.
- B. Leveling pad shall be prepared to insure full contact to the base surface of the concrete units.
- 3.03 Modular Unit Installation
- A. First course of units shall be placed on the leveling pad at the appropriate line and grade. Alignment and level shall be checked in all directions and insure that all units are in full contact with the base and properly seated.
- B. Place the front of units side-by-side. Do not leave gaps between adjacent units. Layout of corners and curves shall be in accordance with manufacturer's recommendations.
- C. Install shear/connecting devices per manufacturer's recommendations.
- D. Place and compact drainage fill within and behind wall units. Place and compact backfill soil behind drainage fill. Follow wall erection and drainage fill closely with structure backfill
- not exceed three courses.
- 3.04 Structural Geogrid Installation
- A. Geogrid shall be oriented with the highest strength axis perpendicular to the wall alignment.
- B. Geogrid reinforcement shall be placed at the strengths, drawings or as directed by the Engineer.



manufactured specifically for soil reinforcement applications and shall be manufactured from high tenacity polyester

A. The drainage pipe shall be perforated corrugated HDPE pipe manufactured in accordance with ASTM D-1248.

A. Contractor shall excavate to the lines and grades shown on the construction drawings. Owner's representative shall be responsible for inspecting and approving the excavation

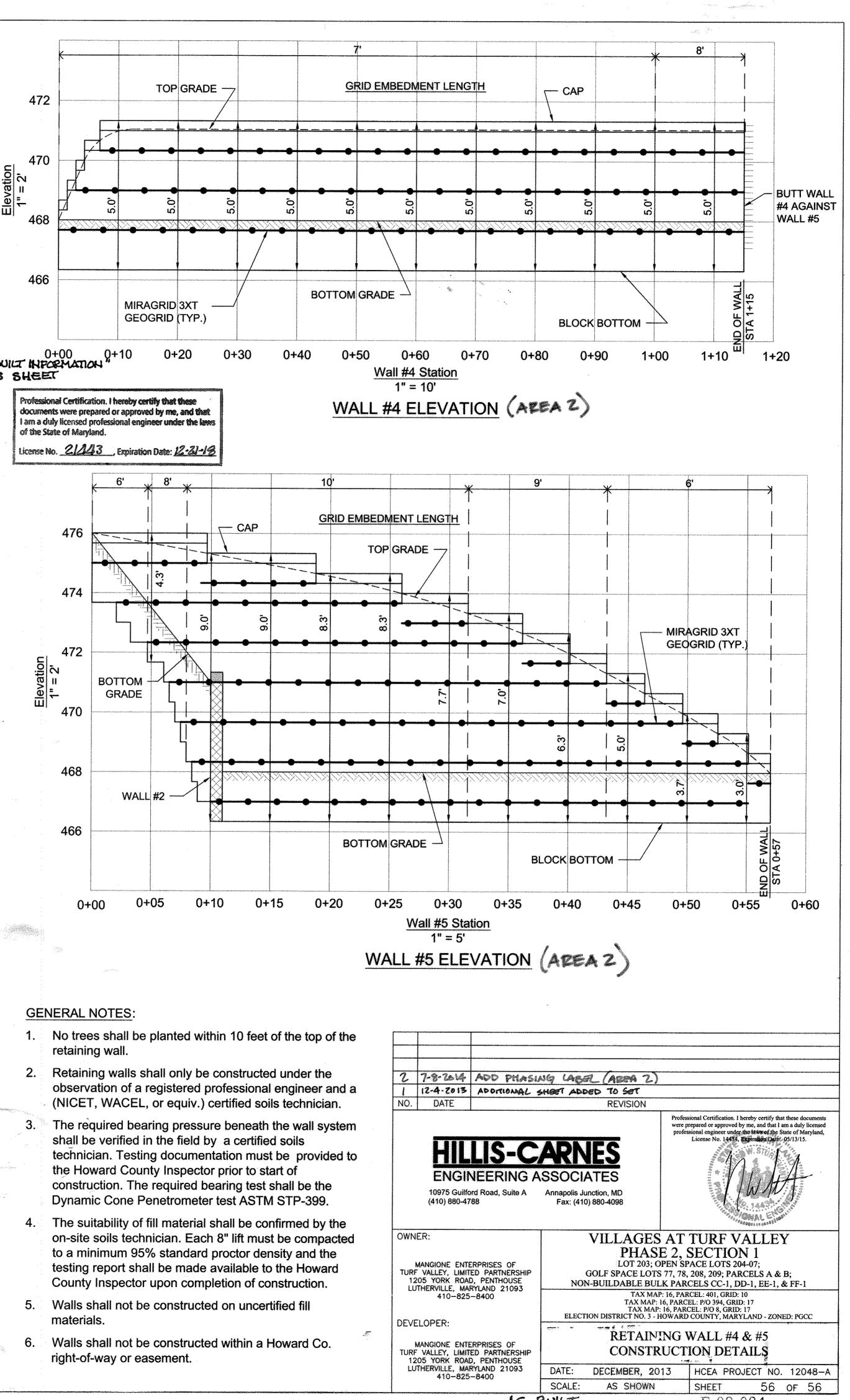
grades shown on the construction drawings, to a minimum thickness of 6 inches and extend laterally a minimum of 6"

