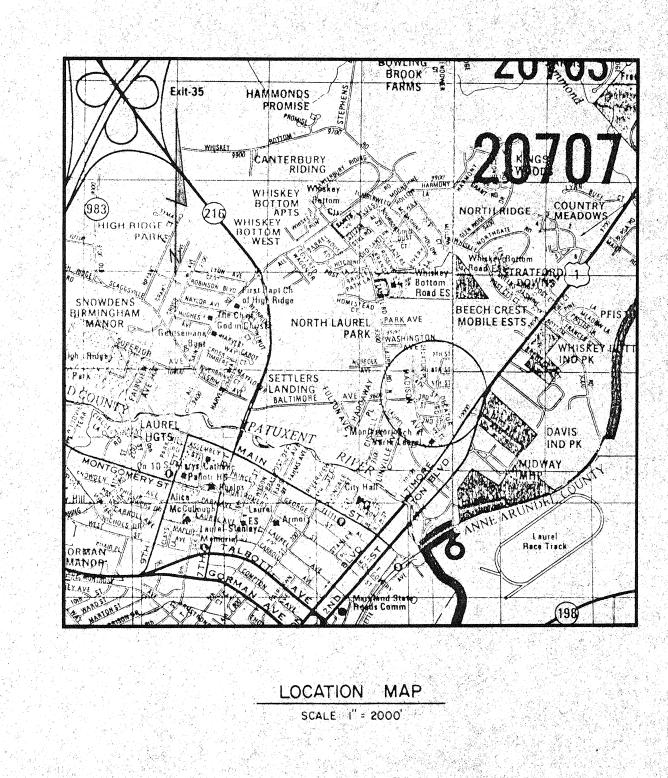
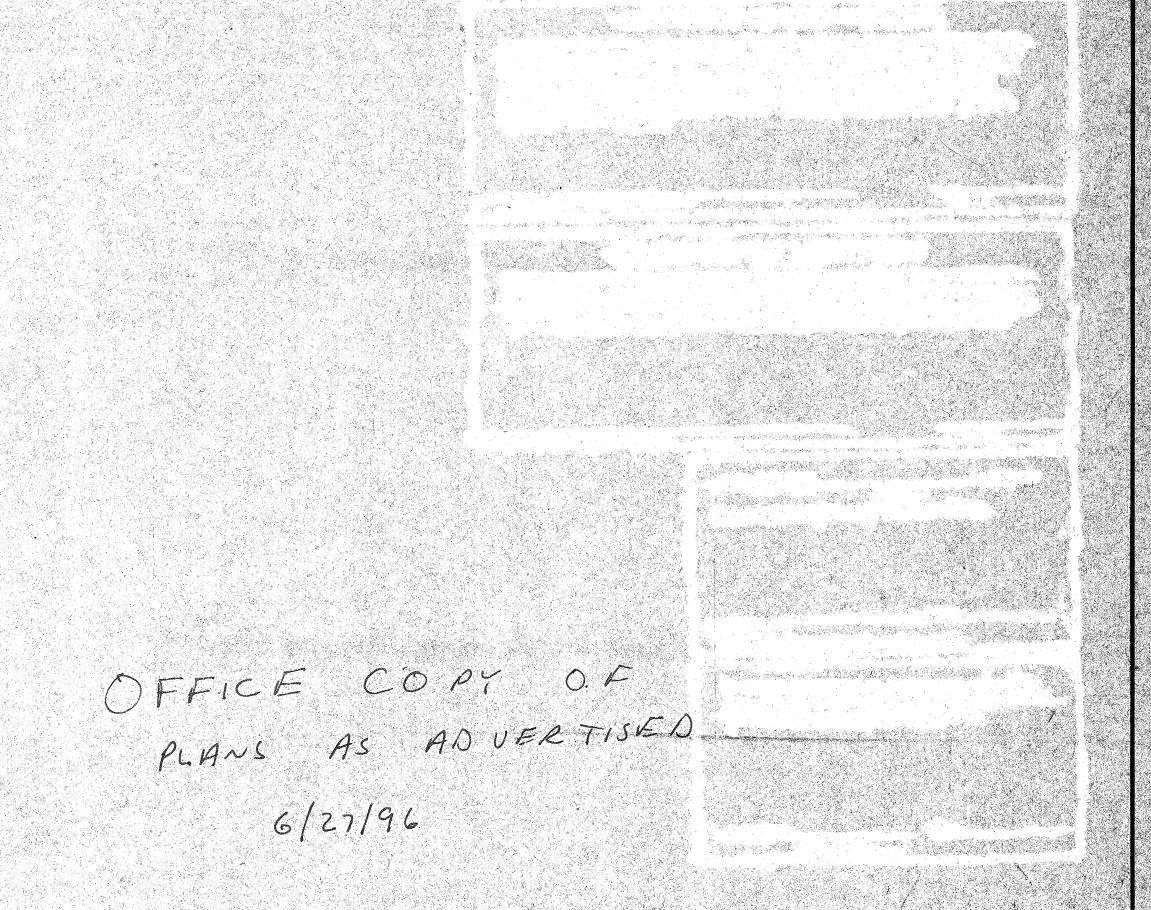
HOWARD COUNTY, MARYLAND DEPARTMENT OF PUBLIC WORKS





