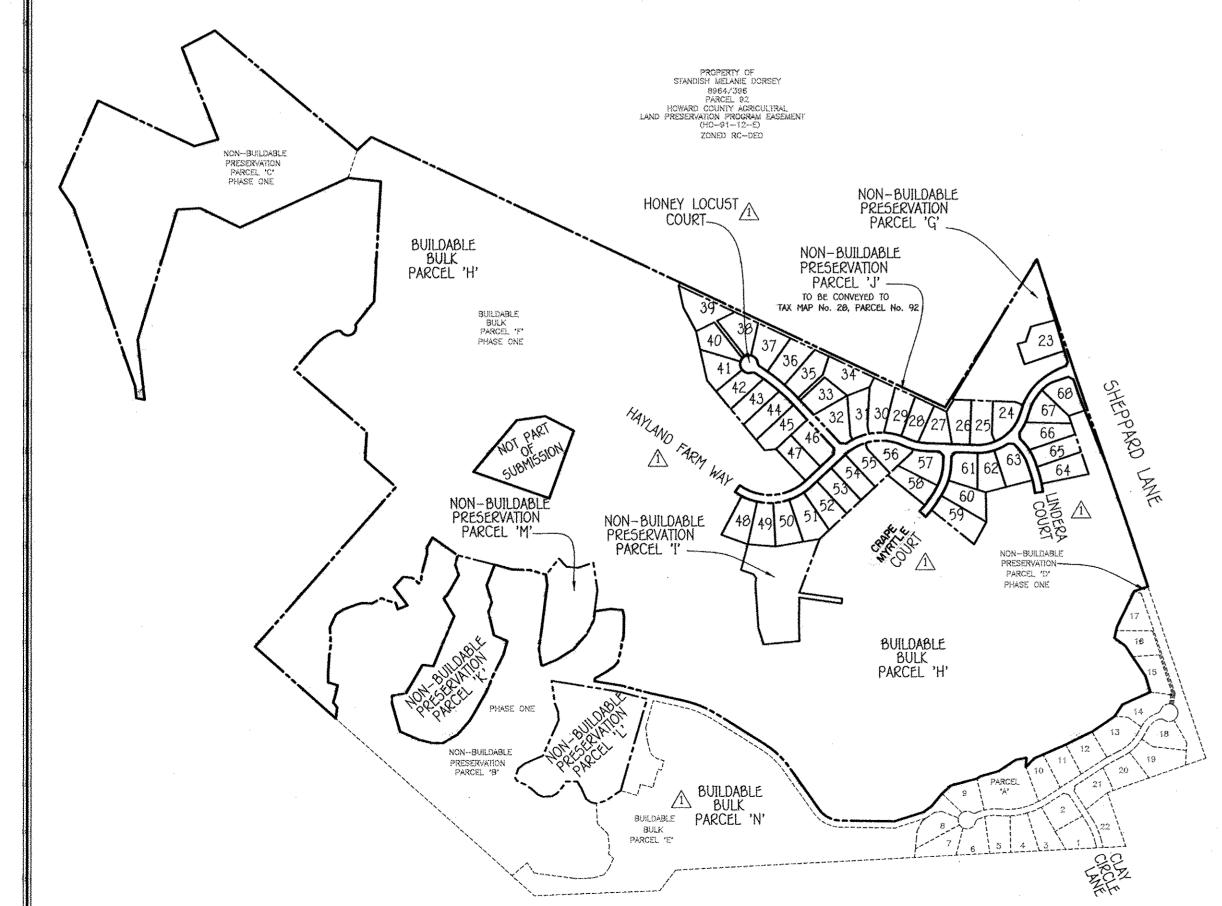
	TRAFFIC CON	VTROL.	SIGN5	
ROAD NAME	CENTERLINE STA.	OFFSET	POSTED SIGN	5IGN CODI
HAYLAND FARM WAY	0+50	44'L	STOP	R1-1
HAYLAND FARM WAY	2+00	18'R	SPEED LIMIT 25	R2-1
HAYLAND FARM WAY	0+48	-4, -44,	KEEP RIGHT	R47
HAYLAND FARM WAY	1+16		KEEP RIGHT	R47
HAYLAND FARM WAY	3+00	14'L	STOP AHEAD	W3~1
HAYLAND FARM WAY	20+20	14'R	ROAD ENDS 500 FT.	
HAYLAND FARM WAY	WAY TEE TURNAROUND SEE SHEET 5		NO PARKING IN TEE TURNAROUND	
LINDERA COURT	TEE TURNAROUND SEE SHEET 7	interpretativi pieneglavi ti dan ministrativi timo d Venya, minis	NO PARKING IN TEE TURNAROUND	
LINDERA COURT	0+25	16'L	STOP	R1-1
LINDERA COURT	1+00	14'R	SPEED LIMIT 25	R2-1
CRAPE MYRTLE CT.	TEE TURNAROUND SEE SHEET 7	Parish Titrick	NO PARKING IN THE TURNAROUND	
CRAPE MYRTLE CT.	0+25	16'L	5TOP	R1-1
CRAPE MYRTLE CT.	1,+00	14'R	SPEED LIMIT 25	R2-1
HONEY LOCUST COURT	0+25	16'L	STOP	R1-1
HONEY LOCUST COURT	1+50	11'8	SPEED LIMIT 25	R2-1

	ROADWAY INFORMATION CHART										
	ROAD NAME	CLASSIFICATION	DESIGN SPEED	R/W WIDTH							
Λ	HAYLAND FARM WAY	PUBLIC ACCESS STREET	30 M.P.H.	50'							
	LINDERA COURT	PUBLIC ACCESS STREET	30 M.P.H.	50'							
	CRAPE MYRTLE COURT	PUBLIC ACCESS STREET	30 M.P.H.	50*							
	HONEY LOCUST COURT	PUBLIC ACCESS PLACE	25 M.P.H.	50'							

STREET LIGHT CHART								
STREET NAME	C.L. STATION	OFF5ET	FIXTURE/POLE TYPE					
SHEPPARD LANE	5+4 <i>8</i>	50° L	150-WATT H.P.S. "PREMIER" PENDANT POST-TOP MOUNTED AT 30' ON A BRONZE FIBERGLASS POLE					

FISHER, COLLINS & CARTER, INC.



FINAL ROAD CONSTRUCTION, GRADING AND STORMWATER MANAGEMENT PLAN

WALNUT CREK

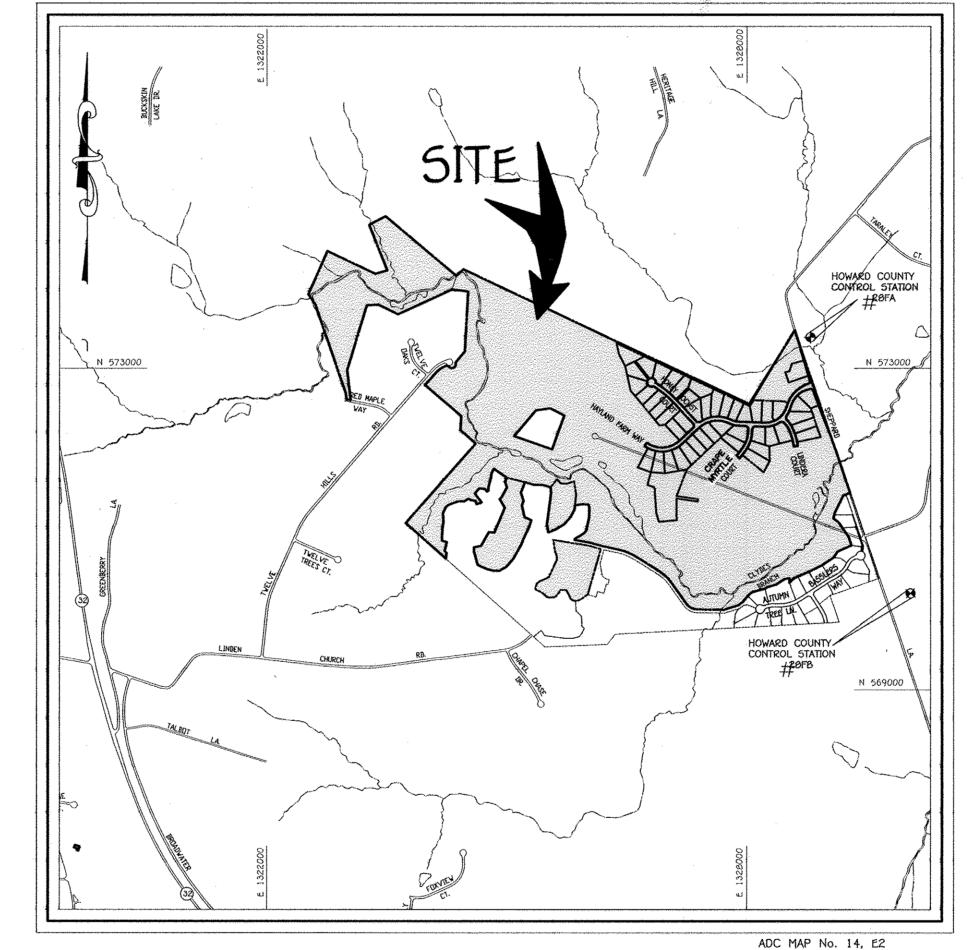
PHASE TWO

Lots 23 - 68, Non-Buildable Preservation Parcels 'C', 'G', 'I', 'J', 'K', 'L' And 'M' & Buildable Bulk Parcels 'H' And 'N'

(Being A Resubdivision Of Buildable Bulk Parcels 'F' & 'E' And a Revision To Non-Buildable Preservation Parcel 'C' - Walnut Creek, Phase One, Plat No's. 20631 Thru 20647

ZONING: RC-DEO & RR-DEO

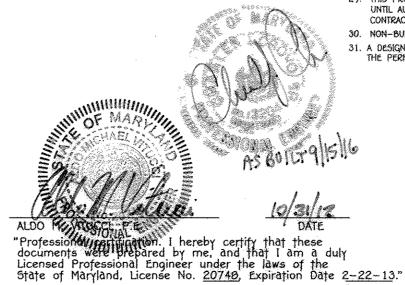
TAX MAP NO. 28 GRID Nos. 4, 5, 10-12, 17 AND 18 PARCEL No. 49



VICINITY MAP SCALE: 1" = 1200"

FIFTH ELECTION DISTRICT HOWARD COUNTY, MARYLAND

I HEREBY CERTIFY, BY MY SEAL, THAT THE FACILITIES SHOWN ON THIS PLAN WERE CONSTRUCTED AS SHOWN ON THIS "AS-BUILT" PLAN MEET THE APPROVED PLANS AND SECUPICATIONS. CHARUSS J. CROND, GR. PE NO. 13204 AS-BUILT 9/15/10



Diase Schwer, Ading CHIEF, BUREAU OF HIGHWAYS APPROVED: DEPARTMENT OF PLANNING AND ZONING

1. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST STANDARDS AND SPECIFICATIONS OF HOWARD COUNTY PLUS MSHA STANDARDS AND 2. THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS /

3. THE CONTRACTOR SHALL NOTIFY "MISS UTILITY" AT 1-800-257-7777 AT LEAST 48

BUREAU OF ENGINEERING / CONSTRUCTION INSPECTION DIVISION AT (410) 313-

ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCO), ALL STREET AND REGULATORY SIGNS SHALL BE IN

PROJECTED BY HOWARD COUNTY GEODETIC CONTROL STATIONS NO. 20 FA AND NO. 28 FB HOWARD COUNTY MONUMENT NO. 28FA V N 572,456.665 E 1,320,957.66 ELEV. = 348.296

D. ZONING: RC-DEO & RR-DEO E. ELECTION DISTRICT: FIFTH F. TOTAL TRACT AREA: 361.259 AC.+ G. NO. OF BUILDABLE LOTS: 46 (PHASE TWO)

H. NO. OF OPEN SPACE LOTS: 0 I. NO. OF NON-BUILDABLE PRESERVATION PARCELS: J. NO. OF BUILDABLE BULK PARCELS: 2 L. AREA OF OPEN SPACE LOTS: 0.00 AC. *
M. AREA OF NON-BUILDABLE PRESERVATION PARCELS: 67.179 AC. *

. TOTAL AREA OF ROADWAY TO BE DEDICATED: 5.252 AC.+

P. PREVIOUS FILE NOS.: SP-06-007 APPROVAL DATE: 5/31/06, BA-05-52E, BA-90-33E, BA-93-49E & WP-00-007 (SEE NOTE 10 BELOW), F-07-076

"CLARK FAMILY CEMETERY" - HO. CO. ID #20-2. THE PLANNING BOARD APPROVED THE CEMETERY ACCOMMODATION AND BOUNDARY DOCUMENTATION PLAN ON MARCH 30, 2006 SUBJECT TO THE FOLLOWING CONDITIONS. 2. THE DEVELOPER AND/OR THE WALNUT CREEK H.O.A. SHALL REGULARLY MAINTAIN THE CEMETERY AREA.

9. ALL FILL AREAS WITHIN ROADWAYS AND UNDER STRUCTURES SHALL BE COMPACTED TO A MINIMUM OF 95% COMPACTION OF AASHTO T-180.

10. THE FOREST CONSERVATION REQUIREMENTS PER SECTION 16.1200 OF THE HOWARD COUNTY CODE AND THE FOREST CONSERVATION MANUAL FOR THE ENTIRE SUBDIVISION WILL BE FULFILLED FOREST AFFORESTATION FOR A TOTAL OF 91.23 ACRES. A SURETY FOR ONSITE FOREST RETENTION @ \$0.20/5F FOR 2,594,869 SF. = \$518,974.00 AND ON-SITE AFFORESTATION @ \$0.50/5F FOR 1,379,110 SF. = \$689,555.00 IS REQUIRED. TOTAL SURETY AMOUNT FOR THE ENTIRE SUBDIVISION = \$1,208,529.00 THE FOREST CONSERVATION PROVIDED WITH PHASE TWO ARE AS FOLLOWS:

18.00 AC. OF FOREST CONSERVATION EASEMENT (CREDITED AND NON-CREDITED REFORESTATION. CALCULATION USED FOR PHASE TWO FOREST REQUIREMENT: 59.57 TOTAL RETENTION ACRES/160 TOTAL UNITS = 0.3723 (46 UNITS X 0.3723 = 17.13 AC.*)
31.66 TOTAL PLANTING ACRES/160 TOTAL UNITS = 0.1979 (46 UNITS X 0.1979 = 9.10 AC.*)
A SURETY FOR ONSITE FOREST RETENTION @ \$0.20/5F FOR 746,103 5F. = \$149,237.00 AND ON-SITE AFFORESTATION @ \$0.50/SF FOR 396,396 SF. = \$198,198.00 IS REQUIRED. TOTAL SURETY AMOUNT FOR THIS SUBMISSION = \$347,435.00.

THE FOREST CONSERVATION SURETY IN THE AMOUNT OF \$347,435.00 IS BE PAID AS PART OF THE

1. THE WAIVER PETITION APPROVAL APPLIES ONLY TO THE TEMPORARY DEFERRAL FOR ESTABLISHING THE FOREST CONSERVATION EASEMENTS FOR THIS SUBDIVISION BASED ON THE APFO PHASING SCHEDULE FOR THIS PROJECT. EACH SUBSEQUENT PHASE OF DEVELOPMENT MUST ESTABLISH A PROPORTIONATE AREA OF FOREST CONSERVATION EASEMENTS AND PROVIDE THE NECESSARY AREA OF FOREST RETENTION AND AFFORESTATION PLANTING AS REQUIRED BY THE FOREST CONSERVATION WORKSHEET FOR THIS PROJECT TO SATISFY ITS OBLIGATION. THE ENTIRE AREA OF FOREST CONSERVATION OBLIGATION MUST BE PROVIDED WITH THE

PROCESSING AND RECORDING OF THE LAST PHASE OF DEVELOPMENT FOR THIS PROJECT. 2. THE APPLICANT/DEVELOPER MUST CONTINUE PROCESSING THE SUBDIVISION PLANS FOR WALNUT CREEK AND MEET ALL APPLICABLE

11. STORMWATER MANAGEMENT FACILITIES: B.M.P. NO. 3 & B.M.P. NO. 4 JOINTLY MAINTAINED BY THE HOMEOWNER'S ASSOCIATION AND HOWARD COUNTY, MARYLAND WET EXTENDED DETENTION FACILITIES (P-3) FOR WOV & CPV

> STORMWATER MANAGEMENT WILL BE PROVIDED IN ACCORDANCE WITH HOWARD COUNTY AND MARYLAND 378 SPECIFICATIONS. RECHARGE VOLUME WILL BE PROVIDED THROUGH THE USE OF GRASS CHANNELS ALONG THE PROPOSED ROADWAYS. WATER QUALITY AND CHANNEL PROTECTION VOLUME WILL BE PROVIDED BY A MICRO-POOL (EXTENDED DETENTION) POND, ONE BIO-RETENTION FACILITY AND LEVEL SPREADERS. VOLUME ARE NOT REQUIRED FOR THIS SITE.

12. THE PROPOSED WATER AND SEWER SYSTEMS SHALL BE PRIVATE. SEE CONTRACT No. 50-4440-D FOR LOW PRESSURE SYSTEM. 13. THE SUBJECT PROPERTY IS LOCATED OUTSIDE OF THE METROPOLITAN DISTRICT.

14. TOPOGRAPHIC CONTOURS BASED ON HARFORD AERIAL SURVEYS, INC.

15. FOR FLAG OR PIPESTEM LOTS, REFUSE COLLECTION, SNOW REMOVAL AND ROAD MAINTENANCE IS TO BE PROVIDED AT THE JUNCTION OF THE FLAG OR PIPESTEM

16. THE GEOTECHNICAL REPORT FOR THIS PROJECT WAS PREPARED BY HERBST BENSON &

17. THE FOREST STAND DELINEATION AND WETLAND DELINEATION FOR THIS PROJECT WAS PREPARED BY ECO-SCIENCE PROFESSIONALS, INC., DATED SEPTEMBER, 2005 AND APPROVED ON MAY 31, 2006.

CARTER, INC. DATED JULY, 2005, AND SUPPLEMENTED WITH INFORMATION OBTAINED FROM HO, CO. CAPITAL PROJECT D-1028A. THE FLOODPLAIN STUDY WAS APPROVED UNDER SP-06-007 DATED 5/31/06. 19. SOILS INFORMATION TAKEN FROM SOIL MAP NO. 18, SOIL SURVEY

HOWARD COUNTY, MARYLAND, JULY 1968 ISSUE.

20. THERE ARE STEEP SLOPES LOCATED ON THIS PROPERTY AS DEFINED BY "SLOPES THAT AVERAGE 25% OR GREATER OVER 10 VERTICAL FEET", PER SECTION 16.108(b)(55) OF THE HOWARD COUNTY SUBDIVISION AND LAND DEVELOPMENT REGULATIONS.

TOTAL AREA OF 25% OR GREATER SLOPES = 8.9 AC.+ 21. AS PER SECTION 104.F.4.b OF THE ZONING REGULATIONS, ONLY ONE EASEMENT HOLDER IS REQUIRED FOR PRESERVATION PARCELS DESIGNED SOLELY FOR SWM FACILITIES OR COMMUNITY SEWERAGE DISPOSAL SYSTEMS.

A. NON-BUILDABLE PRESERVATION PARCEL 'G' B. NON-BUILDABLE PRESERVATION PARCEL 'I' OWNED: HOMEOWNER'S ASSOCIATION EASEMENT HOLDER: HOWARD COUNTY, MARYLAND & HOA EASEMENT HOLDERS: HOWARD COUNTY, MARYLAND

22. NO CLEARING, GRADING OR CONSTRUCTION IS PERMITTED WITHIN THE WETLANDS, STREAM OR THEIR REQUIRED BUFFERS

23. THE LANDSCAPE SURETY FOR THE 80 SHADE AND 100 EVERGREEN TREES IN THE AMOUNT OF \$42,600.00 FOR PERIMETER LANDSCAPE REQUIREMENTS OF SECTION 16.124 OF THE HOWARD COUNTY CODE AND LANDSCAPE MANUAL WILL BE POSTED WITH THE DEVELOPER'S AGREEMENT FOR THIS SUBDIVISION. FINANCIAL SURETY FOR THE REQUIRED 230 STREET TREES WILL BE POSTED AS

PART OF THE DEVELOPER'S AGREEMENT IN THE AMOUNT OF \$69,000.00 24. BUILDABLE BULK PARCEL 'H' RETAINS THE RIGHT TO BE FURTHER SUBDIMDED IN ACCORDANCE WITH THE DEO CLUSTER REGULATIONS IN SECTION 106 OF THE HOWARD COUNTY

ZONING REGULATIONS. THE RESUBDIMISION OF THIS BULK PARCEL INTO RESIDENTIAL LOTS WILL REQUIRE DENSITY FROM AN OFF-SITE LOCATION WITHIN THE RC-DEO DISTRICT. 25. THIS SUBDIVISION PLAN IS SUBJECT TO THE AMENDED FIFTH EDITION OF THE SUBDIVISION AND

DEVELOPMENT REGULATIONS AND THE 2004 ZONING REGULATIONS PER COUNCIL BILL NO. 45-2003 AND DEVELOPMENT OR CONSTRUCTION ON THESE LOTS OR PARCELS MUST COMPLY WITH SETBACK AND BUFFER REGULATIONS IN EFFECT AT THE

TIME OF SUBMISSION OF A BUILDING OR GRADING PERMIT APPLICATION. 26. WELLS SHALL BE ORILLED ON LOTS PRIOR TO RECORDATION OF THE FINAL RECORD PLAT. 27. LOTS 23 THRU 68 ARE TO BE SERVED BY A PUBLIC SHARED SEPTIC FACILITY

LOCATED ON NON-BUILDABLE PRESERVATION PARCEL '8' (F-07-076), WALNUT CREEK, PHASE ONE 28. SIGN POSTS: ALL SIGN POST USED FOR TRAFFIC CONTROL SIGNS INSTALLED IN THE COUNTY RIGHT-OF-WAY SHALL BE MOUNTED ON A 2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE POST (14 GAUGE) INSERTED INTO A 2-1/2" GALVANIZED STEEL, PERFORATED, SQUARE TUBE SLEEVE (12 GAUGE) - 3' LONG, A GALVANIZED STEEL POLE CAP SHALL BE MOUNTED ON TOP OF EACH POST;

29. THIS PROJECT IS SUBJECT TO WASTEWATER DISCHARGE PERMIT NUMBER 05-DP-3530 AND IS EFFECTIVE FROM AUGUST 1, 2006 UNTIL AUGUST 1, 2011 WHEN IT EXPIRES. THE SHARED SEPTIC SYSTEM CONTRACT No. IS 50-4441-0 AND THE COLLECTION SYSTEM

31. A DESIGN MANUAL WAIVER FROM SECTION 5.2.7.A.4., WHICH REQUIRES THAT A WET POND SHALL HAVE A POND DRAIN CAPABLE OF DEWATERING

WALNUT CREEK

Lots 23 - 60, Non-Buildable Preservation Parcels 'C', 'G', 'I', 'J', 'K', 'L' And 'M' & Buildable Bulk Parcels 'H' And 'N' (Being A Resubdivision Of Buildable Bulk Parcels 'F' & 'E' And a Revision To Non-Buildable Preservation Parcel 'C' - Walnut Creek, Phase One, Plat No's. 20631 Thru 20647

ZONED: RC-DEO & RR-DEO TAX MAP No. 20 GRID Nos. 4, 5, 10-12, 17, AND 10 PARCEL No. 49 FIFTH ELECTION DISTRICT HOWARD COUNTY, MARYLAND DATE: SEPTEMBER, 2008

