SHEET INDEX COVER SHEET FINAL ROAD PROFILE FINAL ROAD PROFILE FINAL ROAD PROFILE FINAL GRADING & SEDIMENT CONTROL FINAL GRADING & SEDIMENT CONTROL PLAN SEDIMENT EROSION CONTROL DETAILS STORM DRAIN DRAINAGE AREA MAP STORMDRAIN PROFILES STORMWATER MANAGEMENT DETAILS STORMWATER MANAGEMENT DETAILS LANDSCAPE PLAN FOREST CONSERVATION PLAN FOREST CONSERVATION DETAILS RETAINING WALL **RETAINING WALL** RETAINING WAL RETAINING WALL TRAFFIC STRIPING PLAN TRAFFICGTRIPING PLAN TE DATA 22 TRAFFIC SIGNAL FLAN

GROSS AREA OF PARCEL: 2,158,354 SQ. FT. (49.55 ACRES)

AREA OF RIGHT OF WAY: 111949.20 SQ. FT. (2.57 ACRES), AREA OF EX. FLOODPLAIN: 92.019 SQ.FT (2.12 ACRES)

NET AREA OF PROJECT: 2,065,920.40 SQ. FT. (47.43 ACRES) NUMBER OF PROPOSED RESIDENTIAL LOTS/PARCELS: 22

NUMBER OF PROPOSED PRESERVATION PARCELS: 7

BY VOGEL & ASSOCIATES DATED MARCH 2001

4. THE TOPOGRAPHY SHOWN HEREON IS BASED ON AN POTOMAC AERIAL SURVEYS DATED 4/14/01. WATER AND SEWER FOR THIS PROJECT WILL BE 6. STORMWATER MANAGEMENT TO BE PROVIDED FOR

THE DEVELOPMENT. WATER QUALITY TO BE PROVIDED BY:

10. APFO TRAFFIC STUDY PREPARED BY THE TRAFFIC GROUP DATED

WITH A CONTIGUOUS AREA OF 20,000 SF OR GREATER.

13. STREET LIGHTING IS NOT REQUIRED FOR THIS SITE.

MANUAL SHALL BE COMPLIED WITH.

BURIAL/CEMETARY LOCATIONS ON SITE.

11. THERE IS NO GRADING OR DISTURBANCE WITHIN STEEP SLOPES ONSITE

14. THE STORMWATER MANAGEMENT SYSTEM SHOWN ON THIS PLAN IS AN APPROXIMATION OF ITS ULTIMATE SIZE AND SHAPE. IT IS UNDERSTOOD THAT THIS SYSTEM

HAS NOT BEEN FINAL DESIGNED AND ITS SIZE AND SHAPE MAY CHANGE ALTERING

18. A NOISE STUDY WAS PREPARED BY ROBERT H. VOGEL, DATED SEPTEMBER 2002. THE NOISE CONTOUR IS

20. AS A CONSEQUENCE OF ITS SUBMISSION FOR COUNTY REVIEW PRIOR TO NOVEMBER 15, 2001,

APPROVAL PRIOR TO NOVEMBER 1. 2001. IT IS SUBJECT TO COMPLIANCE WITH COUNTY

22. THE FOREST CONSERVATION EASEMENT TO BE RECORDED AS PART OF SHADOW SHADE, PLAT

23. PRESERVATION PARCELS "A", "E", "F" & "I" TO BE PRIVATELY OWNED WITH THE HOA AND

25. PLANNING AND ZONING FILE NUMBERS: S-02-03, F-99-25, F-86-13, P-03-02.

(257,440 SF X 0.50)

(16,553 SF X 0.50) \$8,277

28. STREET TREES ARE REQUIRED FOR THIS SUBDIVISION IN ACCORDANCE WITH SECTION 16.124(e)(1) OF

30. TREE PROTECTION FENCING WILL BE PROVIDED AT THE LIMITS OF DISTURBANCE WHERE GRADING IS

31. NO CLEARING, GRADING OR CONSTRUCTION IS PERMITTED WITHIN THE WETLANDS. STREAMS

32. FINANCIAL SURETY FOR THE REQUIRED TOTAL 145 STREET TREES HAS BEEN POSTED AS PART OF THE DEVELOPER'S AGREEMENT IN AMOUNT OF \$43,500

33. FINANCIAL SURETY FOR THE REQUIRED LANDSCAPING (IN THE AMOUNT OF \$45,450) AND FENCING (832 LF @ \$10 PER LF) HAS BEEN POSTED AS A PART OF THE DPW DEVELOPER'S AGREEMENT IN THE TOTAL AMOUNT OF \$53,770

29. LOTS 18-20 AND 5-6 & PRESERVATION PARCEL A WILL UTILIZE A USE-IN-COMMON DRIVEWAY. HOWARD COUNTY STANDARD DETAIL NO. R-6.06 WILL BE UTILIZED FOR THE ENTRANCE AT THE INTERSECTION OF

COUNCIL BILL 50-2001 WHICH AMENDS PORTIONS OF THE ZONING REGULATIONS. THE EXISTING DRIVEWAY FROM WOODSTOCK ROAD WILL BE REMOVED WEST OF LOT 10

THIS PROJECT IS SUBJECT TO COMPLIANCE WITH THE FOURTH EDITION OF THE SUBDIVISION AND LAND DEVELOPMENT REGULATIONS. IN ADDITION, BECAUSE IT DID NOT HAVE PRELIMINARY

NUMBER OF BUILDABLE ENTITIES PROPOSED: 21 LOTS & 1 PRESERVATION PARCEL = 22

HOWARD COUNTY AS EASEMENT HOLDERS. PRESERVATION PARCELS "B", "C", & "D" ARE TO BE OWNED

OWNED BY THE HOA FOR THE PURPOSE OF SUPPORTING SIDE SLOPE GRADING FOR DENALI DRIVE.

27. FOREST CONSERVATION REQUIREMENTS PER SECTION 16.1202 OF THE HOWARD COUNTY CODE AND THE

FOREST CONSERVATION MANUAL FOR THIS SUBDIVISION WILL BE FULFILLED BY THE RETENTION OF EXISTING FOREST IN THE AMOUNT OF 6.46 AC. AND REFORESTATION OF 5.91 AC. FOR A TOTAL FOREST

ENVIRONMENTAL PRESERVATION PARCEL. PRESERVATION PARCEL "E" TO BE USE TO PRESERVE

BY THE HOA WITH HOWARD COUNTY AS THE EASEMENT HOLDER. NON-BUILDABLE BULK PARCEL "G" & "H"TO BE

ENVIRONMENTALLY SENSITIVE RESOURCES.

PRESERVATION PARCELS "B", "C" & "D" ARE TO ENCOMPASS STORMWATER MANAGEMENT FACILITIES, THE 5

DENSITY RECEIVING SUBDIVISION WILL BE FULFILLED IN THE CREATION OF PRESERVATION PARCELS "B", "C" & "D".

24. FOREST CONSERVATION PLAN PREPARED BY DNR QUALIFIED PROFESSIONAL LARRY J. THOMPSON, DATED JULY 2004.

PARCEL 60. PLAT NO F-00-50: 6 DEOS FROM THE ERDMAN SENDING PLAT, TAX MAP 28, BLOCK 5, PARCEL 76,

\$128,720

34. THIS PLAN IS SUBJECT TO A DESENMENUAL WAIVER APPROVED APRIL 25 2011 SECTION 6.2.54 TO PERMIT A 36" OAK TREE TO REMAIN WITHIN 12' OF STORMWATER MANAGEMENT FACILITY #2 TOE OF SLOPE.

12. ALL LANDSCAPING REQUIREMENTS AS SET FORTH IN THE LANDSCAPE

THE NUMBER OF UNITS ALLOCATED FOR THIS DEVELOPMENT. 15. THIS PROPERTY IS NOT WITHIN THE METROPOLITAN DISTRICT. 16. TO THE BEST OF THE OWNERS KNOWLEDGE, THERE ARE NO

17. TRASH COLLECTION AND RECYCLING WILL BE CURBSIDE PICKUP

19. DENSITY: NUMBER OF ENTITIES PERMITTED BY RIGHT: 49.55/4.25 = 11

PARCELS 304 & 102 PLAT NO. RE-03-06

NO. 13808 WILL NOT BE RETAINED AS PART OF THIS SUBDIVISION.

26. SEDIMENT AND EROSION CONTROL WILL BE PROVIDED FOR THIS SITE.

THE REMAINING 0.38 AC OF REFORESTATION WILL BE SATISFIED

FINANCIAL SURETY IN THE AMOUNT OF \$185,000 WILL BE PAID

THE SUBDIVISION REGULATIONS AND THE LANDSCAPE MANUAL.

OR THEIR BUFFERS AND THE FOREST CONSERVATION EASEMENTS.

WITH THE FC MAINTENANCE AGREEMENT. THE SURETY AMOUNT IS SUBJECT

BASED ON THE PROJECTED 2020 TRAFFIC STUDY.

OF REUWER'S BREEZEWOOD FARMS, PLAT NO. 10063.

PRESERVATION PARCEL "A" TO BE A BUILDABLE

CONSERVATION OBLIGATION AREA OF 12.75 AC.

THROUGH FEE-IN-LIEU IN THE AMOUNT OF:

TO THE FINAL PLAN APPROVAL.

ADJACENT TO ENVIRONMENTAL AREAS.

THE PUBLIC ROAD.

RETENTION OF 6.46 ACRES (281,389 SF X 0.20)...

PLANTING TREES IN UNFORESTED SWM CREDIT AREAS AND OTHER REFORESTATION AREAS, TOTAL 5.91 ACRES

AREA OF STEEP SLOPES (NOT LOCATED IN FLOOD PLAIN: 132,279 SQ. FT. (3.04 ACRES)

AREA OF PROPOSED BUILDABLE PRESERVATION PARCEL 'A': 554400.0 SQ. FT. (12.73 ACRES)

AREA OF PROPOSED NON-BUILDABLE BULK PARCEL 'G', 'H': 81887.0 SQ. FT. (1.88 ACRES)

FACILITY NO. 1: A MICROPOOL EXTENDED DETENTION POND (HAZARD CLASS 'A') Cpv and WQv,

FACILITY NO. 3: A MICROPOOL EXTENDED DETENTION OF WQ. (HAZARD CLASS 'A') WQv only

AREA OF PROPOSED RESIDENTIAL LOTS 1-21: 1089435.6 SQ. FT. (25.01 ACRES)

TOTAL APPROXIMATE LIMIT OF DISTURBANCE: 250,034 SF± (5.74 AC.)

1. ALL ASPECTS OF THE PROJECT ARE IN CONFORMANCE WITH THE LATES HOWARD COUNTY STANDARDS UNLESS WAIVERS HAVE BEEN APPROVED. 2. DEED REFERENCE: 3419/444 (PARCEL '102'); 1453/747 (PARCEL '304 3. THE PROJECT BOUNDARY IS BASED ON A BOUNDARY SURVEY PREPAR

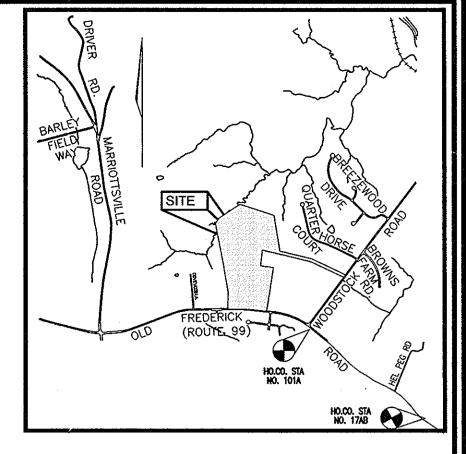
OWNED AND MAINTAINED BY H.O.A.

FACILITY NO. 2: EXTENDED DETENTION OF WQ. (HAZARD CLASS 'A') WQv on

4TH ELECTION DISTRICT EXISTING ZONING: RC-DEO

FINAL ROAD CONSTRUCTION PLANS THE PRESERVE AT WAVERLY GLEN

LOTS 1-21, PRESERVATION PARCELS A-FIT NO NON-BUILDBALE BULK PARCEL GIB A RESUBDIVISION OF LOTS 1 & 2 AND PARCEL A OF SHADOW SHADE PLAT NO. 13808 AND LOT 1 OF HIGHPOINT AT BREEZEWOOD FARMS PLAT NO. 6385 HOWARD COUNTY, MARYLAND



10IA N 600,995.172' E 1,345,340.347

	PHASING (CHART
YEAR	PHASE	ALLOCATIONS
2004	1	7
2005	2	12

	COORDINATE TAB	BLE
NO.	NORTHING	EASTING
50	N 601454.93	E 1343594.75
51	N 601801.42	E 1343607.35
52	N 602440.89	E 1369759.0984
53	N 585508.195	E 1343349.57
54	N 603346.93	E 1343422.93
55	N 603630.92	E 1343858.86
56	N 603397.68	E 1344560.23
57	N 602652.16	E 1344677.91
58	N 602487.22	E 131345337.91
59	N 602150.75	E 131346042.48
60	N 602109.98	E 1346011.83
61	N 602434.71	E 1345331.84
62	N 602686.29	E 1344313.17
63	N 602342.39	E 1344276.52
64	N 602308.81	E 1344394.34
65	N 601407.06	E 1344541.84

CURVE TABLE							
CURVE	RADIUS	LENGTH	TANGENT	CHORD	BEARING	DELTA	
C1	160.00	148.13	79.85	142.90	\$28'18'32"W	53'2'42"	
C2	150.00	86.96	44.74	85.75	N38 13'21"E	33'13'3"	
С3	150.00	40.82	20.54	40.70	N13'49'01"E	15'35'38"	
C4	150.00	88.77	45.73	87.48	S18'12'54"E	33'54'26"	
C5	150.00	49.46	24.96	49.24	N10'42'30"W	18'53'39"	
C6	25.00	15.39	7.95	15.15	N37'47'28"W	35'16'16"	
					*	• • •	

OWNER/DEVELOPER THE PRESERVE AT WAVERLY GLEN, LLC 3675 PARK AVENUE, SUITE 301 ELLICOTT CITY, MARYLAND 21043 (410) 480-0023

COVER SHEET

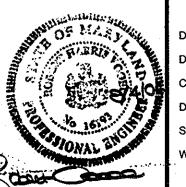
THE PRESERVE AT WAVERLY GLEN LOTS 1-21, PRESERVATION PARCELS A-F AND I NON-BUILDABLE BULK PARCEL G+H A RESUBDIVISION OF LOTS 1 & 2 AND PARCEL A OF SHADOW SHADE PLAT NO. 13808 AND LOT 1 OF HIGHPOINT AT BREEZEWOOD FARMS PLAT NO. 6385

REF. S-02-03, P-03-02, F-99-25, F-86-13 TAX MAP #10 BLOCK 23

PARCELS '304' & '102' HOWARD COUNTY, MARYLAND **3** ELECTION DISTRICT

ROBERT H. VOGEL ENGINEERING, INC. ENGINEERS · SURVEYORS

8407 MAIN STREET ELLICOTT CITY, MARYLAND 21043 TEL: 410.461.7666 FAX: 410.461.8961



Chief, Division of Land Development

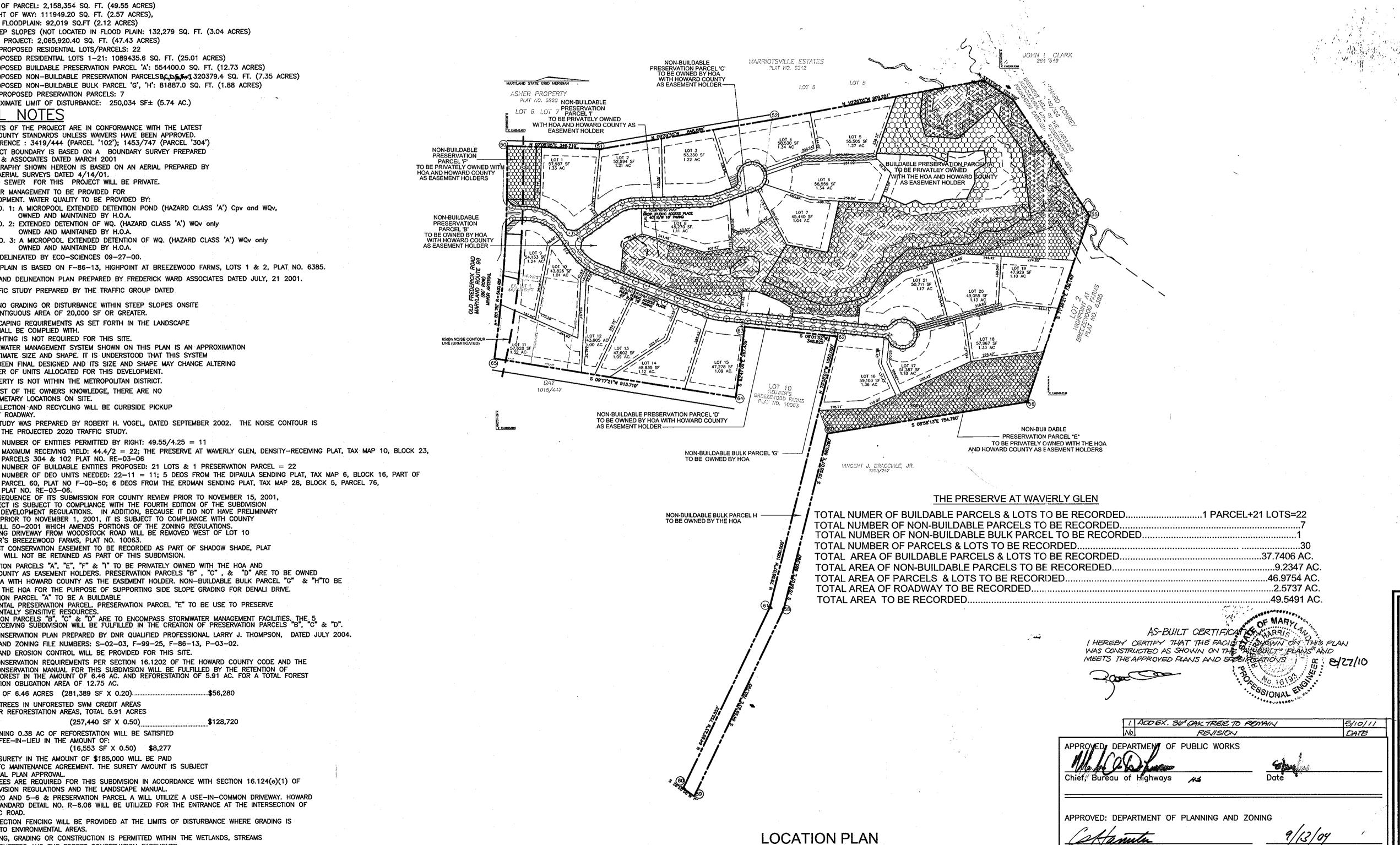
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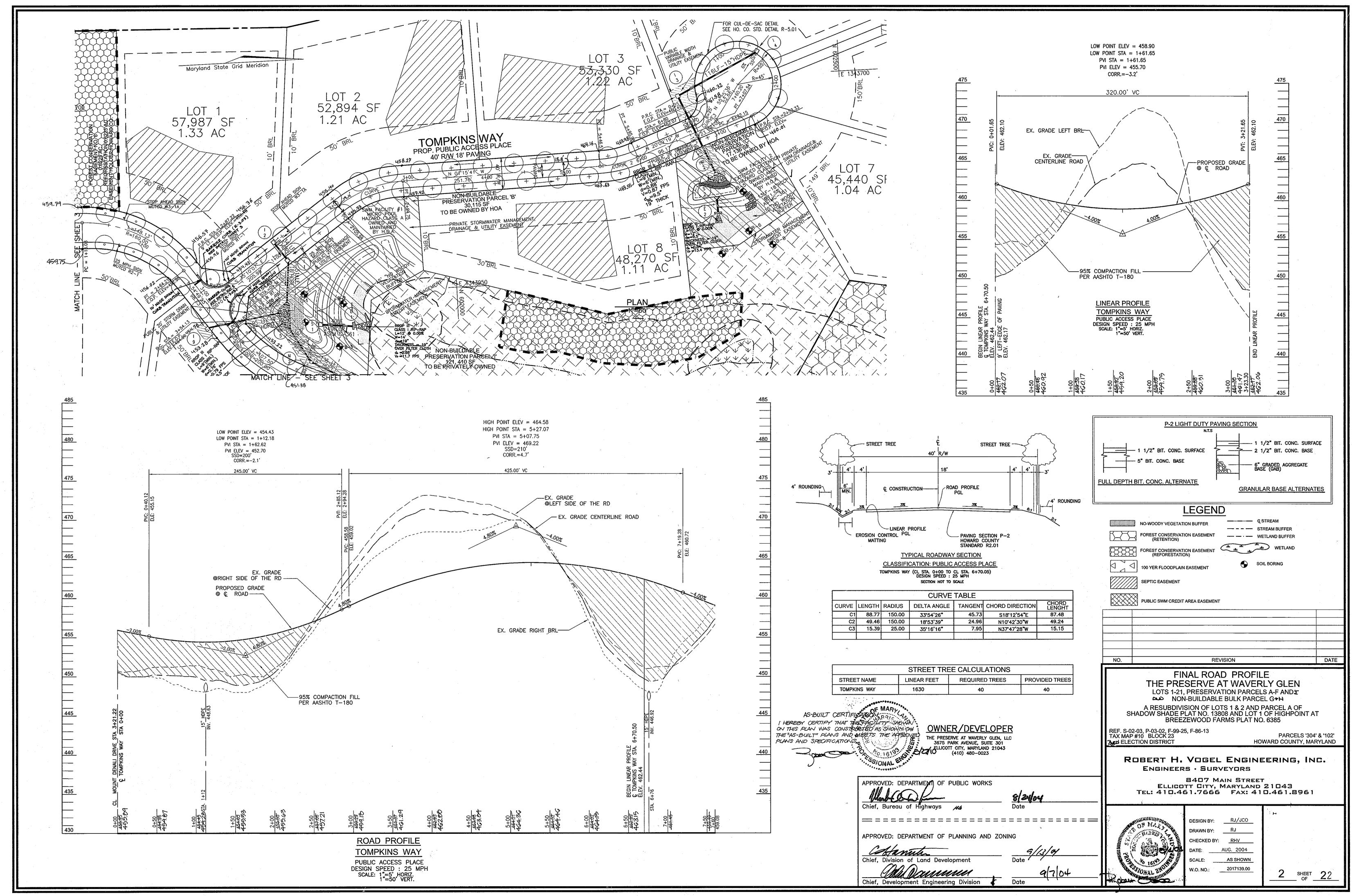
Chief, Development Engineering Division

