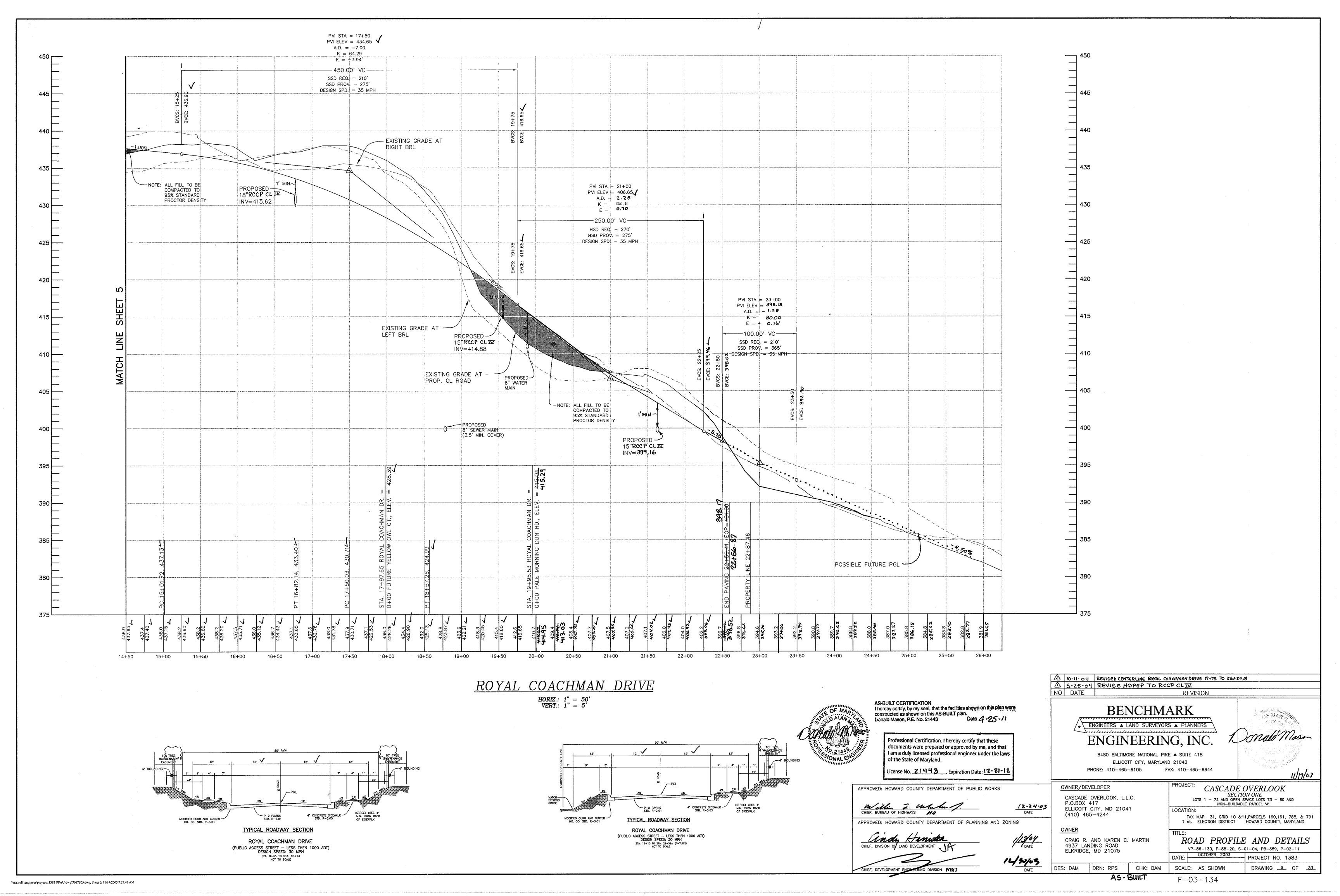
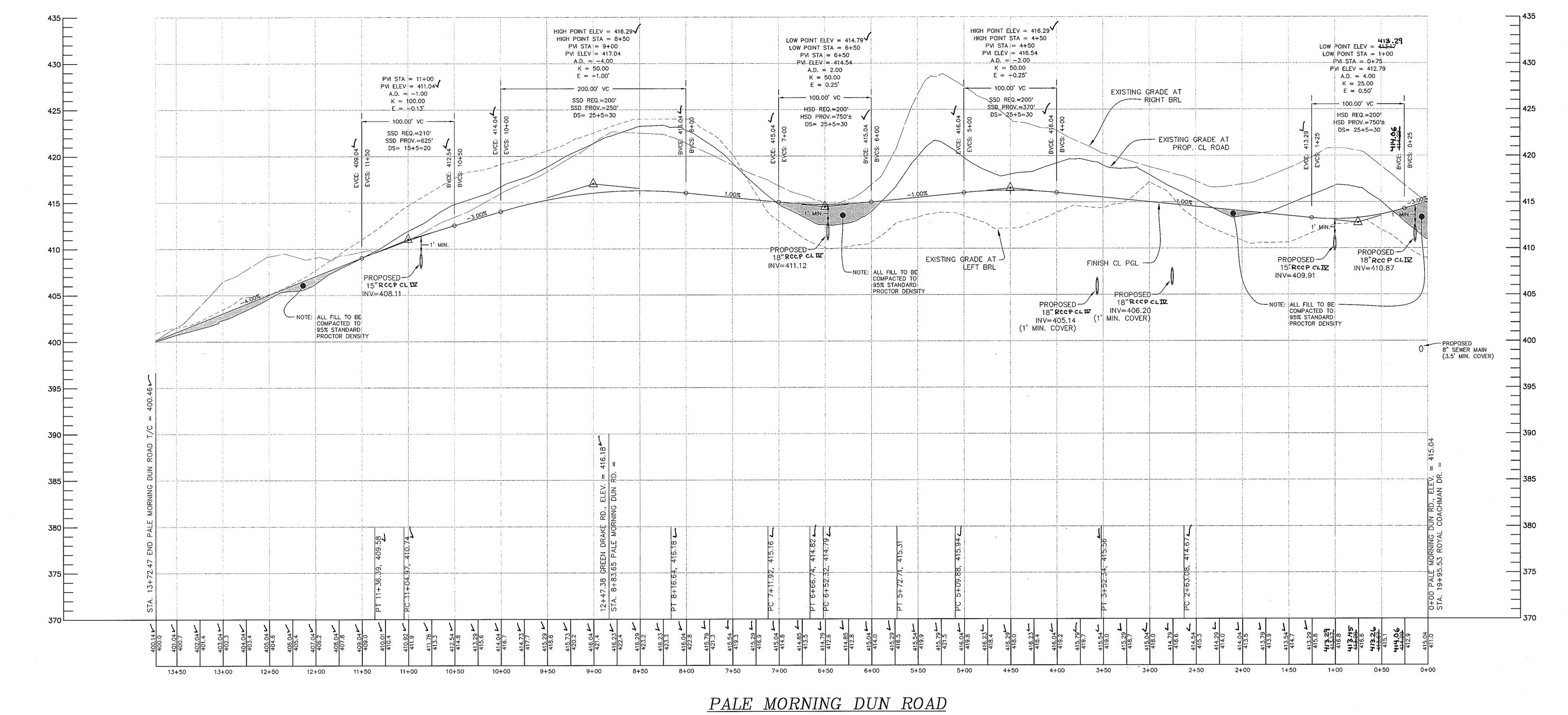
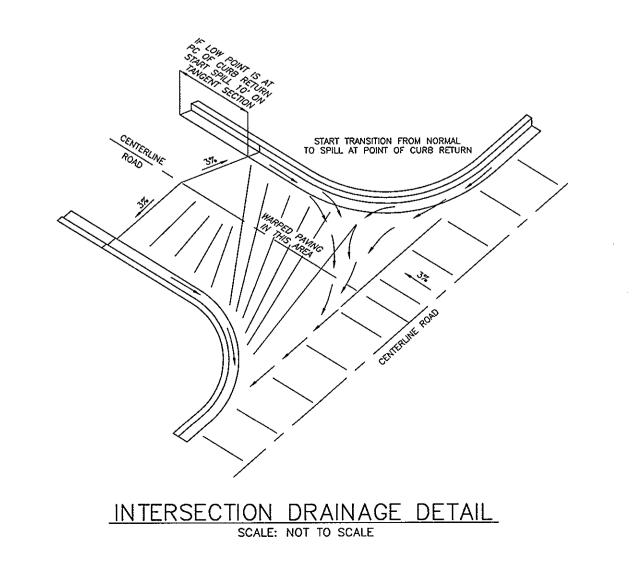


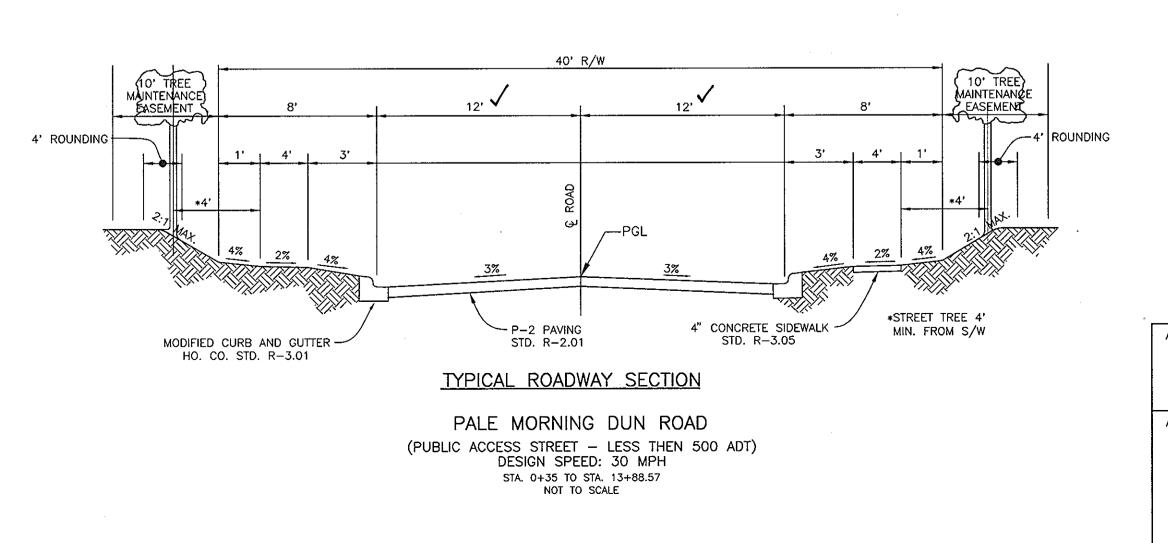
\(\)\tsa\\vo11\engineer\projects\(\)\1383 PFAU\dwg\\7017S05.dwg, Sheet 5, \(\)\(11/14/2003\) 9.00.05 AM

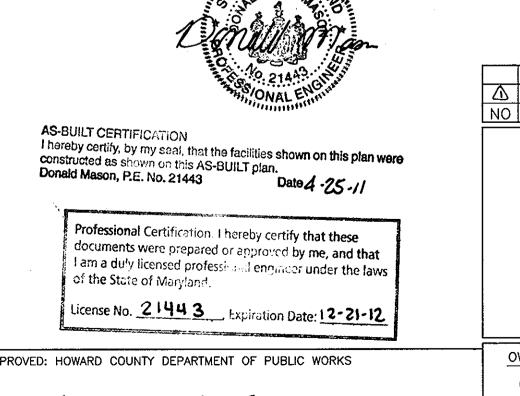


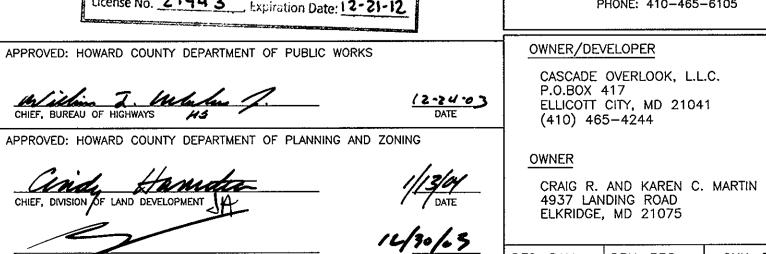


HORIZ.: 1" = 50' VERT.: 1" = 5'

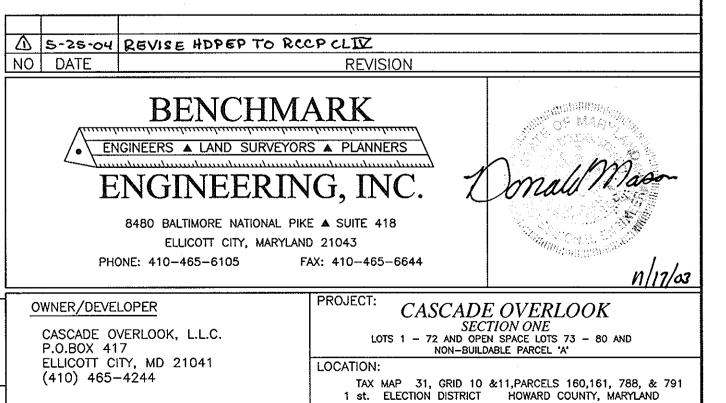








MENT ENGINEERING DIVISION MAJ



ROAD PROFILE AND DETAILS VP-86-130, F-88-20, S-01-04, PB-359, P-02-11 DATE: OCTOBER, 2003 PROJECT NO. 1383 SCALE: AS SHOWN DRAWING _7_ OF _33_

\tsa\vol1\engineer\projects\1383 PFAU\dwg\7017S05.dwg, Sheet 7, 11/14/2003 9:01:35 AM

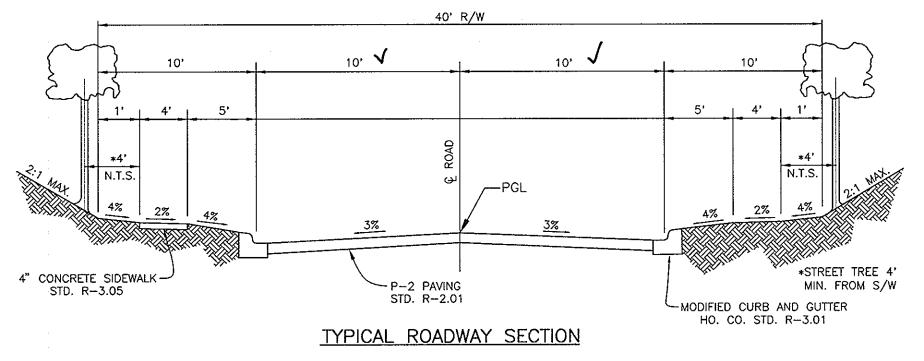
AS-BUILT

CHK: DAM

DRN: RPS

F-03-134

HIGH POINT ELEV = 440.81 HIGH POINT STA = 7+69.46PVI STA = 8+44.46PVI STA = 6+00PVI ELEV = 441.68PVI ELEV = 439.24A.D. = -8.00A.D. = -6.50K = 25.00K = 30.77E = -2.00'E = -1.62-200.00' VC----200.00' VC-SSD REQ. = 110' SSD REQ. = 110' SSD PROV. = 180° SSD PROV. = 200' DESIGN SPD. = 20 MPH DESIGN SPD. = 20 MPH 1.00% . . . LOW POINT ELEV = 414.69 LOW POINT STA = 1+70.45HSD REQ. = 160'PVI STA = 2+25HSD PROV. = 220' PVI ELEV = 411.12DESIGN SPD. = 25 MPH A.D. = 11.00 K = 27.27PVI STA = 11+85.38 - Possible future PGL - E = 4.13' PVI ELEV = 417.82 A.D. = 3.00+300.00, ∧C++ K = 16.67HSD REQ. = 160'E = 0.19HSD PROV. = 160' 50.00' VC DESIGN SPD. = 25 MPH EXISTING GRADE AT LEFT BRL FINISH CL PGL - NOTE: ALL FILL TO BE COMPACTED TO 95% STANDARD - EXISTING GRADE AT FINISH CL PGL -PROCTOR DENSITY PROP. CL ROAD 18"RCCP CLIV INV=411.79 - EXISTING GRADE AT (1' MIN. COVER) RIGHT BRL PROPOSED PROPOSED -8" SEWER MAIN 15"RCCP CLIV (3.5' MIN. COVER) INV=411.58 5+50 11+00 3+50 4+00 5+00 GREEN DRAKE ROAD WITH POSSIBLE FUTURE CONNECTION 40' R/W ∆ 5-25-04 REVISE HDPEP TO RCCP CLIV
NO DATE REV



GREEN DRAKE ROAD (PUBLIC ACCESS STREET - LESS THEN 200 ADT)
DESIGN SPEED: 20 MPH STA. 0+35 TO STA. 1+38.50 (T-TURN) STA. 11+28.20 TO 12+20.49 (T-TURN) NOT TO SCALE

\\tsa\vol1\engineer\projects\1383 PFAU\dwg\7017S05.dwg, Sheet 8, 11/14/2003 9:02:18 AM



AS-BUILT CERTIFICATION
I hereby certify, by my seal, that the facilities shown on this plan were constructed as shown on this AS-BUILT plan.
Denald Mason, P.E. No. 21443
Date 4-25-//

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 21443 Expiration Date: 12-21-12

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS 12-24-03 APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

BENCHMARK ENGINEERS A LAND SURVEYORS A PLANNERS ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE ▲ SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105

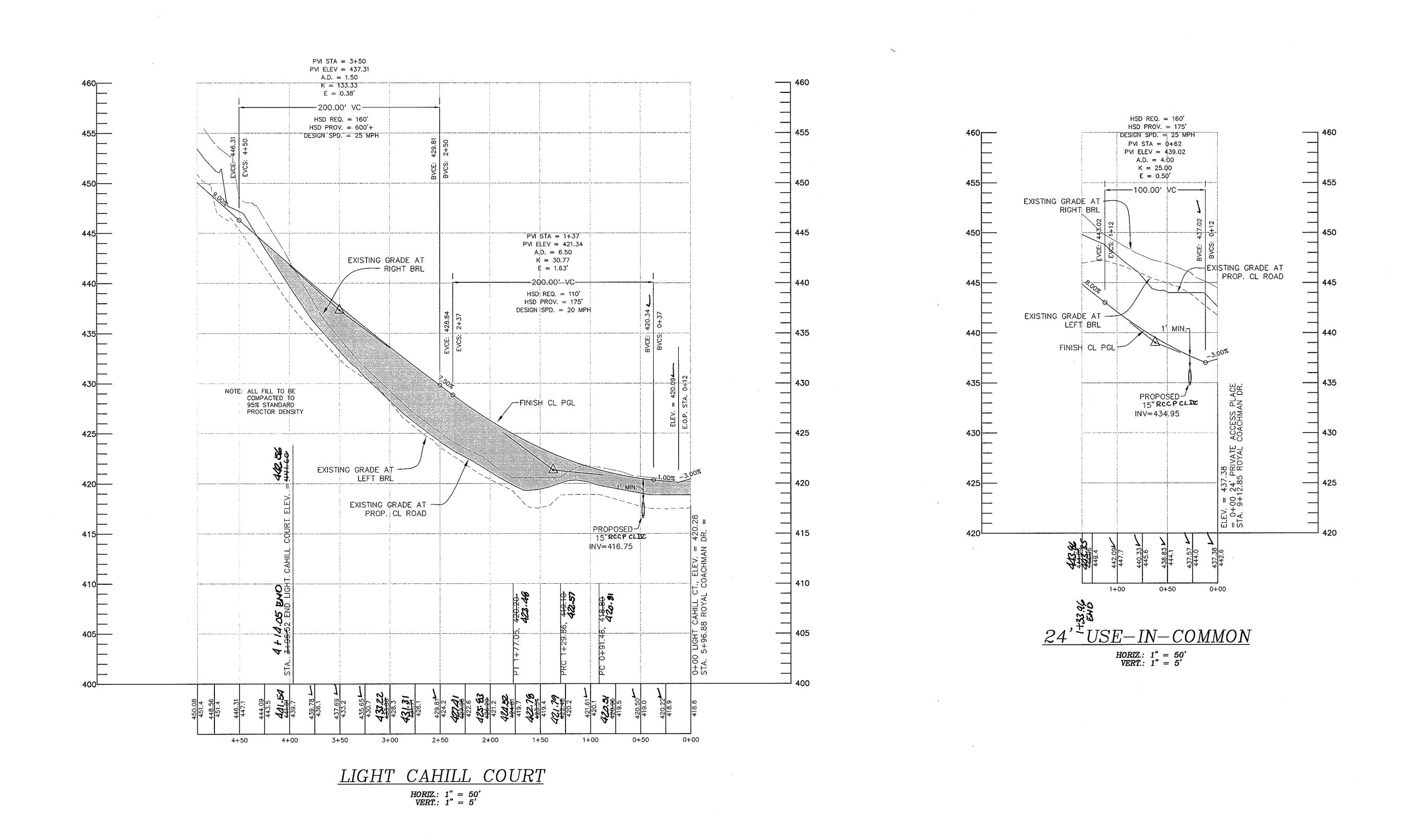
CT: CASCADE OVERLOOK

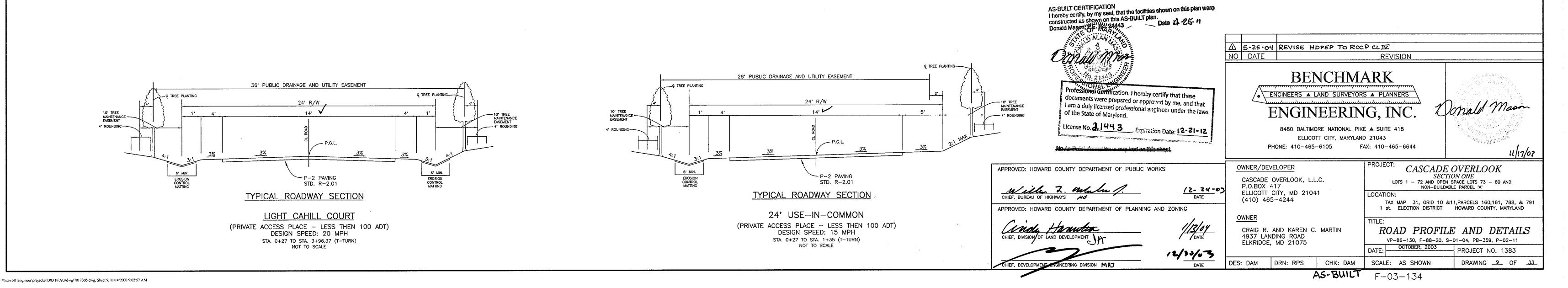
SECTION ONE
LOTS 1 - 72 AND OPEN SPACE LOTS 73 - 80 AND
NON-BUILDABLE PARCEL 'A' OWNER/DEVELOPER CASCADE OVERLOOK, L.L.C. P.O.BOX 417 ELLICOTT CITY, MD 21041 (410) 465-4244

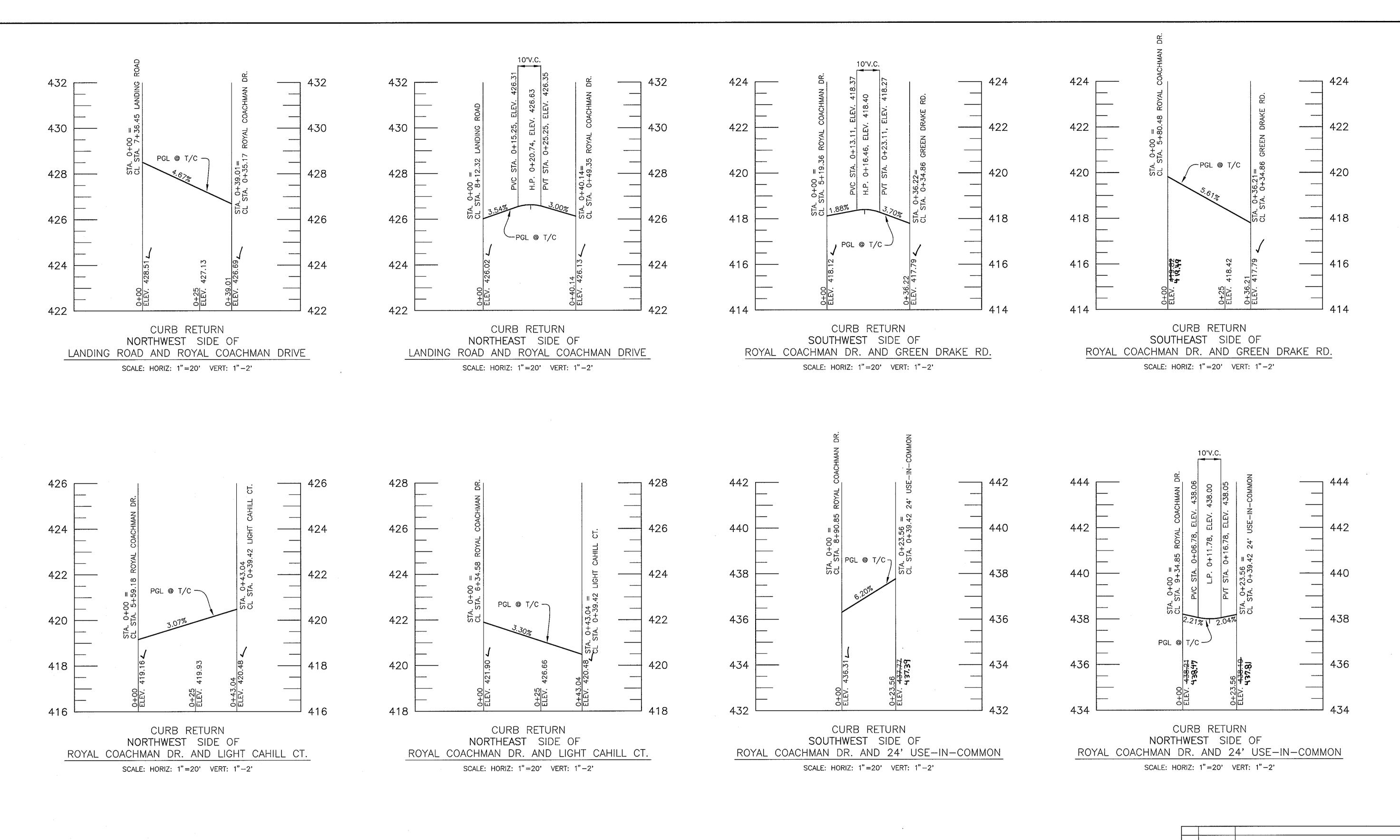
TAX MAP 31, GRID 10 &11, PARCELS 160,161, 788, & 791 st. ELECTION DISTRICT HOWARD COUNTY, MARYLAND ROAD PROFILE AND DETAILS CRAIG R. AND KAREN C. MARTIN 4937 LANDING ROAD ELKRIDGE, MD 21075

DATE: OCTOBER, 2003 PROJECT NO. 1383 DES: DAM DRN: RPS CHK: DAM SCALE: AS SHOWN DRAWING 8 OF 33

F - 03 - 134









AS-BUILT CERTIFICATION
I hereby certify, by my seal, that the facilities shown on this plan were constructed as shown on this AS-BUILT plan.
Donald Mason, P.E. No. 21443
Date 4-25-//

Professional Certification I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

License No. 21443 Expiration Date: 12-21-12

NO DATE REVISION BENCHMARK ● ENGINEERS ▲ LAND SURVEYORS ▲ PLANNERS ENGINEERING, INC.