NO SCALE

AG-BUILT REVISED PARCELS 'E' 'J', 'H' & 'G', ROAD NAMES AND 9/25/12 TITLE BLOCK DATE

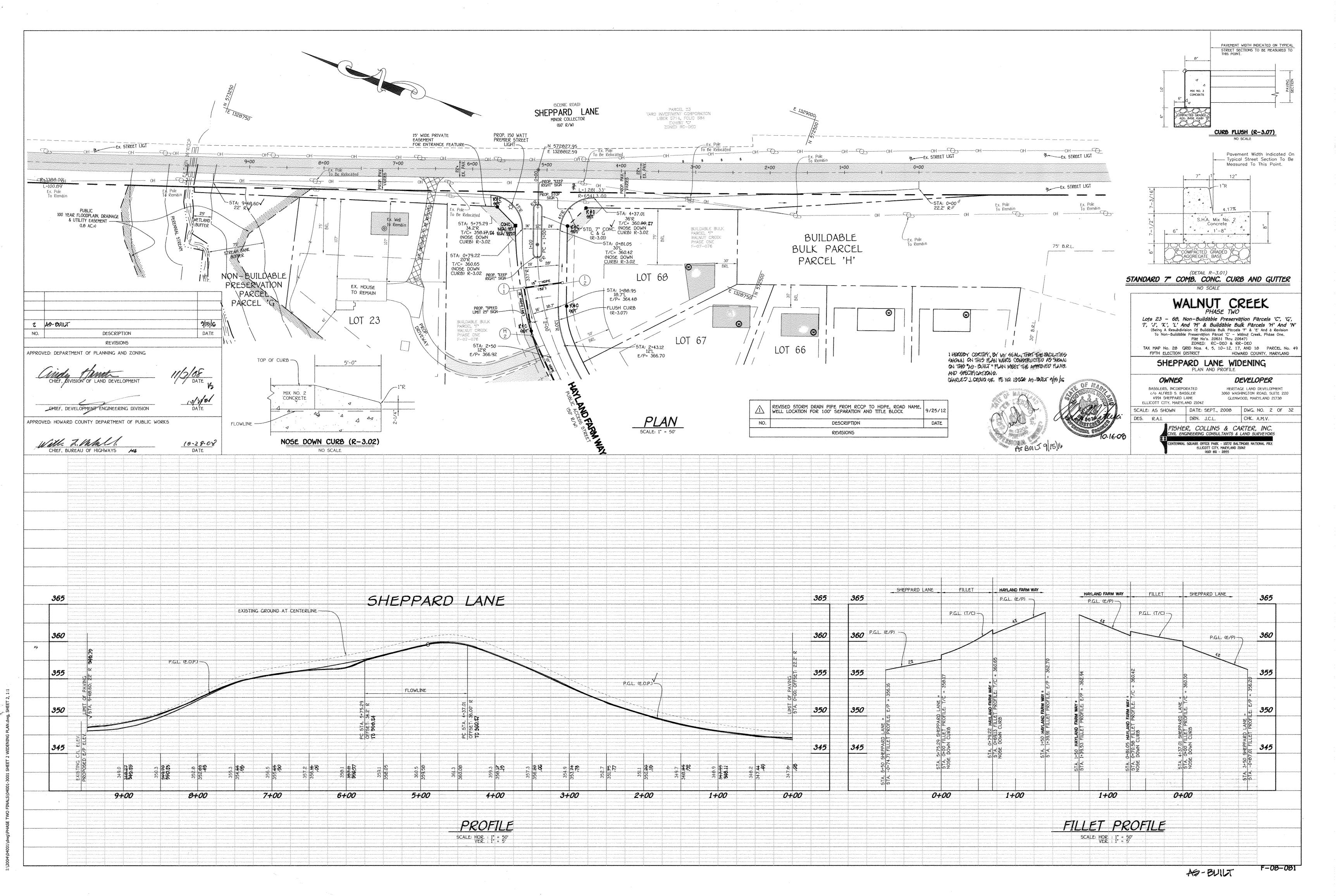
OWNER BASSLERS, INCORPORATED c/o ALFRED 5. BASSLER 4994 SHEPPARD LANE ELLICOTT CITY, MARYLAND 21042

15950 NORTH AVE. LISBON, MARYLAND 21765 (410) 489-7900

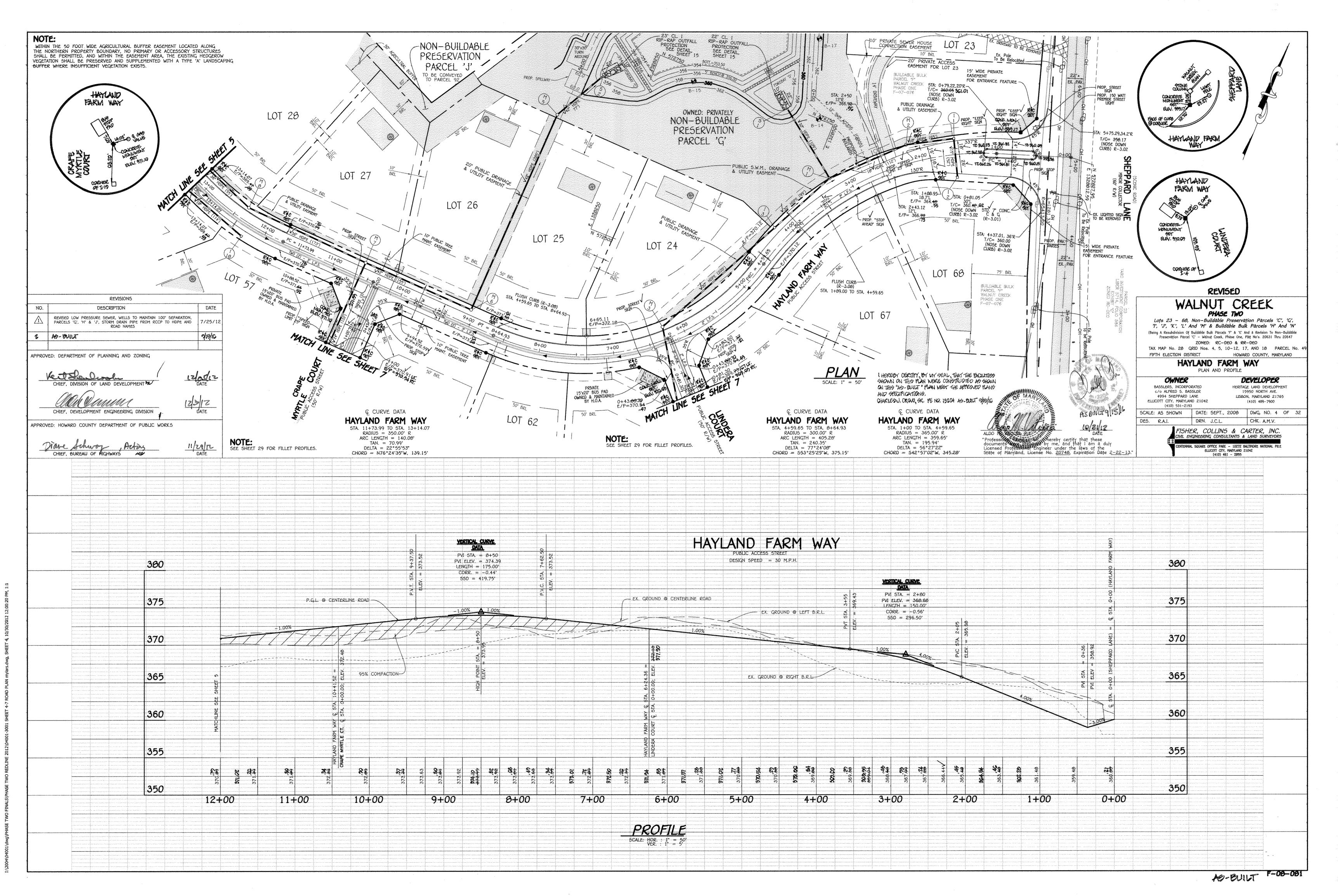
DEVELOPER

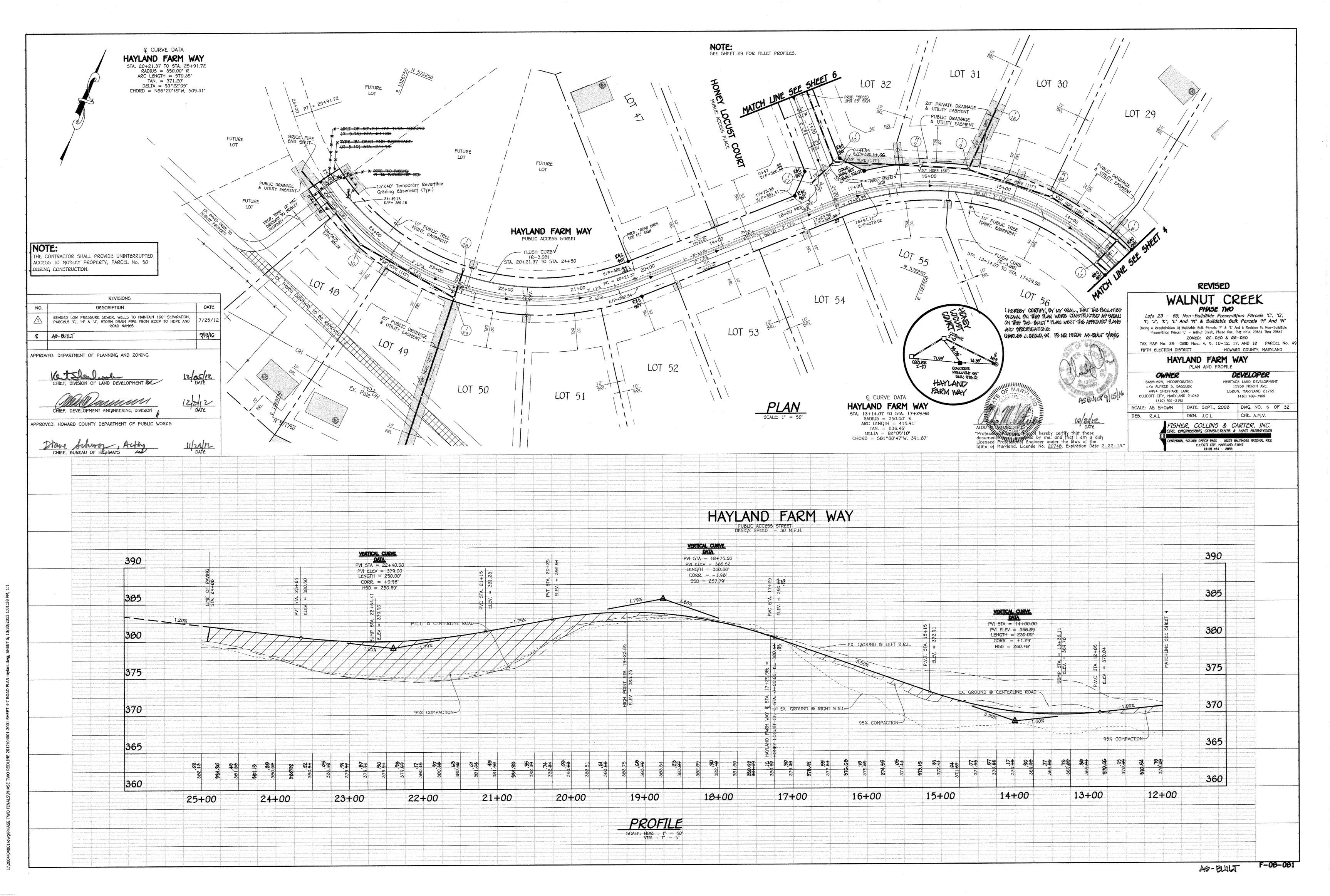
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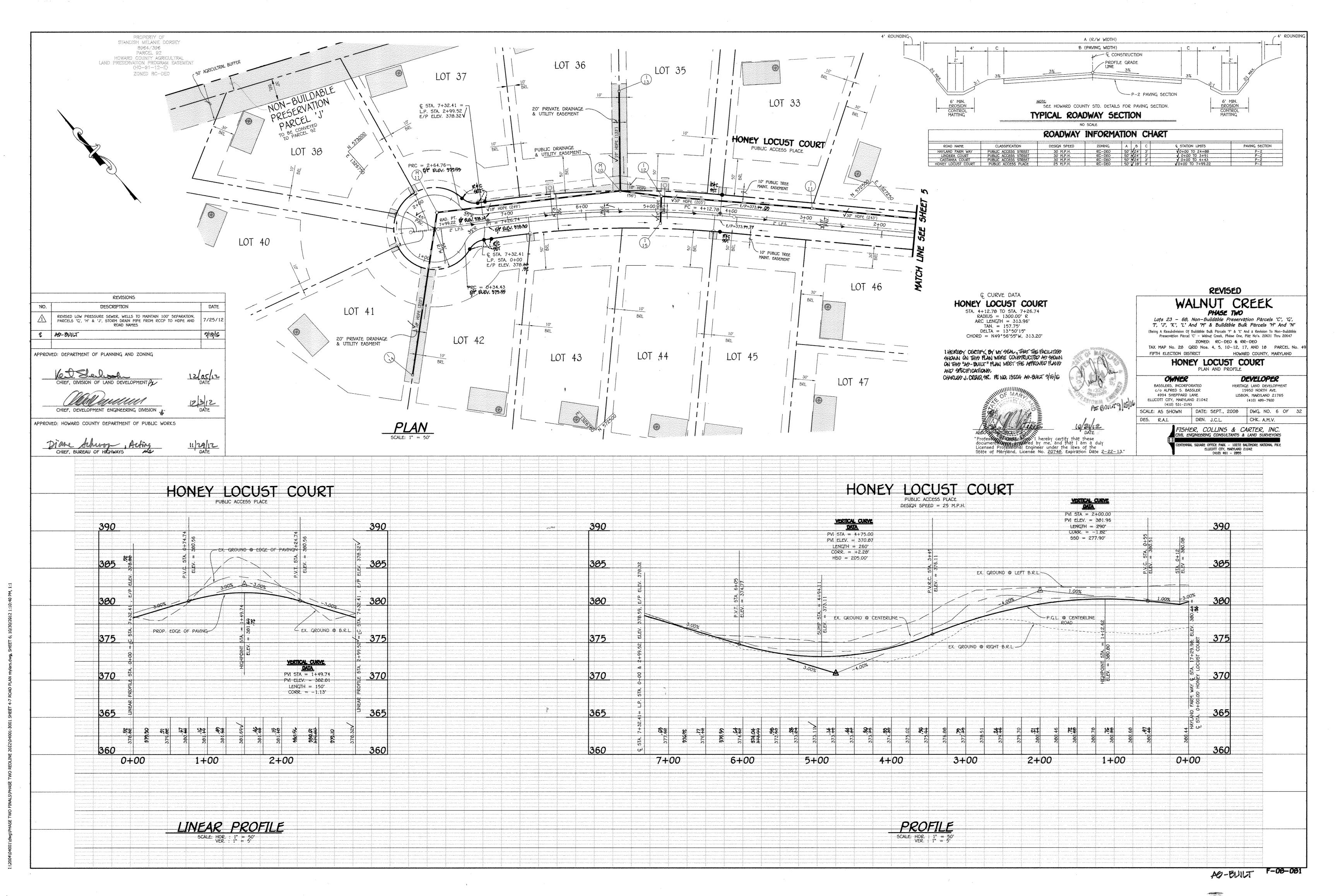
5HEET 1 OF 32

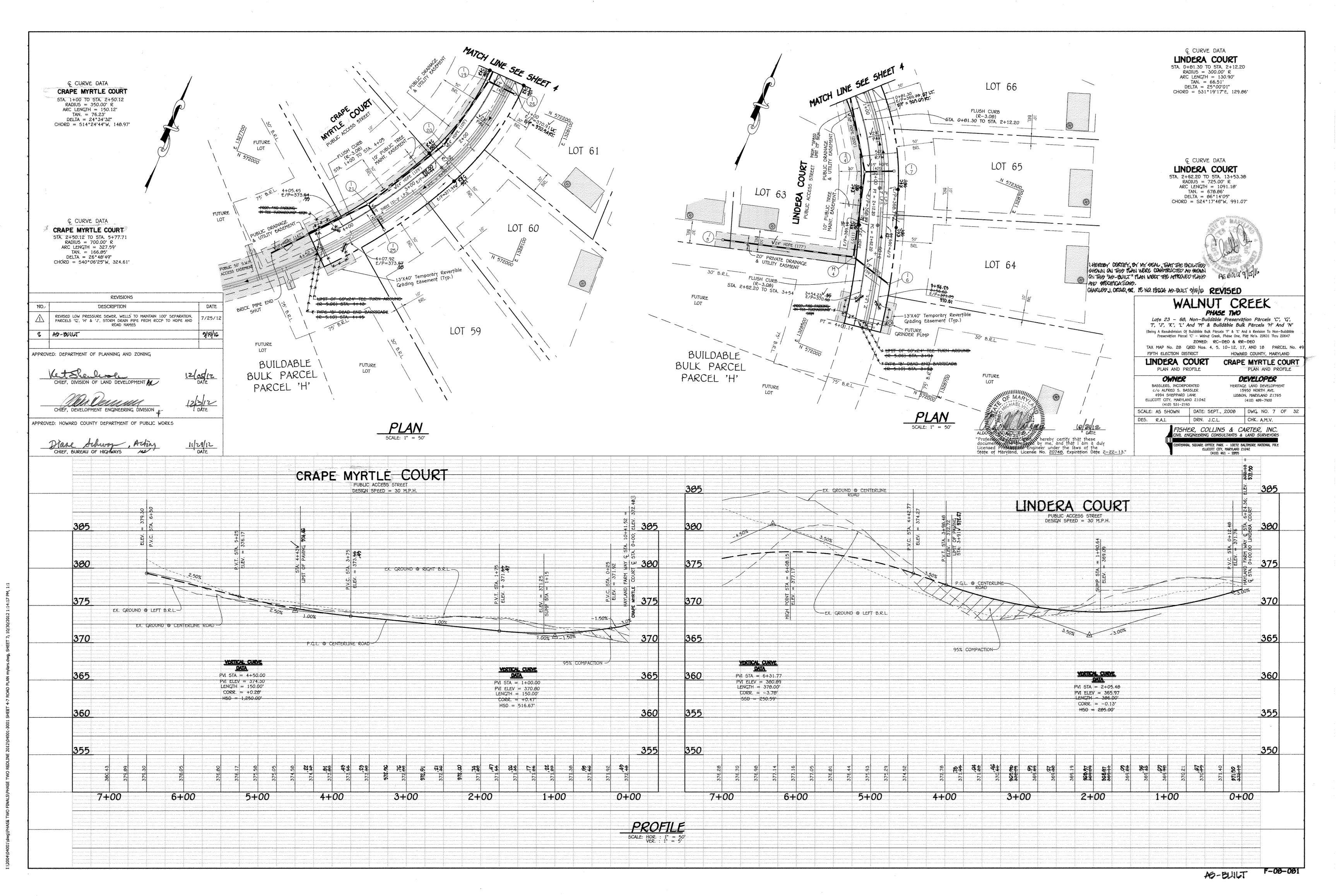


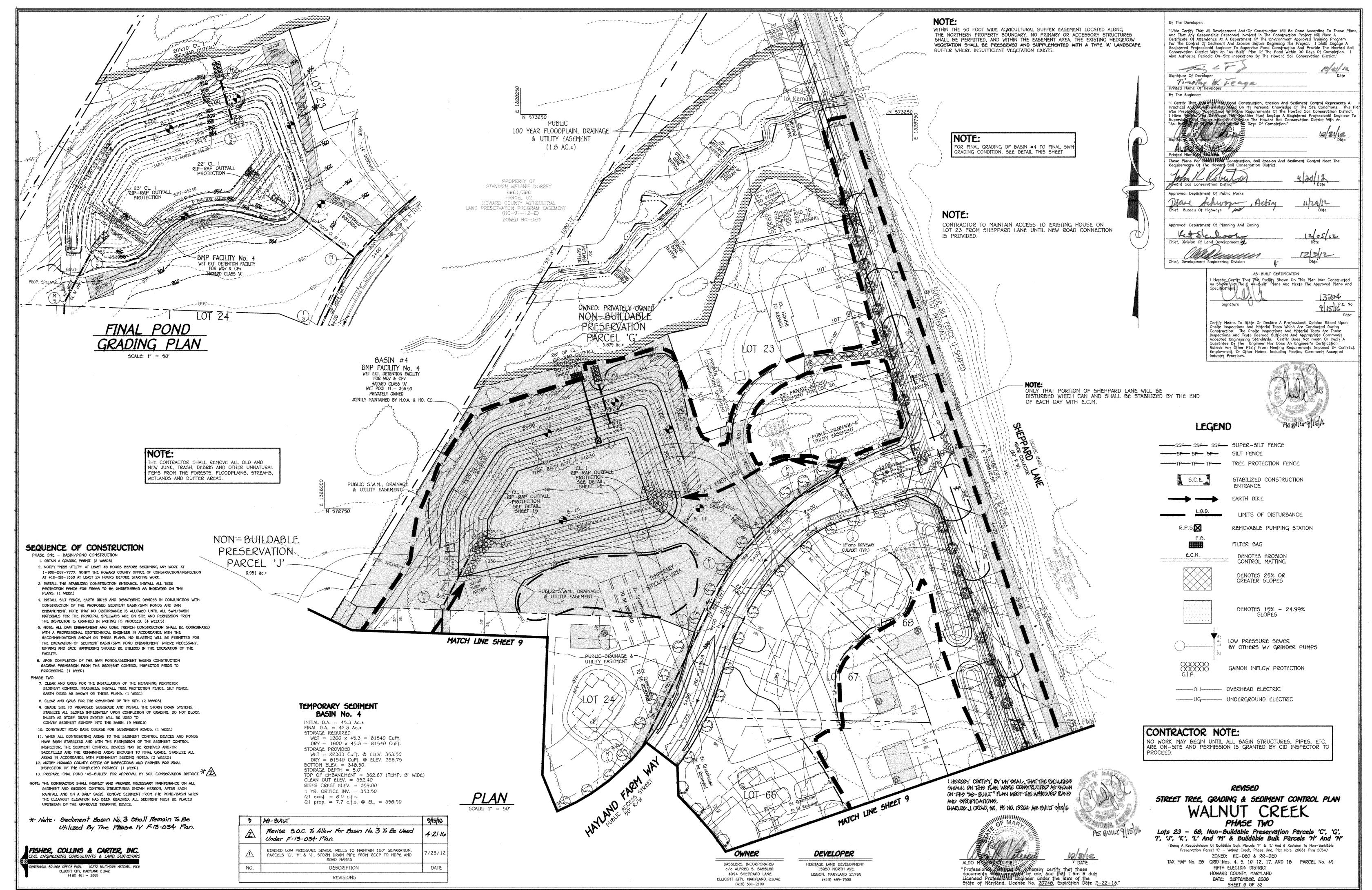
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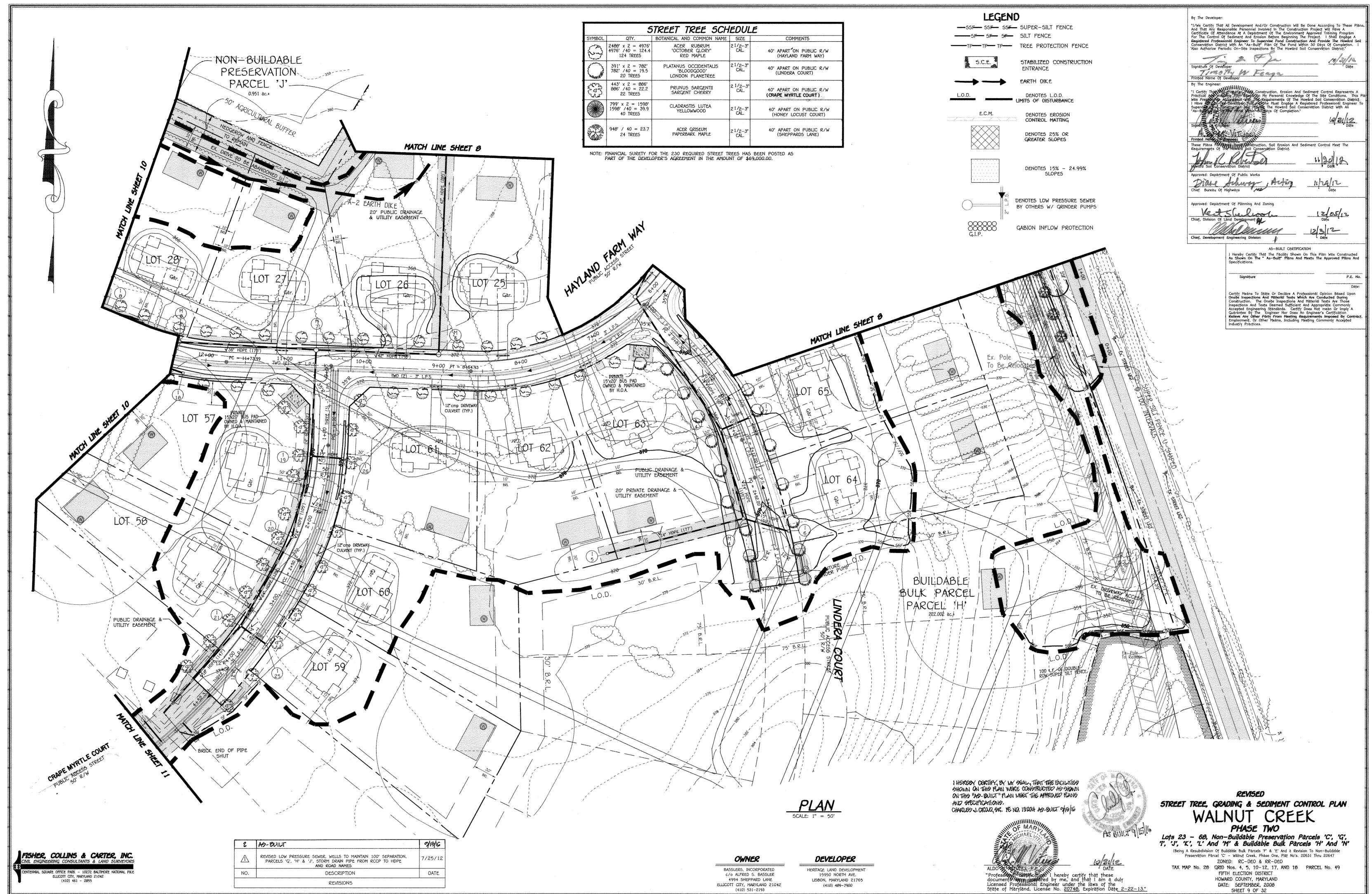