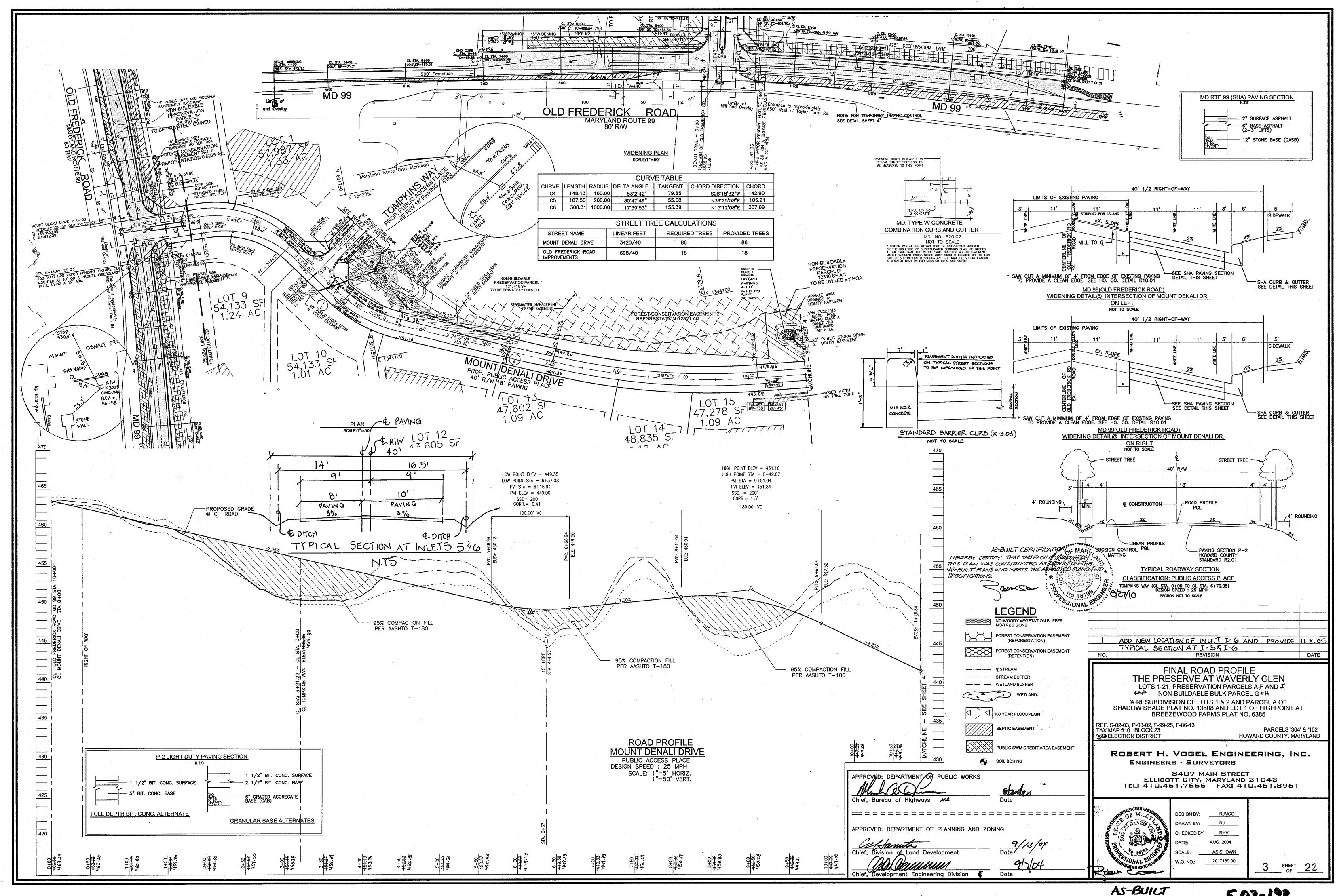
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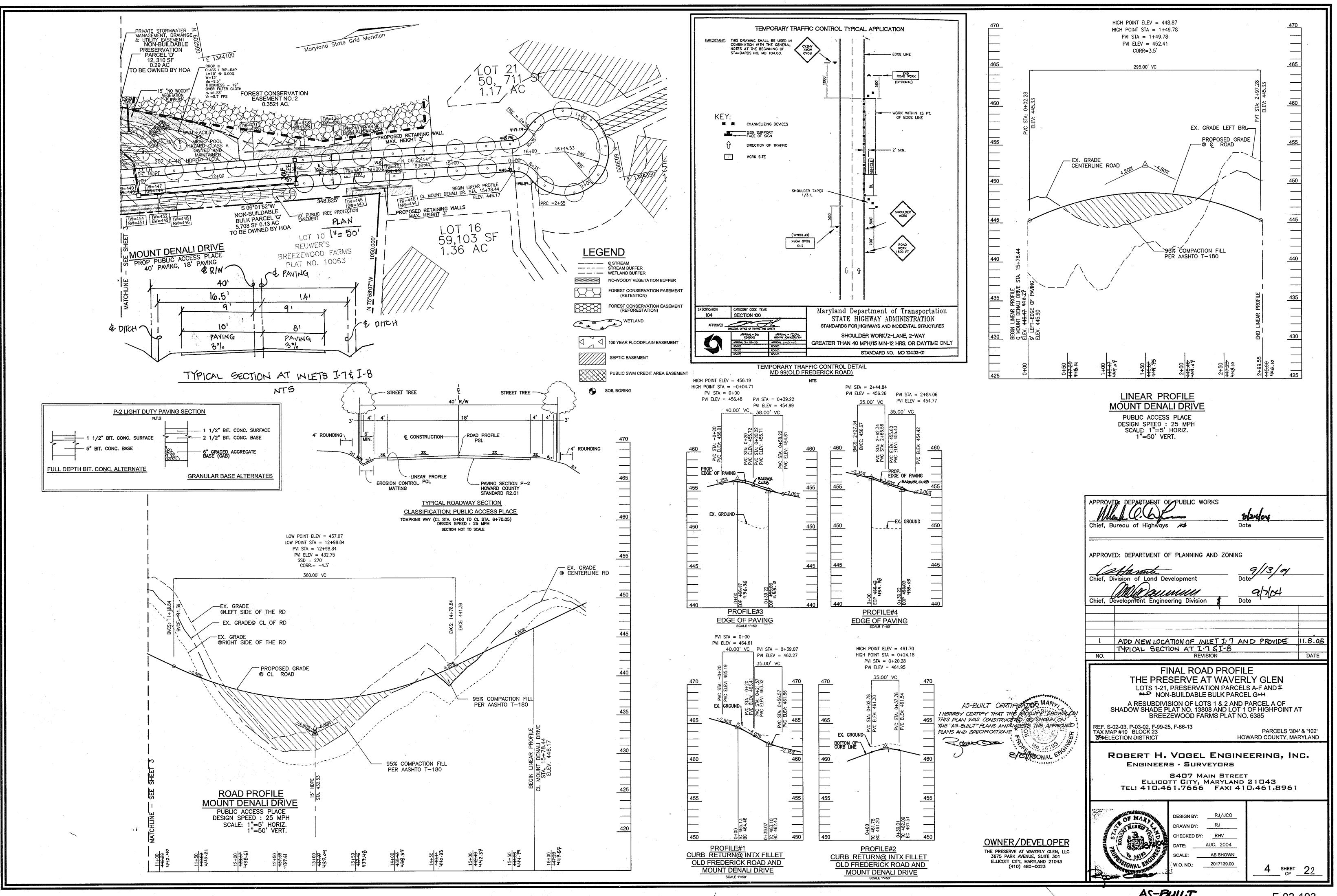
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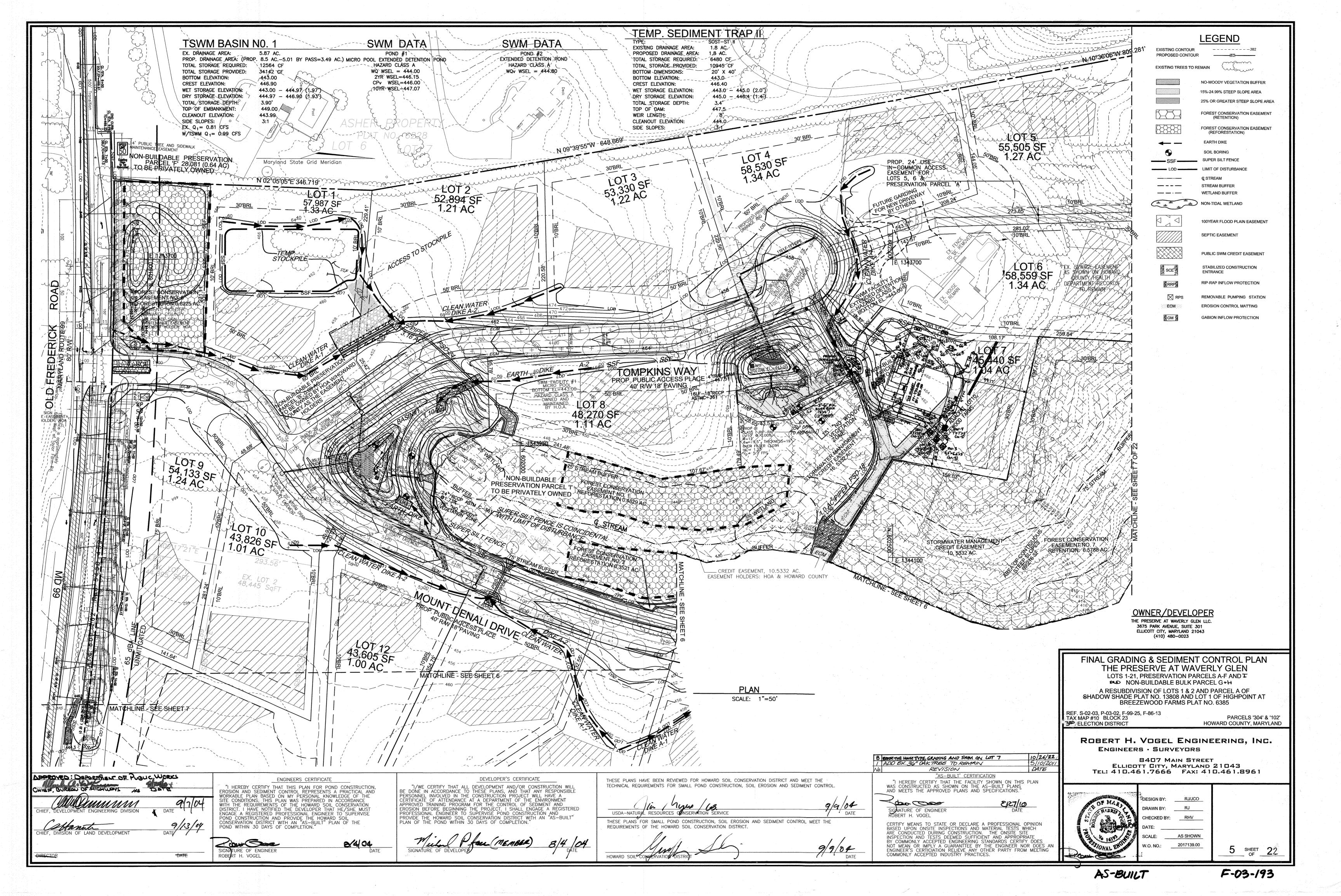
DESIGN BY:	RJ/JCO
DRAWN BY:	RJ
CHECKED BY:	AS SHOWN
DATE:	AUG. 2004
SCALE:	AS SHOWN
W.O. NO.: _	2017139.00