8480 BALTIMORE NATIONAL PIKE ▲ SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644

11/17/03 CASCADE OVERLOOK

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

/2-24-03 DATE APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

OWNER/DEVELOPER CASCADE OVERLOOK, L.L.C. P.O.BOX 417 ELLICOTT CITY, MD 21041 (410) 465-4244

ELKRIDGE, MD 21075

SECTION ONE

LOTS 1 - 72 AND OPEN SPACE LOTS 73 - 80 AND

NON-BUILDABLE PARCEL "A" TAX MAP 31, GRID 10 &11, PARCELS 160, 161, 788, & 791
1 st. ELECTION DISTRICT HOWARD COUNTY, MARYLAND

CRAIG R. AND KAREN C. MARTIN 4937 LANDING ROAD

ROAD PROFILE

14/30/13

DES: DAM DRN: RPS CHK: DAM AS-BUILT

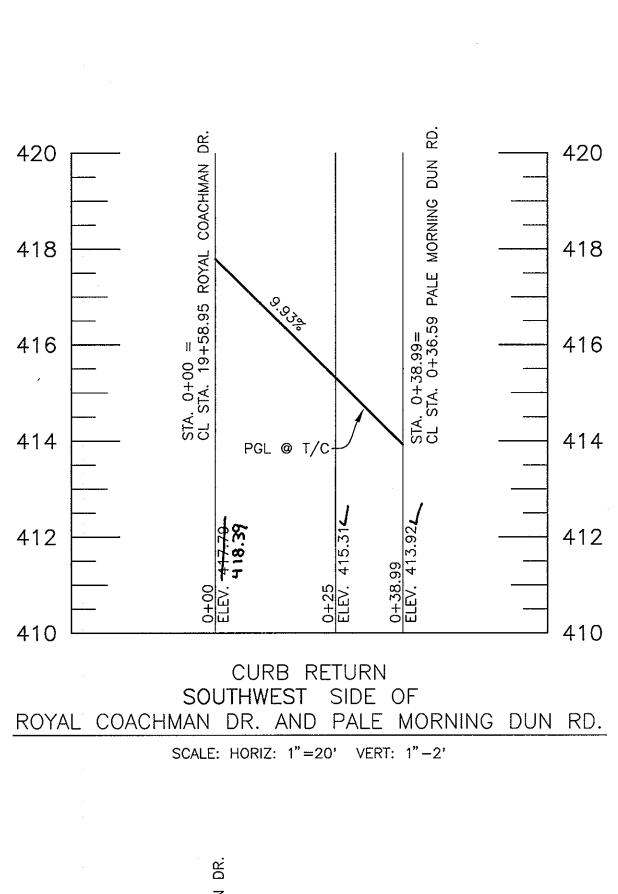
VP-86-130, F-88-20, S-01-04, PB-359, P-02-11 PROJECT NO. 1383 SCALE: AS SHOWN DRAWING 10 OF 33 F-03-134

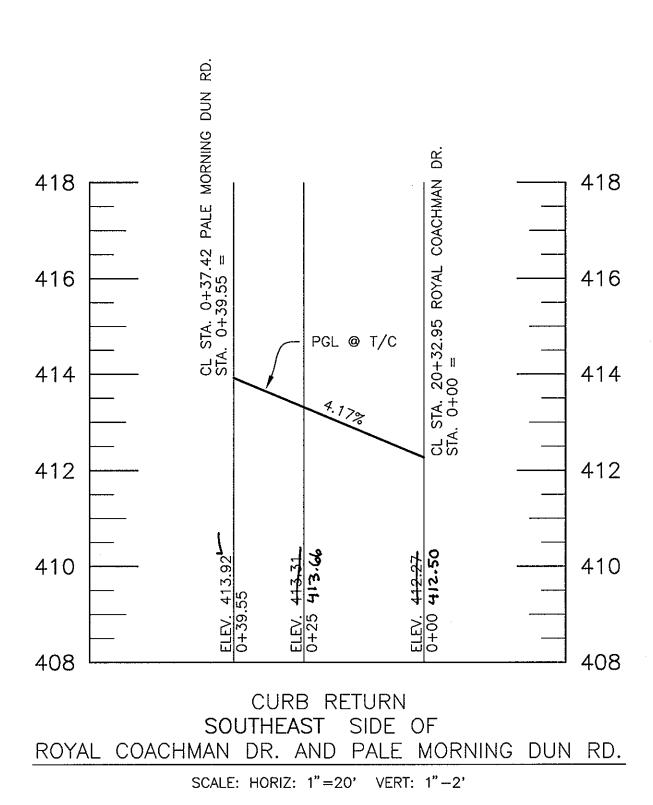
GENERAL NOTES:

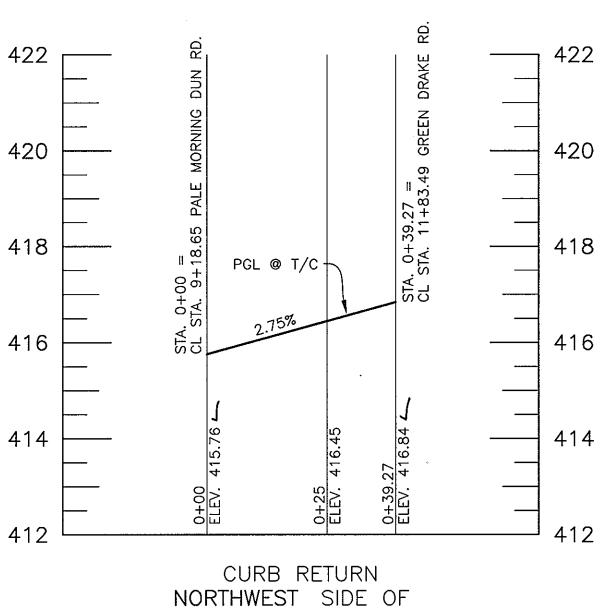
1. PGL GRADE SHOWN IS REPRESENTATIVE OF THE NORMAL TOP OF CURB.

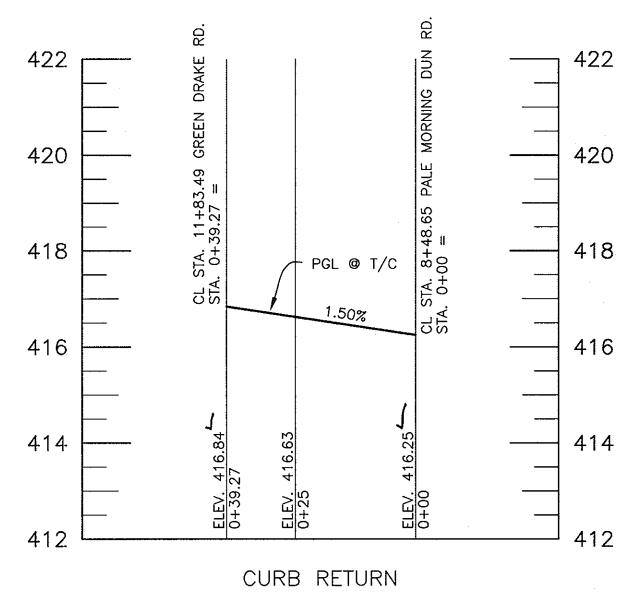
2. MODIFICATIONS TO THE PGL GRADE SHOWN WILL BE NECESSARY FOR RETURNS WITH HANDICAP RAMPS PROPOSED.

\\tsa\\rol1\engineer\projects\1383 PFAU\dwg\7017\$10.dwg, Sheet 10, 11/14/2003 11:44:14 AM



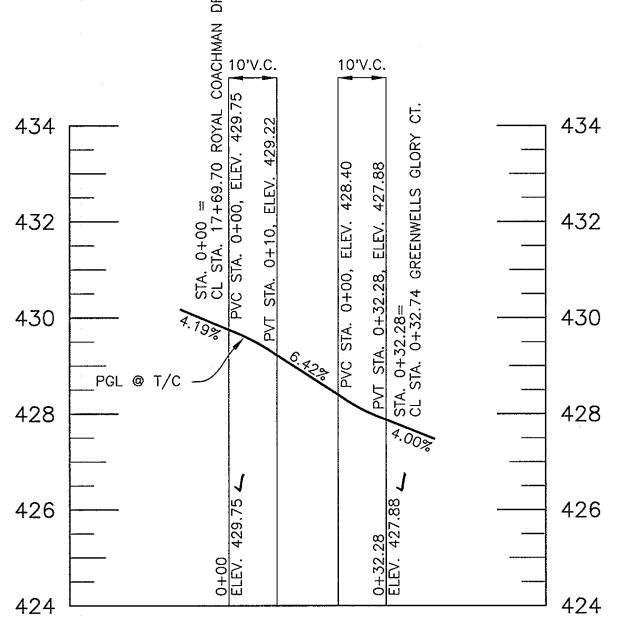




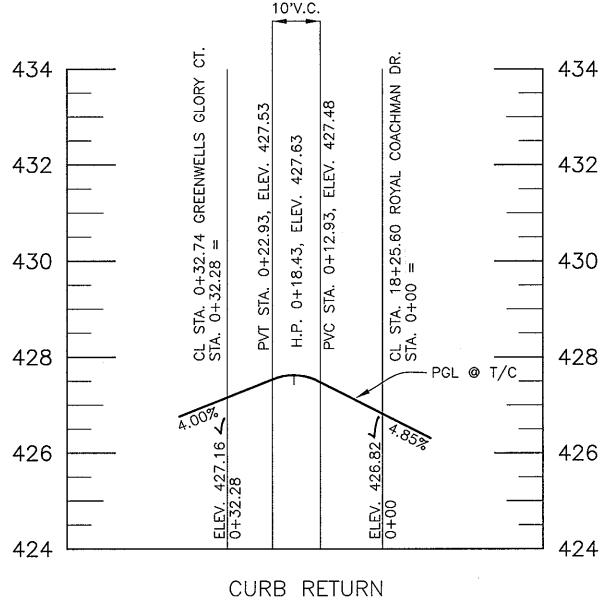


PALE MORNING DUN RD. AND GREEN DRAKE RD. SCALE: HORIZ: 1"=20' VERT: 1"-2'

NORTHEAST SIDE OF PALE MORNING DUN RD. AND GREEN DRAKE RD. SCALE: HORIZ: 1"=20' VERT: 1"-2'



CURB RETURN NORTHWEST SIDE OF MARCH BROWN RD. ROYAL COACHMAN DR. AND GREENWELLS GLORY CT. SCALE: HORIZ: 1"=20' VERT: 1"-2'



NORTHEAST SIDE OF MARCH BROWN RD. ROYAL COACHMAN DR. AND GREENWELLS GLORY CT. SCALE: HORIZ: 1"=20' VERT: 1"-2'



AS-BUILT CERTIFICATION
I hereby certify, by my seal, that the facilities shown on this plan were constructed as shown on this AS-BUILT plan.
Donald Mason, P.E. No. 21443
Date 4-25-//

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS 12-74-03 DATE APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING andy Hamitta

OWNER/DEVELOPER CASCADE OVERLOOK, L.L.C. P.O.BOX 417 ELLICOTT CITY, MD 21041 (410) 465-4244

CT: CASCADE OVERLOOK

SECTION ONE
LOTS 1 - 72 AND OPEN SPACE LOTS 73 - 80 AND
NON-BUILDABLE PARCEL 'A' TAX MAP 31, GRID 10 &11,PARCELS 160,161, 788, & 791 st. ELECTION DISTRICT HOWARD COUNTY, MARYLAND

ROAD PROFILE AND DETAILS VP-86-130, F-88-20, S-01-04, PB-359, P-02-11

CRAIG R. AND KAREN C. MARTIN 4937 LANDING ROAD ELKRIDGE, MD 21075 DATE: OCTOBER, 2003 PROJECT NO. 1383 DES: DAM DRN: RPS DRAWING 11 OF 33 CHK: DAM SCALE: AS SHOWN

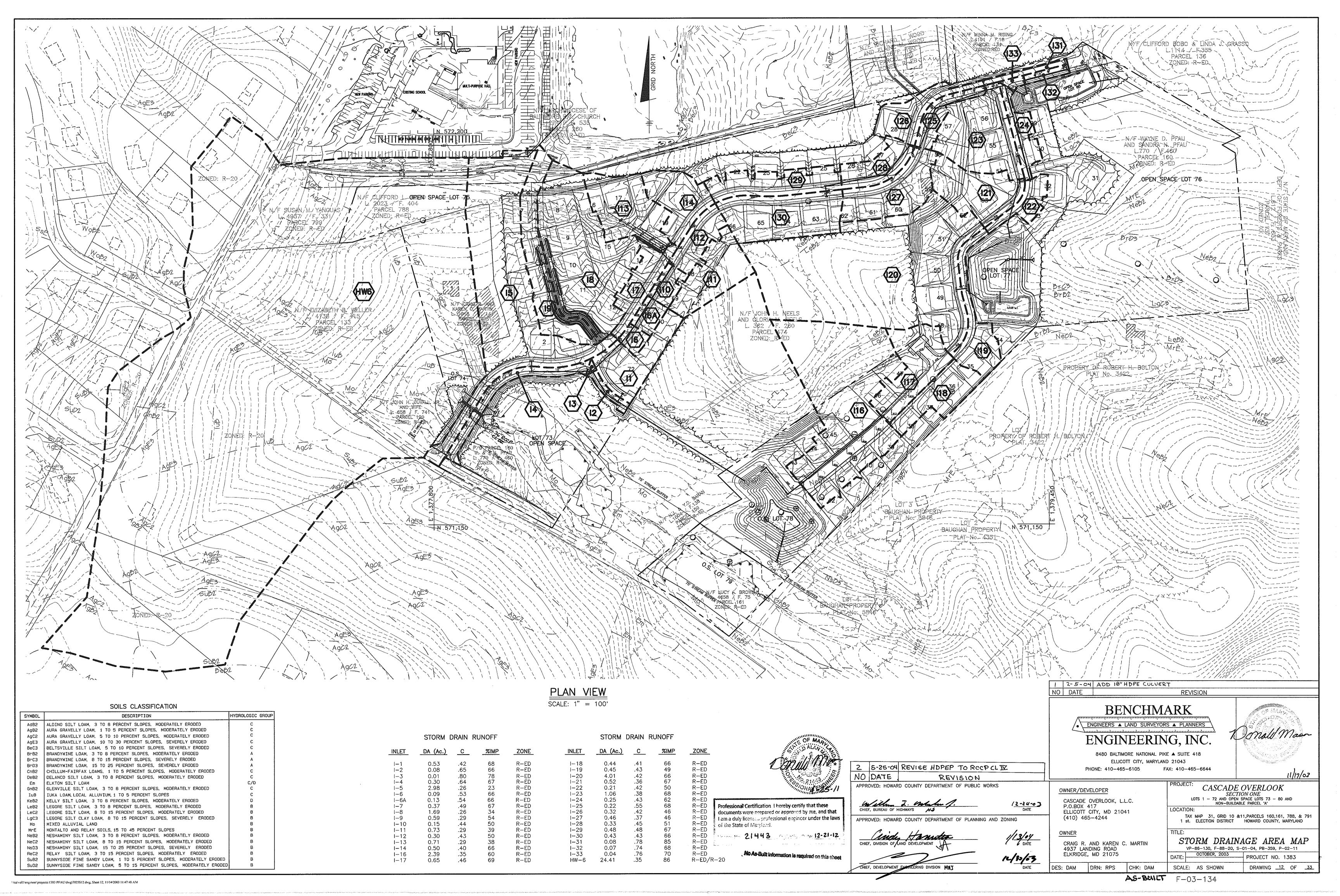
GENERAL NOTES:

1. PGL GRADE SHOWN IS REPRESENTATIVE OF THE NORMAL TOP OF CURB.

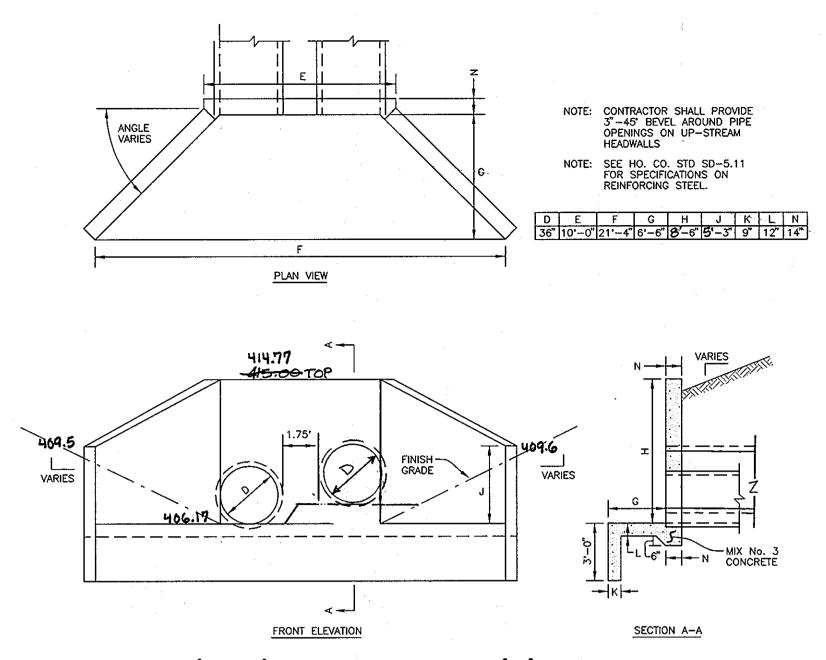
2. MODIFICATIONS TO THE PGL GRADE SHOWN WILL BE NECESSARY FOR RETURNS WITH HANDICAP RAMPS PROPOSED.

rofessional Certification Thereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 21443 Expiration Date: 12-21-12

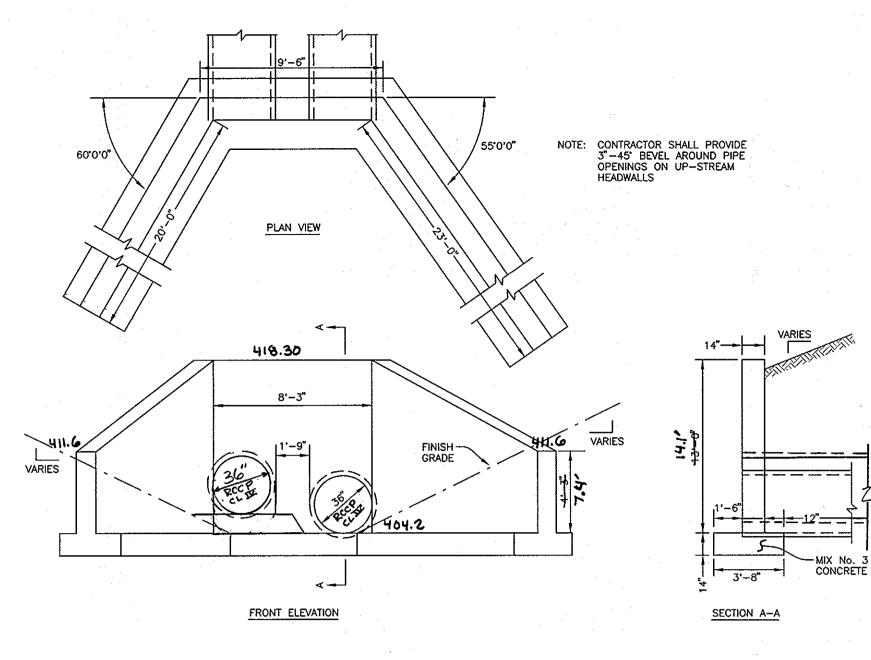
> AS-BUILT F-03-134



	<u> </u>	STRUCTURE SCH STORM INLETS				
NO.	TYPE	LOCATION	INV. IN	INV. OUT	TOP ELEV.	HO. CO. STD.
1-1	A-5	CL STA. 1+33.79 O/S 10.43' LEFT GREEN DRAKE RD.	411.21 -		415.35415.21	SD - 4.01 OR 4.40
1 -2	A-5	CL STA. 1+33.79 O/S 10.43' RIGHT GREEN DRAKE RD. 408.7	411.41 (15"-NE)	408.26 🗸	415.21 🗸	SD - 4.01 OR 4.40
1-3	A-5	CL STA. 0+31.85 O/S 10.43' RIGHT GREEN DRAKE RD.	411.51 (18"-NW) 412.96 (18"-NE)	L.	418.16 🗸	SD ~ 4.01 OR 4.40
1-4	A-5	CL STA. 3+86.37 O/S 12.43' RIGHT ROYAL COACHMAN DR.	413.30	413.13 🗸	416.83 🗸	SD - 4.01 OR 4.40
I-5	A-5	CL STA. 3+86.37 O/S 12.43' LEFT ROYAL COACHMAN DR.		H13.81 413.58	416.83 🗸	SD - 4.01 OR 4.40
1-6	A-5	CL STA. 6+37.44 O/S 12.43' RIGHT ROYAL COACHMAN DR.	416.28 (18"-NE) 415.48 (18"-NW)	415.28 🗸	422.29 🗸	SD - 4.01 OR 4.40
-6A	A-5	CL STA. 6+95.11 O/S 12.43' RIGHT ROYAL COACHMAN DR.	121.35421.28	421.254 21.08	425.08 🗸	SD - 4.01 OR 4.40
1-7	A5	CL STA. 6+37.44 O/S 12.43' LEFT ROYAL COACHMAN DR.		415.76✓	422.29 ✓	SD - 4.01 OR 4.40
1–8	TYPE 'D' INLET	CL STA. 0+47.42 O/S 14.33' RIGHT LIGHT CAHILL CT.	416.60 🗸	416.35 🗸	419.33 419.64-	SD - 4.11 OR 4.39
1-9	TYPE 'D' INLET	CL STA. 0+47.42 O/S 14.33' LEFT LIGHT CAHILL CT.	_		419.64	SD - 4.11 OR 4.39
-10	A-5	CL STA. 7+80.75 O/S 12.43' RIGHT ROYAL COACHMAN DR.	423.10 (18"-NE) 423.10 (15"-SE)		430.20 🗸	SD - 4.01 OR 4.40
-11	TYPE 'D' INLET	N 571709.01 E 1378463.16	_		1257926.66	SD - 4.11 OR 4.39
-12	A-5	CL STA. 8+87.85 O/S 12.43' RIGHT ROYAL COACHMAN DR.	33. ²⁸ 4 33.11	33.17 432.91	436.47 🗸	SD - 4.01 OR 4.40
-13	TYPE 'D' INLET	CL STA. 0+34.33 O/S 14.33' RIGHT 24' USE-IN-COMMON	<u> </u>	134.454 34.17	437.23 🗸	SD - 4.11 OR 4.39
-14	A-5	CL STA. 9+37.85 O/S 12.43' LEFT ROYAL COACHMAN DR.	- 43	5.65 435.26 L	38 . 774 38.58	SD - 4.01 OR 4.40
-16	A-5	CL STA. 13+72.90 O/S 17.00' RT PALE MORNING DUN RD.	96,773 97.00	396.80 🗸	400.36 🗸	SD - 4.01 OR 4.40
-17	A-10	CL STA. 10+86.05 O/S 12.43' RT PALE MORNING DUN RD,	107-114 07:99	407.74 🗸 1	111,40 4 11:61	SD - 4.02 OR 4.41
-18	A-5	CL STA. 10+86.05 O/S 12.43' LF PALE MORNING DUN RD.	_	408.25~	411.61	SD - 4.01 OR 4.40
-19	A-5	CL STA. 6+46.82 O/S 12.43' LF PALE MORNING DUN RD.	410.92 🗸	410.72 (18"-N) 408.31 (10"-S)	415.01	SD - 4.01 OR 4.40
-20	A-10	CL STA. 6+46.82 O/S 12.43' RT PALE MORNING DUN RD.	_	411.31	415.01 🗸	SD - 4.02 OR 4.41
-21	A-5	CL STA. 2+75.35 O/S 12.43' RT PALE MORNING DUN RD. &	05 ^{,86} 4 06.06 i	05.564 05.86	415.01	SD - 4.01 OR 4.40
-22	A-5	CL STA. 2+75.35 O/S 12.43' LF PALE MORNING DUN RD. 4	06.144 06.54 i	06 <i>p</i> 34 06.34	415.01 🗸	SD - 4.01 OR 4.40
-23	A-5	CL STA. 1+00.00 O/S 12.43' RT PALE MORNING DUN RD.	110G C-E	410.05 🗸	413.39 🗸	SD - 4.01 OR 4.40
-24	A-5	CL STA. 1+00.00 O/S 12.43' LF PALE MORNING DUN RD.	408.44 (15" -N) 409.77 (15" -W) 414.74 (15" -N) 415.00 (18" -W)	108.19	413.39 🗸	SD - 4.01 OR 4.40
-25	A-5	CL STA. 19+55.94 O/S 12.43' RT ROYAL COACHMAN DR.	414.74 (15"-N) 415.00 (18"-W)	4139 ⁵ 413.76	418.36	SD - 4.01 OR 4.40
-26	A-5	CL STA. 19+55.94 O/S 12.43' LF ROYAL COACHMAN DR.	414.32	114.74 415.00	418.36 🗸	SD - 4.01 OR 4.40
-27	A-5	CL STA. 16+77.52 O/S 12.43' RT ROYAL COACHMAN DR.	429.83 🗸	429.63 V	433.78 🗸	SD - 4.01 OR 4.40
-28	A-5	CL STA. 16+77.52 O/S 12.43' LF ROYAL COACHMAN DR.	430.42 🗸	430.22 🗸	433.78 🗸	SD - 4.01 OR 4.40
-29	A-5	CL STA. 13+92.17 O/S 12.43' LF ROYAL COACHMAN DR.	434.82 V	434.54 🗸	438.44 🗸	SD - 4.01 OR 4.40
_30	A-5	CL STA. 13+92.17 O/S 12.43' RT ROYAL COACHMAN DR.	_	435.10 V	438.44 🗸	SD - 4.01 OR 4.40
-31	A-5	CL STA. 21+63.65 O/S 12.43' LF ROYAL COACHMAN DR.		399.23 🗸	403.38 ✓	SD - 4.01 OR 4.40
-32	A-5	CL STA. 21+63.65 0/S 12.43' RT ROYAL COACHMAN DR.	349.09	398.89 /	403.38	SD - 4.01 OR 4.40
-33	A-5	CL STA. 20+61.37 O/S 12.43' RT ROYAL COACHMAN DR.	398.38	398.18 🗸	410.29 1	SD - 4.01 OR 4.40
		STORM MANHOLI	·	<u> </u>		
	· · · · · · · · · · · · · · · · · · ·			IND/ OUT	TOP ELEV.	HO. CO. STD.
NO.	TYPE	LOCATION 5 1779001 00	INV. IN	INV. OUT		
M-1	4'-0" MANHOLE			469.31	415.8±√ 39 ⁷⁸⁾ 398.00	G - 5.12
√-2 -	4'-0" MANHOLE			<u> </u>	398.00 ×	G - 5.12
√ 1−3	4'-0" MANHOLE	N 571231.19 E 1378748.96	394.96 ✓ 406.99 ✓	394.76 🗸	412.0± ✓	G - 5.12
√-4 	4'-0" MANHOLE	N 571865.93 E 1379235.95	406.99 V	406.79 🗸	412.0± v	G - 5.12
M-5	4'-0" MANHOLE		1	ľ	<u> </u>	G - 5.12
M-6	4'-0" MANHOLE	CL STA. 0+13.34 0/S 13.75' LF PALE MORNING DUN RD. U	423.93		414.23 428.15 √	G - 5.12
M-7	4'-0" MANHOLE	CL STA. 18+03.65 O/S 16.05' RT ROYAL COACHMAN DR.		423.73 \(\square \)	36.204 36.39	G - 5.12
8-N	4'-0" MANHOLE	CL STA. 15+66.04 O/S 15.08' LF ROYAL COACHMAN DR. 4	• • • • • • • • • • • • • • • • • • • •	431.80 ·	136.59-	G - 5.12
		END SECTIONS			·	
NO.	TYPE	LOCATION	INV. IN	INV. OUT	TOP ELEV.	HO. CO. STD.
	18" RCCP END SECTIO	· · · · · · · · · · · · · · · · · · ·	392.11 🗸	392.00 🗸	_	SD - 5.52
	18" BECTIO		392.11 🗸	392.00 🗸	_	SD - 5.52
S-3	15" CL TO END SECTIO	N 567726.38 E 1380744.93	402.56 🗸	402.50 🗸		SD - 5.52
	15" CCE END SECTIO		409.32	409,29	-	SD- 5.52
	18" CC TE END SECTIO		404.08 🗸	404.00 🗸		SD - 5.52
	18" RCEP END SECTIO		404.03 🗸	404.00 🗸	_	SD - 5.52
S-8	24" HOPE END SECTIO		394.23	394.20	_	SD - 5.52
		HEADWALLS	<u></u>	·		
NO.	TYPE	LOCATION	INV. IN	INV. OUT	TOP ELEV.	HO. CO. STD.
HW1	TYPE 'A' HEADWALL	N 571053.01 E 1378719.91	384.62 🗸	_	388.62 🗸	SD - 5.11(30"PIPE)
HW2	TYPE 'A' HEADWALL	N 571860.35 E 1379404.67	398.00 🗸	_	402.00 🗸	SD - 5.11(30"PIPE)
HW3	TYPE 'C' ENDWALL	N 571747.38 E 1379274.67	405.00 🗸	-	406.75 🗸	SD - 5.21(12"PIPE)
-IW4	TYPE 'C' ENDWALL	N 572306.53 E 1379375.53	398.00 🗸	-404.19	00.00 3 98.75 -	SD - 5.21(12"PIPE)
HW5	SEE DETAIL THIS SHEE	T N 571441.69 E 1377931.66	405.19	405.60/404.50	418.50	SEE DETAIL THIS SHEE
HW6	SEE DETAIL THIS SHEE	T N 571482.95 E 1377875.32 407.10	407.50/406.50	406.16 -	414.764 15.00-	SEE DETAIL THIS SHEE
HW7	TYPE 'C' ENDWALL	N 572308.59 E 1379390.67	398.00 ✓	<u> </u>	400.25 🗸	SD - 5.21(18"PIPE)
144 /		OUTFALL STRUCTU	JRES			
1447		LOCATION	INV. IN	INV. OUT	TOP ELEV.	HO. CO. STD.
	TYPE	N 571121.25 E 1378735.73	385.00 🗸	385.00 🗸	397.00 436	
NO.	TYPE SEE DETAIL	17 07 12 1.20 - 6 10/0/00.70	398.50 🗸	398.50 🗸	410.00 436	
NO. S-1	SEE DETAIL	N 571854.42 E 1370332.03		1 355.55		-
NO. S-1 S-2	SEE DETAIL SEE DETAIL	N 571854.42 E 1379332.93	403.58	403.58	408.83 . /	<u></u>
NO. S-1 S-2 S-3	SEE DETAIL SEE DETAIL SEE DETAIL	N 571744.02 E 1379313.36	403.58 394.98 ✓	403.58 394.98 ✓	408.83	
NO. S-1 S-2 S-3 S-4	SEE DETAIL SEE DETAIL SEE DETAIL SEE DETAIL	N 571744.02 E 1379313.36 N 572335.75 E 1379568.82	394.98 🗸	394.98 🗸	408.83 ✓ 401.83 ✓	
NO. S-1 S-2 S-3	SEE DETAIL SEE DETAIL SEE DETAIL 1) STRUCTURE 2) STRUCTURE	N 571744.02 E 1379313.36 N 572335.75 E 1379568.82 ELEVATION AND LOCATION FOR MANHOLES IS AT THE TOP AN ELEVATION AND LOCATION FOR CURB INLETS IS AT THE TOP	394.98 D CENTER OF R OF CURB AT MI	394.98 V	401.83 V	-
NO. S-1 S-2 S-3	SEE DETAIL SEE DETAIL SEE DETAIL SEE DETAIL 1) STRUCTURE 2) STRUCTURE 3) STRUCTURE	N 571744.02 E 1379313.36 N 572335.75 E 1379568.82 ELEVATION AND LOCATION FOR MANHOLES IS AT THE TOP AN ELEVATION AND LOCATION FOR CURB INLETS IS AT THE TOP ELEVATION AND LOCATION FOR ENDSECTIONS IS AT THE CONTINUE OF T	394.98 D CENTER OF R OF CURB AT MI	394.98 V	401.83 V	-
NO. S-1 S-2 S-3	SEE DETAIL SEE DETAIL SEE DETAIL SEE DETAIL 1) STRUCTURE 2) STRUCTURE 3) STRUCTURE 4) PRECAST STI 5) ALL STORM	N 571744.02 E 1379313.36 N 572335.75 E 1379568.82 ELEVATION AND LOCATION FOR MANHOLES IS AT THE TOP AN ELEVATION AND LOCATION FOR CURB INLETS IS AT THE TOP	394.98 D CENTER OF R OF CURB AT MI NECTION OF PIPE	394.98 V	401.83 V	-