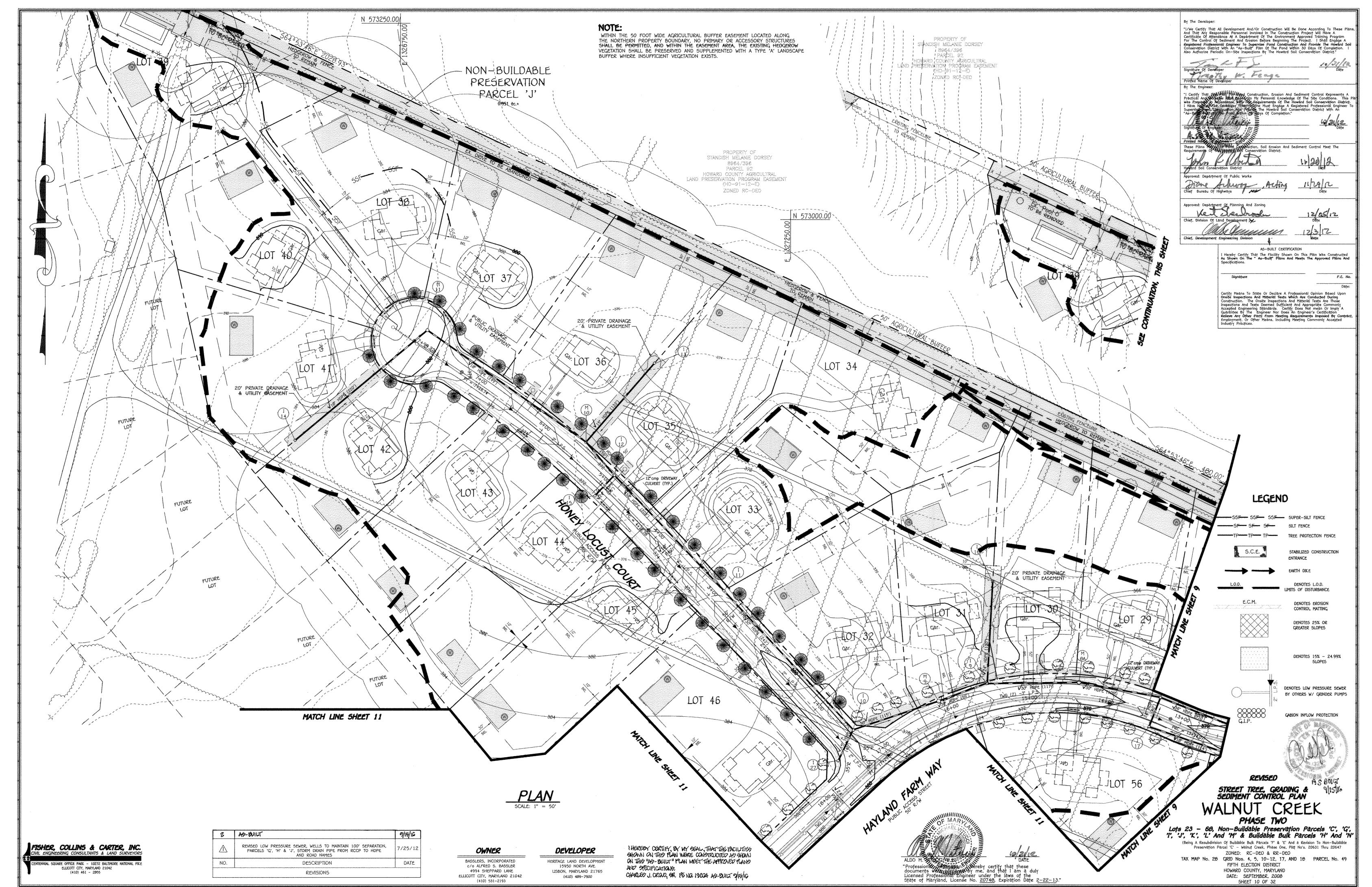




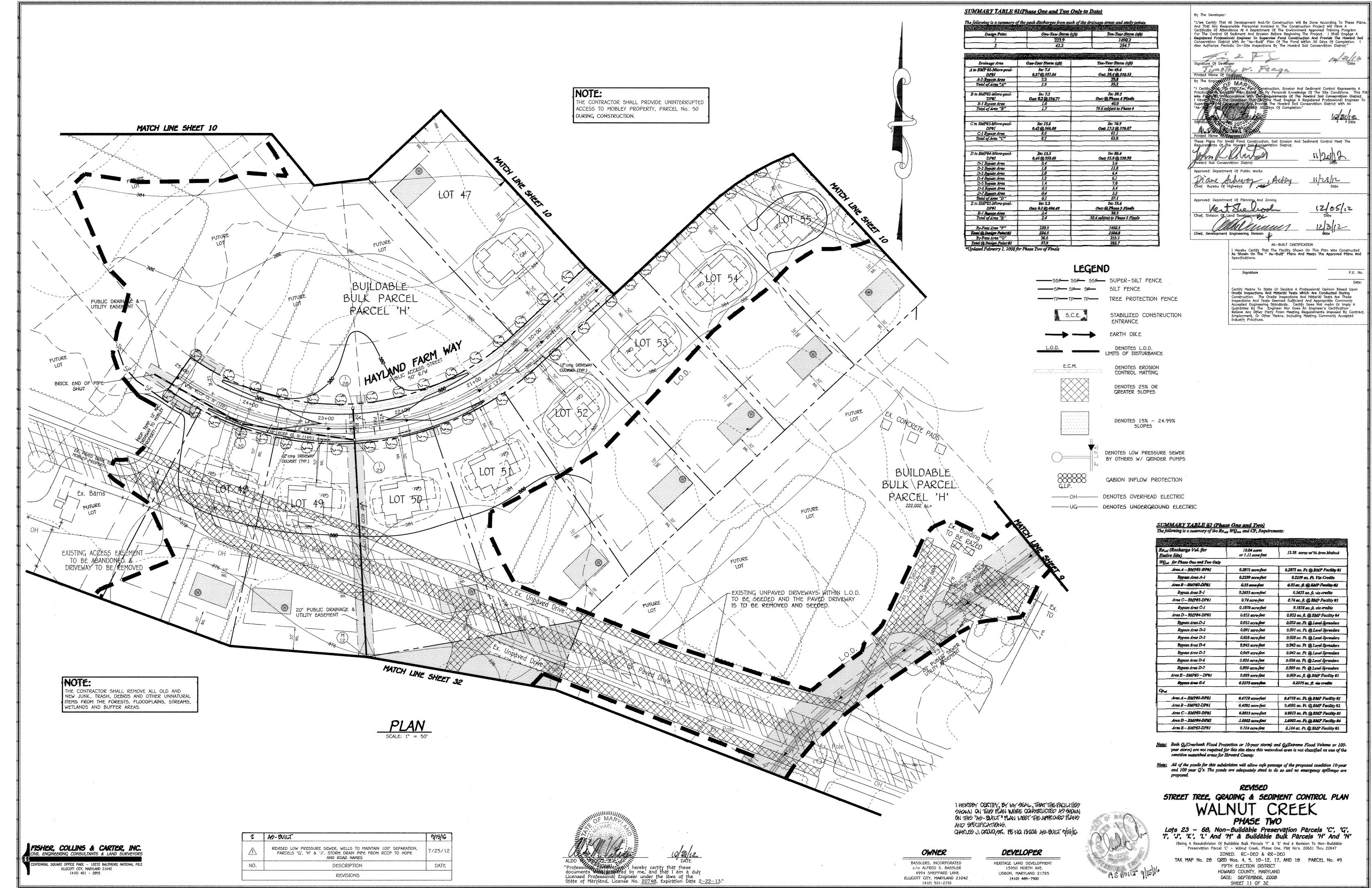


AS-BUILT

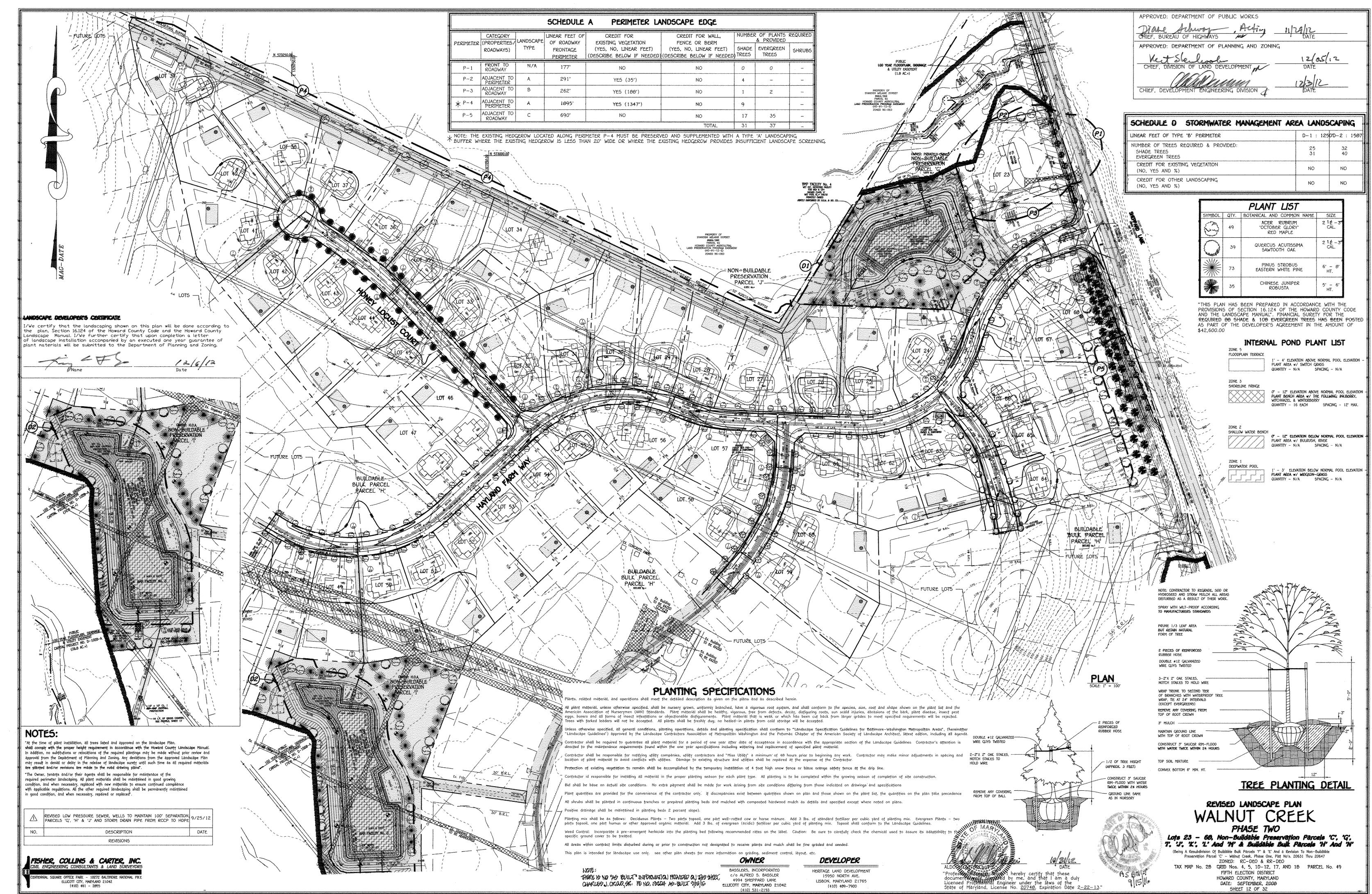




HO-BUILT



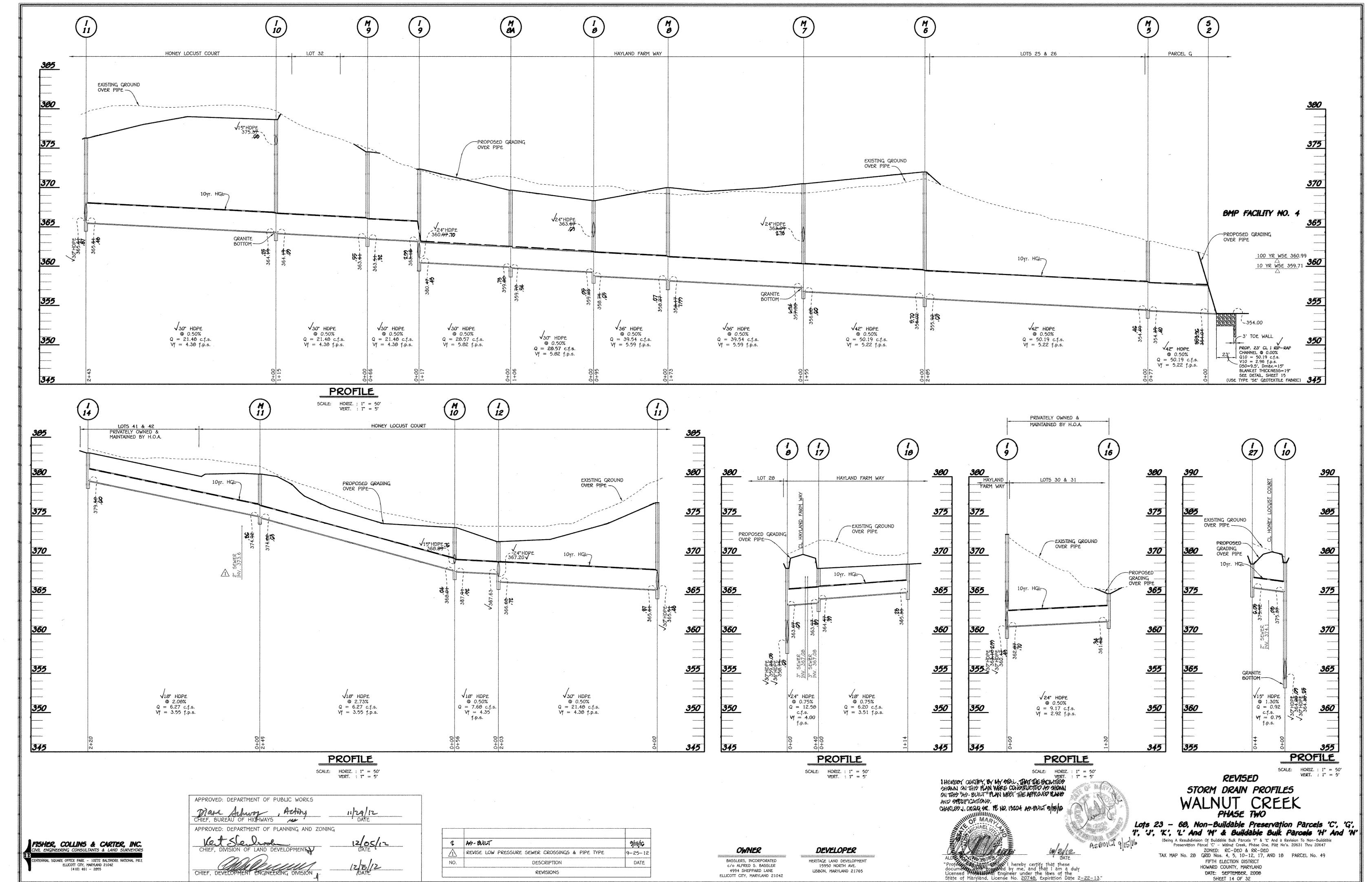
AG-BUILT



F-08-081

AD-BUILT

I:\2004\04001\dwg\PHASE TWO FINALS\PHASE TWO REDLINE 20\$2\04001-8001 SHEET 13-14 STORM DRAIN PROFILES.dwg, Mode



A6-8UNT

*** - ADS FLARED END SECTION OR EQUAL

* - BENOTES THROAT ELEVATION AS-BUILT TOP BLOATIONS SHOWN FOR THE "D" INLEST ARE TOP OF SLAB (PRIVATE) - DENOTES OWNED AND MAINTAINED BY H.O.A. ** - DENOTES LOCATION OF END OF PIPE/BEGINNING OF END SECTION

PIPE	SCHEDULE	(PUBLIC)
SIZE	CLA55	LENGTH
15"	HOPE	275'
18"	HOPE	821'
24"	HOPE	598'
30"	HOPE	1453'
36'	HOPE	541'
42"	HOPE	517'
48"	HOPE	337'
8" 8"	O.I.P. PONO ORAIN H.O.P.E. PONO ORAIN	71' 180'

NOTE: HOPE MAY BE SUBSTITUTED WITH RCCP PIPE MATERIAL.

D.I.P. POND DRAIN

PIPE	SCHEDULE	(PRIVATE)
SIZE	CLA55	LENGTH
15"	HOPE	133'
18"	HOPE	142'
24"	HOPE	307'

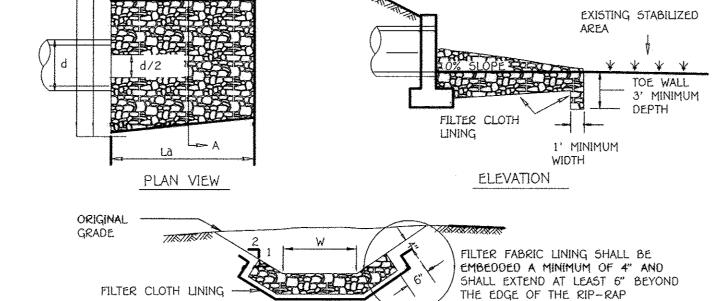
NOTE: HOPE MAY BE SUBSTITUTED WITH RCCP PIPE MATERIAL.

CONSTRUCTION SPECIFICATIONS FOR RIP-RAP OUTFALLS

- 1. The subgrade for the filter, riprap or gabion shall be prepared to the required lines and grades. Any fill required in the subgrade shall be compacted to a density of approximately that of the surrounding
- 2. The rock or gravel shall conform to the specified grading limits when installed respectively in the riprap or filter.
- 3. Filter cloth shall be protected from punching, cutting or tearing. Any damage other than an occasional shall hole shall be repaired by placing another piece of cloth over the damaged part or by completely replacing the cloth. All overlaps whether for repairs or for joining two pieces of cloth shall be a minimum of one foot.

4. Stone for the riprap or gabion outlets may be placed by equipment. Both

shall each be constructed to the full course thickness in one operation and in such a manner as to avoid displacement of underlying materials. The stone for riprap or gabion outlets shall be delivered and placed in a manner that will insure that it is reasonably homogenous with the smaller stones and spalls filling the voids between the larger stones. Riprap shall be placed in a manner to prevent damage to the filter blanket or filter cloth. Hand placement will be required to the extent necessary to prevent damage to the permanent works.



SECTION A-A

RIP-RAP CHANNEL DETAIL

RIP-RAP CHANNEL DESIGN DATA															
AREA (5.F.)	WETTED PERIMETER	R	R 2/3	5	5 1/2	W	d	n	V (f.p.s.)	Q 100 (c.f.s.)	D 50	P SIZE	BLANKET THICKNESS	PIPE SIZE	La
7.69	9.81	0.7839	0.8502	0.005	0.0707	5.0'	1.08	0.04	2.23	17.19	9.5"	15"	19"	3 <i>0</i> "	22'
16.94	14.14	1.1980	1.1280	0.005	0.0707	6.5'	1.71	0.04	2.96	50.19	9.5"	15"	19"	42"	23'
19.03	15.03	1.2661	1.1704	0.005	0.0707	7.0	1.80	0.04	3.07	58.50	9.5"	15"	19"	48"	26'
11.98	13.19	0.9083	0.9378	0.005	0.0707	Ø.0°	1.16	0.04	2.46	29.50	9.5"	15"	19"	24" @ POND #3	20'
21.73	16.54	1.3138	1.1996	0.005	0.0707	8.5'	1.80	0.04	3.15	68.50	9.5"	15"	19"	30" @ POND #4	28'
	(5.F.) 7.69 16.94 19.03 11.90	(5.F.) PERIMETER 7.69 9.81 16.94 14.14 19.03 15.03 11.90 13.19	(5.F.) PERIMETER R 7.69 9.81 0.7839 16.94 14.14 1.1980 19.03 15.03 1.2661 11.90 13.19 0.9083	(5.F.) PERIMETER R R 2/3 7.69 9.81 0.7839 0.8502 16.94 14.14 1.1980 1.1280 19.03 15.03 1.2661 1.1704 11.90 13.19 0.9083 0.9378	AREA (5.F.) PERIMETER R R 2/3 5 7.69 9.81 0.7839 0.8502 0.005 16.94 14.14 1.1980 1.1280 0.005 19.03 15.03 1.2661 1.1704 0.005 11.90 13.19 0.9083 0.9370 0.005	AREA (5.F.) PERIMETER R R 2/3 5 5 1/2 7.69 9.81 0.7839 0.8502 0.005 0.0707 16.94 14.14 1.1980 1.1280 0.005 0.0707 19.03 15.03 1.2661 1.1704 0.005 0.0707 11.90 13.19 0.9083 0.9370 0.005 0.0707	AREA (5.F.) PERIMETER R R 2/3 5 5 1/2 W 7.69 9.81 0.7839 0.8502 0.005 0.0707 5.0' 16.94 14.14 1.1980 1.1280 0.005 0.0707 6.5' 19.03 15.03 1.2661 1.1704 0.005 0.0707 7.0' 11.90 13.19 0.9083 0.9378 0.005 0.0707 8.0'	AREA (S.F.) PERIMETER R R 2/3 S 5 1/2 W d 7.69 9.81 0.7839 0.8502 0.005 0.0707 5.0' 1.08 16.94 14.14 1.1980 1.1280 0.005 0.0707 6.5' 1.71 19.03 15.03 1.2661 1.1704 0.005 0.0707 7.0' 1.80 11.98 13.19 0.9083 0.9378 0.005 0.0707 8.0' 1.16	AREA (S.F.) PERIMETER R R 2/3 S S 1/2 W d n 7.69 9.81 0.7839 0.8502 0.005 0.0707 5.0' 1.08 0.04 16.94 14.14 1.1980 1.1280 0.005 0.0707 6.5' 1.71 0.04 19.03 15.03 1.2661 1.1704 0.005 0.0707 7.0' 1.80 0.04 11.90 13.19 0.9083 0.9370 0.005 0.0707 8.0' 1.16 0.04	AREA (S.F.) PERIMETER R R 2/3 5 5 1/2 W d n (f.p.s.) 7.69 9.81 0.7839 0.8502 0.005 0.0707 5.0' 1.08 0.04 2.23 16.94 14.14 1.1980 1.1280 0.005 0.0707 6.5' 1.71 0.04 2.96 19.03 15.03 1.2661 1.1704 0.005 0.0707 7.0' 1.80 0.04 3.07 11.90 13.19 0.9083 0.9370 0.005 0.0707 8.0' 1.16 0.04 2.46	AREA (5.F.) PERIMETER R R 2/3 5 5 1/2 W d n (f.p.s.) (c.f.s.) 7.69 9.81 0.7839 0.8502 0.005 0.0707 5.0' 1.08 0.04 2.23 17.19 16.94 14.14 1.1980 1.1280 0.005 0.0707 6.5' 1.71 0.04 2.96 50.19 19.03 15.03 1.2661 1.1704 0.005 0.0707 7.0' 1.80 0.04 3.07 50.50 11.90 13.19 0.9083 0.9370 0.005 0.0707 0.0' 1.16 0.04 2.46 29.50	AREA (S.F.) PERIMETER R R 2/3 S S 1/2 W d n (f.p.s.) $(c.f.s.)$ 0.000 RP-RIMETER R R 2/3 S S 1/2 W d n (f.p.s.) 0.000 RP-RIMETER R R R 2/3 S S S 1/2 W d n 0.000 RP-RIMETER R R R 2/3 S S S S S S S S S S S S S S S S S S S	AREA (S.F.) PERIMETER R R $^{2/3}$ S S $^{1/2}$ W d n $^{1/2}$ (C.f.s.) $^{1/2}$ V Q 1000 RP-RAP SIZE (C.f.s.) $^{1/2}$ 0.769 9.81 0.7839 0.8502 0.005 0.0707 5.0' 1.08 0.04 2.23 17.19 9.5' 15'' 16.94 14.14 1.1980 1.1280 0.005 0.0707 6.5' 1.71 0.04 2.96 50.19 9.5'' 15'' 19.03 15.03 1.2661 1.1704 0.005 0.0707 7.0' 1.80 0.04 3.07 50.50 9.5'' 15'' 11.90 13.19 0.9083 0.9370 0.005 0.0707 0.0' 1.16 0.04 2.46 29.50 9.5'' 15''	AREA (S.F.) PERIMETER R R $^{2/3}$ S S $^{1/2}$ W d 0 (f.p.s.) 0 (c.f.s.) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	AREA (S.F.) PERIMETER R R 2/3 5 5 1/2 W d n (f.p.s.) Q 100 RP-RAP SIZE BLANKET FIPE SIZE 7.69 9.81 0.7839 0.8502 0.005 0.0707 5.0' 1.08 0.04 2.23 17.19 9.5" 15" 19" 30' 16.94 14.14 1.1980 1.1280 0.005 0.0707 6.5' 1.71 0.04 2.96 50.19 9.5" 15" 19" 42" 19.03 15.03 1.2661 1.1704 0.005 0.0707 7.0' 1.80 0.04 2.46 29.50 9.5" 15" 19" 24" POND #3

* DENOTES 100 YEAR Q USED FOR DESIGN

2 AG-EVILT 9/15/16 Changed pipe slope from I-29 to stub, low pressure -25-12 sewer crossings and pipe material & Road Names DATE DESCRIPTION **REVISIONS**

NOTE: FILTER CLOTH SHALL BE GEOTEXTILE CLASS C

EXISTING GROUND

10γr. HGb

V15" HOPE

© 1.00%

Q = 2.51

c.f.s.Vf = 2.04

f.p.s.