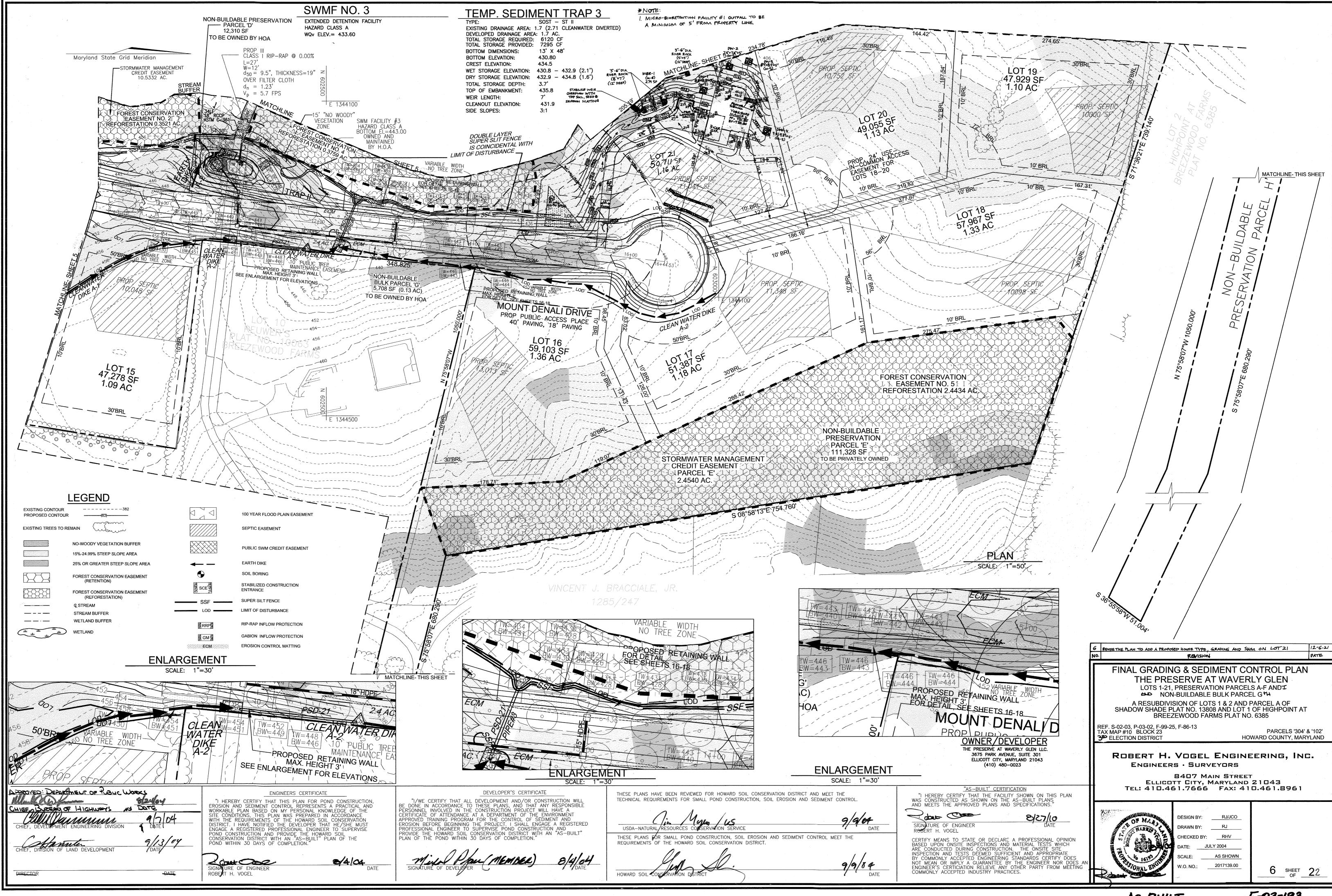


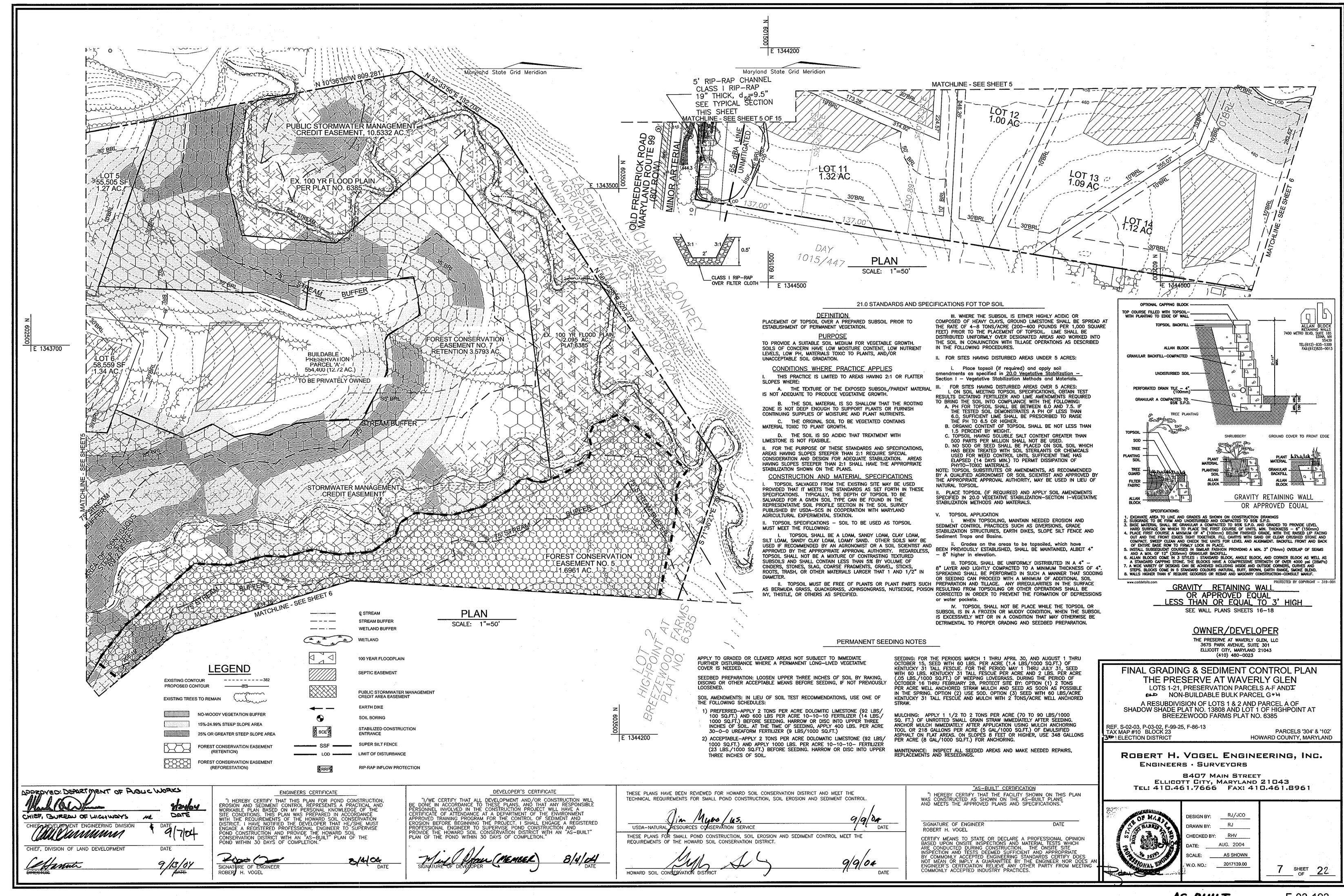


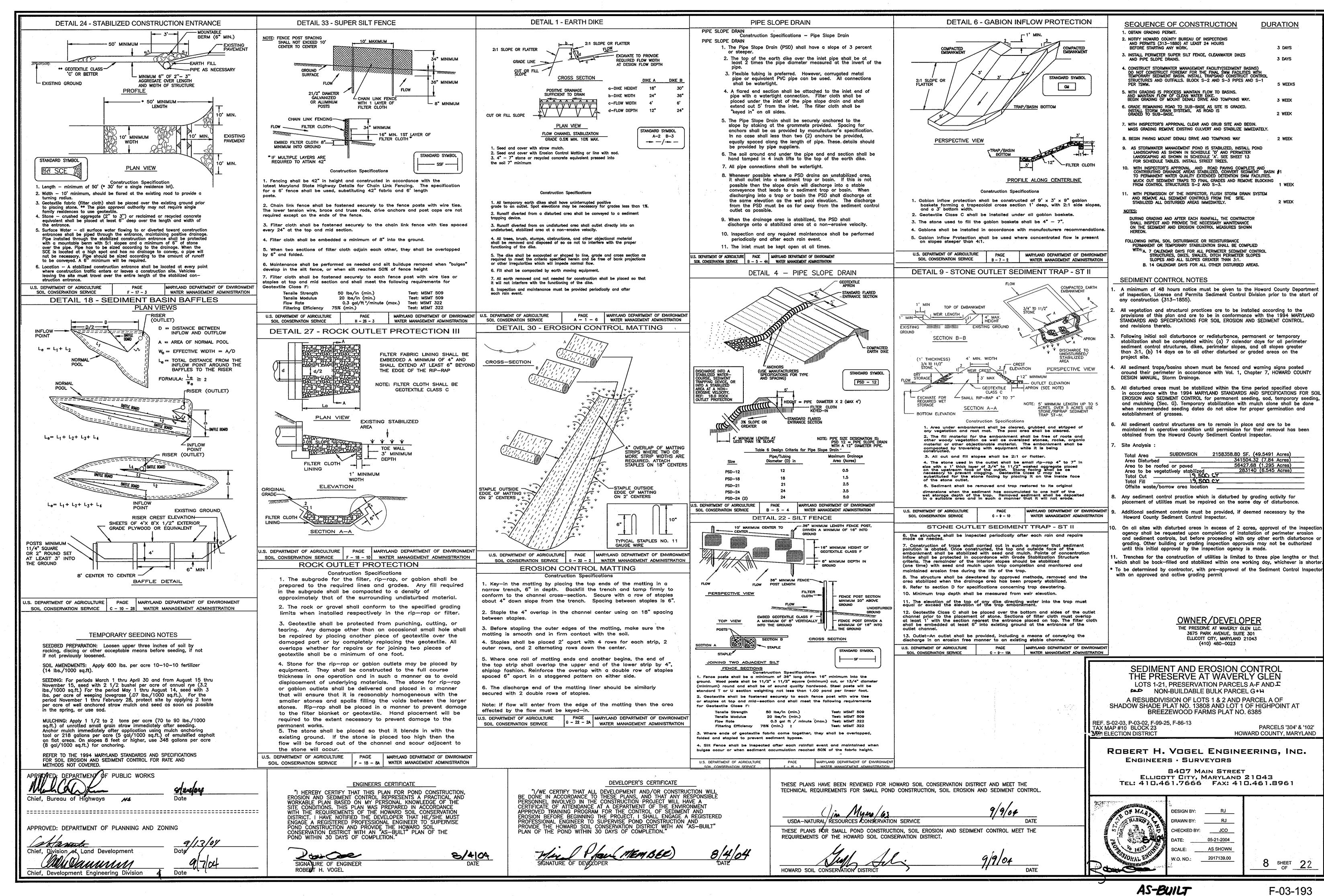


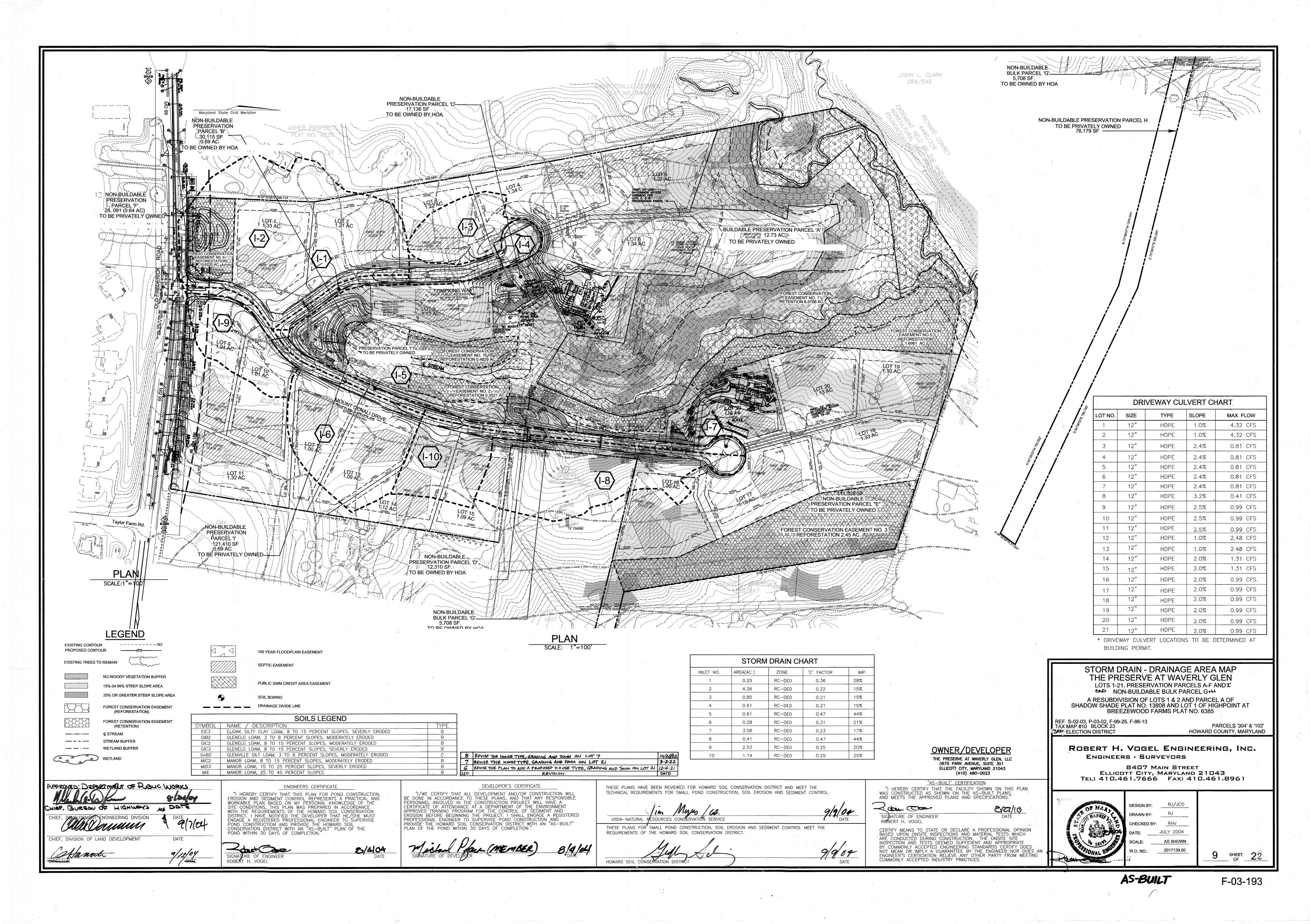


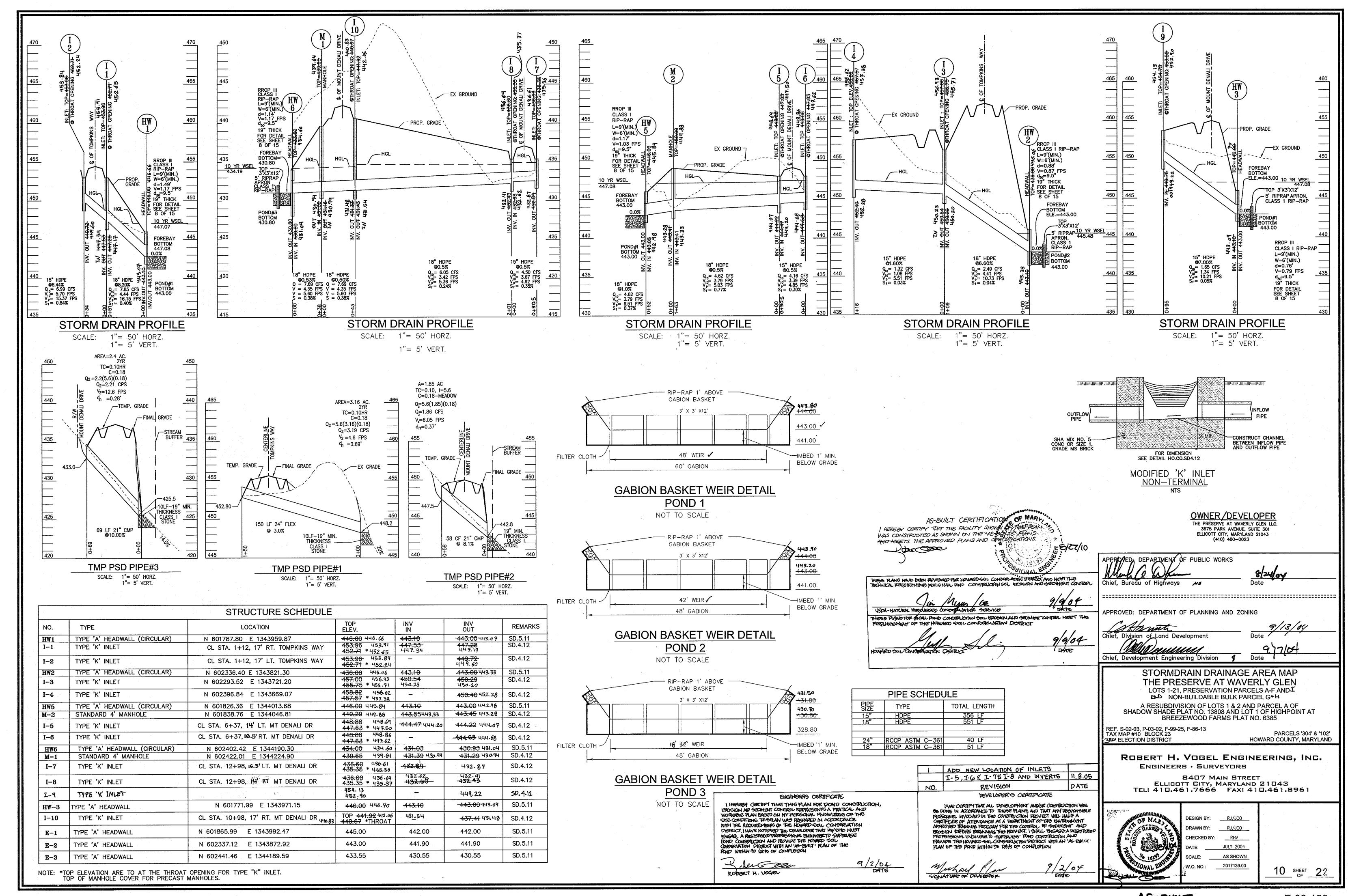


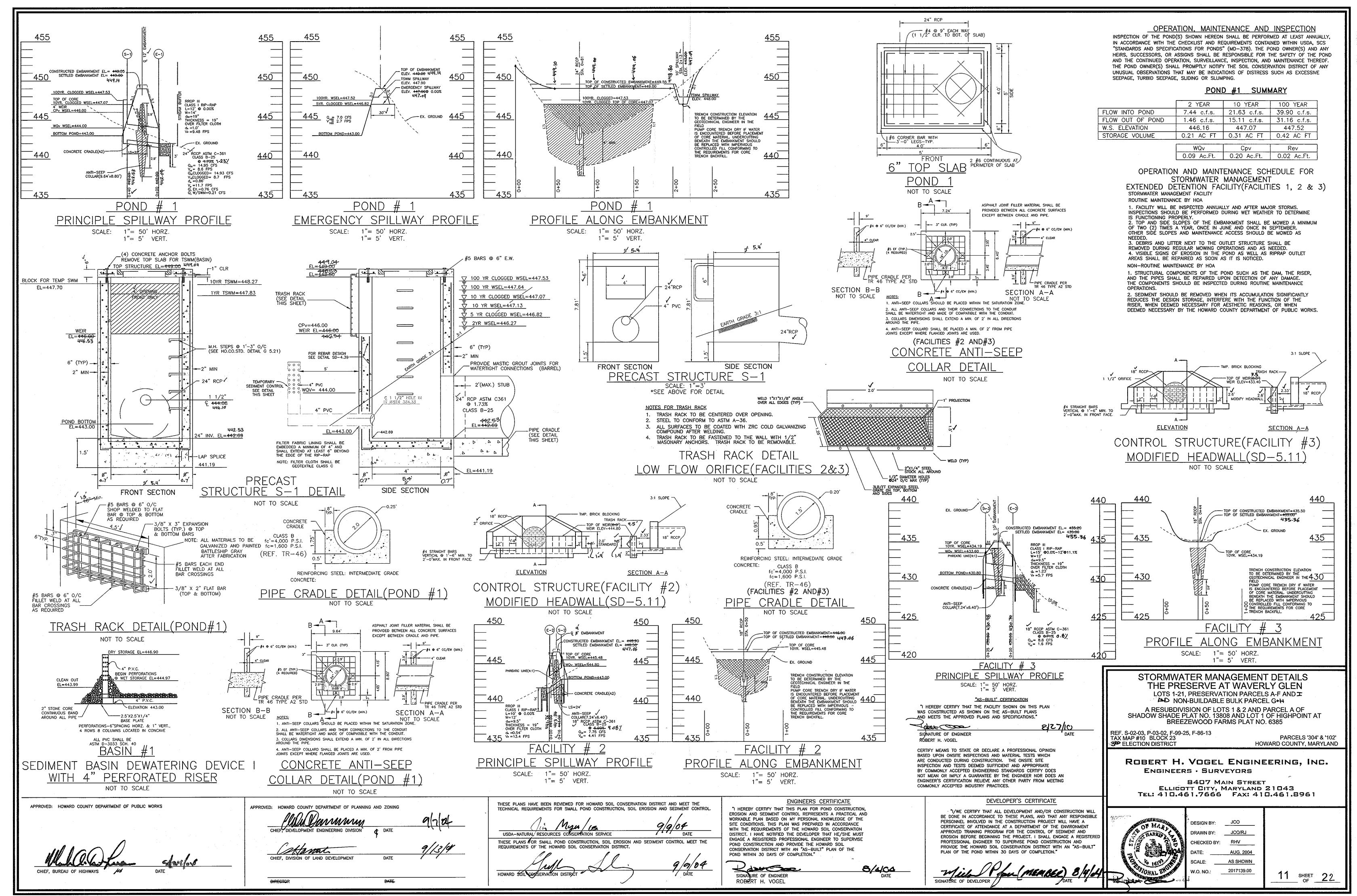


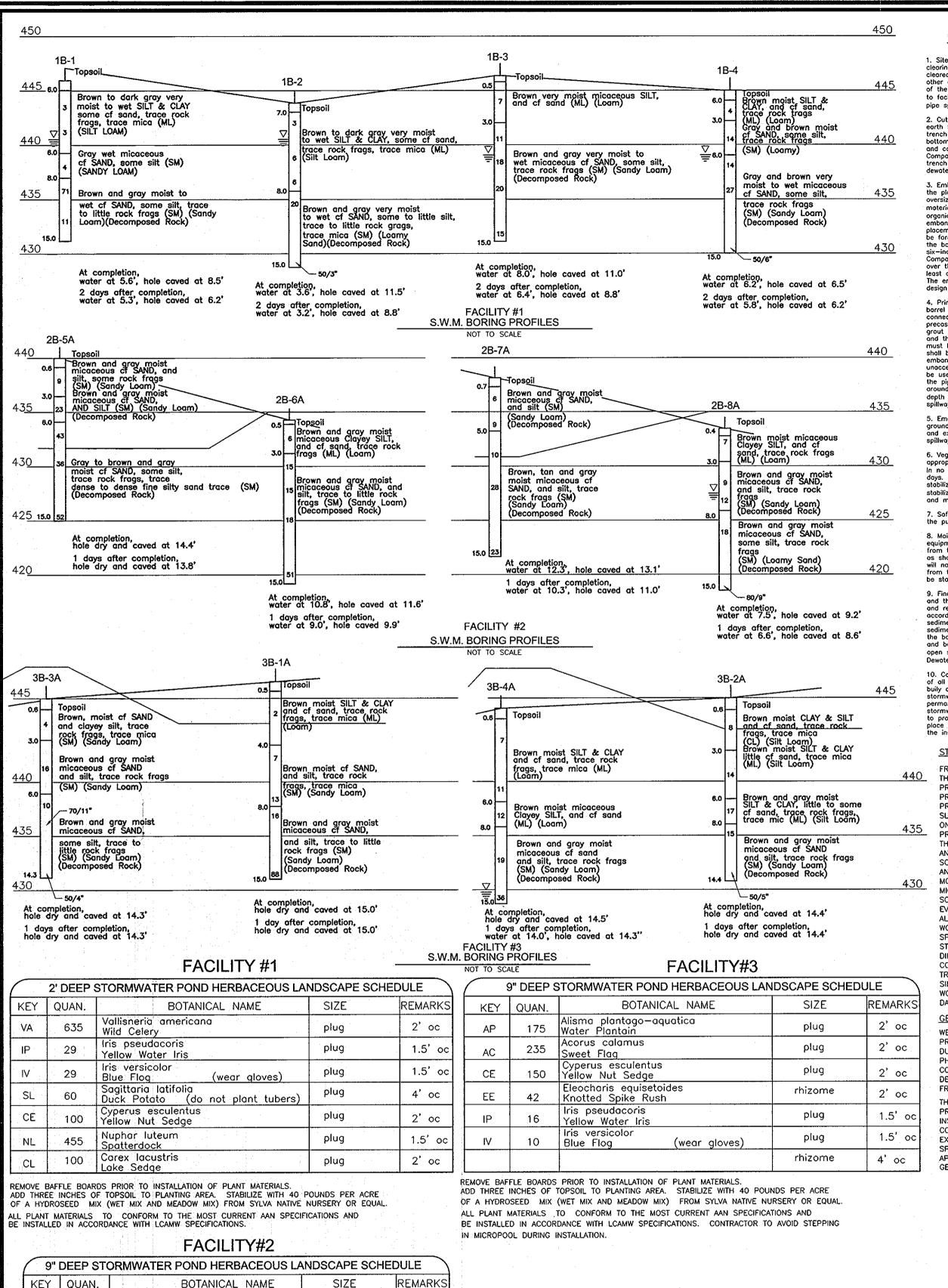












2' oc

2' oc

1.5' 00

REMOVE BAFFLE BOARDS PRIOR TO INSTALLATION OF PLANT MATERIALS.

IN MICROPOOL DURING INSTALLATION.

CHIEF, DIVISION OF LAND DEVELOPMENT

DIRECTOR

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

ADD THREE INCHES OF TOPSOIL TO PLANTING AREA. STABILIZE WITH 40 POUNDS PER ACRE

OF A HYDROSEED. MIX (WET MIX AND MEADOW MIX). FROM SYLVA NATIVE NURSERY OR EQUAL.

BE INSTALLED IN ACCORDANCE WITH LCAMW SPECIFICATIONS. CONTRACTOR TO AVOID STEPPING

DATE

rhizome

plug

rhizome

BOTANICAL NAME

(wear gloves)

Alisma plantago—aquatica

Water Plantain

Sweet Flag

Acorus calamus

Cyperus esculentus

Knotted Spike Rush Iris pseudacoris

Eleocharis equisetoides

Yellow Nut Sedge

Yellow Water Iris

Iris versicolor

Blue Floa

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

325

150

112

80

16

16

Sediment Basin#1 Construction Specifications

clearing and grubbing. Area where the embankment is to be placed shall be cleared, grubbed and stripped of topsoil to remove trees, vegetation, roots or other objectionable material. The pool area shall not be cleared until completi of the dam embankment unless the pool area is to be used for borrow. In orde o facilitate clean-out and restoration, the pool area (measured at the top of the Cut-off Trench: A cut-off trench shall be excavated along the centerline of earth fill embankments. The minimum depth shall be four feet. The cut-off trench shall extend up both obutments to the riser crest elevation. The minimum bottom width shall be two feet, but wide enough to permit operation of excavation and compaction equipment. The side slopes shall be no steeper than 1:1. Compaction requirements shall be the same as those for the embankment. The trench shall be dewatered during the backfilling-compaction operations. For developing see Section 0.

. Embankment: The fill material shall be taken from approved areas shown on the plans. It shall be clean material soil free of roots, woody vegetation, oversized stones, rocks, or other objectionable material. Relatively pervious materials such as sand or gravel (Unified Soil Classes GW, GP, SW & SP) or organic materials (Unified Soil Classes OL and OH) shall not be placed in the embonkment. Areas on which fill is to be placed shall be scanfied prior to placement of fill. The fill material shall contain sufficient moisture so that it can be formed by hand into a ball without crumbling. If water can be squeezed from the ball, it is too wet for proper compaction. Fill material shall be placed in six—inch by eight—inch thick continuous lifts of the entire length of the fill. Compaction shall be obtained by routing and hauling the construction equipment over the fill so that the entire surface of each layer of the fill is traversed by all east one wheel or tread track of the equipment or by the use of a compactor embankment shall be constructed to an elevation 10 percent higher than the

4. Principal Spillway: Steel risers shall be securely attached to the barrel or barrel stub by welding the full circumference making a watertight structural connection. Concrete risers shall be poured with the principal spillway in place precast with voids around the principal spillway filled with concrete or shrink proof grout for watertight connection. The barrel stub must be attached to the riser and the riser base shall be watertight. All connections between barrel sections must be achieved by approved watertight. All connections between barrel sections must be achieved by approved watertight band assemblies. The barrel and riser shall be placed on a firm, smooth foundation of impervious soil as the embankment is constructed. Breaching the embankment to install the barrel is unacceptable. Pervious materials such as sand, grovel or crushed stone shall not be used as backfill around the pipe or anti-seep collars. The fill material around the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under and account the pipe spillway shall be placed in four inch lifts and hand compacted under an account the pipe spillway shall be placed in four inch lifts and hand compacted under the pipe spillway shall be placed in four inch lifts and hand compacted under the pipe spillway shall be placed in four inch lifts and hand compacted the pipe spillway shall be placed in four inch lifts and hand compacted the pipe spillway shall be placed in four inch lifts and hand compacted the pipe spillway shall be placed in four inch lifts and hand compacted the pipe spillway shall be placed in four inch lifts around the pipe to at least the same density as the adjacent embankment. A depth of 1.5 times the pipe diameter (min.) shall be backfill over the principal

5. Emergency Spillway: The emergency spillway shall be installed in undisturbed ground. The achievement of planned elevations, grades, design width, entrance and exit channel slopes are critical to the successful operation of the emergency spillway and must be constructed within a tolerance of + 0.2 feet. In no case shall the embankment remain unstabilized for more than seven (7) days. Once constructed, the top and outside face of the embankment shall be stabilized with seed and mulch. The remainder of the interior slopes should be stabilized (one time) with seed and mulch upon basin completion and monitoring and maintained erosion free during the life of the basin.

7. Sofety: Local requirements concerning fencing and signs shall be met, warning the public of hazards of soft sediment and floodwater. Maintenance: Repair all damage caused by soil erosion and construction equipment at or before the end of each working day. Sediment shall be removed from the basin when it reaches the specified distance below the top of the riser as shown on the riser. This sediment shall be placed in such a manner that is will not erode from the site. The sediment shall not be deposited downstream the substitute of the sediment shall not be deposited downstream. from the embankment, adjacent to a stream or floodplain. Disposal areas must

Final Disposal: When temporary structures have served their intended purpose and the contributing drainage area has been properly stabilized, the embankment and resulting sediment deposits are to be leveled or otherwise disposed of in accordance with the approved sediment control plan. The proposed use of a sediment basin site will often dictate final disposition of the basin and any sediment contained therein. If the site is scheduled for future construction, the the basin material trapped sediments must be removed and safely disposed of the and basin shall be backfilled with structural fill. When the basin area is to remain open space, the pond may be pumped dry (using methods in Section D -Dewatering), grading, and back filled.

 Conversion to Stormwater Management Structure: After permanent stabilization of all disturbed contributory drainage areas, temporary sediment basins if initially buily and certified to meet permanent standards, may be converted to permanent stormwater management structures. To convert the basin from temporary to stormwater management design plans. Additional grading may also be necessary to provide the required storage volume in the basin. Conversion can only take place after all disturbed area have been permanently stabilized to satisfaction of the inspection authority and storm drains have been flushed.

STORM WATER MANAGEMENT CONSTRUCTION RECOMMENDATIONS FROM REVIEW OF THE TEST BORING INFORMATION IN CONJUNCTION WITH THE PROPOSED CONSTRUCTION, IT APPEARS THAT THE SUBSOILS AT PROBABLE INVERT GRADE WILL BE SATISFACTORY TO SUPPORT THE PRINCIPAL SPILLWAY CONSTRUCTION PROVIDED THAT DESIGN BEARING PRESSURES DO NOT EXCEED 2000 PSF. THE PROPOSED CUT AND FILI SLOPES OF 2.5H:1V OR FLATTER SHOULD PROVE TO BE STABLE IN THE ON-SITE NATIVE SOILS. PRIOR TO CONSTRUCTION OF THE CORE TRENCH AND EMBANKMENT FILLS. THE PROPOSED EMBANKMENT AREA SHOULD BE STRIPPED OF VEGETATION AND TOPSOIL TO EXPOSE UNDISTURBED NATIVE SOILS. THE EXPOSED SOIL SURFACE SHOULD BE PROOF ROLLED TO A UNIFORM CONDITION AND ANY EXCEPTIONALLY SOFT OR HIGHLY UNSTABLE AREAS UNDERCUT TO MORE FIRM MATERIALS. GIVEN THE GRANULAR AND SOMEWHAT MICACEOUS NATURE OF THE SURFACE SOILS, IT IS EXPECTED THAT

SOME SHIFTING AND SHALLOW RUTTING OF THE SOILS WILL BE EVIDENT. THESE WILL NOT NECESSARILY REQUIRE UNDERCUTTING ALTHOUGH THE PONDS MAY NOT MEET MD 378/2000 REQUIREMENTS. WE WOULD RECOMMEND THAT EARTHWORK CONSTRUCTION MEET THE SPECIFICATION AND THAT THE CORE TRENCH BE EXCAVATED TO THE STANDARD 4-FOOT DEPTH, 4-FOOT BOTTOM WIDTH AND 1:1 SIDE SLOPE DIMENSIONS DUE TO THE APPARENT LOOSE GRANULAR OR SOFT CLAYEY CONDITIONS ENCOUNTERED IN MAY OF THE TEST BORINGS. FOR CORE TRENCH CONSTRUCTION, THE CL SOILS AND IMMEDIATELY UNDERLYING SIMILAR ML SOILS IN THE PROPOSED CUT PORTION OF BORING B-2A WOULD IN OUR OPINION BE ACCEPTABLE FOR USE AS CORE TRENCH OR DAM CORE FILL FROM A MATERIAL-TYPE STANDPOINT.

