# GENERAL NOTES

- ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST STANDARDS AND SPECIFICATIONS OF HOWARD COUNTY PLUS MSHA STANDARDS AND SPECIFICATIONS
- 2. THE CONTRACTOR SHALL NOTIFY THE DEPARTMENT OF PUBLIC WORKS/CONSTRUCTION INSPECTIONS DIVISION AT (410) 313-1880 AT LEAST FIVE (5) WORKING DAYS PRIOR TO THE START OF WORK.
- 3. THE CONTRACTOR SHALL NOTIFY THE FOLLOWING UTILITIES OR AGENCIES AT LEAST FIVE (5) DAYS PRIOR TO ANY EXCAVATION WORK:

MISS UTILITY 1-800-257-7777 C&P TELEPHONE COMPANY (410) 725-9976 HOWARD COUNTY BUREAU OF UTILITIES (410) 313-4900 AT&T CABLE LOCATION DIVISION (410) 393-3533 BALTIMORE GAS & ELECTRIC (410) 685-0123 (410) 531-5533 (410) 313-1880

STATE HIGHWAY ADMINISTRATION HOWARD COUNTY DEPT. OF PUBLIC WORKS/ CONSTRUCTION INSPECTION DIVISION

LOCATION: SIXTH ELECTION DISTRICT - TAX MAP 46 - PARCEL 229 & 352 - BLOCK 15 DEED REFERENCE: 3355/223 & 2508/188. ZONING: R-20

TOTAL TRACT AREA: 37.66 ACRES ± NUMBER OF PROPOSED LOTS: 71 (66 BUILDABLE) ACREAGE OF PROPOSED BUILDABLE LOTS: 21.68 ACRES ± OPEN SPACE REQUIRED: 30% OR 11.30 ACRES. OPEN SPACE PROVIDED: 11.47 ACRES (11.36 ACRES CREDITED). RECREATIONAL OPEN SPACE REQUIRED (66 UNITS X 200 SQ. FT.): 13,200 SQ. FT. (0.30 ACRES) RECREATIONAL OPEN SPACE PROVIDED: 21,346 SQ. FT. (13,200 SQ. FT. CREDITED). PROPOSED ROAD DEDICATION: 4.51 ACRES ± AREA OF 100 YEAR FLOODPLAIN : N/A DPZ REFERENCE #: F-00-169; SP-01-01; WP-01-65; S-01-081; P-05-03.

5. TWO FOOT CONTOUR TOPOGRAPHY AND EXISTING CONDITIONS BASED ON FIELD RUN TOPOGRAPHIC SURVEY BY MILDENBERG, BOENDER & ASSOCIATES, INC. IN APRIL 2000. OFF-SITE TOPOGRAPHY WEST OF THE PROPERTY LINE ON WSSC PROPERTY SUPPLEMENTED BY HOWARD COUNTY 200 SCALE TOPOGRAPHIC MAPS. BOUNDARY SHOWN HEREON BASED ON FIELD RUN MONUMENTED BOUNDARY SURVEY PERFORMED ON OR ABOUT MAY 2000 BY MILDENBERG, BOENDER & ASSOCIATES, INC.

6. COORDINATES BASED ON NAD '83 MARYLAND COORDINATE SYSTEM AS PROJECTED BY HOWARD COUNTY GEODETIC CONTROL STATIONS NO. 46EA & 46EB. N 536,185,423 ELEV. 415.10

STA. No. 46EB

E 1,338,091.710 N 534,750.221 E 1,337,742.800

ELEV. 413.24

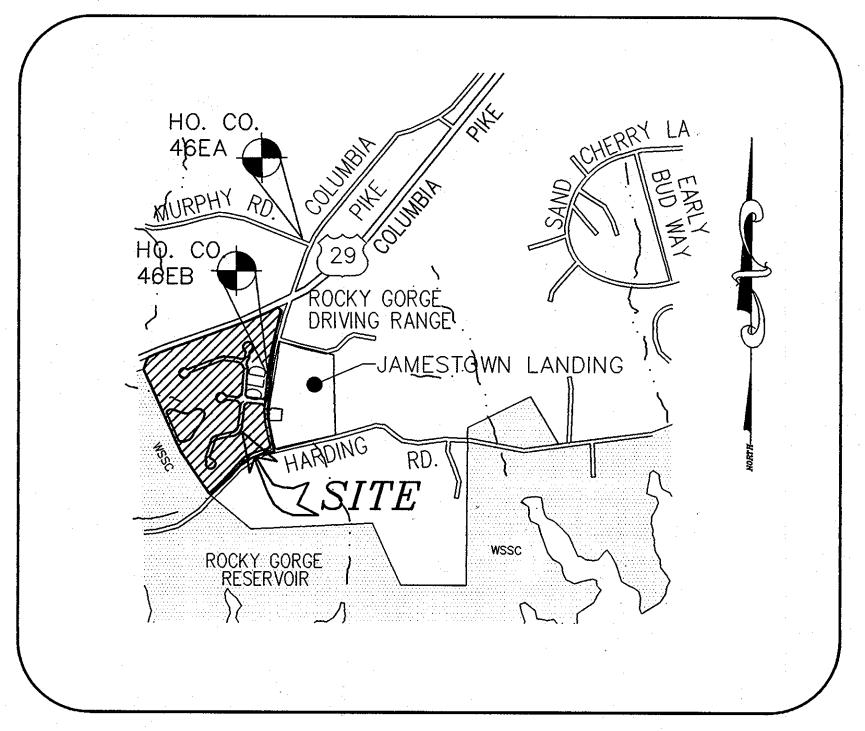
7. STREET LIGHTS WILL BE REQUIRED IN THIS DEVELOPMENT IN ACCORDANCE WITH THE DESIGN MANUAL STREET LIGHT PLACEMENT AND THE TYPE OF FIXTURE AND POLE SELECTED SHALL BE IN ACCORDANCE WITH THE LATEST HOWARD COUNTY DESIGN MANUAL, VOLUME III (1993) AND AS MODIFIED BY "GUIDELINES FOR STREET LIGHTS IN RESIDENTIAL DEVELOPMENTS (JUNE 1993). THE JUNE 1993 POLICY INCLUDES GUIDELINES FOR LATERAL AND LONGITUDINAL PLACEMENT. A MINIMUM SPACING OF 20' SHALL BE MAINTAINED BETWEEN ANY STREET LIGHT AND ANY TREE

- 8. WATER AND SEWER ARE PUBLIC BY EXTENSION OF CONTRACT #24-3904-D & 24-3905-D. 9. GEOTECHNICAL REPORT PREPARED BY GEO-TECHNOLOGY ASSOCIATES, INC. ON OCTOBER 20, 2000
- 10. TRAFFIC CONTROL DEVICES, MARKINGS, AND SIGNING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). ALL STREET AND REGULATORY SIGNS SHALL BE IN PLACE PRIOR TO THE PLACEMENT OF ANY ASPHALT.
- 11. COMPACTION IN FILL AREAS TO BE 95% AS DETERMINED PER AASHTO T-180.
- 12. CONTRACTOR TO VERIFY THE LOCATION OF ALL EXISTING UTILITIES ON SITE PRIOR TO COMMENCING CONSTRUCTION.
- 13. FOREST CONSERVATION EASEMENT(S) HAS BEEN ESTABLISHED TO FULFILL THE REQUIREMENTS OF SECTION 16.200 OF HOWARD COUNTY FOREST CONSERVATION ACT. NO CLEARING, GRADING OF CONSTRUCTION IS PERMITTED WITHIN THE FOREST CONSERVATION EASEMENT, EXCEPT AS SHOWN ON AN APPROVED ROAD CONSTRUCTION DRAWING OR SITE DEVELOPMENT PLAN. HOWEVER, FOREST MANAGEMENT PRACTICES AS DEFINED IN THE DEED OR CONSERVATION EASEMENT ARE ALLOWED.
- 14. FOREST CONSERVATION OBLIGATIONS IN ACCORDANCE WITH SECTION 16.1200 OF THE HOWARD COUNTY CODE AND FOREST CONSERVATION ACT FOR THIS SUBDIVISION HAS BEEN FULFILLED BY RETENTION OF 2.75 ACRES AND AFFORESTATION OF 4.65 ACRES. FINANCIAI SURETY FOR THE ON-SITE RETENTION OF 2.75 ACRES (119,790 SQ.FT.) IN THE AMOUNT OF \$23,958.00 AND AFFORESTATION OF 4.65 ACRES (202,554 SQ.FT.) IN THE AMOUNT OF \$101,277.00 HAS BEEN POSTED AS PART OF THE DPW DEVELOPER'S AGREEMENT IN THE AMOUNT OF \$125,235.00.
- 15. THIS PLAN HAS BEEN PREPARED IN ACCORDANCE WITH THE PROVISIONS OF SECTION 16.124 OF THE HOWARD COUNTY CODE AND THE LANDSCAPE MANUAL. FINANCIAL SURETY FOR THE REQUIRED LANDSCAPING (109 SHADE TREES, 118 EVERGREENS) HAS BEEN POSTED AS PART OF THE DPW DEVELOPERS AGREEMENT IN THE AMOUNT OF \$50,400.00.
- 16. PROPERTY IS LOCATED WITHIN THE METROPOLITAN DISTRICT.
- 17. GRAVES, IDENTIFIED AS CEMETERY SITE 46-4 ON THE HOWARD COUNTY CEMETERY INVENTORY, EXIST ON-SITE ON PROPOSED OPEN SPACE LOT 68 WHERE INDICATED. ON-SITE TESTING HAS BEEN PERFORMED TO DETERMINE LOCATION OF THE EXTENTS OF THE GRAVE SITES. THESE EXTENTS HAVE BEEN FIELD VERIFIED BY MILDENBERG. BOENDER & ASSOCIATES, INC. IN OCTOBER 2000. NO GRADING MAY BE CONDUCTED WITHIN 30 FEET OF THE LIMITS OF THE GRAVE SITES. THE PLANNING BOARD APPROVED THE ACCOMMODATION OF, AND ACCESS TO THE CEMETERY A SPECIAL SUBJECT ON JANUARY 24, 2001 UNDER S-01-08. THE DESIGN ON THESE PLANS IS CONSISTENT WITH THE APPROVED DESIGN.
- 18. ALL EXISTING STRUCTURES ON-SITE ARE TO BE REMOVED.
- 19. ALL STORM DRAIN PIPES TO BE HDPE PIPE UNLESS OTHERWISE NOTED.
- 20. STORMWATER MANAGEMENT REQUIREMENTS WILL BE MET ON-SITE VIA A WET POND AND STONE TRENCH. STORM WATER MANAGEMENT FACILITY WILL BE PRIVATELY OWNED, OPERATED AND MAINTAINED.
- 21. STORMWATER MANAGEMENT FACILITIES WILL BE REQUIRED ON OPEN SPACE LOT 67 SHOWN ON THIS PLAT IN ACCORDANCE WITH THE DESIGN MANUALS. PRIOR TO SIGNATURE APPROVAL OF THE SITE DEVELOPMENT PLAN, THE DEVELOPER WILL BE REQUIRED TO EXECUTE THE DEVELOPER'S AGREEMENT FOR THE CONSTRUCTION OF THE STORMWATER MANAGEMENT FACILITY AND A MAINTENANCE AGREEMENT.
- 22. WETLAND AND FOREST STAND DELINEATIONS PREPARED BY WILDMAN ENVIRONMENTAL SERVICES IN OCTOBER 2000 AND WETLAND LOCATIONS VERIFIED IN JULY 2004.
- 23. NO HISTORIC STRUCTURES EXIST ON-SITE. SITE IS NOT ADJACENT
- TO A DESIGNATED SCENIC ROAD.
- 24. NOISE STUDY APPROVED UNDER P-05-03, JAMESTOWN LANDING, SECTION II.
- 25. DRIVEWAYS SHALL BE PROVIDED PRIOR TO RESIDENTIAL OCCUPANCY TO ENSURE SAFE ACCESS FOR FIRE AND EMERGENCY VEHICLES PER THE FOLLOWING MINIMUM REQUIREMENTS:
  - WIDTH 12 FEET (14 FEET SERVING MORE THAN ONE RESIDENT). SURFACE - 6 INCHES OF COMPACTED CRUSHER RUN BASE WITH TAR AND CHIP COATING. GEOMETRY - MAXIMUM 15% GRADE, MAXIMUM 10% GRADE CHANGE AND MINIMUM OF 45-FOOT RADIUS.
  - STRUCTURES (CULVERT/BRIDGES) CAPABLE OF SUPPORTING 25 GROSS TONS (H25 LOADING). E) DRAINAGE ELEMENTS - CAPABLE OF SAFELY PASSING 100-YEAR FLOOD WITH NO MORE THAN
  - 1 FOOT DEPTH OVER DRIVEWAY SURFACE.
  - F) STRUCTURE CLEARANCES MINIMUM 12 FEET
- G) MAINTENANCE SUFFICIENT TO ENSURE ALL WEATHER USE.

PROVIDED IN ACCORDANCE WITH HO. CO. STD. DTL. R-4.01.

- 26. FOR FLAG OR PIPESTEM LOTS, REFUSE COLLECTION, SNOW REMOVAL AND ROAD MAINTENANCE ARE PROVIDED TO THE JUNCTION OF THE FLAG OR PIPESTEM AND ROAD RIGHT-OF-WAY LINE AND NOT TO THE PIPESTEM LOT DRIVEWAY.
- 27. IF MATERIAL IS WITHIN 8% OF OPTIMUM MOISTURE, WORKING THE MATERIAL UNTIL REQUIRED COMPACTION IS ACHIEVED IS CONTRACTOR'S RESPONSIBILITY.
- 28. SIDEWALKS SHALL MEET CURRENT ADA REQUIREMENTS. HANDICAP RAMPS AT THE ENTRANCE SHALL BE
- 29. THE 65 dBA NOISE CONTOUR LINE DRAWN ON THIS SUBDIVISION PLAN IS ADVISORY AS REQUIRED BY THE HOWARD COUNTY DESIGN MANUAL, CHAPTER 5, REVISED FEBRUARY, 1992 AND CANNOT BE CONSIDERED TO EXACTLY LOCATE THE 65 dBA NOISE EXPOSURE. THE 65 dBA NOISE LINE WAS ESTABLISHED BY HOWARD COUNTY TO ALERT DEVELOPERS, BUILDERS, AND FUTURE RESIDENTS THAT AREAS BEYOND THIS THRESHOLD MAY EXCEED GENERALLY ACCEPTED NOISE LEVELS ESTABLISHED BY THE U.S. DEPARTMENT OF HOUSING AND URBAN DEVELOPMENT.

# ROAD CONSTRUCTION PLANS JAMESTOWN LANDING. SIXTH ELECTION DISTRICT HOWARD COUNTY, MARYLAND



SCALE: 1'=1000'

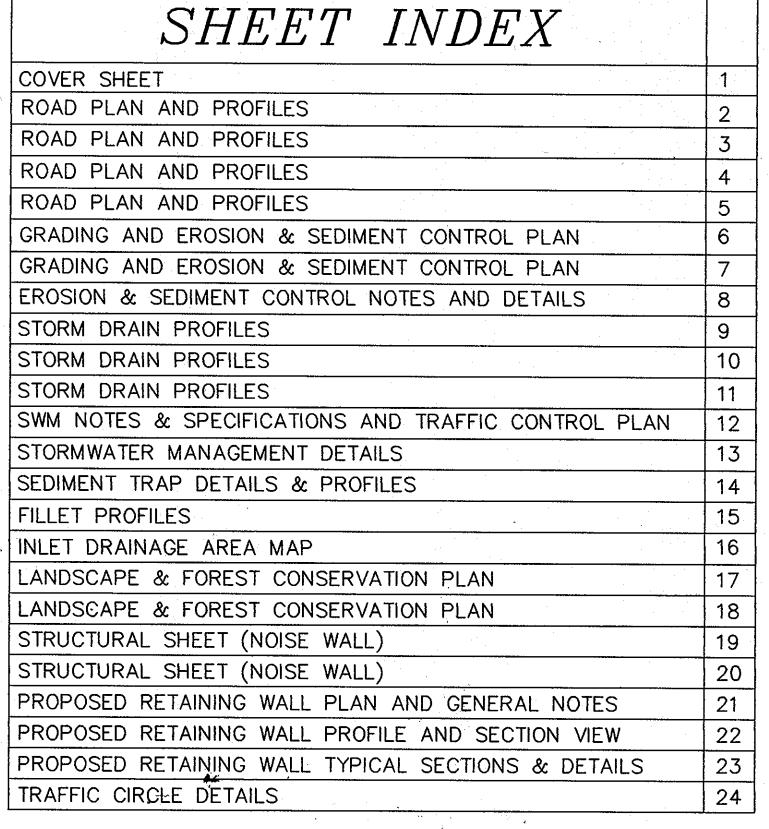
30. THIS PLAN IS SUBJECT TO THE FOURTH EDITION OF THE SUBDIVISION REGULATIONS AND

- TO THE 1993 ZONING REGULATIONS AMENDED BY CB50-2001.
- 31. PROPOSED NOISE BERM TO BE MAINTAINED BY THE HOA.
- 32. PROPOSED STRUCTURES MUST MAINTAIN A TWO-FOOT SETBACK FROM PUBLIC DRAINAGE
- 32. REQUEST TO WAIVER BASEMENT GRAVITY SEWER SERVICE TO LOTS 14, 30, 31, 42, 43, & 46, FIRST FLOOR AND BASEMENT GRAVITY SEWER SERVICE FOR LOTS 44 & 45, AND CLEARANCE REQUIREMENTS FOR LOTS 28, 29, 32, 39, AND 40 WERE APPROVED ON DECEMBER 3, 2003 AND MARCH 5, 2004 SUBJECT TO THE FOLLOWING CONDITIONS : THE SEWER BETWEEN MANHOLES 126 AND 127 SHALL BE DUCTILE IRON PIPE CLASS 54
- WITH FIELD LOCK GASKETS.

  THE SEWER HOUSE CONNECTIONS FOR LOTS 28 AND 29 SHALL BE RELOCATED TO THE LOW POINT OF THE LOTS AT THE REAR OF THE PROPERTIES.
- 3. A NOTE SHALL BE PLACED ON THE WATER AND SEWER PLAN AND ON THE SITE DEVELOPMENT PLAN REGARDING ACCESS TO PUBLIC WATER AND SEWER MAINS.
- 33. ON MAY 10, 2005, A WAIVER WAS GRANTED TO MODIFY STANDARD DETAILS (INLET TYPES SD-4.02 ADN SD-4.14) TO ALLOW THE STRUCTURES TO BE GREATER THAN AND LESS THAN THE COUNTY STANDARDDEPTH REQUIREMENTS. THIS APPROVAL WAS SUBJECT TO ADDING THE APPROPRIATE DESIGN INFORMATION TO THE PLANS.

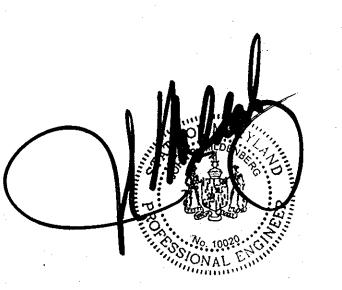
#### **DEVELOPER**

JAMESTOWN LANDING. LC 6820 ELM STREET, SUITE 200 McLEAN, VIRGINIA 22101 (703) 734–9730

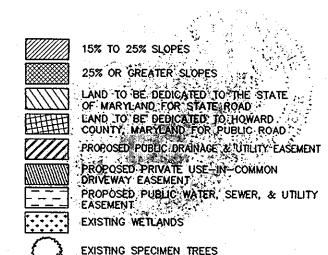


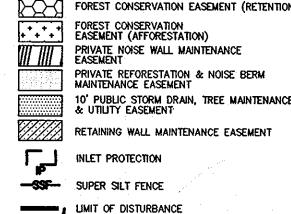


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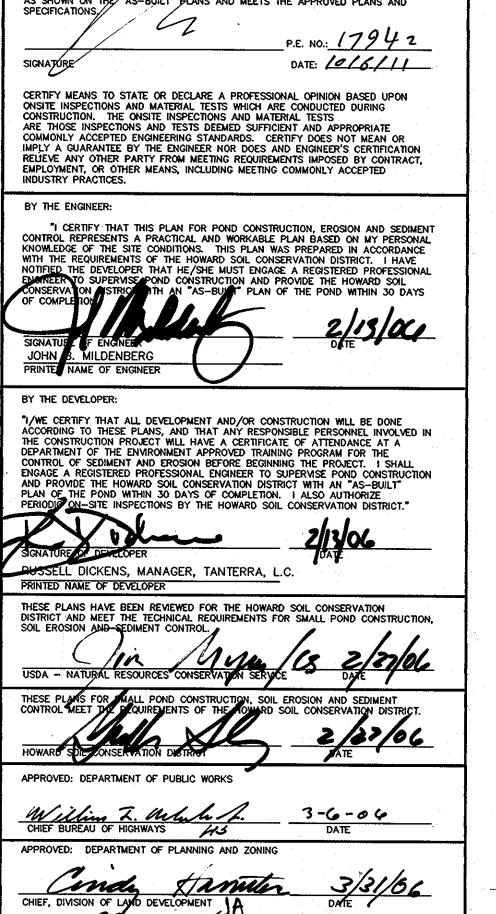


#### <u>LEGEND</u>





PRIVATE REFORESTATION & NOISE BERM MAINTENANCE EASEMENT RETAINING WALL MAINTENANCE EASEMENT INLET PROTECTION -SSF- SUPER SILT FENCE LIMIT OF DISTURBANCE STABILIZED CONSTRUCTION ENTRANCE