PROFILE

SCALE: HORIZ. : 1" = 50"

VERT. : 1'' = 5'

OVER PIPE -

PROPOSED GRADING

360

365 360 355 V15" HDPE @ 1.00% Q = 3.50c.f.s. 350 350 Vf = 2.85f.p.s. 345 PROFILE

SCALE: HORIZ. : 1" = 50"

VERT. : 1" = 5'

PARCEL

CHANNEL @ 0.00% Q10 = 50.50 c.f.s. V10 = 3.07 f.p.s. D50=9.5', Dmax.=15"

√48" HDPE

Ø 0.51%

Q = 50.50

c.f.s. Vf = 4.65

370

BLANKET THICKNESS=19" SEE DETAIL, THIS SHEET

(USE TYPE 'SE' GEOTEXTILE FABRIC)

BMP FACILITY NO. 3

370

365

360

355

350

345

365

100 YR WSE 371.48

10 YR W5E 370.07

369.00-

3' TOE WALL

19)

EXISTING GROUND

OVER PIPE -

PROPOSED GRADING OVER PIPE-

26

360 V24" HDPE @ 1.00% Q = 10.69c.f.s. 355 Vf = 3.40f.p.s. PROFILE

SCALE: HORIZ. : 1" = 50' VERT. : 1" = 5'

Licensed Professional Engineer under the laws of the State of Maryland, License No. <u>20748</u>, Expiration Date 2<u>-22-13</u>.

I HEREBY CERTIFY, BY MY SEAL, THAT THE PACILLYIES SHOWN ON THIS FLAN WERE CONSTRUCTED AS SHOWN ON THIS "AG-BUILT" PLAN MEET THE APPROVED PLANS AND GROUNCATIONS. CHARLES J. CROWD, GR. PE NO. 13204 AS-BUILT 9/15/16



REVISED STORM DRAIN PROFILES WALNUT CREEK

APPROVED: DEPARTMENT OF PUBLIC WORK'S

HAYLAND FARM WAY

EXISTING GROUND

OVER PIPE -

Q = 44.79 c.f.s.

Vf = 6.34 f.p.s.

PROFILE

5CALE: HORIZ. : 1" = 50"

-¥24"HDPE/ 371.43√

 $\begin{pmatrix} I \\ 29 \end{pmatrix}$

LOTS 49 & 50

V48" HDPE

@ 0.56%

Q = 58.50 c.f.s.

LOTS 49 & 50 HAYLAND FARM WAY

EXISTING GROUND OVER PIPE -

10yr. HGL

PROPOSED GRADING OVER PIPE-

Vf = 4.65 f.p.s.

20

PROPOSED GRADING

OVER PIPE-

APPROVED: DEPARTMENT OF PLANNING AND ZONING

14/05/12

375

370

365

355

350

345

√36" HDPE @ 0.62%

Q = 44.31 c.f.s.

Vf = 6.27 f.p.s.

PHASE TWO Lots 23 - 60, Non-Buildable Preservation Parcels 'C', 'G', T, 'J', 'K', 'L' And 'M' & Buildable Bulk Parcels 'H' And 'N' (Being A Resubdivision Of Buildable Bulk Parcels 'F' & 'E' And a Revision To Non-Buildable

Preservation Parcel 'C' - Walnut Creek, Phase One, Plat No's, 20631 Thru 20647 ZONED: RC-DEO & RR-DEO TAX MAP No. 28 GRID Nos. 4, 5, 10-12, 17, AND 18 PARCEL No. 49 FIFTH ELECTION DISTRICT HOWARD COUNTY, MARYLAND

OWNER BASSLERS, INCORPORATED c/o ALFRED 5. BASSLER 4994 SHEPPARD LANE ELLICOTT CITY, MARYLAND 21042

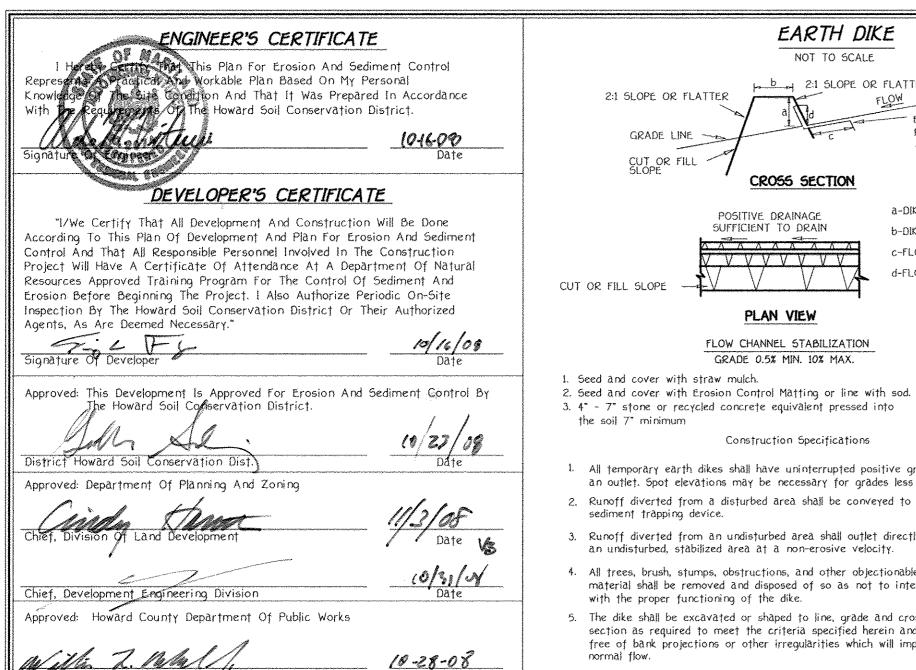
DEVELOPER HERITAGE LAND DEVELOPMENT 15950 NORTH AVE. LISBON, MARYLAND 21765

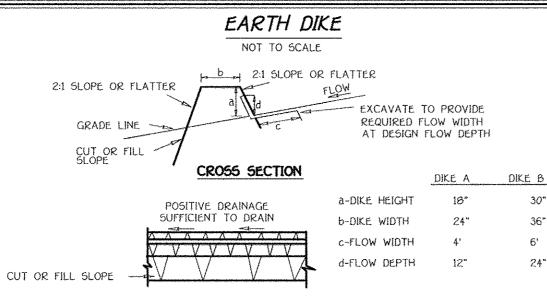
FISHER, COLLINS & CARTER, INC. IL ENGINEERING CONSULTANTS & LAND SURVEYOR: square office park – 10272 Baltimore national pik ELLICOTT CITY, MARYLAND 21042

49-BULT

DATE: SEPTEMBER, 2008

SHEET 15 OF 32





PLAN VIEW FLOW CHANNEL STABILIZATION GRADE 0.5% MIN. 10% MAX.

A-2 B-3

STANDARD SYMBOL

the soil 7" minimum Construction Specifications

1. All temporary earth dikes shall have uninterrupted positive grade to an outlet. Spot elevations may be necessary for grades less than 1%. 2. Runoff diverted from a disturbed area shall be conveyed to a

3. Runoff diverted from an undisturbed area shall outlet directly into an undisturbed, stabilized area at a non-erosive velocity.

4. All trees, brush, stumps, obstructions, and other objectionable material shall be removed and disposed of so as not to interfere with the proper functioning of the dike.

5. The dike shall be excavated or shaped to line, grade and cross section as required to meet the criteria specified herein and be free of bank projections or other irregularities which will impede

6. Fill shall be compacted by earth moving equipment.

sediment trapping device.

7. All earth removed and not needed for construction shall be placed so that it will not interfere with the functioning of the dike.

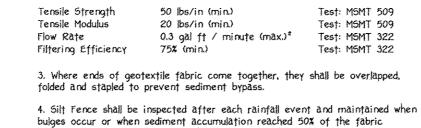
8. Inspection and maintenance must be provided periodically and after each rain event.

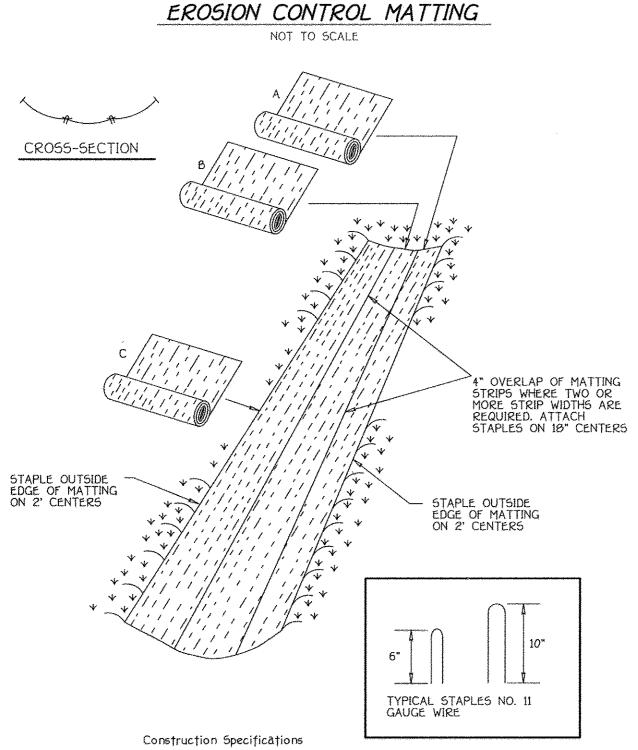
36" MINIMUM LENGTH FENCE POST, 10' MAXIMUM CENTER TO DRIVEN A MINIMUM OF 16" INTO CENTER . 16" MINIMUM HEIGHT O GEOTEXTILE CLASS F - 8" MINIMUM DEPTH IN GROUND PERSPECTIVE VIEW POST LENGTH CLOTH -FENCE POST SECTION MINIMUM 20" ABOVE GROUND EMBED GEOTEXTILE CLASS F TOP VIEW A MINIMUM OF 8" VERTICALLY - FENCE POST DRIVEN A MINIMUM OF 16" INTO INTO THE GROUND THE GROUND CROSS SECTION SECTION B STANDARD SYMBO STAPLE JOINING TWO ADJACENT SILT FENCE SECTIONS Construction Specifications 1. Fence posts shall be a minimum of 36" long driven 16" minimum into the around. Wood posts shall be 11/2" x 11/2" square (minimum) cut, or 13/4" diameter (minimum) round and shall be of sound quality hardwood. Steel posts will be standard T or U section weighting not less than 1.00 pond per linear foot. 2. Geotextile shall be fastened securely to each fence post with wire ties or staples at top and mid-section and shall meet the following requirements for Tensile Strenath 50 lbs/in (min.) Test: MSMT 509 Tensile Modulus 20 lbs/in (min.) Test: MSMT 322 Flow Rate 0.3 gal ft / minute (max.)2 Filtering Efficiency 75% (min.) Test: MSMT 322 folded and stapled to prevent sediment bypass.

SILT FENCE

UNDISTURBED

NOT TO SCALE





Key-in the matting by placing the top ends of the matting in a narrow trench, 6" in depth. Backfill the trench and tamp firmly to conform to the channel cross-section. Secure with a row of staples about 4" down slope from the trench. Spacing between staples is 6".

2. Staple the 4" overlap in the channel center using an 18" spacing between staples

Before stapling the outer edges of the matting, make sure the matting is smooth and in firm contact with the soil.

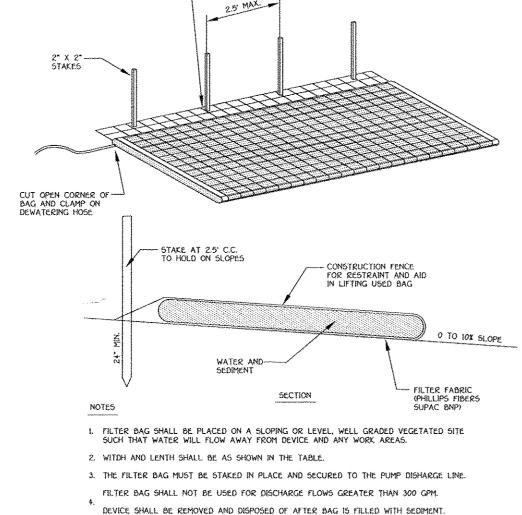
Staples shall be placed 2' apart with 4 rows for each strip. 2 outer rows, and 2 alternating rows down the center.

Where one roll of matting ends and another begins, the end of the top strip shall overlap the upper end of the lower strip by 4", shiplap fashion. Reinforce the overlap with a double row of staples spaced 6" apart in a staggered pattern on either side.

The discharge end of the matting liner should be similarly secured with 2 double rows of staples.

Note: If flow will enter from the edge of the matting then the area effected by the flow must be keyed-in.

FISHER, COLLINS & CARTER, INC. SQUARE OFFICE PARK - 10272 BALTIMORE NATIONAL PI ELLICOTT CITY, MARYLAND 21042 (410) 461 - 2955

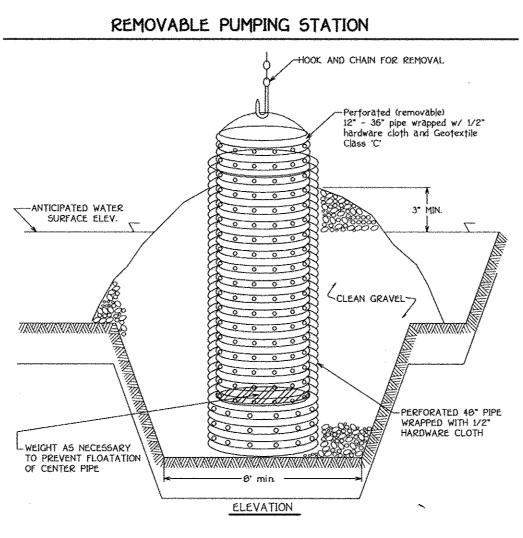


DEVICE SHALL BE REMOVED AND DISPOSED OF AFTER BAG IS FILLED WITH SEDIMENT 5. SEDIMENT FROM BAG SHALL BE SPREAD IN AN UPLAND AREA.

AVAILABLE FROM: INDIAN VALLEY INDUSTRIES, INC. JOHNSON CITY, NEW YORK 13790 (800) 659-5111 1801-A WILLIS ROAD RICHMOND, VIRGINIA 23237 TOLL FREE 1-800-448-3636

FILTER BAG DETAIL

PRICE AND COMPANY, IN

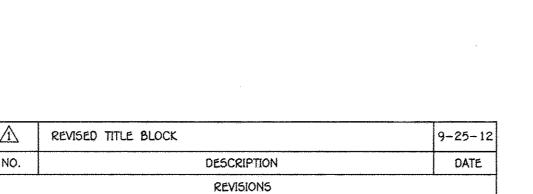


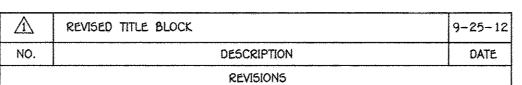
Construction Specifications 1. The outer pipe should be 48" dia. or shall, in any case, be at least 4" greater in diameter than the center pipe. The outer pipe shall be wrapped with 1/2" hardware cloth to prevent backfill material from entering the perforations.

2. After installing the outer pipe, backfill around outer pipe with 2" aggregate or clean gravel.

3. The inside stand pipe (center pipe) should be constructed by perforating a corrugated or PVC pipe between 12" and 36" in diameter. The perforations shall be 1/2" X 6" slits or 1" diameter holes 6" on center. The center pipe shall be wrapped with 1/2" hardware cloth first, then wrapped again with Geotextile Class C.

4. The center pipe should extend 12" to 10" above the anticipated water surface elevation or riser crest elevation when dewatering a basin.





OWNER DEVELOPER

BASSLERS, INCORPORATED

c/o ALFRED 5. BASSLER

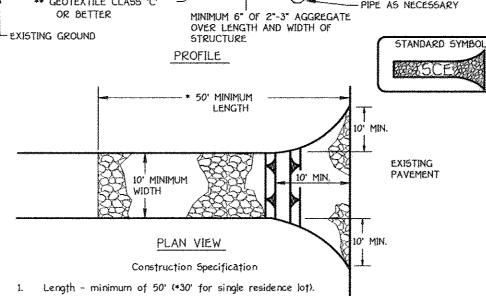
4994 SHEPPARD LANE

ELLICOTT CITY MARYLAND 21042

HERITAGE LAND DEVELOPMENT 15950 NORTH AVE. LISBON MARYLAND 21765

--- EARTH FILL ** GEOTEXTILE CLASS 'C' - PIPE AS NECESSARY OR BETTER MINIMUM 6" OF 2"-3" AGGREGATE OVER LENGTH AND WIDTH OF

· MOUNTABLE



STABILIZED CONSTRUCTION ENTRANCE

NOT TO SCALE

2. Width - 10' minimum, should be flared at the existing road to provide a turning radius. Geotextile fabric (filter cloth) shall be placed over the existing ground prior to placing

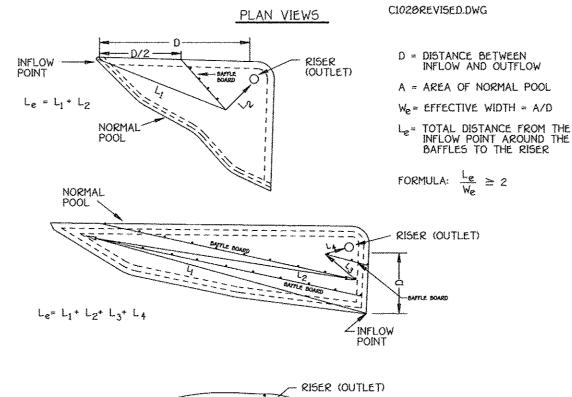
3. stone. **The plan approval authority may not require single family residences to use

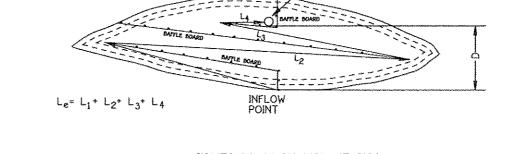
Stone - crushed aggregate (2" to 3") or reclaimed or recycled concrete equivalent shall 4. be placed at least 6" deep over the length and width of the entrance.

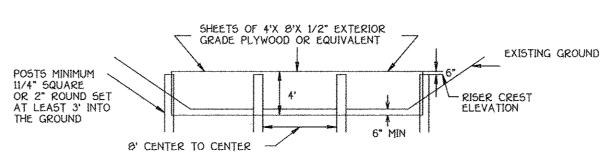
Surface Water - all surface water flowing to or diverted toward construction entrances shall be piped through the entrance, maintaining positive drainage. Pipe installed through the stabilized construction entrance shall be protected with a mountable berm with 5:1 slopes and a minimum of 6" of stone over the pipe. Pipe has to be sized according to the drainage. When the SCE is located at a high spot and has no drainage to convey a pipe will not be necessary. Pipe should be sized according to the amount of runoff to be conveyed. A 6" minimum will be required

Location - A stabilized construction entrance shall be located at every point where construction traffic enters or leaves a construction site. Vehicles leaving the site must travel over the entire length of the stabilized construction entrance.

SEDIMENT BASIN BAFFLES







BAFFLE DETAIL

SEDIMENT CONTROL NOTES

1) A MINIMUM OF 48 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT OF INSPECTIONS, LISCENSES AND PERMITS, SEDIMENT CONTROL

DIVISION PRIOR TO THE START OF ANY CONSTRUCTION (313-1855). 2) ALL VEGETATIVE AND STRUCTURAL PRACTICES ARE TO BE INSTALLED

ACCORDING TO THE PROVISIONS OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE MOST CURRENT MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL AND REVISIONS THERETO. 3) FOLLOWING INITIAL SOIL DISTURBANCE OR RE-DISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: a) 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, PERIMETER SLOPES AND ALL SLOPES STEEPER THAN 3:1, b) 14 DAYS AS TO ALL OTHER

DISTURBED OR GRADED AREAS ON THE PROJECT SITE. 4) ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 12, OF

THE HOWARD COUNTY DESIGN MANUAL, STORM DRAINAGE. 5) ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDING (SEC. 51), SOD (SEC. 54), TEMPORARY SEEDING (SEC. 50), AND MULCHING (SEC. 52). TEMPORARY STABILIZATION WITH MULCH ALONE CAN ONLY BE DONE WHEN RECOMMENDED SEEDING DATES DO NOT ALLOW FOR PROPER GERMINATION AND ESTABLISHMENT OF GRASSES.

6) ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE MAINTAINED IN OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR. 7) SITE ANALYSIS:

TOTAL AREA OF SITE AREA DISTURBED AREA TO BE ROOFED OR PAVED AREA TO BE VEGETATIVELY STABILIZED

OFFSITE WASTE/BORROW AREA LOCATION N/A CU.YDS. 8) ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF UTILITIES MUST BE REPAIRED ON THE SAME DAY OF

DISTURBANCE

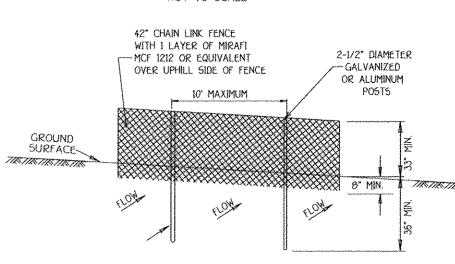
ACRES

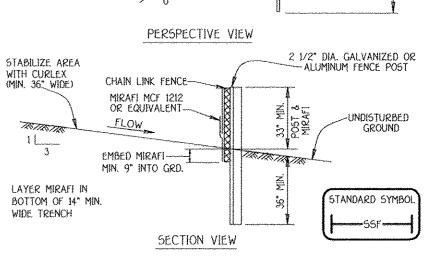
ACRES

9) ADDITIONAL SEDIMENT CONTROLS MUST BE PROVIDED, IF DEEMED NECESSARY BY THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR. 10) ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING. OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL

APPROVAL BY THE INSPECTION AGENCY IS MADE. 11) TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE NOTE LENGHTS OR THAT WHICH SHALL BE BACK-FILLED AND STABILIZED WITHIN ONE PHORE IS NO "AS BUILT" INFORMATION PROMPED ON THE HEAT, HOWARD COUNTY, MARYLAND WORKING DAY, WHICHEVER IS SHORTER.

SUPER SILT FENCE DETAIL NOT TO SCALE





CONSTRUCTION SPECIFICATIONS

1. FENCING SHALL BE 42" HIGH CHAIN CONSTRUCTED IN ACCORDANCE WITH THE LATEST MARYLAND STATE HIGHWAY ADMINISTRATION STANDARD DETAILS 690.01 AND 690.02 FOR CHAIN U FENCING. THE SPECIFICATIONS FOR A 6'-0" FENCE SHALL BE USED, SUBSTITUTING 42" FABRIC AND 8' POSTS. POSTS SHALL BE PLACED WITHOUT CONCRETE EMBEDMENT.