- E. Maximum stacked vertical height of wall units, prior to unit drainage fill and backfill placement and compaction, shall
- lengths, and elevations shown on the construction design
- C. The geogrid shall be laid horizontally on compacted backfill and attached to the modular wall units. Place the next

- course of modular concrete units over the geogrid. The geogrid shall be pulled taut, and anchored prior to backfill placement on the geogrid.
- D. Geogrid reinforcements shall be continuous throughout their embedment lengths and placed side-by-side to provide 100% coverage at each level. Spliced connections between shorter pieces of geogrid or gaps between adjacent pieces of geogrid are not permitted.
- 3.05 Reinforced Backfill Placement
- A. Reinforced backfill shall be placed, spread, and compacted in such a manner that minimizes the development of slack in the geogrid and installation damage.
- B. Reinforced backfill shall be placed and compacted in lifts not to exceed 6 inches where hand compaction is used, or 8 - 10 inches where heavy compaction equipment is used. Lift thickness shall be decreased to achieve the required density as required.
- C. Reinforced backfill shall be compacted to 95% of the maximum density as determined by ASTM D698. The moisture content of the backfill material prior to and during compaction shall be uniformly distributed throughout each laver and shall be + 3% to - 3% of optimum.
- D. Only lightweight hand-operated equipment shall be allowed within 3 feet from the tail of the modular concrete unit.
- E. Tracked construction equipment shall not be operated directly upon the geogrid reinforcement. A minimum fill thickness of 6 inches is required prior to operation of tracked vehicles over the geogrid. Tracked vehicle turning should be kept to a minimum to prevent tracks from displacing the fill and damaging the geogrid.
- F. Rubber tired equipment may pass over geogrid reinforcement at slow speeds, less than 10 MPH. Sudden braking and sharp turning shall be avoided.
- G. At the end of each day's operation, the Contractor shall slope the last lift of reinforced backfill away from the wall units to direct runoff away from wall face. The Contractor shall not allow surface runoff from adjacent areas to enter the wall construction site.