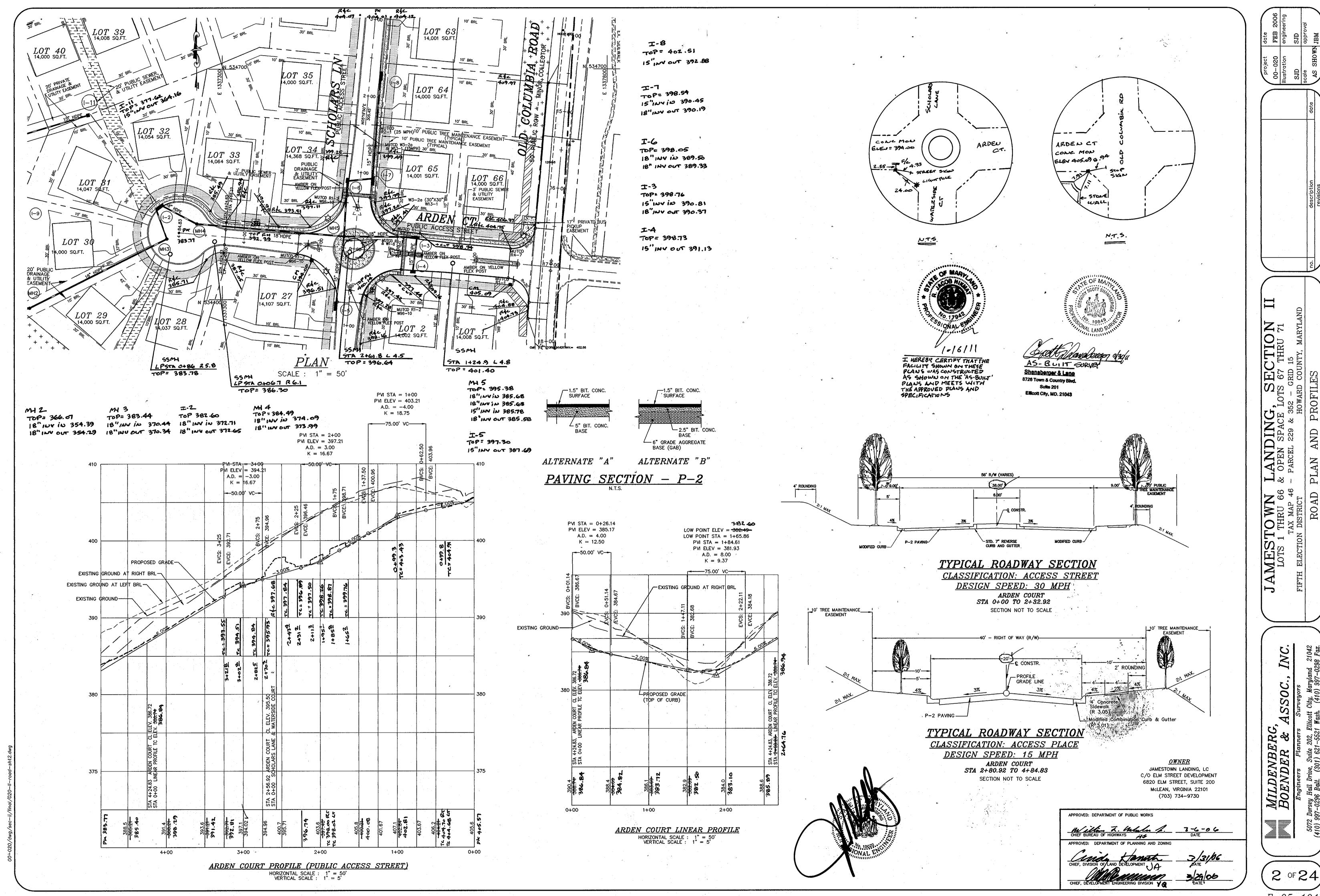


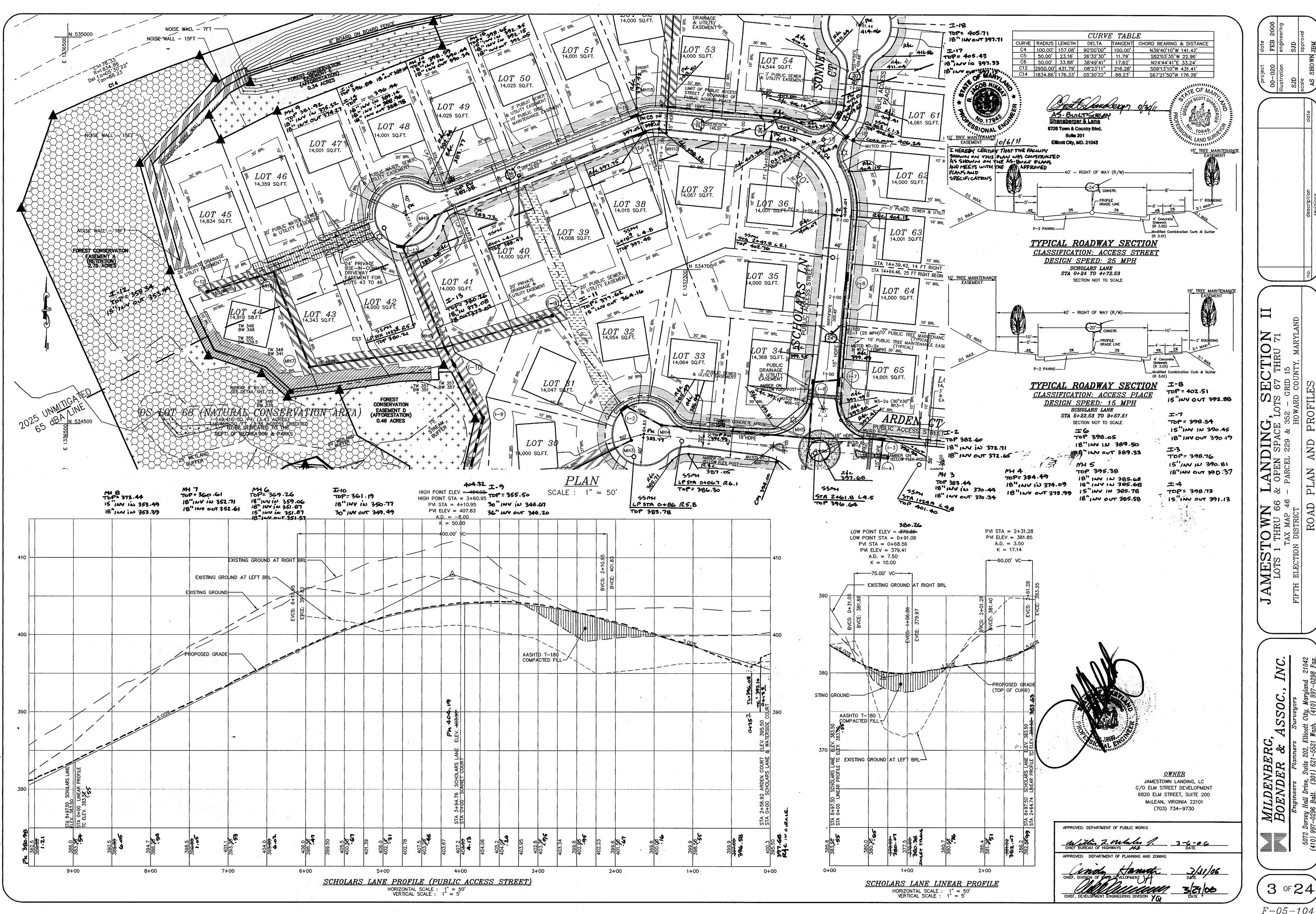
THE FACILITY SHOWN ON THIS PLAN WAS CONSTRUCTED

F-05-104

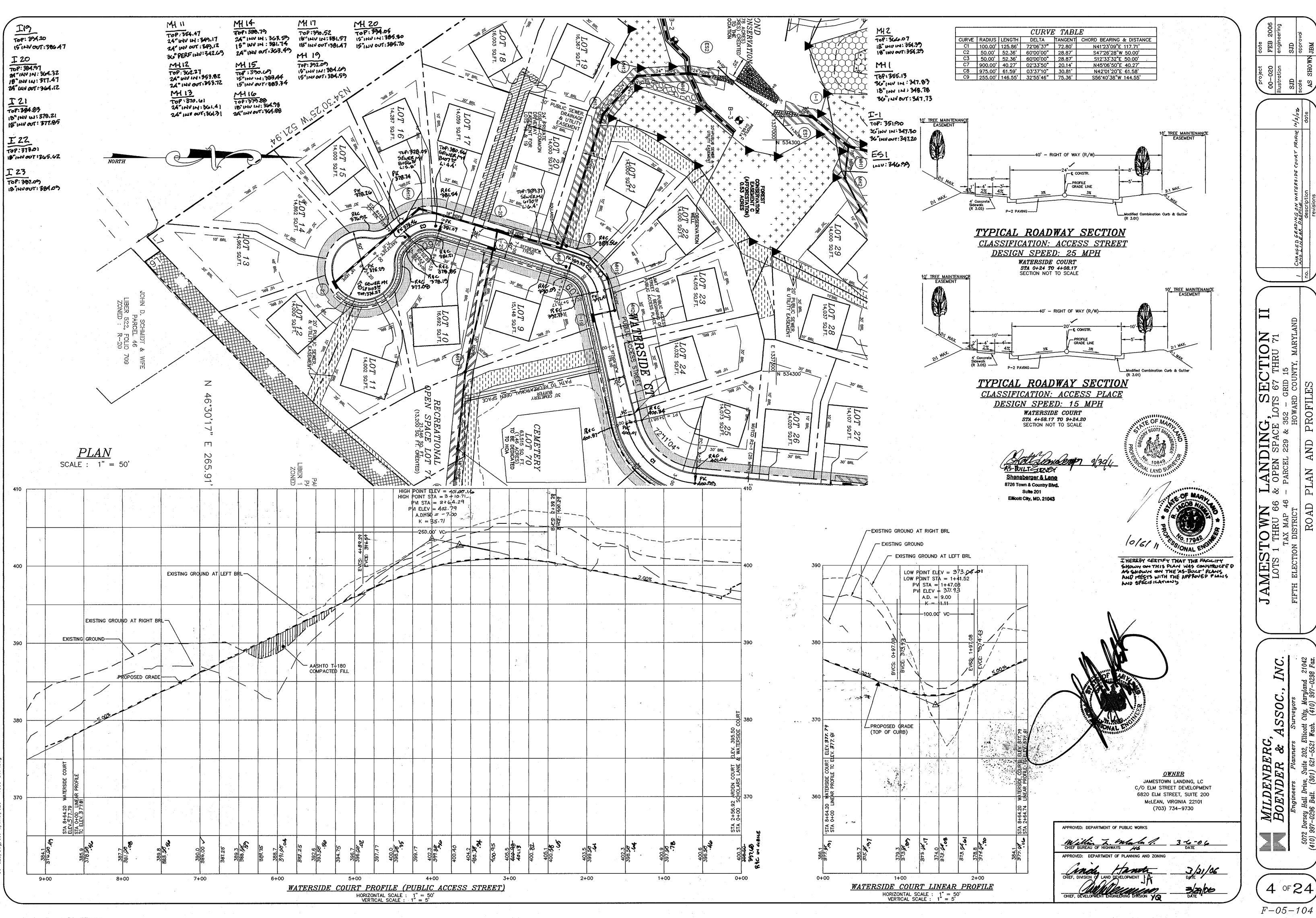
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MILDENBE BOENDER



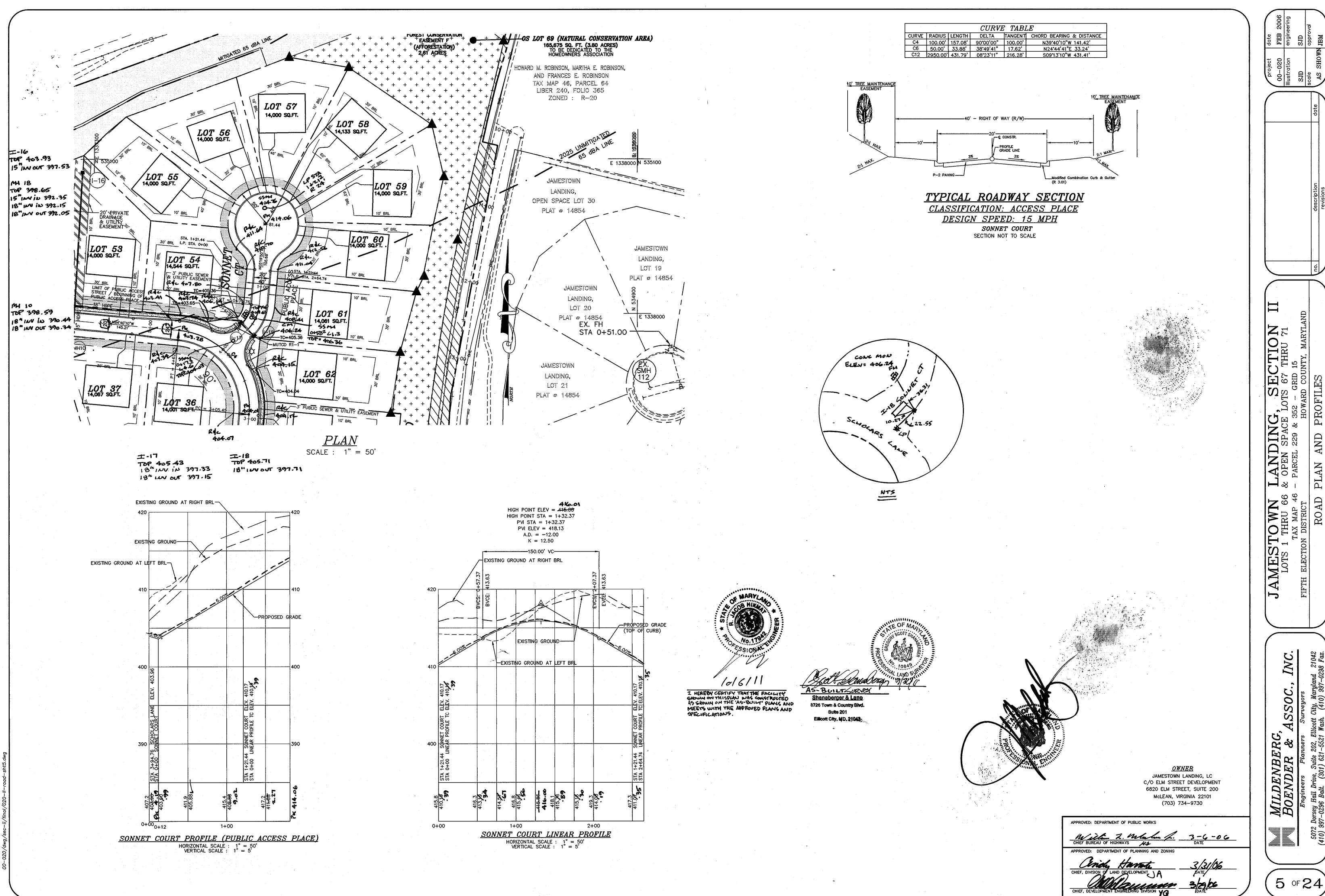


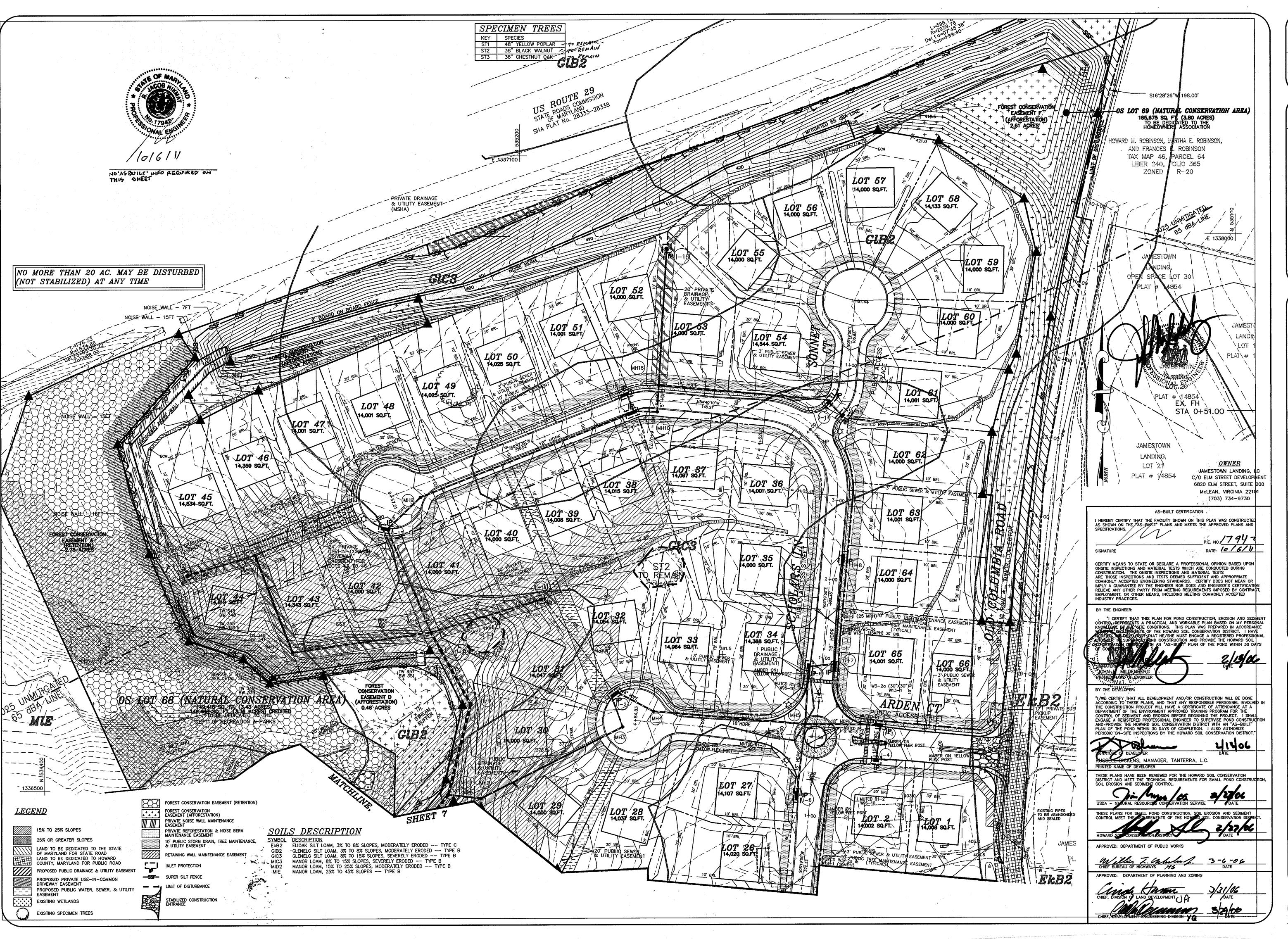
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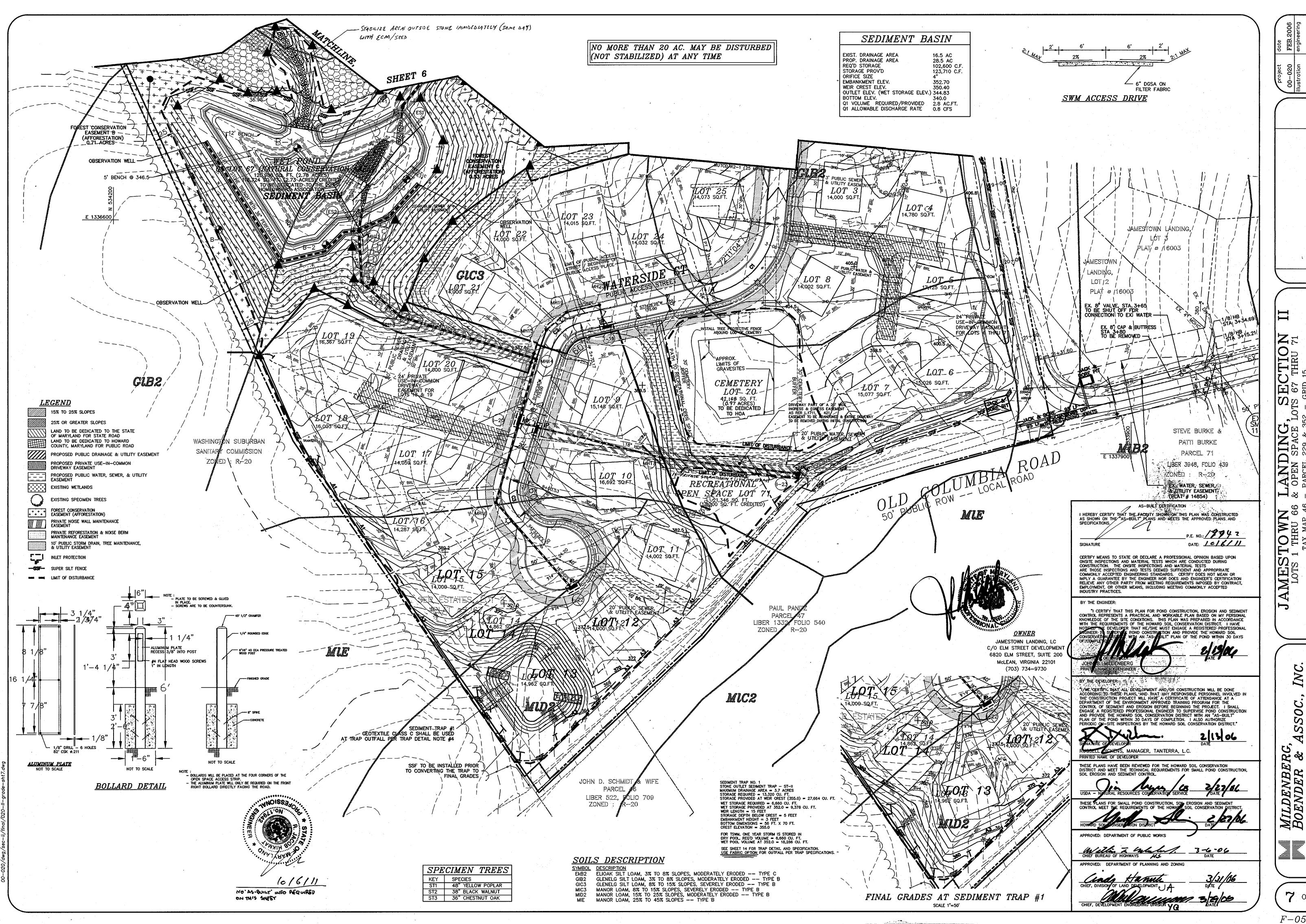


JAMESTOWN LANDING, SECTION ILANDING, SECTION ILAN LANDING & OPEN SPACE LOTS 67 THRU 71

TAX MAP 46 - PARCEL 229 & 352 - GRID 15

HOWARD COUNTY, MARYLAND GRADING AND EROSION & SEDIMENT CONTROL PLAN

6 of 24



TOWN
1 THRU 66
TAX MAP 46
ON DISTRICT FIFTH ELECTI GRADING

> 7 of 24 F-05-104

APPLY TO GRADED OR CLEARED AREAS NOT SUBJECT TO IMMEDIATE FURTHER DISTURBANCE WHERE A PERMANENT LONG-LIVED VEGETATIVE COVER IS NEEDED.

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISKING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED.

SOIL AMENDMENTS: IN LIEU OF SOIL TEST RECOMMENDATIONS, USE ONE OF THE FOLLOWING SCHEDULES: 1) PREFERRED - APPLY 2 TONS PER ACRES DOLOMITIC LIMESTONE (92 LBS/1000 SQ.FT.) AND 600 LBS. PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ.FT.) BEFORE SEEDING. HARROW OR DISK INTO UPPER THREE INCHES OF SOIL. AT TIME OF SEEDING, APPLY 400 LBS. PER ACRE 30-0-0 UREAFORM FERTILIZER (9 LBS./1000 SQ.FT.).

ACCEPTABLE - APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS./1000 SQ.FT.) AND 1000 LBS. PER ACRE 10-10-10 FERTILIZER (23 LBS./1000 SQ.FT.) BEFORE SEEDING. HARROW OR DISK INTO UPPER THREE INCHES OF SOIL.

SEEDING - FOR THE PERIODS MARCH 1 THRU APRIL 30, AND AUGUST 1 THRU OCTOBER 15, SEED WITH 60 LBS. PER ACRE 1.4 LBS/1000 SQ.FT.) OF KENTUCKY 31 TALL FESCUE. FOR THE PERIOD MAY 1 THRU JULY 31, SEED WITH 60 LBS. KENTUCKY 31 TALL FESCUE PER ACRE AND 2 LOBS. PER ACRE (.05 LBS./1000 SQ.FT.) OF WEEPING LOVEGRASS. DURING THE PERIOD OF OCTOBER 16 THRU FEBRUARY 28, PROTECT SITE BY: OPTION (1) - 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING. OPTION (2) - USE SOD. OPTION (3) -SEED WITH 60 LBS. /ACRE KENTUCKY 31 TALL FESCUE AND MULCH WITH 2 TONE /ACRE WELL ANCHORED STRAW.

MULCHING - APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS./1000 SQ.FT) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ.FT.) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ.FT.) FOR ANCHORING.

MAINTENANCE - INSPECT ALL SEEDING AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND RESEEDINGS.

#### TEMPORARY SEEDING NOTES

APPLY TO GRADED OR CLEARED AREAS LIKELY TO BE REDISTURBED WHERE A SHORT-TERM VEGETATIVE COVER IS NEEDED. SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISKING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, FOR NOT PREVIOUSLY LOOSENED.

SOIL AMENDMENTS: APPLY 600 LBS. PER ACRE 10-10-10 FERTILIZER (14 LBS./1000 SQ.FT.)

SEEDING: FOR PERIODS MARCH 1 THRU APRIL 30 AND FROM AUGUST 15 THRU OCTOBER 15, SEED WITH 2-1/2 BUSHEL PER ACRE OF ANNUAL RYE (3.2 LBS./1000 SQ.FT.) FOR THE PERIOD MAY 1 THRU AUGUST 14, SEED WITH 3 LBS. PER ACRE OF WEEPING LOVEGRASS (.07 LBS./1000 SQ.FT.). FOR THE PERIOD NOVEMBER 16 THRU NOVEMBER 28, PROTECT SITE BY APPLYING 2 TONS PER ACRÉ OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING, OR USE SOD.

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS./1000 SQ.FT.) OF UNROTTED WEED FREE SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GAL PER ACRE (5 GAL/1000 SQ.FT.) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GAL PER ACRE (8 GAL/1000 SQ.FT.) FOR ANCHORING.

REFER TO THE 1983 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR ADDITIONAL RATES AND METHODS NOT COVERED.

#### STANDARD SEDIMENT CONTROL NOTES

- 1) A MINIMUM OF 48 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT OF INSPECTIONS, LICENSES AND PERMITS, SEDIMENT CONTROL DIVISION PRIOR TO THE START OF NAY CONSTRUCTION, (313-1855).
- 2) ALL VEGETATIVE AND STRUCTURAL PRACTICES ARE TO BE INSTALLED ACCORDING TO THE PROVISIONS OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE MOST CURRENT "MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL", AND REVISIONS THERETO.
- 3) FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: A) 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, PERIMETER SLOPES AND ALL SLOPES GREATER THAN 3:1, B) 14 DAYS AS TO ALL OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITE.
- 4) ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 12, OF THE HOWARD COUNTY DESIGN MANUAL, STORM DRAINAGE.
- 5) ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1991 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDING (SEC.51), SOD (SEC. 54), TEMPORARY SEEDING (SEC. 50) AND MULCHING (SEC.52). TEMPORARY STABILIZATION WITH MULCH ALONE CAN ONLY BE DONE WHEN RECOMMENDED SEEDING DATES DO NOT ALLOW FOR PROPER GERMINATION AND ESTABLISHMENT OF GRASSES.
- 6) ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE MAINTAINED IN OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD
- TOTAL AREA OF SITE: \_\_\_ AREA DISTURBED: \_ ACRES AREA TO BE ROOFED OR PAVED: AREA TO BE VEGITATIVELY STABILIZED: \_ ACRES TOTAL CUT-TOTAL WASTE/BORROW AREA LOCATION:

THESE QUANTITIES ARE FOR PERMIT PURPOSES ONLY. CONTRACTOR IS REQUIRED TO PROVIDE HIS OWN QUANTITIES MEASUREMENTS.

- ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF UTILITIES MUST BE REPAIRED ON THE SAME DAY OF DISTURBANCE.
- ADDITIONAL SEDIMENT CONTROL MUST BE PROVIDED, IF DEEMED NECESSARY BY THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.
- 10) ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING. OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL BY THE INSPECTION AGENCY IS MADE.
- 11) TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGTHS OR THAT WHICH CAN BE BACK FILLED AND STABILIZED WITHIN ONE WORKING DAY, WHICHEVER IS SHORTER.

### STANDARD AND SPECIFICATIONS FOR TOPSOIL

#### **DEFINITION**

PLACEMENT OF TOPSOIL OVER A PREPARED SUBSOIL PRIOR TO ESTABLISHMENT OF PERMANENT VEGETATION.

#### **PURPOSE**

TO PROVIDE A SUITABLE SOIL MEDIUM FOR VEGETATIVE GROWTH. SOILS OF CONCERN HAVE LOW MOISTURE CONTENT, LOW NUTRIENT LEVELS, LOW pH, MATERIALS TOXIC TO PLANTS, AND/OR UNACCEPTABLE SOIL GRADATION.