NORTH LAUREL DRAINAGE IMPROVEMENT CAPITAL PROJECT D - 1081

General Notes:

- The contractor shall notify the Department of Public Works/Bureau of Construction Inspection at (410) 313-1870 at least five (5) working days prior to the start of work.
- Standard Details for this Contract shall be the Howard County Standard Details as Supplemented by the Maryland State Highway Administration Standard Details and Specifications.
- For details not shown on the drawings, and for materials and construction methods, the Contractor shall abide by the Howard County Design Manual IV. "Standard Specifications and Details for Construction and the special provisions. In the event of any discrepancy between these two sources, the latter shall govern.
- Failure to specify any work which would normally be required to complete the project shall not relieve the Contractor of his responsibility to perform
- Approximate location of existing utilities are shown for the Contractor's information from the best information available and are not guaranteed. Neither the County nor the Engineer accepts any liability or cost claims for utilities not shown, miscalculated, utilities under construction, or work which may go in place during execution of this contract. The Contractor shall locate all existing utilities by test pitting well in advance of construction activities to determine horizontal and vertical alignment and take all necessary precautions to protect the existing utilities and maintain uninterrupted service. Cost of test pitting shall be included in the unit price hid for Jest Pit. Item. Any damage incurred due to the Contractor's operation shall be repaired or replaced immediately at the Contractor's expense. Clear all utilities by a minimum of 6-inches vertically.
- Where test pits have been made on existing utilities. They are noted by the symbol # . The results of test pits are included on the storm drain plans.
- All utility poles must be cleared by 5 feet. If the storm drain piping or structure work is within five feet of a utility pole, the pole must be braced.
- All top elevations for the proposed inlets and manholes are approximate, and are to be verified in the field by the Contractor and the Engineer. Where the proposed storm drains cross vegetated areas, positive drainage shall be maintained and the top elevations adjusted accordingly as directed by the Engineer.
- Location points for Inlets, Manholes and Structures:

ITEM	HORIZONTAL LOC	VERTICAL LO
Curb Type Inlets	Center Face of Curb	Top of Curb
Grate Type Inlets	Center of Grate	Top of Grate
Manholes	Center of Cover	Top of Cover
Endwalls	Center of Wall	Top of Wall