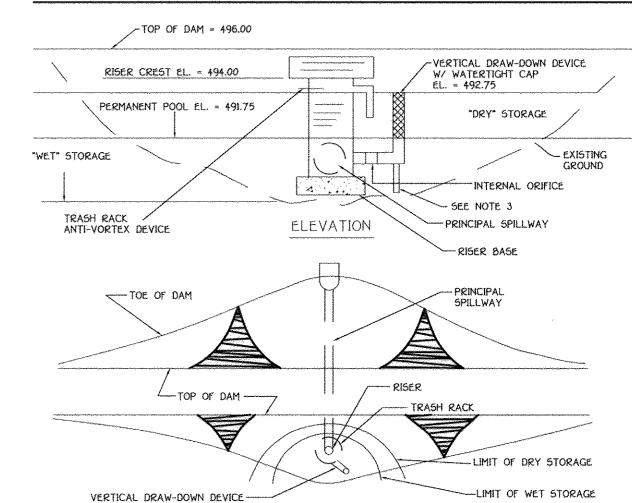
2. CHAIN LINK FENCE SHALL BE FASTENED SECURELY TO FENCE POSTS WITH WIRE TIES OR STAPLES. THE LOWER TENSION WIRE, BRACE AND TRUSS RODS, ANCHORS AND POST CAPS ARE NOT REQUIRED EXCEPT ON THE ENDS OF THE FENCE. 3. FILTER CLOTH TO BE FASTENED SECURELY TO CHAIN LINK FENCE

WITH TIES SPACED EVERY 24" AT TOP AND MID SECTION. 4. FILTER CLOTH SHALL BE IMBEDDED A MINIMUM OF 9" INTO THE

5. WHEN TWO SECTIONS OF DIVERSION CLOTH ADJOIN EACH OTHER THEY SHALL BE OVERLAPPED BY SIX INCHES AND FOLDED. 6. MAINTENANCE SHALL BE PERFORMED AS NEEDED.

Fabric	: Properties	Value	Test Method		
Grab Tens	ile Strength (lbs.)	90	ASTM D1682		
	at Failure (%)	50	A5TM D1682		
•	st Strength (PSI)	190	A5TM D3706		
	Strength (lbs.)	40	ASTM D751		
Slurry Flow	/ Rate (gal/min/sf)	0.3	Virginia DOT VTM-51		
Equivalent	Opening Size	40-80	US Std Sieve CW-02215		
Utraviolet	Radiation Stability	(%) 90	ASTM G-26		
	Desig	n Criteria			
Slope	Slope Steepness	Slope Length (maximum)	Silt Fence Leng (māximum)		
0 - 10%	0 - 10:1	Unlimited	Unlimited		
10 - 20%	10:1 - 5:1	400 feet	1,500 feet		
20 - 33%	5:1 - 3:1	300 feet	1,000 feet		
33 - 50%	3:1 - 2:1	200 feet	500 feet		
50% +	2:1 +	100 feet	250 feet		

VERTICAL DRAW-DOWN DEVICE



PLAN VIEW

CONSTRUCTION SPECIFICATIONS

1. PERFORATIONS IN THE DRAW-DOWN DEVICE MAY NOT EXTEND INTO THE WET STORAGE. 2. THE TOTAL AREA OF THE PERFORATIONS MUST BE GREATER THAN 2 TIMES THE AREA OF THE INTERNAL ORIFICE.

3. THE PERFORATED PORTION OF THE DRAW-DOWN DEVICE SHALL BE WRAPPED WITH 1/2" HARDWARE CLOTH AND GEOTEXTILE FABRIC. THE GEOTEXTILE FABRIC SHALL MEET THE SPECIFICATIONS FOR GEOTEXTILE CLASS E.

4. PROVIDE SUPPORT OF DRAW-DOWN DEVICE TO PREVENT SAGGING AND FLOATATION. AN ACCEPTABLE PREVENTATIVE MEASURE IS TO STAKE BOTH SIDES OF DRAW-DOWN DEVICE WITH I" STEEL ANGLE, OR I' BY 4" SQUARE OR 2" ROUND WOODEN POSTS SET 3' MINIMUM INTO THE GROUND THEN JOINING THEM TO THE DEVICE BY WRAPPING WITH 12 GAUGE

SEDIMENT CONTROL NOTES AND DETAILS

WALNUT CREEK

ASBUILT

Lots 23 - 68, Non-Buildable Preservation Parcels 'C', 'G', 'I', 'J', 'K', 'L' And 'M' & Buildable Bulk Parcels 'H' And 'N' (Being A Resubdivision Of Buildable Bulk Parcels 'F' & 'E' And a Revision To Non-Buildable Preservation Parcel 'C' - Walnut Creek, Phase One, Plat No's. 20631 Thru 20647. ZONED: RC-DEO & RR-DEO TAX MAP No. 28 GRID Nos. 4, 5, 10-12, 17, AND 18 PARCEL No. 49

CHARLES J. OKONO, SR. PENO. 19204 AS-BUILT 9/15/16

FIFTH ELECTION DISTRICT

20.0 STANDARDS AND SPECIFICATIONS VEGETATIVE STABILIZATION DEFINITION

Using vegetation as cover for barren soil to protect it from forces that cause erosion

Vegetative stabilization specifications are used to promote the establishment of vegetation on exposed soil. When soil is stabilized with vegetation, the soil is less likely to erode and more likely to allow infiltration of rainfall, thereby reducing sediment loads and run-off to downstream areas, and improving wildlife habitat and visual resources

CONDITIONS WHERE PRACTICE APPLIES

This practice shall be used on denuded areas as specified on the plans and may be used on highly erodible or critically eroding areas. This specification is divided into Temporary Seeding, to quickly establish vegetative cover for short duration O(up to one year), and Permanent Seeding, for long term vegetative cover. Éxamples of applicable areas for Temporary Seeding are temporary Soil Stockpiles, cleared areas being left idle between construction phases, earth dikes, etc. and for Permanent Seeding are lawns, dams, cut and fill slopes and other areas at final grade, former stockpile and staging areas, etc. EFFECTS ON WATER QUALITY AND QUANTITY

Planting vegetation in disturbed areas will have an effect on the water budget, especially on volumes and rates of runoff, infiltration evaporation, transpiration, percolation, and groundwater recharge. Vegetation, over time, will increase organic matter content and improve the water holding capacity of the soil and subsequent plant growth. Vegetation will help reduce the movement of sediment, nutrients, and other chemicals carried by runoff to receiving waters. Plants will also help protect groundwater supplies by assimilating those substances present within the root zone. Sediment control devices must remain in place during grading, seedbed preparation, seeding, mulching and vegetative establishment to prevent large quantities of sediment and associated chemicals and nutrients from washing into surface waters.

SECTION 1 - VEGETATIVE STABILIZATION METHODS AND MATERIALS

i. Install erosion and sediment control structures (either temporary of permanent) such as diversions, grade stabilization structures, berms, waterways, or sediment control basins. ii. Perform all grading operations at right angles to the slope. Final grading and shaping is not usually

necessary for temporary seeding. iii. Schedule required soil tests to determine soil amendment composition and application rates for sites

having disturbed area over 5 acres. 3. Soil Amendments (Fertilizer and Lime Specifications) Soil tests must be performed to determine the exact ratios and application rates for both lime and fertilizer on sites having disturbed areas over 5 acres. Soil analysis may be performed by the University of Maryland or a recognized commercial laboratory. Soil samples taken for engineering

purposes may also be used for chemical analyses. ii. Fertilizers shall be uniform in composition, free flowing and suitable for accurate application by approved equipment. Manure may be substituted for fertilizer with prior approval from the appropriate approval authority. Fertilizers shall all be delivered to the site fully labeled according to the applicable state fertilizer laws and shall bear the name, trade name or trademark and warrantee of the producer.

iii. Lime materials shall be ground limestone (hydrated or burnt lime may be substituted) which contains at least 50% total oxides (calcium oxide plus magnesium oxide). Limestone shall be ground to such fineness that at least 50% will pass through a *100 mesh sieve and 90-100% will pass through a *20 mesh sieve.

iv. Incorporate lime and fertilizer into the top 3-5" of soil by disking or other suitable means. Seedbed Preparation

Temporary Seeding

a. Seedbed preparation shall consist of loosening soil to a depth of 3" to 5" by means of suitable agricultural or construction equipment, such as disc harrows or chisel plows or rippers mounted on construction equipment. After the soil is loosened it should not be rolled or dragged smooth, but left in the roughened condition. Sloped areas (greater than 3:1) should be tracked leaving the surface in an irregular condition with ridges running parallel to the contour of the slope. b. Apply fertilizer and lime as prescribed on the plans.

In corporate lime and fertilizer into the top 3-5" of soil by disking or other suitable means. . Permanent Seeding

a. Minimum soil conditions required for permanent vegetative establishment:

1. Soil pH shall be between 6.0 and 7.0. Soluble salts shall be less than 500 parts per million (ppm). The soil shall contain less than 40% clay, but enough fine grained material (>30% silt plus clay) to provide the capacity to hold a moderate amount of moisture. An exception is if lovegrass or serecia lespedezas is to be planted, then a sandy soil (<30% silt

plus clay) would be acceptable. Soil shall contain 1.5% minimum organic matter by weight. Soil must contain sufficient pore space to permit adequate root penetration. 6. If these conditions cannot be met by soils on site, adding topsoil is required

in accordance with Section 21 Standard and Specification for Topsoil. Areas previously graded in conformance with the drawings shall be maintained in a true and even grade, then scarified or otherwise loosened to a depth of 3-5" to permit bonding of the topsoil to the surface area and to create horizontal erosion check slots to prevent topsoil to the surface area and to create horizontal erosion check slots to prevent topsoil from

Apply soil amendments as per soil test or as included on the plans. Mix soil amendments into the top 3-5" of topsoil by disking or other suitable means. Lawn areas should be raked to smooth the surface, remove large objects like stones and branches, and ready the area for seed and application. Where site conditions will not permit normal seedbed preparation, loosen surface soil by dragging with a heavy chain or other equipment to roughen the surface. Steep slopes (steeper than 3:1) should be tracked by a dozer leaving the soil in an irregular condition with ridges running parallel to the contour of the slope. The top 1-3" of soil should be loose and triable. Seedbed loosening may not be necessary on

D. Seed Specifications All seed must meet the requirements of the Maryland State Seed Law. All seed shall be subject to re-testing by a recognized seed laboratory. All seed used shall have been tested within the 6 months immediately preceding the date of sowing such material on this job.

Note: Seed tags shall be made available to the inspector to verify type and rate of seed used. ii. Inoculant - The inoculant for treating legume seed in the seed mixtures shall be a pure culture of nitrogen-fixing bacteria prepared specifically for the species. Inoculants shall not be used later than the date indicated on the container. Add fresh inoculant as directed on package. Use four times the recommended rate when hydroseeding. Note: It is very important to keep inoculant as cool as possible until used. Temperatures above 75°-80° F. can weaken bacteria and make the inoculant less effective.

Hydroseeding: Apply seed uniformly with hydroseeder (slurry includes seed and fertilizer), broadcast or drop seeded, or a cultipacker seeder.

a. If fertilizer is being applied at the time of seeding, the application rates amounts will not

exceed the following: nitrogen; maximum of 100 lbs. per acre total of soluble nitrogen; P205 (phosphorous); 200 lbs/ac; K20 (potassium); 200 lbs/ac.

b. Lime - use only ground agricultural limestone, (Up to 3 tons per acre may be applied by formally, not more than 2 tons are applied by h time. Do not use burnt or hydrated lime when hydroseeding

Seed and fertilizer shall be mixed on site and seeding shall be done immediately and without interruption. ii. Dry Seeding: This includes use of conventional drop or broadcast spreaders. Seed spread dry shall be incorporated into the subsoil at the rates prescribed on the Temporary or Permanent Seeding Summaries or Tables 265 or 26. The seeded area shall then be rolled with a weighted roller to provide good seed to soil contact.

Where practical, seed should be applied in two directions perpendicular to each other. Apply half the seeding rate in each direction. iii. Drill or Cultipacker Seeding: Mechanized seeders that apply and cover seed with soil. Cultipacking seeders are required to bury the seed in such a fashion as to provide at

least 1/4 inch of soil covering. Seedbed must be firm after planting.

b. Where practical, seed should be applied in two directions perpendicular to each other. Apply half the seeding rate in each direction.

F. Mulch Specifications (In order of preference)
i. Straw shall consist of thoroughly threshed wheat, rye or oat straw, reasonable bright in color, and shall not be musty, moldy, caked, decayed, or excessively dusty and shall be free of noxious weed seeds as specified in the Maryland Seed Law. ii. Wood Cellulose Fiber Mulch (WCFM)

a. WCFM shall consist of specially prepared wood cellulose processed into a uniform fibrous physical state. b. WCFM shall be dyed green or contain a green dye in the package that will provide an appropriate color to facilitate visual inspection of the uniformly spread slurry.
 c. WCFM, including dye, shall contain no germination or growth inhibiting factors.

WCFM materials shall be manufactured and processed in such a manner that the wood cellulose fiber mulch will remain in uniform suspension in water under agitation and will blend with seed, fertilizer and other additives to form a homogeneous slurry. The mulch material shall form a blotter-like ground cover, on application, having moisture absorption and percolation properties and shall cover and hold grass seed in contact with the soil without inhibiting the growth of the grass seedlings.

e. WCFM material shall contain no elements or compounds at concentration levels that will be phytol-toxic.

f. WCFM must conform to the following physical requirements: fiber length to approximately 10 mm., diameter approximately 1 mm., pH range of 4.0 to 8.5, ash content of 1.6% maximum and water holding capacity of 90% minimum.

Note: Only sterile straw mulch should be used in areas where one species of grass is desired.

Mulching Seeded Areas - Mulch shall be applied to all seeded areas immediately after seeding. If grading is completed outside of the seeding season, mulch along shall be applied as prescribed in this section and maintained until the seeding season returns and seeding can be performed in accordance with these specifications. ii. When straw mulch is used, it shall be spread over all seeded areas at the rate of 2 tons/acre. Mulch shall be applied to a uniform loose depth of between 1° and 2°. Mulch applied shall achieve a uniform distribution and depth so that the soil surface is not exposed. If a mulch anchoring tool is

to be used, the rate should be increased to 2.5 tons/acre. iii. Wood cellulose fiber used as a mulch shall be applied at a net dry weight of 1,500 lbs. per acre. The wood cellulose fiber shall be mixed with water, and the mixture shall contain a maximum of 50 lbs. of wood cellulose fiber per 100 gallons of water. H. Securing Straw Mulch (Mulch Anchoring): Mulch anchoring shall be performed immediately following mulch

application to minimize loss by wind or water. This may be done by one of the following methods (listed by preference), depending upon size of area and erosion hazard: i. A mulch anchoring tool is a tractor drawn implement designed to punch and anchor mulch into the soil surface a minimum of two (2) inches. This practice is most effective on large areas, but is limited to flatter slopes where equipment can operate safely. If used on sloping

land, this practice should be used on the contour if possible.

ii. Wood cellulose fiber may be used for anchoring straw. The fiber binder shall be applied at a net dry weight of 750 pounds/acre. The wood cellulose fiber shall be mixed with water and he mixture shall contain a maximum of 50 pounds of wood cellulose fiber per 100 gallons iii. Application of liquid binders should be heavier at the edges where wind catches mulch, such as in Valleys and crest of banks. The remainder of area should be appear uniform after binder application. Synthetic binders – such as Acrylic DLR (Agro-Tack), DCA-70 Petroset, Terra Tax

II, Terra Tack AR or other approved equal may be used at rates recommended by the manufacturer to anchor mulch.

iv. Lightweight plastic netting may be stapled over the mulch according to manufacturer's recommendations. Netting is usually available in rolls 4' to 15' feet wide and 300 to 3,000 feet long. I. Incremental Stabilization - Cut Slopes

i. All cuts slopes shall be dressed, prepared, seeded and mulched as the work progresses. Slopes shall be excavated and stabilized in equal increments not to exceed 15'. ii. Construction sequence (Refer to Figure 3 below):

a. Excavate and stabilize all temporary swales, side ditches, or berms that will be

used to convey runoff from the excavation b. Perform Phase 1 excavation, dress, and stabilize. c. Perform Phase 2 excavation, dress and stabilize. Overseed Phase 1 areas as

necessary.
d. Perform final phase excavation, dress and stabilize. Overseed previously seeded

Note: Once excavation has begun the operation should be continuous from grubbing through the completion of grading and placement of topsoil (if required) and permanent seed and mulch. Any interruptions int he operation of completing the operation out of the seeding season will necessitate the application of temporary stabilization.

J. Incremental Stabilization of Embankments - Fill Slopes Embarkments shall be constructed in lifts as prescribed on the plans. Slopes shall be stabilized immediately when the vertical height of the multiple lifts reaches 15°, or when the grading operation ceases as prescribed in the plans.

iii. At the end of each day, temporary berms and pipe slope drains should be constructed along the top edge of the embankment to intercept surface runoff and convey it down the slope in a non-erosive manner à sediment trapping device. iv. Construction sequence: Refer to Figure 4 (below).

a. Excavate and stabilize all temporary swales, side ditches, or berms that will be used to divert runoff around the fill. Construct slope silt fence on low side of fill as shown in Figure 5, unless other methods shown on the plans address this area.

Place Phase 1 embankment, dress and stabilize.

Place Phase 2 embankment, dress and stabilize.

Place final phase embankment, dress and stabilize. Overseed previously seeded

Note: Once the placement of fill has begun the operation should be continuous from grubbing through the completion of and placement of topsoil (if required) grading and permanent seed and mulch. any interruptions in the operation or completing the operation out of the seeding season will necessitate the application of temporary stabilization.

SECTION 2 - TEMPORARY SEEDING

Vegetation - annual grass or grain used to provide cover on disturbed areas for up to 12 months. For longer duration of vegetative cover, Permanent Seeding is required.

A. Seed mixtures - Temporary Seeding i. Select one or more of the species or mixtures listed in Table 26 for the appropriate Plant Hardiness Zone (from Figure 5) and enter them in the Temporary seeding summary below, along

with application rates, seeding dates and seeding depths. If this summary is not put on the plans and completed, then Table 26 must be put on the plans.

ii. For sites having soil tests performed, the rates shown on this table shall be deleted and the rates recommended by the testing agency shall be written in Soil tests are not required for Temporary Seeding.

5e	ed Mixture (Hard From	ness Zone <u>6b</u> Table 26)		Fertilizer Rate	Lime Rate		
No.	5pecies	Application Rate (lb/ac)	Seeding Dates	Seeding Depths	(10-10-10)			
1	BARLEY OATS RYE	122 96 140	3/1 - 5/15, 8/15 - 10/15	1" - 2" 1" - 2" 1" - 2"	600 b/ac (15 b/1000sf)	2 tons/ac (100 lb/1000sf)		

SECTION 3 - PERMANENT SEEDING

Seeding grass and legumes to establish groung cover for a minimum of one year on disturbed areas

A. Seed mixtures - Permanent Seeding

i. Select one or more of the species or mixtures listed in Table 25 for the appropriate Plant Hardiness Zone (from Figure 5) and enter them in the Permanent Seeding Summary below, along with application rates and seeding dates. Seeding depths can be estimated using Table 26. If this summary is not put on the construction plans and completed, then Table 25 must be put on the plans. Additional planting specifications for exceptional sites such as shorelines, streambanks, or dunes or for special purposes such as wildlife or aesthetic treatment may be found in USDA-SCS Techinical Field Office Guide, Section 342 - Critical Area Planting. For special lawn maintenance areas, see Sections IV Sod and V Turfgrass.

ii. For sites having disturbed area over 5 areas, the rates shown on this table shall be deleted and the rates recommended by the soil testing agency shall be written in

iii. For areas receiving low maintenance, apply ureaform fertilizer (46-0-0) at 3 1/2 lbs/1000 sq. ft. (150 lbs/ac), in addition to the above soil amendments shown in the table below, to be performed at the time of seeding.

mannes Administration and the second	5eed Mixture (Hardiness Z From Table		The state of the s	Lime Rate				
No.	Species	Application Rate (lb/ac)	Seeding Dates	Seeding Depths	Z-	P205	K20	134.15
3	TALL FESCUE (05%) PERENNIAL RYE GRASS (10%) KENTUCKY BLUEGRASS (5%)	125 15 10	3/1 - 5/15, 8/15 - 10/15	1" - 2"	90 lb/ac (2.0 lb/	175 lb/ac	175 lb/ac	2 tons/ac (100 lb/
10	TALL FESCUE (80%) HARD FESCUE (20%)	120 30	3/1 - 5/15, 8/15 - 10/15	1* - 2*	1000sf)	1000sf)	1000sf)	1000sf)

TOPSOIL NOTES

Placement of topsoil over a prepared subsoil prior to establishment of permanent vegetation.

d. The soil is so acidic that treatment with limestone is not feasible.

To provide a suitable soil medium for vegetative growth. Soils of concern have low moisture content, low nutrient levels, low pH, materials toxic to plants, and/or unacceptable soil gradation. Conditions Where Practice Applies

1. This practice is limited to areas having 2:1 or flatter slopes where: a. The texture of the exposed subsoil/parent material is not adequate to produce vegetative growth. b. The soil material is so shallow that the rooting zone is not deep enough to support plants or furnish continuing supplies of moisture and plant nutrients. c. The original soil to be vegetated contains material toxic to plant growth.

II. For the purpose of these Standards and Specifications, areas having slopes steeper than 2:1 require special consideration and design for adequate stabilization. Areas having slopes steeper than 2:1 shall have the appropriate stabilization shown on the plans. Construction and Material Specifications

Topsoil salvaged from the existing site may be used provided that it meets the standards as set forth in these specifications. Typically, the depth of topsoil to be salvaged for a given soil type can be found in the representative soil profile section in the Soil Survey published by USDA-SCS in cooperation with Maryland Agricultural Experimental Station

Topsoil Specifications - Soil to be used as topsoil must meet the following: i. Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay loam, loamy sand. Other soils may be used if recommended by an agronomist or soil scientist and approved by the appropriate approval authority. Regardless, topsoil shall not be a mixture of contrasting textured subsoils and shall contain less than 5% by volume of cinders, stones, slag, coarse

fragments, gravel, sticks, roots, trash, or other materials larger than 11/2" in diameter Topsoil must be free of plants or plant parts such as bermuda grass, quackgrass, Johnsongrass, nutsedge, poison ivy, thistle, or others as specified.

iii. Where the subsoil is either highly acidic or composed of heavy clays, ground limestone shall be spread at the rate of 4-8 tons/acre (200-400 pounds per 1.000 square feet) prior to the placement of topsoil. Lime shall be distributed uniformly over designated areas and worked into the soil in conjunction with tillage operations as described in the following procedures.

i. Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization - Section I - Vegetative Stabilization Methods and Materials.

for sites having, disturbed areas under 5 acres:

For sites having disturbed areas over 5 acres: i. On soil meeting Topsoil specifications, obtain test results dictating fertilizer and lime

amendments required to bring the soil into compliance with the following

a. pH for topsoil shall be between 6.0 and 7.5. If the tested soil demonstrates a pH of less than 6.0, sufficient lime shall be perscribed to raise the pH to 6.5 or higher.

b. Organic content of topsoil shall be not less than 1.5 percent by weight c. Topsoil having soluble salt content greater than 500 parts per million shall not be used. d. No sod or seed shall be placed on soil which has been treated with soil sterilants or

chemicals used for weed control until sufficient time has elapsed (14 days min.) to permit dissipation of phyto-toxic materials. Note: Topsoil substitutes or amendments, as recommended by a qualified agronomist or soil scientist and approved by the appropriate approval authority, may be used in lieu of natural topsoil.

ii. Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative

i. When topsoiling, maintain needed erosion and sediment control practices such as diversions, rade Stabilization Structures, Earth Dikes, Slope Silt Fence and Sediment Traps and Basins.

ii. Grades on the areas to be topsoiled, which have been previously established, shall be maintained, albeit 4" - Θ" higher in elevation.

iii. Topsoil shall be uniformly distributed in a 4" - 8" layer and lightly compacted to a minimum thickness of 4". Spreading shall be performed in such a manner that sodding or seedine can proceed with a minimum of additional soil preparation and tillage. Any irregularities in the surface resulting from topsoiling or other operations shall be corrected in order to prevent the formation of depressions or water pockets.

iv. Topsoil shall not be placed while the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet or in a condition that may otherwise be detrimental to proper grading and seedbed preparation

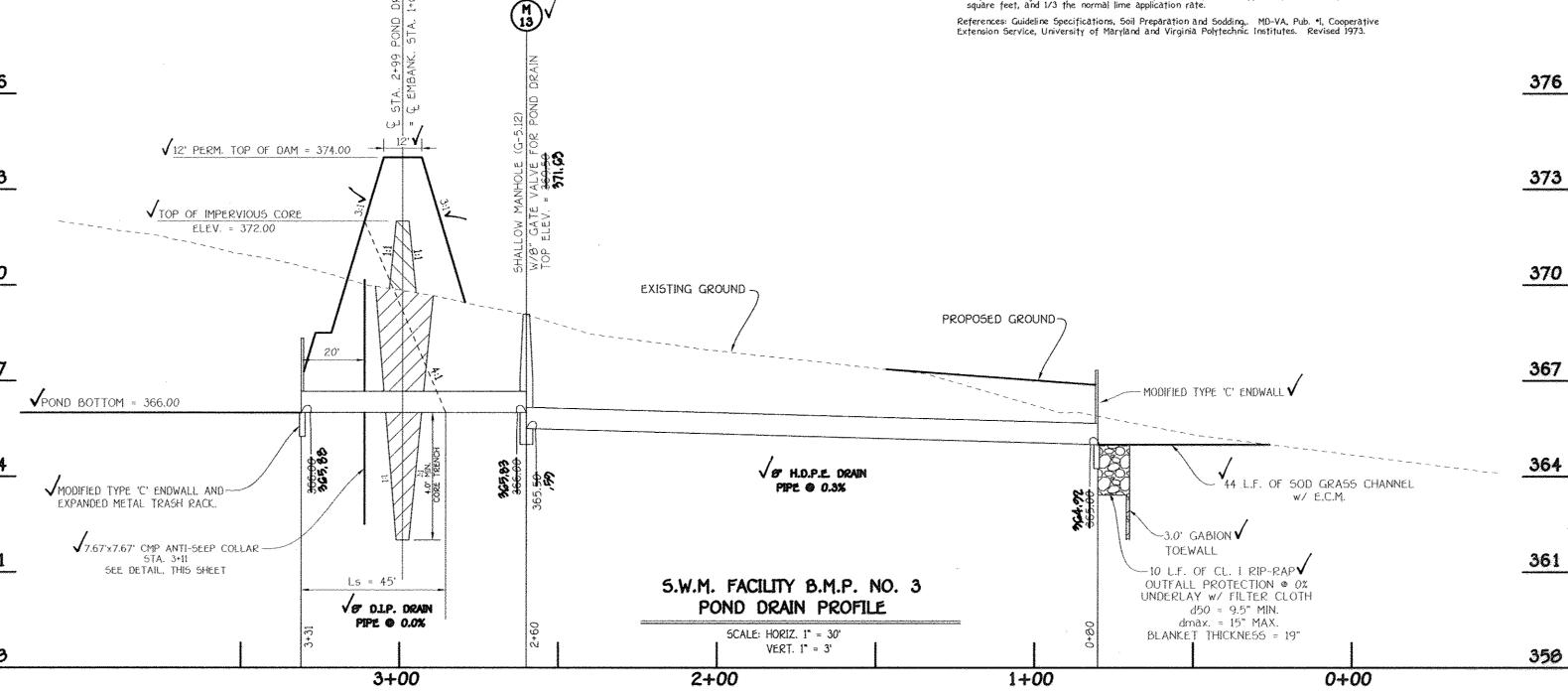
Alternative for Permanent Seeding - Instead of applying the full amounts of lime and commercial fertilizer, composted sludge and amendments may be applied as specified below: i. Composted Sludge Material for use as a soil conditioner for sites having disturbed areas over

acres shall be tested to prescribe amendments and for sites having disturbed areas under 5 acres shall conform to the following requirements:

a. Composted sludge shall be supplied by, or originate from, a person or persons that are permitted (at the time of acquisition of the compost) by the Maryland Department of the Environment under COMAR 26.04.06.

b. Composted sludge shall contain at least I percent nitrogen, 1.5 percent phosphorus, and 0.2 percent potassium and have a Ph of 7.0 to 0.0. If compost does not meet these requirements, the appropriate constituents must be added to meet the requirements prior to use.

c. Composted sludge shall be applied at a rate of 1 ton/1,000 square feet. ed sludge shall be amended with a potassium fertilizer applied at the rate of 4 lb/1.000 square feet, and 1/3 the normal time application rate.