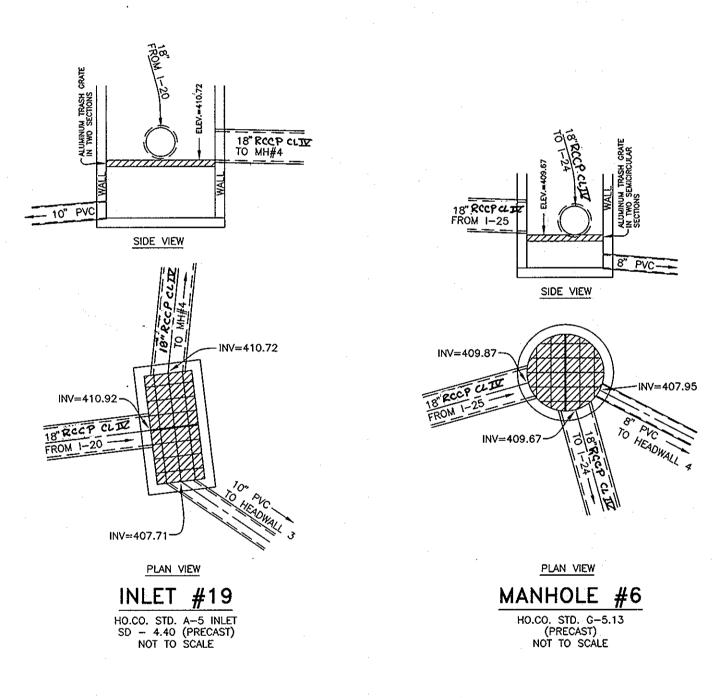


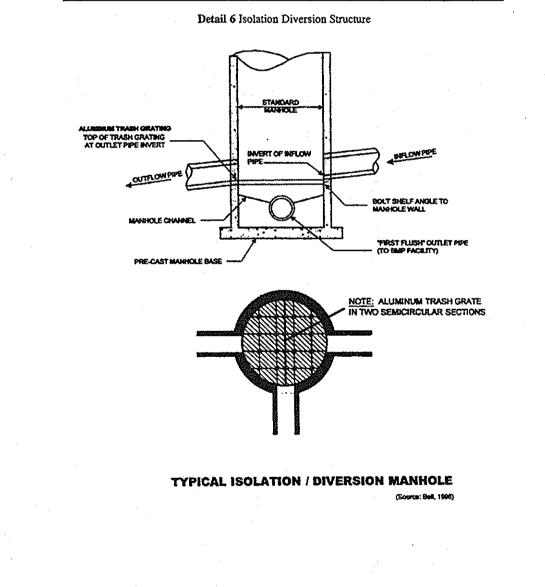


'HW5' MODIFIED TYPE 'A' HEADWALL NOT TO SCALE

	PI	PE SCH	EDULE	
RUN	SIZE	LENGTH	SLOPE	TYPE & CLASS
HW-1 TO S-1	30"	75'	0.50%	RCCP IV
ES-1 TO I-2	18"	404'	4.03%	RCCP W
I-2 TO M-1	18"	37'	2.30%	RCCP W
M-1 TO I-3	. 18"	72'	2.42%	RCCP W
I-3 TO I-4	18"	162'	1.00%	RCCP IV
I-4 TO I-5	15"	28'	1.00%	RCCP IV
I-2 TO I-1	15"	28'	1.00%	RCCP IV
I-3 TO I-6	18"	109'	2.14%	RCCP IV
I-6 TO I-6A	. 18"	61'	7.93%	RCCP IV
I-6A TO I-10	18"	86'	1.89%	RCCP IV
I-10 TO I-12	18"	107'	9.16%	Recp IV
I-12 TO I-13	18"	50'	2.13%	RCCP W
I-13 TO I-14	15"	47'	1.00%	RCCP. IV
I-10 TO I-11	15"	94'	1.00%	RCCP IV
1-6 TO 1-7	. 18"	28'	1.00%	RCCP IV
I-7 TO I-8	18"	39'	1.00%	RCCP IV
I-8 TO I-9	15"	29'	1.00%	RCCP W
ES-2 TO M-2	18"	15'	4.00%	RCCP IV
M-2 TO M-3	18"	92'	2.00%	RCCP W
M-3 TO I-16	18"	43'	4.28%	RCCP IV
I-16 TO I-17	18"	285'	3.77%	RCCP IV
I-17 TO I-18	15"	26'	1.00%	RCCP IV

PIPE SCHEDULE					
RUN	SIZE	LENGTH	SLOPE	TYPE & CLASS	
HW2 TO S-2	36"	64'	0.78%	RCCP IV	
ES-3 TO S-3	15"	36'	3.03%	RCCP IV	
HW3 TO I-19	10"	70'	4.70%	PVC SCH 40	
ES-5 TO M-4	18"	96'	2.84%	RCCP IV	
M-4 TO I-19	18"	83'	4.45%	RCCP IV	
I-19 TO I-20	18 "	28'	1.38%	RCCP IV	
ES-6 TO M-5	18"	34'	1.20%	RCCP IV	
M-5 TO I-21	18"	122'	1.00%	RCCP W	
I-21 TO I-22	18"	28'	1.00%	RCCP W	
I-22 TO I-24	18"	178'	0.93%	RCCP IV	
I-24 TO M-6	18"	87'	1.42%	RCCP IV	
M-6 TO I-25	18"	54'	7.27%	RCCP W	
I-25 TO M-7	18"	143'	6.13%	RCCP IV	
M-7 TO I-27	18"	117'	4.89%	RCCP IV	
I-27 TO I-28	18"	28'	1.38%	RCCP IV	
I-28 TO M-8	18"	98'	1.48%	RCCP IV	
M-8 TO I-29	18"	166'	1.48%	RCCP IV	
I-29 TO I-30	15"	28'	1.00%	RCCP IV	
I-24 TO I-23	15"	28'	1.00%	RCCP IV	
I-25 TO I-26	15"	28'	0.90%	RCCP IV	
HW4 TO M-6	8"	60,	12.47%	PVC SCH 40	
ES-8 TO S-4	24"	69,	1.00%	HDPE	
HW7 TO I-33	18"	37'	0.5%	RCCB IN	
I-33 TO I-32	18"	102'	0.5%	RCCP W	
I-32 TO I-31	15"	28'	0.50%	RCCP IV	
HW5 TO HW6	2-36"	70'	2.86%	RCCP IV	
ES-9 TO ES-10	18"	90'	0.50%	RCCP IV	





DIVERSION STRUCTURE DETAILS



AS-BUILT CERTIFICATION
I hereby certify, by my seal, that the facilities shown on this plan were constructed as control on the AS-STREET plan.
Consideration, FER 116, 218449
Date 4/25/11

Professional Certification I hereary cartify that these documents were preserved as augicine by me, and that Lam a duly licensed professional augusta under the laws of the State of Marytand.

Littense No. 21443 . Excitation Date: 12-21-12

4 5-25-201 REVISE PIPE RUN FROM ESS TO S4 TO BE HOPE.

3 10-11-04 REV. I-31, I-32, I-33, PIPE RUNS HW7 TO I-33, I-33 TO I-32. 2 5-25-04 REVISE HOPEP TO RCCP CLIX 1 2-5-04 ADD ES-10 AND REVISE HW5 AND HW6 NO DATE REVISION

> BENCHMARK ● ENGINEERS ▲ LAND SURVEYORS ▲ PLANNERS ENGINEERING, INC.

8480 BALTIMORE NATIONAL PIKE ▲ SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644

DRAWING 13 OF 33

CASCADE OVERLOOK, L.L.C. P.O. BOX 417 ELLICOTT CITY, MD 21041 (410) 465-4244

OWNER/DEVELOPER

CT: CASCADE OVERLOOK
SECTION ONE
LOTS 1 - 72 AND OPEN SPACE LOTS 73 - 80 AND
NON-BUILDABLE PARCEL 'A' TAX MAP 31, GRID 10 &11, PARCELS 160,161, 788, & 791 1 st. ELECTION DISTRICT HOWARD COUNTY, MARYLAND

TILE: STRUCTURE AND PIPE SCHEDULE & STORMWATER MANAGEMENT DETAILS VP-86-130, F-88-20, S-01-04, PB-359, P-02-11 DATE: OCTOBER, 2003 PROJECT NO. 1383

SCALE: AS SHOWN DRN: RPS

APPROVED:	HOWARD COUNT	DEPARTMENT	OF PUBLIC	WORKS
m/ii	elin 7 4	elada J.		
	EAU OF HIGHWAYS	HS		
PPROVED:	HOWARD COUNTY	DEPARTMENT	OF PLANNING	G AND

CRAIG R. AND KAREN C. MARTIN 4937 LANDING ROAD ELKRIDGE, MD 21075 DES: DAM

AS-BUILT F-03-134

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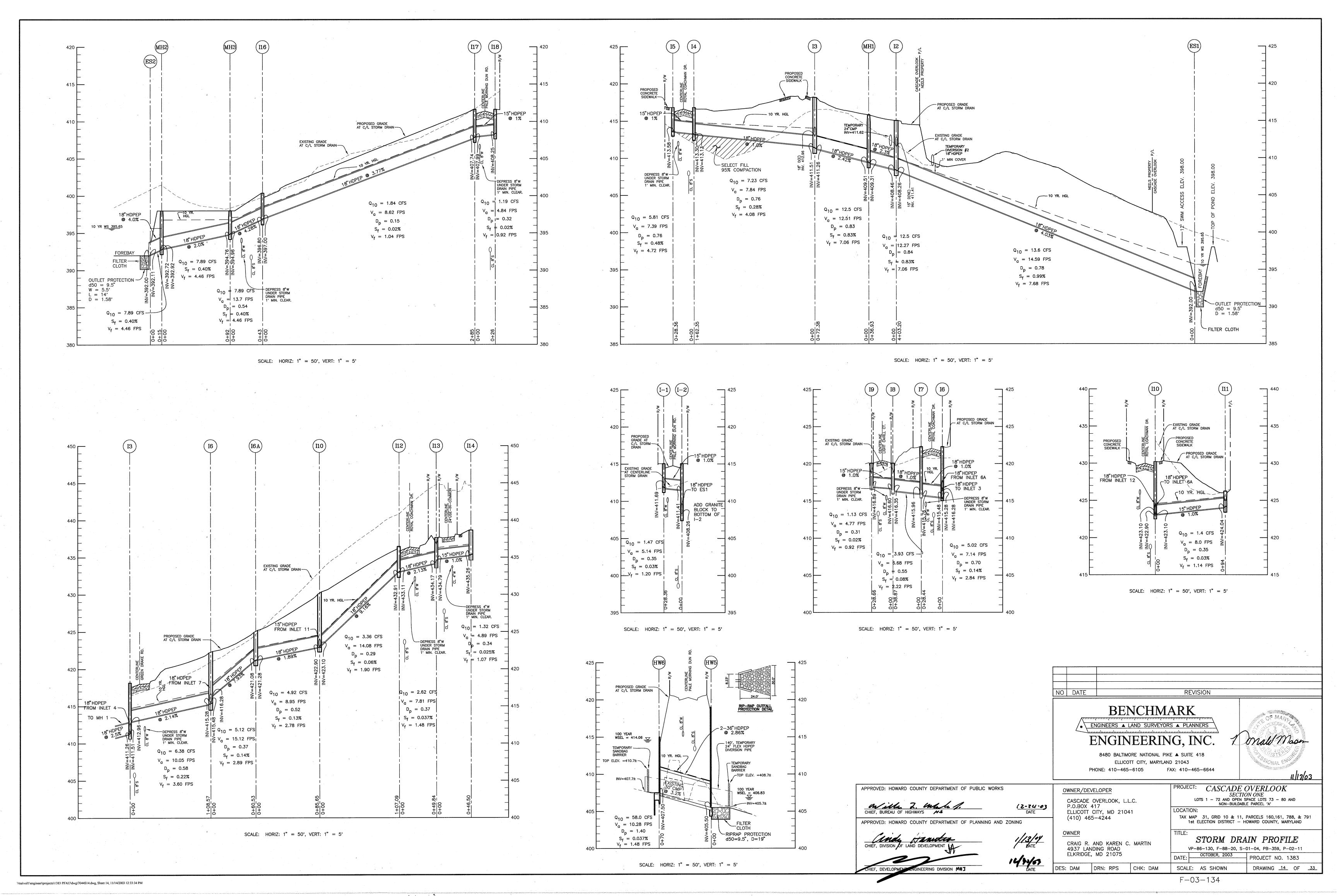
ES-10 18' RCCP END SECTION

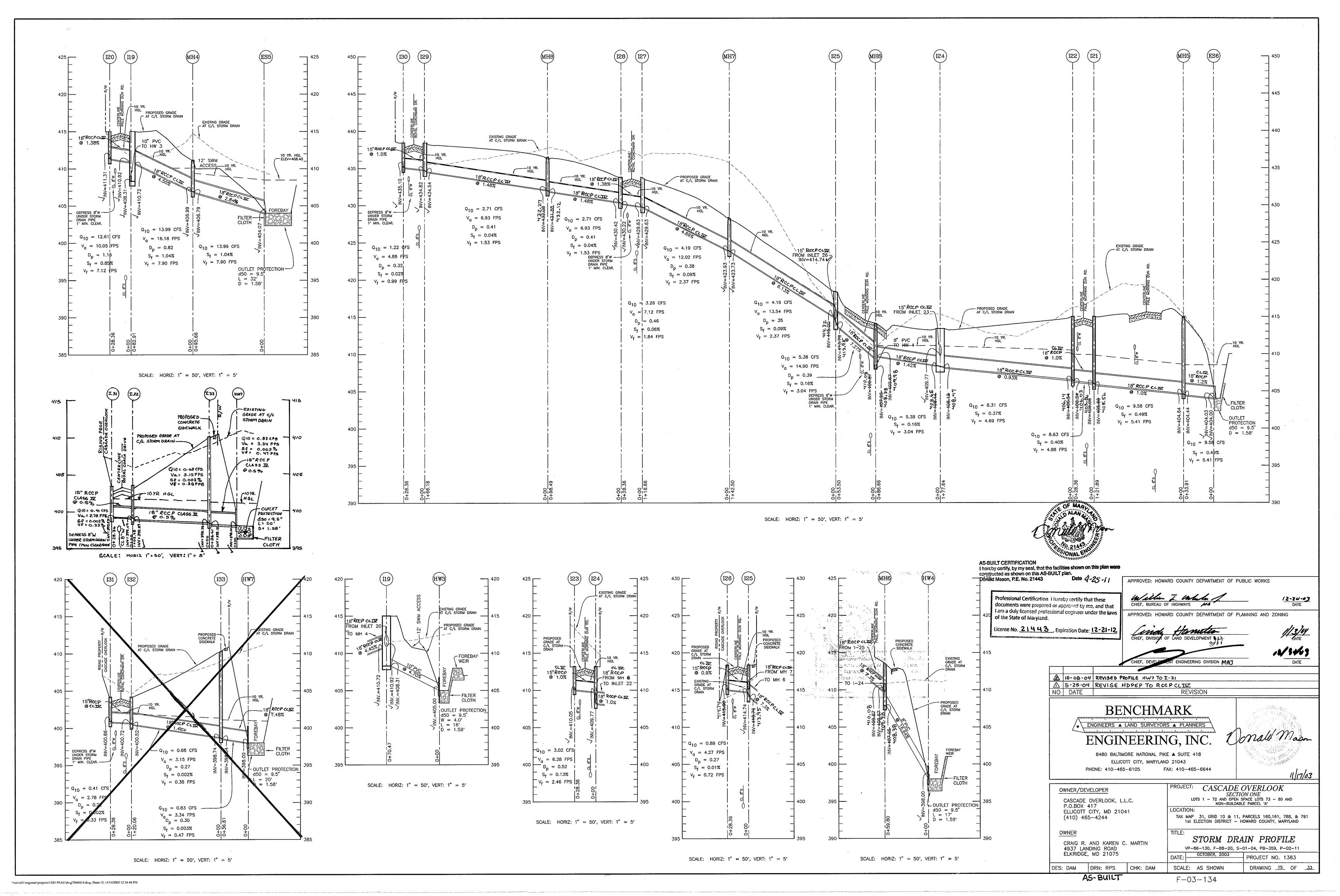
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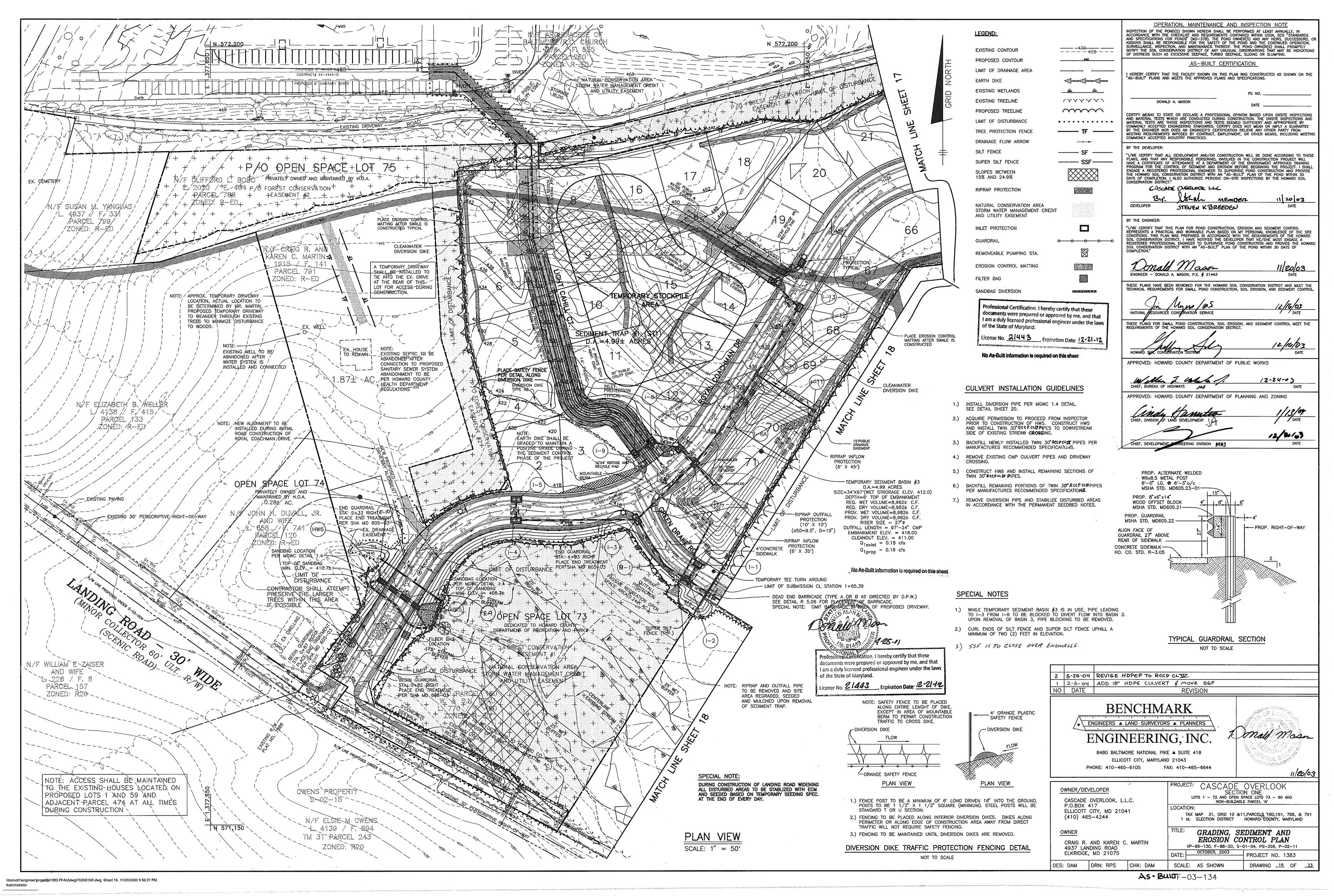
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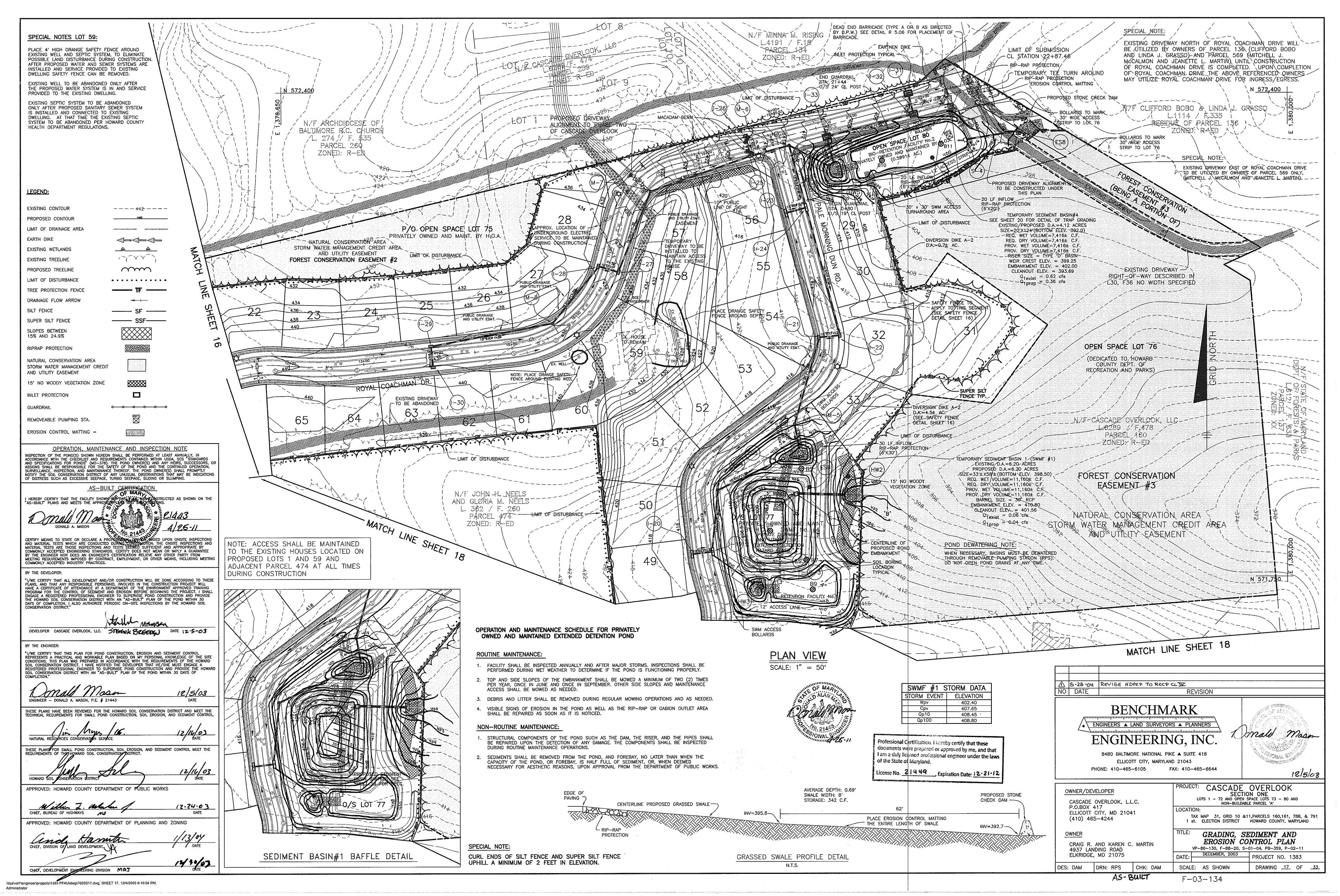
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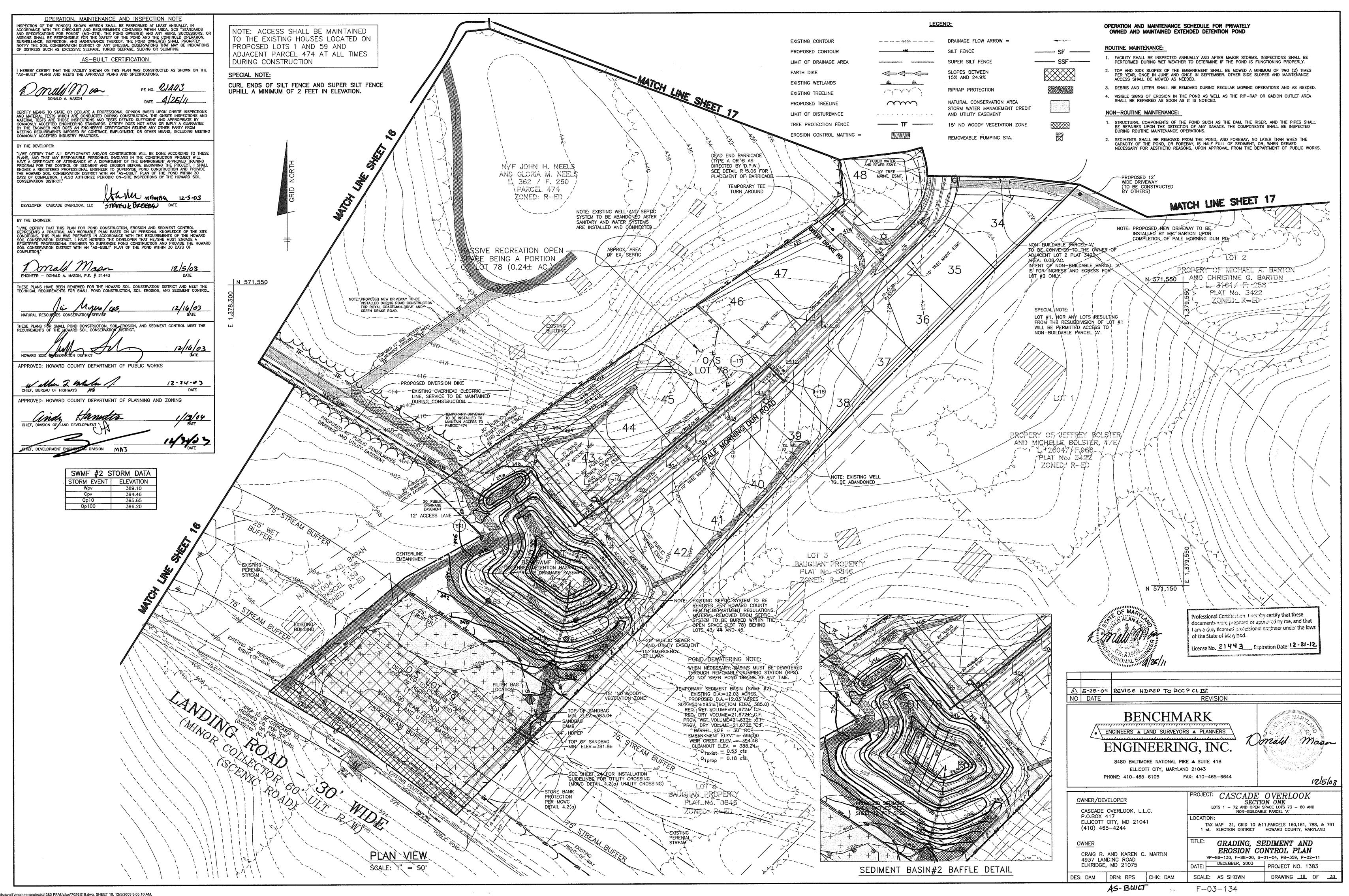
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SEDIMENT CONTROL NOTES

- 1. A MINIMUM OF 24 HOURS NOTICE MUST BE GIVEN TO THE HOWARD COUNTY DEPARTMENT OF INSPECTION, LICENSES AND PERMITS, SEDIMENT CONTROL DIVISION PRIOR TO THE START OF ANY CONSTRUCTION, (313-1850).
- ALL VEGETATIVE AND STRUCTURAL PRACTICES ARE TO BE INSTALLED ACCORDING TO THE PROVISIONS OF THIS PLAN AND ARE TO BE IN CONFORMANCE WITH THE MOST CURRENT "MARYLAND STANDARDS AND SPECIFICATION FOR SOIL EROSION AND SEDIMENT CONTROL", REVISIONS THERETO.
- FOLLOWING INITIAL SOIL DISTURBANCE OR REDISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED WITHIN: A) 7 CALENDAR DAYS FOR ALL PERIMETER SEDIMENT CONTROL STRUCTURES, DIKES, PERIMETER SLOPES AND ALL SLOPES GREATER THAN 3:1, B) 14 DAYS AS TO ALL OTHER DISTURBED OR GRADED
- ALL SEDIMENT TRAPS/BASINS SHOWN MUST BE FENCED AND WARNING SIGNS POSTED AROUND THEIR PERIMETER IN ACCORDANCE WITH VOL. 1, CHAPTER 12, OF THE HOWARD
- ALL DISTURBED AREAS MUST BE STABILIZED WITHIN THE TIME PERIOD SPECIFIED ABOVE IN ACCORDANCE WITH THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR PERMANENT SEEDINGS (SEC. 51) SOD (SEC. 54), TEMPORARY SEEDING (SEC. 50) AND MULCHING (SEC. 52). TEMPORARY STABILIZATION WITH MULCH ALONE CAN ONLY BE DONE WHEN RECOMMENDED SEEDING DATES DO NOT ALLOW FOR PROPER GERMINATION AND ESTABLISHMENT OF GRASSES.
- ALL SEDIMENT CONTROL STRUCTURES ARE TO REMAIN IN PLACE AND ARE TO BE MAINTAINED IN OPERATIVE CONDITION UNTIL PERMISSION FOR THEIR REMOVAL HAS BEEN OBTAINED FROM THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.