- Topography is based on field surveys dated July 1989, June 1990, July 1993, and August 1994, performed by Purdum and Jeschke. The contractor will be responsible for verification of any discrepancies. Elevations shown are based on Howard County Surveys Datum and vertical controls are based on USGS datum of 1929. Coordinates shown are referred to the Howard County Department of Public Works Coordinate System.
- 11. All pipe elevations shown are invert elevations.
- Trees are to be protected from damage to maximum extent. Trees located outside the construction strip are not to be removed or damaged by the contractor.
- Contractor shall remove trees, stumps and roots along line of excavation as directed by the engineer. Payment for such removal shall be included in the unit price bid for furnishing and laying storm drain pipe.
- The Contractor shall be responsible for the restoration of all existing driveways damaged during construction. See measurements and payment in special provisions. Existing bituminous concrete driveway shall be restored as follows:
 - 4—inch depth of Bituminous Concrete Base Course 1-inch depth of Bituminous Concrete Surface Course
 - Concrete driveways shall be replaced with a typical section as shown on Standard Detail Ř 6.03.
- Existing water house services, that are in conflict with the proposed storm drainage facilities, shall be adjusted by the Contractor.
- Existing fences, mailboxes, signs and shrubs disturbed by the work shall be reconstructed or replaced in kind.
- All slopes and/or disturbed areas shall receive 2-inch depth of topsoil and sodding except, where otherwise indicated on the plans, or as directed by the engineer. All grading shall be done in a manner to insure positive
- All construction shall be in accordance with the latest standards and specifications of Howard County and MSHA standards and specifications. if applicable.
- Contractor shall notify the following utilities or agency at least (5) days before starting work shown on these plans: Miss Utility 1-800-257-7777 Baltimore Gas and Electric Company — Underground Electric Distribution Engineering "Damage Control" 234—6313 Baltimore Gas and Electric Company — Underground Gas Distribution Engineering "Damage Control" 234—5533 Chesapeake and Potomac Telephone Company 597—8585 Howard County Cable T.V. 461-1156 Howard County Division of Traffic Engineering 313-2430 Howard County Bureau of Utilities 313-4900 Howard County Division of Construction Inspection 313-1870
- Right of Way lines shown on these plans are shown for assistance in interpreting the plans. For any Fee Right of Way and Easement information see Right of Way Plats.

CHIEF, TRANSPOTATION AND

- 21. Light poles and fixtures for street lights shall be in accordance with the latest Howard County Design Manual, Volume III, Roads and Bridges.
- 22. Place regulation "Men Working" and warning signs as required to comply with the latest editions of the Maryland State Highway Administration Manual of Traffic Control for Highway Construction and Maintenance Operations, and the Manual on Uniform Traffic Control Devices and all subsequent addendum.
- Geotechnical investigation was performed by E2SI in 1990 and summarized in a report titled "Subsurface Investigation North Laurel Storm Drain Improvements" dated October 1990. Additional test pits were performed by Froehling and Robertson, Inc. in October 1991.
- Wetland study was conducted in May 1994 and no wetlands were found.
- Stormwater management control is not part of this project since the various regulatory storm events are not adversely impacted by this project.
- 14"x 9" CMPA is 12 gauge throughout construction and manufactured per AASHTO M-36. Lengths indicated for the 14"x 9" CMPA include end sections.
- It is the Contractor's responsibility to schedule all utility relocations and pole bracings, sufficiently in advance of his activities to avoid conflict or delays.
- Use 6 inch depth of select material, 57 stone or approved equal under all culvert pipe / box as bedding.