OWNER

BASSLERS, INCORPORATED

C/O ALFRED S. BASSLER

4994 SHEPPARD LANE

ELLICOTT CITY, MARYLAND 21042 (410) 531-2193

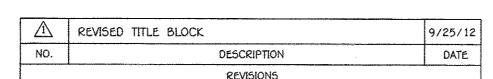
DEVELOPER

HERITAGE LAND DEVELOPMENT

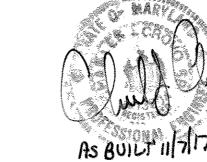
15950 NORTH AVE.

LISBON, MARYLAND 21765

(410) 489-7900



REVISIONS



THEREON CORTEN, BY MY GEAL, THAT THE FACILITIES GLOWN ON THIS PLAN WOOD CONFIDENCES AS GLOWN a) THE "ME-BUILT" PLAN MICET THE APPROVED PLANS AND GROUPICATIONS. CHARLES J. CRONG GR. MENO, 19204 AG-BUILT 11/7/17

GABION INFLOW PROTECTION NOT TO SCALE

STANDARD SYMBOL GM PERSPECTIVE VIEW

PROFILE ALONG CENTERLINE

Construction Specifications

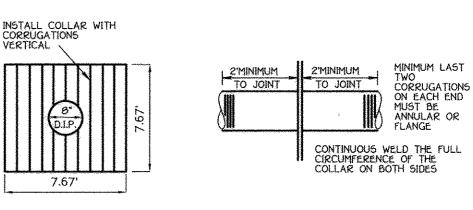
1. Gabion inflow protection shall be constructed of 9' x 3' x 9" gabion baskets forming a trapezoidal cross section 1' deep, with 2:1 side slopes, and a 3' bottom width.

2. Geotextile Class C shall be installed under all gabion baskets.

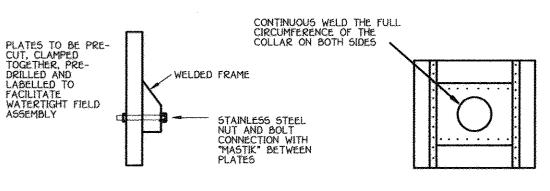
3. The stone used to fill the gabion baskets shall be 4" - 7". 4. Gabions shall be installed in accordance with manufacturers

5. Gabion Inflow Protection shall be used where concentrated flow is present on slopes steeper than 4:1.

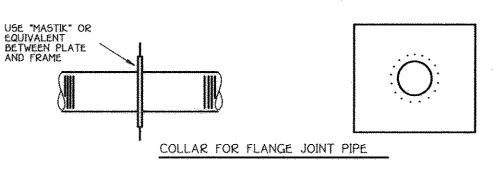
TYPICAL ANTI-SEEP COLLARS



COLLAR WELDED IN PLACE ON BARREL SECTION



ANTI-SEEP COLLAR DESIGN



SEDIMENT AND EROSION CONTROL DETAILS

PHASE TWO Lots 23 - 68, Non-Buildable Preservation Parcels 'C', 'G', 'I', 'J', 'K', 'L' And 'M' & Buildable Bulk Parcels 'H' And 'N' (Being A Resubdivision Of Buildable Bulk Parcels 'F' & 'E' And a Revision To Non-Buildable Preservation Parcel 'C' - Walnut Creek, Phase One, Plat No's. 20631 Thru 20647 ZONED: RC-DEO & RR-DEO

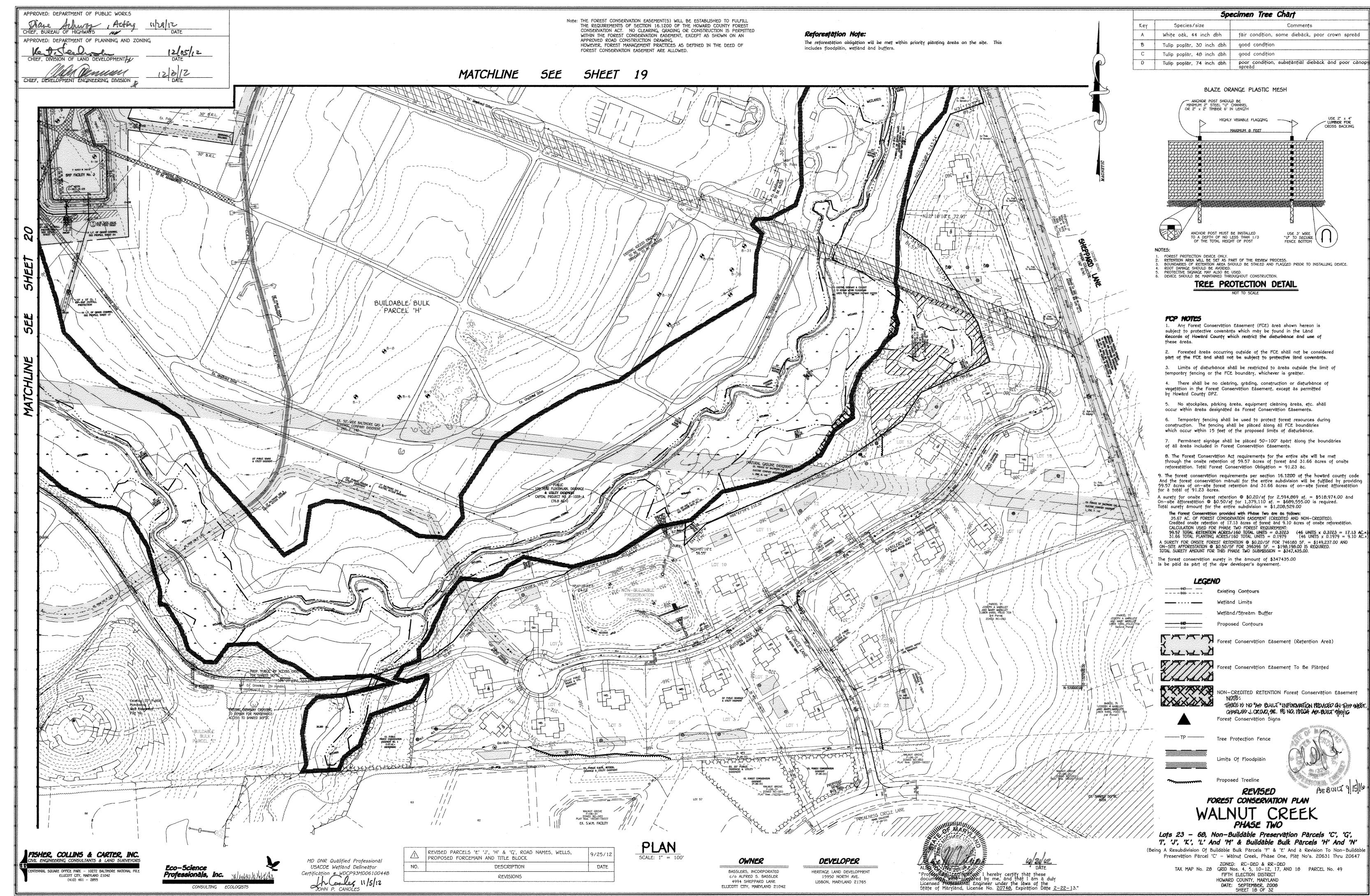
GRID Nos. 4, 5, 10-12, 17, AND 18 PARCEL No. 49 FIFTH ELECTION DISTRICT HOWARD COUNTY, MARYLAND DATE: SEPTEMBER, 2008

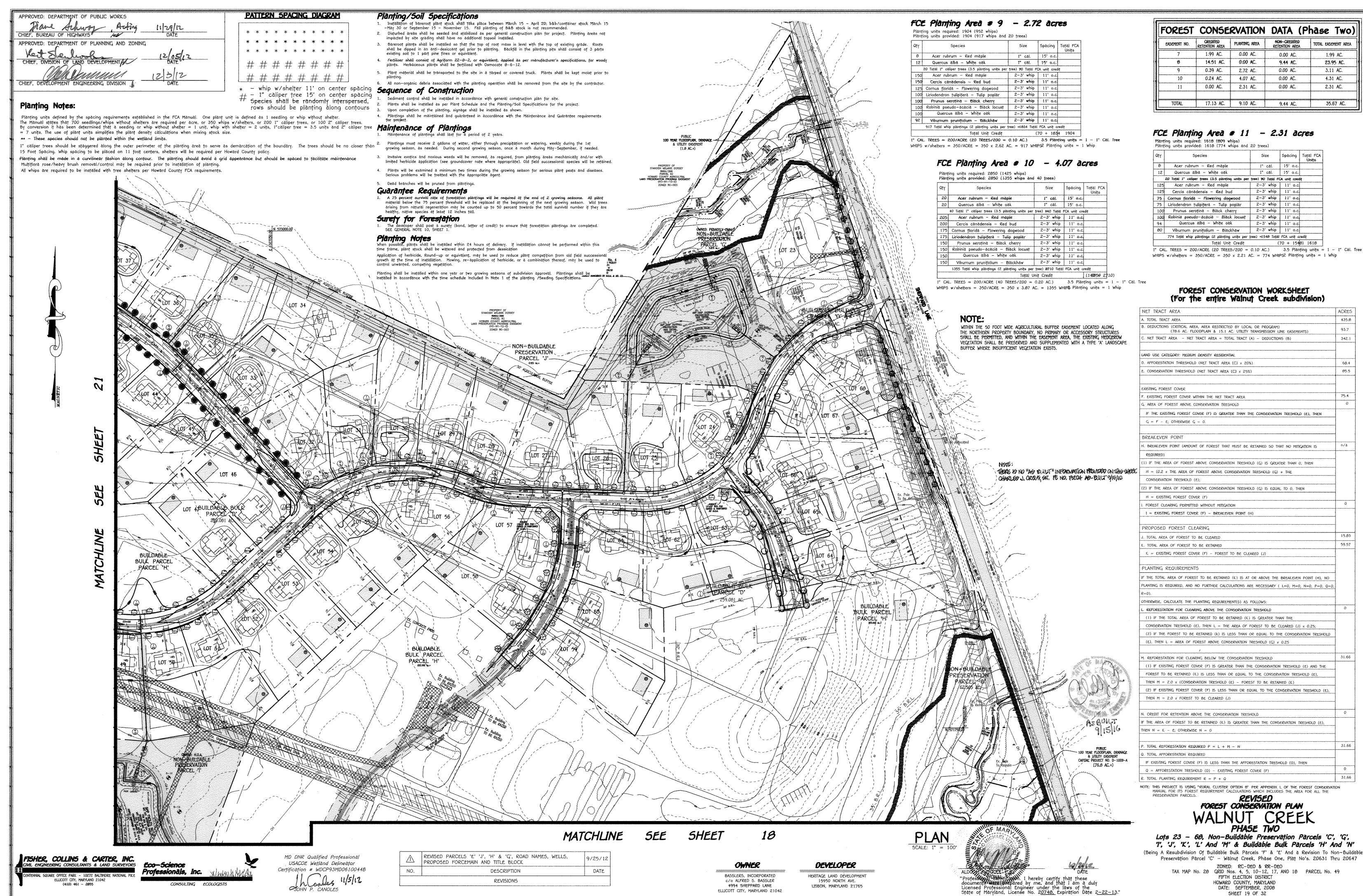
FISHER, COLLINS & CARTER, INC.

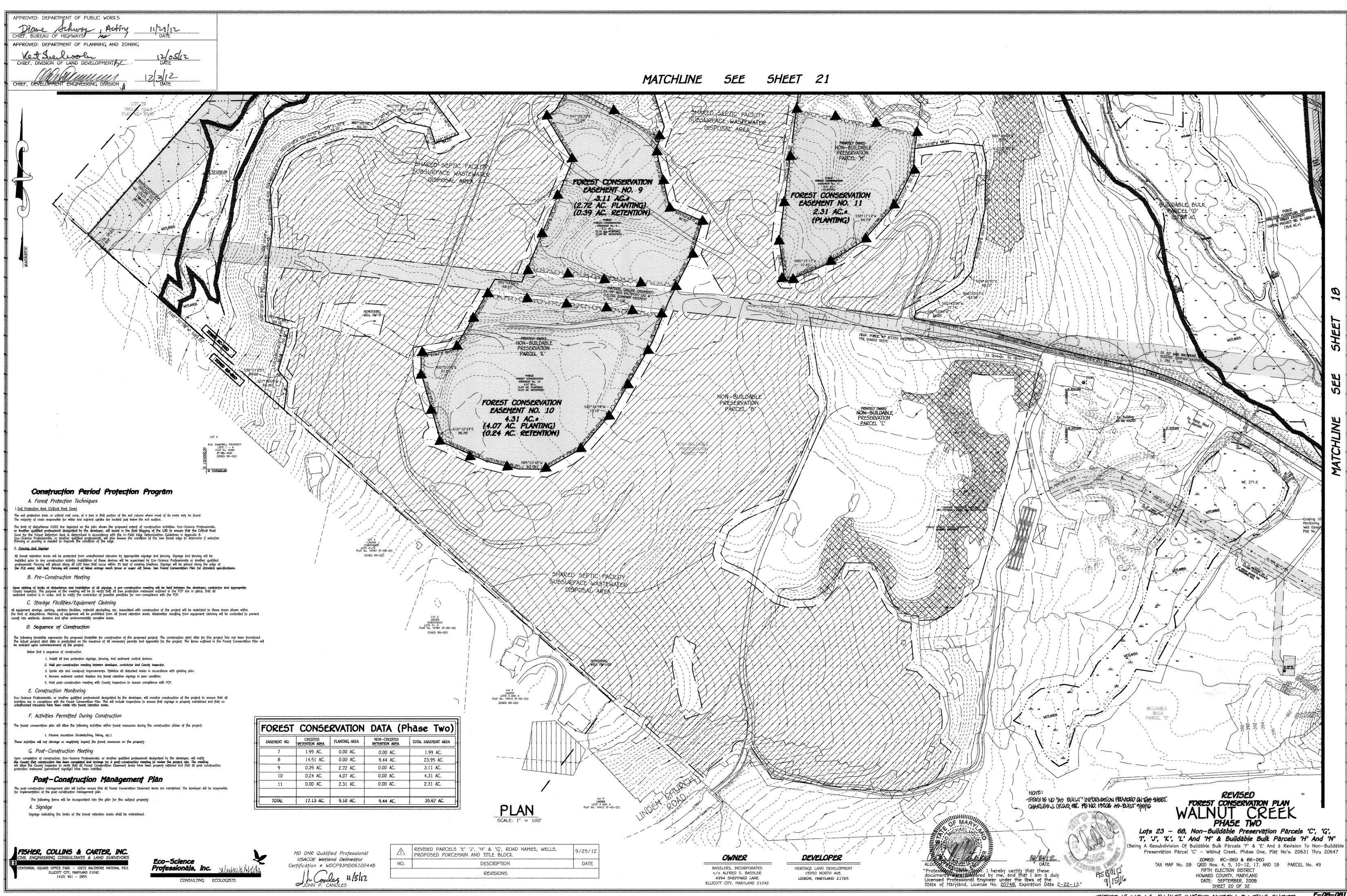
TENNIAL SQUARE OFFICE PARK - 10272 BALTIMORE NATIONAL PI

ELLICOTT CITY, MARYLAND 21042

(410) 461 - 2855

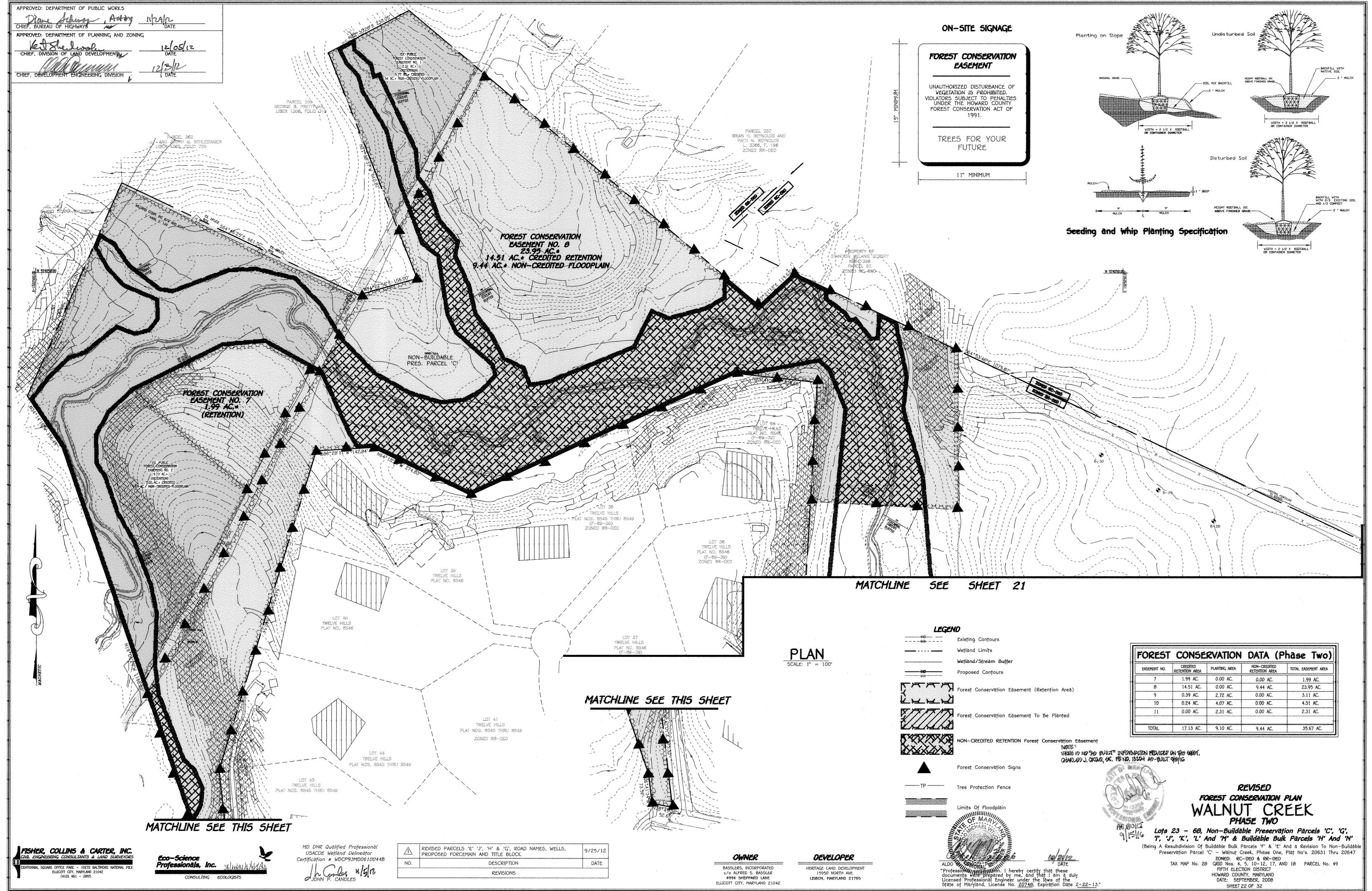








THERE IS NO 49-BUILT INFORMATION ON THIS SHEET



These specifications are appropriate to all ponds within the scope of the Standard for practice MD-376. All references to ASTM and AASHTO specifications apply to the most recent version.

Site Preparation

Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the toe of the embankment.

Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet structure shall be cleared.

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

EARTH FILL

Material - The fill material shall be taken from approved designated borrow areas. It shall be free of roots, stumps, wood, rubbish, stones greater than 6", frozen or other objectionable materials. Fill material for the center of the embarkment, and cut off trench shall conform to Unified Soil Classification GC. SC. CH. or CL and must have at least 30% passing the *200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer. Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment.

Placement - Areas on which fill is to be placed shall be scarified prior to placement of fill. Fill materials shall be placed in maximum 8-inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment.

Compaction - The movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not be so wet that water can be squeezed out

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within +2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

Cut Off Trench - The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability.

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition, the core shall be placed concurrently with the outer shell of the embankment.

Structure Backfill

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2,000 ohm-cm. Material shall be placed such that a minimum of 6" (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sides of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a structure or pipe unless there is a compacted fill of 24" or greater over the structure or pipe. Backfill material outside the structural backfill (flowable fill) zone shall be of the type and quality conforming to the specified for the core of the embankment or other embankment materials.

Pipe Conduits

All pipes shall be circular in cross section.

Corrugated Metal Pipe - All of the following criteria shall apply for corrugated

1. Materials - (Polymer Coated steel pipe) - Steel pipes with polymeric coatings shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 & M-246 with watertight coupling bands or flanges. Materials - (Aluminum Coated Steel Pipe) - This pipe and its appurtenances shall

conform to the requirements of AASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Stel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability. shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.

Materials - (Aluminum Pipe) - This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-196 or M-211 with watertight coupling banks or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be between 4 and 9.