SITE ANALYSIS:	
TOTAL AREA OF SITE	36.78 ACRES
AREA DISTURBED	21.74 ACRES
AREA TO BE ROOFED OR PAVED	11.51 ACRES
AREA TO BE VEGETATIVELY STABILIZED	10.23 ACRES
TOTAL CUT	71003cy
TOTAL FILL	30388c _Y

OFFSITE WASTE/BORROW AREA LOCATION ANY SEDIMENT CONTROL PRACTICE WHICH IS DISTURBED BY GRADING ACTIVITY FOR PLACEMENT OF UTILITIES MUST BE REPAIRED ON THE SAME DAY OF DISTURBANCE

SEE NOTE #12

ADDITIONAL SEDIMENT CONTROL MUST BE PROVIDED, IF DEEMED NECESSARY BY THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR.

COUNTY DESIGN MANUAL, STORM DRAINAGE.

- 10. ON ALL SITES WITH DISTURBED AREAS IN EXCESS OF 2 ACRES, APPROVAL OF THE INSPECTION AGENCY SHALL BE REQUESTED UPON COMPLETION OF INSTALLATION OF PERIMETER EROSION AND SEDIMENT CONTROLS, BUT BEFORE PROCEEDING WITH ANY OTHER EARTH DISTURBANCE OR GRADING, OTHER BUILDING OR GRADING INSPECTION APPROVALS MAY NOT BE AUTHORIZED UNTIL THIS INITIAL APPROVAL BY THE INSPECTION AGENCY IS MADE.
- TRENCHES FOR THE CONSTRUCTION OF UTILITIES IS LIMITED TO THREE PIPE LENGTHS OR THAT WHICH CAN BE BACK FILLED AND STABILIZED WITHIN ONE WORKING DAY,
- WASTE WILL BE HAULED TO AN APPROVED WASTE DISPOSAL SITE, WITH A ACTIVE

TEMPORARY SEEDBED PREPARATIONS

APPLY TO GRADED OR CLEARED AREAS LIKELY TO BE REDISTURBED WHERE A SHORT-TERM VEGETATIVE COVER IS NEEDED.

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED. SOIL AMENDMENTS: APPLY 600 LBS PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ FT) SEEDING: FOR PERIOD MARCH 1 THROUGH APRIL 30 AND FROM AUGUST 15 THROUGH NOVEMBER 15, SEED WITH 2-1/2 BUSHELS PER ACRE OF ANNUAL RYE (3.2 LBS/1000 SQ FT). FOR THE PERIOD MAY 1 THROUGH AUGUST 14, SEED WITH 3 LBS PER ACRE OF WEEPING LOVEGRASS (.07 LBS/1000 SQ FT). FOR THE PERIOD NOVEMBER 16 THROUGH FEBRUARY 28. PROTECT SITE BY APPLYING 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING, OR USE SOD.

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ FT) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ FT) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES, 8 FT. OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ FT) FOR ANCHORING

REFER TO THE 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL FOR RATE AND METHODS NOT COVERED.

PERMANENT SEEDBED PREPARATIONS

SEEDBED PREPARATION: LOOSEN UPPER THREE INCHES OF SOIL BY RAKING, DISCING OR OTHER ACCEPTABLE MEANS BEFORE SEEDING, IF NOT PREVIOUSLY LOOSENED. IN LIEU OF SOIL TEST RECOMMENDATIONS, USE ON OF THE FOLLOWING

PREFERRED - APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS/1000 SQ FT) AND 600 LBS PER ACRE 10-10-10 FERTILIZER (14 LBS/1000 SQ FT) BEFORE SEEDING, HARROW OR DISC INTO UPPER THREE INCHES OF SOIL, AT TIME OF SEEDING, APPLY 400 LBS PER ACRE 30-0-0- UREAFORM FERTILIZER

ACCEPTABLE - APPLY 2 TONS PER ACRE DOLOMITIC LIMESTONE (92 LBS/1000 SQ FT) AND 1000 LBS PER ACRE 10-10-10 FERTILIZER (23 LBS/1000 SQ FT) BEFORE SEEDING. HARROW OR DISC INTO UPPER THREE INCHES OF SOIL.

SEEDING: FOR THE PERIODS MARCH 1 THROUGH APRIL 30 AND AUGUST 1 THROUGH OCTOBER 15, SEED WITH 60 LBS PER ACRE (1.4 LBS/1000 SQ FT) OF KENTUCKY 31 TALL FESCUE PER ACRE AND 2 LBS PER ACRE (.05 LBS/1000 SQ FT) OF WEEPING LOVEGRASS. DURING THE PERIOD OF OCTOBER 16 THROUGH FEBRUARY 28, PROTECT SITE BY: OPTION (1) 2 TONS PER ACRE OF WELL ANCHORED STRAW MULCH AND SEED AS SOON AS POSSIBLE IN THE SPRING. OPTION (2) USE SOD. OPTION (3) SEED WITH 60 LBS PER ACRE OF KENTUCKY 31 TALL FESCUE AND MULCH WITH 2 TONS PER ACRE OF WELL ANCHORED STRAW.

MULCHING: APPLY 1-1/2 TO 2 TONS PER ACRE (70 TO 90 LBS/1000 SQ FT) OF UNROTTED SMALL GRAIN STRAW IMMEDIATELY AFTER SEEDING. ANCHOR MULCH IMMEDIATELY AFTER APPLICATION USING MULCH ANCHORING TOOL OR 218 GALLONS PER ACRE (5 GAL/1000 SQ FT) OF EMULSIFIED ASPHALT ON FLAT AREAS. ON SLOPES 8 FEET OR HIGHER, USE 348 GALLONS PER ACRE (8 GAL/1000 SQ FT) FOR ANCHORING.

MAINTENANCE: INSPECT ALL SEEDED AREAS AND MAKE NEEDED REPAIRS, REPLACEMENTS AND RESEEDINGS.

SEQUENCE OF CONSTRUCTION

NOTIFY SEDIMENT CONTROL DIVISION 48 HOURS PRIOR TO START OF CONSTRUCTION

OBTAIN GRADING PERMITOR A LETTER OF AUTHORIZATION FROM MDE. APPLICATION TRACKING NO. 03-NT-0297/200364843 MUST BE OBTAINED PRIOR TO DISTURBANCE OF THE STREAM OR WETLANDS FOR THE ROAD AND/OR UTILITY CROSSING.

INSTALL STABILIZED CONSTRUCTION ENTRANCES, TREE PROTECTION FENCES, SILT FENCES, SUPER SILT FENCES, TEMPORARY DIVERSION PIPES AND TEMPORARY CLEANWATER DIVERSION DIKES.

INSTALL SEDIMENT TRAPS AND BASINS.

INSTALL EARTH DIKES AND ANY REMAINING SEDIMENT CONTROL DEVICES. JPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, BRING ROAD BEDS ONLY O SUBGRADE AND STABILIZE SLOPES IN ACCORDANCE WITH TEMPORARY SEEDBED NOTES.

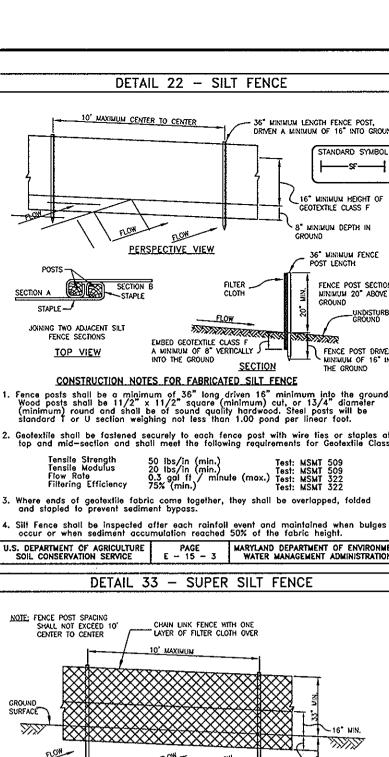
UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, MASS GRADE LOTS AND STABILIZE IN ACCORDANCE WITH TEMPORARY SEEDBED NOTES.

UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, INSTALL STORM DRAINS, WATER, SEWER AND UTILITY LINES. INLET 1-5 TO BE TEMPORARILY BLOCKED. DAY 150-170 UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, INSTALL CURB

AND GUTTER AND INSTALL PAVING. DAY 171-180 COMPLETE GRADING OF SITE AND STABLIZE DISTURBED AREAS IN ACCORDANCE WITH THE

DAY 181-190 UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, REMOVE SEDIMENT TRAPS AND REMAINING SEDIMENT CONTROL DEVICES, AND STABILIZED DISTURBED AREAS IN ACCORDANCE WITH THE PERMANENT SEEDBED NOTES.

DAY 191-200 UPON APPROVAL OF THE HOWARD COUNTY SEDIMENT CONTROL INSPECTOR, CONVERT SEDIMENT BASINS TO STORMWATER MANAGEMENT FACILITIES/ BIO-RETENTION FACILITIES. SHAPE FACILITIES PER FINAL GRADES SHOWN ON THE PLANS AND STABILIZE DISTURBED AREAS IN ACCORDANCE WITH THE



FLOW

PERSPECTIVE VIEW

CHAIN LINK FENCE ----

16" MIN. 1st LAYER FILTER CLOTH

FLOW.

_____SSF____

FLOW

8" MIN--)

A-2 B-3 TLOW PLOW → -/-> -CUT OR FILL SLOPE CROSS SECTION PLAN VIEW FLOW CHANNEL STABILIZATION GRADE 0.5% MIN. 10% MAX. . Seed and cover with Erosion Control Matting or line with sod. 4" — 7" stone or recycled concrete equivalent pressed into the soil 7" minimum. CONSTRUCTION SPECIFICATIONS All temporary earth dikes shall have uninterrupted positive grade to an outlet. Spot elevations may be necessary for grades less than 1%. Runoff diverted from a disturbed area shall be conveyed to a sediment trapping device.

DETAIL 1 - EARTH DIKE

Runoff diverted from an undisturbed area shall outlet directly into an undisturbed, All trees, brush, stumps, obstructions, and other objectional material shall be remove and disposed of so as not to interfere with the proper functioning of the dike.

. The dike shall be excavated or shaped to line, grade and cross section as required to meet the criteria specified herein and be free of bank projections or other irregularities which will impede normal flow. 5. Fill shall be compacted by earth moving equipment. All earth removed and not needed for construction shall be placed so that it will not interfere with the functioning of the dike. 3. Inspection and maintenance must be provided periodically and after each rain event.

SUPER SILT FENCE

CONSTRUCTION SPECIFICATIONS

Chain link fence shall be fastened securely to the fence posts with wire ties. The lower tension wire, brace and truss rods, drive anchors and post caps are not required except on the ends of the fence.

Filter cloth shall be fastened securely to the chain link fence with ties spaced every 24" at the top and mid section.

When two sections of filter cloth adjoin each other, they shall be overlapped by 6" and folded.

50 lbs/in (min.) 20 lbs/in (min.) 0.3 gai/ft /minute (max.)

200 feet

100 feet

50 feet

SUPER SILT FENCE DESIGN CRITERIA

Filter cloth shall be embedded a minimum of 8" into the ground.

10:1 - 5:1

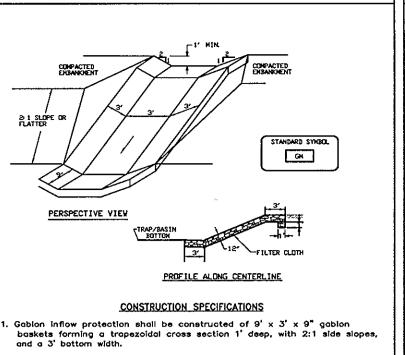
5:1 - 3:1

2:1 +

Fencing shall be 42" in height and constructed in accordance with the latest Maryland State Highway Details for Chain Link Fencing. The specification for a 6' fence shall be used, substituting 42" fabric and 6' length posts.

DETAIL 5 - RIP-RAP INFLOW PROTECTION 10' MINIMUL RRP TRAP/BASIN BOTTOM PERSPECTIVE VIEW 2 1 2 CROSS SECTION CONSTRUCTION SPECIFICATIONS Filter cloth shall be installed under all rip—rap. Filter cloth shall be Geotextile Class

Rip—Rap used for the lining may be recycled for permanent outlet protection if the basin is to be converted to a stormwater management facility. Gabien inflow Protection may be used in lieu of Rip-Rap Inflow Protection.



DETAIL 6 - GABION INFLOW PROTECTION

Geotextile Ciass C shall be installed under all gabion baskets

3. The stone used to fill the gabion baskets shall be 4'' - 7''. Gabions shall be installed in accordance with manufacturers recommendation 5. Gabion inflow Protection shall be used where concentrated flow is present on slopes steeper than 4:1.

BASIN DRAWDOWN SCHEMATIC

VERTICAL DRAW-DOWN DEVICE

PAGE MARYLAND DEPARTMENT OF ENVIRONMEN
B - 7 - 2 WATER MANAGEMENT ADMINISTRATION

⊠ æs Perforated (removable)
12" - 36" pipe wropped w/ 1/2"
hardware cloth and Geotextile
Class 'E' ANTICIPATED WATER SURFACE ELEV. CLEAN GRAVEL ELEVATION (CUT AWAY) Construction Specifications

DETAIL 20A - REMOVABLE PUMPING STATION

 The outer pipe should be 48° dia, or shall, in any case, be at least 4° greater in diameter than the center pipe. The outer pipe shall be wrapped with 1/2° hardware cloth to prevent backfill material from entering the perforations. 3. The inside stand pipe (center pipe) should be constructed by perforating a corrugated or PVC pipe between 12° and 38° in diameter. The perforations shall be 1/2° X 6° sits or 1° diameter holes 6° on center. The center pipe shall be wrapped with 1/2° hardware cloth first, then wrapped again with Geotextile Class C.

SCE SCE Construction Specifications 1. Length - minimum of 50" (+30" for single residence lot). 2. Width- 10' minimum, should be flored at the existing road to provide a turning Geotextile fabric (filter cloth) shall be placed over the existing ground prior to placing stone, ==The plan approval authority may not require single family residence to use geotextile. Stone - crushed aggregate (2" to 3") or reclaimed or recycled concrete equivolent shall be placed at least 6" deep over the length and width of the entrance. 5. Surface Water — all surface water floring to an diverted toward construction entrances shall be piped through the entrance, maintaining positive drainage. Pipe installed through the stabilized construction entrance shall be protected with a mounted berg with 5:1 slopes and a minimum of 5" of stone over the pipe. Pipe has to be sized according to the drainage. When the SCE is located at a high spot and has no drainage to convey a pipe will not be necessary. Pipe should be sized according to the amount of runoff to be conveyed. A 5" minimum will be required. Location — A stabilized construction entrance shall be located at every point where construction traffic enters or leaves a construction site. Vehicles leaving the site must travel over the entire length of the stabilized construction entrance.

OR BETTER CLASS "C"

DETAIL 24 - STABILIZED CONSTRUCTION ENTRANCE

PROFILE

950' MINIMUM LENGTH

- EXISTING PAVEMEN

EARTH FILL PIPE AS NECESSARY

4. The center pipe should extend 12" to 18" above the anticipated water surface DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT SOIL CONSERVATION SERVICE D - 12 - 5 WATER MANAGEMENT ADMINISTRATION S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT SOIL CONSERVATION SERVICE F - 17 - 3 WATER MANAGEMENT ADMINISTRATION

DETAIL 14 - TYPICAL ANTI-SEEP COLLARS

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws

License No. 21443 _. Expiration Date: 12-21-12

No As-Built information is required on this sheet

BY THE DEVELOPER: "I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE ACCORDING TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS—BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION I. ALSO AUTHORIZE PERIODIC ON—SITE INSPECTIONS BY THE HOWARD SOIL DAYS OF COMPLETION. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTIONS BY THE HOWARD SOIL CONSERVATION DISTRICT."

CASCAGE OUERLOOK LLC BY: Itam MEMBER STEVEN K BREEDEN

BY THE ENGINEER:

1/WE CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL "I/WE CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, ENCOSION AND SEDIMENT CONTROL
REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE
CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD
SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A
REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD
CONSERVATION DISTRICT WITH ALL AND PROVIDE THE POND WITHIN AD DAYS OF

- DONALD A. MASON, P.E. # 2144 THESE PLANS HAVE BEEN REVIEWED FOR THE HOWARD SOIL CONSERVATION DISTRICT AND MEET THE

THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION, AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. REQUIREMENTS OF THE

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

Willia I. whole 12-24-03 CHIEF, BUREAU OF HIGHWAYS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

REVISION

CHIEF, DIVISION OF LAND DEVELOPMENT CHIEF. DEVELOPME

BENCHMARK ENGINEERS ▲ LAND SURVEYORS ▲ PLANNERS

8480 BALTIMORE NATIONAL PIKE ▲ SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105

11/17/03

12/14/03

1/13/04

OWNER/DEVELOPER CASCADE OVERLOOK, L.L.C. P.O.BOX 417 ELLICOTT CITY, MD 21041 (410) 465-4244

CRAIG R. AND KAREN C. MARTIN

LOTS 1 - 72 AND OPEN SPACE LOTS 73 - 80 AND LOCATION:

GRADING, SEDIMENT AND EROSION CONTROL DETAILS VP-86-130, F-88-20, S-01-04, PB-359, P-02-11 OCTOBER, 2003 PROJECT NO. 1383

CASCADE OVERLOOK

AS-BUILT

The riser shall have a base attached with a watertight connection and shall have sufficient weight to prevent flotation of the riser. Two approved bases for risers 10' or less in height are: 1. A concrete base 18' thick with the riser embedded 9' in the base. 2. A 1/4" minimum thickness steel plate attached to the riser by a continuous weld around the circumference of the riser to form a watertight connection. The plate shall have 2" of stone, gravel, or compacted earth placed on it to prevent flotation. In either case, each side of the square base shall be twice the riser diameter. Note: For risers greater than ten feet high computations shall be made to design a base which will prevent floatation. The minimum factor of safety shall be 1.20 (downward forces = 1.20 x upward forces). U.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT U.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT SOIL CONSERVATION SERVICE C - 9 - 70 WATER MANAGEMENT ADMINISTRATION SOIL CONSERVATION SERVICE C - 9 - 70 WATER MANAGEMENT ADMINISTRATION U.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT SOIL CONSERVATION SERVICE C - 10 - 25 WATER MANAGEMENT ADMINISTRATION

21' CMP

U.S. DEPARTMENT OF AGRICULTURE
SOIL CONSERVATION SERVICE

DETAIL 16 - CONCENTRIC TRASH RACK AND ANTI-VORTEX DEVICE AND ANTI-VORTEX DEVICE (cont'd) 16 ga. 16 ga. 42 16 15 #6 Rebar 14 17 #8 Rebar 42' TRASH RACK 60 14 19 12 21 1-1/4" pipe or 1-1/4 x 1-1/4 x1/4 10 მთ 78 12 25 MINIMUM 1-1/2' pipe or 1-1/2 x 1-1/2 x1/4 90 12 29 8 50 RISER 27' Ø 8 ga., 2x2x1/4 w/stiffener angle 2" pipe or 2x2x3/16 angle SECTION A-A 1/2×1/4 angle SEE DESIGN TABLE TO RIGHT FOR DIMENSIONS SUPPORT BAR SIZE 3/4 DIAMETER MINIMUM, BARS ARETO BE VELDED TO THE TOP OF THE RISER OR ATTACHED BY STRAPS TO THE TOP OF THE RISER 2-1/2 x2-1/2x 2-1/2x2-1/2x1/4 5/16 angle Note: The above trash rack and anti-vortex device information is only for corrugated metal pipe. Concrete risers must neet the requirements MARYLAND DEPARTMENT OF ENVIRONMENT
WATER MANAGEMENT ADMINISTRATION DETAIL 8 - PIPE OUTLET SEDIMENT TRAP - ST 1

PIPE OUTLET SEDIMENT TRAP - ST 1 (cont'd) Construction operations shall be carried out in such a manner that erosion and water pollution are abated. Dice constructed, the top and outside face of the embandment shall be stabilized with seed and mulch. Points of concentrated inflow shall be protected in accordance with Grade Stabilization Structure criteria. The renainder of the interior slopes should be stabilized (one time) with seed and mulch upon trap completion and onitored and maintained erosion free during the life of the trap.

cloth shall be replaced as necessary to prevent clogging.

3. The total trap volume as measured from the bottom to riser crest elevation shall be 3600 cubic feet per acre of oralinage area (see Table 9). The top of embarkment must be ≥ 1' above the riser crest elevation.

19. Where discharge occurs at the property line, local ordinances and drainage easement requirements shall be met.

DES: DAM | DRN: RPS

PERMANENT SEEDBED NOTES. \\tsa\vol1\engineer\projects\1383 PFAU\dwg\7023S19.dwg, SHEET 19, 11/14/2003 1:32:53 PM

must be added to meet the requirements prior to use. c. Composted sludge shall be applied at a rate of 1 ton/1,000 square feet.

stabilized area at a non-erosive velocity.

10 - 20%

20 - 33%

D.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT U.S. DEPARTMENT OF AGRICULTURE PAGE MARYLAND DEPARTMENT OF ENVIRONMENT SOIL CONSERVATION SERVICE F - 26 - 34 WATER MANAGEMENT ADMINISTRATION SOIL CONSERVATION SERVICE F - 26 - 34 WATER MANAGEMENT ADMINISTRATION

. Rip—Rap should blend into existing ground.

. Rip-Rap Inflow Protection shall be used where the slope is between 4:1 and 10:1, for slopes flatter than 10:1 use Earth Dike or Temporary Swale lining criteria.

5. DEPARTMENT OF AGRICULTURE | PAGE | MARYLAND DEPARTMENT OF ENVIRONMEN SOIL CONSERVATION SERVICE | B - 6 - 2 | WATER MANAGEMENT ADMINISTRATION

TREE PROTECTION FENCE

BLAZE ORANGE PLASTIC MESH MIN. 2" STEEL "U" CHANNEL HIGHLY MAIBLE FLAGGING USE 2"x4" LUMBER FOR CROSS BRACIN

ANCHOR POSTS MUST BE
INSTALLED TO A DEPTH OF
NO LESS THAN 1/3 OF
THE TOTAL HEIGHT OF THE POST. NOTES:). FOREST PROTECTION DEVICE ONLY. 2. RETENTION AREA WILL BE SET AS PART OF THE REVIEW PROCESS. 3. BOUNDARIES OF RETENTION AREA SHOULD BE STAKED AND FLAGGED PRIOR TO INSTALLING DEVICES. A AVOID ROOT DAMAGE WHEN PLACING ANCHOR POSTS.

DEMCE SHOULD BE PROPERLY MAINTAINED DURING CONSTRUCTION

PROTECTIVE SIGNAGE IS ALSO REQUIRED.

TOP OF DAM LIMIT OF DRY STORAGE --PLAN VIEW

TOP OF DAM

RISER CREST ELEVATION

--- SEE NOTE 4 INTERNAL ORIFICE TRASH RACK / ANTI-VORTEX DEVICE PRINCIPAL SPILLWAY ELEVATION RISER BASE CIPAL SPILLWAY _TRACK RACK Construction Specifications . Perforations in the draw-down device may not extend into the wet storage. 2. The total area of the perforations must be greater than 2 times the area of the internal orifice. The perforated portion of the draw-down device shall be wrapped with 1/2° hordware cloth and geotextile fabric. The geotextile fabric shall meet the specifications for Geotextile Class E.

COLLAR WELDED IN PLACE ON BARREL SECTION STAINLESS STEEL NUT AND BOLT CONNECTION WITH MASTIK BETWEEN PLATES USE "MASTIK" O EQUIVALENT BETWEEN PLATE AND FRAME

D - DISTANCE BETWEEN INFLOW AND OUTFLOW

L. TOTAL DISTANCE FROM THE INFLOW POINT AROUND THE BAFFLES TO THE RISER

DETAIL 18 - SEDIMENT BASIN BAFFLES

INSTALL COLLAR WITH CORRUGATIONS VERTICAL

ANTI-SEEP COLLAR DESIGN COLLAR FOR FLANGE JOINT PIPE

DETAIL 16 - CONCENTRIC TRASH RACK

PERSPECTIVE VIEW

18" HINIMUM THICKNESS TO HINIMUM LENGTH VATERTIGHT BANDS

GEOTEXTILE CLASS C

with equipment while it is being constructed.

EMBANKMENT SECTION

THROUGH RISER

2. The fill naterial for the embankment shall be free of roots or other woody

vegetation as well as oversized stones, rocks, organic naterial, or other objectionable naterial. The embankment shall be compacted by traversing

4. Sedinent shall be removed and the trap restored to its original dimensions when the sediment has accumulated to one half of the wet storage depth of the trap (900cf/ac). The sediment shall be deposited in a suitable area and in such a nanner that it will not erode.