#### CONDITIONS WHERE PRACTICE APPLIES

- THIS PRACTICE IS LIMITED TO AREAS HAVING 2:1 OR FLATTER SLOPES WHERE:
- a. THE TEXTURE OF THE EXPOSED SUBSOIL/PARENT MATERIAL IS NOT ADEQUATE TO PRODUCE VEGETATIVE
- b. THE SOIL MATERIAL IS SO SHALLOW THAT THE ROOTING ZONE IS NOT DEEP ENOUGH TO SUPPORT PLANTS OR FURNISH CONTINUING SUPPLIES OF MOISTURE AND PLANT NUTRIENTS.

REQUIRE SPECIAL CONSIDERATION AND DESIGN FOR ADEQUATE STABILIZATION. AREAS HAVING SLOPES STEEPER

c. THE ORIGINAL SOIL TO BE VEGETATED CONTAINS MATERIAL TOXIC TO PLANT GROWTH.

THAN 2:1 SHALL HAVE THE APPROPRIATE STABILIZATION SHOWN ON THE PLANS.

d. THE SOIL IS SO ACIDIC THAT TREATMENT WITH LIMESTONE IS NOT FEASIBLE

#### CONSTRUCTION AND MATERIAL SPECIFICATIONS

II. FOR THE PURPOSE OF THESE STANDARDS AND SPECIFICATIONS, AREAS HAVING SLOPES STEEPER THAN 2:1

- TOPSOIL SALVAGED FROM THE EXISTING SITE MAY BE USED PROVIDED THAT IT MEETS THE STANDARDS AS SET FORTH IN THESE SPECIFICATION. TYPICALLY, THE DEPTH OF TOPSOIL TO BE SALVAGED FOR A GIVEN SOIL TYPE CAN BE FOUND IN THE REPRESENTATIVE SOIL PROFILE SECTION IN THE SOIL SURVEY PUBLISHED BY USDA-SCS IN COOPERATION WITH MARYLAND AGRICULTURAL EXPERIMENTAL STATION.
- TOPSOIL SPECIFICATIONS SOIL TO BE USED AS TOPSOIL MUST MEET THE FOLLOWING
  - TOPSOIL SHALL BE A LOAM, SANDY LOAM, CLAY LOAM, SILT LOAM, SANDY CLAY LOAM, LOAMY SAND, OTHER SOILS MAY BE USED IF RECOMMENDED BY AN AGRONOMIST OR SOIL SCIENTIST AND APPROVED THE APPROPRIATE APPROVAL AUTHORITY. REGARDLESS, TOPSOIL SHALL NOT BE A MIXTURE OF CON-TRASTING TEXTURED SUBSOILS AND SHALL CONTAIN LESS THAN 5% BY VOLUME OF CINDERS, STONES, SLAG, COARSE FRAGMENTS, GRAVEL, STICKS, ROOTS, TRASH, OR OTHER MATERIALS LARGER THAN 1 1/2" IN
  - TOPSOIL MUST BE FREE OF PLANTS OR PLANT PARTS SUCH AS BERMUDA GRASS, QUACKGRASS, JOHNSON-SON GRASS, NUTSEDGE, POISON IVY, THISTLE, OR OTHERS AS SPECIFIED.
  - WHERE THE SUBSOIL IS EITHER HIGHLY ACIDIC OR COMPOSED OF HEAVY CLAYS, GROUND LIMESTONE SHALL BE SPREAD AT THE RATE OF 4-8 TONS/ACRE (200-400 POUNDS PER 1,000 SQUARE FEET) PRIOR TO THE PLACEMENT OF TOPSOIL. LIME SHALL BE DISTRIBUTED UNIFORMLY OVER DESIGNATED AREAS AND WORKED INTO THE SOIL IN CONJUNCTION WITH TILLAGE OPERATIONS AS DESCRIBED IN THE FOLLOWING
- III. FOR SITES HAVING DISTURBED AREAS UNDER 5 ACRES:

PERSPECTIVE VIEW 36' MINIMUM FENCE POST LENGTH

Fence posts shall be a minimum of 36' long driven 16' minimum into the

ground. Wood posts shall be 11/2" x 11/2" square (ninimum) cut, or 13/4" diameter (ninimum) round and shall be of sound quality hardwood. Steel posts will be

tandard T or U section weighting not less than 1.00 pand per linear foot.

ir staples at top and mid-section and shall neet the following requirements

20 lbs/in (min.)

Where ends of geotextile fabric cone together, they shall be overlapped

bulges occur or when sediment accumulation reached 50% of the fabric height.

PROFILE

. Vidth - 10' minimum, should be flared at the existing road to provide a turning

Geotextile fabric (filter cloth) shall be placed over the existing ground prior to placing stone. MMThe plan approval authority may not require single family residences to use geotextile.

intrances shall be piped through the entrance, maintaining positive drainage. Pipe installed through the stabilized construction entrance shall be protected with a

. Location - A stabilized construction entrance shall be located at every point

where construction traffic enters or leaves a construction site. Vehicle's leaving the site must travel over the entire length of the stabilized construction entranc

stable bern with 5:1 stopes and a minimum of 6' of stone over the pipe. Pipe ha to be sized according to the drainage. When the SCE is located at a high spot and has no drainage to convey a pipe will not be necessary. Pipe should be sized according to the amount of runoff to be conveyed. A 6' minimum will be required.

Stone - crushed aggregate (2" to 3") or reclaimed or recycled concre equivalent shall be placed at least 6' deep over the length and width of the

DETAIL 24 - STABILIZED CONSTRUCTION ENTRANCE

MINIMUM 6' OF 2'-3' AGGREGATE OVER LENGTH AND VIDTH OF STRUCTURE

STAPLE/

Tensile Hodulus

olded and stapled to prevent sediment bypass

GEBTEXTILE CLASS 'C'-

OR BETTER

DETAIL 22 - SILT FENCE

PLACE TOPSOIL (IF REQUIRED) AND APPLY SOIL AMENDMENTS AS SPECIFIED IN 20.0 VEGETATIVE STABILIZATION - SECTION I - VEGETATIVE STABILIZATION METHODS AND MATERIALS.

- FENCE POST DRIVEN HINIMUM OF 16' INT

--- PIPE AS NECESSAR

IV. FOR SITES HAVING DISTURBED AREAS OVER 5 ACRES:

DISSIPATION OF PHYTO-TOXIC MATERIALS.

- ON SOIL MEETING TOPSOIL SPECIFICATIONS, OBTAIN TEST RESULTS DICTATING FERTILIZER AND LIME AMENDMENTS REQUIRED TO BRING THE SOIL INTO COMPLIANCE WITH THE FOLLOWING:
  - a. ph for topsoils shall be between 6.0 and 7.5. If the tested soil demonstrates a ph of LESS THAN 6.0, SUFFICIENT LIME SHALL BE PERSCRIBED TO RAISE THE pH TO 6.5 OR HIGHER.
  - b. ORGANIC CONTENT OF TOPSOIL SHALL BE NOT LESS THAN 1.5 PERCENT BY WEIGHT.
  - c. TOPSOIL HAVING SOLUBLE SALT CONTENT GREATER THAN 500 PARTS PER MILLION SHALL NOT BE USED. d. NO SOD OR SEED SHALL BE PLACED ON SOIL WHICH HAS BEEN TREATED WITH SOIL STERILANTS OR CHEMICALS USED FOR WEED CONTROL UNTIL SUFFICIENT TIME HAS ELAPSED (14 DAYS MIN.) TO PERMIT

NOTE: TOPSOIL SUBSTITUTES OR AMENDMENTS, AS RECOMMENDED BY A QUALIFIED AGRONOMIST OR SOIL SCIENTIST AND APPROVED BY THE APPROPRIATE APPROVAL AUTHORITY, MAY BE USED IN LIEU OF NATURAL

ii. PLACE TOPSOIL (IF REQUIRED) AND APPLY SOIL AMENDMENTS AS SPECIFIED IN 20.0 VEGETATIVE STABILIZATION - SECTION I - VEGETATIVE STABILIZATION METHODS AND MATERIALS.

#### V. TOPSOIL APPLLICATION

\* FOR REMOVABLE PUMP STATION DETAIL AND SPECIFICATIONS, BAFFLE DETAIL, AND BASIN DRAW DOWN SCHEMATIC HORIZONTAL DRAWN DOWN DEVICE, SEE SHEETS 13 & 14.

SILT FENCE

Silt Fence Design Criterio

Slope Length

125 feet

100 feet

60 feet

Note: In areas of less than 2% slope and sandy soils (USDA general classificatio system, soil Class A) maximum slope length and sitt fence length will be unlimited. In these areas a silt fence may be the only perimeter control

Slope Steepness

Flatter than 50:1

10-1 to 5-1

5-1 to 3-1

2:1 SLOPE OR

Silt Fence Length

unlimited

750 feet

500 feet

125 feet

E - 15 - 3A WATER MANAGEMENT AD

DETAIL 5 - RIP-RAP INFLOW PROTECTION

. Rip-rap tined inflow channels shall be 1' in depth, have a trapezoidal

2. Filter cloth shall be installed under all rip-rap. Filter cloth shall

3. Entrance and exit sections shall be installed as shown on the detail

 Rip-rap used for the lining may be recycled for permanent outlet. protection if the basin is to be converted to a stornwater management

5. Gabion Inflow Protection may be used in lieu of Rip-rap Inflow

and 10:1, for slopes flatter than 10:1 use Earth Dike or Temporary Swale

6. Rip-rap should blend into existing ground

cross section with 21 or flatter side slopes and 3' (min.) bottom width. The channel shall be lined with 4' to 12' rip-rap to a depth of 18'.

1,000 feet

- WHEN TOPSOILING, MAINTAIN NEEDED EROSION AND SEDIMENT CONTROL PRACTICES SUCH AS DIVERSIONS, GRADE STABILIZATION STRUCTURES, EARTH DIKES, SLOPE SILT FENCE AND SEDIMENT TRAPS AND BASINS.
- ii. GRADES ON THE AREAS TO BE TOPSOILED, WHICH HAVE BEEN PREVIOUSLY ESTABLISHED, SHALL BE
- MAINTAINED, ALBEIT 4" 8" HIGHER IN ELEVATION. iii. TOPSOIL SHALL BE UNIFORMLY DISTRIBUTED IN A 4" TO 8" LAYER AND LIGHTLY COMPACTED TO A MINIMUM
- THICKNESS OF 4". SPREADING SHALL BE PERFORMED IN SUCH A MANNER THAT SODDING OR SEEDING CAN PROCEED WITH A MINIMUM OF ADDITIONAL SOIL PREPARATION AND TILLAGE. ANY IRREGULARITIES IN THE SURFACE RESULTING FROM TOPSOILING OR OTHER OPERATIONS SHALL BE CORRECTED IN ORDER TO PREVENT THE FORMATION OF DEPRESSIONS OR WATER POCKETS.
- iv. TOPSOIL SHALL NOT BE PLACED WHILE THE TOPSOIL OR SUBSOIL IS IN A FROZEN OR MUDDY CONDITION, WHEN THE SUBSOIL IS EXCESSIVELY WET OR IN A CONDITION THAT MAY OTHERWISE BE DETRIMENTAL TO PROPER GRADING AND SEEDBED PREPARATION.
- ALTERNATIVE FOR PERMANENT SEEDING INSTEAD OF APPLYING THE FULL AMOUNTS OF LIME AND COMMERCIAL FERTILIZER, COMPOSTED SLUDGE AND AMENDMENTS MAY BE APPLIED AS SPECIFIED BELOW:
  - COMPOSTED SLUDGE MATERIAL FOR USE AS A SOIL CONDITIONER FOR SITES HAVING DISTURBED AREAS OVER 5 ACRES SHALL BE TESTED TO PRESCRIBE AMENDMENTS AND FOR SITES HAVING AREAS UNDER 5 ACRES SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
  - a. COMPOSTED SLUDGE SHALL BE SUPPLIED BY, OR ORIGINATE FROM, A PERSON OR PERSONS WHO ARE PERMITTED (AT THE TIME OF ACQUISITION OF THE COMPOST) BY THE MARYLAND DEPARTMENT OF THE ENVIRONMENT UNDER COMAR 26.04.06.
  - b. COMPOSTED SLUDGE SHALL CONTAIN AT LEASE 1 PERCENT NITROGEN, 1.5 PERCENT PHOSPHOURUS, AND 0.2 PERCENT POTASSIUM AND HAVE A Ph OF 7.0 TO 8.0. IF COMPOST DOES NOT MEET THESE REQUIREMENTS, THE APPROPRIATE CONSTITUENTS MUST BE ADDED TO MEET THE REQUIREMENTS PRIOR TO USE. c. COMPOSTED SLUDGE SHALL BE APPLIED AT A RATE OF 1 TON/1,000 SQUARE FEET.
  - iv. COMPOSTED SLUDGE SHALL BE AMENDED WITH A POTASSIUM FERTILLIZER APPLIED AT THE RATE OF 4 LB/1,000 SQUARE FEET, AND 1/3 THE NORMAL LIME APPLICATION RATE.

REFERENCES: GUIDELINE SPECIFICATIONS, SOIL PREPARATION AND SODDING. MD-VA, PUB. #1, COOPERATIVE EXTENSION SERVICE, UNIVERSITY OF MARYLAND AND VIRGINIA POLYTECHNIC INSTITUTES. REVISED 1973.

DETAIL 33 - SUPER SILT FENCE

CHAIN LINK FENCING

required except on the ends of the fence.

every 24° at the top and mid section.

for a 6' fence shall be used, substituting 42' fabric and 6' length

4. Filter cloth shall be embedded a minimum of 8' into the ground

2. Chain link fence shall be fastened securely to the fence posts with wire ties. The lower tension wire, brace and truss rods, drive anchors and post caps are not

3. Filter cloth shall be fastened securely to the chain link fence with ties spaced

5. When two sections of filter cloth adjoin each other, they shall be overlapped

6. Maintenance shall be performed as needed and silt buildups removed when "bulges" develop in the silt fence, or when silt reaches 50% of fence height

20 lbs/in (nin.)\*
0.3 gal/ft\*/ninute (nax.)

DETAIL 23C - CURB INLET PROTECTION (COG OR COS INLETS)

staples at top and mid section and shall neet the following requirements for

#### SEQUENCE OF CONSTRUCTION

1. OBTAIN GRADING PERMIT. (1 DAY)

2. CONSTRUCT STABILIZED CONSTRUCTION ENTRANCE (1 DAY) 3. CONSTRUCT SEDIMENT TRAP NO. 1 AND SEDIMENT BASIN NO. 1. SEDIMENT BASIN NO. 1 IS TO INCLUDE TEMPORARY LOW FLOW ORIFICE AT INVERT INDICATED AND 8" PVC PIPE RISER CONSTRUCT RIP-RAP OUTLET CHANNEL FROM BASIN, AND ECM ON DISTURBED AREAS OF OUTFALL CHANNEL. INSTALL SUPER SILT FENCE BELOW TRAP AND BASIN PER PLAN (4 DAYS).

4. CONSTRUCT REMAINING SUPER SILT FENCES AND EARTH DIKES (3 DAYS)

5. RECEIVE PERMISSION FROM THE SEDIMENT CONTROL INSPECTOR TO PROCEED.

6. CONSTRUCT STORM DRAIN FROM 110 TO ES1. DIRECT RUNOFF TO CONSTRUCTED INLETS UNTIL ALL CONTRIBUTING AREAS HAVE RECEIVED PERMANENT STABILIZATION

8. BEGIN CONSTRUCTION ON REMAINING UTILITIES (90 DAYS).

7. WITH PERMISSION FROM THE INSPECTOR, BEGIN MASS GRADING (45 DAYS). DELAY CONSTRUCTION ON LOTS 13 AND 14 UNTIL DISTURBED AREA DRAINING TO THE TRAP IS STABILIZED.

9. WHEN DISTURBED AREAS TO SEDIMENT TRAP 1 HAS BEEN STABILIZED, AND WITH THE APPROVAL OF THE SEDIMENT CONTROL INSPECTOR, REMOVE TRAP AND CONSTRUCT SITE TO FINAL GRADES INDICATED. (5 DAYS)

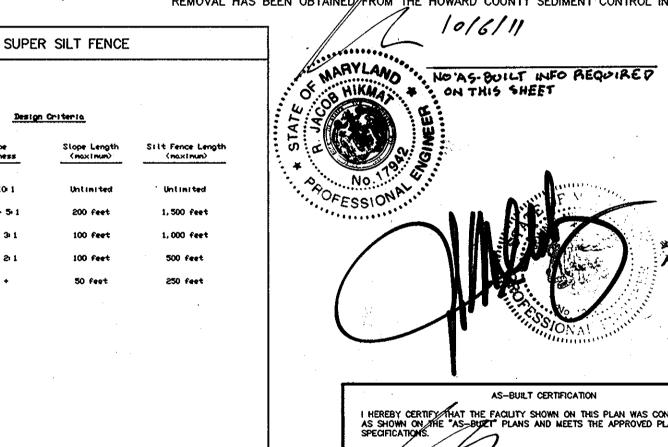
10. PLANT AFFORESTATION AREA PER PLANS. (5 DAYS) 11. WHEN ALL CONTRIBUTING AREAS TO SEDIMENT AND EROSION CONTROL DEVICES HAVE BEEN STABILIZED, AND WITH PERMISSION OF SEDIMENT CONTROL INSPECTOR, REMOVE SEDIMENT AND

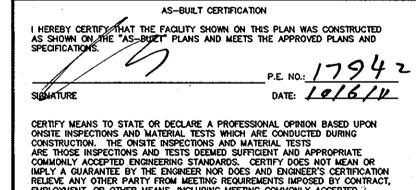
EROSION CONTROL DEVICES AND STABILIZE REMAINING DISTURBED AREAS. (15 DAYS) 12. WITH PERMISSION OF SEDIMENT CONTROL INSPECTOR, RECONSTRUCT SEDIMENT BASIN TO PERMANENT DESIGN CONTOURS. REMOVE TEMPORARY DEWATERING STANDPIPE. BRICK SHUT TEMPORARY LOW FLOW ORIFICE. CONSTRUCT WEIR WALL PER FINAL CONFIGURATION AND CONSTRUCT LOW FLOW PLATE.

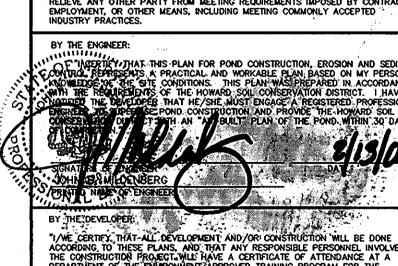
ATTACH HALF SHELL TRASH RACK, AND CONSTRUCT REMAINDER OF STORM DRAIN SYSTEM. (3 DAYS). 13. STABILIZE REMAINING DISTURBED AREAS. (2 DAYS)

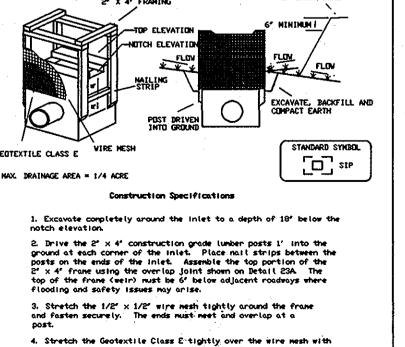
## EROSION AND SEDIMENT CONTROL NOTES

- 1. ALL SEDIMENT CONTROL OPERATIONS ARE TO BE DONE IN ACCORDANCE WITH SECTION 219 OF THE HOWARD COUNTY VOLUME IV DESIGN MANUAL AND THE STANDARDS AND SPECIFICATIONS FOR SEDIMENT CONTROL IN DEVELOPING AREAS.
- 2. ALL EROSION AND SEDIMENT CONTROL DEVICES SHALL BE INSTALLED AS THE FIRST ORDER OF
- 3. ALL EXCAVATED MATERIALS SHALL BE STOCKPILED ON THE UPGRADE SIDE OF THE MAIN TRENCH.
- 4. EXCAVATION AND BACKFILL SHALL BE LIMITED TO THAT WHICH CAN BE STABILIZED WITHIN ONE
- 5. IMMEDIATELY FOLLOWING BACKFILL OF THE SEWER TRENCH, ALL DISTURBED AREAS ARE TO BE STABILIZED IN ACCORDANCE WITH THE PERMANENT STABILIZATION AND SEEDING NOTES SHOWN
- 6. THROUGHOUT THE PROJECT, THE CONTRACTOR SHALL REGULARLY INSPECT ALL SEDIMENT CONTROL DEVICES AND PROVIDE ALL NECESSARY MAINTENANCE TO INSURE THAT ALL DEVICES ARE IN
- 7. ALL SEDIMENT CONTROL FACILITIES SHALL REMAIN IN PLACE UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.









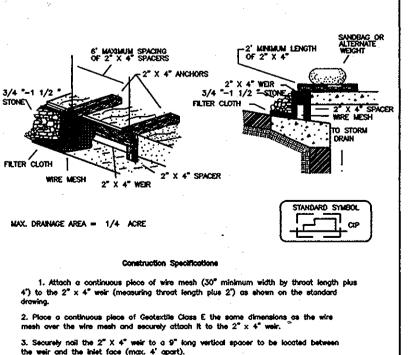
DETAIL 23A - STANDARD INLET PROTECTION

0 - 10:1

10 1 - 5 1

6. If the inlet is not in a sump, construct a compacted earth dike across the ditch line directly below it. The top of the earth dike should be at least 6' higher than the top of the frame.

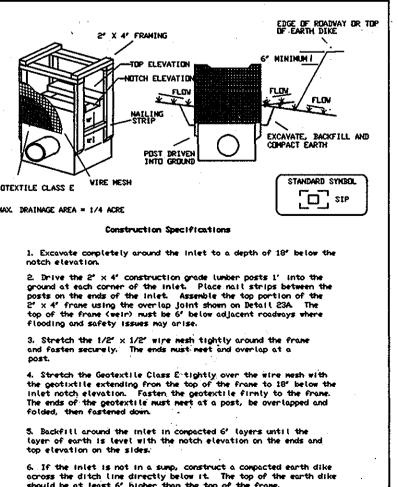
7. The structure must be inspected periodically and after each rain and the geotextile replaced when it becomes clogged



the weir and the injet face (max. 4' apart). i. Place the assembly against the inlet throat and noti (minimum 2' lengths of

earth or asphalt dike to direct the flow to the inlet.

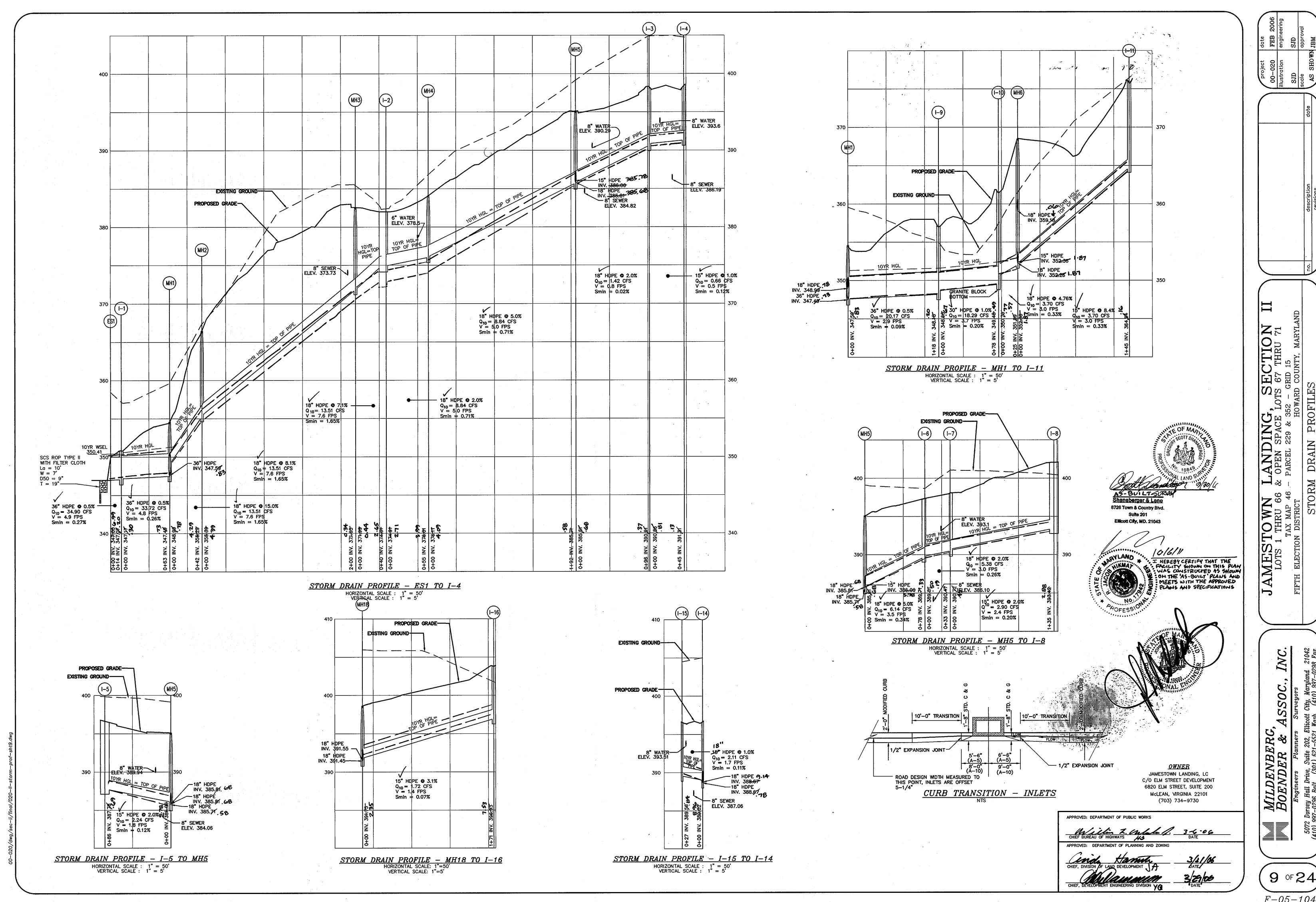
2" x 4" to the top of the weir at spacer locations). These 2" x 4" anchors shall extend across the inlet top and be held in place by sandbags or alternate weight. i. The assembly shall be placed so that the end spacers are a minimum 1' beyon 6. Form the 1/2 " x 1/2 " wire mesh and the geotextile fabric to the concrete gutter and against the face of the curb on both sides of the links. Place clean 3/4 " x 1 1/2 " tone over the wire mesh and geotextile in such a manner to prevent water from 7. This type of protection must be inspected frequently and the filter cloth and stone replaced when clagged with sediment.