2. Coupling bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at least 24 mils in

3. Connections- All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be

FISHER, COLLINS & CARTER, INC. NNIAL SQUARE OFFICE PARK - 10272 BALTIMORE NATIONAL PIL

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All connections shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24-inches in diameter: flanges on both ends of the pipe with a circular 3/8 inch closed cell neoprene gasket, prepunched to the flange bolt circle, sandwiched between adjacent flanges; a 12-inch wide standard lap type band with 12-inch wide by 3/8-inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2-inch greater than the corrugation depth. Pipes 24-inches in diameter and larger shall be connected by a 24-inch long annular corrugated band using a minimum of 4 (four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12-inches on the end of each pipe. Flanged joints with 3/8-inch closed cell gaskets the full width of the

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or a neoprene bead.

4. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide

5. Backfilling shall conform to "Structure Backfill". 6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced

1. Materials - Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C-361.

2. Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in the "Stucture Backfill" section of this standard. Gravel bedding is not permitted.

3. Laying pipe - Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

4. Backfilling shall conform to "Structure Backfill". 5. Other details (Anti-seep collars, valves, etc.) shall be as shown on the drawings.

Plastic Pipe

The following criteria shall apply for plastic pipe:

1. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" - 10" inch pipe shall meet the requirement of AASHTO M252 Type 5, and 12" through 24" inch shall meet the requirement of AASHTO M294 Type 5.

2. Joints and connections to anti-seep collars shall be completely watertight.

3. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide

4. Backfilling shall conform to "Structure Backfill".

5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Drainage Diaphragms - When a drainage diaphragm is used, a registered professional engineer will supervise the design and construction inspection. Concrete

Concrete shall meet the requirements of Maryland Department of Transportation,

State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3. Rock Riprap

Rock riprap shall meet the requirements of Maryland Department of Transportation,

Materials, Section 311. Geotextile shall be placed under all riprap and shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard

State Highway Administration Standard Specifications for Construction and

Specifications for Construction and Materials, Section 921.09, Class C. Care of Water during Construction

All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees. cofferdams, drainage channels, and stream diversions necessary to protect the areas to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the excavations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the extent that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall

Stabilization

All borrow areas shall be graded to provide proper drainage and left in a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

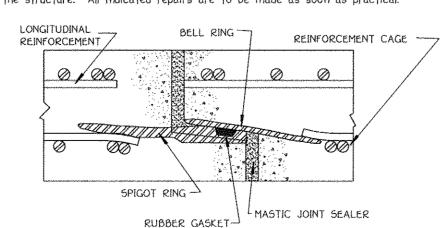
Erosion and Sediment Control

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and

OPERATION AND MAINTENANCE

sediment control measures.

An operation and maintenance plan in accordance with Local or State Regulations will be prepared for all ponds. As a minimum, the dam inspection checklist located in Appendix A shall be included as part of the operation and maintenance plan and performed at least annually. Written records of maintenance and major repairs needs to be retained in a file. The issuance of a Maintenance and Repair Permit for any repairs or maintenance that involves the modification of the dam or spillway from its original design and specifications is required. A permit is also required for any repairs or reconstruction that involve a substantial portion of the structure. All indicated repairs are to be made as soon as practical.



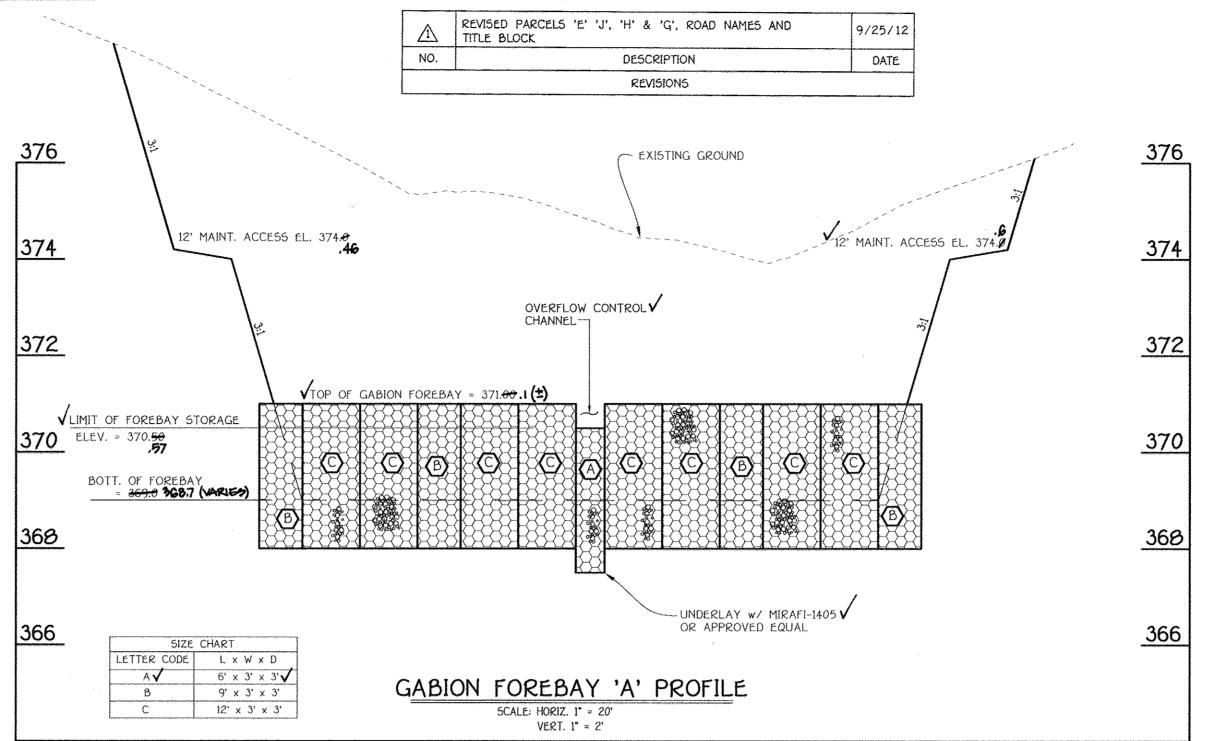
NOTE: PROVIDE MASTIC JOINT SEALER FROM OUTSIDE OF PIPE

JOINTS PRIOR TO INSTALLING BARREL UNDERGROUND

CONCRETE PIPE JOINT DETAIL PRESSURES TO 125 FEET OF HEAD

ASTM DESIGNATION C361

DIAMETERS 12 THRU 160 INCH



5WM Pond Construction Recommendations By The Developer: A. General Design Recommendations "I/We Certify That All Development And/Or Construction Will Be Done According To These Plans, It is recommended that the geotechnical aspects of the SWM pond design And That Any Responsible Personnel Involved In The Construction Project Will Have A and construction be in accordance with MD 378/2000 specifications. Certificate Of Attendance At A Department Of The Environment Approved Training Program For The Control Of Sediment And Erosion Before Beginning The Project. I Shall Engage A Registered Professional Engineer To Supervise Pond Construction And Provide The Howard Soil Variable soil conditions are present and, consequently, the bearing Conservation District With An "As-Built" Plan Of The Pond Within 30 Days Of Completion. 1 conditions at various elevations vary widely. If appears that the majority of Also Authorize Periodic On-Site Inspections By The Howard Soil Conservation District." the underlying native soils will be acceptable to support design bearing pressures of 2 KSF. Open footings excavated below the natural water table will be subject to disturbance by the upward flow of water and will require pumping and stabilization for acceptability C. Earth Slope Stability Printed Name Of Developer Cut slopes located beyond the dam embankment zones shall be evaluated in the field at time of construction for conditions which may cause slope instability, including high plasticity clay soils with little sand component or groundwater seepage above the toe of slope. Remedial measures may include but not limited to any of the following: "I Certify That This Plan For Pond Construction, Erosion And Sediment Control Represents A 1. Flatten the cut slopes to 4:1. Practical And Workable Plan Based On My Personal Knowledge Of The Site Conditions. This Plan Was Prepared In accordance With The Requirements Of The Howard Soil Conservation District. Was Prepared to Accordance With The Requirements Of The Howard Soil Conservation District.

I Have Notified The Developer That He/She Must Engage A Registered Professional Engineer To Supervise People Construction And Provide The Howard Soil Conservation District With An "As-Built Plana". The Pend Within 30 Days Of Completion."

1046-00 2. Provide toe and slope drains to control any groundwater. The design, location and spacing to be determined by the Geotechnical Engineer at the time of grading, as needed. 3. Undercut and replace unstable soils as directed by the Geotechnical Engineer The type and extent of remedial work will be addressed at the time of construction. D. Core Trench Core trenches shall be excavated to the typical MD 378/2000 specified dimensions below stripped existing grade or at least 2 feet into original soils below any undercut backfill, whichever depth is greater. Depending upon the design elevations, excavations below the water table may result in instabilities of the core trench walls and bases requiring pumping and These Plans For Small Pond Construction, Soil Erosion And Sediment Control Meet The stabilization for acceptable installation of the core trench backfill Requirements Of The Howard Soil Conservation District. E. Permanent Ground Water Control Ground water should be anticipated in excavations carried below the Howard Soil Conservation Distric water levels indicated by the borings. Also, since the boring program was performed in extended dry weather, it is likely that ground water levels will rise in the wetter seasons requiring additional ground water control. Approved: Department Of Public Works addition to slope drains for cut soils, it may be necessary to construct pilot channels in the basins to direct seepage into low flow orifices to prevent shallow accumulation of water. Chief Bureau Of Highways 🖊 TOP OF DAM = 374.00 Approved: Department Of Planning And Zoning PERMANENT FLEV =372.00 Chief Development Engineering Division TYPICAL SECTION THRU EMERGENCY SPILLWAY AS-BUILT CERTIFICATION NOT TO SCALE Hereby Certify That The Facility Shown On This Plan Was Constructed As Shown On Them As-Built" Plans And Meets The Approved Plans And STD. HOWARD COUNTY V

TOP ELEV. = 373.75

BACKFILL W/IMPERVIOUS COMPACT TO ASSURE 95% DENSITY ~ EX. GROUND -ADDITIONAL WALL CONNECTION ~ 4.0" TOUNGE AND GROOVE SHALL BE KEPT OVERLAP PUMPED DRY PROVIDE ASPHALT-CONSTRUCTION. JOINT FILLER MATERIAL RISER WALL -No. 6 DOWELS BENT AS SHOWN CORE TRENCH DETAIL NOT TO SCALE KEYED JOINT DETAIL WALL SECTION TO WALL SECTION

MAX. 2' STUB WITH WATER PROVIDE 3.0' x 3.0' x 0.5' LBRICK WEIR TO EL. 371.25 TIGHT CONNECTION -CONCRETE SUPPORT COLLAR TO BE MONOLITHICALLY OR BASIN USI POURED WITH THE RISER AT THE PLANT 3.25' x 5.00' 2nd STAGE WEIR ELEV. = 370:00 (NORTH SIDE) - WATERSTOP AND STIFFENER RING **√**24" A5TM C-361, B-25 © 1.47% CONCRETI CRADLE KEY JOINT JOINT SEATING CONTRACTOR COMPOUND (TYP. CONTRACTOR WILL HAVE TO ADJUST GRADE AT RISER TO PROVIDE A MINIMUM OF ONE FOOT (1') CLEARANCE BETWEEN TRASH RACKS AND PROPOSED GRADE. L1" CLEAR PROFILE VIEW A No. 4 BARS CONCRETE RISER DETAIL

5CALE : 1" = 2"

FRAME AND COVER (G-5,51)

Certify Means To State Or Declare A Professional Opinion Based Upon Onsite Inspections And Material Tests Which Are Conducted During Construction. The Onsite Inspections And Material Tests Are Those inspections And Tests Deemed Sufficient And Appropriate Commonly Accepted Engineering Standards. Certify Does Not mean Or Imply A Guarantee By The Engineer Nor Does An Engineer's Certification Relieve Any Other Party From Meeting Requirements Imposed By Contract, Employment, Or Other Means, Including Meeting Commonly Accepted Industry Practices.

WHEN CONVERTING SEDIMENT BASIN TO

PERM. POND, CUT 10" PVC PIPE OFF AT

WEST

F-08-08

RISER AND INSTALL A 3.5" ORIFICE

PLATE AT INV. 369,00

*NOTE:

10-28-08

EXISTING GROUND المنافع المناف OVERFLOW CONTROLY CHANNEL-V TOP OF GABION FOREBAY = 371.00 .1 (VARIES) ✓ LIMIT OF FOREBAY STORAGE ELEV. = 370.50 VBOTT. OF FOREBAY = 369.0.12 (VARIES) UNDERLAY W/ MIRAFI-1405 ✓ OR APPROVED EQUAL 366 SIZE CHART LETTER CODE L X W X D 6' x 3' x 3'√ 9' x 3' x 3' 12' x 3' x 3'

GABION FOREBAY 'B' PROFILE

VERT. 1" = 2"

OWNER

BASSLERS, INCORPORATED

c/o ALFRED 5. BASSLER

4994 SHEPPARD LANE

ELLICOTT CITY, MARYLAND 21042

DEVELOPER

HERITAGE LAND DEVELOPMENT

15950 NORTH AVE.

LISBON, MARYLAND 21765

SCALE: HORIZ. 1" = 20"

" o.c. (EACH WAY) CONC. CRADLE MAX. 2' STUB -WITH WATERTIGHT CONNECTION STD. HOWARD COUNTY FRAME AND COVER (G-5.51) ----TOP ELEV. = 373.75 LAP WHEN ABOVE PIPE STEPS (G-5.21) BRICK WEIR TO EL. 371.25 3.5" LOW FLOW ORIFIC EXPANDED METAL TRASH RACK FOR BASIN USE 369.00 368.77 SEE DETAIL, SHEET 26 3.25' x 5.00' 2nd STAGE WELL (NORTH SIDE) 369.50 NORTH DIA. LOW FLOW ORIFICE (E SIDE) STD. HO. CO. STEPS (G-5.21) **√** 6'~8" IHEREBY CERTIFY, BY MY SEAL, THAT THE PACILITIES SHOWN ON THYS PLAN WERE CONSTRUCTED AS SHOWN ON THIS "AS-BUILT" PLAN MEET THE APPROVED PLANS AND SPECIFICATIONS RISER PLAN VIEW CHARDES J. CROVO, SR. PENO. 13024 AS-BUILT 11/7/17 - KEY JOINT

10-16-00

10" PVC FOR BASIN USE: INV. 369.00%

CONTRACTOR NOTE: ∨ WHEN CONVERTING SEDIMENT BASIN TO PERM. POND, CUT 10" PVC PIPE OFF AT LI" CLEAR RISER AND INSTALL A 3.5" ORIFICE PLATE AT INV. 369.00 √6'-8" PROFILE VIEW B CONCRETE RISER DETAIL

documents were prepared by me, and that I am a duly

State of Maryland, License No. 20748, Expiration Date 2-22-09.

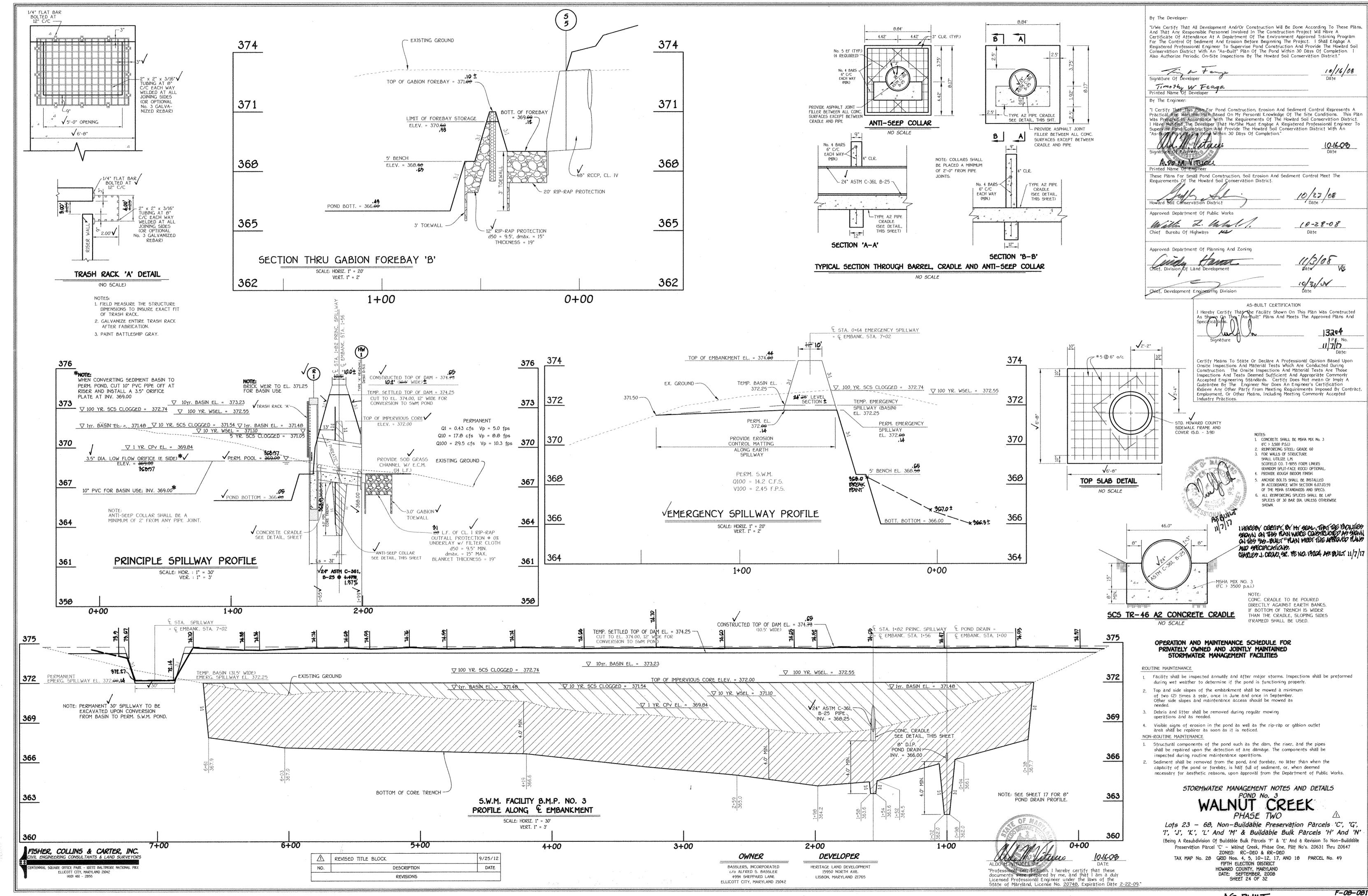
STORMWATER MANAGEMENT NOTES AND DETAILS POND No. 3

"I'. 'J'. 'K', 'L' And 'M' & Buildable Bulk Parcels 'H' And 'N' (Being A Resubdivision Of Buildable Bulk Parcels 'F' & 'E' And a Revision To Non-Buildable Preservation Parcel 'C' - Walnut Creek, Phase One, Plat No's. 20631 Thru 20647 ZONED: RC-DEO & RR-DEO

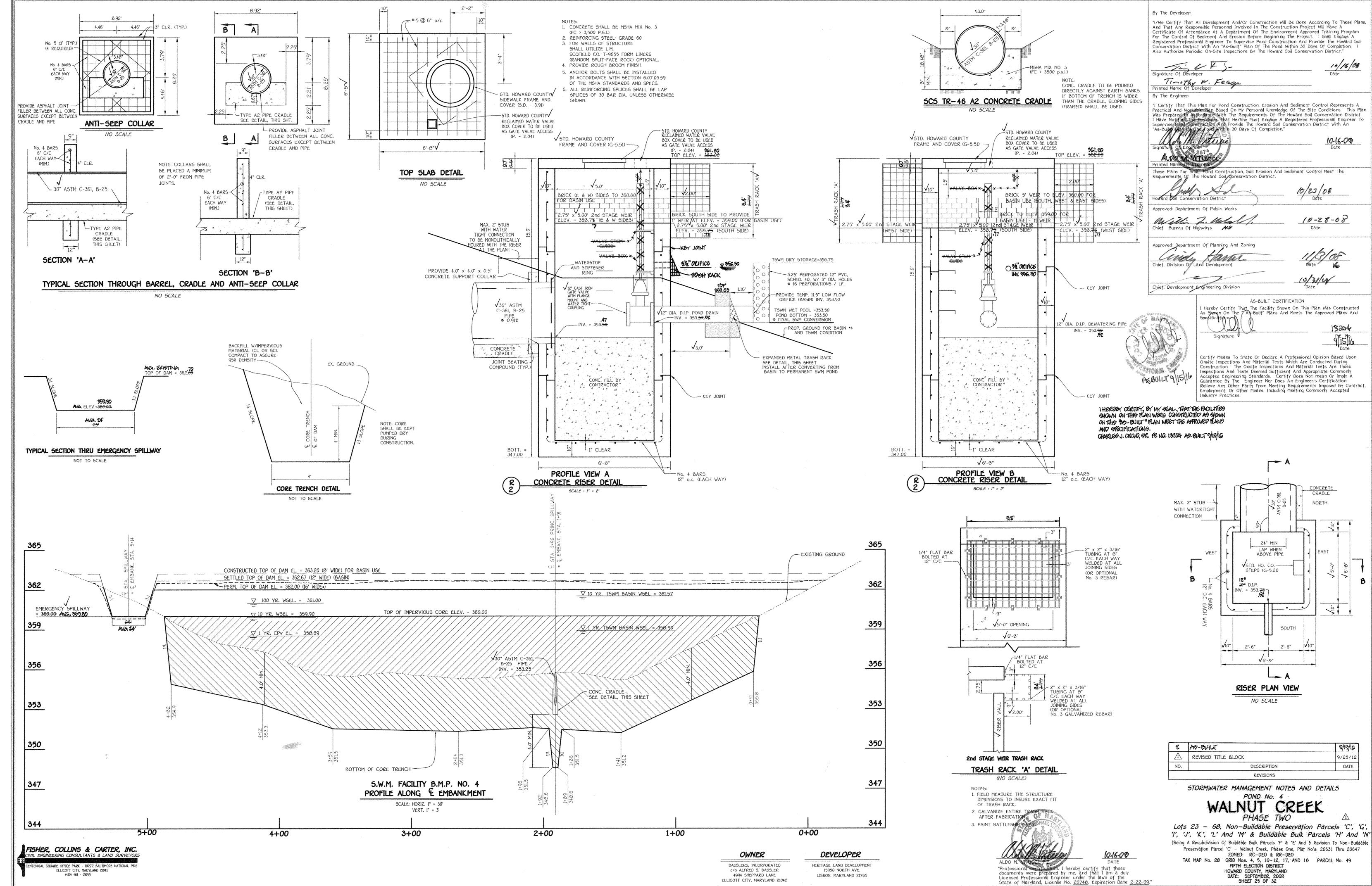
TAX MAP No. 28 GRID Nos. 4, 5, 10-12, 17, AND 18 PARCEL No. 49 FIFTH ELECTION DISTRICT HOWARD COUNTY, MARYLAND DATE: SEPTEMBER, 2008 SHEET 23 OF 32