GEOTECHNICAL MONITORING

WE RECOMMEND THAT HERBST/BENSON & ASSOCIATES BE RETAINED TO PROVIDE THE GEOTECHNICAL MONITORING AND TESTING SERVICES DURING THE EARTHWORK AND PRINCIPAL SPILLWAY CONSTRUCTION PHASES OF THE WORK. THIS IS TO OBSERVE COMPLIANCE WITH DESIGN CONCEPTS, SPECIFICATIONS OR RECOMMENDATIONS AND TO ALLOW DESIGN CHANGES IN THE EVENT THAT SUBSURFACE CONDITIONS DIFFER FROM THOSE ANTICIPATED PRIOR TO THE START OF CONSTRUCTION. THE EARTHWORK CONSTRUCTION INCLUDING STRIPPING, UNDERCUTTING, PROOF ROLLING AND CONTROLLED FILL PLACEMENT SHOULD BE INSPECTED WITH IN-PLACE DENSITY TESTS TAKEN TO VERIFY CONSTRUCTION ACCORDING TO THE SPECIFICATIONS. ALSO, THE OPEN EXCAVATIONS SUCH AS THOSE FOR THE CUTOFF TRENCH AND PRINCIPAL SPILLWAY SHOULD BE EXAMINED AND THE EXPOSED SOIL CONDITIONS APPROVED FOR COMPETENCY. WE WILL PROVIDE THE INDICATED GEOTECNICAL MONITORING AND TESTING SERVICES UPON REQUEST.

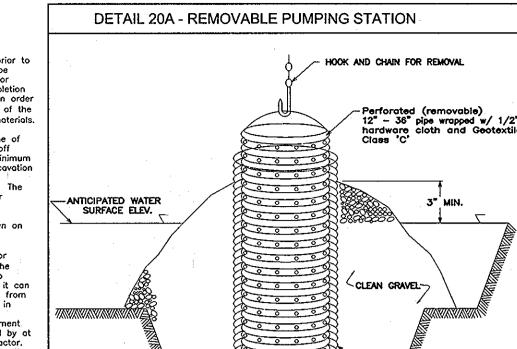
THESE PLANS HAVE BEEN REVIEWED FOR HOWARD SOIL CONSERVATION DISTRICT AND MEET THE

USDA-NATURAL RESOURCES CONSERVATION SERVICE

REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT

TECHNICAL REQUIREMENTS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL.

THESE PLANS FOR MALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL MEET THE



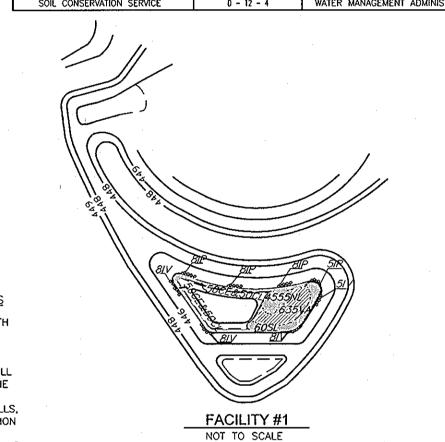
Weight as Necessary $^{?}$

0000

000

000

. The outer pipe should be 48" dia. or shall, in any case, be at least 4" greater i diameter than the center pipe. The outer pipe shall be wrapped with 1/2" hardwaloth to prevent backfill material from entering the perforations.



FACILITY #2

Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped of topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp

breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the tow of Areas to be covered by the reservoir will be cleared of all trees, brush, logs, fences, rubbish and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet

These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent version.

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoit will be stockpiled in a suitable location for use on the embankment and other designated areas.

Material — The fill material shall be taken from approved designated borrow areas. It shall be free of roots, stumps, wood, rubbish, stones greater than 6", frozen or other objectionable materials. Fill material for the center of the embankment, and cut off trench shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer. Materials used in the outer shell of the embankment must have the capability to support vegetation of the

Placement — Areas on which fill is to be placed shall be scanfied prior to placement of fill. Fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The

entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with the equipment used. The fill material shall contain sufficient moisture so that if formed into a ball it will not crumble, yet not be s

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within $+\-2\%$ of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be

with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be place so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet of the riser. Cut Off Trench — The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be at least four feet below existing grade or 4. Backfilling shall conform to "Structure Backfill".

as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with 5. Other details (anti-seep collars, valves, etc.) shall be shown on the drawings.

width of the core shall be a minimum of four feet. The height shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability. In addition the core shall be placed currently with the outer shell of the embankment. Structure Backfill

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining ill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operated closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100-200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and a minimum resistivity of 2,000 ohm-cm. Material shall be placed such that minimum of 6 (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sides of the pipe. t only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure lowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the packfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of the structure. Under no circumstances shall equipment be driven over any part of a structure or pip unless there is a compacted fill of 24" or greater over the structure or pipe. Backfill (flowable fill)zone shall be of the type and quality conforming to that specified for the core of the embankment or other embankment materials.

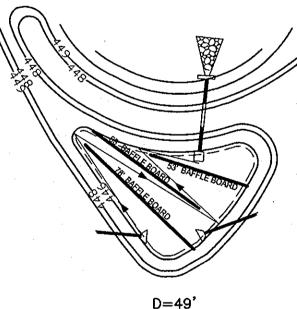
POND BOTTOM SOIL CONDITIONS

All pipes shall be circular in cross section.

Corrugated Metal Pipe — All of the following criteria shall apply for corrugated metal pipe:

1. Materials — (Polymer Coated steel pipe)— Steel pipes with polymeric coating shall have a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the

Materials — (Aluminum Coated Steel Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-274 with watertight coupling bands or flanges. Aluminum Coated Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged of the conditions were conditions were conditioned by the conditions were conditions were conditioned by the condition were conditioned by the conditions were conditioned by the condition were conditioned by the conditioned by the condition were conditional were conditional were c otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt.



A=3648 SF

We= 74

Le = 242'

 $\frac{Le}{We} = 3.27$

BAFFLE DETAIL

SEDIMENT BASIN NO.

NOT TO SCALE

1 Date Coo SIGNATURE OF ENGINEER ROBERT H. VOGEL

CERTIFY MEANS TO STATE OR DECLARE A PROFESSIONAL OPINION BASED UPON ONSITE INSPECTIONS AND MATERIAL TESTS WHICH ARE CONDUCTED DURING CONSTRUCTION. THE ONSITE SITE INSPECTION AND TESTS DEEMED SUFFICIENT AND APPROPRIATE BY COMMONLY ACCEPTED ENGINEERING STANDARDS CERTIFY DOES NOT MEAN OR IMPLY A GUARANTEE BY THE ENGINEER NOR DOES AN ENGINEER'S CERTIFICATION RELIEVE ANY OTHER PARTY FROM MEETING COMMONLY ACCEPTED INDUSTRY PRACTICES.

Materials — (Aluminum Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-196 or M-211 with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated

Coupling, bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the

pipe. Metals must be insulated from dissimilar materials with use of rubber or plastic insulating materials at lease

. Connections — All connections with pipes must be completely watertight. The drain pipe or barrel connection to

pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.

the riser shall be welded all around when the pipe and riser are metal. Anti-seep collars shall be connected to the

All connections shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches diameter: flanges on both ends of the pipe with a circular 3/8 inch thick

closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by a 24 inch long annular corrugated band using a minimum of 4(four) rods and lugs, 2 on each connecting pipe end. A 24-inch wide by 3/8-inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8'inch closed cell gaskets the full width of the flange is also acceptable.

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or

4. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth

Materials — Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or

2. Bedding — Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length.

This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used as described in the "Structure Backfill" section of this standard. Grant the control of the standard of the standard of the control of the standard of the

3. Laying pipe — Bell and spigot pipe shall be placed with the bell end upstream. Joints shall be made in accordance

shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation from the original line and grade of the pipe. The first joint must be located within 4 feet from the riser.

1. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241.

Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4" -10" inch pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" inch shall meet the

3. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth

Drainage Diaphragms – When a drainage diaphragm is used, a registered professional engineer will supervise the

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3.

Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration

Geotexile shall be placed under all riprap and shall meet requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

All work on permanent structures shall be carried out in areas free from water. The Contractor shall construct and maintain all temporary dikes, levees; cofferdams, drainage channels, and stream diversions necessary to protect to be

occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary

pumping and other equipment required for removal of water from various parts of the work and for maintaining th

excavations, foundation, and other parts of the work free from water as required or directed by the engineer for

constructing each part of the work free from water as required or directed by the engineer for constructing each part

of the work. After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream

diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water

from the required excavation and the foundation shall be accomplished in a manner and to the extent that will

of all construction operations. During the placing and compacting of material in required excavations, the water

may require draining the water sumps from which the water shall be pumped.

Area Planting (MD-342) or as shown on the accompanying drawings.

shall detail erosion and sediment control measures.

Erosion and Sediment Control

level at the locations being refilled shall be maintained below the bottom of the excavation at such locations which

All borrow areas shall be graded to provide proper drainage and left I a sightly condition. All exposed surfaces of

the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical

Construction operations will be carried out in such a manner that erosion will be controlled and water and air

pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans

If broken rock fragments are encountered at finished pond bottom, under cut a minimum of 12" below basin grade and to a horizontal distance of at least 18" beyond each edge of the

"AS-BUILT" CERTIFICATION

broken rock and backfill with fine—grained ML or CL soils compacted to a firm condition. This procedure should be performed under the supervision of the project Geotechnical Engineer.

"I HEREBY CERTIFY THAT THE FACILITY SHOWN ON THIS PLAN

WAS CONSTRUCTED AS SHOWN ON THE AS-BUILT PLANS

AND MEETS THE APPROVED PLANS AND SPECIFICATIONS."

. Backfilling shall conform to "Structure Backfill".
. Other details (anti-seep collars, volves, etc.) shall be as shown on the drawings.

Plastic Pipe — The following criteria shall apply for plastic pipe:

Standard Specifications for Construction Materials, Section 311.

compacted to provide adequate support.

Backfilling shall conform t\u00e5tr\u00fccture Backfill .

2. Joints and connections to anti-seep collars shall be completely watertight.

5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Reinforced Concrete Pipe — All of the following criteria shall apply for reinforced concrete pipe:

compacted to provide adequate support

per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be in contact with oncrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized balts

may be used for connections. The pH of the surrounding soils shall be between 4 and 9.

STORMWATER MANAGEMENT DETAILS THE PRESERVE AT WAVERLY GLEN LOTS 1-21, PRESERVATION PARCELS A-F AND NON-BUILDABLE BULK PARCEL G + 14 A RESUBDIVISION OF LOTS 1 & 2 AND PARCEL A OF SHADOW SHADE PLAT NO. 13808 AND LOT 1 OF HIGHPOINT AT BREEZEWOOD FARMS PLAT NO. 6385

REF. S-02-03, F-99-25, F-86-13 TAX MAP #10 BLOCK 23

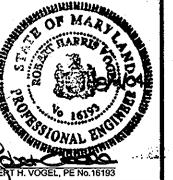
当等 ELECTION DISTRICT

PARCELS '304' & '102' HOWARD COUNTY, MARYLAND



FREDERICK WARD ASSOCIATES, INC. ENGINEERS 7125 Riverwood Drive Columbia, Maryland 21046-2354

Phone: 410-290-9550 Fax: 410-720-6226 SURVEYORS | Bel Air, Maryland Columbia, Maryland Warrenton, Virginia



A WAY	DESIGN BY:	JCO/RJ
HAPRI	DRAWN BY:	RJ
6-34-3E	CHECKED BY:	RHV
	DATE:	MAY, 2003
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ALTHUR		

12 SHEET 22

EMBANKMENT SHALL BE CONSTRUCTED OF APPROVED MATERIALS FROM THE EXCAVATIONS OR FROM OTHER SOURCES. THE MATERIAL SHALL BE FREE FROM ORGANIC MATERIALS, TRASH, MUCK, ROOTS, FROST AND OTHER

FACILITY #3

B. BEFORE DEPOSTING FILLS, THE GROUND SURFACES SHALL BE CLEARED OF ALL REFUSE, BRUSH, GRASS, ROOTS, ICE AND FROZEN MATERIAL. ALL ORGANIC MATTER AND OTHERWISE UNSUITABLE SOILS SHALL BE REMOVED FROM THE SURFACES TO BE FILLED. THE EXPOSED SURFACE SHALL BE PLOWED OR SCARIFIED IF REQUIRED TO A DEPTH OF SIX INCHES. SOILS SO SCARIFIED, OR WHICH HAVE BEEN DISTURBED BY GRUBBING AND STRIPPING OPERATIONS, SHALL BE COMPACTED TO UNDISTURBED SOIL BELOW BY DISCING, LEVELING, ROLLING, AND COMPACTING AT THE MOISTURE CONTENT AND TO THE DENSITY SPECIFIED BELOW FOR COMPACTED EMBANKMENTS.

COMPACTED FILL

WHERE FILLS ARE MADE ON HILLSIDES OR SLOPES, THE SLOPE OF THE ORIGINAL GROUND UPON WHICH THE FILL S TO BE PLACED SHALLED PLOWED OR SCARIFIED DEEPLY, OR WHERE THE SLOPE RATIO OF THE ORIGINAL GROUND IS STEEPER THAN 5 HORIZONTAL TO 1 VERTICAL, THE BANK SHALL BE STEPPED OR BENCHED, WHEN CONSIDERED NECESSARY BY THE ENGINEER, TO PERMIT PLACEMENT OF THE FILL IN HORIZONTAL LAYERS.

D. PLACING, SPREADING AND COMPACTING FILL MATERIALS: 1. THE FILL MATERIALS SHALL BE PLACED IN LAYERS WHICH, BEFORE COMPACTION SHALL NOT EXCEED 8 NCHES WITH THE EXCEPTION OF CLAY SOILS WHICH SHALL BE PLACED AND COMPACTED IN MAXIMUM 6-INCHES THICK LOOSE LIFTS. EACH LAYER SHALL BE SPREAD UNIFORMLY AND EVENLY AND SHALL BE THOROUGHLY BLADE MIXED DURING THE SPREADING TO INSURE UNIFORMITY OF MATERIALS IN EACH LAYER AND INDIVIDUAL ROCK PIECES OR SOIL CLUMPS NO GREATER THAN HALF THE LOOSE LIFT THICKNESS. AFTER EACH LAYER HAS BEEN PLACED, MIXED AND SPREAD EVENLY, IT SHALL BE THOROUGHLY COMPACTED

TO NOT LESS THAN 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D 698. 3. THE MOISTURE CONTENT OF THE FILL SHALL BE AS REQUIRED IN ORDER TO ATTAIN THE DEGREE OF COMPACTION SPECIFIED.

4. COMPACTION SHALL BE BY APPROVED MULTIPLE-WHEEL PNEUMATIC TIRED ROLLER, VIBRATORY ROLLERS OR OTHER TYPES OF ACCEPTABLE ROLLERS. 5. THE FILLING OPERATION SHALL BE CONTINUED AS SPECIFIED ABOVE UNTIL THE FILL HAS BEEN BROUGHT TO THE SUBGRADE SHOWN ON THE PLANS.

6. THE FILL SHALL BE CONSTRUCTED IN SUCH A MANNER THAT THE SURFACE WILL BE SLOPES TO DRAIN AT ALL TIMES, AND ALL FILL SHALL BEDEPOSITED TO PREVENT EXCESSIVE MOISTURE ACCUMULATION FROM

. WHEN THE WORK INTERRUPTED BY RAIN, FILING SHALL NOT BE RESUMED UNTIL TESTS INDICATE THAT THE MOISTURE CONTENT AND DENSITY OF THE TOP 6 INCHES OF FILL CONFORM TO THE ABOVE SPECIFICATION

ENGINEERS CERTIFICATE

"I HEREBY CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL. CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE

Tobal Cons SIGNATURE OF ENGINEER ROBERT H. VOGEL

POND WITHIN 30 DAYS OF COMPLETION."

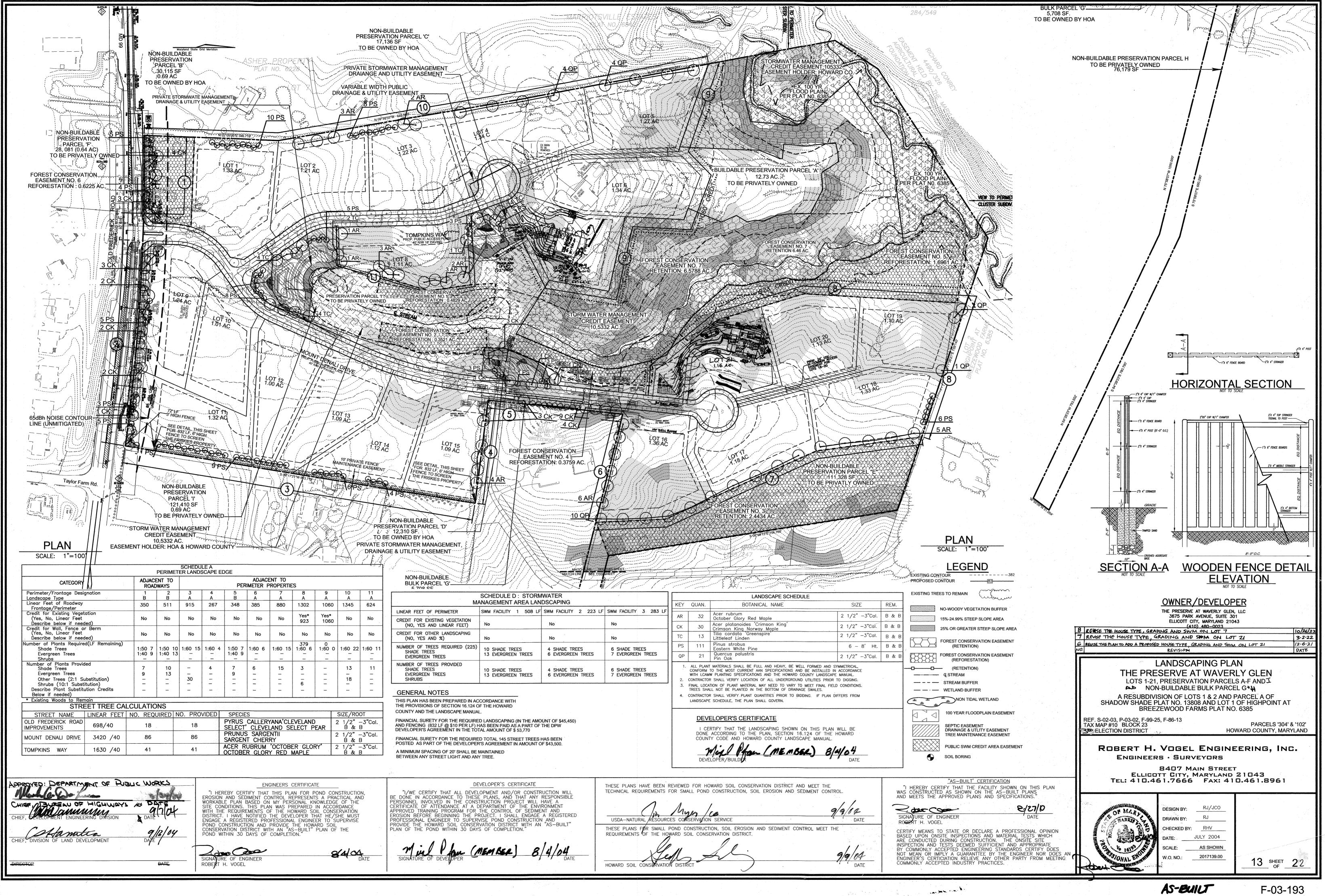
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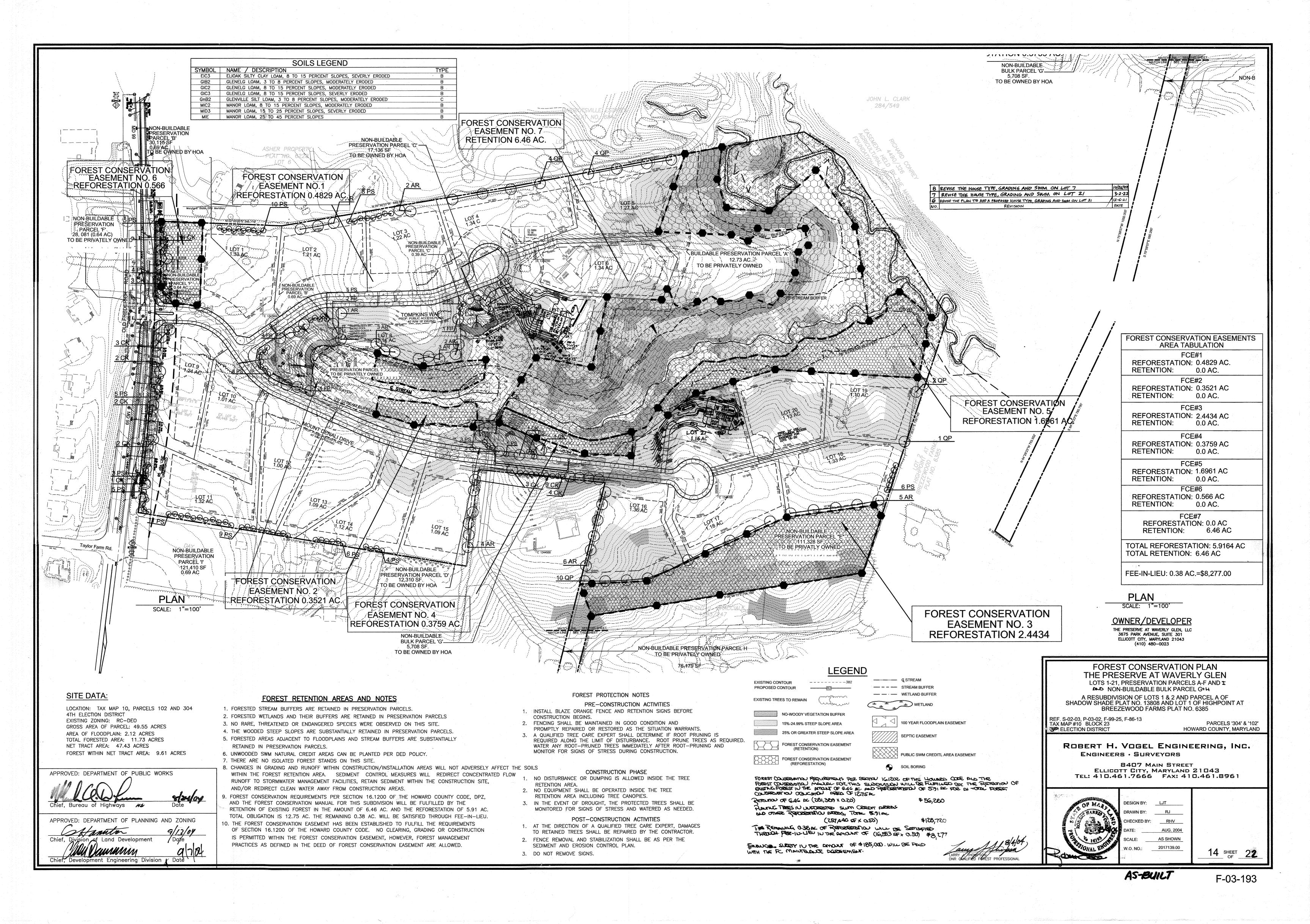
"I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE IN ACCORDANCE TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