A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONA TE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE KENS MANAGER, TANTERRA, L.C. THESE PLANS HAVE BEEN REVIEWED FOR THE HOWARD SOIL CONSERVATION

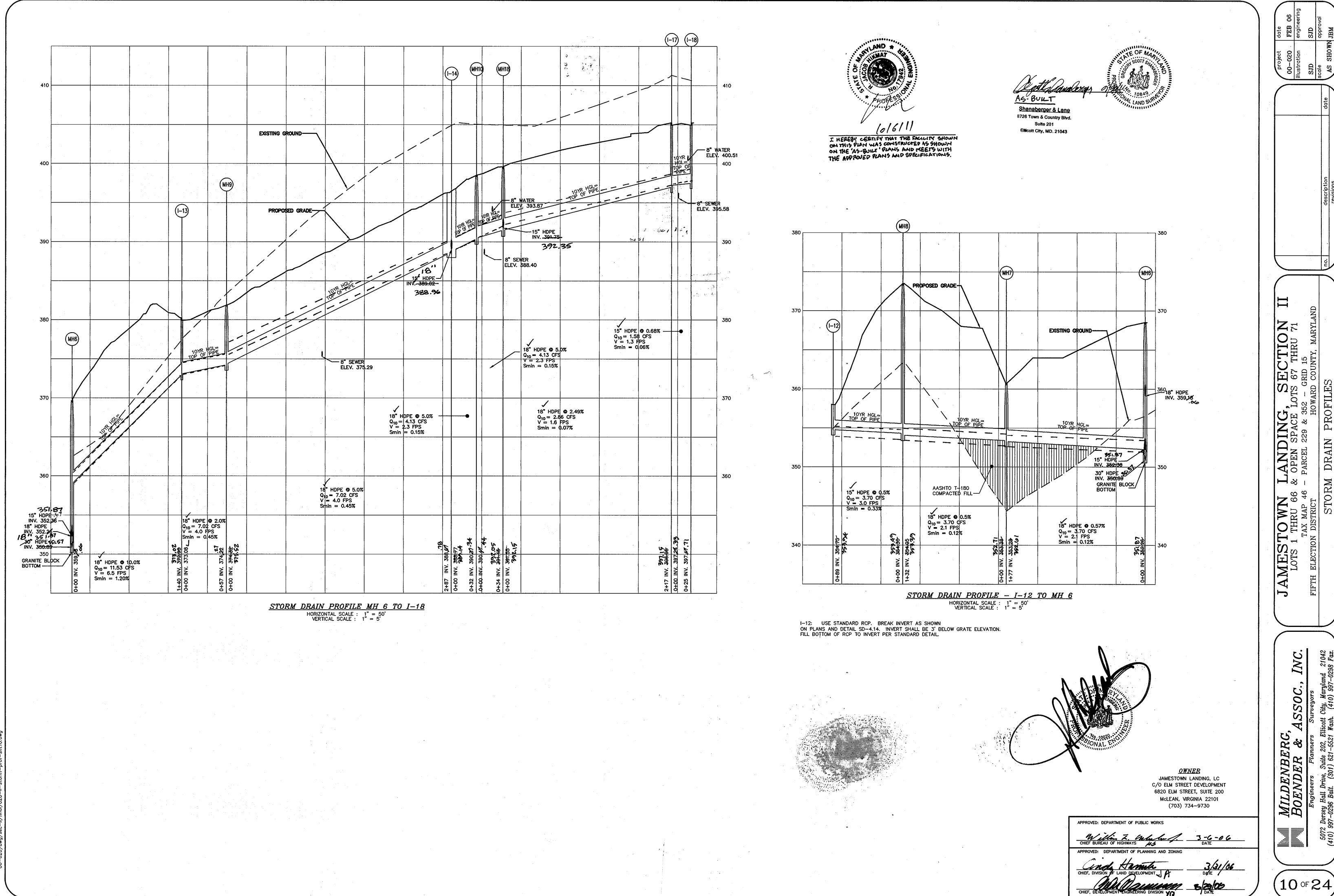
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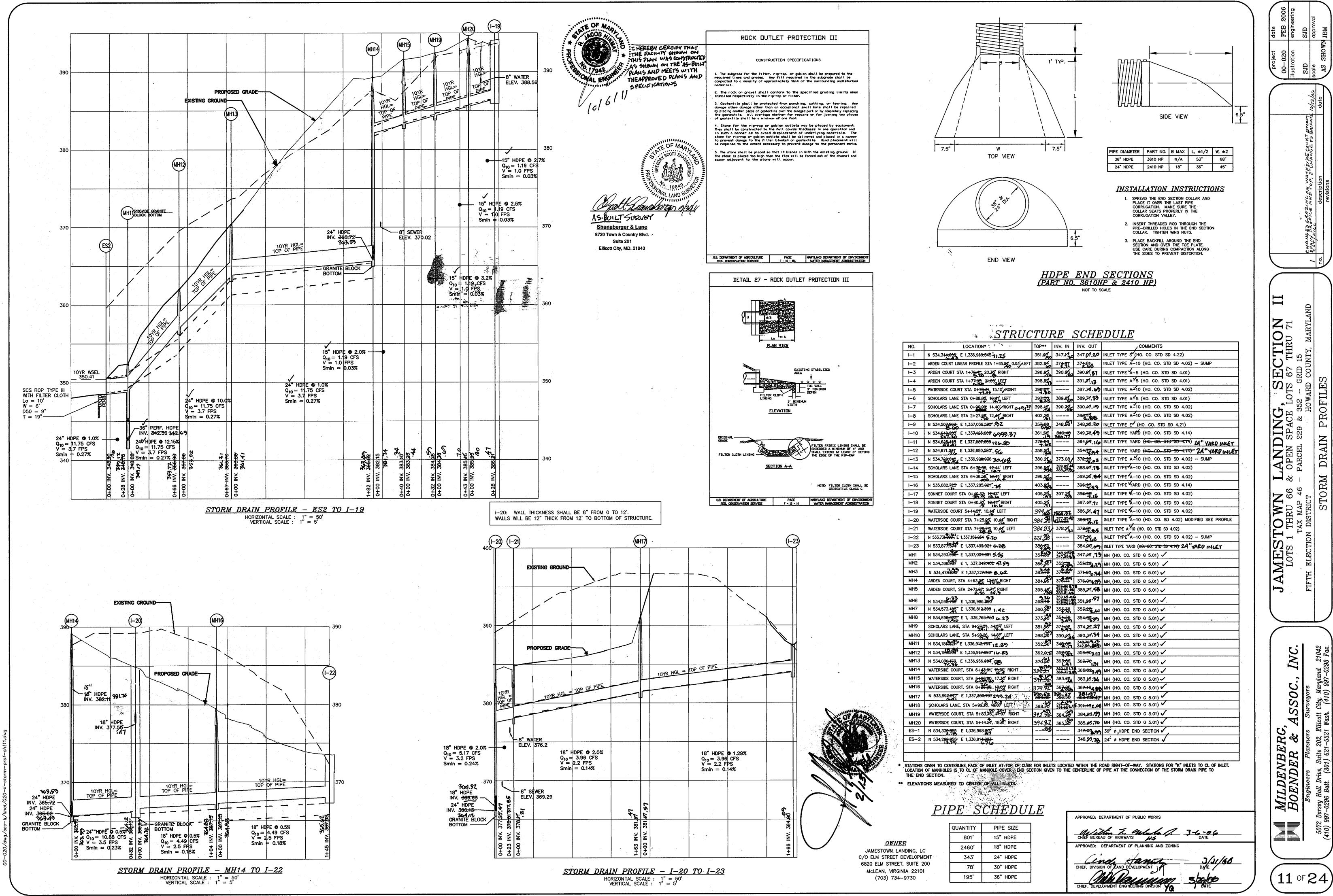
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ORAIN





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DRAIN

#### MD-378 POND SPECIFICATIONS (JANUARY 2000) PIPE CONDUIT CONSTRUCTION SPECIFICATIONS THESE SPECIFICATIONS ARE APPROPRIATE TO ALL PONDS WITHIN THE SCOPE OF THE STANDARD FOR PRACTICE MD-378. ALL REFERENCES TO ASTM AND AASHTO SPECIFICATIONS APPLY TO THE MOST RECENT ALL PIPES SHALL BE CIRCULAR IN CROSS SECTION. CORRUGATED METAL PIPE — ALL OF THE FOLLOWING CRITERIA SHALL APPLY FOR CORRUGATED METAL PIPE: I. MATERIALS - (POLYMER COATED STEEL PIPE) - STEEL PIPES WITH POLYMERIC COATINGS SHALL HAVE A MINIMUM COATING THICKNESS OF 0.01 INCH (10 MIL) ON BOTH SIDES OF THE PIPE. THIS PIPE AND ITS APPURTENANCES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO SPECIFICATIONS M-245 & M-246 WITH WATERTIGHT COUPLING AREAS DESIGNATED FOR BORROW AREAS, EMBANKMENT, AND STRUCTURAL WORKS SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL. ALL TREES, VEGETATION, ROOTS AND OTHER OBJECTIONABLE MATERIAL SHALL BE REMOVED. CHANNEL BANKS AND SHARP BREAKS SHALL BE SLOPED TO NO STEEPER THAN 1:1. ALL TREES SHALL BE CLEARED AND GRUBBED WITHIN 15 FEET OF THE TOE OF THE BANDS OR FLANGES. MATERIALS — (ALUMINUM COATED STEEL PIPE) — THIS PIPE AND ITS APPURTENANCES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO SPECIFICATION M-274 WITH WATERTIGHT COUPLING BANDS OR FLANGES. ALUMINUM COATED STEEL PIPE, WHEN USED WITH FLOWABLE FILL OR WHEN SOIL AND/OR WATER CONDITIONS WARRANT THE AREAS TO BE COVERED BY THE RESERVOIR WILL BE CLEARED OF ALL TREES, BRUSH, LOGS, FENCES, RUBBISH AND OTHER OBJECTIONABLE MATERIAL UNLESS OTHERWISE DESIGNATED ON THE PLANS. TREES, BRUSH, AND STUMPS SHALL BE CUT APPROXIMATELY LEVEL WITH THE GROUND SURFACE. FOR DRY STORM NEED FOR INCREASED DURABILITY, SHALL BE FULLY BITUMINOUS COATED PER REQUIREMENTS OF AASHTO SPECIFICATION M-190 TYPE A. ANY ALUMINUM COATING DAMAGED OR OTHERWISE REMOVED SHALL BE REPLACED WITH COLD APPLIED BITUMINOUS COATING COMPOUND. ALUMINUM SURFACES THAT ARE TO BE IN CONTACT WITH CONCRETE SHALL BE PAINTED WITH ONE COAT OF ZINC CHROMATE PRIMER OR TWO COATS OF ASPHALT. WATER MANAGEMENT PONDS, A MINIMUM OF A 25-FOOT RADIUS AROUND THE INLET STRUCTURE SHALL BE MATERIALS - (ALUMINUM PIPE) - THIS PIPE AND ITS APPURTENANCES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO SPECIFICATION M-196 OR M-21 L WITH WATERTIGHT COUPLING BANDS OR FLANGES. ALUMINUM PIPE, WHEN ALL CLEARED AND GRUBBED MATERIAL SHALL BE DISPOSED OF OUTSIDE AND BELOW THE LIMITS OF THE DRAIN AND RESERVOIR AS DIRECTED BY THE OWNER OR HIS REPRESENTATIVE. WHEN SPECIFIED, A SUFFICIENT QUANTITY OF TOPSOIL WILL BE STOCKPILED IN A SUITABLE LOCATION FOR USE ON THE USED WITH FLOWABLE FILL OR WHEN SOIL AND/OR WATER CONDITIONS WARRANT FOR INCREASED DURABILITY, SHALL E FULLY BITUMINOUS COATED PER REQUIREMENTS OF AASHTO SPECIFICATION M-190 TYPE A. ALUMINUM SURFACES EMBANKMENT AND OTHER DESIGNATED AREAS. THAT ARE TO BE IN CONTACT WITH CONCRETE SHALL BE PAINTED WITH ONE COAT OF ZINC CHROMATE PRIMER OR TWO COATS OF ASPHALT. HOT DIP GALVANIZED BOLTS MAY BE USED FOR CONNECTIONS. THE PH OF THE EARTH FILL SURROUNDING SOILS SHALL BE BETWEEN 4 AND 9. MATERIAL - THE FILL MATERIAL SHALL BE TAKEN FROM APPROVED DESIGNATED BORROW AREAS. IT SHALL 2. COUPLING BANDS, ANTI-SEEP COLLARS, END SECTIONS, ETC., MUST BE COMPOSED OF THE SAME MATERIAL AND COATINGS AS THE PIPE. METALS MUST BE INSULATED FROM DISSIMILAR MATERIALS WITH USE OF RUBBER OR PLASTIC INSULATING MATERIALS AT LEAST 24 MILS IN THICKNESS. BE FREE OF ROOTS, STUMPS, WOOD, RUBBISH, STONES GREATER THAN 6", FROZEN OR OTHER OBJECTIONABLE MATERIALS. FILL MATERIAL FOR THE CENTER OF THE EMBANKMENT AND CUT OFF TRENCH SHALL CONFORM TO UNIFIED SOIL CLASSIFICATION CC, SC, CH, OR CL AND MUST HAVE AT LEAST 30%, PASSING THE #200 SIEVE. CONSIDERATION MAY BE GIVEN TO THE USE OF OTHER MATERIALS IN THE 3. CONNECTIONS - ALL CONNECTIONS WITH PIPES MUST BE COMPLETELY WATERTIGHT. THE DRAIN PIPE OR BARREL EMBANKMENT IF DESIGNED BY A GEOTECHNICAL ENGINEER. SUCH SPECIAL DESIGNS MUST HAVE CONSTRUCTION SUPERVISED BY A GEOTECHNICAL ENGINEER. MATERIALS USED IN THE OUTER SHELL OF THE EMBANKMENT MUST HAVE THE CAPABILITY TO SUPPORT VEGETATION OF THE QUALITY REQUIRED TO PREVENT CONNECTION TO THE RISER SHALL BE WELDED ALL AROUND WHEN THE PIPE AND RISER ARE METAL. ANTI-SEEP COLLARS SHALL BE CONNECTED TO THE PIPE IN SUCH A MANNER AS TO BE COMPLETELY WATERTIGHT. DIMPLE ALL CONNECTIONS SHALL USE A RUBBER OR NEOPRENE GASKET WHEN JOINING PIPE SECTIONS. THE END OF EACH PLACEMENT - AREAS ON WHICH FILL IS TO BE PLACED SHALL BE SCARIFIED PRIOR TO PLACEMENT OF FILL. PIPE SHALL BE RE-ROLLED AN ADEQUATE NUMBER OF CORRUGATIONS TO ACCOMMODATE THE BANDWIDTH. THE FOLLOWING TYPE CONNECTIONS ARE ACCEPTABLE FOR PIPES LESS THAN 24 INCHES IN DIAMETER: FLANGES ON FILL MATERIALS SHALL BE PLACED IN MAXIMUM 8 INCH THICK (BEFORE COMPACTION) LAYERS WHICH ARE TO BE CONTINUOUS OVER THE ENTIRE LENGTH OF THE FILL. THE MOST PERMEABLE BORROW MATERIAL SHALL BE PLACED IN THE DOWNSTREAM PORTIONS OF THE EMBANKMENT. THE PRINCIPAL SPILLWAY MUST BE BOTH ENDS OF THE PIPE WITH A CIRCULAR 3/8 INCH CLOSED CELL NEOPRENE GASKET, PRE-PUNCHED TO THE FLANGE BOLT CIRCLE, SANDWICHED BETWEEN ADJACENT FLANGES; A 12-INCH WIDE STANDARD LAP TYPE BAND INSTALLED CONCURRENTLY WITH FILL PLACEMENT AND NOT EXCAVATED INTO THE EMBANKMENT. WITH 12-INCH WIDE BY 3/8-INCH THICK CLOSED CELL CIRCULAR NEOPRENE GASKET; AND A 12-INCH WIDE HUGGER COMPACTION - THE MOVEMENT OF THE HAULING AND SPREADING EQUIPMENT OVER THE FILL SHALL BE CONTROLLED SO THAT THE ENTIRE SURFACE OF EACH LIFT SHALL BE TRAVERSED BY NOT LESS THAN ONE TREAD TRACK OF HEAVY EQUIPMENT OR COMPACTION SHALL BE ACHIEVED BY A MINIMUM OF FOUR COMPLETE PASSES OF A SHEEPSFOOT, RUBBER TIRED OR VIBRATORY ROLLER. FILL MATERIAL SHALL CONTAIN SUFFICIENT MOISTURE SUCH THAT THE REQUIRED DEGREE OF COMPACTION WILL BE OBTAINED WITH THE EQUIPMENT USED. THE FILL MATERIAL SHALL CONTAIN SUFFICIENT MOISTURE SO THAT IF FORMED INTO A BALL IT WILL NOT CRUMBLE, YET NOT BE SO WET THAT WATER CAN BE SQUEEZED OUT. WHEN REQUIRED BY THE REVIEWING AGENCY THE MINIMUM REQUIRED DENSITY SHALL NOT BE LESS THAN 95% OF MAXIMUM DRY DENSITY WITH A MOISTURE CONTENT WITHIN 2% OF THE OPTIMUM. EACH LAYER OF FILL SHALL BE COMPACTED AS NECESSARY TO OBTAIN THAT DENSITY, AND IS TO BE CERTIFIED BY THE ENGINEER AT THE TIME OF CONSTRUCTION. ALL COMPACTION IS TO BE DETERMINED BY AASHTO METHOD T-99 (STANDARD CUT OFF TRENCH - THE CUTOFF TRENCH SHALL BE EXCAVATED INTO IMPERVIOUS MATERIAL ALONG OR PARALLEL TO THE CENTERLINE OF THE EMBANKMENT AS SHOWN ON THE PLANS. THE BOTTOM WIDTH OF THE TRENCH SHALL BE GOVERNED BY THE EQUIPMENT USED FOR EXCAVATION, WITH THE MINIMUM WIDTH BEING FOUR FEET. THE DEPTH SHALL BE AT LEAST FOUR FEET BELOW EXISTING GRADE OR AS SHOWN ON THE PLANS. THE SIDE SLOPES OF THE TRENCH SHALL BE 1 TO 1 OR FLATTER. THE BACKFILL SHALL BE COMPACTED WITH CONSTRUCTION EQUIPMENT, ROLLERS, OR HAND TAMPERS TO ASSURE MAXIMUM DENSITY AND MINIMUM PERMEABILITY. EMBANKMENT CORE - THE CORE SHALL BE PARALLEL TO THE CENTERLINE OF THE EMBANKMENT AS SHOWN ON THE PLANS. THE TOP WIDTH OF THE CORE SHALL BE A MINIMUM OF FOUR FEET. THE HEIGHT SHALL EXTEND UP TO AT LEAST THE 10 YEAR WATER ELEVATION OR AS SHOWN ON THE PLANS. THE SIDE SLOPES SHALL BE 1 TO 1 OR FLATTER, THE CORE SHALL BE COMPACTED WITH CONSTRUCTION EQUIPMENT, ROLLERS. OR HAND TAMPERS TO ASSURE MAXIMUM DENSITY AND MINIMUM PERMEABILITY. IN ADDITION, THE CORE SHALL BE PLACED CONCURRENTLY WITH THE OUTER SHELL OF THE EMBANKMENT BACKFILL ADJACENT TO PIPES OR STRUCTURES SHALL BE OF THE TYPE AND QUALITY CONFORMING TO THAT SPECIFIED FOR THE ADJOINING FILL MATERIAL THE FILL SHALL BE PLACED IN HORIZONTAL LAYERS NOT TO COMPACTED BY HAND TAMPERS OR OTHER MANUALLY DIRECTED COMPACTION EQUIPMENT. THE MATERIAL NEEDS TO FILL COMPLETELY ALL SPACES UNDER AND ADJACENT TO THE PIPE. AT NO TIME DURING THE BACKFILLING OPERATION SHALL DRIVEN EQUIPMENT BE ALLOWED TO OPERATE CLOSER THAN FOUR FEET, MEASURED HORIZONTALLY, TO ANY PART OF A STRUCTURE. UNDER NO CIRCUMSTANCES SHALL EQUIPMENT BE DRIVEN OVER ANY PART OF A CONCRETE STRUCTURE OR PIPE, UNLESS THERE IS A COMPACTED FILL OF 24" OR GREATER OVER THE STRUCTURE OR PIPE STRUCTURE BACKFILL MAY BE FLOWABLE FILL MEETING THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION, STATE HIGHWAY ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION 313 AS MODIFIED. THE MIXTURE SHALL HAVE A 100-200 PSI; 28 DAY UNCONFINED COMPRESSIVE STRENGTH. THE FLOWABLE FILL SHALL HAVE A MINIMUM PH OF 4.0 AND A MINIMUM RESISTIVITY OF 2,000 OHM-CM. MATERIAL SHALL BE PLACED SUCH THAT A MINIMUM OF 6" (MEASURED PERPENDICULAR TO THE OUTSIDE OF THE PIPE) OF FLOWABLE FILL SHALL BE UNDER (BEDDING), OVER AND, ON THE SIDES OF THE PIPE. IT ONLY NEEDS TO EXTEND UP TO THE SPRING LINE FOR RIGID CONDUITS. AVERAGE SLUMP OF THE FILL SHALL BE 7". TO ASSURE FLOWABILITY OF THE MATERIAL, ADEQUATE MEASURES SHALL BE TAKEN (SAND BAGS, ETC.). TO PREVENT FLOATING THE PIPE. WHEN USING FLOWABLE FILL, ALL METAL PIPE SHALL BE BITUMINOUS COATED. ANY ADJOINING SOIL FILL SHALL BE PLACED IN HORIZONTAL LAYERS NOT TO EXCEED FOUR INCHES IN THICKNESS AND COMPACTED BY HAND TAMPER OR OTHER MANUALLY DIRECTED COMPACTION EQUIPMENT. THE MATERIAL SHALL COMPLETELY FILL ALL VOIDS ADJACENT TO THE FLOWABLE FILL ZONE. AT NO TIME DURING THE BACKFILLING OPERATION SHALL DRIVEN EQUIPMENT BE ALLOWED TO OPERATE CLOSER THAN FOUR FEET, MEASURED HORIZONTALLY, TO ANY PART OF A STRUCTURE. UNDER NO CIRCUMSTANCES SHALL EQUIPMENT BE DRIVEN OVER ANY PART OF A STRUCTURE OR PIPE UNLESS THERE IS A COMPACTED FILL OF 24" OR GREATER OVER THE STRUCTURE OR PIPE. BACKFILL MATERIAL OUTSIDE THE STRUCTURAL BACKFILL CORE OF THE EMBANKMENT OR OTHER EMBANKMENT MATERIALS.