5. The structure shall be inspected periodically and after each rain and repairs

ANTI-SEEP
COLLAR

ANTI-SEEP
COLLAR

WET STORAGE

BOTTON

NOTE: RISER EMBEDDED 9' INTO CONCRETE OR 1/4' STEEL. PLATE ATTACHED TO RISER VITH A CONTINUOUS VELD ON BUTTOM AND 2' OF STONE PLACED ON STEEL PLATE

TVICE THE RISER DIAMETER

Silt Fence Length

Unlimited

1,500 feet

1,000 feet

250 feet

Topsoil shall be a loam, sandy loam, clay loam, silt loam, sandy clay loam, loamy sand. Other soils may be used if recommended by an agronomist or soil scientist and approved by the appropriate approval authority. Regardless, topsoil shall not be a mixture of contrasting texture subsoils and shall contain less than 5% by volume of cinders, stones, slag, coarse fragments, gravel, sticks, roots, trash, or other materials larger than $1-1/2^{\circ}$ in diameter. Topsoil must be free of plants or plant parts such as Bermuda grass, quack grass, Johnson grass, nutsedge, poison ivy, thistle, or others as specified.

iii. Where the subsoil is either highly acidic or composed of heavy clays, ground limestone shall be spread at the rate of 4-8 tons/acre (200-400 pounds per 1,000 square feet) prior to the placement of topsoil. Lime shall be distributed uniformly over designated areas and worked into the soil in conjunction with tillage operations are described in the following procedures. III. For sites having disturbed areas under 5 acres:

Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization — Section I — Vegetative Stabilization Methods and IV. For sites having disturbed areas over 5 acres:

TOPSOIL SPECIFICATIONS

I. Topsoil salvaged from the existing site may be used provided that it meets that standards as set forth in these specifications. Typically, the depth of topsoil to be salvaged for a given soil type can be found in the representative soil profile section in the Soil Survey published

y UŚDA—SCS in cooperation with Maryland Agricultural Experimental Station.

II. Topsoil Specifications - Soil to be used as topsoil must meet the following:

I. On soil meeting Topsoil specifications, obtain test results dictating fertilizer and lime amendments required to bring the soil into compliance with the following: a. pH for topsoil shall be between 6.0 and 7.5. If the tested soil demonstrates a pH of less than 6.0, sufficient lime shall be prescribed to raise the pH to 6.5 or higher.

b. Organic content or topsoil shall be not less than 1.5 percent by weight.

c. Topsoil having soluble salt content greater than 500 parts per million shall No sod or seed shall be placed on soil which has been treated with soil

sterilants or chemicals used for weed control until sufficient time has elapsed (14 days min.) to permit dissipation of phyto—toxic materials. Note: Topsoil substitutes or amendments, as recommended by a qualified agronomist a soil scientist and approved by the appropriate approval authority, may be used in lieu of

i. Place topsoil (if required) and apply soil amendments as specified in 20.0 Vegetative Stabilization — Section I — Vegetative Stabilization Methods and V. Topsoil Application When topsoiling, maintain needed erosion and sediment control practices such as

diversions, grade stabilization structures, earth dikes, slope silt fence and sediment Grades on the areas to be topsoiled, which have been previously established, shall be maintained, albeit 4" - 8" higher in elevation. iii. Topsoil shall be uniformly distributed in a 4" — 8" layer and lightly compacted to a minimum thickness of 4". Spreading shall be performed in such a manner that sodding or seeding can proceed with a minimum of additional soil preparation and

tillage. Any irregularities in the surface resulting from topsoiling or other operations shall be corrected in order to prevent the formation of depressions or iv. Topsoil shall not be placed while the topsoil or subsoil is in a frozen or muddy condition, when the subsoil is excessively wet or in a condition that may otherwise be detrimental to proper grading and seedbed preparation.

VI. Alternative for Permanent Seeding — Instead of applying the full amounts of lime and commercial fertilizer, composted sludge and amendments may be applied as specified Composted Sludge Material for use as a soil conditioner for sites having distributed areas over 5 acres shall be tested to prescribe amendments and for sites having disturbed areas under 5 acres shall conform to the following requirements:

a. Composted sludge shall be supplied by, or originate from, a person or persons that are permitted (at the time of acquisition of the compost) by the Maryland Department of the Environment under COMAR 26.04.06. Composted sludge shall contain at least 1 percent nitrogen, 1.5 percent phosphorus, and 0.2 percent potassium and have a pH of 7.0 to 8.0. If compost does not meet these requirements, the appropriate constituents

iv. Composted sludge shall be amended with a potassium fertilizer applied at the rate of 4 lb/1,000 square feet, and 1/3 the normal lime application rate. References: Guidelines Specifications, Soil Preparation and Sodding. MD-VA, Pub. #1, Cooperative Extension Service, University of Maryland and Virginia Polytechnic Institutes, Revised 1973.

10. Above the wet storage elevation, the riser shall be perforated with 1/2" wide by 6' long slits or 1' diameter holes spaced 6' vertically and horizontally. No perforations will be allowed within 6' of the horizontal barrel.

11. The riser shall be wrapped with 1/2' handware cloth (wire) then wrapped with Geotextile Class E. The filter cloth shall extend 6' above the highest slit and 6' below the lowest slit. Where ends of filter cloth come together, they shall be overlapped, folded and fastened to prevent bypass. Filter 12. Straps or connecting bands shall be used to hold the filter cloth and wine fabric in place. They shall be placed at the top and bottom of the cloth. 13. Fill naterial around the pipe spillway shall be hand compacted in 4' layers. A minimum of 2' of hand-compacted backfill shall be placed over the pipe spillway before crossing it with construction equipment. 14. The riser shall be anchored with either a concrete base or steel plate base to prevent flotation. Concrete bases shall be at least twice the riser diameter and 12' deep with the riser embedded 9'. Steel plate bases shall be at least twice the riser diameter, 1/4' ninimum thickness and attached to the bottom of the riser by a continuous weld to form a watertight connection. Then place 2' of stone, gravel or tamped earth on the plate.

15. Anti seep collars shall be constructed in accordance with plans (ref. table 16 and Details 13 and 14). 16. Concentric trash rack and anti-vortex device design details are on Detail 16. 17. Refer to Section D for dewatering requirements of sediment traps. the discharge in an erosion free manner to an existing stable channel.

PLACED AT RIGHT ANGLES IN

PROJECTING THROUGH SIDES

OF RISER TO HELP ANCHOR

RISER TO THE CONCRETE

BASE. RE-BARS TO PROJECT

HINIMUM OF 1/4 RISER

VETER BEYOND THE

VETER BEYOND THE

VETER BEYOND THE

VETER BEYOND THE

- RISER 27' CMP

DETAIL 15 - RISER BASE DETAIL

4937 LANDING ROAD ELKRIDGE, MD 21075

NO DATE

CHK: DAM

F-03-134

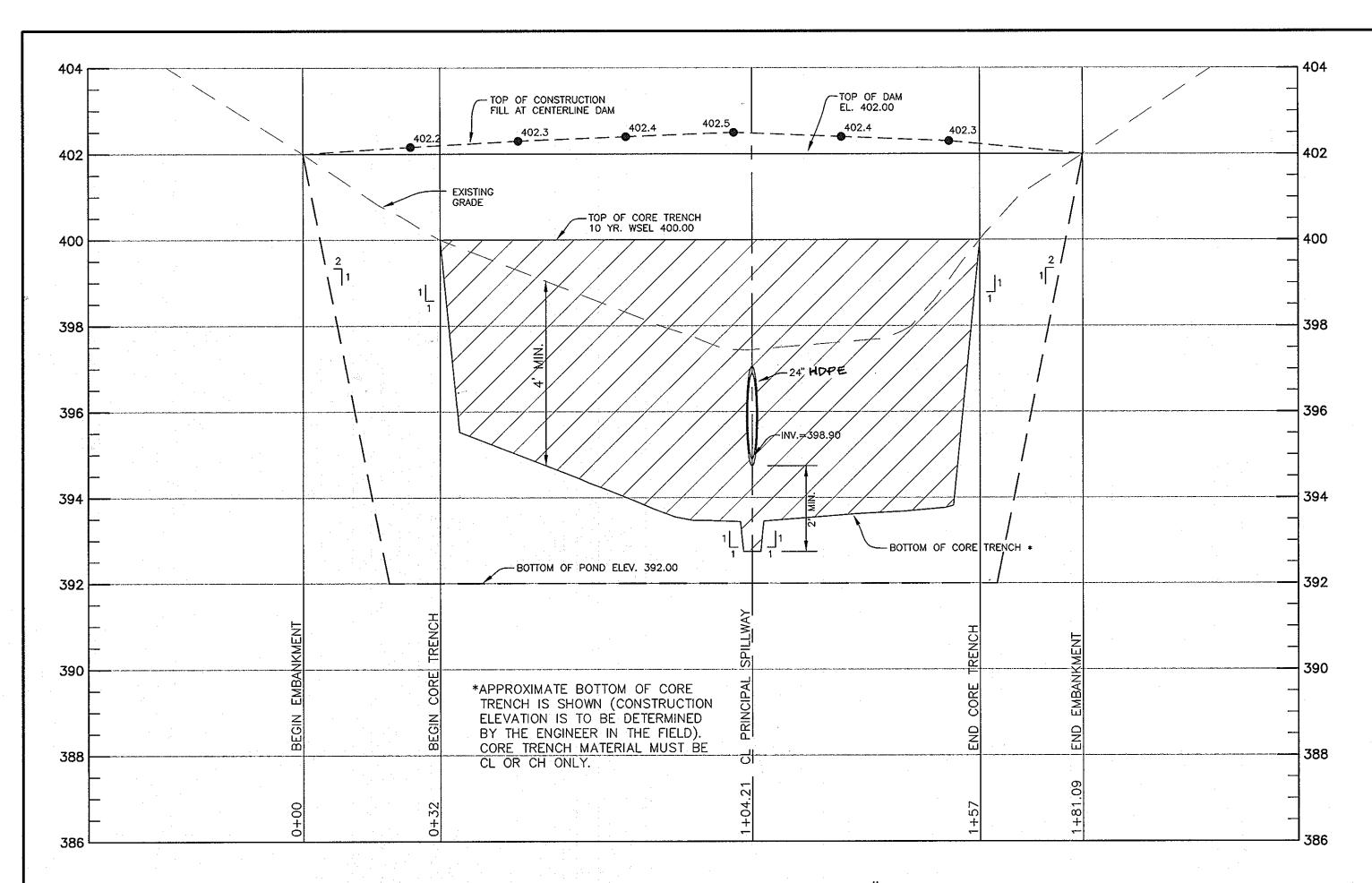
DATE

FAX: 410-465-6644

TAX MAP 31, GRID 10 &11, PARCELS 160, 161, 788, & 791 1 st. ELECTION DISTRICT HOWARD COUNTY, MARYLAND

SCALE: AS SHOWN

DRAWING 19 OF 33



PROFILE ALONG CL EMBANKMENT BASIN #4

HORIZONTAL: 1'' = 20'VERTICAL: 1"=2'

CONSTRUCTION SPECIFICATIONS FOR SEDIMENT BASINS

BY THE DEVELOPER: "I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE ACCORDING TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION. I ALSO AUTHORIZE PERIODIC ON—SITE INSPECTIONS BY THE HOWARD SOIL CONSERVATION DISTRICT." CASCADE OVERWORLL Itzyll MEMBER DEVELOPER

STEVEN K BREEDEN

BY THE ENGINEER: "I/WE CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF

11/17/03

THESE PLANS HAVE BEEN REVIEWED FOR THE HOWARD SOIL CONSERVATION DISTRICT AND MEET THE TECHNICAL REQUIREMENTS FOR SMALL POND CONSTRUCTION, SOIL EROSION, AND SEDIMENT CONTROL. 12/14/03

THESE PLANS FOR SMALL POND CONSTRUCTION, SOIL EROSION, AND SEDIMENT CONTROL MEET THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT.

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

12-24-03 APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

Condy Hamilie

1. Site Preparation: Perimeter sediment control devices must be installed prior to clearing and grubbing. Areas where the embankment is to be placed shall be cleared, grubbed, and stripped of topsoil to remove trees, vegetation, roots or other objectionable material. The pool area shall not be cleared until completion of the dam embankment unless the pool area is to be used for borrow. In order to facilitate clean-out and restoration, the pool area (measured at the top of the pipe spillway) shall be cleared of all brush, trees, and other objectionable materials.

2. Cut-off Trench: A cut-off trench shall be excavated along the centerline of earth fill embankments. The minimum depth shall be four feet. The cut-off trench shall extend up both abutments to the riser crest elevation. The minimum bottom width shall be two feet, but wide enough to permit operation of excavation and compaction equipment. The side slopes shall be no steeper than 1:1. Compaction requirements shall be the same as those for the embankment. The trench shall be dewatered during the backfilling-compaction operations. For dewatering see Section D.

3. Embankment: The fill material shall be taken from approved areas shown on the plans. It shall be clean mineral soil free of roots, woody vegetation, oversized stones, rocks, or other objectionable material. Relatively pervious materials such as sand or gravel (Unified Soil Classes GW, GP, SW & SP) or organic materials (Unified Soil Classes OL and OH) shall not be placed in the embankment. Areas on which fill is to be placed shall be scarified prior to placement of fill. The fill material shall contain sufficient moisture so that it can be formed by hand into a ball without crumbling. If water can be squeezed out of the ball, it is too wet for proper compaction. Fill material shall be placed in six-inch to eight-inch thick continuous lifts over the entire length of the fill. Compaction shall be obtained by routing and hauling the construction equipment over the fill so that the entire surface of each layer of the fill is traversed by at least one wheel or tread track of the equipment or by the use of a compactor. The embankment shall be constructed to an elevation 10 percent higher than the design height to allow for settlement.

4. Principal Spillway: Steel risers shall be securely attached to the barrel or barrel stub by welding the full circumference making a watertight structural connection. Concrete risers shall be poured with the principal spillway in place or precast with voids around the principal spillway filled with concrete or shrink proof grout for watertight connection. The barrel stub must be attached to the riser at the same percent (angle) of grade as the outlet conduit. The connection between the riser and the riser base shall be watertight. All connections between barrel sections must be achieved by approved watertight band assemblies. The barrel and riser shall be placed on a firm, smooth foundation of impervious soil as the embankment is constructed. Breaching the embankment to install the barrel is unacceptable. Pervious materials such as sand, gravel, or crushed stone shall not be used as backfill around the pipe or anti-seep collars. The fill material around the pipe spillway shall be placed in four inch lifts and hand compacted under and around the pipe to at least the same density as the adjacent embankment. A depth of 1.5 times the pipe diameter (min.) shall be backfilled over the principal spillway and hand compacted before crossing it with construction equipment.

5. Emergency Spillway: The emergency spillway shall be installed in undisturbed ground. The achievement of planned elevations, grades, design width, entrance and exit channel slopes are critical to the successful operation of the emergency spillway and must be constructed within a tolerance of + 0.2 feet.

6. Vegetative Treatment: Stabilize the embankment in accordance with the appropriate vegetative Standard and Specifications immediately following construction. In no case shall the embankment remain unstabilized for more than seven (7) days. Once constructed, the top and outside face of the embankment shall be stabilized with seed and mulch. The remainder of the interior slopes should be stabilized (one time) with seed and mulch upon basin completion and monitored and maintained erosion free during the life of the basin.

7. Safety: Local requirements concerning fencing and signs shall be met, warning the public of hazards of soft sediment and floodwater.

8. Maintenance: Repair all damage caused by soil erosion and construction equipment at or before the end of each working day. Sediment shall be removed from the basin when it reaches the specified distance below the top of the riser as shown on the riser. This sediment shall be placed in such a manner that it will not erode from the site. The sediment shall not be deposited downstream from the embankment, adjacent to a stream or floodplain. Disposal areas must be stabilized.

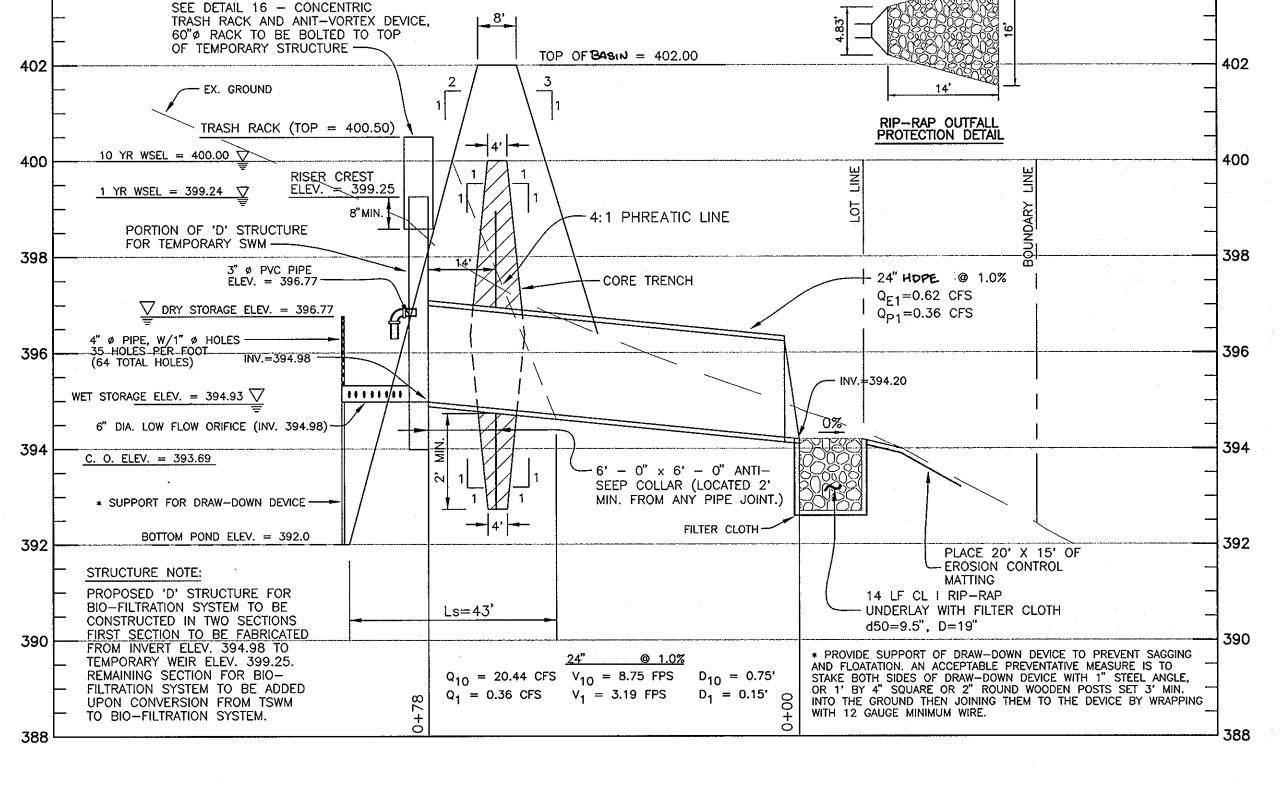
9. Final Disposal: When temporary structures have served their intended purpose and the contributing drainage area has been properly stabilized, the embankment and resulting sediment deposits are to be leveled or otherwise disposed of in accordance with the approved sediment control plan. The proposed use of a sediment basin site will often dictate final disposition of the basin and any sediment contained therein. If the site is scheduled for future construction, then the basin material and trapped sediments must be removed and safely disposed of and the basin shall be backfilled with a structural fill. When the basin area is to remain open space, the pond may be pumped dry (using methods in Section D - Dewatering), graded, and back filled.

10. Conversion to Stormwater Management Structure: After permanent stabilization of all disturbed contributory drainage areas, temporary sediment basins, if initially built and certified to meet permanent standards, may be converted to permanent stormwater management structures. To convert the basin from temporary to permanent use, the outlet structure must be modified in accordance with approved stormwater management design plans. Additional grading may also be necessary to provide the required storage volume in the basin. Conversion can only take place after all disturbed areas have been permanently stabilized to the satisfaction of the inspection authority and storm drains have been flushed.



Professional Certification | thereby certify that these documents were prepared or approved by me, and that I am a duly licensed processional engancer under the laws of the State of Maryland. License No. 21443 . Expiration Date: 12-21-12

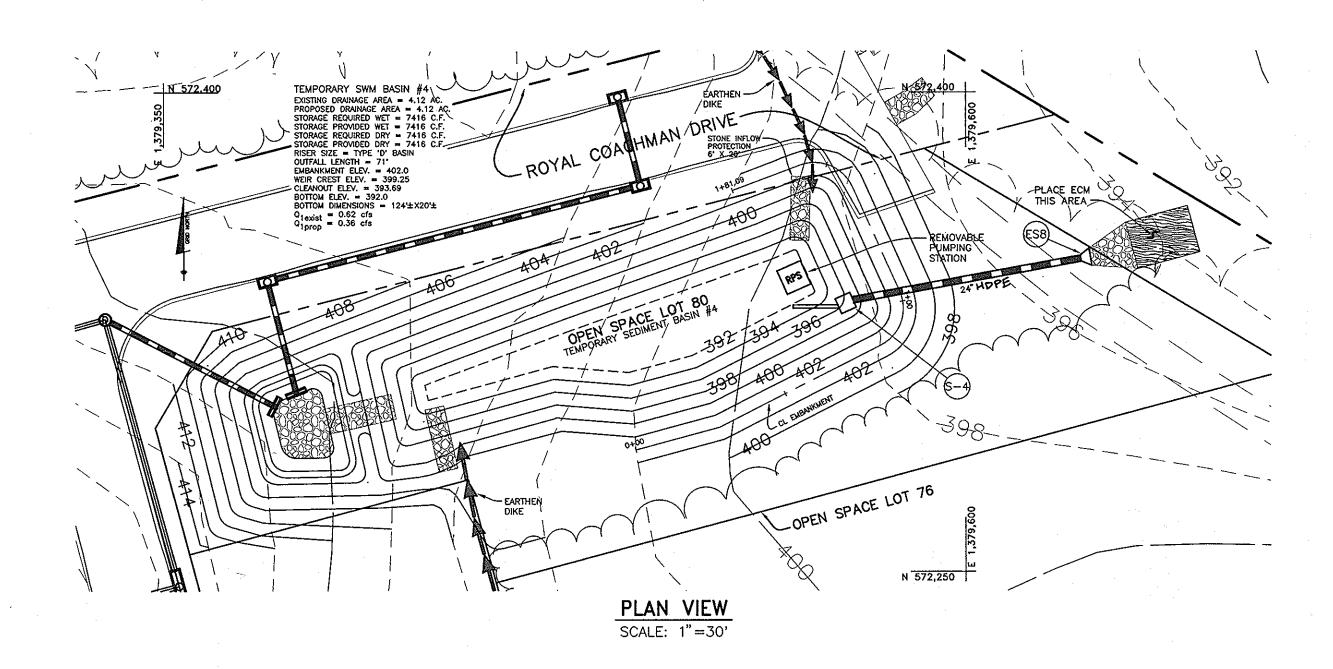
No As-Built information is required on this sheet

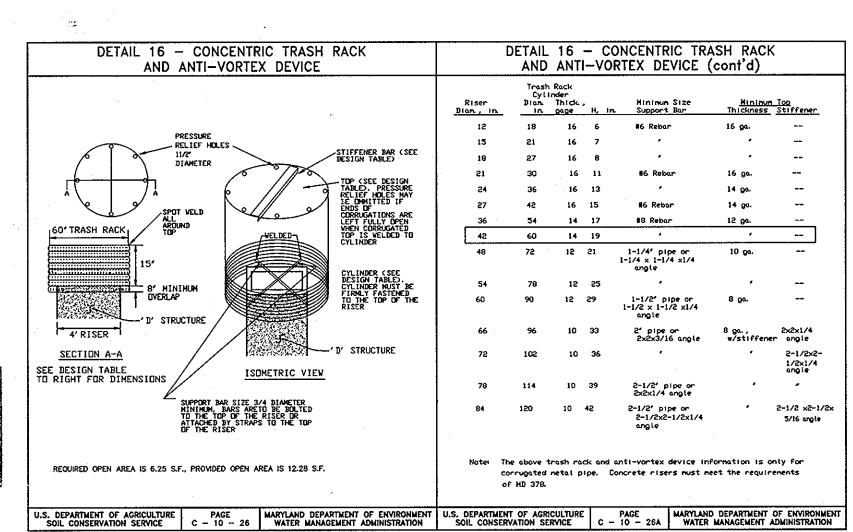


NOTE: 3"Ø PVC PIPE FOR TEMPORARY SWM TO BE CUT OFF AND CAPPED AFTER CONVERSION TO BIO-FILTRATION FACILITY

TEMP. SWM BASIN #4 EMBANKMENT SECTION THRU RISER

HORIZONTAL: 1''=20'VERTICAL: 1"=2





2 5-25-2011 REVISE PIPE FROM ESS TO SA TO BE HOPE 1 5-25-04 REVISE HDPEP TO RCCP CL.TV NO DATE REVISION **BENCHMARK** ENGINEERS & LAND SURVEYORS & PLANNERS ENGINEERING, INC. 8480 BALTIMORE NATIONAL PIKE ▲ SUITE 418 ELLICOTT CITY, MARYLAND 21043 PHONE: 410-465-6105 FAX: 410-465-6644 CASCADE OVERLOOK OWNER/DEVELOPER SECTION ONE

LOTS 1 - 72 AND OPEN SPACE LOTS 73 - 80 AND

NON-BUILDABLE PARCEL 'A' CASCADE OVERLOOK, L.L.C. P.O.BOX 417 LOCATION: ELLICOTT CITY, MD 21041 TAX MAP 31, GRID 10 &11, PARCELS 160, 161, 788, & 791