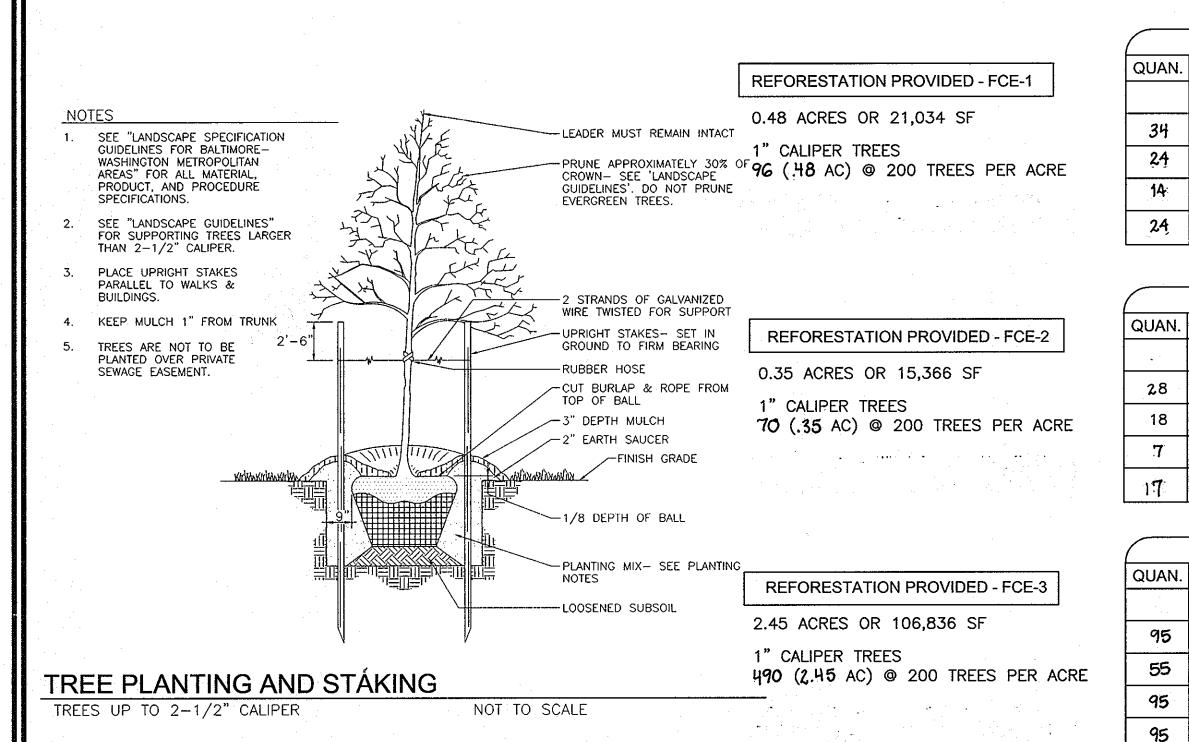
DEVELOPER'S CERTIFICATE

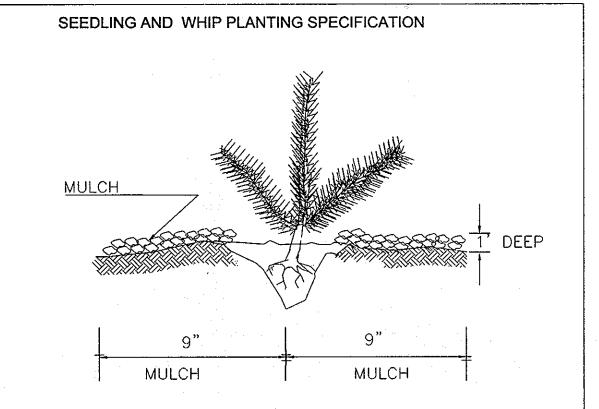
1 Poper (MEMBER) 8/4/04

AS-BUILT









NOTE: 1). PLANT MIX TO BE 1/3 PIONEER & 2/3 MID TO LATE SUCCESSIONAL SPECIES

SMALLER STOCK. 3). WHEN SHRUBS ARE SPECIFIED, PLANT THEM IN CLUSTERS.

4). DO NOT PLANT TREES IN A GRID PATTERN.

DENSITY CHART

2). PLANT LARGER STOCK AND EVERGREENS AROUND PERIMETER TO PROTECT INTERIOR

SIZE	QTY. PER ACRE	AVERAGE SPACING		
2 -3" CAL. 1" CALIPER WHIPS	100 200 350	20' x 20' 15'x15' 11'x11'		

. RANDOMLY SPACE SEEDLINGS / WHIPS SO THAT NO MORE THAN 5 OF

AREA #6 BECAUSE OF ITS PROMINENT LOCATION AT THE ENTRY TO THE SITE.

4. REFORESTATION MANAGEMENT RECORDS SHOULD BE KEPT FOR HOWARD COUNTY REVIEW.

GROWING SEASON, B. MIDSUMMER AND C. FALL. ADJUST WATERING, PEST CONTROL,

2 - 3 INCH CALIPER TREES ON AROUND PERIMETER OF REFORESTATION

WEEDING AND FEEDING AS NEEDED. AFTER ONE-YEAR REPLACE ANY TREES THAT DID NOT SURVIVE

ADJUST WATERING, PEST CONTROL, WEEDING AND FEEDING AS NEEDED. REPLACE ANY TREES THAT DID

NOT SURVIVIVE YEAR 2 DURING THE NEXT PLANTING SEASON UP TO 50% OF ORIGINAL INSTALLATION.

7. CONTRACTOR TO FOLLOW ALL STATE AND COUNTY GUIDELINES FOR AFFORESTATION & REFORESTATION.

PLANT 1" CAL TREES AROUND PERIMETER OF REFORESTATION AREA.

5. YEAR 1: INSPECT TREES FOR HEALTH AND VIGOR AT: A. BEGINNING OF

6. YEAR 2: INSPECT TREES FOR HEALTH AND VIGOR IN APRIL & OCTOBER.

ANY PARTICULAR SPECIES ARE PLANTED IN SUCCESSION.

3. ALL PROPOSED PLANT MATERIAL TO BE NATIVE PLANT SPECIES.

NO ORNAMENTAL CULTIVARS TO BE USED.

DURING THE NEXT PLANTING SEASON.

REFORESTATION PROVIDED - FCE-4

0.38 ACRES OR 16,375 SF

1" CALIPER TREES 76 (.38 AC) @ 200 TREES PER ACRE

REFORESTATION PROVIDED - FCE-5

1.70 ACRES OR 73,920 SF

1" CALIPER TREES

340 (1.70 AC) @ 200 TREES PER ACRE

REFORESTATION PROTECTION SIGNAGE	FOREST RETENTION AREA SIGNAGE		
Min. 11"———————————————————————————————————	Min. 11"		
Forest Conservation Area	FOREST RETENTION AREA		
REFORESTATION PROJECT Min. 15"	MACHINERY, DUMPING OR STORAGE OF ANY MATERIAL IS Min. 15"		
Trees for Your Future	VIOLATORS ARE SUBJECT TO FINES AS IMPOSED BY THE MARYLAND FOREST		

1. BOTTOM OF SIGNS TO BE HIGHER THAN TOP OF TREE PROTECTION FENCE. 2. SIGNS TO BE PLACED APPROXIMATELY 100' FEET APART.

3. ATTACHMENT OF SIGNS TO TREES IS PROHIBITED.

QUANTITIES AND SPACING REVISION TOTAL

	PLANT SCHEDULE							
QUAN.	BOTANICAL NAME	SIZE	SPACING (FT)					
352	Acer rubrum Red Maple	1" Cal.	15 X 15					
131	Liquidambar straciflua American Sweet Gum	1" Cal.	15 X 15					
49.	Fagus Grandifolia American Beech	1" Cal.	15 X 15					
72	Tilia Cordata "Greenspine" Little Leaf Linden	1" Cal-	15 × 15					
188	Platanus occidentalis Sycamore	1" Cal.	15 × 15					
69	Prunus serotina Black Cherry	1" Cal.	15 × 45					
185	Quercus palustris Pin Oak	1" Cal.	15 × 15					
7	Fagus Grandifolia American Beech	1" Cal.	20 × 20					
22	Acer rubrum Red Maple	2"-3" CAL.	20 × 20					
7	Platanus occidentalis Sycamore	2"-3" CAL.	20 × 20					
** 7	Acer platanoides 'Crimson King' Crimson King Norway Maple (CK)	$2\frac{1}{2}$ -3" CAL.	20 × 20					

** CREDITS FROM LANDSCAPING PRUNUS SEROTINA 2" CAL. 20 × 20 BLACK CHERRY QUERCUS PALUSTRUS 20 × 20 2" CAL PIN OAK FOREST CONSERVATION WORKSHEET

SIZE

え" CAL.

2" CAL.

2" CAL.

. 2" CAL

Z" CAL

20 X 20

20 X 20

20 × 20

PLANT SCHEDULE - FCE - 6

BOTANICAL NAME

Acer platanoides 'Crimson King'

Crimson King Norway Maple (CK) 2

Platanus occidentalis

Acer rubrum Red Maple

Sycamore

Pin Oak

USE 2" X 4" LUMBER FOR CROSS BEARING

- ANCHOR POSTS MUST BE

Prunus serotina

Quercus palustris

Fagus Grandifolia

** CREDITS FROM LANDSCAPING

American Beech

Black Cherry

HOWARD COUNTY

49.55 AC.

2.11 AC

0.00 AC

47.44 AC

20% X D = 9.49 AC

25% X D = 11.86 AC

= 3.15 AC

= 6.46 AC

0.00 AC

6.29 AC

ZONED RC-DEO SPACING (FT) **NET TRACT AREA:** 20 X 20 TOTAL TRACT AREA AREA WITHIN 100 YEAR FLOODPLAIN 20 X 20 AREA TO REMAIN IN AGRICULTURAL PRODUCTION NET TRACT AREA 20 × 20 LAND USE CATEGORY

INPUT THE NUMBER "1" UNDER THE APPROPIATE LAND USE ZONING, AND LIMIT TO ONLY ONE ENTRY.

IDA 1 0 0 0 0

CONSERVATION THRESHOLD

AFFOREST THRESHOLD

EXISTING FOREST COVER: EXISTING FOREST COVER (EXCLUDING FLOODPLAIN) 9.61 AC 0.12 AC AREA OF FOREST ABOVE AFFORESTATION THRESHOLD = AREA OF FOREST ABOVE CONSERVATION THRESHOLD = 0.00 AC

BREAK EVEN POINT:

 $(.2 \times 1) + F = BREAK EVEN POINT (11.86 AC)$ FOREST RETENTION WITH NO MITIGATION = 9.61 AC CLEARING PERMITTED WITHOUT MITIGATION = 0.00 AC

PROPOSED FOREST CLEARING:

TOTAL AREA OF FOREST TO BE CLEARED

TOTAL AREA OF FOREST TO BE RETAINED

TOTAL AFFORESTATION REQUIRED

PLANTING REQUIREMENTS: REFORESTATION FOR CLEARING ABOVE CONSERVATION THRESHOLD = 0.00 ACREFORESTATION FOR CLEARING BELOW CONSERVATION THRESHOLD 6.29 AC CREDIT FOR RETENTION ABOVE CONSERVATION THRESHOLD (M-F) = 0.00 AC TOTAL REFORESTATION REQUIRED (N+P-Q) 6.29 AC

TOTAL REQUIRED FOREST CONSERVATION OBLIGATION IS 12.75 ACRES. (RETENTION-6.46 ACRES, REFORESTATION-6.29 ACRES COST ESTIMATE: (For bonding purposes, only) RETENTION OF 6.46 ACRES (281,398 SF X 0.20) \$56,280

TOTAL REFORESTATION AND AFFORESTATION REQUIRED

PLANTING TREES IN UNFORESTED SWM CREDIT AREAS AND OTHER REFORESTATION AREAS, TOTAL 5.91 ACRES (257,440 SF X 0.50) \$128,720 THE REMAINING 0.38 AC OF REFORESTATION WILL BE SATISFIED

THROUGH FEE-IN-LIEU IN THE AMOUNT OF: (16,554 SF X 0.50) \$8,277

FINANCIAL SURETY IN THE AMOUNT OF \$185,000 WILL BE PAID WITH THE FC MAINTENANCE AGREEMENT. THE SURETY AMOUNT IS SUBJECT TO THE FINAL PLAN APPROVAL.

PLANT SCHEDULE - FCE - 1

PLANT SCHEDULE - FCE - 2

PLANT SCHEDULE - FCE - 3

PLANT SCHEDULE - FCE - 4

PLANT SCHEDULE - FCE - 5

BOTANICAL NAME

BOTANICAL NAME

SIZE

1" Cal-

1" Cal.

1" Càl.

1" Cal.

SIZE

1" Cal.

1^u Cal.

1" Cal.

1" Cal.

SIZE

1" Cal.

1" Cal.

1" Cal.

1" Cal.

1" Cal.

1" Cal.

SIZE

1". Cal.

1" Cal.

1" Cal.

1" Cal.

1" Cal

SIZE

1" Cal.

1" Cal.

1". Cal.

1" Cal.

1"- Cal.

1" Cal.

SPACING (FT)

15 X 15

15 X 15

15 × 15

15 X 15

SPACING (FT)

15 X 15

15 X 15

15 X 15

15 X 15

SPACING (FT)

15 X 15

SPACING (FT)

15 × 15

15 x 15

15 x 15

15 x 15

15 x 15

SPACING (FT)

15 X 15

15 × 15

BOTANICAL NAME

BOTANICAL NAME

3年 Acer rubrum Red Maple

Sycamore

| Prunus serotina

Quercus palustris Pin Oak

Black Cherry

Acer rubrum

Platanus occidentalis

BOTANICAL NAME

Liquidambar straciflua

Tilia cordata 'Greenspire'

American Sweet Gum

Platanus occidentalis

Littleleaf Linden

Acer rubrum Red Maple

Sycamore

Pin Oak

QUAN.

15

QUAN.

97

Prunus serotina

Quercus palustris

Black Cherry

Acer rubrum

Sycamore

Prunus serotina

Quercus palustris Pin Oak

Black Cherry

Platanus occidentalis

Tilia cordata Greenspire

Liquidambar straciflua

<u>American Sweet Gum</u>

Platanus occidentalis

Tilia cordata 'Greenspire

Fagus Grandifolia

American Beech

Prunus serotina

Quercus palustris

Littleleaf Linden

Black Cherry

Acer rubrum Red Maple

Sycamore

Pin Oak

32 | Acer rubrui

Red Maple

Sycamore

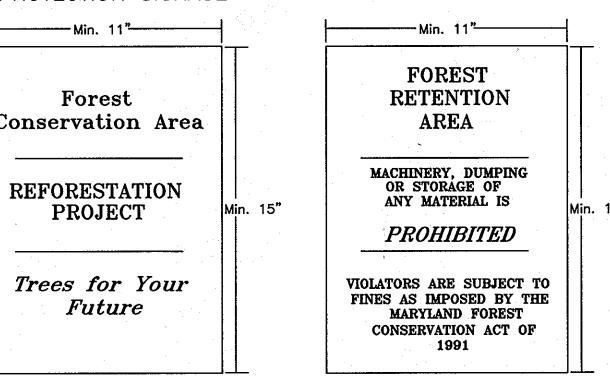
Pin Oak

Prunus serotina

Quercus palustris

Black Cherry

Platanus occidentalis



REVISED PLANT FCE SCHEDULES - PLANT SIZE, 6-8-05 DATE

OWNER/DEVELOPER THE PRESERVE AT WAVERLY GLEN, LLC 3675 PARK AVENUE, SUITE 301 ELLICOTT CITY, MARYLAND 21043 (410) 480-0023

TOTAL

1072 (5.36 AC) @ 200 TREES PER ACRE

(** CREDIT FOR 7 21/2-3" CAL. TREES FROM LANDSCAPING)

REFORESTATION PROVIDED - FCE-6

.57 (.57 AC) @ 100 TREES PER ACRE

0.57 ACRES OR 24,674 SF

CRITICAL ROOT ZONE

ANCHOR POSTS SHOULD BE MINIMUM 2" STEEL "U" CHANNEL OR 2" X 2" TIMBER, 6' IN LENGTH.

3. EXACT LOCATION OF TRENCH SHOULD BE IDENTIFIED.

2" CALIPER TREES

2 1/2" - 3" CALIPER TREES

QUAN.

--- TREE PROTECTION FENCE

/ | / | DISTURBANCE LINE LIMIT OF DISTURBANCE

TRENCH WITHIN 1' OF LIMIT OF

2' MINIMUM DEPTH

`── 6" MAXIMUM WIDTH

1. RETENTION AREAS TO BE ESTABLISHED AS PART OF THE FOREST CONSERVATION PLAN REVIEW PROCESS.

HIGHLY VISABLE FLAGGING

MAXIMUM 8 FEET

1. FOREST PROTECTION DEVICE ONLY.

4. ROOF DAMAGE SHOULD BE AVOIDED.

5. PROTECTION SIGNAGE SHOULD BE USED.

2. RETENTION AREA WILL* BE SET AS PART OF THE REVIEW PROCESS

6. DEVICE SHOULD BE MAINTAINED THROUGHOUT CONSTRUCTION.

BLAZE ORANGE PLASTIC MESH

TYPICAL TREE PROTECTION FENCE DETAIL

NO SCALE

BOUNDARIES OF RETENTION AREA SHOULD BE STAKED AND FLAGGED PRIOR TO INSTALLING DEVICE.

2. BOUNDARIES OF RETENTION AREAS TO BE STAKED, FLAGGED AND/OR FENCED PRIOR TRENCHING.

5. ROOTS SHOULD BE CLEANLY CUT USING VIBRATORY KNIFE OR OTHER ACCEPTABLE EQUIPMENT.

4. TRENCH SHOULD BE IMMEDIATELY BACKFILLED WITH SOIL REMOVED OR ORGANIC SOIL.

ROOT PRUNING

22

REFORESTATION PROVIDED - TOTAL

5.92 ACRES OR 257,875 SF

57 (0.57 AC)@ 100 TREES PER ACRE

1" CALIPER TREES

2" CALIPER TREES

1714

APPROVED: DEPARTMENT OF PLANNING AND ZONING Manula

Chief, Division of Land Development Manumi Date Chief, Development Engineering Division

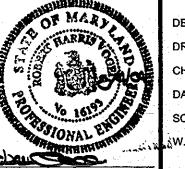
FOREST CONSERVATION DETAILS THE PRESERVE AT WAVERLY GLEN LOTS 1-21, PRESERVATION PARCELS A-F AND I NON-BUILDABLE BULK PARCEL G+H A RESUBDIVISION OF LOTS 1 & 2 AND PARCEL A OF SHADOW SHADE PLAT NO. 13808 AND LOT 1 OF HIGHPOINT AT BREEZEWOOD FARMS PLAT NO. 6385

REF. S-02-03, P-03-02, F-99-25, F-86-13 TAX MAP #10 BLOCK 23 SE ELECTION DISTRICT

HOWARD COUNTY, MARYLAND ROBERT H. VOGEL ENGINEERING, INC.

ENGINEERS . SURVEYORS 8407 MAIN STREET ELLICOTT CITY, MARYLAND 21043

TEL: 410.461.7666 FAX: 410.461.8961

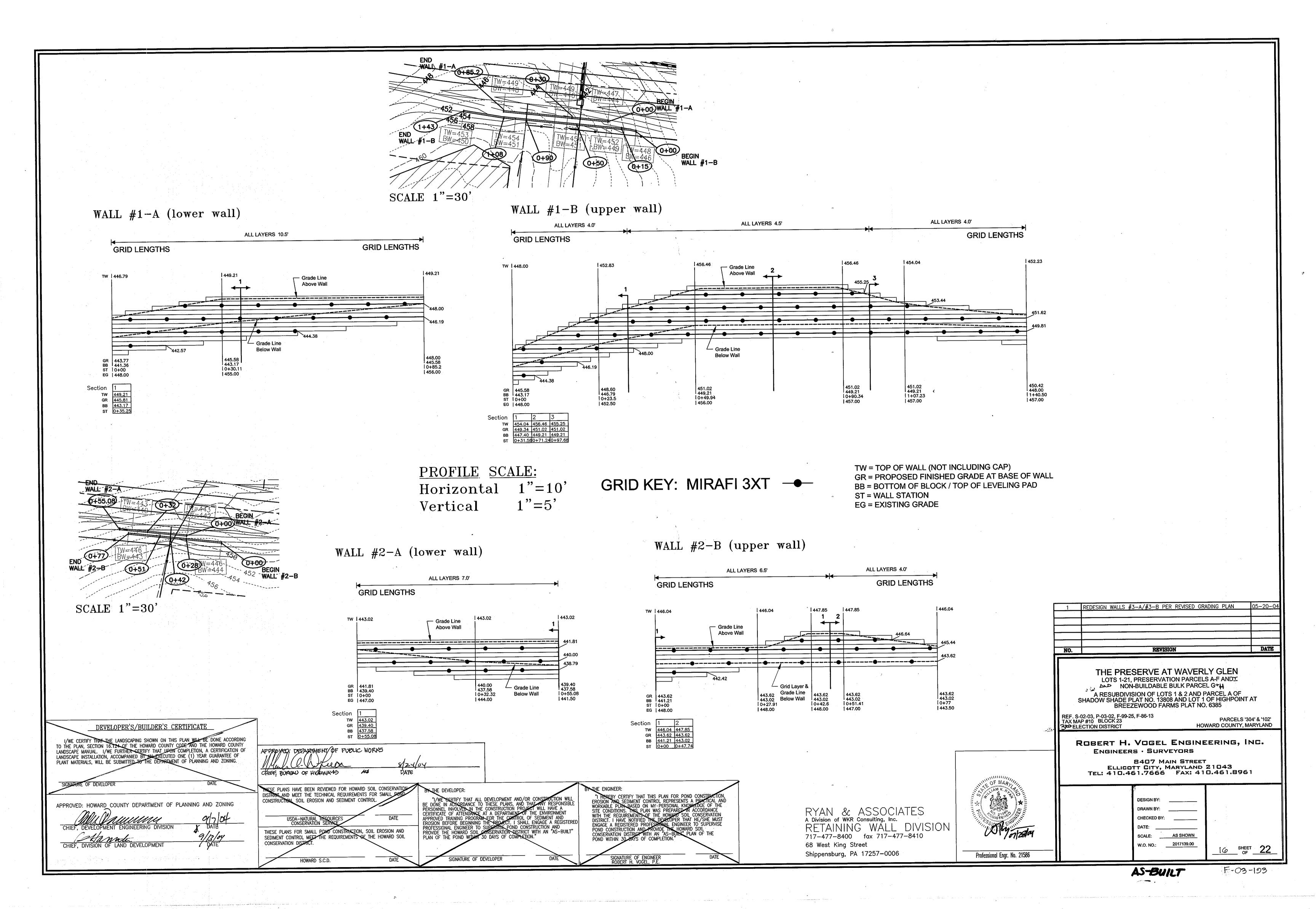


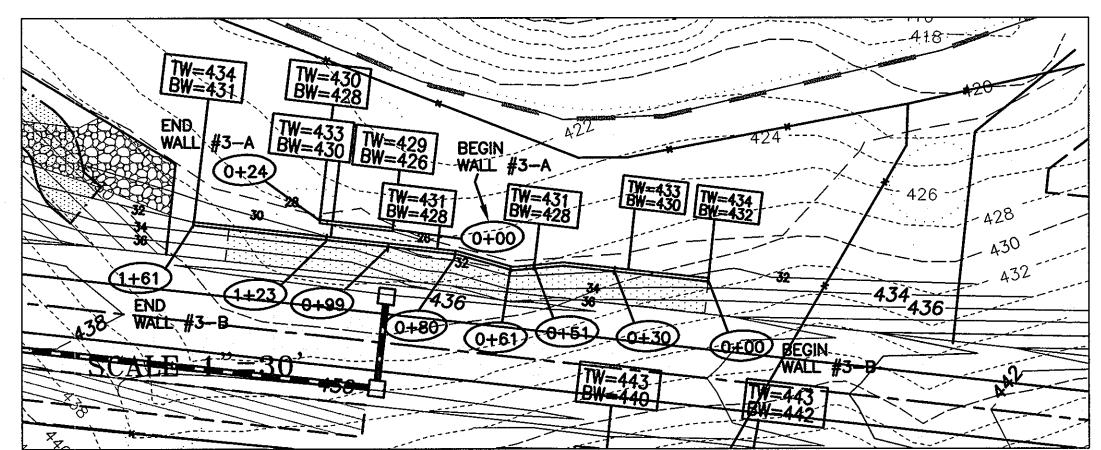
RJ/JCO CHECKED BY: AUG. 2004 AS SHOWN 2017139.00