INDEX OF SHEETS

SHEET	1	TITLE SHEET
SHEET	2	INDEX & NOTES
SHEET	3	PLAN
SHEET	4	PLAN
SHEET	5	PLAN
SHEET	6	PLAN
SHEET	7	CHARTS
SHEET	8	PROFILES
SHEET	9	PROFILES
SHEET	10	PROFILES
SHEET	11	DETAILS - ENDWALL
SHEET	12	DETAILS - HEADWALL
SHEET	13	GRADING PLAN
SHEET	14	GRADING PLAN
SHEET	15	GRADING PLAN
SHEET	16	SEDIMENT & EROSION CONTROL
SHEET	17	SEDIMENT & EROSION CONTROL
SHEET	18	SEDIMENT & EROSION CONTROL
SHEET	19	SEDIMENT & EROSION CONTROL DETAILS
SHEET	20	SEDIMENT & EROSION CONTROL DETAILS
SHEET	21	6" WATER RELOCATION

THIS DEVELOPMENT PLAN IS APPROVED FOR SOIL EROSION AND SEDIMENT CONTROL BY THE HOWARD SOIL CONSERVATION DISTRICT.	
APPROVED DATE HOWARD S.C.D.	4

REVIEWED FOR HOUARD	S.C.D.	
AND MEETS TECHNICAL REQUIREMENTS		
Q.G. Crafieldie	15/96	
SIGNATURE DAT	Έ/	
U.S. SOIL CONSERVATION SERVICE		

600' SCALE MAP NO.

LEGEND

cc	Ex. Catv Light Cable
	Ex. Contours
	Ex. Storm Drain
	Prop. Contours
TITE	Ex Guard Rail
	Prop. Guard Rail
	Prop. Storm Drain
	Prop. Gabions
392950	Prop. Riprap
ττττ	Elec. and Tele. Poles
	Guy Wire
	Light Pole
	Test Pit Location
•	Test Boring Location
	Bench Mark

HOWARD SOIL CONSERVATION DISTRICT

DEVELOPER & ENGINEER CERTIFICATES

() By the Developer:

"I/We certify that all development and/or construction will be done according to these plans, and that any responsible personnel involved in the construction project will have a Certificate of Attendance at a Department of the Environment Approved Training Program for the Control of Sediment and Erosion before beginning the project. I shall engage a registered professional engineer to supervise pond construction and provide the Howard Soil Conservation District with an "as-built" plan of the pond within 30 days of completion. I also authorize periodic on-site inspections by the Howard Soil Conservation

hald of	Epom
Signature (of)	Developer
Fonald G	.Lepson
	below signature

() By the Engineer:

BLOCK NO.

"I certify that this plan for pond construction, erosion and sediment control represents a practical and workable plan based on my personal knowledge of the site conditions. This plan was prepared in accordance with the requirements of the Howard Soil Conservation District. I have notified the developer that he/she must engage a registered professional engineer to supervise pond construction and provide the Howard Soil Conservation District with an "as-built" plan of the pond within 30 days of completion

			/	
Kiel and H. Bereil		7/2	5/95	
 Signature of Engineer	•	Dat	e	
Richard H. Berich	<u>.</u>			
Print name below signature			and the second	vila ĝe