1500 FEET

TYPE BAND WITH 0-RING GASKETS HAVING A MINIMUM DIAMETER OF 1/2 INCH GREATER THAN THE CORRUGATION DEPTH, PIPES 24 INCHES IN DIAMETER AND LARGER SHALL BE CONNECTED BY A 24 INCH LONG ANNULAR CORRUGATED BAND USING A MINIMUM OF 4 (FOUR) RODS AND LUGS, 2 ON EACH CONNECTING PIPE END. A 24-INCH WIDE BY 3/8-INCH THICK CLOSED CELL CIRCULAR NEOPRENE GASKET WILL BE INSTALLED WITH 12 INCHES ON THE END OF EACH PIPE, FLANGED JOINTS WITH 3/8 INCH CLOSED CELL GASKETS THE FULL WIDTH OF THE HELICALLY CORRUGATED PIPE SHALL HAVE EITHER CONTINUOUSLY WELDED SEAMS OR HAVE LOCK SEARNS WITH INTERNAL CAULKING OR A NEOPRENE BEAD. 4. BEDDING — THE PIPE SHALL BE FIRMLY AND UNIFORMLY BEDDED THROUGHOUT ITS ENTIRE LENGTH. WHERE ROCK OR SOFT, SPONGY OR OTHER UNSTABLE SOIL IS ENCOUNTERED, ALL SUCH MATERIAL SHALL BE REMOVED AND REPLACED WITH SUITABLE EARTH COMPACTED TO PROVIDE ADEQUATE SUPPORT. 5. BACKFILLING SHALL CONFORM TO "STRUCTURE BACKFILL." 6. OTHER DETAILS (ANTI-SEEP COLLARS, VALVES, ETC.) SHALL BE AS SHOWN ON THE DRAWINGS. REINFORCED CONCRETE PIPE - ALL OF THE FOLLOWING CRITERIA SHALL APPLY FOR REINFORCED CONCRETE PIPE: 1. MATERIALS — REINFORCED CONCRETE PIPE SHALL HAVE BELL AND SPIGOT JOINTS WITH RUBBER GASKETS AND SHALL EQUAL OR EXCEED ASTM C-361. 2. BEDDING - REINFORCED CONCRETE PIPE CONDUITS SHALL BE LAID IN A CONCRETE BEDDING/CRADLE FOR THEIR ENTIRE LENGTH. THIS BEDDING/CRADLE SHALL CONSIST OF HIGH SLUMP CONCRETE PLACED UNDER THE PIPE AND UP THE SIDES OF THE PIPE AT LEAST 50% OF ITS OUTSIDE DIAMETER WITH A MINIMUM THICKNESS OF 6 INCHES. WHERE A CONCRETE CRADLE IS NOT NEEDED FOR STRUCTURAL REASONS, FLOWABLE FILL MAY BE USED AS DESCRIBED IN THE "STRUCTURE BACKFILL" SECTION OF THIS STANDARD. GRAVEL BEDDING IS NOT PERMITTED. 3. LAYING PIPE — BELL AND SPIGOT PIPE SHALL BE PLACED WITH THE BELL END UPSTREAM. JOINTS SHALL BE MADE IN ACCORDANCE WITH RECOMMENDATIONS OF THE MANUFACTURER OF THE MATERIAL. AFTER THE JOINTS ARE SEALED FOR THE ENTIRE LINE, THE BEDDING SHALL BE PLACED SO THAT ALL SPACES UNDER THE PIPE ARE FILLED. THE SHALL BE EXERCISED TO PREVENT ANY DEVIATION FROM THE ORIGINAL LINE AND GRADE OF THE PIPE. THE FIRST JOINT MUST BE LOCATED WITHIN 4 FEET FROM THE RISER. 4. BACKFILLING SHALL CONFORM TO "STRUCTURE BACKFILL." 5. OTHER DETAILS (ANTI-SEEP COLLARS, VALVES, ETC.) SHALL BE AS SHOWN ON THE DRAWINGS. PLASTIC PIPE - THE FOLLOWING CRITERIA SHALL APPLY FOR PLASTIC PIPE: 1. MATERIALS - PVC PIPE SHALL BE PVC-1120 OR PVC-1220 CONFORMING TO ASTM D1785 OR ASTM D-2241 CORRUGATED HIGH DENSITY POLYETHYLENE (HDPE) PIPE, COUPLINGS AND FITTINGS SHALL CONFORM TO THE FOLLOWING: 4" - 10" INCH PIPE SHALL MEET THE REQUIREMENTS OF AASHTO M252 TYPE S, AND 12" THROUGH 24" INCH SHALL MEET THE REQUIREMENTS OF AASHTO M294 TYPE S. 2. JOINTS AND CONNECTIONS TO ANTI-SEEP COLLARS SHALL BE COMPLETELY WATERTIGHT. 3. BEDDING -THE PIPE SHALL BE FIRMLY AND UNIFORMLY BEDDED THROUGHOUT ITS ENTIRE LENGTH. WHERE ROCK OR SOFT, SPONGY OR OTHER UNSTABLE SOIL IS ENCOUNTERED, ALL SUCH MATERIAL SHALL BE REMOVED AND REPLACED WITH SUITABLE EARTH COMPACTED TO PROVIDE ADEQUATE SUPPORT. 4. BACKFILL SHALL CONFORM TO "STRUCTURE BACKFILL." 5. OTHER DETAILS (ANTI-SEEP COLLARS, VALVES, ETC.) SHALL BE AS SHOWN ON THE DRAWINGS. DRAINAGE DIAPHRAGMS – WHEN A DRAINAGE DIAPHRAGM IS USED, A REGISTERED PROFESSIONAL ENGINEER WIL SUPERVISE THE DESIGN AND CONSTRUCTION INSPECTION.

# DRAINAGE CHANNELS, AND STREAM DIVERSIONS NECESSARY TO PROTECT THE AREAS TO BE OCCUPIED BY THE PERMANENT WORKS. THE CONTRACTOR SHALL ALSO FURNISH, INSTALL, OPERATE, AND MAINTAIN ALL NECESSARY PUMPING AND OTHER EQUIPMENT REQUIRED FOR REMOVAL OF WATER FROM VARIOUS PARTS OF THE WORK AND FOR MAINTAINING THE EXCAVATIONS, FOUNDATION, AND OTHER PARTS OF THE WORK FREE FROM WATER AS REQUIRED OR DIRECTED BY THE ENGINEER FOR CONSTRUCTING EACH PART OF THE WORK. AFTER HAVING SERVED THEIR PURPOSE, ALL TEMPORARY PROTECTIVE WORKS SHALL BE REMOVED OR LEVELED AND GRADED TO THE EXTENT REQUIRED TO PREVENT OBSTRUCTION IN ANY DEGREE WHATSOEVER OF THE FLOW OF WATER TO THE SPILLWAY OF OUTLET WORKS AND SO AS NOT TO INTERFERE IN ANY WAY WITH THE OPERATION OR MAINTENANCE OF THE STRUCTURE. STREAM DIVERSIONS SHALL BE MAINTAINED UNTIL THE FULL FLOW CAN BE PASSED THROUGH THE PERMANENT WORKS. THE REMOVAL OF WATER FROM THE REVIEW OF THE PASSED THROUGH THE PERMANENT WORKS. EXCAVATION AND THE FOUNDATION SHALL BE ACCOMPLISHED IN A MANNER AND TO THE EXTENT THAT WILL MAINTAIN STABILITY OF THE EXCAVATED SLOPES AND BOTTOM REQUIRED EXCAVATIONS AND WILL ALLOW SATISFACTORY PERFORMANCE OF ALL CONSTRUCTION OPERATIONS. DURING THE PLACING AND COMPACTING OF MATERIAL IN REQUIRED EXCAVATIONS, THE WATER LEVEL AT THE LOCATIONS BEING REFILLED SHALL BE MAINTAINED BELOW THE BOTTOM OF THE EXCAVATION AT SUCH LOCATIONS WHICH MAY REQUIRE DRAINING THE WATER SUMPS FROM WHICH THE WATER SHALL BE ALL BORROW AREAS SHALL BE GRADED TO PROVIDE PROPER DRAINAGE AND LEFT IN A SIGHTLY CONDITION ALL EXPOSED SURFACES OF THE EMBANKMENT, SPILLWAY, SPOIL AND BORROW AREAS, AND BERMS SHALL BE STABILIZED BY SEEDING, LIMING, FERTILIZING AND MULCHING IN ACCORDANCE WITH THE NATURAL RESOURCES CONSERVATION SERVICE STANDARDS AND SPECIFICATIONS FOR CRITICAL AREA PLANTING (MD-342) OR AS SHOWN ON THE ACCOMPANYING DRAWINGS. EROSION AND SEDIMENT CONTROL CONSTRUCTION OPERATIONS WILL BE CARRIED OUT IN SUCH A MANNER THAT EROSION WILL BE ONTROLLED AND WATER AND AIR POLLUTION MINIMIZED. STATE AND LOCAL LAWS CONCERNING POLLUTION ABATEMENT WILL BE FOLLOWED. CONSTRUCTION PLANS SHALL DETAIL EROSION AND SEDIMENT CONTROL MEASURES. LOB OF BOWNG NO. 15-1 CARSON AL La (1) 24 34 3

CONCRETE SHALL MEET THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION, STATE

ROCK RIPRAP SHALL MEET THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION,

GEOTEXTILE SHALL BE PLACED UNDER ALL RIPRAP AND SHALL MEET THE REQUIREMENTS OF MARYLAND DEPARTMENT OF TRANSPORTATION, STATE HIGHWAY ADMINISTRATION STANDARD

CONTRACTOR SHALL CONSTRUCT AND MAINTAIN ALL TEMPORARY DIKES, LEVEES, COFFERDAMS,

SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION 921.09, CLASS C.

CARE OF WATER DURING CONSTRUCTION

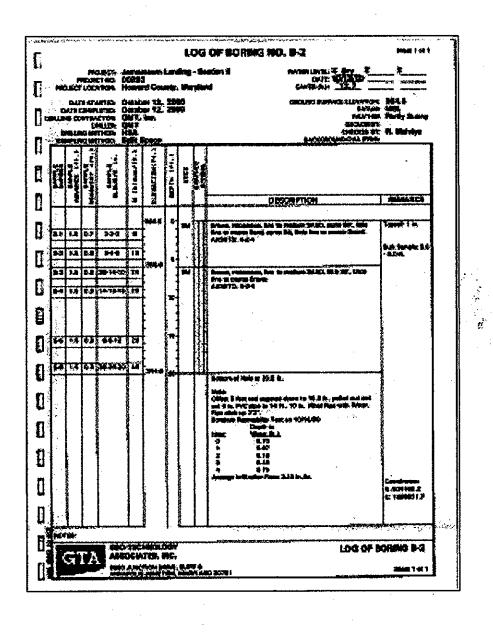
STATE HIGHWAY ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS,

ALL WORK ON PERMANENT STRUCTURES SHALL BE CARRIED OUT IN AREAS FREE FROM WATER. THE

HIGHWAY ADMINISTRATION STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MATERIALS, SECTION

CONCRETE

414, MIX NO. 3.



OPERATION. MAINTENANCE AND INSPECTION

INSPECTION OF THE POND(S) SHOWN HEREON SHALL BY PERFORMED AT LEAST ANNUALLY, IN

ARDS AND SPECIFICATIONS FOR PONDS\* (MD-378). THE POND OWNER(S) AND THE HEIRS

OWNER(S) SHALL PROMPTLY NOTIFY THE SOIL CONSERVATION DISTRICT OF ANY UNUSUAL

OBSERVATIONS THAT MAY BE INDICATIONS OF DISTRESS SUCH AS EXCESSIVE SEEPAGE,

OPERATION. MAINTENANCE SCHEDULE FOR PRIVATELY OWNED AND MAINTAINED

TURBID SEEPAGE, SLIDING OR SLUMPING.

SHALL BE MOWED AS NEEDED.

EXTENDED DETENTION POND

SHALL BE REPAIRED AS SOON AS IT IS NOTICED.

SUCCESSORS OR ASSIGNS SHALL BE RESPONSIBLE FOR THE SAFETY OF THE POND AND THE

CONTINUED OPERATION, SURVEILLANCE, INSPECTION AND MAINTENANCE THEREOF. THE POND

ROUTINE MAINTENANCE:
1. FACILITY SHALL BE INSPECTED ANNUALLY AND AFTER MAJOR STORMS. INSPECTIONS SHALL BE

2. TOP AND SIDE SLOPES OF EMBANKMENT SHALL ME MOWED A MINIMUM TWO (2) TIMES PER

PERFORMED DURING WET WEATHER TO DETERMINE IF THE POND IS FUNCTIONING PROPERLY.

3. DEBRIS AND LITTER SHALL BE REMOVED DURING REGULAR MOWING OPERATIONS AND AS NEEDED.

4. VISIBLE SIGNS OF EROSION IN THE POND AS WELL AS THE RIP-RAP OR CABION OUTLET AREA

NON-ROUTINE MAINTENANCE:

1. STRUCTURAL COMPONENTS OF THE POND SUCH AS THE DAM, THE RISER, AND THE PIPES

SHALL BE REPAIRED UPON THE DETECTION OF ANY DAMAGE. THE COMPONENTS SHALL BE INSPECTED DURING ROUTINE MAINTENANCE OPERATIONS.

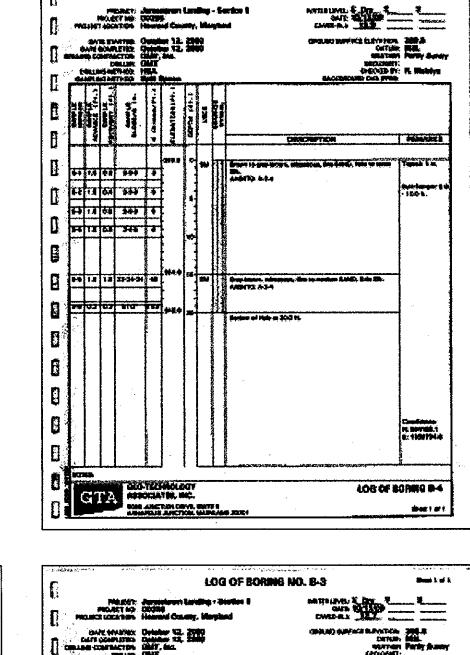
2. SEDIMENT SHALL BE REMOVED FROM THE POND, AND FOREBAY, NO LATER THAN WHEN THE

CAPACITY OF THE POND, OR FOREBAY, IS HALF FULL OF SEDIMENT, OR, WHEN DEEMED

NECESSARY FOR AESTHETIC REASONS, UPON APPROVAL FROM THE DEPARTMENT OF PUBLIC

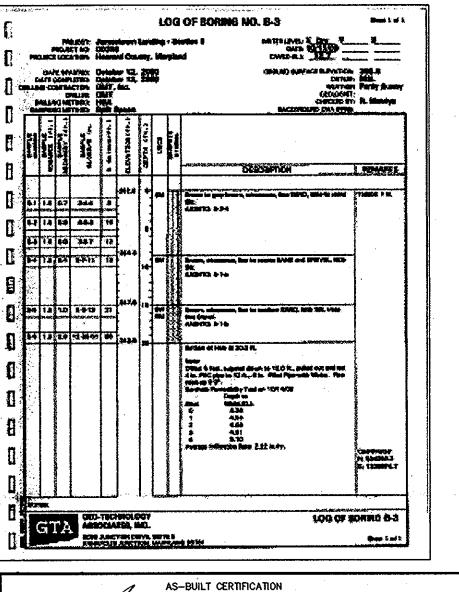
YEAR, ONCE IN JUNE AND ONCE IN SEPTEMBER. OTHER SIDE SLOPES AND MAINTENANCE ACCESS

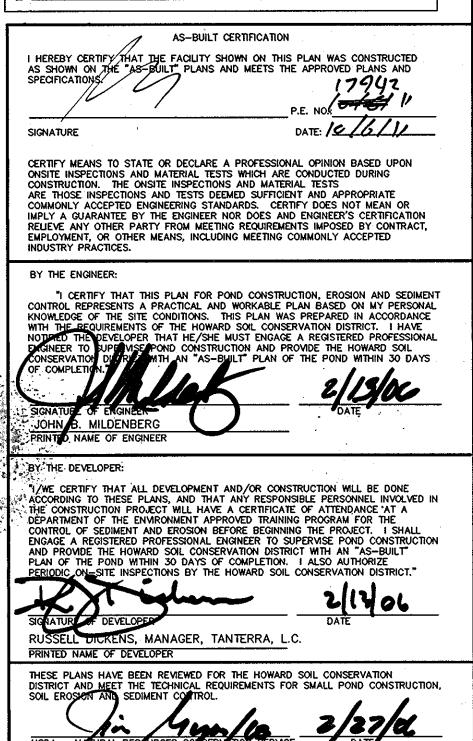
ACCORDANCE WITH THE CHECKLIST AND REQUIREMENTS CONTAINED WITHIN USDA. SCS "STAND-

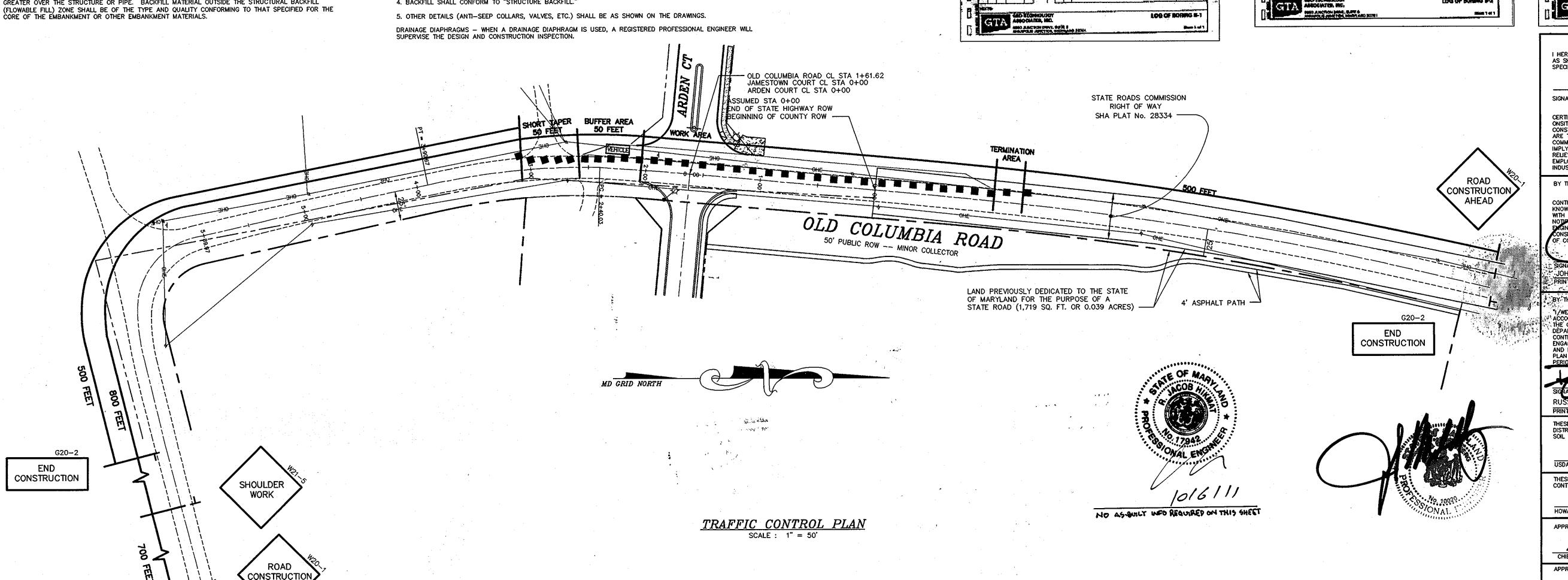


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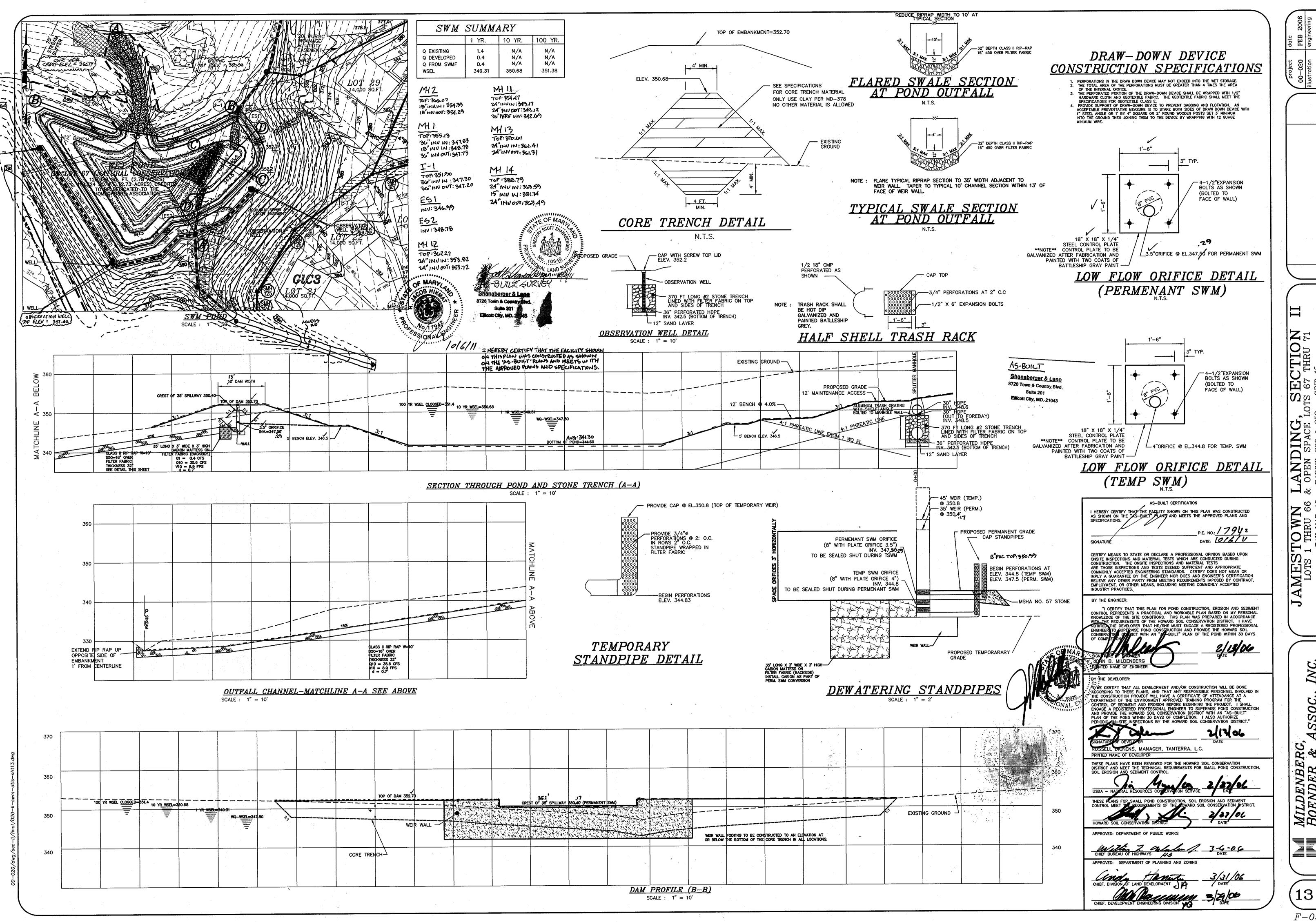
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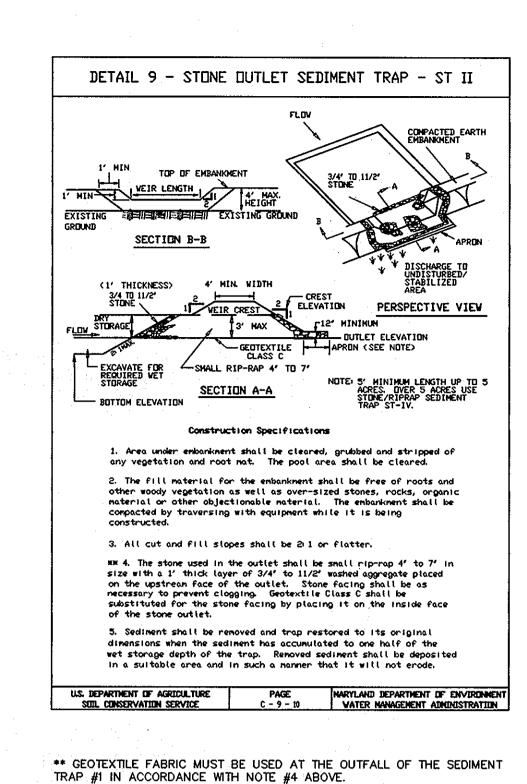
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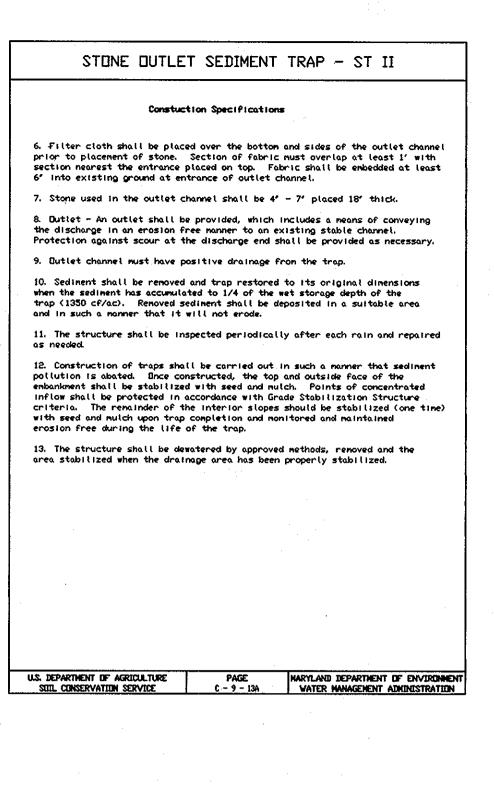
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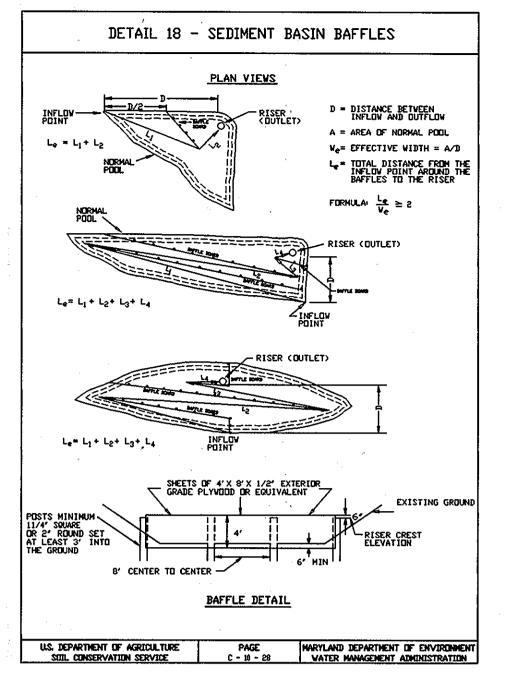