15 SHEET 22

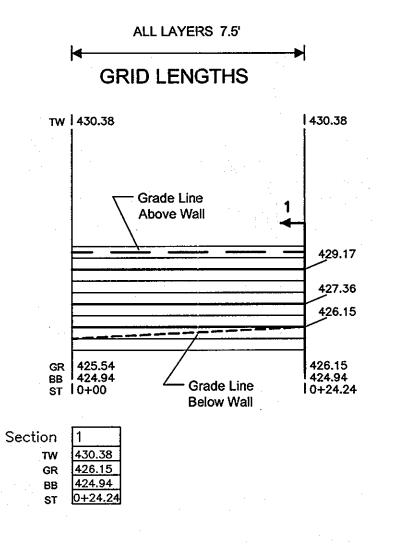
F-03-193

PARCELS '304' & '102'

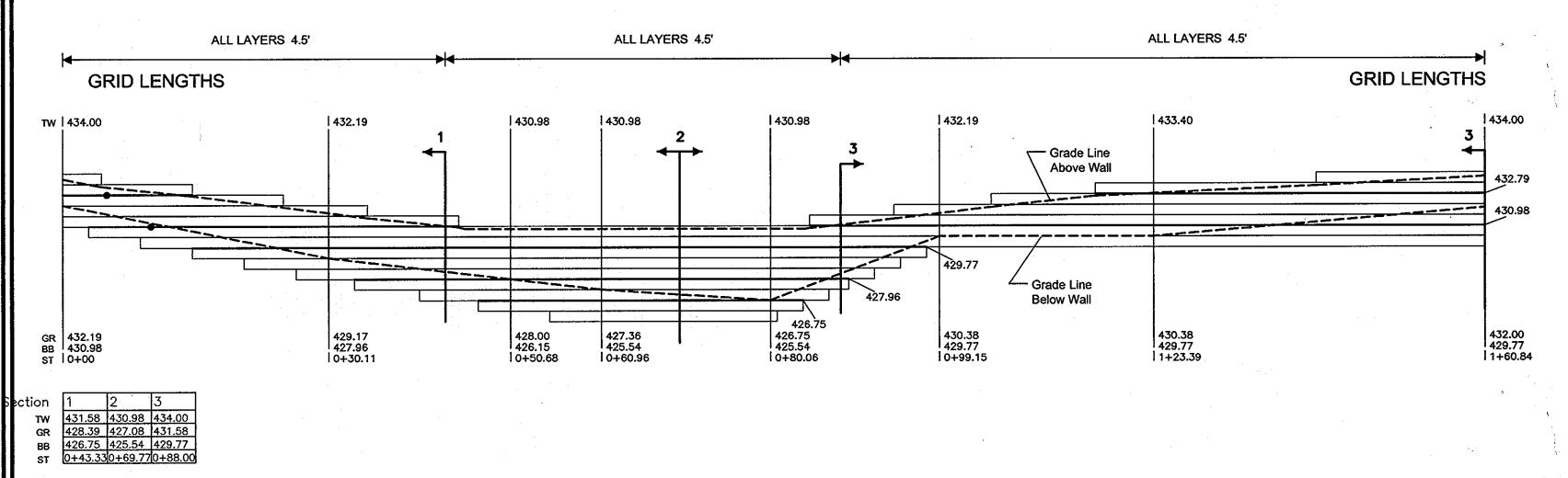




WALL #3-A (lower wall)



WALL #3-B (upper wall)



DEVELOPER'S/BUILDER'S CERTIFICATE

I/WE CERTIFY THAT THE LANDSCAPING SHOWN ON THIS PLAN WILL BE DONE ACCORDING TO THE PLAN, SECTION 16.124 OF THE HOWARD COUNTY CODE AND THE HOWARD COUNTY LANDSCAPE MANUAL. I/WE FURTHER GERTIFY THAT LIPON COMPLETION, A CERTIFICATION OF LANDSCAPE INSTALLATION, ACCOMPANIED BY AN EXECUTED ONE (1) YEAR GUARANTEE OF PLANT MATERIALS, WILL BE SUBMITTED TO THE DEPARTMENT OF PLANNING AND ZONING.

SIGNATURE OF DEVELOPER

DATE

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

CHIEF, DEVELOPMENT ENGINEERING DIVISION

CHIEF, DIVISION OF LAND DEVELOPMENT

DATE

CHIEF, DIVISION OF LAND DEVELOPMENT

DATE

GRID KEY: MIRAFI 3XT →

TW = TOP OF WALL (NOT INCLUDING CAP)
GR = PROPOSED FINISHED GRADE AT BASE OF WALL
BB = BOTTOM OF BLOCK / TOP OF LEVELING PAD

ST = WALL STATION EG = EXISTING GRADE

HOWARD S.C.D.

THESE PLANS HAVE BEEN REVIEWED FOR HOWARD SOIL CONSERVATION DISTRICT AND MEET THE TECHNICAL REQUIREMENTS FOR SMALL BOND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL.

USDA-NATURAL RESOURCES DATE
CONSERVATION SERVICE

THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.

BY THE DEVELOPER:

"I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL
BE DONE IN ACCORDANCE TO THESE PLANS, AND THAT MY RESPONSIBLE
PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A
CERTIFICATE OF ATTENDANSE AT A DEPARTMENT OF THE ENVIRONMENT
APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND
EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED
PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND
PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT"
PLAN OF THE POND WITHIN 30 DAYS OF COMPESSION."

SIGNATURE OF DEVELOPER

SCALE:
Horizontal 1"=10'
Vertical 1"=5'

BY THE ENGINEER:

"I HEREBY CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUNL" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION."

SIGNATURE OF ENGINEER ROBERT H. VOGEL, P.E.

Shipper

GENERAL NOTES

1. SOIL PARAMETERS: Based on review of Boring B-8A from the geotechnical report for this project, an internal angle of friction of 28° was used for the soils in these designs. This will need to be verified during wall construction since borings have not been done at the exact wall locations. The in-situ soils have been classified as ML (silt) and SM (silty sand), so the conservative 28° value was used for the fine-grained ML. CH (fat clay), CL (lean clay), MH (elastic silt) and OH/ OL/ PT (organic) soils are not acceptable for wall construction. If these unsuitable soils are encountered within the foundation zones (the walls' sub-grades) or retained zones (behind the walls' reinforced geogrid zones and extending to distances that are twice the walls' exposed heights) they must be removed and replaced with soils that meet or exceed the design friction angle of 28°. The site geotechnical engineer shall monitor this closely during the construction process. An assumed unit weight of 125 PCF was used (maximum wet density) and fluctuations of 5 PCF higher or lower will not affect these designs. However if the unit weight varies by more than 5 PCF Ryan & Associates (RA) must be notified so that the cross sections can be rerun to verify that all factors of safety are still met. No cohesion was used in any of the external and internal calculations, however a conservative 25# of cohesion was used in the global stability analyses since the site soils exhibit cohesive properties (the borings indicate clay and silt content). See also item number 13 below.

2. BEARING CAPACITY: The wall's sub-grades (the soils under the walls' gravel leveling pads and the soils under the walls' reinforced geogrid zones) must be tested by the site geotechnical engineer prior to wall construction and have minimum allowable bearing capacities of 2,500 PSF. The sub-grades must be virgin (natural undisturbed soil with blow counts (≥12) or suitable fill (≥28°) compacted to 95 % of a standard proctor maximum dry density. Any areas of the sub-grades that test below these maximum pressures will require undercutting and/or geogrid reinforcing.

3. GEOGRID: Mirafi 3XT geogrid, which has a LTDS (Long Term Design Strength) of 1558, was used in these designs. All geogrid substitutions must have prior approval of RA.

4. BLOCK SYSTEM: These designs are valid only for the Allan Block system. Each segmental wall system has unique dimensions, connection devices and interacts differently with geogrids; therefore substitutions of other block types are not permitted without a total redesign.

5. WALL BATTER: These walls were designed with Allan Block units having a 6° batter (3/4" setback per block course). NOTE: it is important for the wall installer and the civil engineer/ surveyor to predetermine the walls' batters during stake out. The bases of the walls will need to be moved forward if there are critical dimensions that need to be met on the high side of the walls.

6. CONSTRUCTION OVERSIGHT: The construction of these walls must be performed under the observation of a Maryland Registered Professional Engineer to ensure that they are built in accordance with the RA General Notes and Specifications.

7. EMBEDMENT: Wall #1-A: 4 blocks. Wall #1-B: 4 blocks decreasing to 3 blocks from station 0+00 to 0+23, 3 blocks from 0+23 to 1+17 and 3 blocks increasing to 4 block from 1+17 to 1+40. Wall #2-A: 4 blocks from station 0+00 to 0+32 and 4 blocks decreasing to 3 blocks from 0+32 to 0+55. Wall #2-B: 4 blocks decreasing to 1 block from station 0+00 to 0+28 and 1 block from 0+28 to 0+77. Wall #3-A: 1 block increasing to 2 blocks from station 0+00 to 0+24. Wall #3-B: 2 blocks from station 0+00 to 0+30, 2 blocks increasing to 3 blocks from 0+30 to 0+51, 3 blocks from 0+51 to 0+61, 3 blocks decreasing to 2 blocks from 0+61 to 0+80, 2 blocks decreasing to 1 block from 0+80 to 0+99, 1 block from 0+99 to 1+23 and 1 block increasing to 3 blocks from 1+23 to 1+61.

8. SLOPES & SURCHARGES: The following loads were applied to the walls on this project. Wall #1—A: a 700 PSF dead load surcharge for the weight of Wall #1—B. Wall #1—B: a 2:1 slope and a 50 PSF live load surcharge for potential pedestrian loads. Wall #2—A: a 540 PSF dead load surcharge for the weight of Wall #2—B. Wall #2—B: A 2:1 slope on Section 1 and a 3:1 slope on Section 2. Wall #3—A: a 620 PSF dead load surcharge for the weight of Wall #1—B. Wall #3—B: a 250 PSF live load surcharge for the proposed roadway and vehicles.

9. WALL PROFILES: The elevation drawings were done to represent the grade changes necessary on the civil drawings. They were done in exact block course increments of .604' (7.25") so they will differ slightly from the civil plan elevations. Minor field changes may be necessary by the wall installer. Lineal footage may be added or subtracted as needed if the wall heights are equal to or less than the design heights. If the wall installer needs to construct the walls to a height that exceeds the design sections, RA must be notified and higher cross sections must be provided before proceeding. NOTE: the cap height of .333' (4") is not shown on the profile drawings however its height may have been used in some cases to achieve the desired TW elevations.

10. SPECIAL HOWARD COUNTY RETAINING WALL SPECIFICATIONS:

a. Retaining walls shall only be constructed under the observation of a Registered Professional Engineer and a (NICET, WACEL, or equivalent) certified soils technician.
b. The required bearing pressure beneath the footing of the wall shall be verified in the field by a

b. The required bearing pressure beneath the footing of the wall shall be verified in the field by a certified soils technician. Testing documentation shall be provided to the Howard County Inspector prior to the start of construction. The required test procedure shall be the Dynamic Cone Penetrometer Test ASTM STP—399.

c. The suitability of the fill material shall be confirmed by the on—site soils technician. Each eight inch lift must be compacted to 95% Standard Proctor Density and the testing report shall be made available to the Howard County Inspector upon completion of the construction.

d. For walls over ten feet in height, one soil boring is required every 100 feet along the length of the wall, copies of the boring reports shall be provided to the Howard County Inspector prior to the start of construction.

11. EXISTING GRADE ELEVATIONS: RA has indicated the existing grade elevations on the profiles. In most cases the walls are being built in major cut situations so these elevations could not be shown on the profiles (they are well above the TW elevations). Additional blocks were buried at the ends of Walls #2-B, #3-A & #3-B to ensure that the all sections of all walls are built on virgin soil and not fill.

12. SPECIFICATIONS: Construction and materials must conform to the attached "Ryan & Associates segmental retaining specifications and installation guidelines for Allan Block" and the Allan Block "Installation Guide for Retaining Walls".

APPROXICE DEPARTMENT OF RUBLIC WORKS

13. GLOBAL STABILITY: Global stability analyses were required because of the terraced walls and slopes. The geogrids were lengthened on the lower walls until 1.3 factors of safety were met. As stated under "Soil Parameters" (item number 1 above), a conservative value of 25# of cohesion was used. The site geotechnical engineer may verify this by performing unconfined compression tests. The geogrid lengths on Walls #1—A and #3—A had to be lengthened significantly. Global analyses were also run with an increased amount of cohesion (50#). The geogrids can be shortened by 1.5' on both Wall #1—A (from 10.5' to 9.0') and on Wall #3—A (from 8.5' to 7.0') with 25# more cohesion. RA will not assume this increased value. If the contractor would like to exercise this option to shorten the geogrid lengths on these two walls, the site geotechnical engineer must perform laboratory tests and provide them to RA prior to wall construction.

8/24/04

RYAN & ASSOCIATES

717-477-8400 fax 717-477-8410

RETAINING WALL DIVISION

A Division of WKR Consulting, Inc.

Shippensburg, PA 17257-0006

68 West King Street

14. FACTORS OF SAFETY: The following factors of safety have been met in these designs: Sliding 1.5, Overturning 2.0, Bearing Capacity 2.0, Geogrid Overstress 1.5, Geogrid Pullout 1.5 (from the block and from the soil) and Global Stability 1.3.

15. BACK SLOPES: Water management is especially critical since there are back slopes above portions of these walls. Since water is being directed to the walls, the top 8" of compacted fill over the reinforced geogrid zones must have impermeable soil (clay— such as CL, GC or SC) or an underlying geomembrane (see RA Specifications for details). The soil may come to the tops of the walls (tops of caps) so that the water is directed over them (sheet flow) or swales may be constructed (clay, concrete or asphalt: see attached detail) behind the walls to divert the water around the ends of the wall. These swales should have minimum depths of 8" and minimum 1−2% slopes laterally from the high points to the ends of the walls. In no case should the surface water be allowed to pond and saturate the reinforced geogrid zones or be introduced into the 12" drainage layers. NOTE: the soils in the back slopes and retained zones must be virgin (natural undisturbed soil with blow counts (≥12) or suitable fill (≥28) compacted to 95% of a standard proctor maximum dry density. This must be verified by the site geotechnical engineer.

16. SEPARATE 81/2" X 11" SUBMITTAL: These 24" X 36" sheets were done in conjunction with an 81/2" X 11" submittal. The cross section calculations and global stability analyses are included in the 8 1/2" X 11" submittal.

17. DESIGN SOFTWARE: Internal and external wall calculations were performed with NCMA's SRWall (version 3.22) and the global stability calculations were done with G-Slope. A table has been included ("Cross Section Details and Factors of Safety") which has the following information: section location (area of wall referenced), total wall height, loads applied, factors of safety (for sliding, overturning, bearing capacity and global stability) and bearing pressure (the weight exerted by the wall). Factors of safety of 1.5 were also met for: geogrid pullout (from the soil and from the block), geogrid overstress (geogrid rupture) and connection (block to geogrid).

18. MANDATORY REAR DRAIN PIPES: Because these walls are being built in "cut" situations and water is likely to follow the vertical seams between the in—situ retained soils and the infill soils, RA is requiring that rear drainpipes be installed at the bottom of the reinforced geogrid zones of the lower walls (#1-A, #2-A & #3-A). This is in addition to the mandatory 4" drainpipes in the 12" drainage layer at the front of the wall. These rear drain tiles shall be a continuous 4" perforated pipes that are surrounded by a minimum of 12" of clean gravel (#57 or equivalent). They shall be wrapped in filter fabric and be vented through the wall face at maximum 30' O.C. (with the use of tees and solid 4" pipes running perpendicular to the wall face) or be connected into the project's stormwater system (below grade). If they are vented through the wall faces they will need to be positioned high enough (just above proposed finished grade elevation at the front of the walls) so that positive flow to the wall faces is maintained.

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	NO.		REVISION		DATE
	REF. S-C TAX MAI 3801 ELE	LOTS 1-21 A RESUBDIVI HADOW SHADE F BREE 02-03, P-03-02, F-99-2 #10 BLOCK 23 CTION DISTRICT ENGINEER ELLICO		S A-F AND I EL G+H PARCEL A OF OF HIGHPOINT A D. 6385 PARCELS '304 WARD COUNTY, MA EERING, IN	4' & '102' RYLAND
-			DESIGN BY: DRAWN BY: CHECKED BY: DATE: SCALE: AS SHOWN W.O. NO.: 2017139.00	17 SHEET OF	22

REDESIGN WALLS #3-A/#3-B PER REVISED GRADING PLAN

Professional Engr. No. 21586

ALLAN BLOCK TERRACED WALL SECTION FINISHED GRADE CAPSTONE - ABOVE WALL .333' H X 1.469' W X 1.0'D (0.44 SQ. FT.) —— ALLAN BLOCK UNITS 7.25' H X 1.469' W IMPERMEABLE FILL TO MINIMUM WALL BATTER THICKNESS OF 8" TO AVOID SATURATION OF REINFORCED ZONE X 1.0' D (0.9 SQ. FT.) FROM VERTICAL MAXIMUM GEOGRID SPACING IS MINIMUM DRAINAGE IMPERMEABLE FILL TO MINIMUM THICKNESS OF 8" TO AVOID SATURATION OF REINFORCED ZONE THREE COURSES (SEE ELEVATION DRAWINGS FOR LENGTH, STRENGTH (3/4")LAYER AND PLACEMENT) setback per course) $\geq 28 \varnothing$ ✓ RETAINED SOIL -≥ 28 Ø FINISHED GRADE~ BELOW WALL 4" PERFORATED DRAIN PIPE VENTED TO DAYLIGHT (A CHIMNEY DRAIN OR COMPOSITE BLANKET IS ALSO REQUIRED IN CASES OF GROUND WATER OR CONTINUOUS WATER SEEPAGE.) ∠ INFILL SOIL ——— ≥ 28 Ø EMBEDMENT DEPTH (VARIES - SEE ELEVATION DRAWNGS) APPROVED SUB-GRADE ≥ 2,500 PSF 4" PERFORATED CONTINUOUS DRAIN PIPE VENTED TO DAYLIGHT JUST ABOVE FINISHED GRADE THROUGH THE WALL FACE AT STATIONS 0+82, 1+65 & 2+48 (SEE ELEVATION DRAWING) PAD: .5'X 2.0' MINIMUM (#57, CR6, 21A, 2A, 2B, ETC.) (VIRGIN UNDISTURBED SOIL OR SUITABLE FILL COMPACTED TO 1" BURIED PER 1' EXPOSED (1 BLOCK MINIMUM) 95% OF A STANDARD PROCTOR MAXIMUM DRY DENSITY) THE WALL INSTALLER AND SITE GEOTECHNICAL ENGINEER SHALL DETERMINE WHICH METHOD SHOULD BE USED TO MAINTAIN POSITIVE FLOW OF WATER OUT OF THE

MATERIAL ESTIMATE* FOR:										
WALL#	TOTAL SQ. FT.	(.9 S. F.) 6° BLOCK	(.44 S. F.) CAPS	(.44 S. F.) LITE STONES**	SQ. YDS. 3XT <u>GRID</u>	CU. YDS. DRAIN GRAVEL	CU. YDS. LEVELING PAD GRAVEL	FT. DRAIN <u>PIPE</u>	FT. WALL LENGTH	
1-A	487	510	60	4	275	29	7	89	85	
1-B	949	995	101	21	250	56	11	148	141	
2-A	301	315	40	0	135	18	4	58	55	
2-B	356	365	56	6	145	21	6	81	77	
3-A	147	155	17	0	75	9	2	25	24	
3-B	739	760	114	<u>10</u>	200	44	<u>13</u>	<u>169</u>	<u>161</u>	
TOTAL	2,979	3,100	388	41	1,080	176	42	570	543	

ADPROVED: DEPARTMENT OF PUBLIC WORKS

NOTE: Quantities from the Allan Block software were increased by the following percentages: block & caps 3.0.%, grid 15% and gravel 5%.

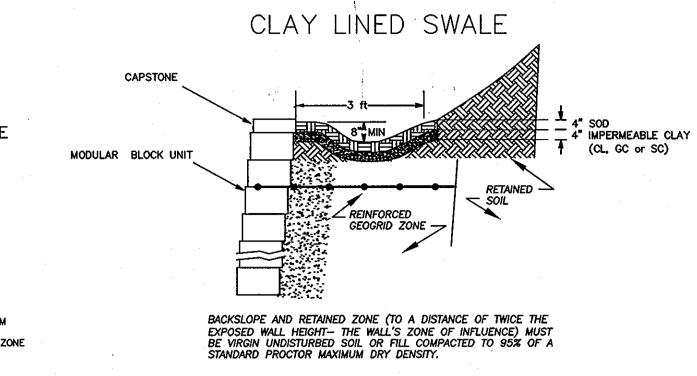
* Ryan & Associates is not responsible for extras or shortages based on this take-off. The recipient is responsible for verifying the accuracy of this design by reviewing the site/ grading plan for this project or by taking field measurements.

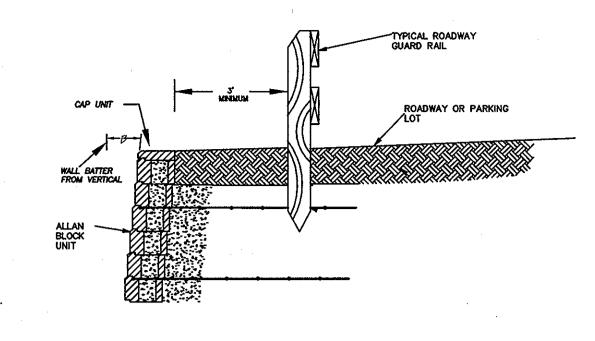
**Lite quantity is based on one unit for each step down transition on the top of the wall.