Print name below signature

NORTH LAUREL DRAINAGE IMPROVEMENT CAPITAL PROJECT D-1081

DEPARTMENT OF PUBLIC WORKS

Howard County Surveying and Drafting Division 313-2417

DIRECTOR OF PUBLIC WORKS InopenMi Saufler 6-13-96

CHEIF, BUREAU OF HIGHWAYS

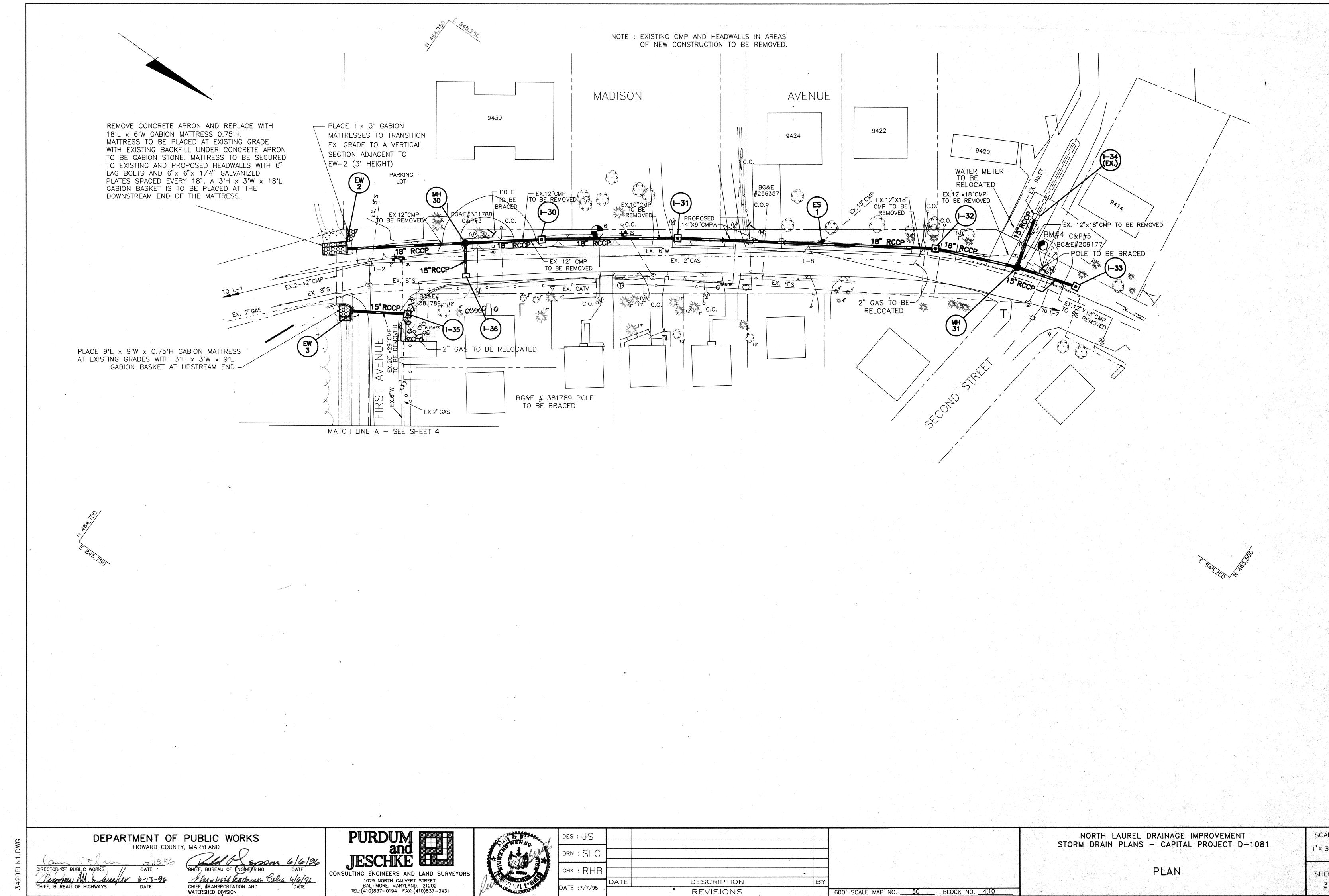
PURDUM & JESCHKE CONSULTING ENGINEERS LAND SURVEYORS 1029 North Calvert Street Baltimore, Maryland 21202 Tel: (410)837-0194 Fax:(410)837-343