AS-BUILT

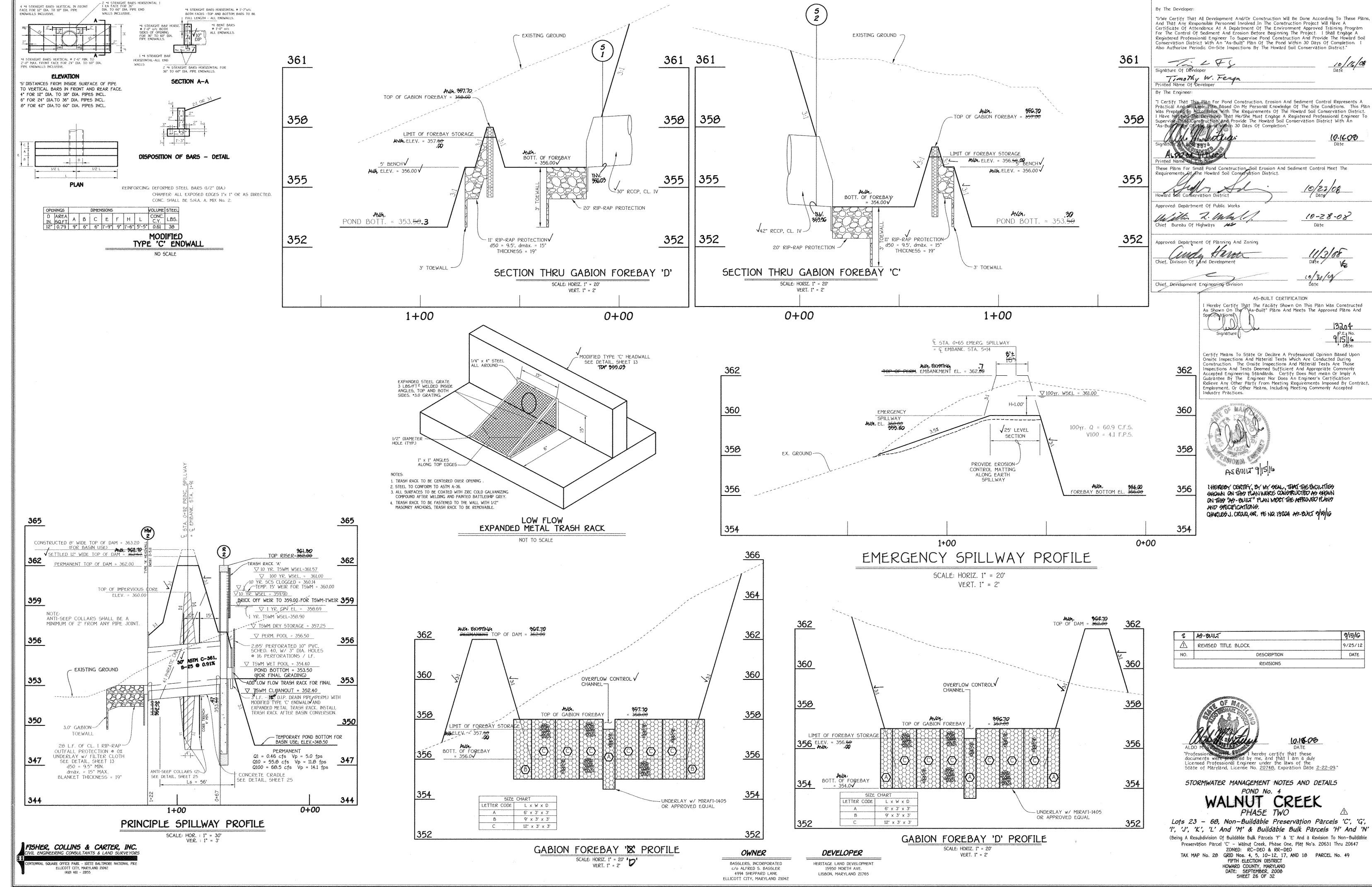
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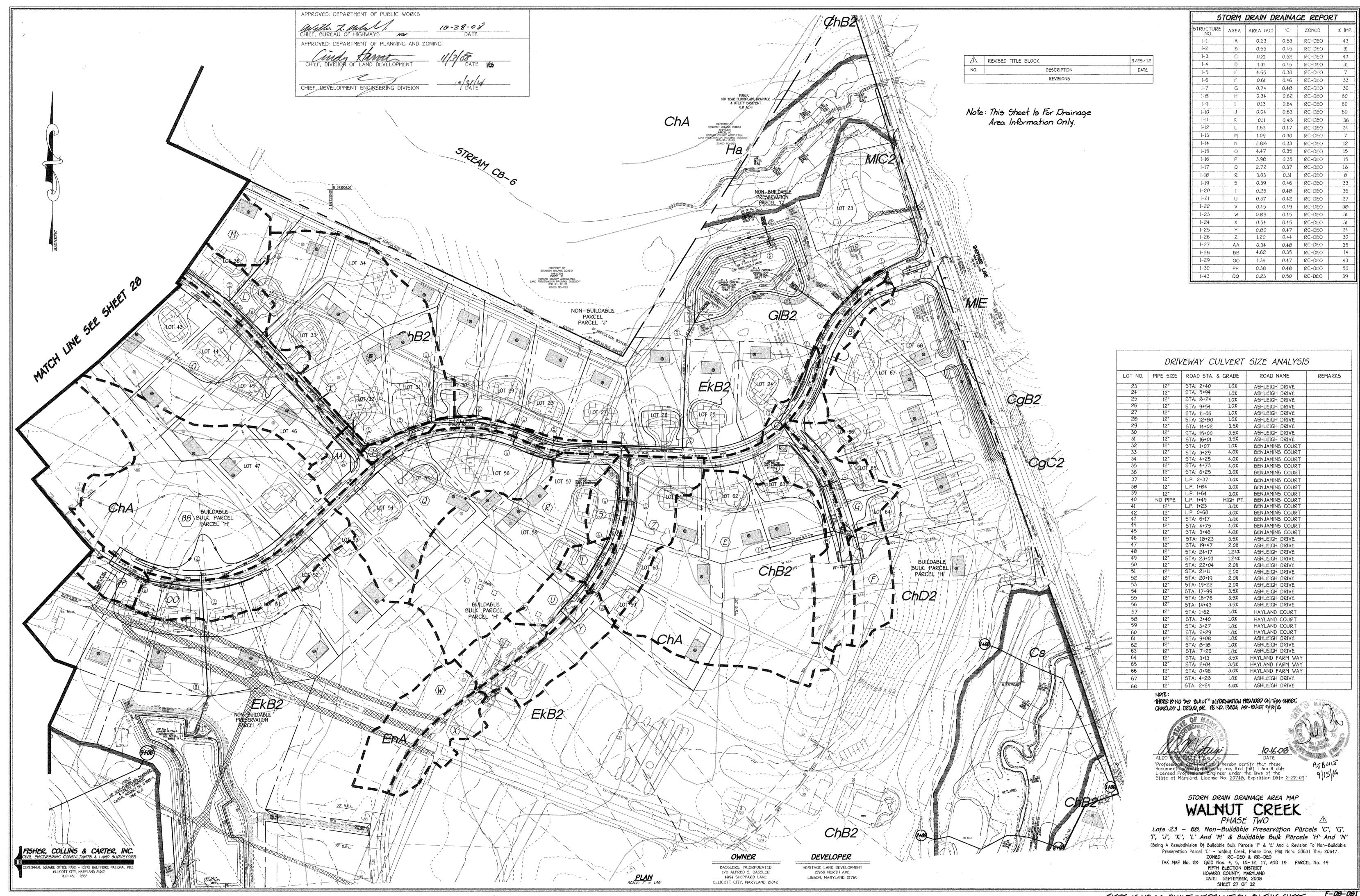
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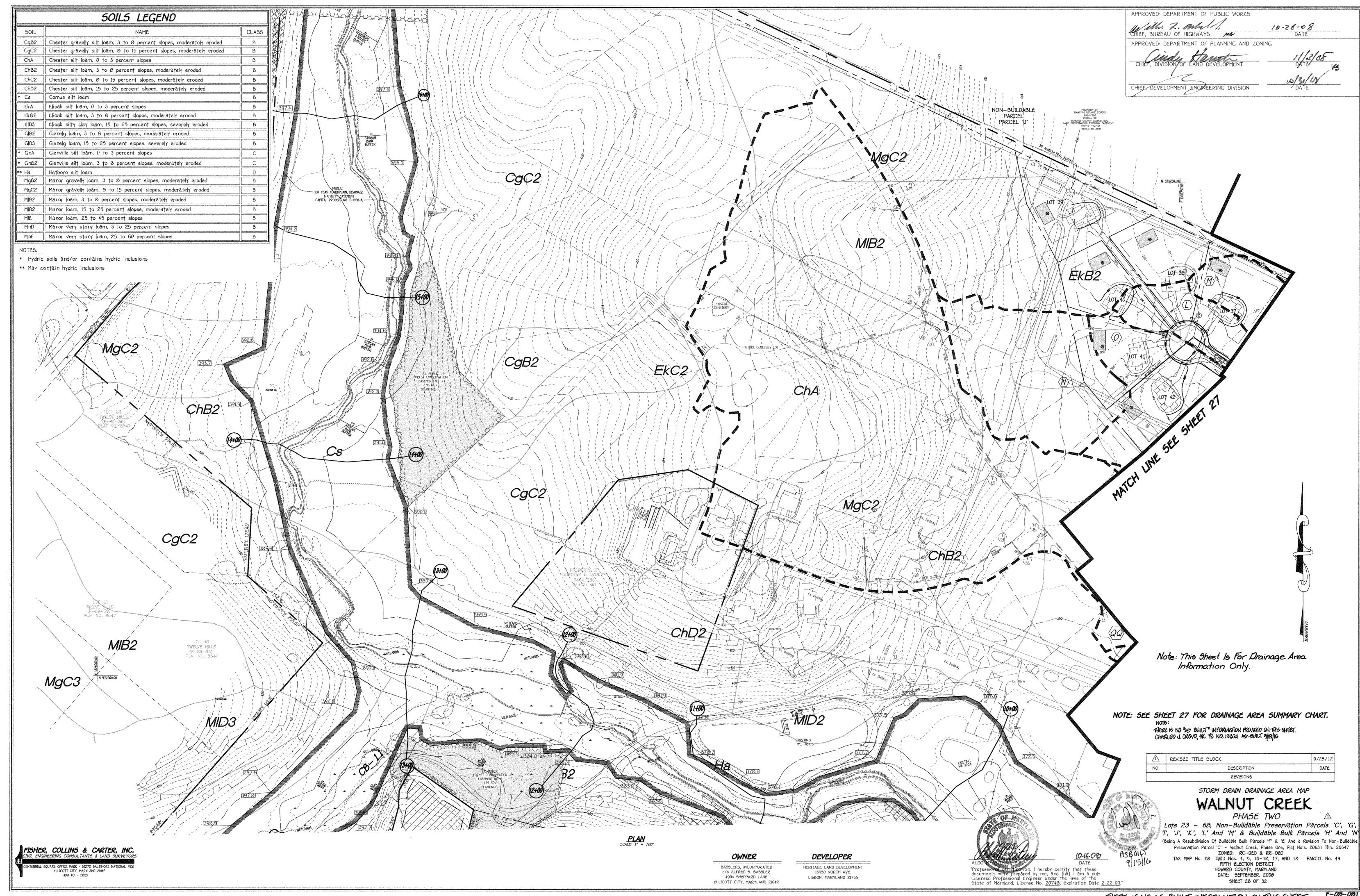


AS-BUILT

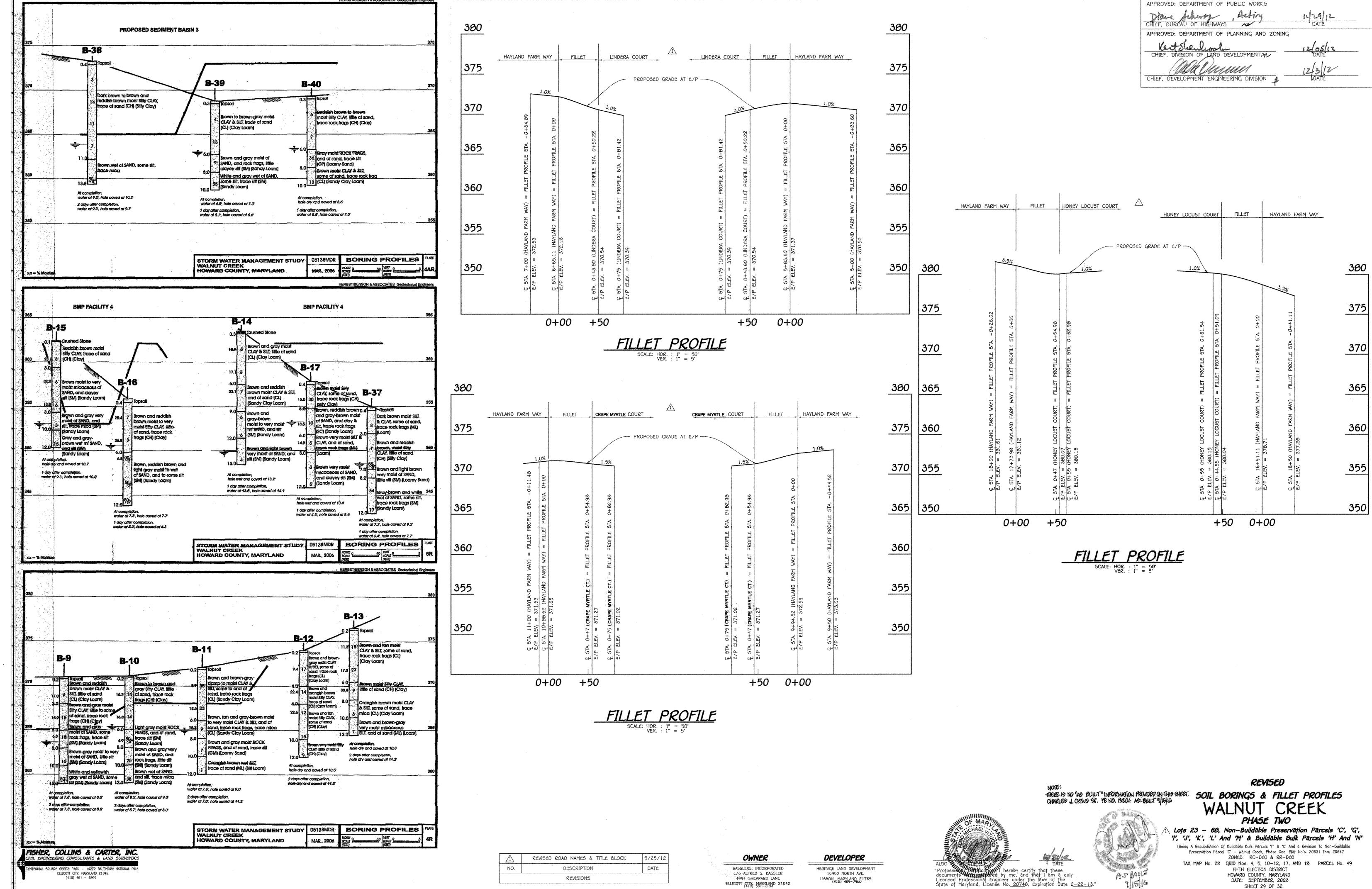


AG-BUILT



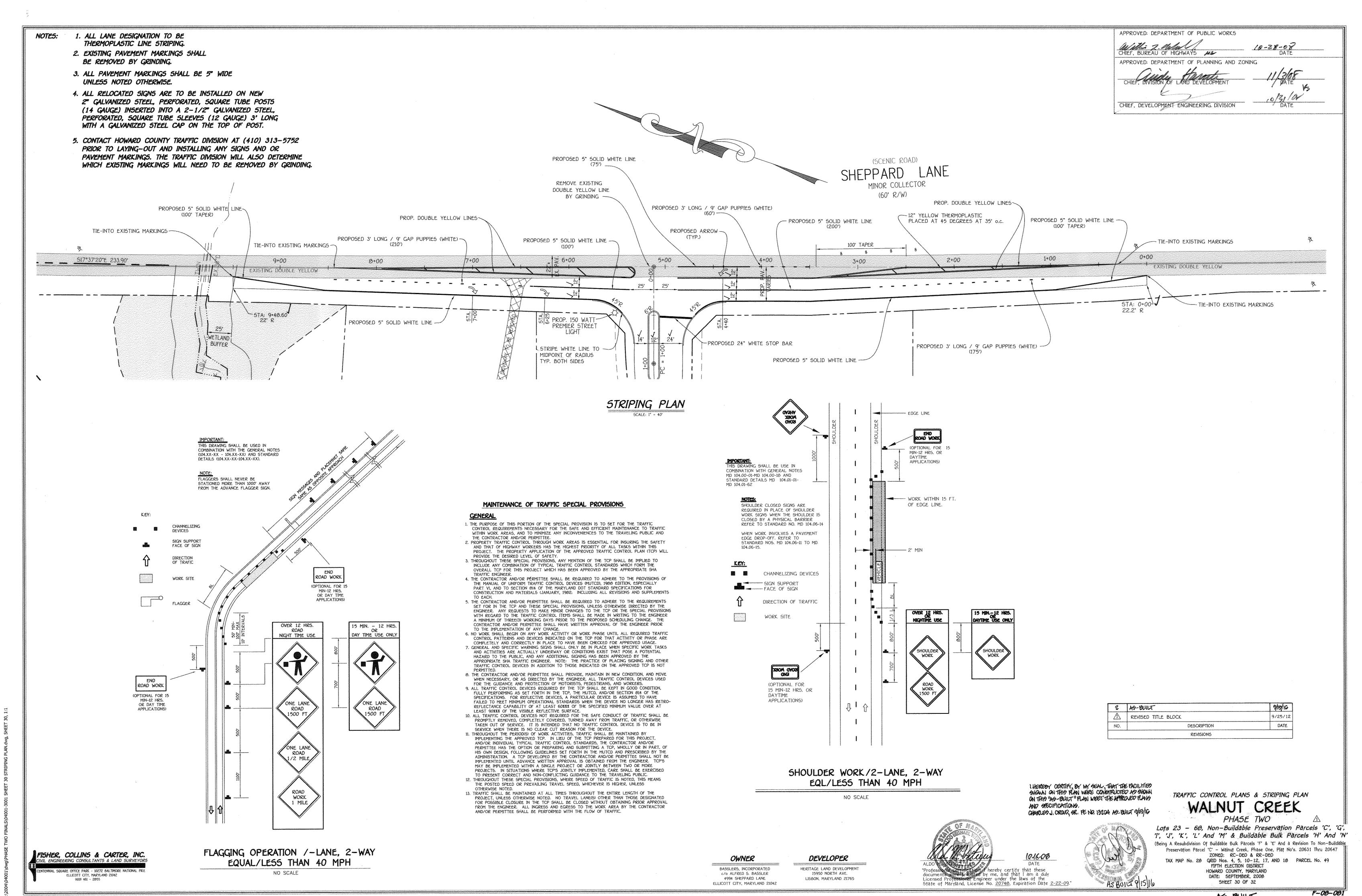


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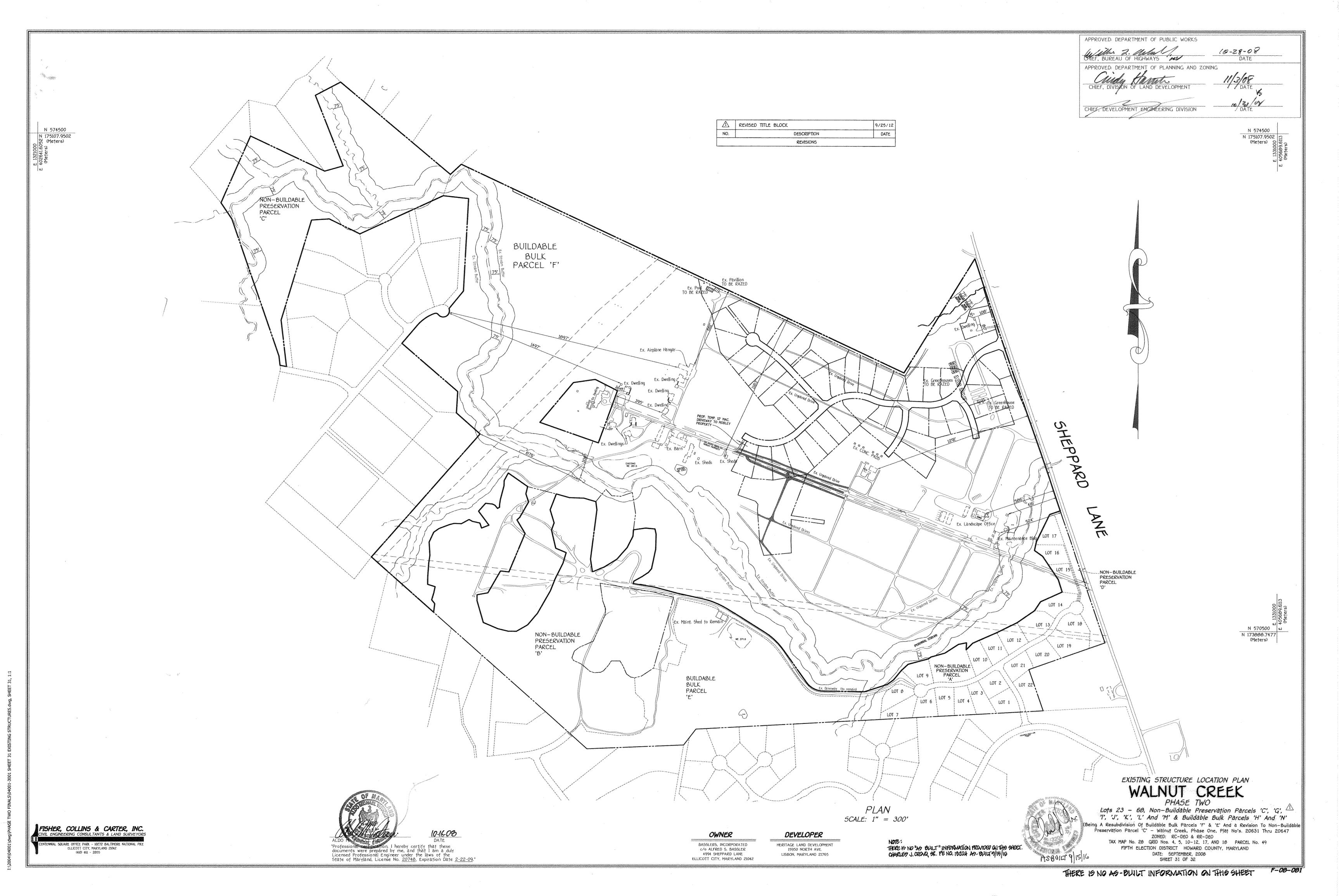


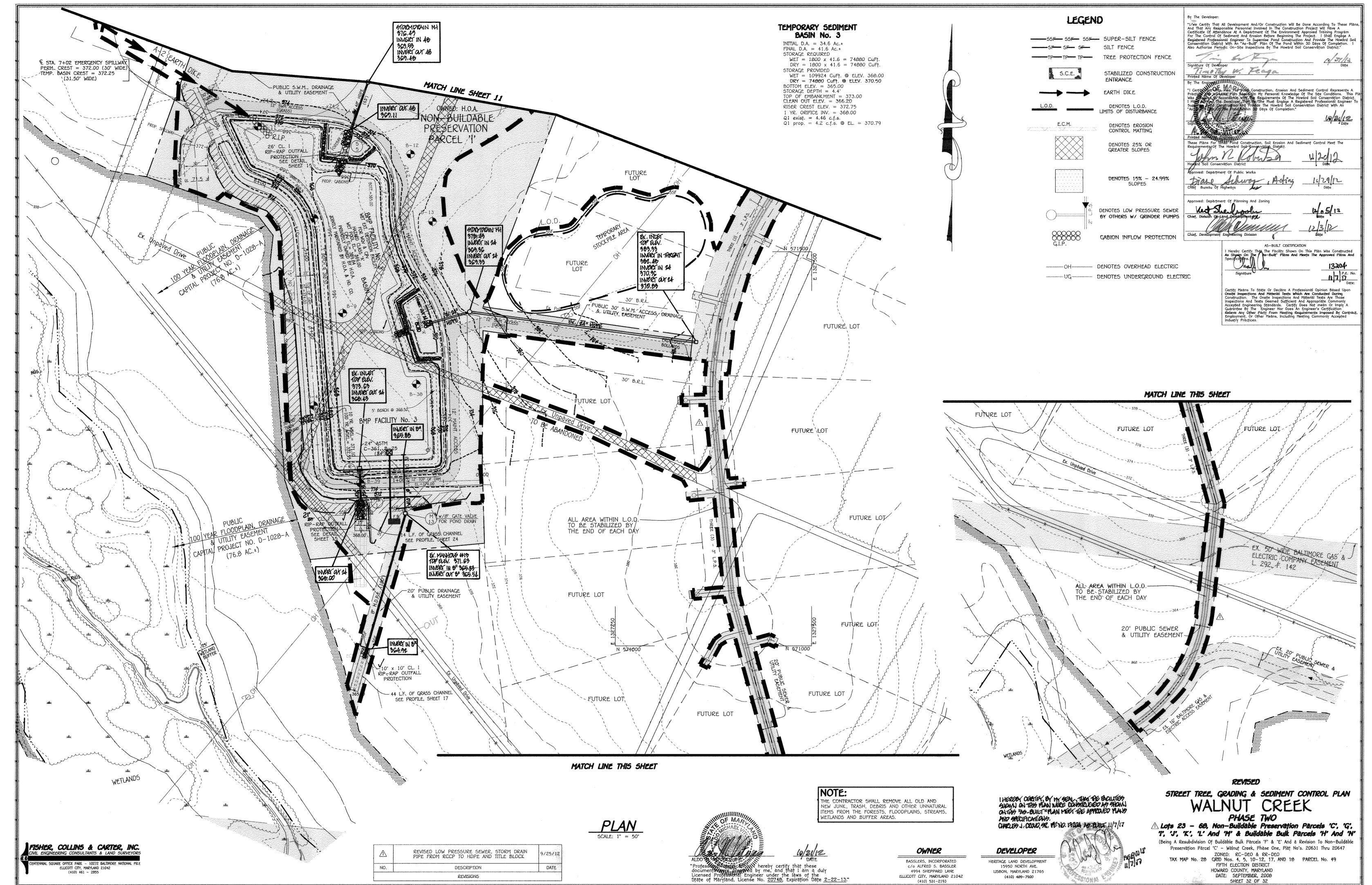
HERBST/BENSON & ASSOCIATES Geotechnical Engineer

08-081



AG-BUILT





AS-BUILT