ASBUILT F-03-134

SCALE: AS SHOWN

1 st. ELECTION DISTRICT HOWARD COUNTY, MARYLAND

TEMPORARY SWM BASIN #4

OCTOBER, 2003 PROJECT NO. 1383

DRAWING 20 OF 33

VP-86-130, F-88-20, S-01-04, PB-359, P-02-11

(410) 465-4244

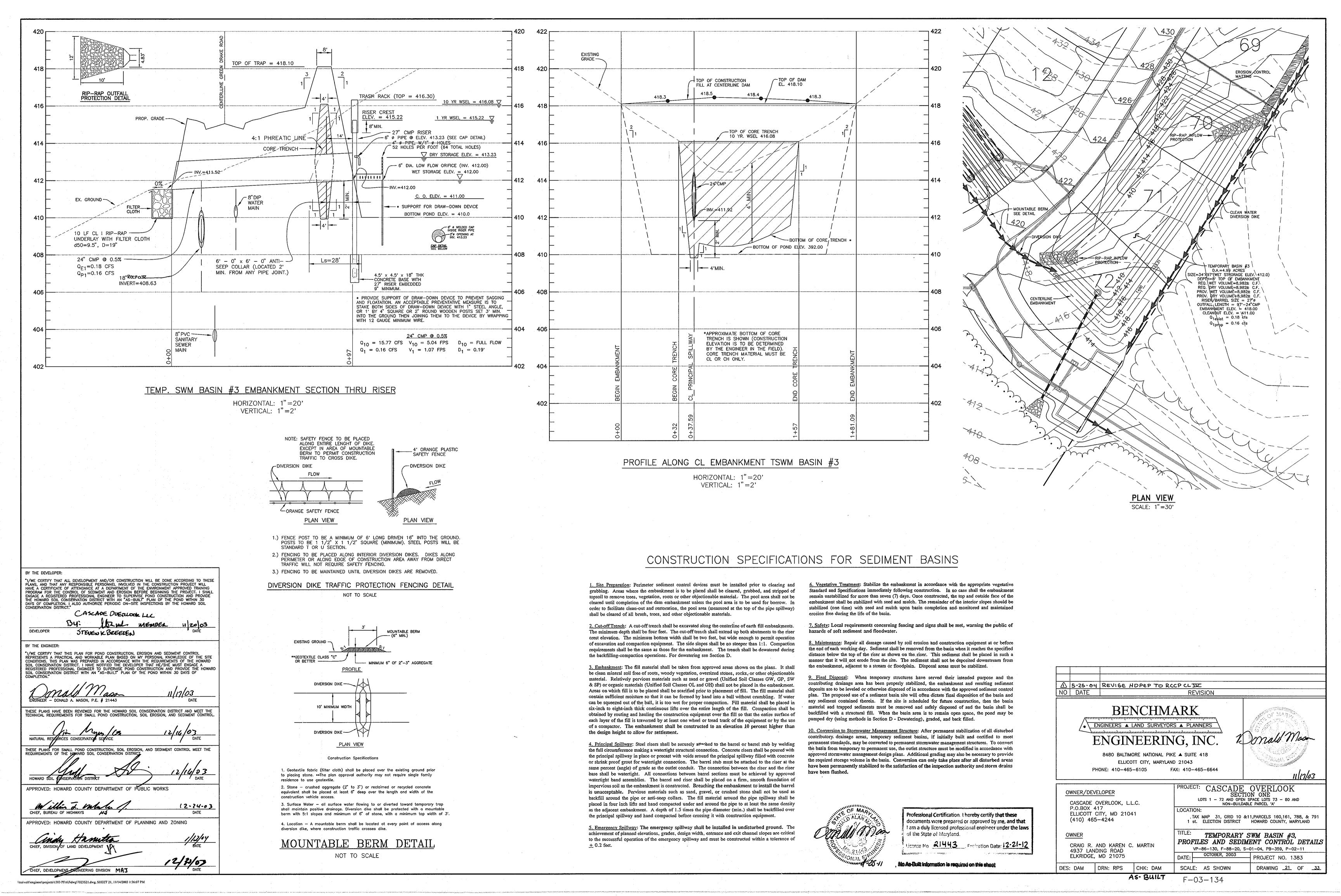
4937 LANDING ROAD

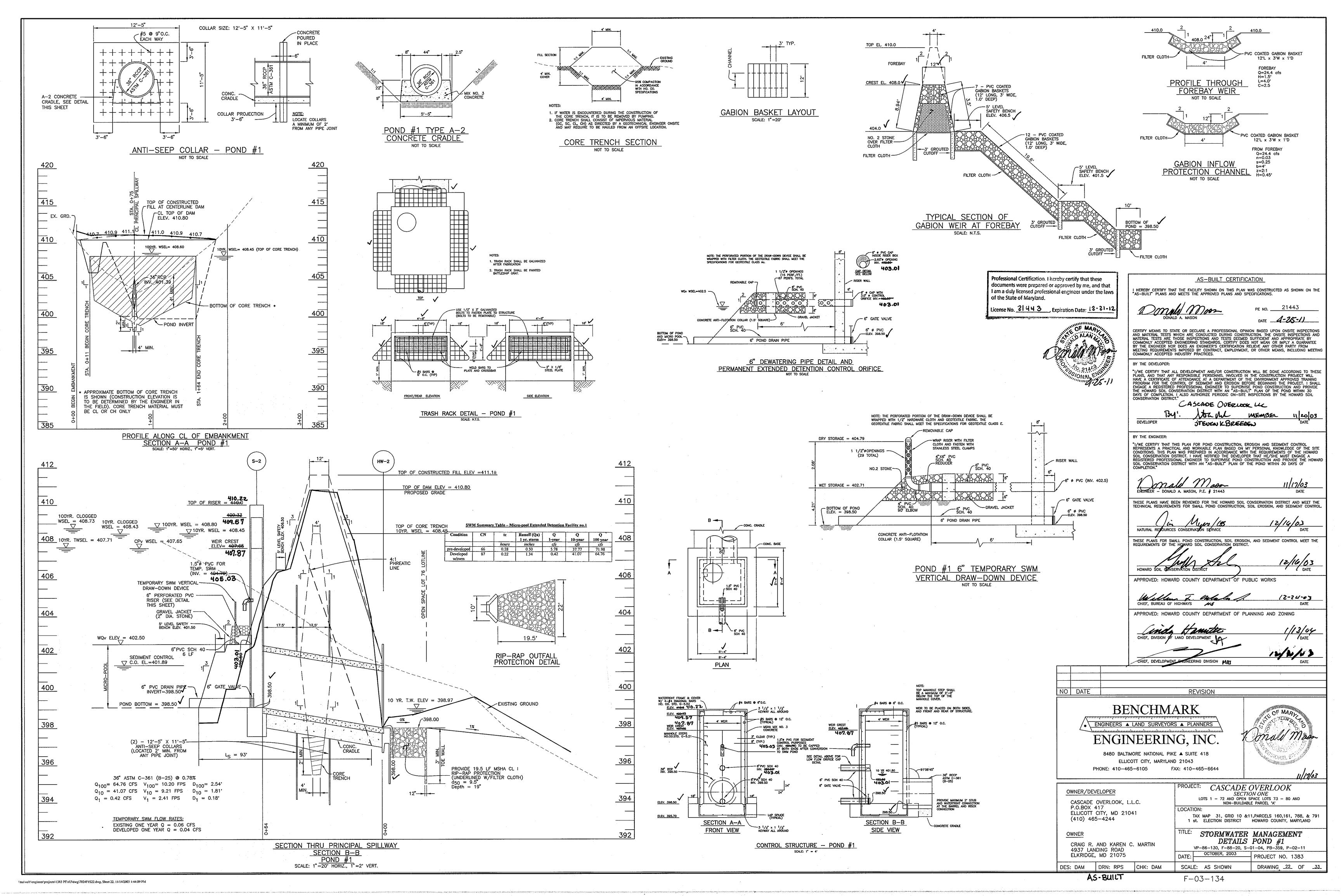
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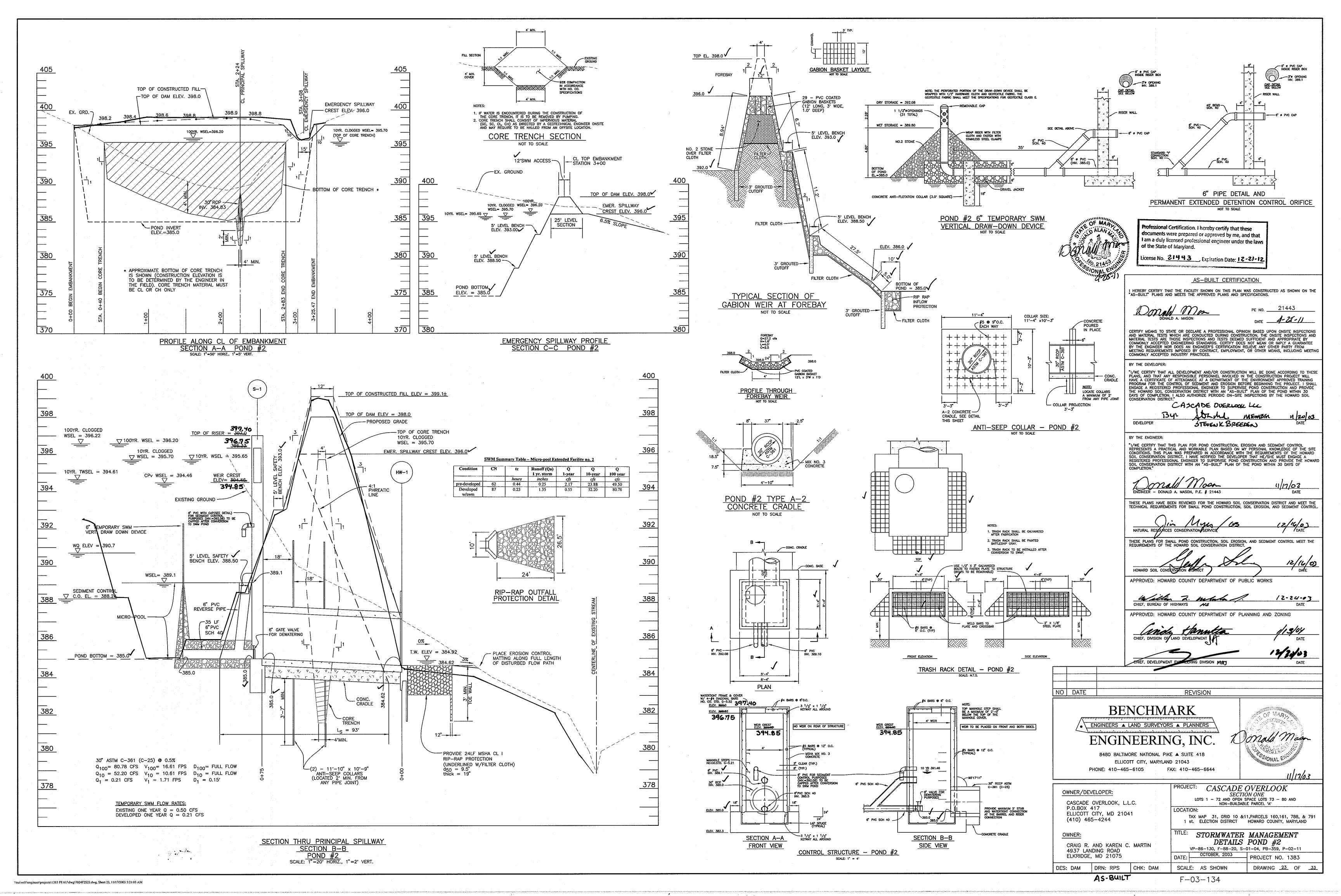
CRAIG R. AND KAREN C. MARTIN

DES: DAM DRN: RPS CHK: DAM

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MGWC 1.4: Diversion Pipe

Temporary measure for dewatering in-channel construction sites

The work should consist of installing flow diversion pipes in combination with sandbag or stone diversions when construction activities occur within the stream channel.

Effective Uses & Limitations

Diversion pipes with an insufficient flow capacity can cause the channel diversion to fail thereby resulting in severe erosion of the disturbed channel section under construction. Therefore, in—channel construction activities should occur only during periods of low flow

Material Specifications

Materials for stream diversions shoulld meet the following requirements: * Riprap: Stone should be washed and have a minimum diameter of 6 inches (15 centimeters).

resistant to puncture and tearing.

 Sandbags: Sandbags should consist of materials which are resistant to ultra-violet radiation. earing, and puncture and should be woven tightly enough to prevent leakage of fill material (i.e., sand, fine gravel, etc.).

* Sheeting: Sheeting should consist of polyethylene or other material which is imprevious and

Installation Guidelines

All erosion and sediment control devices including mandatory filter bags should be installed as the first order of business according to a plan approved by the WMA or local authority. Installation should proceed from upstream to downstream during low flow conditions. If necessary, silt fence or strow bales should be installed around the perimeter of the work area.

Diversion pipes with sandbag or stone barriers should be completed as follows (refer to detail 1.4): 1. Sandbag/stone barriers should be sized and installed as detailed in MCWC 1.5: Sandbag/Stone

- Diversion. The materials should be sized to withstand baseflow velocities. 2. All excavated material should be deposited and stabilized in an approved area outside the
- 00-year floodplain unless otherwise authorized by the WMA. 3. Sediment-laden water from the construction area should be pumped to a filter bag.
- 4. The diversion pipe should have a minimum capacity sufficient to convey the 2-year flow for projects with a duration of two weeks or greater. For projects of shorter duration, the
- capacity of the pipe can be reduced accordingly. 5. If necessary, silt fence or straw bales should be installed around the perimeter of the work
- 6. Sediment control devices are to remain in place until all disturbed areas are stabilized and

MGWC 4.2: Utility Crossing

Temporary in-stream construction

The work should consist of installing erosion control devices in and adjacent to the construction

Installation Guidelines

All erosion and sediment control devices, including filter bags should be implemented as the first order of business according to a plan approved by the WMA or local authority. (See the 1994 Maryland Standards and Specifications for Soil Erosion and Sediment Control.) The proposed construction sequence is as follows (refer to Detail 4.2):

- 1. The contractor should insure that a continuous perimeter control barrier is in place to minimize the amount of pollutants entering the flow. A diversion pipe as shown in MGWC 1.4: Diversion Pipe or other measure should be installed and sandbag or stone barriers as shown in MGWC
- 3. All construction should take place during stream low flows. The length of construction time
- should be limited to a maximum of 5 consecutive days for each crossing. 4. All utility crossings should be placed a minimum of 3 feet (1 meter) beneath the stream bec unless an alternative section is specifically approved by the WMA. For instances where a 3-fool

cover is not viable, two alternate stabilization options are given in the Detail 4.2. A low flow

2. 1.5: Sandbag/Stone Diversion should be constructed according to specifications to divert the

- channel shall be constructed through all riprap placements across the stream bed area should be dewatered, and any disturbed banks should be stabilized. The contractor may elect to construct the utility crossing in two stages. In this case, a WMA approved flow barrier
- 6. Once the crossing is completed, the diversion should be removed from upstream to downstream. Sediment control devices, including perimeter erosion controls, are to remain in place until all disturbed areas are stabilized in accordance with an approved sediment and erosion control plan
- 7. Construction vehicles to cross existing stream by utilizing existing driveway west of utility crossing. By utilizing this existing driveway, eliminates the need to construct temporary vehicle crossing to install utility and minimize impact to stream and wetlands.

"I/WE CERTIFY THAT ALL DEVELOPMENT AND/OR CONSTRUCTION WILL BE DONE ACCORDING TO THESE PLANS, AND THAT ANY RESPONSIBLE PERSONNEL INVOLVED IN THE CONSTRUCTION PROJECT WILL HAVE A CERTIFICATE OF ATTENDANCE AT A DEPARTMENT OF THE ENVIRONMENT APPROVED TRAINING PROGRAM FOR THE CONTROL OF SEDIMENT AND EROSION BEFORE BEGINNING THE PROJECT. I SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF COMPLETION. I ALSO AUTHORIZE PERIODIC ON-SITE INSPECTIONS BY THE HOWARD SOIL CONSERVATION DISTRICT."

CASCADE OVERLOOK LCC DEVELOPER STEVEN KBREEDEN

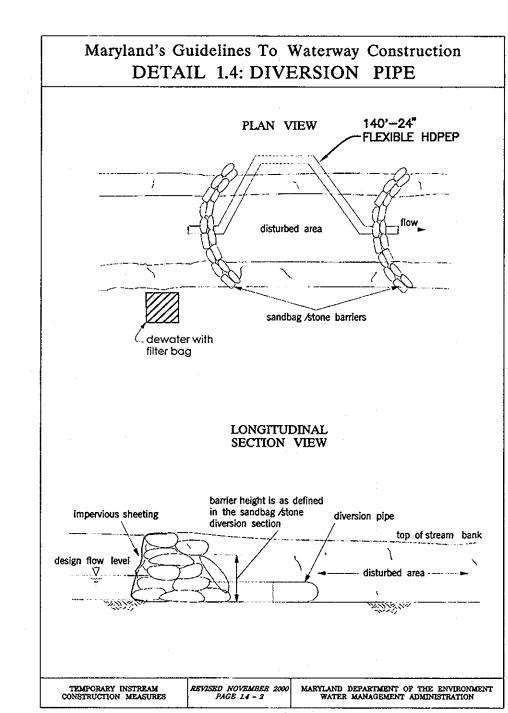
"I/WE CERTIFY THAT THIS PLAN FOR POND CONSTRUCTION, EROSION AND SEDIMENT CONTROL REPRESENTS A PRACTICAL AND WORKABLE PLAN BASED ON MY PERSONAL KNOWLEDGE OF THE SITE CONDITIONS. THIS PLAN WAS PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF THE HOWARD SOIL CONSERVATION DISTRICT. I HAVE NOTIFIED THE DEVELOPER THAT HE/SHE MUST ENGAGE A REGISTERED PROFESSIONAL ENGINEER TO SUPERVISE POND CONSTRUCTION AND PROVIDE THE HOWARD SOIL CONSERVATION DISTRICT WITH AN "AS-BUILT" PLAN OF THE POND WITHIN 30 DAYS OF

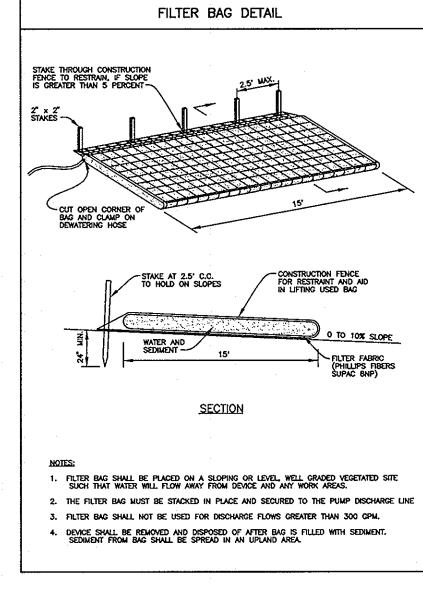
THESE PLANS HAVE BEEN REVIEWED FOR THE HOWARD SOIL CONSERVATION DISTRICT AND MEET THE TECHNICAL REQUIREMENTS FOR SMALL POND CONSTRUCTION, SOIL EROSION, AND SEDIMENT CONTROL

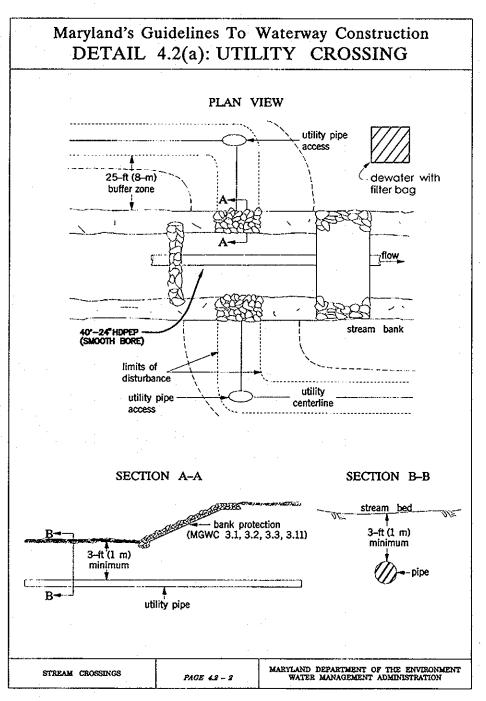
APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

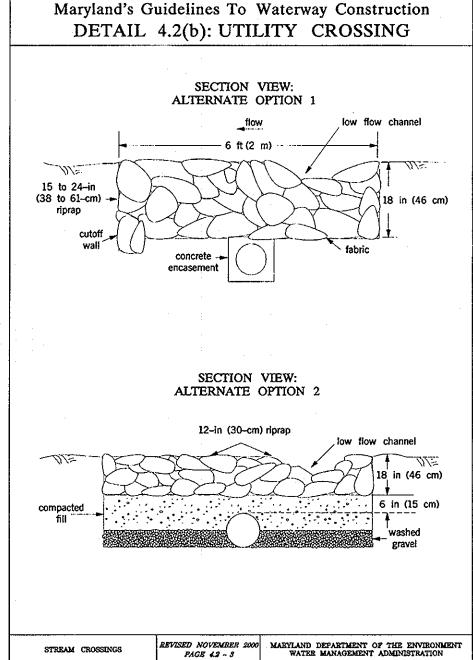
William I. Walnut / 12-24-03 APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

Humate









HILLIS-CARNES ENGINEERING ASSOCIATES, INC. RECOMMENDATIONS

Embankment and Cut-off trench Construction

The site should be stripped of topsoil and any other unsuitable materials from the embankment or structure area in accordance with Soil Conservation Guidelines. After stripping operations have been completed, the exposed subgrade materials should be proofrolled with a loaded dumptruck or similar equipment in the presence of a geotechnical engineer or his representative. For areas that are not accessible to a dump truck, the exposed materials should be observed and tested by a geotechnical engineer or his representative utilizing a Dynamic Cone Penetrometer. Any excessively soft or loose materials identified by proofrolling or penetrometer testing should be excavated to suitable firm soil, and then grades re-established by backfilling with suitable

A representative of the geotechnical Engineer should be present to monitor placement and compaction of fill for each embankment and cut—off trench. In accordance with Maryland Soil Conservation Specification 378, soils considered suitable for the center of embankment and cut-off trench shall conform to Unified Soil Classification GC, SC, CH, or CL. Per SCS 378, consideration may be given to the use of other materials in the embankment if design and construction are supervised by a geotechnical engineer.

It is our professional opinion that in addition to the soil materials described above a fine grained soil, including Silt (ML) with a plasticity index of 10 or more can be utilized for the center of the embankment and core trench. Based on the results of the test pits, it appears that surficial materials in the areas of test pits TP-1 through TP-6, TP-10 and TP-11 include silty clay and clayey silt. These materials should be suitable for core and cut-off trench material. However, exploration with test pits and additional laboratory testing should be conducted prior to construction to identify and quantify potential borrow areas. All fill materials must be placed and compacted in accordance with MD SCS 378 specifications.

Additionally, the following procedures should be utilized to construct the proposed embankments:

- 1. Slope construction should commence at the toes of the proposed slopes and continue upwards as additional fill is placed. The engineered fill placed for slope construction should be benched into the natural slopes in the abutment areas to provide good contact and to prevent the presence of weak zones.
- 2. Typically during slope construction, compaction equipment has difficulty compacting soils along the shoulder. It is therefore important that the bank be overfilled during slope construction and then cut back to the required geometry.
- 3. After construction, the slopes should be promptly vegetated to prevent erosion. Also, to prevent erosion from occurring prior to sprouting of the vegetation, the slopes should be protected with straw or an erosion control geotextile.
- 4. The embankment construction should be done under the supervision of an experienced soil inspector or the Geotechnical Engineer. Sufficient testing during fill placement should be done to verify

It is recommended that test pits be performed with an excavator to determine if rock within the basin areas will require blasting to establish proposed grades. Should it be determined that the rock within the basin areas will require blasting, it is recommended that the blasting within 75 ft. of the riser and principal spillway structures be performed prior to construction of the principal spillways and riser structures.

adequate compaction.

CONSTRUCTION SPECIFICATIONS

These specifications are appropriate to all ponds within the scope of the Standard for practice MD-378. All references to ASTM and AASHTO specifications apply to the most recent

Site Preparation

Areas designated for borrow areas, embankment, and structural works shall be cleared, grubbed and stripped to topsoil. All trees, vegetation, roots and other objectionable material shall be removed. Channel banks and sharp breaks shall be sloped to no steeper than 1:1. All trees shall be cleared and grubbed within 15 feet of the toe of the embankment

and other objectionable material unless otherwise designated on the plans. Trees, brush, and stumps shall be cut approximately level with the ground surface. For dry stormwater management ponds, a minimum of a 25-foot radius around the inlet structure shall be

All cleared and grubbed material shall be disposed of outside and below the limits of the dam and reservoir as directed by the owner or his representative. When specified, a sufficient quantity of topsoil will be stockpiled in a suitable location for use on the embankment and other designated areas.

<u>Material</u> — The fill material shall be taken from approved designated borrow areas. If shall be free of roots, stumps, wood, rubbish, stones greater than 6°, frozen or other shall conform to Unified Soil Classification GC, SC, CH, or CL and must have at least 30% passing the #200 sieve. Consideration may be given to the use of other materials in the embankment if designed by a geotechnical engineer. Such special designs must have construction supervised by a geotechnical engineer.

Materials used in the outer shell of the embankment must have the capability to support vegetation of the quality required to prevent erosion of the embankment

<u>Placement</u> — Areas on which fill is to be placed shall be scarified prior to placement of fill. fill materials shall be placed in maximum 8 inch thick (before compaction) layers which are to be continuous over the entire length of the fill. The most permeable borrow material shall be placed in the downstream portions of the embankment. The principal spillway must be installed concurrently with fill placement and not excavated into the embankment.

<u>Compaction</u> — the movement of the hauling and spreading equipment over the fill shall be controlled so that the entire surface of each lift shall be traversed by not less than one tread track of heavy equipment or compaction shall be achieved by a minimum of four complete passes of a sheepsfoot, rubber tired or vibratory roller. Fill material shall contain sufficient moisture such that the required degree of compaction will be obtained with teh equipment used. The fill material shall contain sufficient moisture so that if formed into a pall it will not crumble, yet not be so wet that water can be squeezed out.

When required by the reviewing agency the minimum required density shall not be less than 95% of maximum dry density with a moisture content within ± 2% of the optimum. Each layer of fill shall be compacted as necessary to obtain that density, and is to be certified by the Engineer at the time of construction. All compaction is to be determined by AASHTO Method T-99 (Standard Proctor).

Cut Off Trench - The cutoff trench shall be excavated into impervious material along or parallel to the centerline of the embankment as shown on the plans. The bottom width of the trench shall be governed by the equipment used for excavation, with the minimum width being four feet. The depth shall be a least four feet below existing grade or as shown on the plans. The side slopes of the trench shall be 1 to 1 or flatter. The backfill shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density

Embankment Core - The core shall be parallel to the centerline of the embankment as shown on the plans. The top width of the cores shall be a minimum of four feet. The neight shall extend up to at least the 10 year water elevation or as shown on the plans. The side slopes shall be 1 to 1 or flatter. The core shall be compacted with construction equipment, rollers, or hand tampers to assure maximum density and minimum permeability in addition, the core shall be placed concurrently with the outer shell of the embankment.

Backfill adjacent to pipes or structures shall be of the type and quality conforming to that specified for the adjoining fill material. The fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material needs to fill completely all spaces under and adjacent to the pipe. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a concrete structure or pipe, unless there is a compacted fill of 24" or greater over the structure or pipe.

Structure backfill may be flowable fill meeting the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 313 as modified. The mixture shall have a 100—200 psi; 28 day unconfined compressive strength. The flowable fill shall have a minimum pH of 4.0 and c ninimum resistively of 2,000 ohm—cm. Material shall be placed such that a minimum of 6° (measured perpendicular to the outside of the pipe) of flowable fill shall be under (bedding), over and, on the sided of the pipe. It only needs to extend up to the spring line for rigid conduits. Average slump of the fill shall be 7" to assure flowability of the material. Adequate measures shall be taken (sand bags, etc.) to prevent floating the pipe. When using flowable fill, all metal pipe shall be bituminous coated. Any adjoining soil fill shall be placed in horizontal layers not to exceed four inches in thickness and compacted by hand tampers or other manually directed compaction equipment. The material shall completely fill all voids adjacent to the flowable fill zone. At no time during the backfilling operation shall driven equipment be allowed to operate closer than four feet, measured horizontally, to any part of a structure. Under no circumstances shall equipment be driven over any part of a structure or give unless there is a compacted fill of 24° or greater over the structure or give. Backfill material outside the structural backfill (flowable fill) zone shall be of the type and quality conforming to that specified for the core of the embankment or other embankment

All pipes shall be circular in cross section

Corrugated Metal Pipe — all of the following criteria shall apply for corrugated metal pipe: 1. Materials — (Polymer Coated steel pipe) — Steel pipes with polymeric coatings shall bave a minimum coating thickness of 0.01 inch (10 mil) on both sides of the pipe. This pipe and its appurtenances shall conform to the requirements of AASHTO Specifications M-245 M-246 with watertight coupling bands or flanges.

Maerials — (Aluminum Coated Steel Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M-274 with watertight coupling bands or flonges. Aluminum Cooted Steel Pipe, when used with flowable fill or when soil and/or water conditions warrant the need for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Any aluminum coating damaged or otherwise removed shall be replaced with cold applied bituminous coating compound. Aluminum surfaces that are to be in contact with concrete shall be painted with one coat of

Materials — (Aluminum Pipe) — This pipe and its appurtenances shall conform to the requirements of AASHTO Specification M—196 or M—211 with watertight coupling bands or flanges. Aluminum Pipe, when used with flowable fill or when soil and/or water conditions warrant for increased durability, shall be fully bituminous coated per requirements of AASHTO Specification M-190 Type A. Aluminum surfaces that are to be contact with concrete shall be painted with one coat of zinc chromate primer or two coats of asphalt. Hot dip galvanized bolts may be used for connections. The pH of the surrounding soils shall be

2. Coupling bands, anti-seep collars, end sections, etc., must be composed of the same material and coatings as the pipe. Metals must be insulated from dissimilar materials with

3. Connections - All connections with pipes must be completely watertight. The drain pipe or barrel connection to the riser shall be welded all ground when the pipe and riser are metal Anti-seep collars shall be connected to the pipe in such a manner as to be completely watertight. Dimple bands are not considered to be watertight.

All connection shall use a rubber or neoprene gasket when joining pipe sections. The end of each pipe shall be re-rolled an adequate number of corrugations to accommodate the bandwidth. The following type connections are acceptable for pipes less than 24 inches in diameter: flanges on both ends of the pipe with a circular 3/8 inch closed cell neoprene gasket, prepunched to the flange bolt circle, sandwiched between adjacent flanges; a 12-inch wide standard lap type band with 12-inch wide by 3/8-inch thick closed cell circular neoprene gasket; and a 12-inch wide hugger type band with o-ring gaskets having a minimum diameter of 1/2 inch greater than the corrugation depth. Pipes 24 inches in diameter and larger shall be connected by a 24 inch long annular corrugated band using a minimum of 4 (four) rods and lugs, 2 on each connecting pipe end. A 24—inch wide by 3/8—inch thick closed cell circular neoprene gasket will be installed with 12 inches on the end of each pipe. Flanged joints with 3/8 inch closed cell gaskets the full width of the

Helically corrugated pipe shall have either continuously welded seams or have lock seams with internal caulking or a neoprene bead.