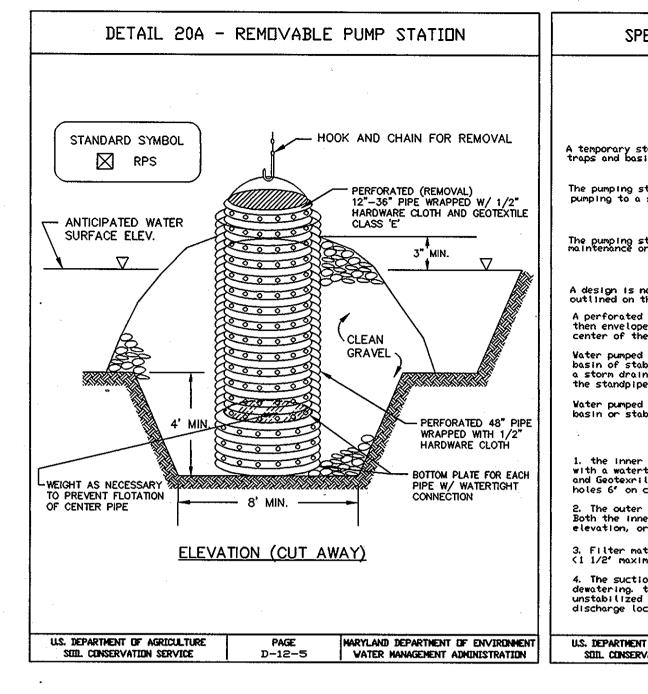
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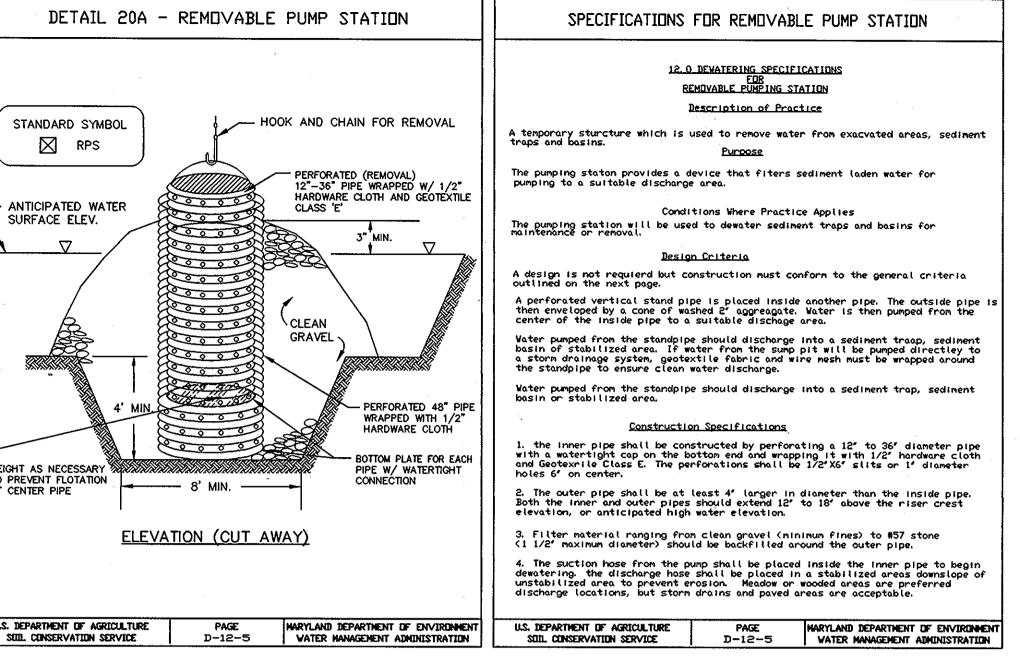
MILDENBERG, BOENDER & A



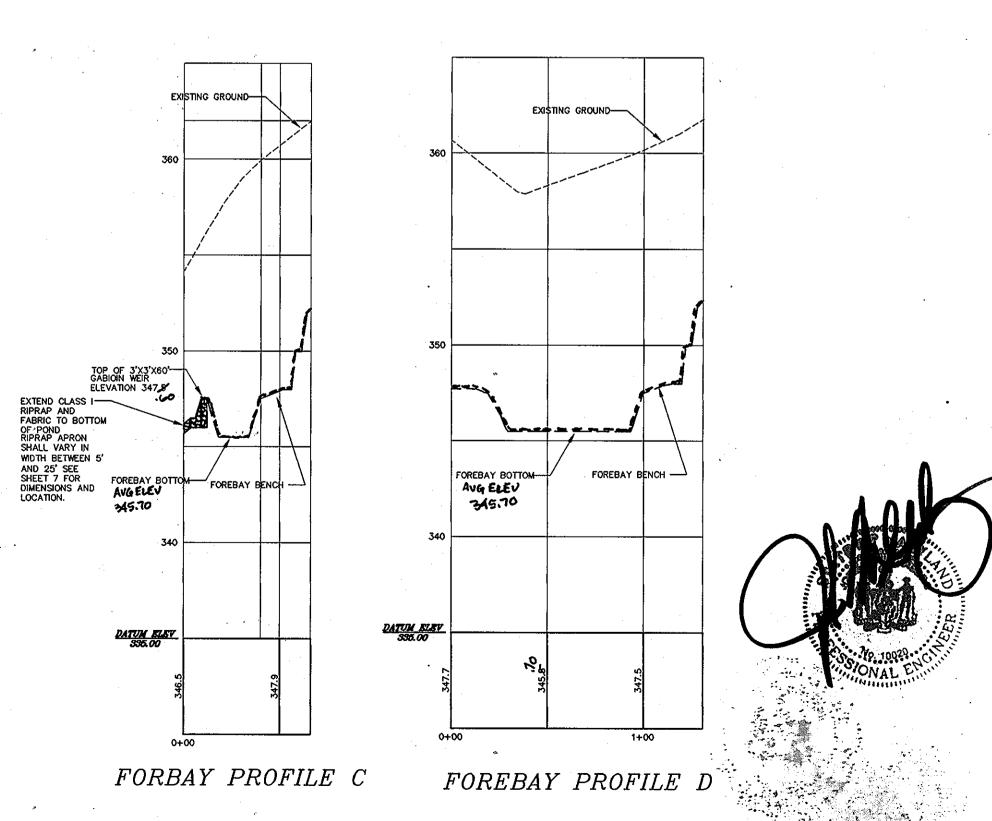


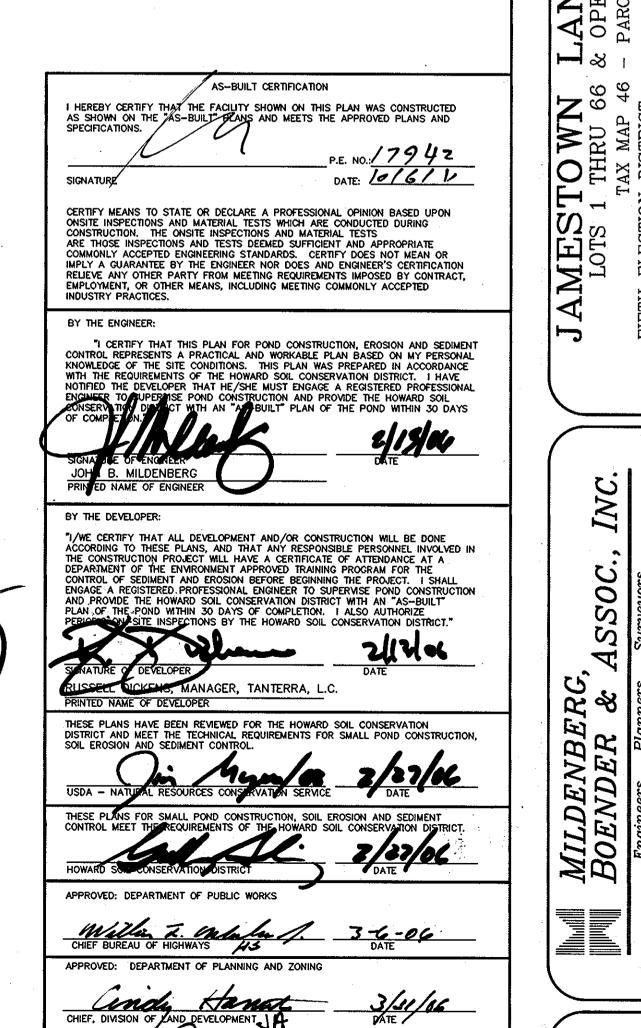






Shanaberger & Lane 8726 Town & Country Blvd. Sulte 201 EMcott City, MD. 21043





I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN WAS CONSTRUCTED AS SHOWN ON THE 'AS-BUILT' PLANS AND MEETS WITH THE APPROVED PLANS AND SPECIFICATIONS.

SOC.

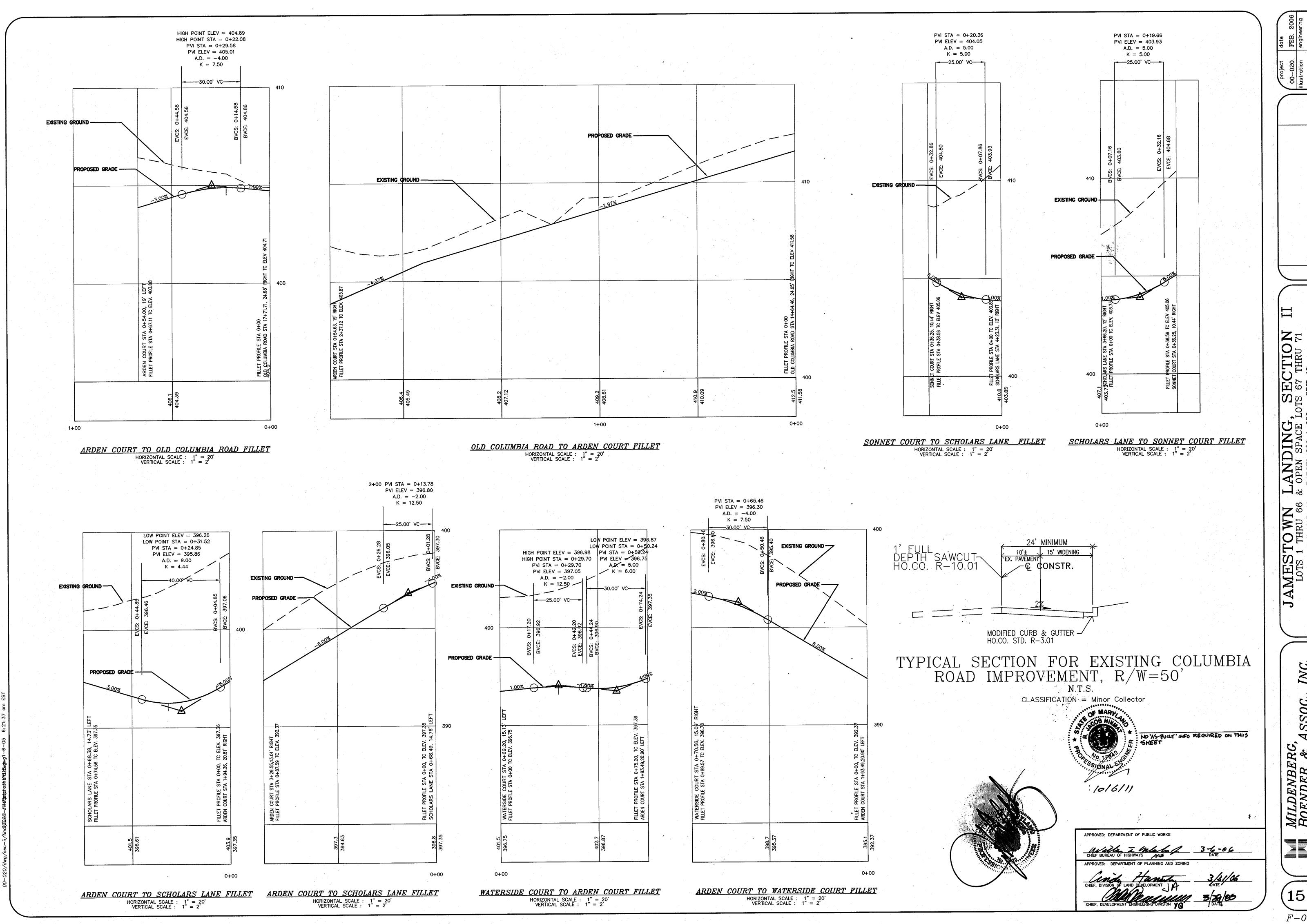
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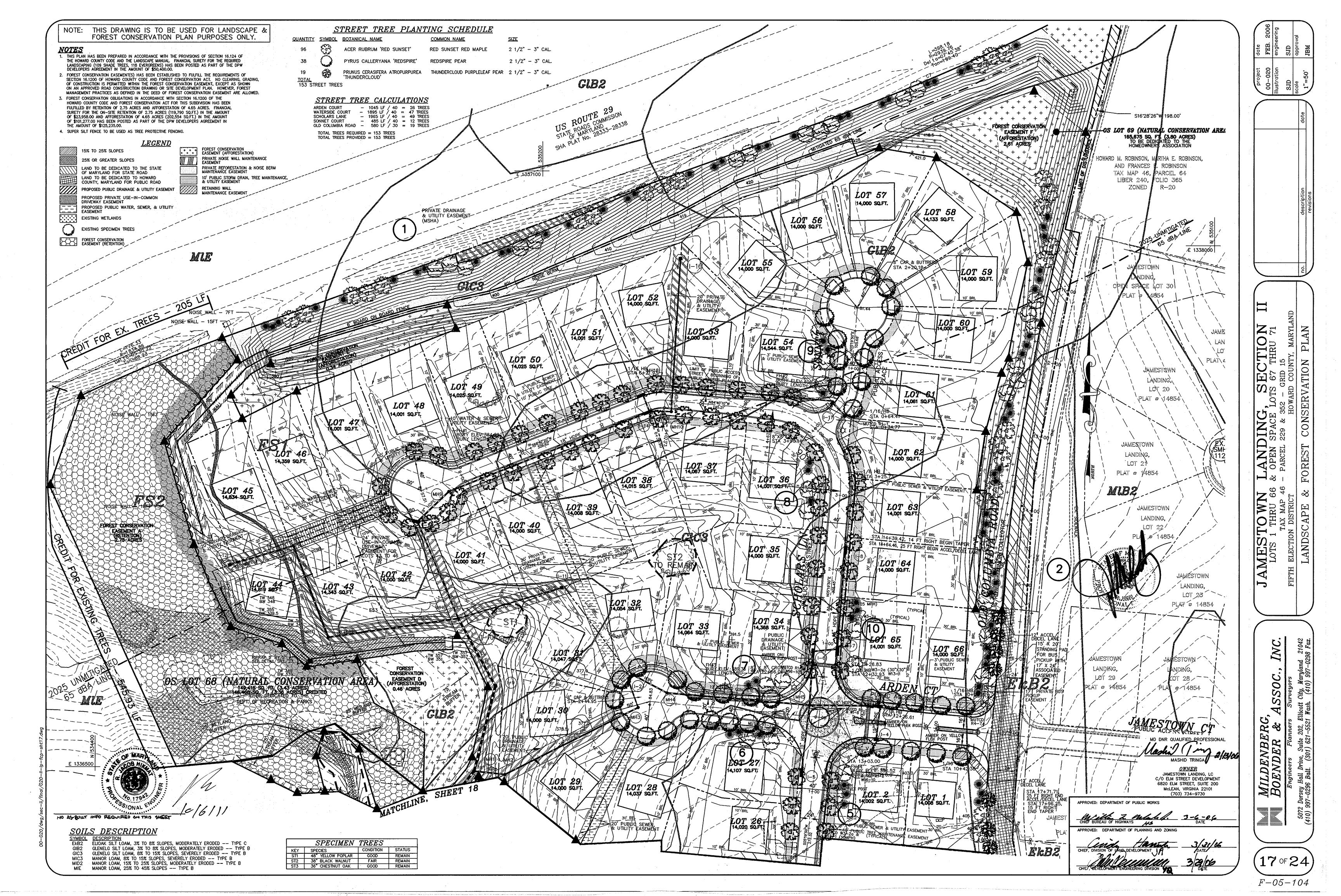


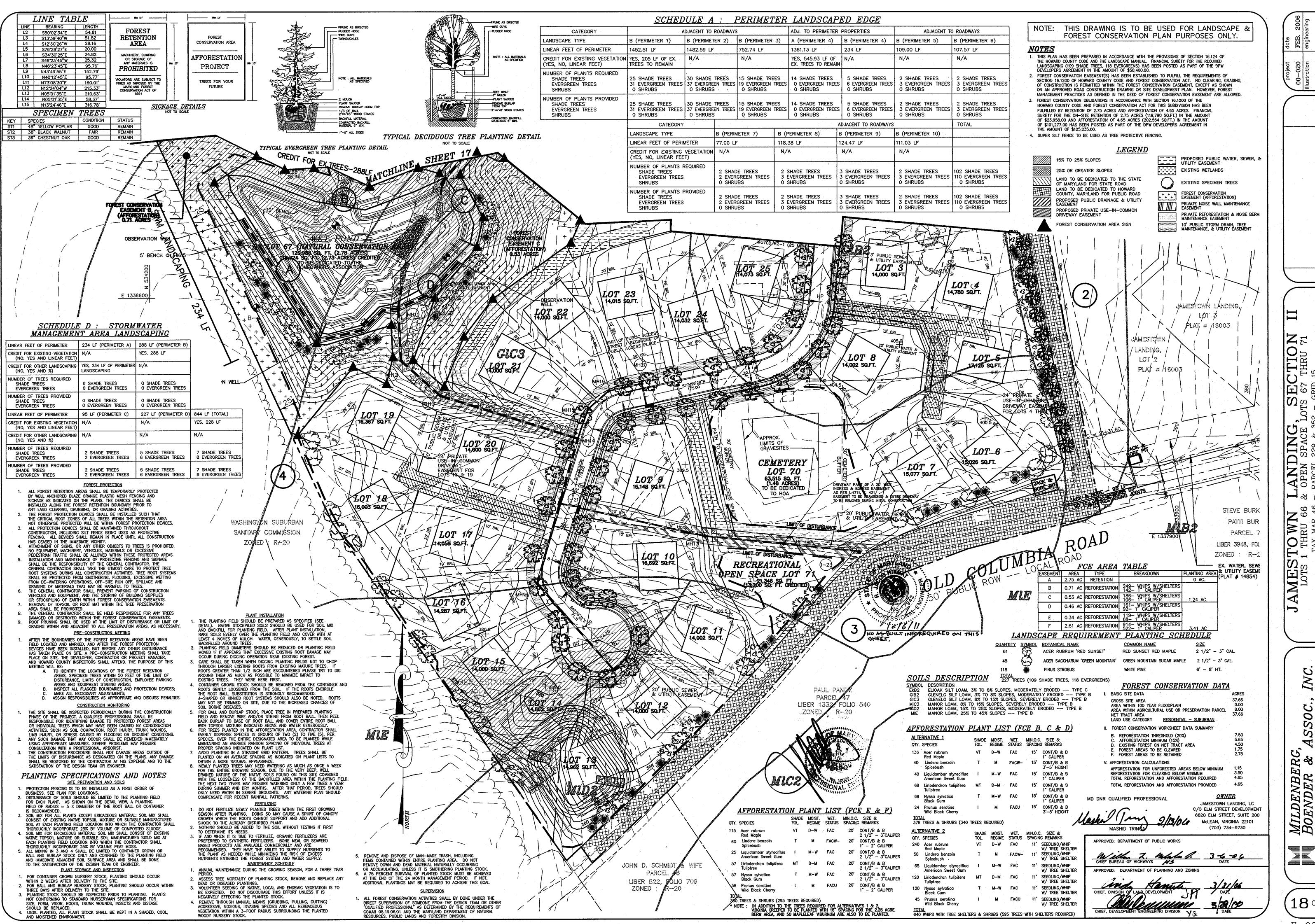
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#### STRUCTURAL NOTES

#### I. BUILDING CODES

- A. ALL CONSTRUCTION SHALL CONFORM WITH THE 2003 IBC BUILDING CODE AND ALL SUBSEQUENT SUPPLEMENTS.
- B. IN ADDITION, ALL CONSTRUCTION SHALL CONFORM WITH THE GOVERNING LOCAL BUILDING CODE.

#### 2. MISCELLANEOUS

- A. SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS MUST BE SUBMITTED BY THE CONTRACTOR OR OWNER FOR REVIEW BY THE ENGINEER. IF THE CONTRACTOR OR OWNER FAILS TO SUBMIT THE SHOP DRAWINGS, THE ENGINEER WILL NOT BE RESPONSIBLE FOR STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT. THE SHOP DRAWINGS SHALL INDICATE ANY DEVIATIONS OR OMISSIONS FROM THE CONTRACT DOCUMENTS. THE GENERAL CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMISSION AND MAKE ALL CORRECTIONS DEEMED NECESSARY.
- B. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS SHOWN ON THE CONTRACT DRAWINGS BEFORE PROCEEDING WITH CONSTRUCTION. ALL DISCREPANCIES AND OMISSIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.
- C. THE CONTRACTOR SHALL NOT SUBMIT REPRODUCTIONS OF THE STRUCTURAL CONTRACT DOCUMENTS AS SHOP DRAWINGS.
- D. SCALES SHOWN ON THE STRUCTURAL CONTRACT DRAWINGS ARE FOR GENERAL INFORMATION ONLY. DIMENSIONAL INFORMATION SHALL NOT BE OBTAINED BY SCALING THE DRAWINGS.

#### 3. RETAINING WALLS

- A. FOOTINGS FOR ALL RETAINING WALLS HAVE BEEN DESIGNED FOR AN ASSUMED NET ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. THE ALLOWABLE SOIL BEARING PRESSURE SHALL BE FIELD VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER AND APPROVED PRIOR TO PLACING FOOTINGS. SHOULD THE ACTUAL SOIL BEARING PRESSURE BE LESS THAN 2000 PSF, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER.
- B. RETAINING WALLS HAVE BEEN DESIGNED WITH BACKFILL MATERIAL HAVING THE FOLLOWING CHARACTERISTICS:
  - = 120 PCF = 30 DEGREES = 0.33

IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO INSURE THE BACK FILL MATERIAL MEETS THESE CHARACTERISTICS AND COMPLIES WITH THE MD-378 CORE SPECIFICATIONS. FILL UNDER THE WEIR WALL SHALL CONFORM TO SC, CH OR CL. ALL OTHER MATERIALS ARE NOT PERMITTED.

- C. RETAINING WALLS HAVE BEEN DESIGNED FOR THE FOLLOWING MINIMUM FACTORS OF SAFETY:
- SLIDING 1.5 OVERTURNING 2.0
- D. CONSTRUCTION OF ALL RETAINING WALLS SHALL BE PERFORMED UNDER THE SUPERVISION OF A REGISTERED GEOTECHINCAL ENGINEER.
- E. ALL RETAINING WALLS SHALL BE BRACED AND SHORED AS REQUIRED DURING BACKFILLING. BOTH SUPPORTING ELEMENTS SHALL BE IN PLACE AND DEVELOPING FULL REQUIRED STRENGTH PRIOR TO BACK FILLING OF WALLS SUPPORTED AT TOP AND BOTTOM.

#### 4. STRUCTURAL FILL

- A. NEW FILL MATERIAL AND EXISTING BASE MATERIAL SHALL BE FREE OF ALL REFUSE, DEBRIS, AND ORGANIC MATTER AND SHALL BE APPROVED FOR USE BY A REGISTERED GEOTECHNICAL ENGINEER.
- B. FILL MATERIAL SHALL BE DEPOSITED IN 8 INCH MAXIMUM LOOSE LIFTS AND COMPACTED TO A DRY DENSITY OF AT LEAST 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D 698. FILL SHALL BE PLACED AND COMPACTED IN 8 INCH LOOSE LIFTS TO DESIRED FINISHED GRADE UNDER THE GUIDANCE AND OBSERVATION OF A PROFESSIONAL GEOTECHNICAL ENGINEER REGISTERED IN THE STATE OF MARYLAND.
- C. WHEN WORK IS INTERRUPTED BY RAIN, FILL OPERATIONS SHALL NOT RESUME UNTIL FIELD TESTS INDICATE THAT THE MOISTURE CONTENT AND SOIL DENSITY OF THE TOP 8 INCHES OF FILL IS WITHIN THE LIMITS SPECIFIED.
- D. ALL FILL MATERIAL SHALL BE PLACED IN SUCH A MANNER THAT THE SURFACE IS SLOPED TO PREVENT THE PONDING OF WATER.

#### 5. CAST IN PLACE CONCRETE

- A. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301)"; AND TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318)".
- B. IN ADDITION TO THE ABOVE, ALL CONCRETE WORK SHALL CONFORM TO THE FOLLOWING:
  - I. RECOMMENDED PRACTICE FOR HOT WEATHER CONCRETING (ACI 305).
- 2. RECOMMENDED PRACTICE FOR COLD WEATHER CONCRETING
- 3. RECOMMENDED PRACTICE FOR CONCRETE FORMWORK (ACI 341).
- C. ALL CONCRETE EXPOSED TO PUBLIC VIEW SHALL CONFORM TO THE REQUIREMENTS FOR ARCHITECTURAL CONCRETE CONTAINED IN ACI 301.
- D. ALL CONCRETE, UNLESS NOTED OTHERWISE, SHALL BE STONE AGGREGATE CONCRETE HAVING A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI. ALL CONCRETE EXPOSED TO WEATHER SHALL HAVE AN AIR ENTRAINMENT OF 5% +/- I%. NO ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL BE PERMITTED. MAXIMUM AGGREGATE SIZE SHALL BE I", AND MAXIMUM SLUMP SHALL BE 4", 3" FOR SLABS ON GRADE. ALL CONCRETE, EXCEPT FOOTINGS, SHALL CONTAIN A WATER REDUCING ADMIXTURE. PORTLAND CEMENT SHALL CONFORM TO ASTM C 150 AND NORMAL WEIGHT AGGREGATES SHALL CONFORM TO ASTM C 33.
- E. ALL REINFORCING BARS SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A 615 GRADE 60. ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A 185. LAP ALL REINFORCING BARS A MINIMUM OF 48 BAR DIAMETERS AND ALL W.W.F. A MINIMUM OF TWO FULL GRIDS, UNLESS OTHERWISE INDICATED.
- F. ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH THE CRSI "MANUAL OF STANDARD PRACTICE", ACI 315" DETAILS AND DETAILING OF CONCRETE REINFORCEMENT", ACI SP 66 "DETAILING MANUAL".