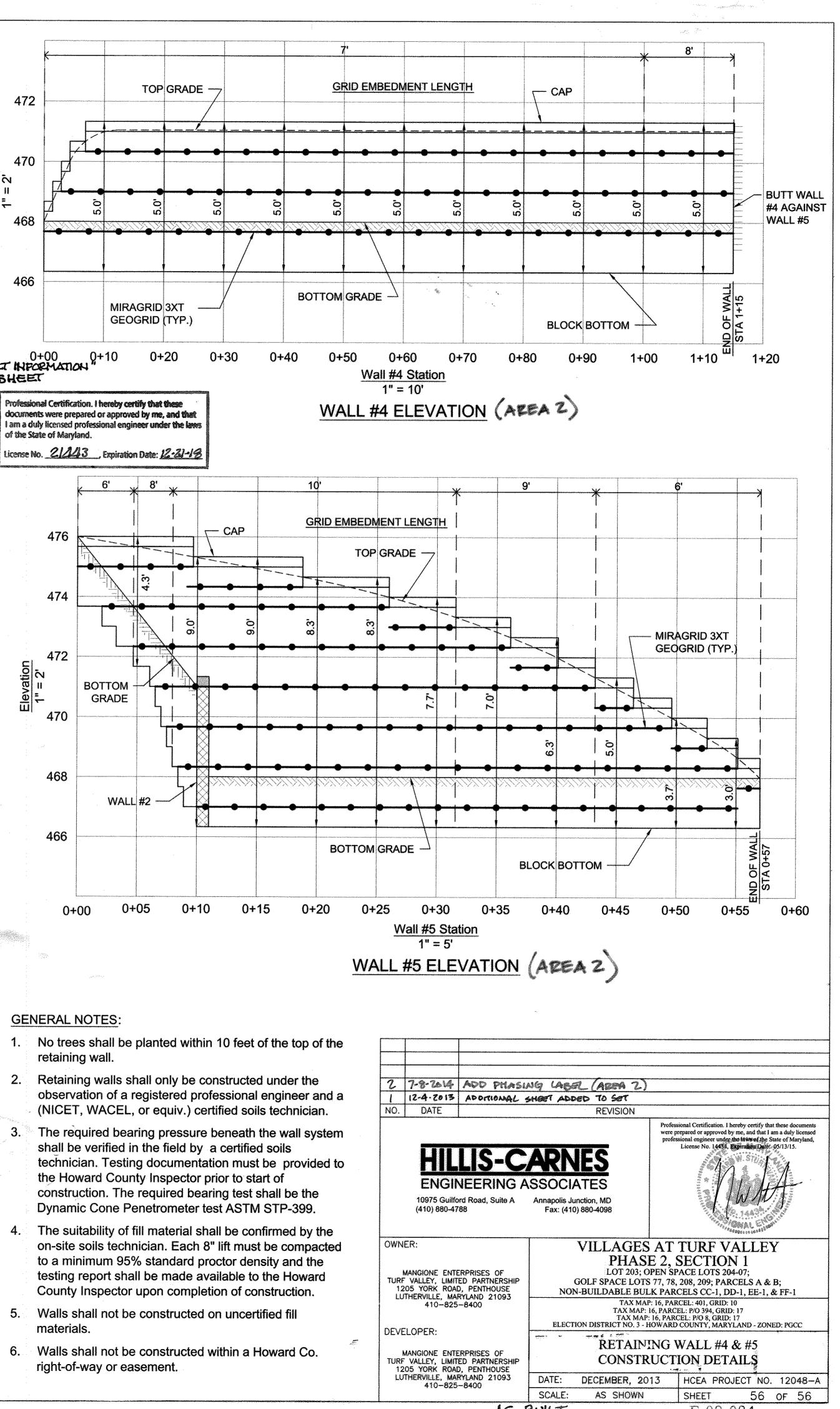
3.06 Cap Installation

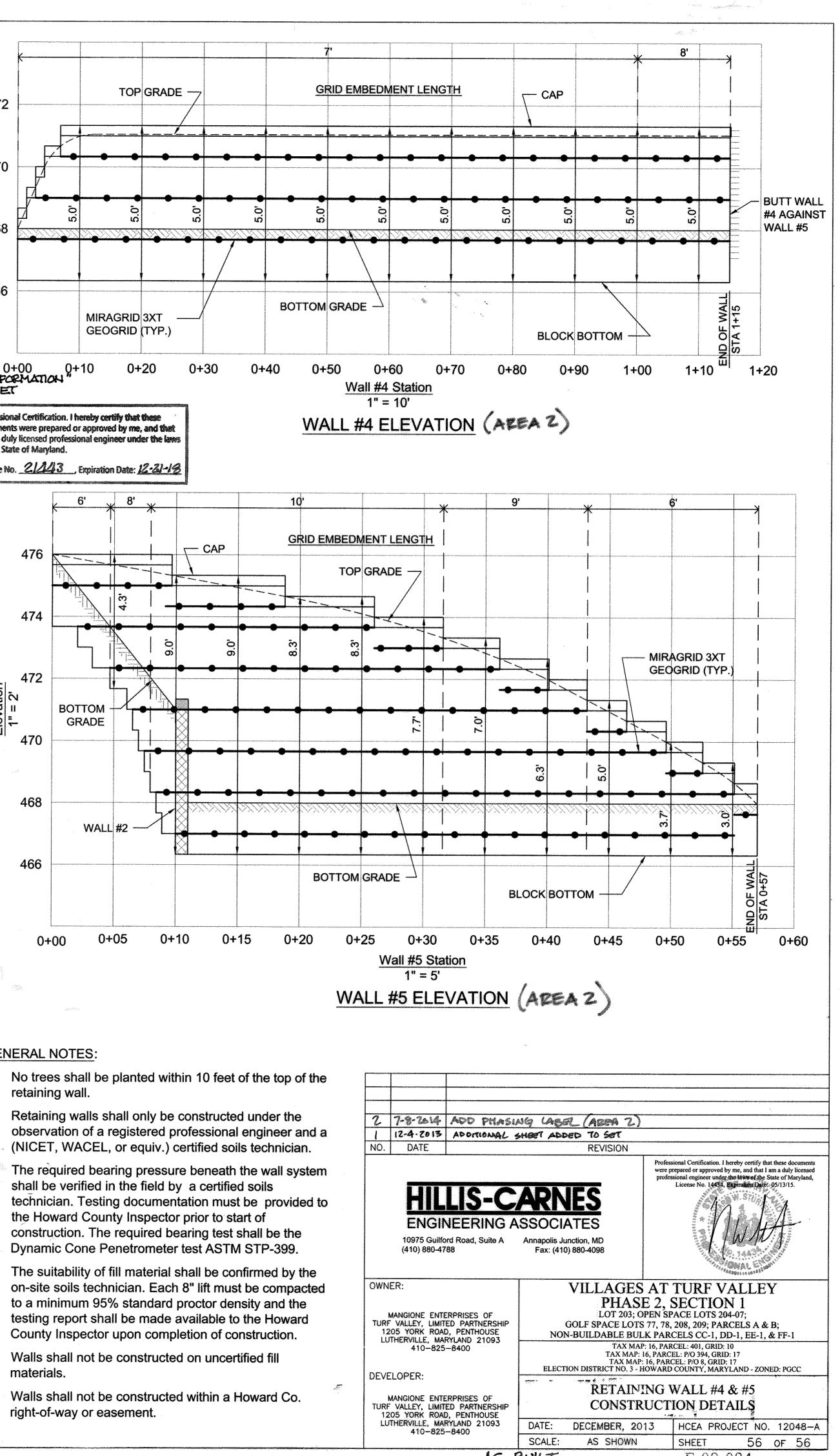
- A. Cap units shall be glued to underlying units with an all-weather adhesive recommended by the manufacturer.
- 3.07 Field Quality Control
- A. The Owner shall engage inspection and testing services, including independent laboratories, to provide quality assurance and testing services during construction.
- B. As a minimum, quality assurance testing should include foundation soil inspection, soil and backfill testing, verification of design parameters, and observation of construction for general compliance with design drawings and specifications.



"THERE IS NO AS-BUILT INFORMATION PROVIDE ON THIS SHEET







1 Alix Ist - AVC WATER ATWOOD SEWER ΔΛ STA. 0+00 STA. 1+15 WALL #5 RIP-RAP OUTLET PROTECTIO 20' 1/BR STA. 0+57 TW=475 TW=474 -BW=468 BW=468 TW=468 4+30.06 BW=468 #STA. 1+00 227 FF=488.22 BF=478.22 WALL# STA. 0+00 WALL #4 & #5 LOCATION PLAN (AREA 2) 1" = 20'

AG. BUILT

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