4. Bedding - The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support. 5. Backfilling shall conform to "Structure Backfill"

6. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

Reinforced Concrete Pipe - All of the following criteria shall apply for reinforced concrete

1. Materials - Reinforced concrete pipe shall have bell and spigot joints with rubber gaskets and shall equal or exceed ASTM C-36

Bedding - Reinforced concrete pipe conduits shall be laid in a concrete bedding/cradle for their entire length. This bedding/cradle shall consist of high slump concrete placed under the pipe and up the sides of the pipe at least 50% of its outside diameter with a minimum thickness of 6 inches. Where a concrete cradle is not needed for structural reasons, flowable fill may be used a described in the "Structure Backfill" section of this standard. Gravel

3. Laying pipe - Bell and spigot pipe shall be places with the bell end upstream. Joints shall be made in accordance with recommendations of the manufacturer of the material. After the joints are sealed for the entire line, the bedding shall be placed so that all spaces under the pipe are filled. Care shall be exercised to prevent any deviation form the original line and grade of the pipe. The first joint must be located within 4 feet from the riser

5. Other details (anti-seep collars, valves, etc.) shall be shown on the drawings.

4. Backfilling shall conform to "Structure Backfill"

Plastic Pipe - The following criteria shall apply for plastic pipe: 1. Materials - PVC pipe shall be PVC-1120 or PVC-1220 conforming to ASTM D-1785 or ASTM D-2241. Corrugated High Density Polyethylene (HDPE) pipe, couplings and fittings shall conform to the following: 4' - 10" inch pipe shall meet the requirements of AASHTO M252 Type S, and 12" through 24" inch shall meet the requirements of AASHTO M294 Type S.

2. Joints and connections to anti-seep collars shall be completely watertight.

3. Bedding — The pipe shall be firmly and uniformly bedded throughout its entire length. Where rock or soft, spongy or other unstable soil is encountered, all such material shall be removed and replaced with suitable earth compacted to provide adequate support.

4. Backfilling shall conform to "Structure Backfill" 5. Other details (anti-seep collars, valves, etc.) shall be as shown on the drawings.

<u>Drainage Diaphragms</u> - When a drainage diaphragm is used, a registered professional

Concrete shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 414, Mix No. 3.

Rock riprap shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 31

Geotextile shall be placed under all riprop and shall meet the requirements of Maryland Department of Transportation, State Highway Administration Standard Specifications for Construction and Materials, Section 921.09, Class C.

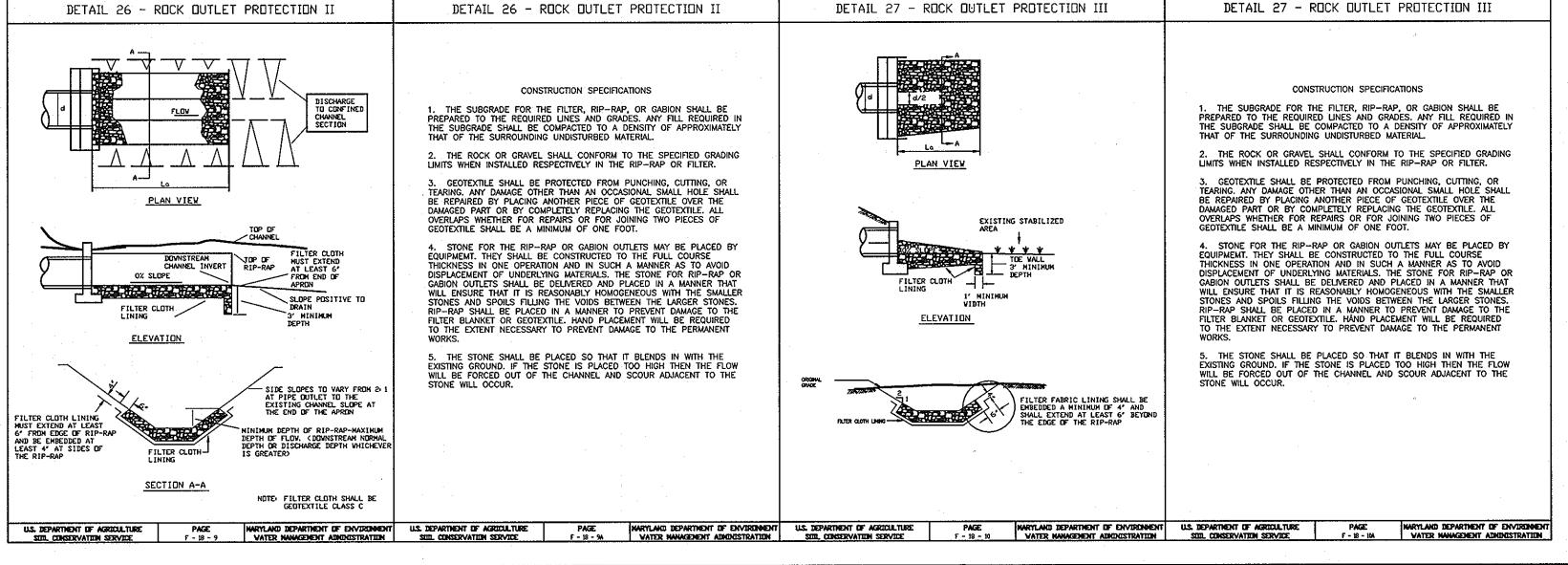
All work on permanent structures shall be carried out in areas free from water. The contractor shall construct and maintain all temporary dikes, levees, cofferdams, drainage channels, and stream diversions necessary to protect the areas to be occupied by the permanent works. The contractor shall also furnish, install, operate, and maintain all necessary pumping and other equipment required for removal of water from various parts of the work and for maintaining the evacuations, foundation, and other parts of the work free from water as required or directed by the engineer for constructing each part of the work After having served their purpose, all temporary protective works shall be removed or leveled and graded to the extent required to prevent obstruction in any degree whatsoever of the flow of water to the spillway or outlet works and so as not to interfere in any way with the operation or maintenance of the structure. Stream diversions shall be maintained until the full flow can be passed through the permanent works. The removal of water from the required excavation and the foundation shall be accomplished in a manner and to the exten that will maintain stability of the excavated slopes and bottom required excavations and will allow satisfactory performance of all construction operations. During the placing and compacting of material in required excavations, the water level at the location being refilled shall be maintained below the bottom of the excavation at such locations which may require draining the water sumps from which the water shall be pumped.

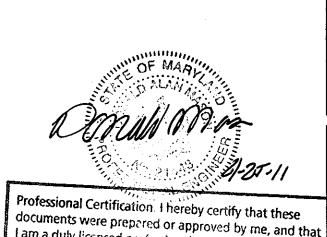
Stabilization

All borrow areas shall be graded to provide proper drainage and left in a sightly condition. All exposed surfaces of the embankment, spillway, spoil and borrow areas, and berms shall be stabilized by seeding, liming, fertilizing and mulching in accordance with the Natural Resources Conservation Service Standards and Specifications for Critical Area Planting (MD-342) or as shown on the accompanying drawings.

<u>Erosion and Sediment Control</u>

Construction operations will be carried out in such a manner that erosion will be controlled and water and air pollution minimized. State and local laws concerning pollution abatement will be followed. Construction plans shall detail erosion and sediment control measures.





documents were prepared or approved by me, and that I am a duly licensed professional engineer under the law of the State of Maryland. License No. 21443 Expiration Date: 12-21-12

The Applipit information is required on this sheet DES: DAM

NO DATE

PROJECT: CASCADE OVERLOOK OWNER/DEVELOPER: SECTION ONE LOTS 1 - 72 AND OPEN SPACE LOTS 73 - 80 AND CASCADE OVERLOOK, L.L.C. P.O.BOX 417 ELLICOTT CITY, MD 21041 TAX MAP 31, GRID 10 &11, PARCELS 160, 161, 788, & 791 (410) 465-4244 1 st. ELECTION DISTRICT HOWARD COUNTY, MARYLAND EROISION AND SEDIMENT CONTROL

DETAILS & STORMWATER MANAGEMENT NOTES CRAIG R. AND KAREN C. MARTIN VP-86-130, F-88-20, S-01-04, PB-359, P-02-11 4937 LANDING ROAD ELKRIDGE, MD 21075 DATE: -DRN: RPS | CHK: DAM SCALE: AS SHOWN

BENCHMARK

ENGINEERING, INC.

8480 BALTIMORE NATIONAL PIKE A SUITE 418

ELLICOTT CITY, MARYLAND 21043

PHONE: 410-465-6105

ENGINEERS A LAND SURVEYORS A PLANNERS

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CHIEF, DEVELOPMENT ENSINEERING DIVISION MAT

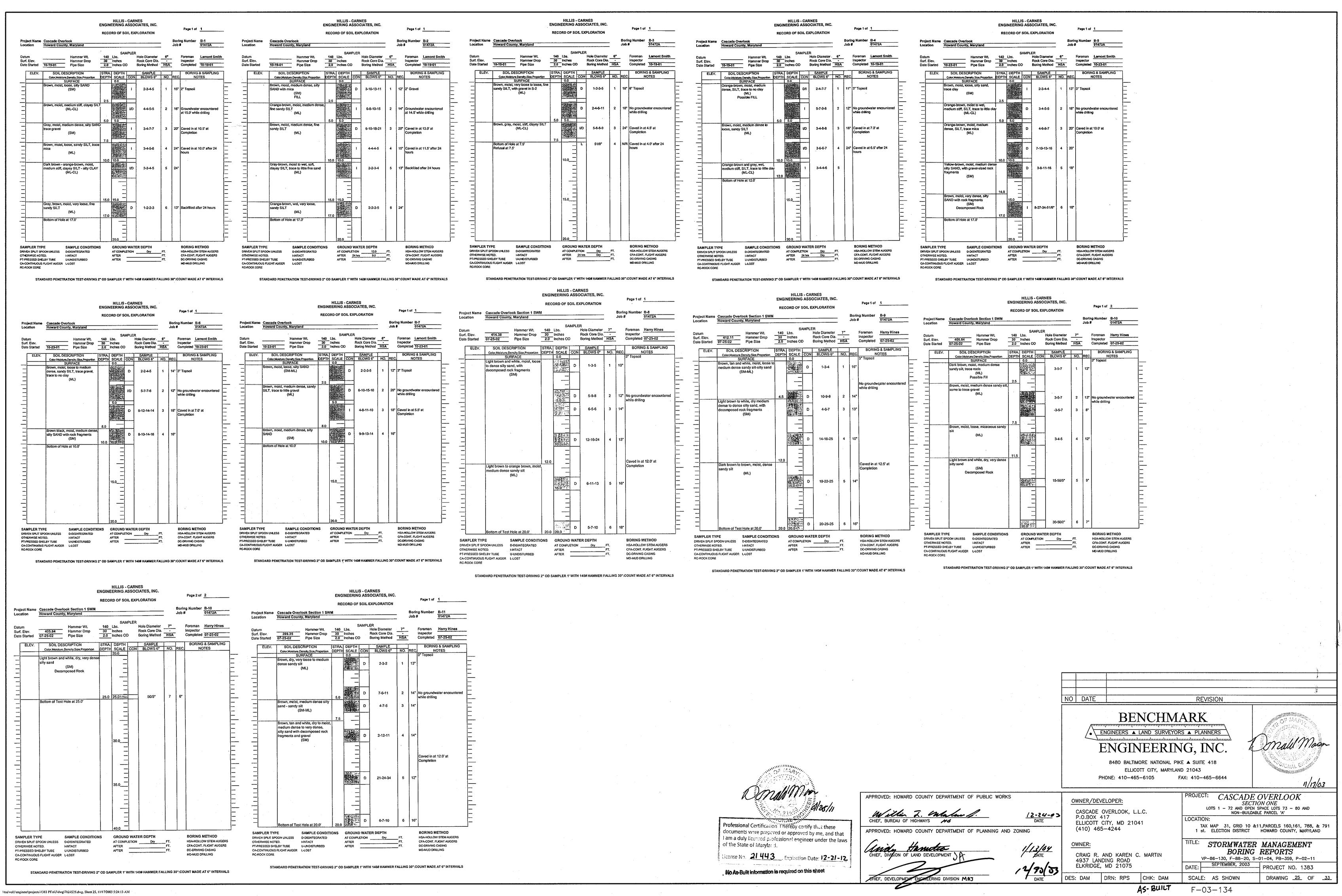
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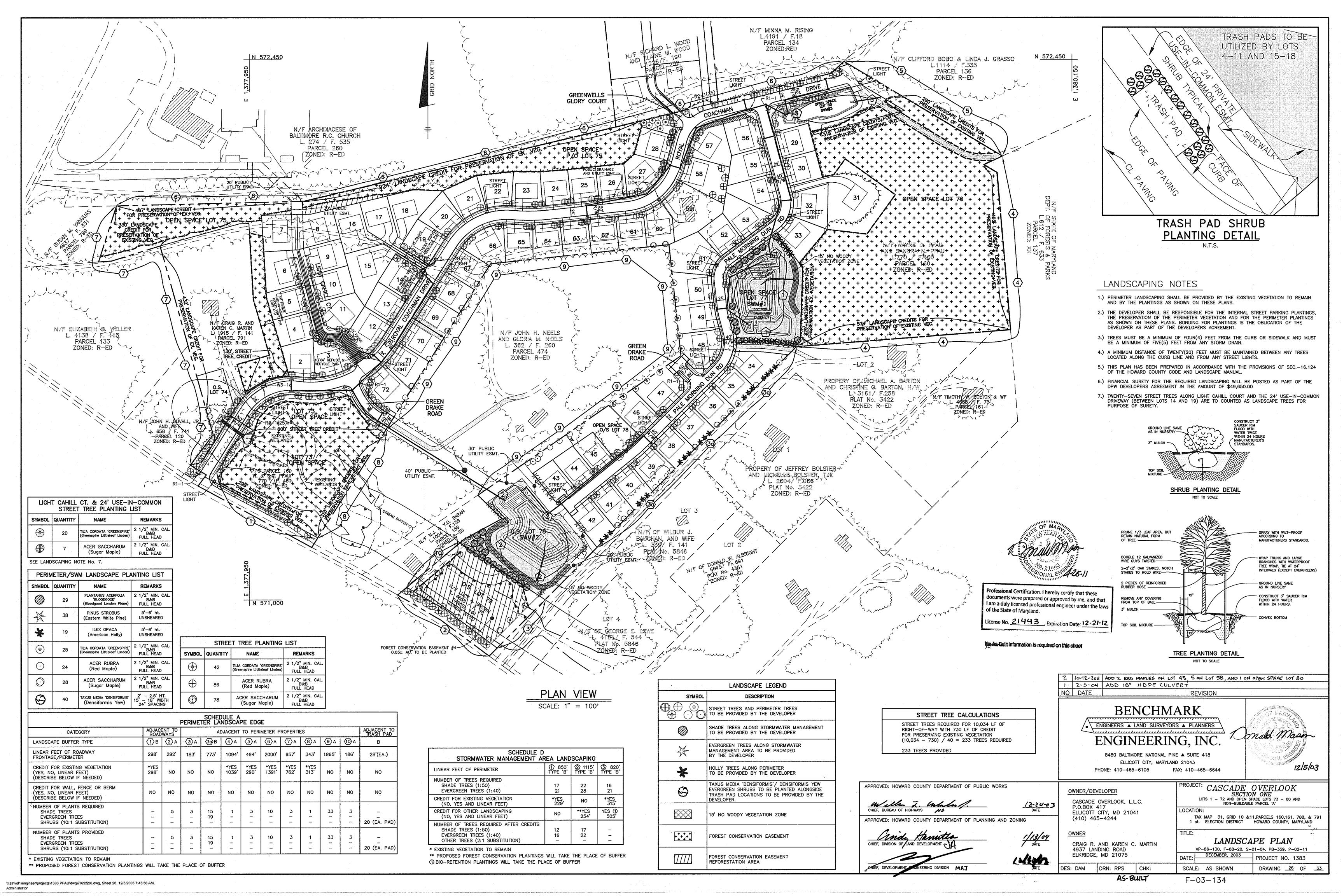
PROJECT NO. 1383

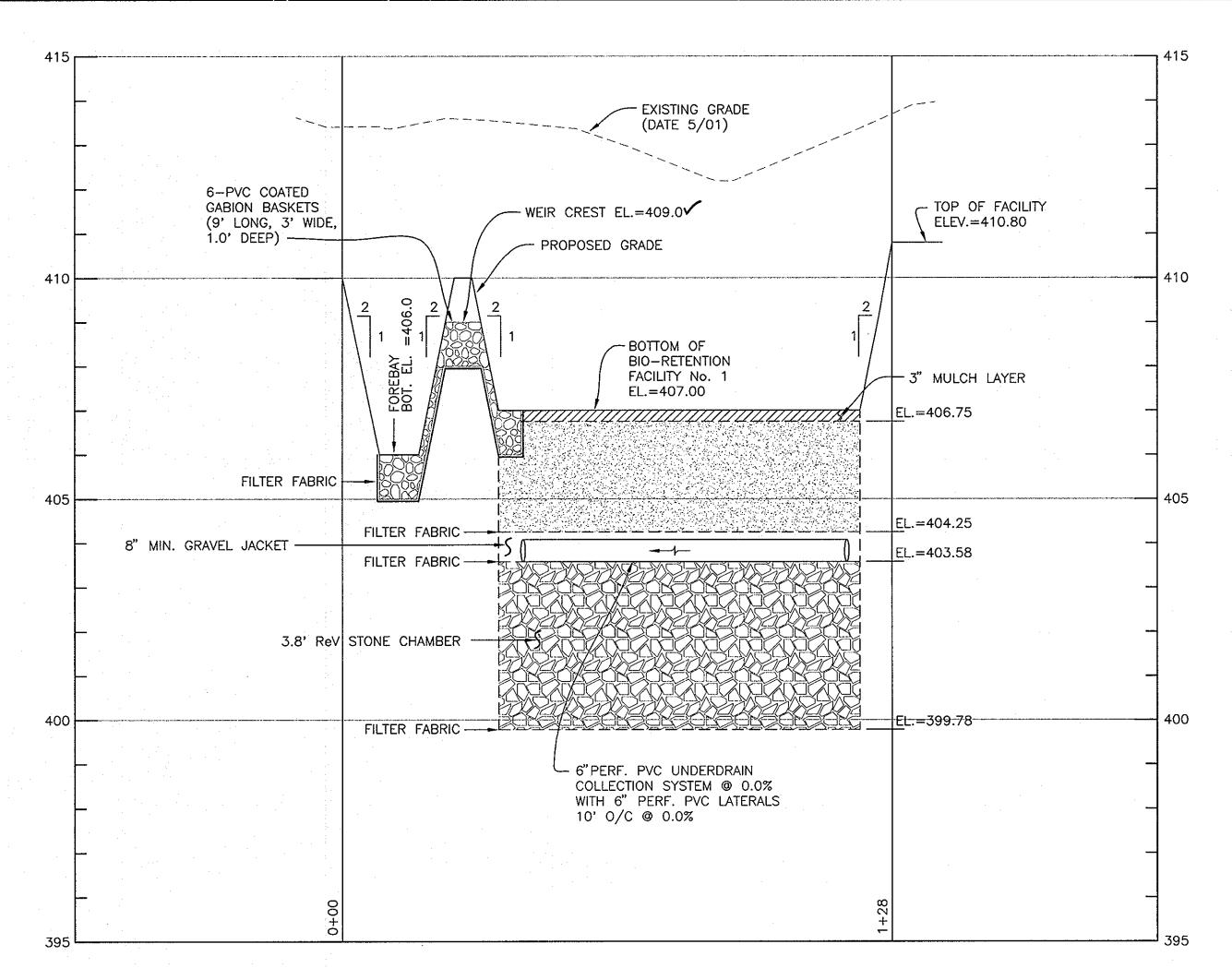
DRAWING <u>24</u> OF <u>33</u>

FAX: 410-465-6644



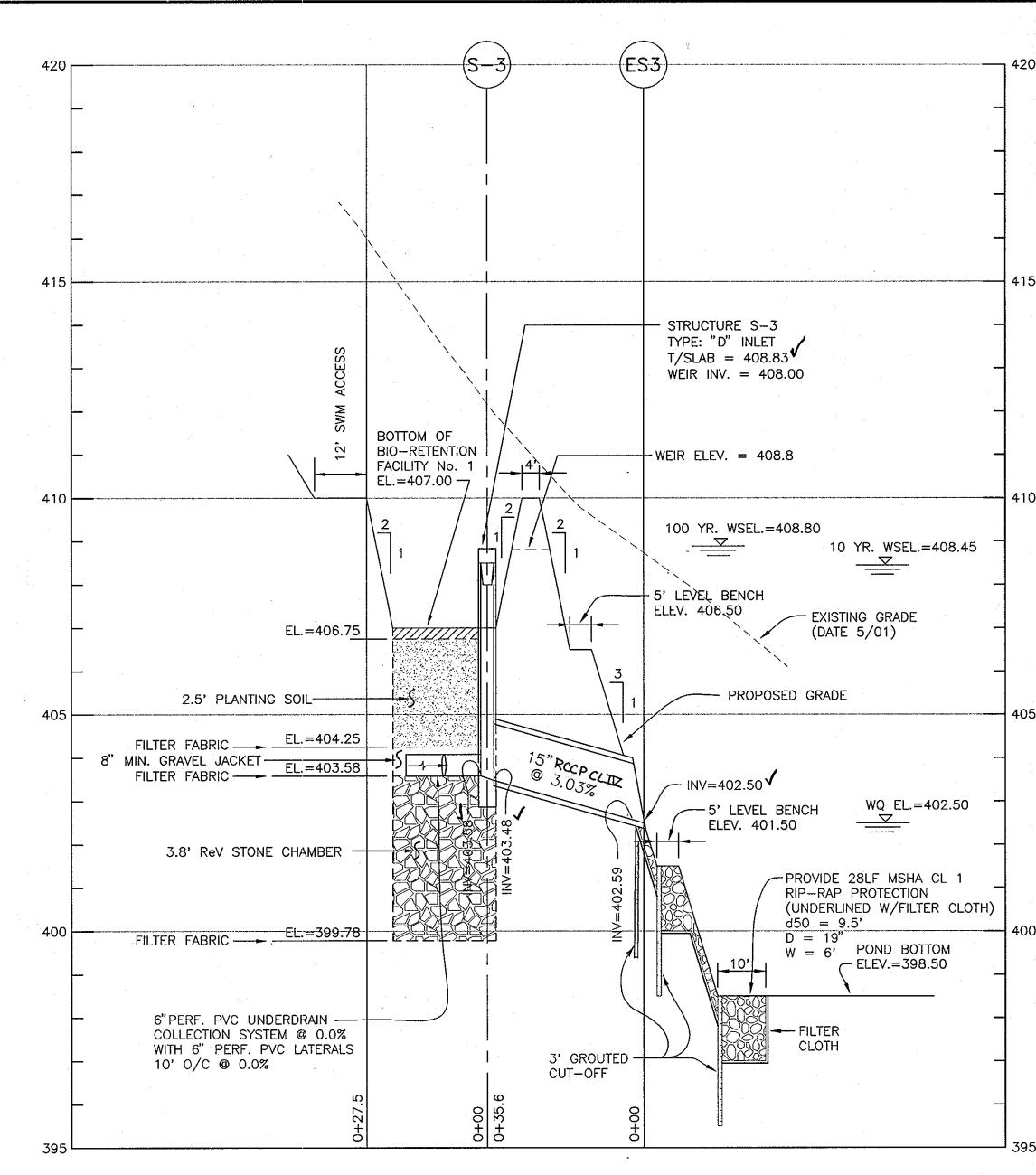
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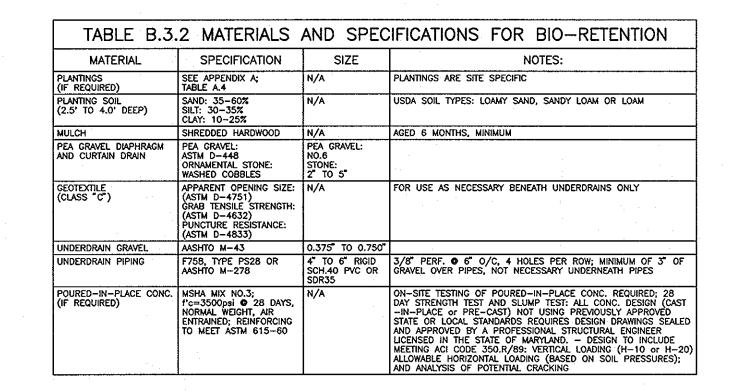
BIO-RETENTION FACILITY #1 SECTION A-A

HORIZONTAL SCALE: 1" = 20' VERTICAL SCALE: 1" = 2'



BIO-RETENTION FACILITY #1 SECTION B-B

HORIZONTAL SCALE: 1" = 20' VERTICAL SCALE: 1" = 2'



OPERATION & MAINTENANCE SCHEDULE FOR BIO-RETENTION AREAS

1. ANNUAL MAINTENANCE OF PLANT MATERIAL, MULCH LAYER AND SOIL LAYER IS REQUIRED. MAINTENANCE OF MULCH AND SOIL IS LIMITED TO CORRECTING AREAS OF EROSION OR WASH-OUT. ANY REPLACEMENT OF MULCH SHALL BE DONE IN THE SPRING. PLANT MATERIAL SHALL BE CHECKED FOR DISEASE & INSECT INFESTATION AND MAINTENANCE WILL ADDRESS DEAD MATERIAL & PRUNING.

2. SCHEDULE OF PLANT INSPECTION WILL BE TWICE A YEAR IN THE SPRING AND FALL. THIS INSPECTION WILL INCLUDE; REMOVAL OF DEAD & DISEASED VEGETATION CONSIDERED BEYOND TREATMENT; TREATMENT OF ALL DISEASED TREES & SHRUBS; AND REPLACEMENT OF ALL DEFICIENT STAKES & WIRES.

3. MULCH SHALL BE INSPECTED EACH SPRING. REMOVE THE PREVIOUS MULCH LAYER BEFORE APPLYING NEW LAYER ONCE EVERY 2 TO 3 YEARS.

4. SOIL EROSION TO BE ADDRESSED ON AN AS-NEEDED BASIS, WITH A MINIMUM OF ONCE PER MONTH AND AFTER HEAVY STORM EVENTS.

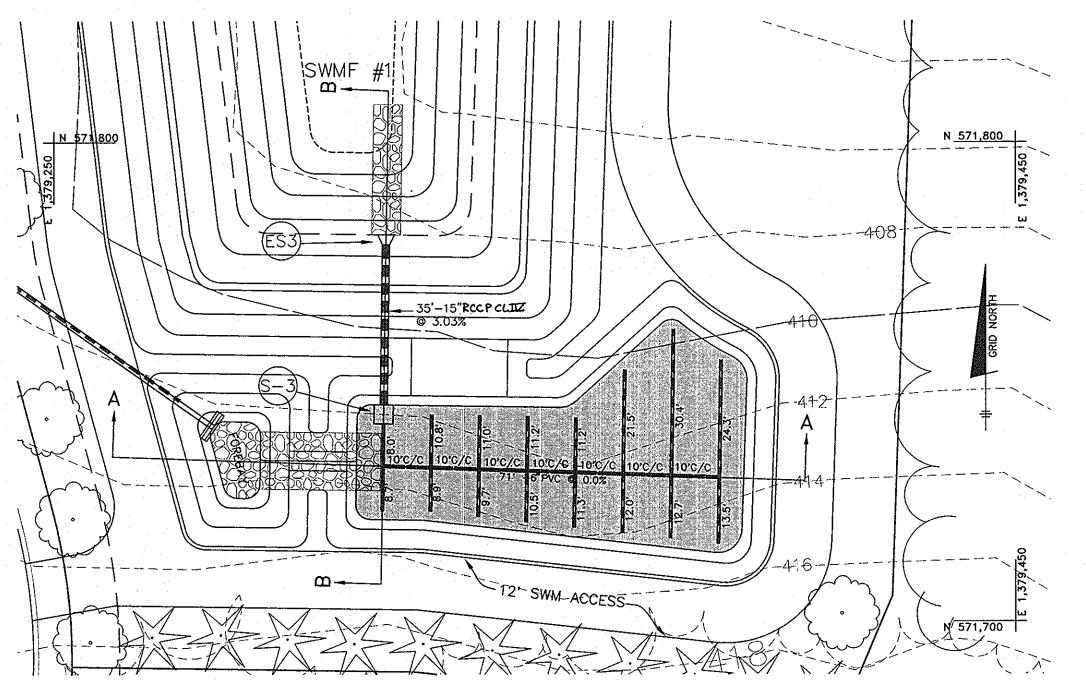
SWMF - LANDSCAPE DATA

HYDROLOGIC ZONE 3 — REGULARLY INUNDATED
SHORELINE FRINGE
(HIGH MARSH)
HYDROLOGIC CONDITION — 0" TO 1'—0" DEEP
HARDINESS — TEMPERATE ZONE 6b (—5" TO 0")

NOTE: REFER TO MDE 2000 MD STORMWATER DESIGN MANUAL VOLUMES 1 & 2 FOR LANDSCAPE CONTRACTOR RESPONSIBILITIES, PRACTICES AND MAINTENANCE DUTIES

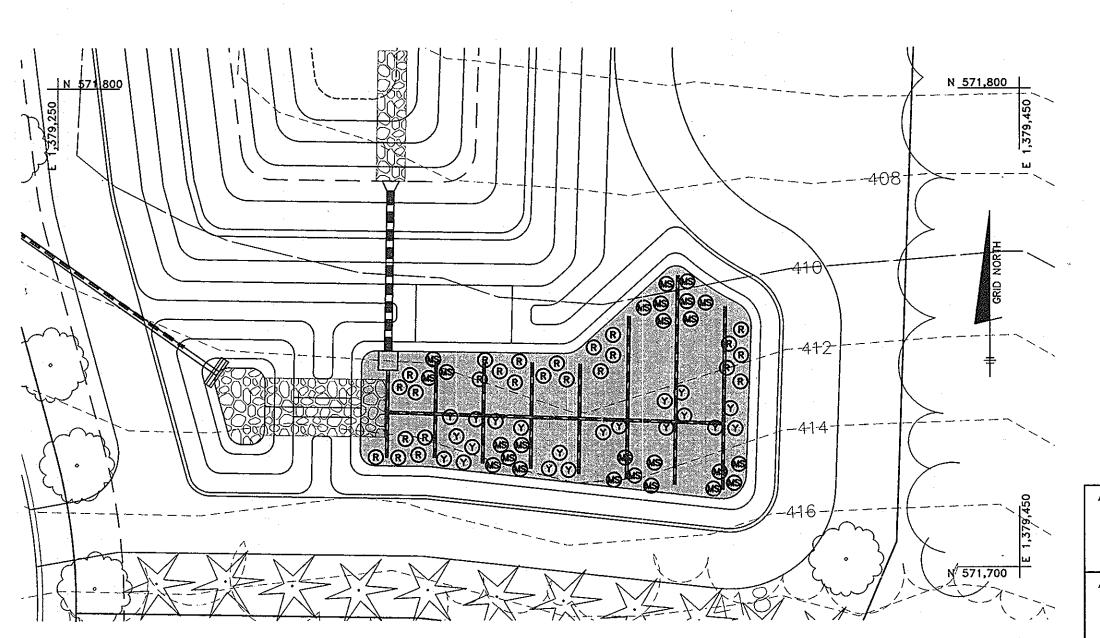
SEE SHEET 4 FOR SEQUENCE OF CONSTRUCTION

SWMF PLANTING LIST					
SYMBOL	QUANTITY	NAME	REMARKS		
R	25	RHODODENDRON MAXIMUM "RHODODENDRON, ROSEBAY"	1.0" — 1.5" HT. 15" —18" WIDTH 18" —24" SPACE MIN.		
(Y)	20	ILEX VOMITORIA "YAUPON HOLLY"	5.0' — 6.0' HT. UNSHEARED		
MS	25	SPIRAEA ALBA MEADOW-SWEET, NARROW LEAF	1.5" — 2.5' HT. 15" —18" WIDTH 18" —24" SPACE MIN.		