- G. ALL CONCRETE MIX DESIGNS, INCLUDING CEMENT CONTENT, WATER CEMENT RATIO, FINE AND COARSE AGGREGATE CONTENT AND ALL ADMIXTURES, SHALL BE REVIEWED BY ENGINEER PRIOR TO PLACING FIRST CONCRETE.
- H. ALL CONCRETE SHALL BE SAMPLED AND TESTED BY THE TESTING AGENCY. THE CONTRACTOR SHALL NOTIFY THE TESTING AGENCY 48 HOURS PRIOR TO THE PLACING OF ANY CONCRETE.
- I. THE CONCRETE STRUCTURE SHALL NOT SUPPORT THE DESIGN LIVE LOAD FOR A MINIMUM OF 28 DAYS AND ALL SHORING AND RESHORING REQUIRED TO SUPPORT THE CONCRETE STRUCTURE DURING CONSTRUCTION SHALL BE DESIGNED AND PROVIDED BY THE CONTRACTOR. SHOP DRAWINGS, SIGNED AND SEALED BY A REGISTERED ENGINEER IN THE STATE OF MARYLAND, SHALL BE SUBMITTED FOR REVIEW. SHOP DRAWINGS SHALL INDICATE THE TYPE, EXTENT, SIZE, AND LOCATION OF ALL SHORING AND RESHORING AS WELL AS THE SEQUENCE OF CONSTRUCTION.
- J. GROUND BLAST FURNACE SLAG MAY BE USED TO REPLACE UP TO 50 PERCENT OF THE PORTLAND CEMENT IN A CONCRETE MIX, AND FLY ASH OR POZZOLAN MAY BE USED TO REPLACE UP TO 25 PERCENT OF PORTLAND CEMENT, SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER AND SHALL CONFORM TO ASTM C 989.
- K. MINIMUM COVER FOR ALL REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE INDICATED:

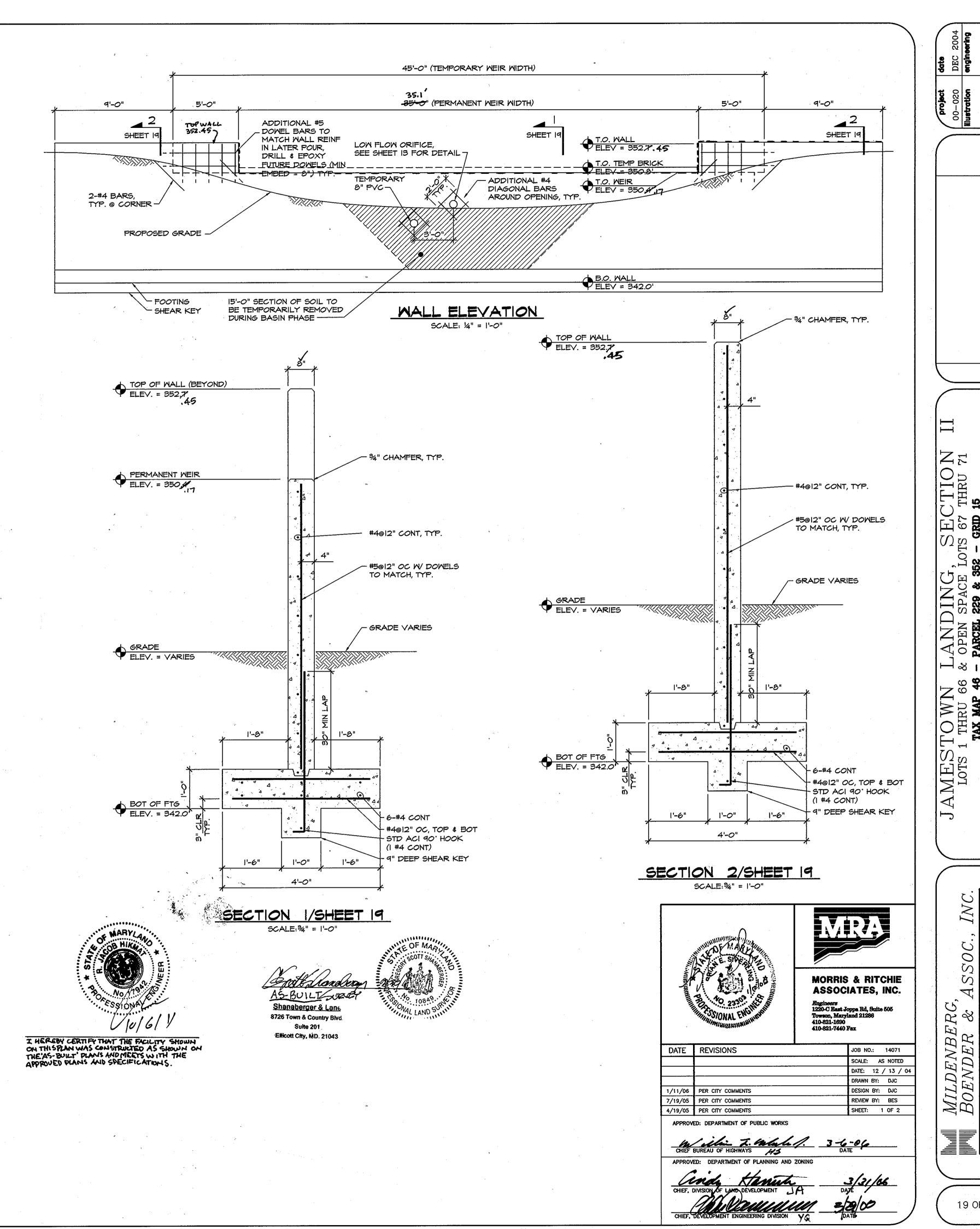
FOUNDATIONS 3 INCHES
WALLS 3/4 INCHES
WALLS BELOW GRADE 2 INCHES

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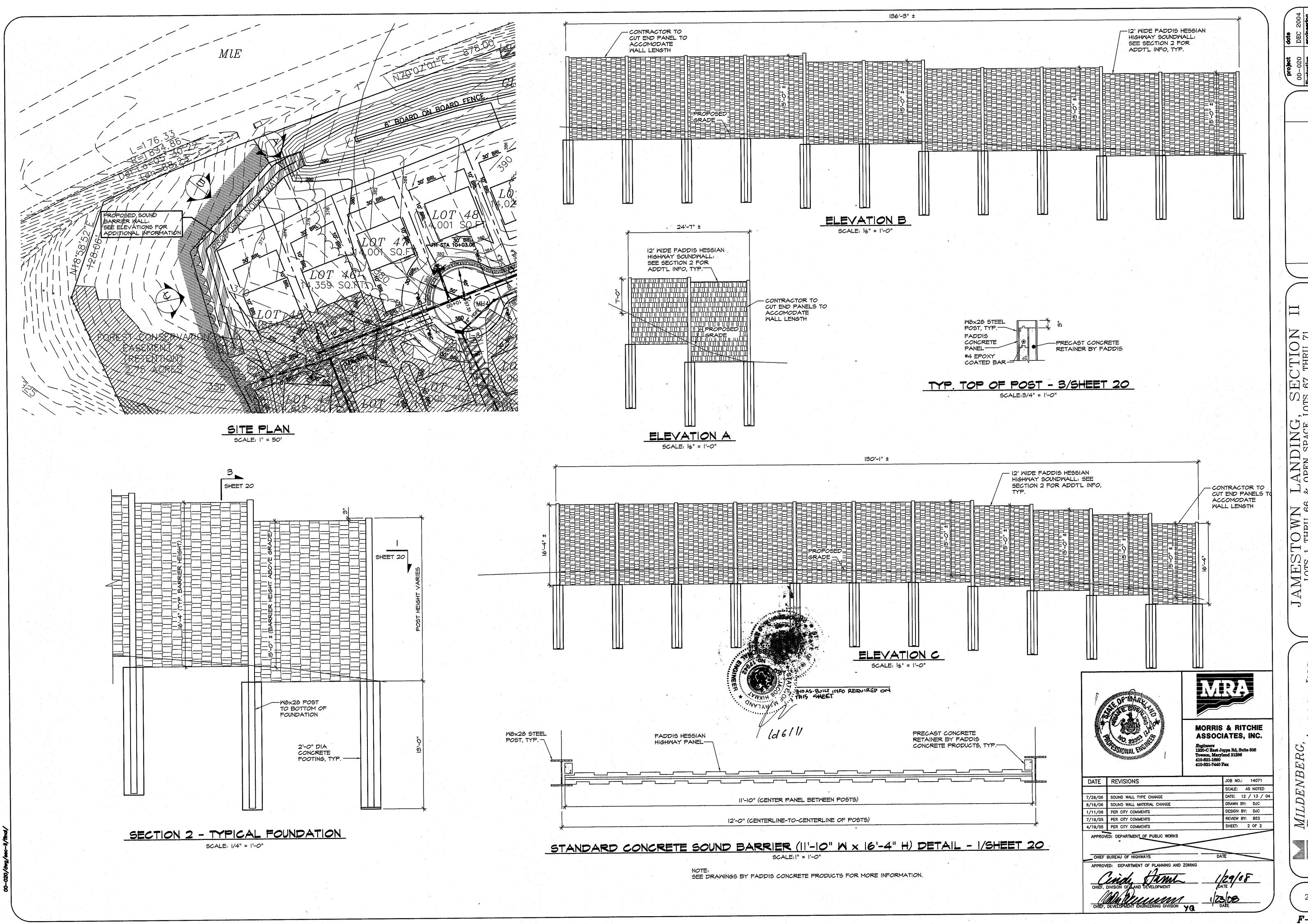
L. THE GENERAL CONTRACTOR SHALL SUBMIT PLANS SHOWING ALL PENETRATIONS THROUGH THE FRAMED CONCRETE SLABS. THE OPENINGS SHALL BE ACCURATELY LOCATED AND DIMENSIONED.

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#### SEGMENTAL RETAINING WALL SPECIFICATIONS

#### PART 1 - GENERAL

- 1.1 Work includes furnishing and installing segmental retaining wall units, geogrid reinforcement, wall fill, and backfill to the lines and grades shown on the construction drawings and as specified herein. The contract also includes the furnishing and installing all appurtenant materials, equipment, and labor required for construction of the geogrid reinforced, segmental retaining wall. All existing and proposed construction and site grading information was referenced from the digital plan entitled "020-||-finalbase-alt.dwg", provided by Mildenberg, Boender & Associates, Inc. received April 12, 2005.
- 1.2 The installation of the 18" HDPE pipe behind the proposed retaining wall should be coordinated with the construction of the wall. The pipe must be installed concurrently or prior to construction

#### 1.2 REFERENCE STANDARDS

- A. ASTM C90-75 (1981 rev) Hollow Load Bearing Masonry Units B. ASTM C140-75 (1981 rev) - Sampling and Testing Concrete Masonry Units
- C. ASTM C145-75 (1981 rev) Solid Load Bearing Concrete Masonry Units
- D. Geosynthetic Research Institute (GRI), GRI-GG4 Determination of Long Term Design Strength
- E. ASTM D 638 Test Method for Tensile Properties of Plastic F. ASTM D 1248 - Specification of Polyethylene Plastics Molding and Extrusion Materials
- G. ASTM D 4218 Test Method for Carbon Black Content in Polyethylene Compounds by the
- Muffle Furnace Technique H. ASTM D 3034 - Specification for Polyvinyl Chloride (PVC) Pipe
- I. ASTM C 1372 Specifications for Segmental Retaining Wall Units

#### 1.3 DELIVERY, STORAGE AND HANDLING

- A. Contractor should check the materials upon delivery to assure that proper material has
- B. Contractor should prevent excessive mud, wet cement, epoxy, and like materials which may affix themselves, from coming in contact with the materials.
- C. Geogrids should be stored above -20 degrees F.
- D. Contractor should protect the materials from damage. Damaged material should not be incorporated into the reinforced retaining wall.

#### 1.4 SUBMITTALS/CERTIFICATION

The contractor shall submit a Manufacturer's certification, prior to the start of the work, that the retaining wall system components meet the requirements of ASTM C 1372 and other requirements specified herein. This certification should be provided to the geotechnical engineer for review and approval prior to wall construction.

#### PART 2 - PRODUCTS

#### 2.1 DEFINITIONS

- A. Geogrid is a high density polyethylene, polyester, or polypropylene grid, specifically fabricated for use as a soil reinforcement.
- B. Concrete retaining wall units are as detailed on the drawings and as specified herein. C. Geosynthetic Drainage Composites are polyethylene net structure with non-woven geotextiles
- bonded to both sides.
- D. Erosion Control Blankets consist of a web of polyolefin fibers securely bounded by polyolefin threads between two high strength polyolefin nets.
- E. Backfill is the soil which is used as fill for the reinforced soil mass.
- F. Foundation soil is the in-situ soil or controlled compacted fill placed below the bottom of the retaining wall and geogrid zone.

#### 2.2 MATERIALS

The contractor should submit manufacturer's catalog and samples of the proposed materials for approval by the project geotechnical engineer a minimum of seven days before the start of construction. Materials should be transported to the site only after approval of the proposed materials by the project geotechnical engineer.

#### A. Concrete Units

- 1. Masonry units should be Keystone Standard II Retaining Wall Units. Substitution of other concrete units may be allowed with the prior approval of the Geotechnical Engineer.
- 2. Concrete wall units should have a minimum 28 day compressive strength of 3000 psi, in accordance with ASTM C-90. The concrete should have adequate freeze/thaw protection with
- a maximum moisture absorption of 6 percent. 3. Modular concrete materials shall conform to the requirements of ASTM C 1372 - Standard
- Specifications for Segmental Retaining Wall Units. 4. The units shall pass 100 freeze/thaw cycles in water with less than 1% weight loss in accordance with ASTM C 1372.
- 5. Exterior dimensions may vary. Units are required to have a minimum of one square foot of
- face area each.
  6. Units should have angled sides and be capable of attaining concave and convex alignment curves in accordance with manufacturer's recommendations.
- 7. Units should be interlocked with non-corrosive reinforced fiberglass pins
- 8. Units should be interlocked as to provide a maximum of 1 inch of setback per block, where required.

#### B. Leveling Pad

Material for leveling pad/footing should consist of compacted free-draining coarse aggregates meeting the requirements of ASTM #57 Stone or Graded Aggregate Base (GAB) per Maryland State Highway Administration Standard Specifications for Construction and Materials. A minimum of 6 inches deep and 22 inches wide compacted leveling pad is required.

#### C. Fiberglass Connecting pins

- 1. Thermoset isopthalic polyester resin pultruded fiberglass reinforcement rods, a minimum one—half inch in diameter.
- 2. Pins should have a minimum flexural strength of 128,000 psi and short beam shear of 6400 psi. 3. For substitute concrete units, use of other compatible connector systems may be allowed with

# the prior approval of the geotechnical engineer.

Geogrid should be Miragrid 10XT or equivalent as approved by the geotechnical engineer. The geogrid should have an allowable strength of 2619 pounds per foot. The allowable strength is defined as the Ultimate Strength divided by reduction factors for creep, durability, installation damage and an overall factor of safety.

#### E. Reinforced Backfill

Reinforced backfill soils for the wall should be non-plastic, controlled fill meeting the requirements of AASHTO A-2-4 or more granular, unless noted otherwise. The geotechnical report for the project indicates that A-2-4 material is present on site. If adequate quantities of this material are not available on-site, imported backfill should meet the above requirements and should be approved by the geotechnical engineer.

#### F. Controlled Fill

Controlled Fill soils to be placed outside the Reinforced Backfill area and where specified should consist of on—site or borrow soils meeting the requirements of AASHTO A—4 or more granular. All fill materials proposed to be placed behind the reinforced backfill should be placed as controlled fill compacted to 95 percent of maximum dry density in accordance with the Standard Proctor, ASTM D-698.

#### G. Low-Permeability Soil

Low-permeability soils to be placed at the top of the wall where specified should consist of sandy, silty or clayey soils meeting the requirements of ML, CL, SM, or SC with a minimum of 25% passing the #200 sieve.

#### H. Drainage Pipe

The drainage pipes should be perforated or slotted PVC pipe manufactured in accordance with ASTM D-3034.

#### I. Filter Fabric

Filter Fabric should be non-woven, polypropylene geotextile, 140 N manufactured by Nicolon Mirafi Group or approved equivalent.

# J. Erosion Control Blanket

Erosion Control Blanket should be Tensar TB 1000 manufactured by the Tensar Corporation or approved equivalent.

The Drainage GeoComposite should be DC4200 Geotextile, manufactured by Tenax Corporation, or approved equivalent. Depending upon the construction sequence and the soil types used for backfill, it may be feasible to eliminate the drainage composite, with the prior approval of GTA.

## L. Yard Drain

The Yard Drain should be a Nyloplast in-line grate connected to an 8-inch PVC pipe, or approved equivalent

#### PART 3 - EXECUTION

A. Excavation

- 1. The contractor should excavate to the lines and grades shown on the construction drawings.

  Under no circumstances should the excavation lines and grades be exceeded, except with owner's approval. The contractor should protect the excavation from sloughing by placing a membrane
- over the face of the excavation 2. Prior to retaining wall construction and the placement of fill, all topsoil should be stripped and removed from the site.
- 3. Excavations should be sloped or otherwise supported in accordance with Occupation Safety and Health Administration (OSHA) and other local and state regulations.

#### B. Foundation Subgrade Preparation

- 1. Foundation soil should be excavated as required for installation of leveling pad, geogrid and other elements and as shown on the construction drawings.
- Foundation soil should be examined by the Engineer to assure that the actual foundation soil strength meets or exceeds assumed design strength. Soils not meeting required strength should be removed and replaced with controlled, compacted material.
- 3. Over—excavated areas should be filled with select and approved material and compacted to 95 percent of maximum dry density in accordance with the Standard Proctor, ASTM D-698.
- 4. Allowable bearing pressure for natural and controlled, compacted fill soils should be as specified
- 5. The exposed foundation subgrade should be proofrolled with a loaded dump truck. Any soft or unstable areas identified during proofrolling should be overexcavated and backfilled with Controlled Fill. 6. Any fills required to establish sloping surfaces in front of the walls should consist of Controlled Fill and should be placed, compacted and field tested in accordance with the requirements
- specified herein.
- 1. The leveling pad should be placed as shown on the construction drawings with a minimum thickness of 6 inches.
- 2. Leveling pad materials should be installed upon undisturbed in-situ soils or controlled,
- Leveling Pad should be prepared to insure complete contact of retaining wall unit with base. Gaps should not be allowed.

#### D. Unit Installation

C. Leveling Pad

- 1. First course of concrete wall units should be placed on the leveling pad. The units should be checked for level and alignment. The first course is the most important to provide accurate checked for level and alignment.
- and acceptable results. 2. Insure that units are in full contact with base.
- 3. Units are placed side by side for full length of wall alignment. Alignment may be done by means of a string line or offset from base line.
- 4. Install fiberglass connecting pin.
- 5. Lay up each course insuring that the connecting pins are inserted through front slot of the unit, and into the receiving slot in the course beneath. Repeat procedure to the extent of
- 6. At the end of each course where the wall changes elevation, units should be turned into the backfill. Units should be laid as to create the minimum radius possible. Unless otherwise shown on the drawings, a minimum of one unit should be installed into the grade. Only the front face of the units should be visible from the side of the wall.
- 7. Convex and concave curves should be made using compac units or by trimming the Standard II units as required in accordance with manufacturer's recommendations.
- 8. Cap units should be installed and bonded with construction adhesive or epoxy cement as required by manufacturer.

9. Contractor should provide positive drainage for the back of the retaining wall during construction.

- 1. All utilities in the vicinity of any retaining wall or geogrid reinforcement must be installed and properly backfilled prior to placing the geogrid soil reinforcement or constructing the wall.
- 2. The geogrid soil reinforcement should be laid horizontally on compacted backfill, connected to the concrete wall units. Hook grid over the fiberglass connecting pin, pull taut, and anchor before backfill is placed on the geogrid.
- 3. Slack in the geogrid at the wall unit connections should be removed in a manner, and to such a degree, as approved by the Engineer.
- 4. Geogrid should be laid at the proper elevation and orientation as shown on the construction
- drawings or as directed by the Engineer. 5. Correct orientation (roll direction) of the geogrid should be verified by the Contractor.
- 6. Geogrid should be secured in-place with staples, pins, sand bags, or backfill as required by fill properties, fill placement procedures, or weather conditions, or as directed by the Engineer.
- a. Uniaxial geogrid does not need to be overlapped in the across the roll direction, except to contain the fill at the slope face when wrap—around facing is used. Uniaxial grid should be overlapped 48" in the rolled direction.
- b. A layer of soil a minimum of 4 inches in thickness should be spread between uniaxial geogrid layers in the area to be overlapped, or as directed.

#### F. Fill Placement

- 1. Wall backfill material should be placed in no more than 8—inch lifts and compacted to 95 percent of the Standard Proctor (ASTM D-698).
- 2. Backfill should be placed, spread, and compacted in such a manner that minimizes the development of wrinkles in and/or movement of the geogrid.
- 3. Only hand-operated compaction equipment should be allowed within 4 feet of the wall face.
- 4. Backfill should be placed from the wall outward to insure that the geogrid remains taut. 5. Tracked construction equipment should not be operated behind or above the wall.
- 6. Rubber—tired equipment may pass over the geogrid reinforcement at slow speeds, less than 10 MPH. Sudden braking and sharp turning should be avoided. 7. Place filter fabric between the unit core fill and the reinforced backfill as shown on plans.
- The filter fabric should be embedded a minimum of two feet into the reinforced fill.
- 8. The finished sloping surface on the toe side of retaining walls should be protected by installing the permanent erosion control blanket and loaming and seeding in accordance with project requirements.

#### G. DRAINAGE

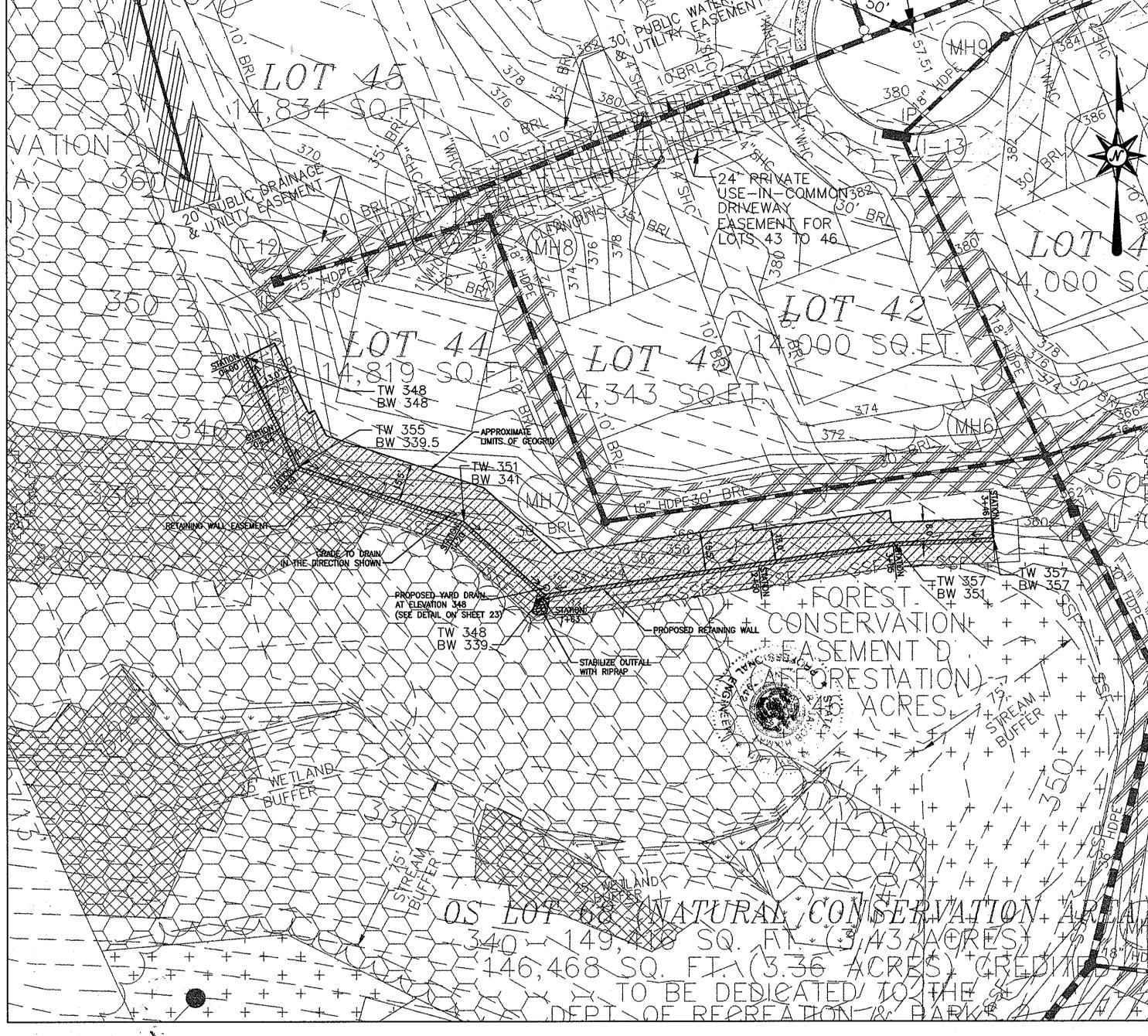
- 1. Drainage fill should be placed behind the wall to the limits shown. The drainage fill should be a minimum of 12—inches thick. The drainage fill should be ASTM #57 stone. The drainage fill should be wrapped in filter fabric (Mirafi 140N or equal) as shown on the drawings.
- 2. Positive drainage should be maintained during and after construction. Soils within the reinforced zone that become wet during construction should be dried to optimum moisture or removed.
- 3. Install the perforated drainage pipes and lateral drainage pipes incrementally along with the installation of concrete units and placement of fill.
- 4. To connect the drainage composite to the 4" PVC pipe, place the bottom of the drain behind the drain pipe and the aggregate/filter fabric. Peel back the bottom of the fabric flap. Wrap the filter fabric from the front to back completely around the pipe and tuck behind the core. Cover all terminal edges of the core with the fabric flap by tucking it behind the core.
- 5. The installation of a yard drain will be required behind the retaining wall at approximate Station 1+63. The yard drain should consist of a Nyloplast in-line grate connected to an 8-inch diameter PVC pipe as shown on the Yard Drain Detail on Sheet 23. Alternate drain types may be accepted with the prior approval of GTA. The yard drain should be constructed in general accordance with the manufacturer's requirements.