DEVELOPER'S/BUILDER'S CERTIFICATE

I/WE CERTIFY THAT THE LANDSCAPING SHOWN ON THIS PLAN WILL BE DONE ACCORDING TO THE PLAN, SECTION 16.124 OF THE HOWARD COUNTY CODE AND THE HOWARD COUNTY LANDSCAPE MANUAL. I/WE FURTHER CERTIFY THAT UPON COMPLETION, A CERTIFICATION OF LANDSCAPE INSTALLATION, ACCOMPANIED BY AN EXECUTED ONE (1) YEAR GUARANTEE OF PLANT MATERIALS, WILL BE SUBMITTED TO THE DEPARTMENT OF PLANNING AND ZONING.

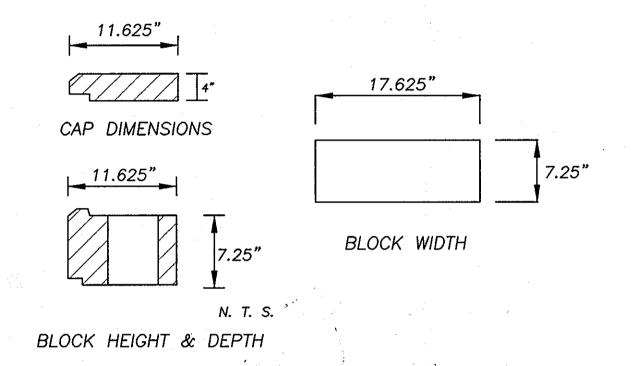
APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

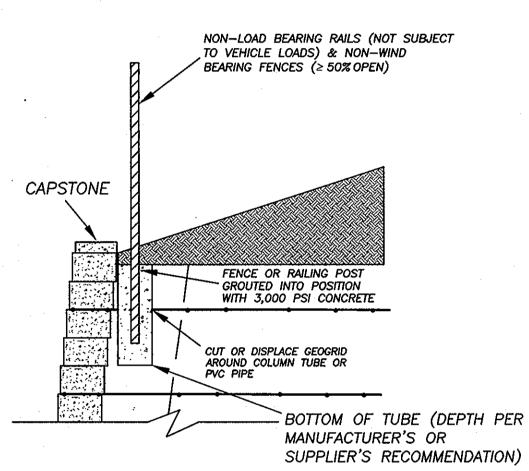




N. T. S.

ALLAN BLOCK BLOCK DIMENSIONS





NON-LOAD BEARING & NON-WIND BEARING (INSTALLED BEHIND WALL)

N.T.S.

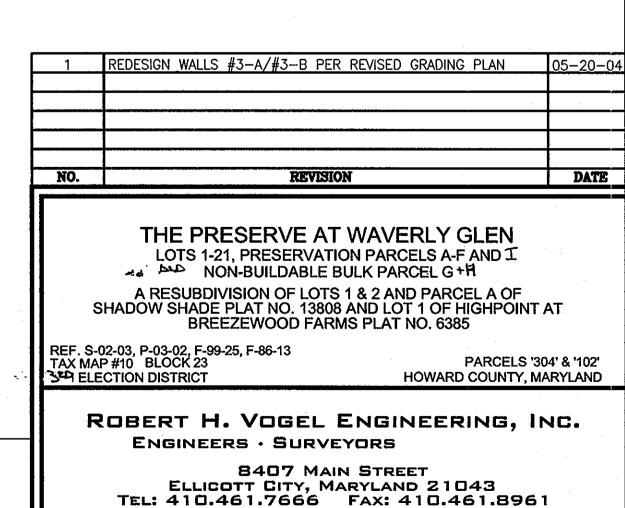
BEARING

CROSS SECTION DETAILS & FACTORS OF SAFETY:

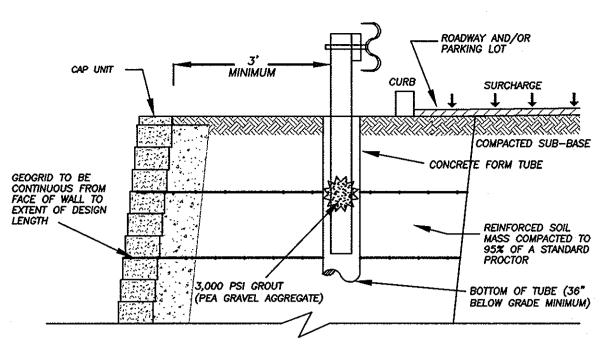
		HEIGHT		SLIDING	OVERTURNING	CAPACITY	PRESSURE	STABILITY
SECTION	STATION	# of courses	LOAD APPLIED	minimum 1.50	minimum 2.00	minimum 2.00	<u>PSF</u>	minimum 1.30
WALL #1-A			·					
1	ENTIRE WALL	10	700 PSF DEAD LOAD	4.73	20.66	3.74	1918	1.308
WALL #1-B								
1	0+00 TO 0+31	11	2:1 SLOPE & 50 PSF LL	1.97	3.89	5.29	1116	1.308
2	0+31 TO 0+98	12	2:1 SLOPE & 50 PSF LL	2.06	4.17	4.87	1161	1.308
. 3	0+98 TO 1+40	10	2:1 SLOPE & 50 PSF LL	2.07	4.34	5.90	959	1.308
WALL #2-A					•			
1	ENTIRE WALL	9	540 PSF DEAD LOAD	3.61	12.40	3.47	1630	1.319
WALL #2-B					de			
1	0+00 TO 0+48	8	2:1 SLOPE	1.60	5.01	7.40	914	1.319
2	0+48 TO 0+77	- 8	3:1 SLOPE	2.05	5.09	4.67	735	1.319
<u>WALL #3-A</u>		ye.						
1	ENTIRE WALL	9	360 PSF DEAD LOAD	4.18	15.53	3,33	1364	1.88
WALL #3-B		(
1	0+00 TO 0+69	8	2:1 BACK SLOPE	1.82	4.76	6.99	763	1.304
2	0+69 TO 0+88	9	2:1 BACK SLOPE	1.89	4.61	5.99	858	1.304
3	0+88 TO 1+61	7	2:1 BACK SLOPE	1.66	4.63	8.12	697	1.304

8/24/04 OHEF, FUREAU OF HIGHWAYS MS THESE PLANS HAVE BEEN REVIEWED FOR HOWARD SOIL CONSERVATION DISTRICT AND MEET THE TECHNICAL REQUIREMENTS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL. "I HEREBY CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 BAYS OF COMPLETION." "I/WE SERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL
BE DONE IN ACSORDANCE TO THESE PLANS, AND THAT ANY RESPONSIBLE
PERSONNEL INVOLVES IN THE CONSTRUCTION PROJECT WILL HAVE A
CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT
APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND
EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED
PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND
PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT"
PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION." USDA-NATURAL RESOURCE
CONSERVATION SERVICE THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT SIGNATURE OF ENGINEER ROBERT H. VOGEL, P.E.

RYAN & ASSOCIATES A Division of WKR Consulting, Inc. RETAINING WALL DIVISION 717-477-8400 fax 717-477-8410 68 West King Street Shippensburg, PA 17257-0006

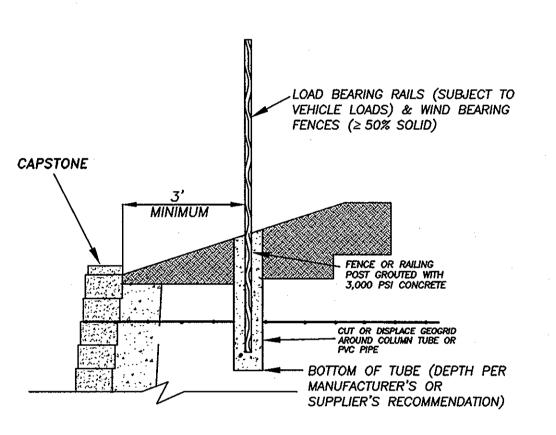


Professional Engr. No. 21586



GUARD RAIL DETAIL N.T.S.

- 1. FOR GALVANIZED POSTS GUARD RAIL CAN BE DRIVEN DIRECTLY THROUGH INSTALLED GEOGRID
- 2. IF POSTS ARE SQUARE (SUCH AS 4X4 WOOD) CONCRETE FORM TUBES MUST BE PLACED DURING WALL CONSTRUCITON AND THE POSTS GROUTED IN LATER, BLUNT POSTS MAY NOT BE DRIVEN INTO GEORID AFTER WALL CONSTRUCTION IS COMPLETE.



LOAD BEARING & WIND BEARING (INSTALLED BEHIND WALL) N.T.S.

NO.		REVISION		DATE		
No.		KEVIDION		DAIL		
THE PRESERVE AT WAVERLY GLEN LOTS 1-21, PRESERVATION PARCELS A-F AND I NON-BUILDABLE BULK PARCEL G+H A RESUBDIVISION OF LOTS 1 & 2 AND PARCEL A OF SHADOW SHADE PLAT NO. 13808 AND LOT 1 OF HIGHPOINT A BREEZEWOOD FARMS PLAT NO. 6385						
	2-03, P-03-02, F-99-2 P#10 BLOCK 23 CTION DISTRICT		PARCELS '30 WARD COUNTY, MA			
R	ROBERT H. VOGEL ENGINEERING, IN ENGINEERS · SURVEYORS 8407 MAIN STREET ELLICOTT CITY, MARYLAND 21043 TEL: 410.461.7666 FAX: 410.461.8961					
		DESIGN BY: DRAWN BY: CHECKED BY: DATE: SCALE: AS SHOWN W.O. NO.: 2017139.00	18 SHEET OF	22		

SPECIFICATIONS FOR SEGMENTAL RETAINING WALL SYSTEMS

PART 1: GENERAL

1.01 Description

A. Work includes furnishing and installing segmental retaining wall (SRW) Units to the lines and grades designated on the Final Design prepared by Ryan & Associates (RA). Also included are furnishing and installing appurtenant materials required for construction of the retaining wall as shown on the RA Final Design.

1.02 Reference Standards

A. ASTM C 140- Sampling and Testing Concrete Masonry Units

B. ASTM D 4595- Tensile Properties of Geotextiles by the Wide-Width Strip Method. C. ASTM D 5262— Test Method for Evaluating the Unconfined Creep Behavior of Geo

Single Rib Geogrid Tensile Strength Geogrid Pullout

F. ASTM D 698-Moisture Density Relationship for Soils, Standard Method G. ASTM D 422- Gradation of Soils H. ASTM 4318-Atterberg Limits of Soil

I. ASTM 3034- Specification for Polyvinyl Chloride (PVC) Plastic Pipe J. ASTM D 1248- Specification for Corrugated Plastic Pipe

A. The following factors of safety must have been met in this design: Sliding 1.5, Overturning 2.0, Bearing Capacity 2.0, Geogrid Overstress 1.5, Geogrid Connection (between the block and the geogrid) and Geogrid Pullout

PART 2: MATERIALS & DESIGN PARAMETERS

2.01 Segmental Retaining Wall Units

A. SRW Units shall be machine formed, Portland Cement concrete blocks specifically designed for retaining wall applications. The SRW Unit currently approved for this project is:

Allan Block as manufactured by Nitterhouse Masonry Products

NOTE: Where Allan Block specifications and reference documents conflict with these specifications, the RA

SRW Units shall be sound and free of cracks or other defects that would interfere with the proper placing of the units or significantly impair the strength or permanence of the structure. Cracking or excessive chipping may be grounds for rejection. Units showing cracks longer than 1/2 inch shall not be used within the wall. Units showing chips visible at a distance of 30 feet from the wall shall not be used within the wall.

C. Concrete used to manufacture SRW Units shall have a minimum 28 days compressive strength of 3,000 PSI a maximum moisture absorption rate, by weight, of 8% as determined in accordance with ASTM C 140. Compressive strength test specimens shall conform to the saw-cut coupon provisions of Section 5.2.4 of ASTM C 140 with the following exception: Coupon shall be taken from the least dimension of the unit of a size and shape

D. SRW Units molded dimensions shall not differ more than + 1/8 inch from that specified, except height which shall be + 1/16 inch as measured in accordance with ASTM C 140.

A. Geosynthetic reinforcement shall consist of geogrids as indicated on the RA Final Design. No geogrid substitutions shall be permitted without the prior approval of RA (a partial redesign may be necessary if geogrids are substituted). NOTE: it is always acceptable to substitute a higher strength geogrid (of the same manufacturer) for a lower strength geogrid.

A. The Allan Block system does not have separate connectors (clips or pins). The raised front lip is the connector and alignment guide; therefore this raised lip must be in tact and structurally sound (cannot be cracked, loose or partially missing). The raised lip also determines the wall's batter (3°, 6° or 12° setback). These walls have been

Material for the leveling pad shall consist of compacted gravel or unreinforced concrete. Typical gravels used for this leveling pad are #57, CR6, 21A, 2A modified, 2B, RC6, RC57, etc. Lean un—reinforced concrete with a strength of 1,500 PSI may also be used for the leveling pad.

2.05 Drainage Aggregate

A. Drainage aggregate shall be clean angular gravel (#57 or equivalent) with a size of 1/2 inch to 1 1/2 inches and less than 10% fines (passing the #200 sieve). Rounded "pea gravel" type aggregate is not permissible since it does not have the necessary frictional properties. Recycled gravel may be used if it meets the above criterion.

2.06 Drainage Pipe A. The drainage collection pipe shall be a 4 inch perforated or slotted PVC or corrugated HDPE pipe.

2.07 Infill Soil: within the reinforced geogrid zone

A. The soils used must meet or exceed the friction angle stated in the RA Final Design (in the General Notes, on the typical wall section and on the structural cross sections). The reinforced material shall be free of debris and organic material (i.e.— no plants, roots, sod, top soil, trash, wood, etc.). The infill soil shall not consist of CH (fat clay). MH (elastic silt) or OH/ OL/ PT (organic) soils.

B. Rocks may be used as infill material as long as they have a maximum size of 6 inches and a mean diameter of 3 inches. Recycled concrete is permissible for infill except with certain polyester geogrids in water applications. In the case of water applications the geogrid manufacturer shall be consulted to see if the alkali in the recycled material will cause corrosive damage to their geogrid.

C. Select gravel (classified by USCS as GP or GW) is always an acceptable substitution in the event suitable soils (those meeting RA's and the site geotechnical engineer's requirements) are not readily available.

2.08 Retained Soil: the area beyond the infill soil and extending to a distance that is twice the wall's

A. This soil must meet or exceed the friction angle stated in the RA Final Design (in the General Notes, on the typical wall section and on the structural cross sections). This soil must be virgin (natural undisturbed with blow counts (≥12) or suitable fill (friction angle ≥ the RA Final Design requirement) compacted to 95% of a standard proctor maximum dry density.

2.09 Foundation Soil: the soil under the wall's gravel leveling pad and the soil under the reinforced geogrid

A. The foundation soil must meet or exceed the minimum allowable bearing capacity stated in the RA Final Design (in the General Notes and on the typical wall section). The sub-arade must be virain (natural undisturbed with blow counts ≥12) or suitable fill (friction angle ≥ the RA Final Design requirement) compacted to 95% of a standard proctor maximum dry density.

A. RA recommends that every retaining wall design be preceded by an in-situ soil investigation by a licensed geotechnical engineer. However, if the owner and/or wall installer elects not to have an investigation conducted RA may assume soil design parameters based on: published data by the Soil Conservation Service (soil maps), a verbal description by the owner and/or wall installer or by RA's previous experience in certain geographic areas. It must be understood that the owner and/or wall installer bears full responsibility to the election not to have

2.11 Site History & Information

A. Many factors other than soil information affect the performance and design of the retaining wall. RA relies on information provided by the owner and/or wall installer when designing a retaining wall. RA bears no responsibility if the owner and/or wall installer omit critical information required to properly design the wall. Information critical to wall design from the site consist of: topographic features (such as slopes), soil types, utilities, storm water management, structures (including buildings, other existing or proposed walls, swimming pools, etc.), site geological phenomenon, groundwater, loads within the wall's zone of influence (such as driveways, patios, roadways, sidewalks, etc.) and any other readily known site factors that could potentially impact the RA Final Design.

PART 3: CONSTRUCTION

A. RA considers all retaining walls to be critical structures, meaning most walls require a considerable financial investment by the owner and failure of a wall will negatively impact a property both financially and from a public safety perspective. The owner or owner's representative is responsible for verifying that the wall installer meets all of the requirements of the RA Final Design (as stated in these specifications and the project's General Notes). This includes all submittals for materials and design, qualifications and proper installation of the wall system. All walls with an exposed height of 6 feet or greater must have the construction certified by a licensed geotechnical/ structural engineer registered in the jurisdiction of the project. Additionally, after the wall has been completed it is highly recommended that it be surveyed to establish the wall's current horizontal and

B. The wall installer's field construction supervisor shall have demonstrated experience and be qualified to direct

. RA provides construction oversight on some retaining wall projects. RA verifies general conformance with the RA Final Design; however, it is the wall installer's ultimate responsibility to construct the structure properly in accordance with the RA Final Design. RA's liablility is limited to the amount of our fees for the scope of work provided for the wall designs and construction oversight.

A. The wall installer shall excavate to the lines and grades shown on the RA Final Design and the project's civil plans. The wall installer shall take precautions to minimize over—excavation. Over—excavation shall be filled with compacted soil (friction angle ≥RA design parameters) or gravel as directed by the site geotechnical engineer.

B. The wall installer shall verify the location of existing structures and utilities prior to excavation. The wall and installer shall ensure that all surrounding structures are protected from the effects of wall excavation. Excavation support (shoring), if required, is the responsibility of the wall installer. All excavation must be conducted in accordance with OSHA (federal) and state safety regulations. All work to construct the wall must be in accordance with 29CFR1??926 sub-part P (OSHA Excavation Safety Requirements).

A. Following excavation, the foundation soils (the soilsunder the wall's gravel leveling pad and the soils under th wall's reinforced geogrid zone) shall be examined by the site geotechnical engineer to assure that the actual foundation soil strength meets or exceeds the minimum allowable bearing capacity in the RA Final Design (stated in the General Notes and shown on the typical wall section). Soils that do not meet the required strength shall be removed and replaced with approved select structural fill or gravel and be compacted to 95% of a standard proctor maximum dry density for the full depth.

B. In cases of poor bearing capacity or fill soils, an enlarged geogrid reinforced leveling pad may be required. This typically consists of a 1 foot deep X 4 foot wide leveling pad with geogrid under (on the sub-grade) and within the gravel (6 inches above the sub-grade). The sub-grade must be compacted with a "J-Tamp" or "Jumping Jack" type compactor with a minimum of three passes prior to geogrid placement. These extra measures will increase the soil's bearing capacity by a minimum of 1,000 PSF (RA shall be consulted if the soil's bearing capacity needs to be increased by more than 1,000 PSF).

A. The leveling pad shall be placed so that its top elevation is the same as the bottom of block ("BB") elevation on the RA Final Design profile drawing. It shall have a minimum thickness of 6 inches and a minimum width of 2 feet. The leveling pad should, at a minimum, extend laterally at least a distance of 6

B. The leveling pad material shall be compacted to 95% of a standard proctor maximum dry density with a vibratory plate compactor to provide a firm level—bearing surface on which to place the first course of SRW Units. A thin layer (not to exceed 1/2 inch) of well-graded sand or stone dust may be used to smooth the

3.05 SRW Unit Installation

A. All SRW Units shall be installed at the proper elevation and orientation as shown on the RA Final Design profile drawing and in conjunction with the project's civil plans. The SRW Units shall be installed in general accordance with the manufacturer's recommendations (RA's Final Design shall govern in any conflict between the

B. The first course of SRW Units shall be placed on the leveling pad. The units shall be leveled side-to-side, ront—to—rear and with adjacent units, and aligned to ensure intimate contact with the leveling pad. The first course is the most important for accurate and acceptable results. Alianment may be done by means of a string line or an offset from the base line to the backs of the blocks. SRW units shall have a minimum 4 inch overlap of units on each successive course so that the wall is interlocked and continuous. No horizontal gaps greater than 1/4 inch between the faces of adjacent units are permitted.

C. Because the wall has a setback, its batter must be predetermined during the stake out process by the civil engineer/surveyor and wall installer. If there are critical dimensions that must be met on the high side of the wall then the base (at the toe) will need to be moved forward to compensate.

D. Lay out of curves and corners shall be installed in accordance with the civil plans and the RA Final Design. Construction techniques for curves and corners shall be in general accordance with the SRW manufacturer's installation guidelines. In general, all tangent angles shown on the civil drawings should be changed into curves to enhance the wall's strength and appearance. Continuous vertical joints are not recommended. Inside and outside 90° corners may be constructed without compromising the wall's integrity if they are properly interlocked. Inside corners should be constructed so that the SRW Units interlock (according to manufacturer's recommendations) and outside corners should incorporate special corner blocks when possible. If special outside corner blocks are not available from the block manufacturer for this project then the manufacturer's quidelines for building structural outside corners shall be followed. If gluing is necessary only industrial grade adhesives or sealants designed for concrete-to-concrete applications may be used (adhesives designed for plastic or wood applications are not acceptable).

SIGNATURE OF DEVELOPER

E. Clean all excess debris from the tops of the SRW Units and install the next course.

F. Repeat procedures to extent of wall height

G. A ±2" tolerance is permitted horizontally for wall batter (block setback). In no case shall a wall go beyond vertical (have a negative batter). Walls shall be built level (not with grade), however a ±1.5 inch tolerance over a 10 foot distance is permitted vertically (as checked from left to right along the wall).

H. Embedment shall be a minimum of 1 inch buried for every 1 foot of wall exposed with one block minimum when the front slope is 4:1 or greater (more level). Walls constructed on 3:1 front slopes or less (more steep) require additional buried blocks. See the profile drawing in the RA Final Design for the exact amount of embedment (the amount of buried block can be determined at each wall station by subtracting the "BB" elevations from the "GR" elevations).

3.06 Geogrid Reinforcement Placement

A. All geogrid reinforcement shall be installed at the proper elevation, length and strength as shown on the profile and structural cross sections in the RA Final Design. Partial geogrid coverage is not acceptable: no gaps shall be present between geogrid layers. 100% coverage is required, however it is not necessary to overlap the geogrid pieces. The geogrid shall be laid horizontally on the compacted infill soil and on top of the concrete SRW Units. The geogrid must be embedded into the SRW Units to the face. The wall installer shall verify that the orientation of the geogrid is in accordance with the geogrid manufacturer's recommendations. The highest strength direction of the geogrid must be perpendicular to the wall face (the geogrid must not be laid parallel to the wall— cannot be rolled out with the wall).

B. Geogrid reinforcement layers shall be one continuous piece for their entire embedment length. Overlapping of the geogrid in the design strength direction (perpendicular to the wall face) is not permitted.