PLAN VIEW

SCALE: 1" = 20'



PLANTING PLAN VIEW

SCALE: 1" = 20'



AS-BUILT CERTIFICATION
I hereby certify, by my seal, that the facilities shown on this plan were constructed as shown on this AS-BUILT plan.
Donald Mason, P.E. No. 21443

Date 4-25-//

Professional Certification. I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland.

License No. 21443 _____ Expiration Date: 12-21-12

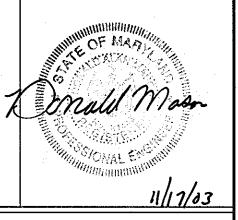
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NO DATE

BENCHMARK

ENGINEERING, INC.

8480 BALTIMORE NATIONAL PIKE A SUITE 418
ELLICOTT CITY, MARYLAND 21043



APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

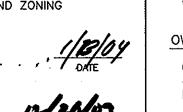
RING DIVISION MAT

HIEF, BUREAU OF HIGHWAYS HS DATE

PROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING

CHIEF, DIVISION OF LAND DEVELOPMENT

CHIEF, DEVELOPMENT ENGLES



OWNER/DEVELOPER

CASCADE OVERLOOK, L.L.C.
P.O.BOX 417
ELLICOTT CITY, MD 21041
(410) 465-4244

LICOTT CITY, MD 210

DES: DAM DRN: RPS CHK: DAM

OWNER

CRAIG R. AND KAREN C. MARTIN

4937 LANDING ROAD

ELKRIDGE, MD 21075

PROJECT: CASCADE OVERLOOK
SECTION ONE
LOTS 1 - 72 AND OPEN SPACE LOTS 73 - 80 AND
NON-BUILDABLE PARCEL 'A'

LOCATION:
TAX MAP 31, GRID 10 &11, PARCELS 160,161, 788, & 791
1 st. ELECTION DISTRICT HOWARD COUNTY, MARYLAND

 BIO—RETENTION PLANTING DETAILS

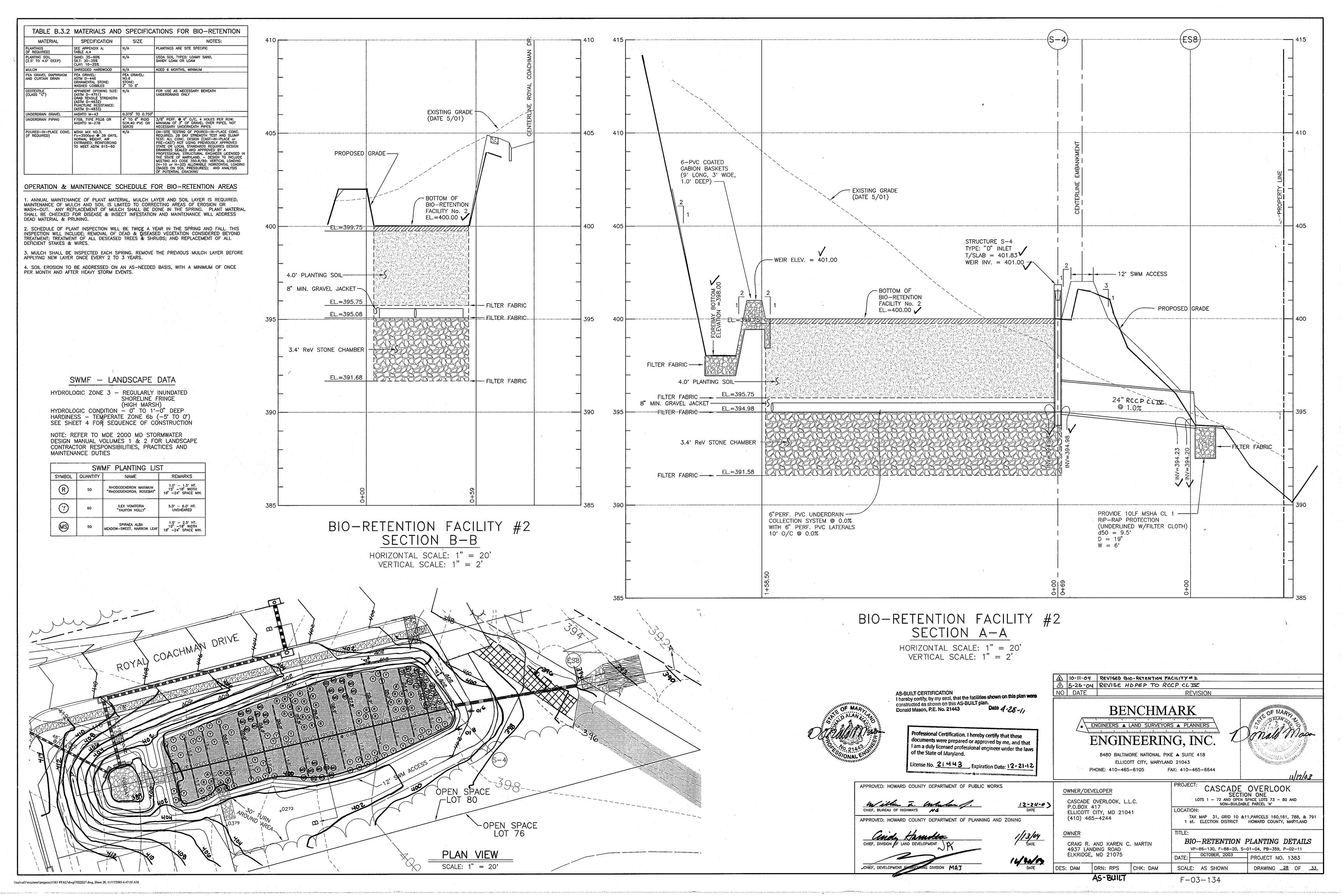
 VP-86-130, F-88-20, S-01-04, PB-359, P-02-11

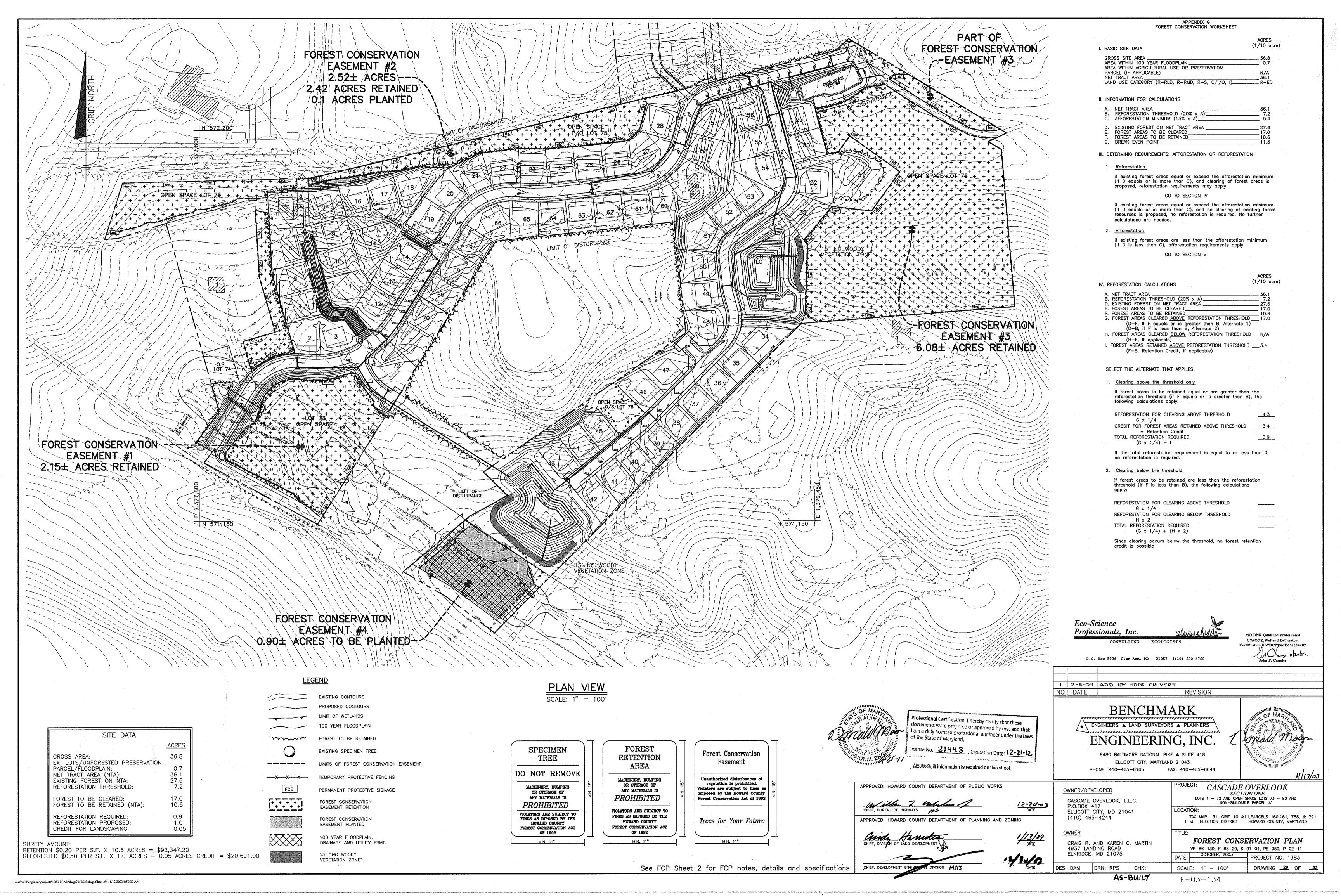
 DATE:
 OCTOBER, 2003
 PROJECT NO. 1383

 SCALE:
 AS SHOWN
 DRAWING 27 OF 33

AS-BUILT F-03-134

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FOREST PROTECTION PROCEDURES - Preconstruction Phase

1) The edge of the woods to be protected will be marked (staked or flagged) in the field per the limits of forest conservation easement shown in the approved site development plan prior to the start of construction activity. All areas within protective easement are to be considered "off limits" to any construction activities. The optional protective fencing shall be installed at the outside edge of forested areas and should be combined with sediment control devices when possible. The limit of the critical root zone and therefore the location of the protective devices is to be determined as follows:

Edge of Forested Area - 1 foot of protective radius/inch of DBH or an eight foot protective radius, whichever is greater.

Critical Root Zone for the forest on this site is an average of 12 feet from the trunk of the tree. Critical root zones for Specimen Tree #1 and #2 are 34' and 30'.

2) Construction activities expressly prohibited within the preservation areas are:

Placing or stockpiling backfill or top soil in protected

Felling trees into protected areas Driving construction equipment into or through protected

Burning in or in close proximity to protected areas

Stacking or storing supplies of nay kind Concrete wash-off areas. Conducting trenching operations Grading beyond the limits of disturbance

Parking vehicles or construction equipment Removal of root mat or topsoil Siting and construction of: Utility lines

Access roads Impervious surfaces Stormwater management devices Staging areas

- Protective fencing (see Figure "Protective Fencing") shall be the responsibility of the general contractor. The general contractor shall affix signs to the fencing at 25' minimum intervals indicating that these areas are "Forest Retention Area" (see Figure "Signage"). The general contractor shall take great care to assure the restricted areas are not violated and theat root systems are protected from smothering, flooding, excessive wetting from dewatering operations, off-site runoff, spillage, and drainage or solutions containing materials hazardous to tree roots.
- The general contractor shall be responsible for any tree damaged or destroyed within the preservation areas whether caused by the contractor, his agents, employees, subcontractors, or licensees.
- 5) Foot traffic shall be kept to a minimum in the protective
- 6) All trees which are not to be preserved within fifty feet of any tree preservation areas are to be removed in a manner that will not damage those trees that are designated for preservation. It is highly recommended that tree stumps within this fifty foot area be around out with a stump arinding machine to minimize damage.
- The general contractor shall designate a "wash out" area onsite for concrete trucks which will not drain toward a protected area.
- 8) A pre-construction meeting shall be held with local authorities before any disturbance has taken place on site.

FORE HIS HHEDING FOR

2/3 EXISTRAG SON AND NO CONFOST

--- GOIL MIK BACKFILL

Undisturbed Soil

Disturbed Soil

Planting on Slope

ODIGHIAL GRADE -

Source: Adapted from Forest Conservation Manual, 1991

Container Grown and B&B Planting Techniques

FOREST PROTECTION PROCEDURES - Construction Phase

Forest and tree conditions should be monitored during construction and corrective measures taken when appropriate

The following shall be monitored:

Soil compaction Root injury - prune and monitor; consider crown

reduction Limb injury — prune and monitor

Flooded conditions - drain and monitor; correct problem Drought conditions — water and monitor; correct problem Other stress signs — determine reason, correct, and

FOREST PROTECTION PROCEDURES - Post Construction Phase

The following measures shall be taken:

- Corrective measures if damages were incurred due to
 - Stress reduction
- Removal of dead or dying trees. This may be done only if trees pose an immediate safety hazard.
- 2) Removal of temporary structures:
 - No burial of discarded materials will occur onsite within the conservation area.
- No open burning within 100 feet of a wooded area.
- All temporary forest protection structures will be removed after construction.
- Remove temporary roads by removing stone or broadcasting mulch; pre-construction elevation should be maintained.
- Aerate compacted soil.
- Replant disturbed sites with trees, shrubs and/or herbaceous plants.
- Retain signs for retention areas or specimen trees.
- h) A County official shall inspect the entire site.
- 3) Future protection measures:
 - Howard County and the developer shall arrange for the dedication of an appropriate forest conservation easement at a later date.

FOREST PROTECTION PROCEDURES - Preconstruction Phase

Stress Reduction and Protection of Specimen Trees Isolated from Forest Retention Areas and General Forest Retention Areas (as they may apply)

Isolated specimen trees that are to be preserved will be examined to determine if stress reduction techniques are needed. Protective measures and their evaluation criteria are provided on this plan only if they are employed herein.

Root Pruning

Evaluation Criteria

Will the critical root zone be affected by construction activities such as grade changes, digging for foundations and

Design Considerations

- Prune prior to construction as shown on the plan (see Figure
- "Root Pruning Detail.") Prune root with a clean cut using proper pruning equipment
- such as a vibratory knife. Exact location of pruning trench should be identified, and immediately backfilled to cover exposed roots after pruning with soil removed other topsoil, peat moss, or other
- suitable material or with other high organic soil. d) For trees over 15" in diameter, root pruning may be done up
- to one year in advance of construction. e) Tree(s) will be monitored for signs of stress.

Crown Reduction or Pruning

Evaluation Criteria

Has the root system been significantly reduced (>30%) or are there dead, damaged, or diseased limbs?

Design Considerations

- a) Reduce only at specified times of the year: Flowering trees - only after flowering and before bud Non-Flowering trees - in late winter, early spring or
- mid summer b) No more than 1/3 of the crown should be removed at one time using acceptable pruning methods (see Figure "Crown Reduction Detail")
- c) Monitor for signs of stress

Watering

Evaluation Criteria

Will construction activities alter the hydrology of the site? Has or will root pruning occur?

Design Considerations

Water only as necessary

Monitor for signs of stress (see Figure "Tree Planting and Maintenance Calendar")

Fertilizing

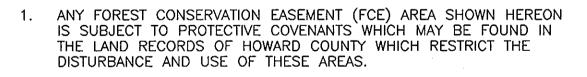
Evaluation Criteria

Is or will be tree(s) be under stressful conditions? Has or will root pruning occur?

Design Considerations

- a) Use low nitrogen and slow release fertilizers. Apply in late fall or early spring (see Figure
- Planting and Maintenance and Calendar") For small trees (<3" in diameter), use punch hole method or pressurized injection method (see Figure "Application of
- Fertilizers by Injection.") For larger trees (>3" diameter), use punch hole method or pressurized injuction method (see Figure "Application of Fertilizers by Injection.")
- Do not apply fertilizer any closer than 3' from tree trunk for pressurized injection method.
- Monitor for signs of stress.

FCP NOTES



- THE FOREST CONSERVATION EASEMENTS HAVE BEEN ESTABLISHED TO FULFILL THE REQUIREMENTS OF SECTION 16.1200 OF THE HOWARD COUNTY CODE, FOREST CONSERVATION ACT. NO CLEARING, GRADING OR CONSTRUCTION IS PERMITTED WITHIN THE FOREST CONSERVATION EASEMENTS; HOWEVER, FOREST MANAGEMENT PRACTICES AS DEFINED IN THE DEED OF FOREST CONSERVATION EASEMENT ARE ALLOWED.
- FORESTED AREAS OCCURRING OUTSIDE OF THE FCE SHALL NOT BE CONSIDERED PART OF THE FCE AND SHALL NOT BE SUBJECT TO PROTECTIVE LAND COVENANTS.
- LIMITS OF DISTURBANCE SHALL BE RESTRICTED TO AREAS OUTSIDE THE LIMIT OF TEMPORARY FENCING OR THE FCE BOUNDARY, WHICHEVER IS GREATER.
- THERE SHALL BE NO CLEARING, GRADING, CONSTRUCTION OR DISTURBANCE OF VEGETATION IN THE FOREST CONSERVATION EASEMENT, EXCEPT AS PERMITTED BY HOWARD COUNTY DPZ.
- NO STOCKPILES, PARKING AREAS, EQUIPMENT CLEANING AREAS, ETC. SHALL OCCUR WITHIN AREAS DESIGNATED AS FOREST CONSERVATION EASEMENTS.
- TEMPORARY FENCING SHALL BE USED TO PROTECT FOREST RESOURCES DURING CONSTRUCTION. THE FENCING SHALL BE PLACED ALONG ALL FCE BOUNDARIES WHICH OCCUR WITHIN 15 FEET OF THE PROPOSED LIMITS OF DISTURBANCE.
- PERMANENT SIGNAGE SHALL BE PLACED 50-100' APART ALONG THE BOUNDARIES OF ALL AREA INCLUDED IN FOREST CONSERVATION EASEMENTS.
- THE FOREST CONSERVATION OBLIGATIONS INCURRED BY THIS SITE DEVELOPMENT PLAN HAVE BEEN MET THROUGH THE RETENTION, IN AN EASEMENT, OF 10.6 ACRES OF EXISTING RETAINED FOREST AND 1.0 ACRES - 0.05 ACRES OF CREDIT FOR REFORESTATION. FOR A TOTAL OF 11.55 ACRES OF FOREST CONSERVATION EASEMENTS WILL BE CREATED FOR THIS PROJECT.

PLANTING SPECIFICATIONS

Planting/Soil Specifications

- Planting of nursery stock shall take place between March 15th and April 30th. Container stock may be planted Septembe
- A twelve (12) inch layer of topsoil shall be spread over all afforestation areas impacted by site grading to assure a suitable planting area. Disturbed areas shall be seeded and stabilized as per general construction plan for project. Planting areas not impacted by site grading shall have no additional topsoil installed. All bareroot planting stock shall have their root systems dipped into an anti-desiccant gel prior to planting.
- Plants shall be installed so that the top of root mass is level with the top of existing grade. Backfill in the planting pits shall consist of 3 parts existing soil to 1 part pine fines or equivalent.
- Fertilizer shall consist of Agriform 22-8-2, or equivalent, applied as per manufacturer's specifications.
- A two (2) inch layer of hardwood mulch shall be placed over the root area of all plantings. Plant material shall be transported to the site in a tarped or covered truck. Plants shall be kept moist prior to planting.
- All non-organic debris associated with the planting operation shall be removed from the site by the contractor.

Sequence of Construction

- Plants shall be installed as per Plant Schedule and the Planting/Soil Specifications for the project. Upon completion of the planting, signage shall be installed as per the Forest Retention Area Protection Devices shown on
- Sheet 2 of the Forest Conservation Plan Plantings shall be maintained and guaranteed in accordance with the Maintenance and Guarantee requirements for

Maintenance of Plantings

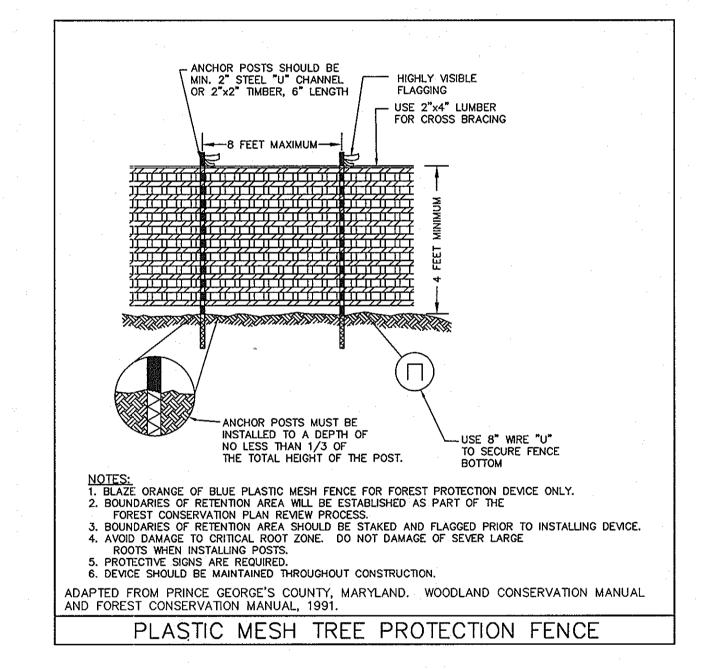
- Maintenance of plantings shall last for a period of 24 months.
- All plant material shall be watered twice a month during the 1st growing season. Watering may be more or less frequent depending on weather conditions. Watering during second growing season, to be once a month during May-September, if
- Invasive exotics and noxious weeds will be removed from reforestation areas. Old field successional species will be
- Plants will be examined a minimum two times during the growing season for serious plant pests and diseases. Serious problems will be treated with the appropriate agent.
- Dead branches will be pruned from plantings.

Guarantee Requirements

- After one growing season, plant material shall be maintained at 90% survival threshold. A 75 percent survival rate of forestation plantings will be required at the end of the 24 month maintenance period. All plant material below the 75
- percent threshold will be replaced at the beginning of the next growing season. The contractor will not be liable for plant loss due to theft or vandalism.

Surety for Retention and Reforestation

The developer shall post a surety (bond, letter of credit) to ensure that reforestation plantings are completed. Upon acceptance of the plantings by the County, the bond shall be released. Financial surety for the Forest Conservation plantings will be posted as part of the DPW developer's agreement. Retention \$0.20 x 10,6 acres = \$92,347.20, and reforested \$0.50 x 1.0 acres - 0.05 acres credit for landscaping = \$20,691.00; total = \$113,038.00



Eco-Science Professionals,

P.O. Box 5006 Glen Arm, MD 21057 (410) 592-6752

BENCHMARK

ENGINEERING, INC.

ENGINEERS A LAND SURVEYORS A PLANNERS



REVISION

Planting Schedule

<u>Size</u>

<u>Size</u>

2-3 whip

2-3' whip

2-3' whip

2-3' whip

2-3' whip

2-3' whip

* - landscape sized trees shall be installed on 20 foot centers in a random pattern throughout

future maintenance. Where possible rows should be made along contour. Shelters required

** Plantings to be spaced on 11 foot centers, plantings should be installed in rows to facilitate

2 1/2" - 3" dia.

2 1/2" - 3" dia.

2 1/2" – 3" dia.

Spacing

<u>Spacing</u>

FCE Planting Area 2 (0.10 acres)

Acer rubrum - Red maple

Quercus alba - White oak

FCE Planting Area 4 (0.9 acres)

Species

50 Acer rubrum - Red maple

50 Betula nigra - River birch

45 Nyssa sylvatica - Black dum

40 Quercus palustris - Pin oak

60 Salix nigra - Black willow

per County regulations.

Planting Notes:

<u>Key:</u>

70 Fraxinus pennsylvanica - Green ash

the easement. The trees should not be placed in a grid.

1. No motorized vehicles permitted within wetlands.

2. Plants should be flagged to aid on location during

planted in grid pattern to facilitate maintenance and

maintenance. 2-3' whip plantings should also be

removal of invasive and exotic species.

Fraxinus pennsylvanica - Green ash

Species

MD DNR Qualified Professiona

8480 BALTIMORE NATIONAL PIKE ▲ SUITE 418 CONTROL OF THE PROPERTY OF THE STATE OF THE ELLICOTT CITY, MARYLAND 21043

APPROVED: HOWARD COUNTY DEPARTMENT OF PUBLIC WORKS

APPROVED: HOWARD COUNTY DEPARTMENT OF PLANNING AND ZONING Undy Kanuta LAND DEVELOPMENT

FAX: 410-465-6644 PHONE: 410-465-6105 ~OWNER/DEVELOPER' CASCADE OVERLOOK, L.L.C. P.O.BOX 417

> ELLICOTT CITY, MD 21041 (410) 465-4244 CRAIG R. AND KAREN C. MARTIN 4937 LANDING ROAD

FOREST CONSERVATION NOTES AND DETAILS VP-86-130, F-88-20, S-01-04, PB-359, P-02-11 PROJECT NO. 1383 SCALE: AS SHOWN DRAWING <u>30</u> OF <u>33</u>

CASCADE OVERLOOK

SECTION ONE LOTS 1 - 72 AND OPEN SPACE LOTS 73 - 80 AND

NON-BUILDABLE PARCEL 'A'

1 st. ELECTION DISTRICT HOWARD COUNTY, MARYLAND

TAX MAP 31, GRID 10 &11, PARCELS 160,161, 788, & 791

F - 03 - 134

\tsa\vol1\engineer\projects\1383 PFAU\dwg\7052S30.dwg, SHEET 30, 11/17/2003 6:52:02 AM

AS-BUILT

Professional Certification I hereby certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland. License No. 21443 Expiration Date: 12-21-12

Ala As-Built information is required on this sheet

12-24-03

DES: DAM DRN: RPS

NO DATE

ELKRIDGE, MD 21075