#### PART 4 - CONSTRUCTION OBSERVATION AND TESTING

- A. Retaining walls should only be constructed under the observation of a Registered Professional Engineer and a certified (NICET, WACEL, or equivalent) soils technician.
- B. The required bearing pressure beneath the footing of the wall should be verified in the field by a certified soils technician. Testing documentation must be provided to the geotechnical engineer prior to the start of wall construction. The required test procedure shall be the Dynamic Cone Penetrometer (DCP) Test ASTM STP-399.
- C. The suitability of fill material should be confirmed by the on-site soils technician.

#### PART 5 - DESIGN CRITERIA

- 1. Required minimum allowable foundation bearing pressure is 2,500 psf.
- 2. Design internal friction angle for reinforced soil = 30 degrees. 3. Design moist unit weight for reinforced soil = 125 pcf.
- 4. Foundation and retained soil internal friction angle = 28 degrees and cohesion = 0 psf. 5. Foundation and retained soil design moist unit weight = 125 pcf.
- 6. Retaining walls are not designed to resist hydrostatic pressure.



RETAINING WALL LOCATION PLAN

3-6-06

The location plan was adopted from the digital plan entitled "020-II-finalbase-alt.DWG", prepared by Mildenberg, Boender & Assoc, Inc., received April 12, 2005 NO ASSOULT INFO REQUIRED ON THIS

APPROVED HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

DEVELOPMENT ENGINEERING DIVISION V&

CHIEF, DIVISION OF LAND DEVELOPMENT \\

Willin I. Walen 1.

CHEIF, BUREAU OF HIGHWAY



## GEO-TECHNOLOGY ASSOCIATES, INC

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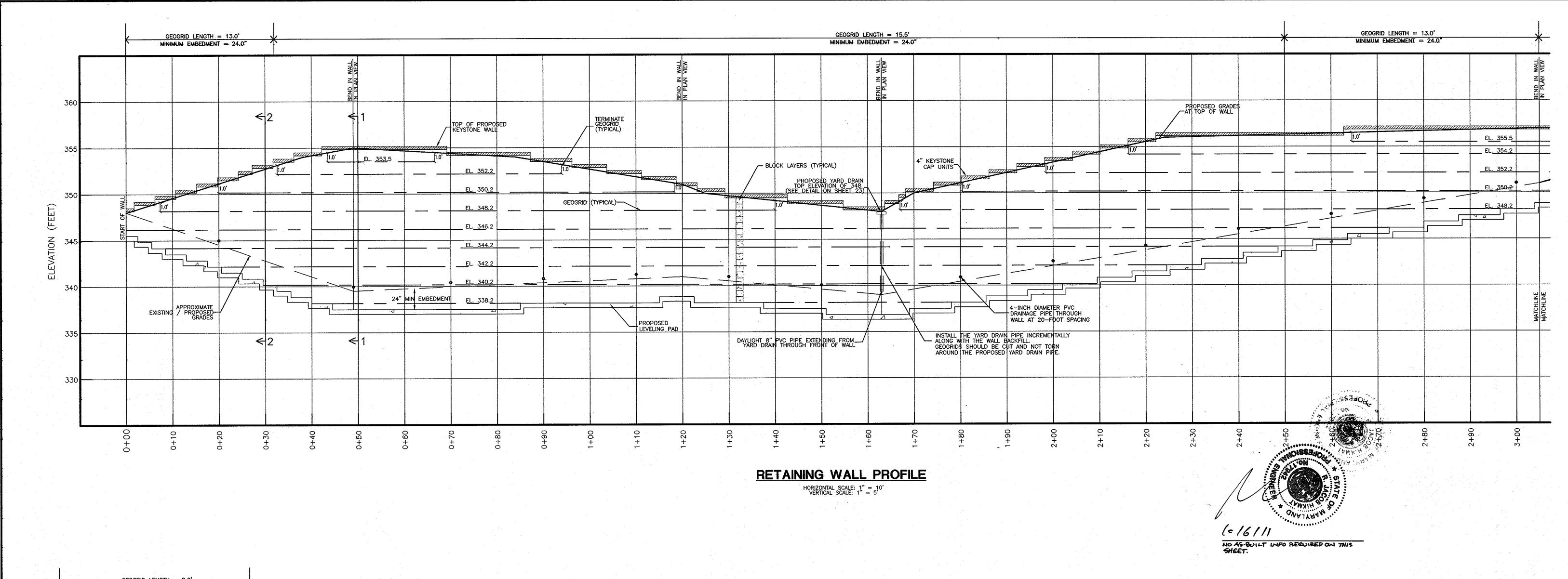


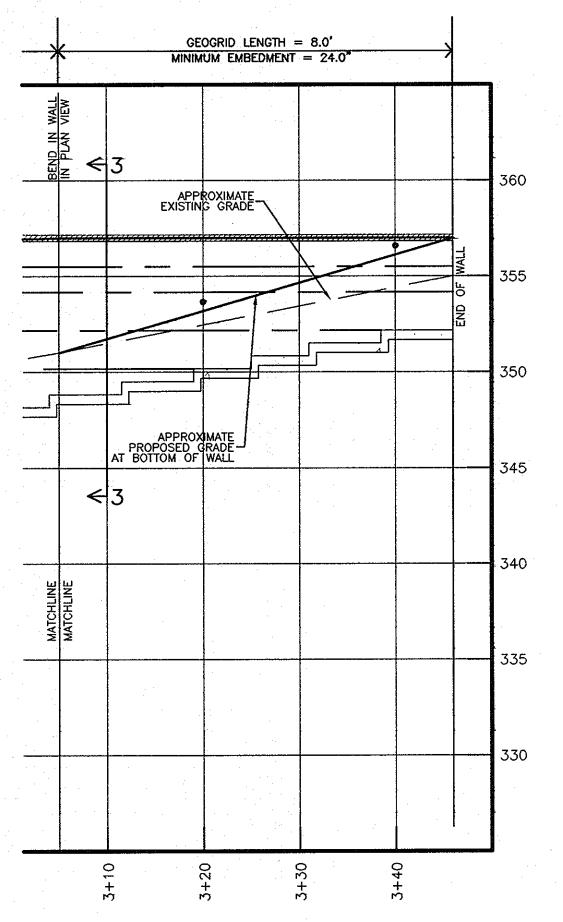
# JAMESTOWN LANDING

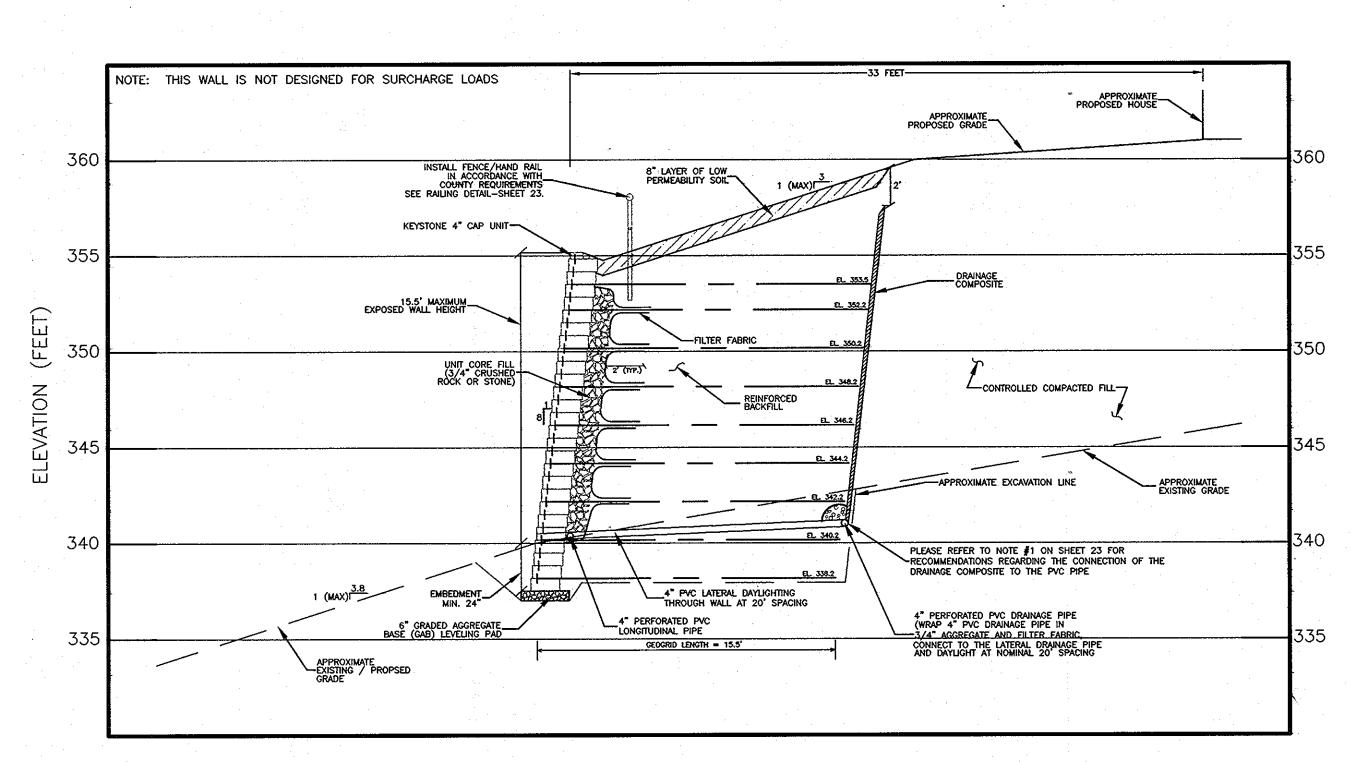
PROPOSED RETAINING WALL PLAN AND GENERAL NOTES

HOWARD COUNTY, MARYLAND

DATE	REVISIONS	JOB NO.: 041333
4/13/05	REVISED TO ADDRESS COUNTY COMMENTS	SCALE: AS SHOWN
	·	DATE: 12/14/04
		DRAWN BY: SPL
		DESIGN BY: SPL
		REVIEW BY: RPM
		SHEET: 21 of 22



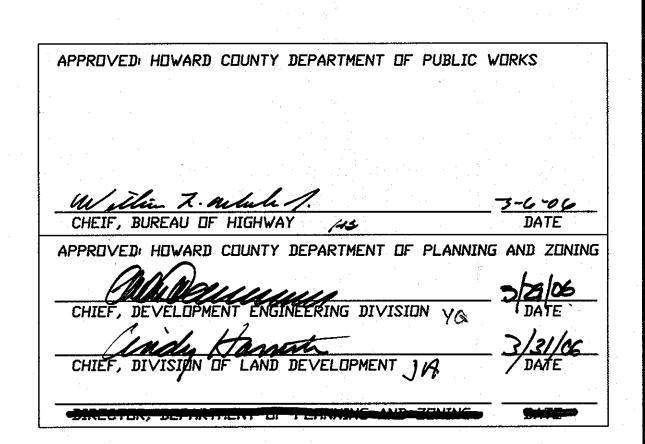




RETAINING WALL - SECTION 1 (APPROXIMATE STATION 0+50)

HORIZONTAL SCALE: 1" = 5'

VERTICAL SCALE: 1" = 5'





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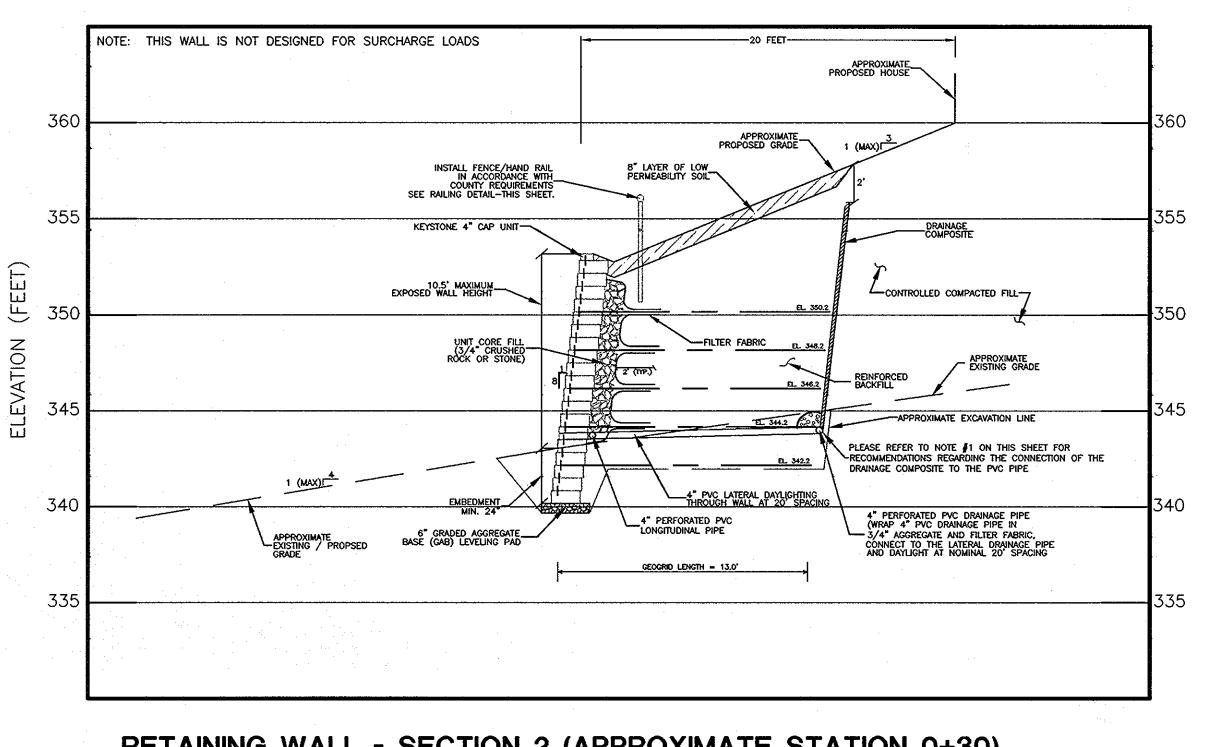


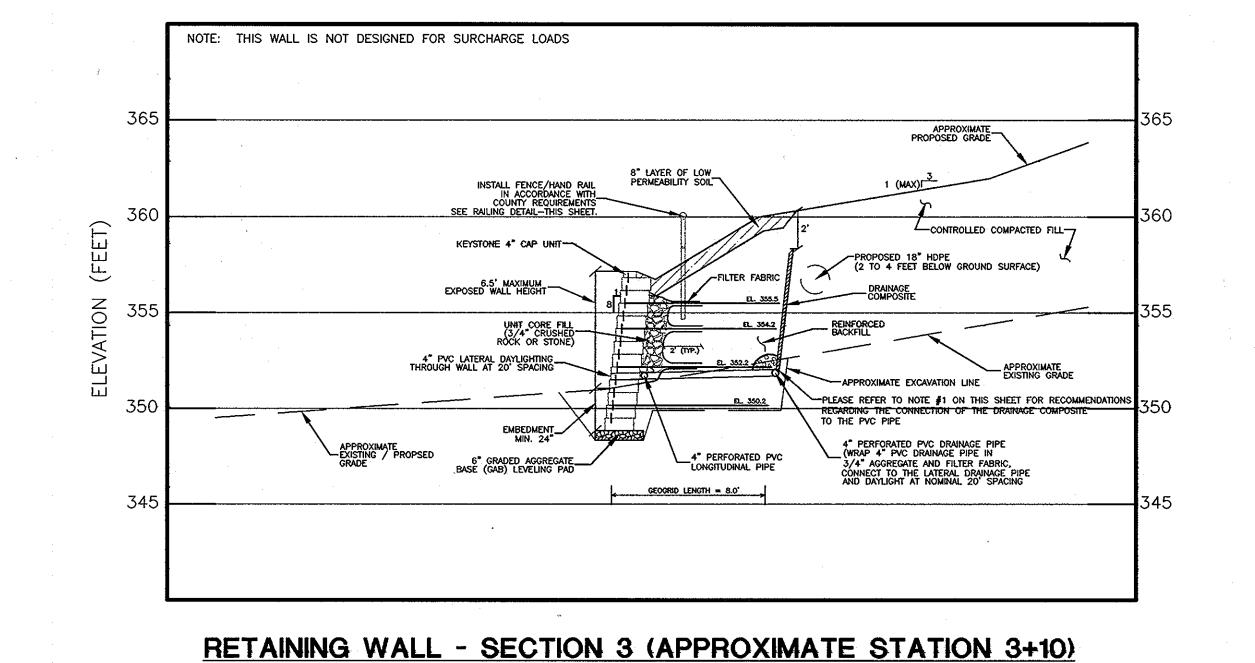
# JAMESTOWN LANDING

PROPOSED RETAINING WALL PROFILE AND SECTION VIEW

HOWARD COUNTY, MARYLAND

DATE	REVISIONS	JOB NO.: 041333
4/13/05	REVISED TO ADDRESS COUNTY COMMENTS	SCALE: AS SHOWN
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÷		DRAWN BY: SPL
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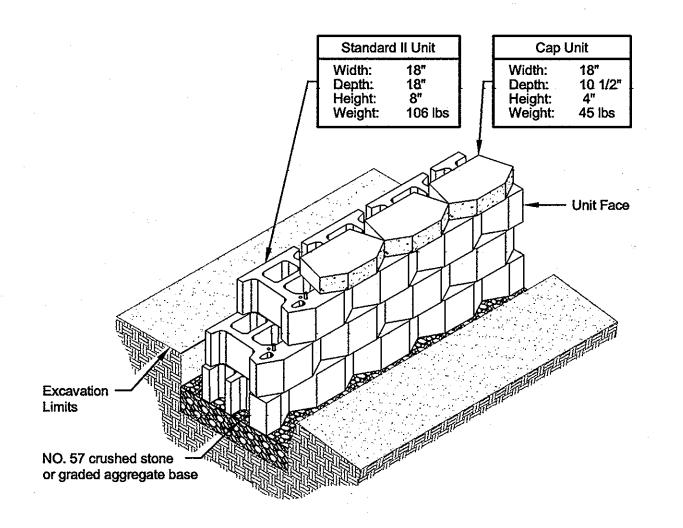


HORIZONTAL SCALE: 1" = 5' VERTICAL SCALE: 1" = 5'

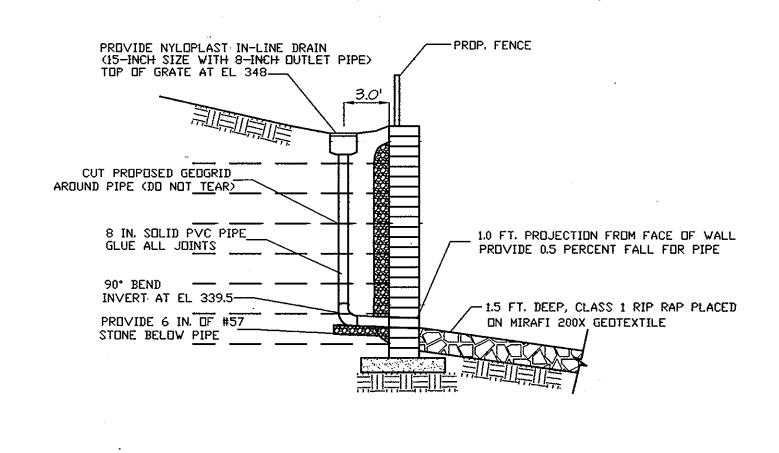
## RETAINING WALL - SECTION 2 (APPROXIMATE STATION 0+30)

HORIZONTAL SCALE: 1" = 5' VERTICAL SCALE: 1" = 5'

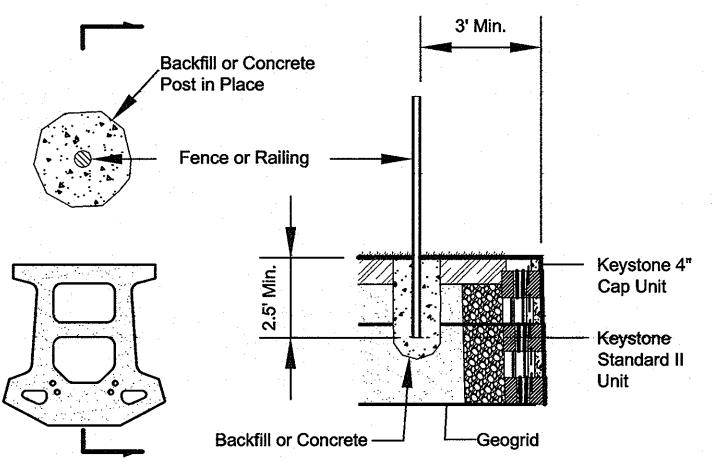
NOTE #1: TO CONNECT THE DRAINAGE COMPOSITE TO THE 4' PVC PIPE, PLACE THE BOTTOM OF THE DRAIN BEHIND THE DRAIN PIPE AND THE AGGREGATE/FILTER FABRIC. PEEL BACK THE BUTTOM OF FABRIC FLAP. WRAP THE FILTER FABRIC FROM FRONT TO BACK COMPLETELY AROUND PIPE AND TUCK BEHIND THE CORE. COVER ALL TERMINAL EDGES OF THE CORE WITH THE FABRIC FLAP BY TUCKING IT BEHIND THE CORE.



Standard II Unit/Base Pad Isometric Section View

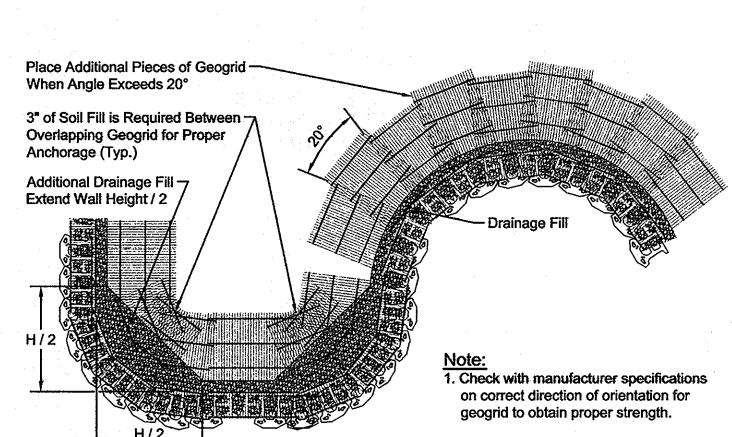


YARD DRAIN DETAIL (APPROX. STATION 1+63)

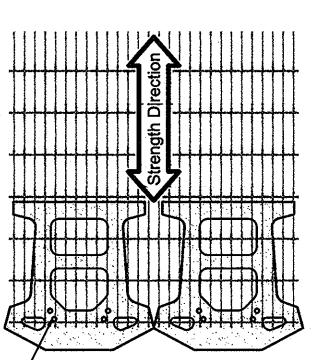


Fence Plan Detail NOT TO SCALE

Fence SectionDetail



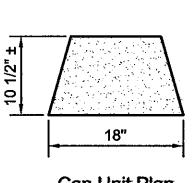
Geogrid Installation on Curves



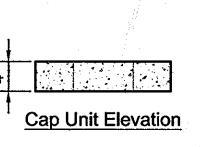
Geogrid is to be Placed on Level Backfill and Extended Over the Fiberglass Pins. Place Next Unit. Pull Grid Taught and Backfill. Stake as required.

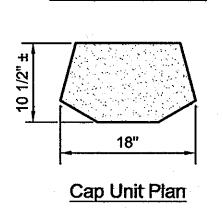
**Cap Unit Elevation** 

**Grid & Pin Connection** 

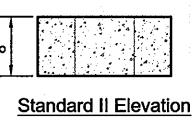


Cap Unit Plan Universal Cap Unit Option



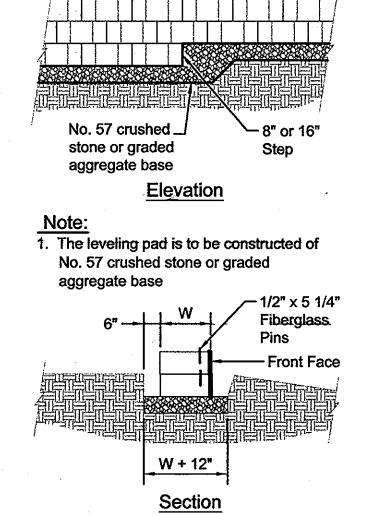


3-Plane Split **Cap Unit Option** 



Standard II Unit NOT TO SCALE

Standard II Plan



**Leveling Pad Detail** 



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NO AS-BUILT INFO REQUIRED ON THIS SHEET.

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# JAMESTOWN LANDING

PROPOSED RETAINING WALL TYPICAL SECTIONS & DETAILS

HOWARD COUNTY, MARYLAND

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		DATE: 12/14/04
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3-6-06

DATE