C. Tracked construction equipment shall not be operated directly on the geogrid. A minimum of 6 inches of backfill is required prior to operation of tracked vehicles over the geogrid. Turning should be kept to a minimum. Rubber-tired equipment may pass over the geogrid reinforcement at slow speeds (less than 5 MPH). C. Concrete storm structures may be located behind a wall and be within the reinforced geogrid

D. The geogrid shall be in tension and free of wrinkles prior to placement of the infill soil. Nominal tension shall be applied to the geogrid and secured in place with staples, stakes or by hand until it is covered by 6

E. For inside & outside corners and inside & outside curves the geogrid shall be placed according to the manufacturer's instructions to provide total geogrid coverage. On outside corners the geogrid should be shifted up or down one course and alternated so that the geogrid comes into the reinforced geogrid zone from both legs of the 90° angle. Geograf layers should never be placed on top of one another: there must be a minimum of 3 inches of compacted infill soil between geogrid layers.

A. Drainage aggregate (clean gravel such as #57 or equivalent) shall be installed behind the entire wall face from the first course below grade to one course from the top of the wall. The drainage gravel shall be placed to a minimum thickness of 12 inches behind the SRW Units. Drainage gravel shall also fill all voids between and within (if hollow) the SRW Units. SRW Units must be filled with drainage aggregate in one course lifts (SRW Units may not be stacked in two or three course lifts and then have the gravel dumped in from the top through multiple courses). An impermeable clay layer (CL, GC or SC) shall be placed on top of the 12" drainage layer. If clayey soils are not readily available, a layer of filter fabric (Mirafi 140N or equivalent) shall be placed on top of the gravel (below the topsoil) to prevent the downward migration of fines.

B. Drainpipes are mandatory and shall be vented to daylight at the end(s) of the wall, at a central low point of the wall, or through the wall face at maximum intervals of 30 feet on center (no more than 6 inches above finished grade when vented through the wall face). The pipe(s) must maintain gravity flow of water outside the reinforced geogrid zone. Water must drain to an outlet and have positive flow. If a continuous pipe is run, it shall daylight into a storm sewer manhole or along a slope at an elevation lower than the lowest point of the pipe within the drainage aggregate. When drainpipes are daylighted at the end(s) of a wall they must be visible and unobstructed. The drainpipes should be checked by the owner on a regular basis to ensure that they remain open (not blocked, filled in, grown over, pinched)

C. Rear drainpipes are required in the following situations: when groundwater can rise and approach within 1 foot of the leveling pad sub-grade, in "cut" situations where the potential exists for storm water to enter the interface between the reinforced geogrid zone and the retained zone and when low permeable soils (CL- lean clay & ML- silt) are used for infill soil. Retaining walls with low permeable soils in the reinforced geogrid zone are more susceptible to being negatively impacted by hydrostatic forces. The owner may elect to install a rear drainpipe to minimize or eliminate potential hydrostatic force buildup leading to potential wall movements RA recommends a rear drainage system for all walls with these soil types in cut situations, however ultimately it is the owner's decision. This rear drainpipe shall be surrounded by a minimum of 12 inches of clean gravel (#57 or equivalent) and surrounded with filter fabric to prevent the migration of fines. This rear drainpipe must vent to daylight or be directed to a storm sewer manhole (see instructions for front drainpipe in section

D. Chimney drains (a second 12 inch layer of drainage aggregate within the rear 1 foot of the reinforced geogrid zone or directly behind the reinforced geogrid zone) must be installed when groundwater is present or likely (to an elevation that is a minimum of 1 foot above predicted levels as given by the site geotechnical engineer), when stated in the RA Final Design or when required by the site geotechnical engineer.

E. All drainage zone aggregate shall be compacted to 95% of a standard proctor maximum dry density with a vibratory plate compactor (minimum of three passes).

3.08 Backfill Placement

A. The infill soil shall be placed as shown in the RA Final Design in the maximum compacted lift thickness of 10 inches and shall be compacted to a minimum of 95% of a standard proctor. maximum dry density (ASTM D 698) at a moisture content within 2% of optimum. The backfill shall be placed and spread in such a manner as to eliminate wrinkles or movement of the geogrid and the SRW units. Compaction testing shall be done at 25%, 50%, 75% and 100% of the wall height or as specified by the site geotechnical engineer.

B. Only a vibratory plate or small—scale vibratory smooth drum compactor equipment shall be allowed within 3 feet of the front of the wall face. Compaction within the 3 feet behind the wall face shall be achieved by at least three (3) passes of the lightweight mechanical plate compactor or roller. Heavy equipment (such as track hoes, ride on rollers, pans, etc.) must be kept back a minimum of 3 feet from the rear of the wall.

C. At the end of each day's operation, the wall installer shall slope the last level of backfill away from the wall facing to direct water runoff away from the wall face.

D. At completion of wall construction if final grading, paving, landscaping and/or storm drainage installation adjacent to the wall is not placed immediately after wall completion, temporary grading shall be provided to ensure that water runoff is not allowed to collect or pond behind the wall until final construction adjacent to the wall is completed.

E. Filter fabric (Mirafi 140N or equivalent) is required when the infill soil is classified as poorly graded sand (SP) or well graded sand (SW) since these soils are non-cohesive and could potentially slough, clogging the gravel drainage layer. Filter fabric is optional between the 12 inch gravel drainage layer and the compacted infill soil if the backfill soils are clayey (CL or SC), gravelly (GC, GM, GP or GW) or silty (ML or SM).

3.09 SRW Caps

A. SRW caps shall be properly aligned and glued (for safety reasons) to the underlying SRW Units with a flexible high-strength concrete adhesive or sealant designed for "concrete to concrete" applications (not for plastic or wood). Rigid adhesive or mortar is not acceptable.

A. When walls are installed in water applications (such as storm water ponds, streams, bulkheads, greas adjacent to flood plains, etc.) all clean gravel must be used as infill up to 1 foot above the 100 year flood elevation, the high water level or the top of berm/spillway. This gravel must be free draining and have less than 10% fines (#57 or equivalent). Filter fabric (Mirafi 140N or equivalent) must go in front of the buried block, under the leveling pad, behind the reinforced geogrid zone (vertically up to the extent of the gravel infill) and on top of the gravel infill horizontally). This is required to prevent the migration of fines into the gravel infill. Rip rap is required in front of the bottom three courses on walls installed in tidal waters. Rip rap is also required when indicated on the civil plans and where pipes with active water flow exit through the

3.11 Rails, Fences & Other Structures

SIGNATURE OF ENGINEER ROBERT H. VOGEL, P.E.

A. The scope of RA for this project does not include fence or railing designs. Typical details have been given to provide general guidelines for the installation of fences, guardrails and railings behind walls. RA cannot give specific details because the type, placement and height of fences and rails vary widely and because the requirements are different depending on the municipality and regulatory authority. RA can provide a project specific fence or rail detail and structural design for an additional fee if given exact information (material type and size and manufacturer's specifications and installation auidelines).

B. Open fences and railings not subject to wind loads (minimum of 50% open and maximum of 50 % solid) may be placed directly behind the wall or in the wall (can be placed in the blocks only if they are a hollow system and if the cores and web alignment will accept the posts) as long as they are not subject to vehicular impact. Solid or semi-solid fences that are subject to wind loads must be kept back a minimum of 3 feet from the rear of the wall to prevent loading of the wall. C. Guardrails subject to vehicular impact must be kept back a minimum of 3 feet from the rear o the wall to prevent loading of the wall. Guardrails may be placed closer than this 3 foot minimum only if a barrier (such as curbing, wheel stops, etc.) is in place to prevent vehicular impact (the overhang of vehicles must be considered when determining this).

D. Light post foundations, sign foundations and similar structures subject to wind loads must be kept back a minimum of 3 feet from the rear of the wall to prevent loading of the wall.

E. In cases where these 3 foot minimum distances cannot be met due to restraints on the site, additional analyses will need to be done to determine methods of stabilization. RA can provide these designs for an additional fee.

3.12 Storm Structures & Utilities

A. Reinforced Concrete Pipes (RCP) may pass through the leveling pad or wall structure without compromising the design. The SRW units may be cut to fit around the pipe and the voids filled with non-shrink grout or type "M" mortar. A concrete collar may be cast around the structure if desired. When a collar is cast, the top of the collar shall line up with an even block course to maintain proper alignment and neat workmanship.

B. The wall may not bear on plastic or steel pipes (such as ADS, CMP, HDPE, PVC, SLCPP, etc.) or utilities (such as electric, gas, phone lines, sewer or water lines, etc.). Grade beams or lintels must be used to bridge these non-load bearing structures. If a specific grade beam or lintel is not specified in the RA Final Design, RA shall be consulted to determine the size, strength and reinforcing of the grade beam or lintel. If these non-load bearing pipes or utilities are located at minimum of 42 inches below the wall's leveling pad then a grade beam or lintel is not necessary.

zone as dictated by the project's civil drawings. If the structure(s) cannot be moved out of the reinforced geogrid zone and the geogrid cannot be installed to its full design length the following shall apply. On small structures (such as collection boxes, concrete pipes less than 18 inches, inlets, manholes, etc.) it is acceptable to shorten the geogrid from the design length and meet the structure. The area between the wall and structure where the geogrid has been shortened must be filled with gravel (#57 or equivalent) and not soil. The gravel must be compacted to 95% of a standard proctor maximum dry density with a vibratory plate compactor. On large structures and in cases where pipes parallel the wall for long distances, RA shall be consulted to determine the

D. The wall's integrity may be compromised if pipes or structures burst or develop leaks and allow water or fluids to saturate the reinforced geogrid zone. RA is not responsible for wall failure that results from pipes or structures that burst or leak and allow water or fluids to saturate the

3.13 Construction Adjacent to Completed Wall

The owner or owner's representative is responsible for ensuring that construction adjacent to the wall by others does not disturb the wall or place temporary construction loads on the wall that exceed design loads, including loads such as water pressure, temporary grades, or equipment loading. Heavy paving or grading equipment shall be kept a minimum of three feet behind the back of the wall face. Equipment with wheel loads in excess of 150 PSF live load shall not be operated with 10 feet of the face of the retaining wall during construction adjacent to the wall. Care should be taken by the general contractor or owner to ensure water runoff is directed away from the wall structure until final grading and surface drainage collection systems are completed

B. Care must be taken when installing appurtenances (such as generators, transformers, etc.) or utilities within the reinforced geogrid zone of the wall. The compaction integrity of the reinforced geogrid zone must be maintained, both below and beside (around) the appurtenance or utility. Neglecting to do so may cause hydrostatic pressure and wall failure.

A. The segmental retaining wall is not a storm water management structure. The wall can accommodate the rainfall above the reinforced geogrid zone but not the watershed (including the retained zone). Therefore it is absolutely essential that surface water be prevented from entering (and ultimately saturating) the reinforced geogrid zone. This is usually accomplished by the site engineer (owner's civil engineer) grading the surface behind the wall to direct surface water to swales that divert the water around the wall ends, to inlets or over the top of the wall through scuppers. If water is directed to the wall (such as applications with back slopes), the top 8 inches of compacted fill over the reinforced geogrid zone must have impermeable soil (such as CL, GC or SC). If clayey soils are not readily available an underlying geomembrane (geosynthetic liner) may also be used. This geomembrane shall be Mirafi G200N, Stratadrain or equivalent. It shall extend downward vertically a minimum of 3 feet behind the reinforced geogrid zone, be laid horizontally on top of the reinforced geogrid zone with a maximum slope of 10:1 and extend forward into the 12 inch gravel drainage layer.

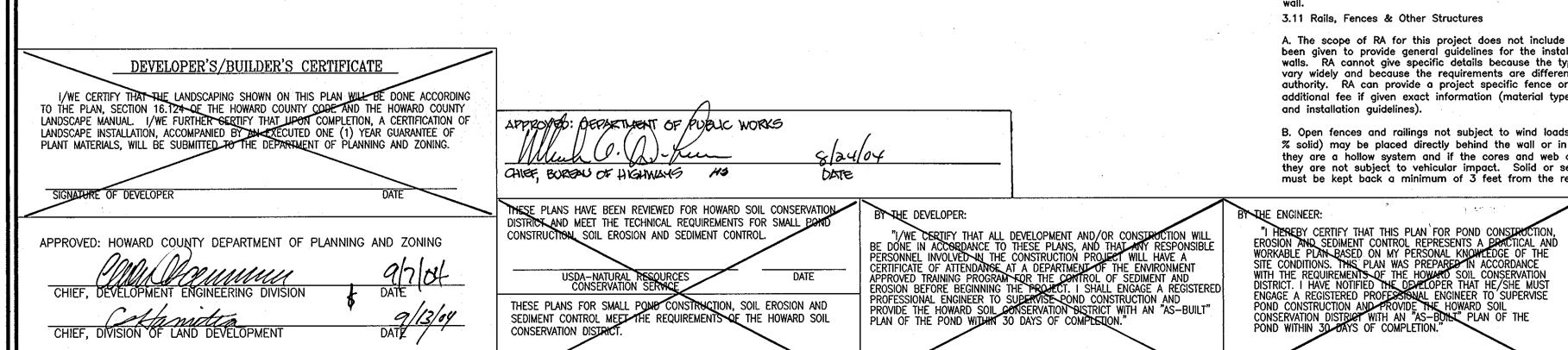
Post Construction Responsibilities

A. Retaining walls are a substantial financial investment. Therefore it is in the owner's best interest that a wall maintenance budget be established within the overall property management budget to monitor and provide preventative maintenance. Retaining wall maintenance, at a minimum, should consist of: checking drainage, inspecting for settling and surveying to verify alignment and batter. This service should be by qualified personnel under the supervision of a licensed geotechnical/structural engineer. RA can provide this service for an additional fee.

B. RA SHOULD BE NOTIFIED AS SOON AS REASONABLY POSSIBLE IF THE RETAINING WALL EXHIBITS CONDITIONS CONTRARY TO THE RA FINAL DESIGN SO THAT RA MAY BE CONSULTED TO PROTECT THE OWNER'S INVESTMENT.

END OF SECTION Revised 09-19-03

The information contained herein is proprietary and is the sole property of RA. It is only intended for use on this project. Reuse of these drawings, sketches, and design computations in any manner is strictly prohibited without written approval from RA. Any other use is subject to penalty



HOWARD S.C.D.

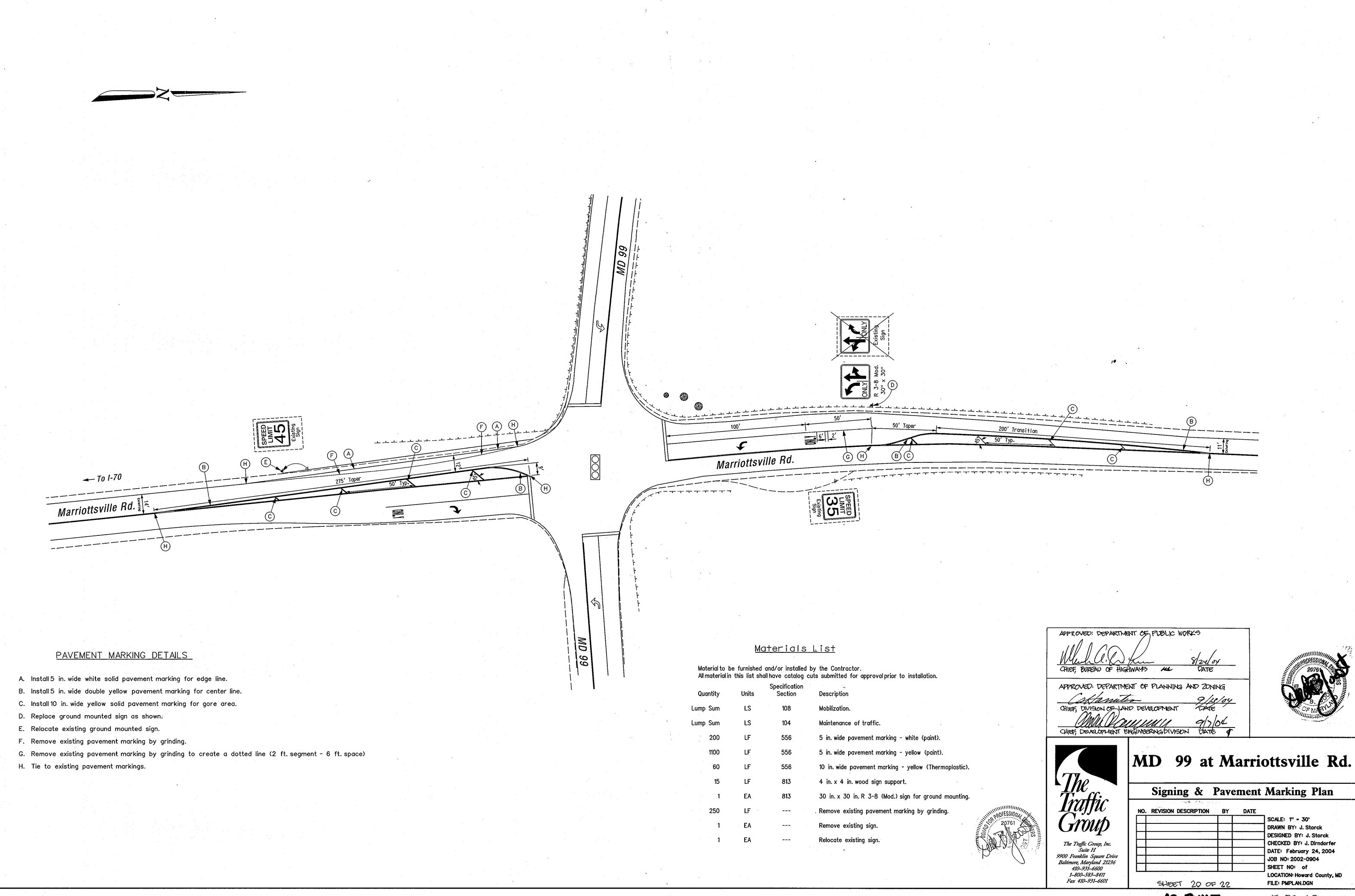
RYAN & ASSOCIATES A Division of WKR Consulting, Inc. RETAINING WALL DIVISION 717-477-8400 fax 717-477-8410 68 West King Street Shippensburg, PA 17257-0006



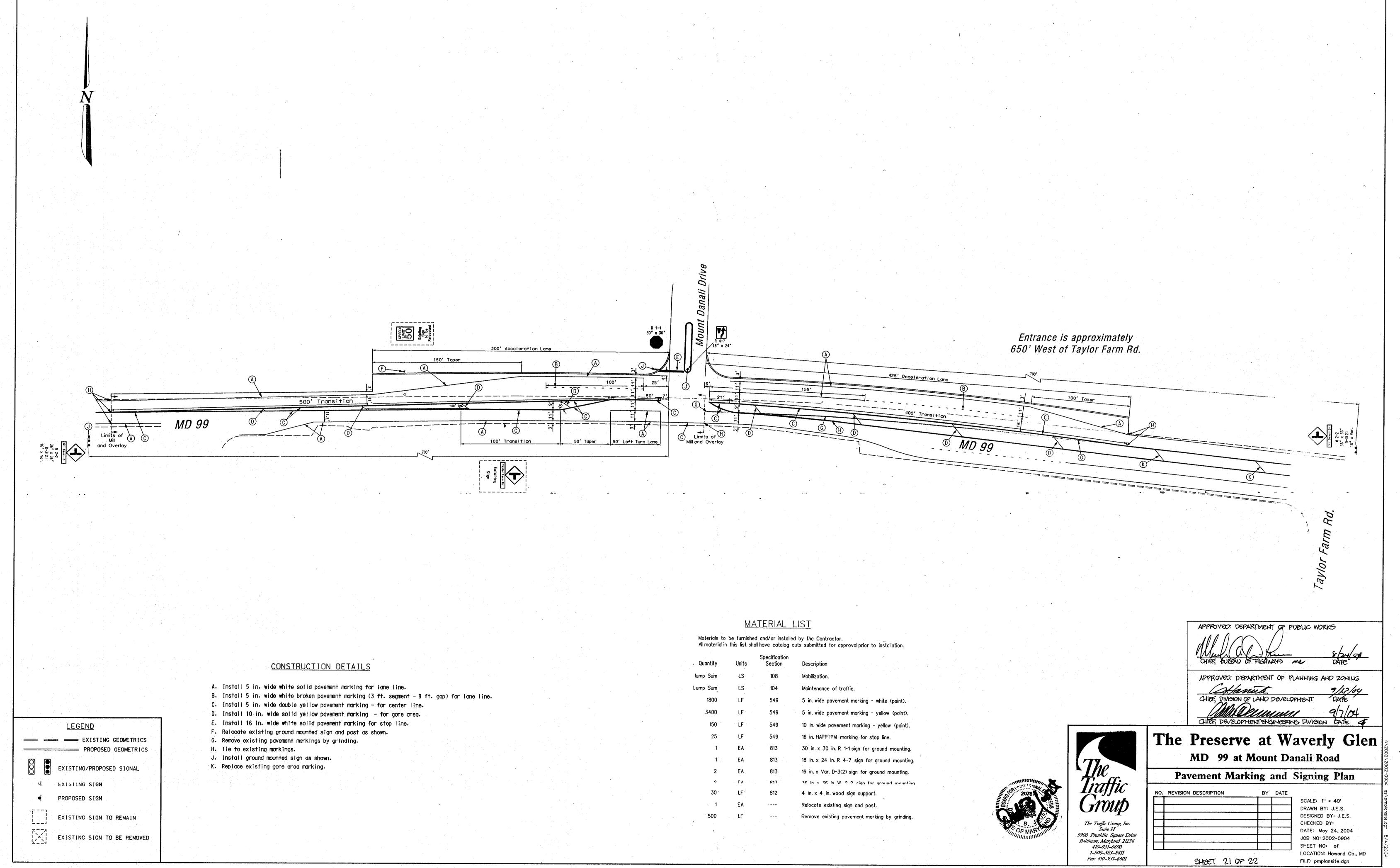
DATE THE PRESERVE AT WAVERLY GLEN LOTS 1-21, PRESERVATION PARCELS A-F AND ± NON-BUILDABLE BULK PARCEL G+H A RESUBDIVISION OF LOTS 1 & 2 AND PARCEL A OF SHADOW SHADE PLAT NO. 13808 AND LOT 1 OF HIGHPOINT AT **BREEZEWOOD FARMS PLAT NO. 6385** REF. S-02-03, P-03-02, F-99-25, F-86-13 TAX MAP #10 BLOCK 23 PARCELS '304' & '102' HOWARD COUNTY MARYLAND ELECTION DISTRICT ROBERT H. VOGEL ENGINEERING, INC. ENGINEERS · SURVEYORS 8407 MAIN STREET ELLICOTT CITY, MARYLAND 21043 TEL: 410.461.7666 FAX: 410.461.8961 DESIGN BY: DRAWN BY: CHECKED BY AS SHOWN SCALE: 2017139.00 W.O. NO.: SHEET __ OF _

REDESIGN WALLS #3-A/#3-B PER REVISED GRADING PLAN

05-20-04



AS-BUILT



AS-BUILT

F-03-193