	DATE. 7\7\95	BY	NO.	REVISION	DATE
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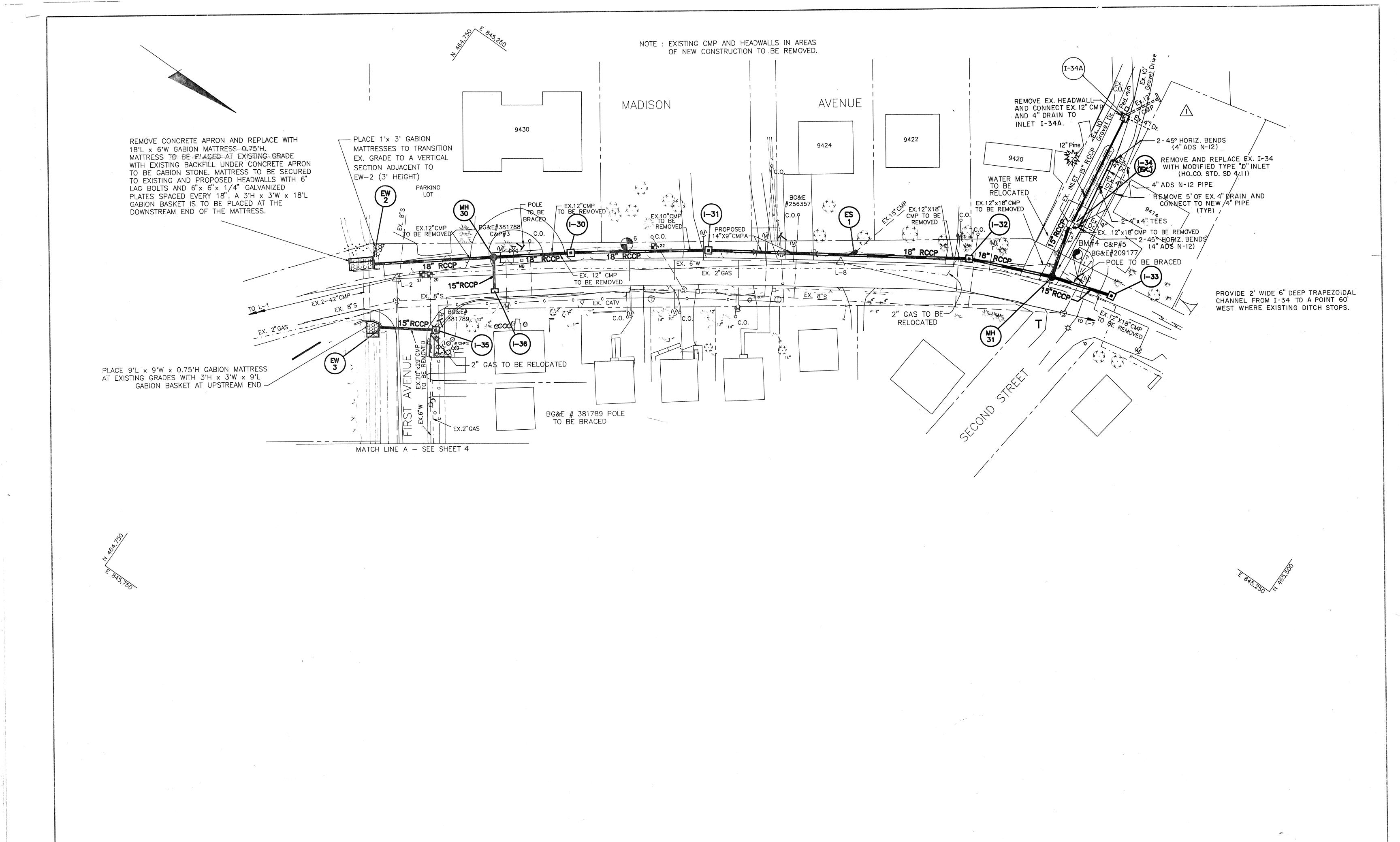


DATE :7/7/95

REVISIONS

P&J W.O.# 3420-00

600' SCALE MAP NO. BLOCK NO. 4,10



DEPARTMENT OF PUBLIC WORKS

1029 NORTH CALVERT STREET BALTIMORE, MARYLAND 21202 TEL: (410)837-0194 FAX: (410)837-3431

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NORTH LAUREL DRAINAGE IMPROVEMENT STORM DRAIN PLANS - CAPITAL PROJECT D-1081

PLAN

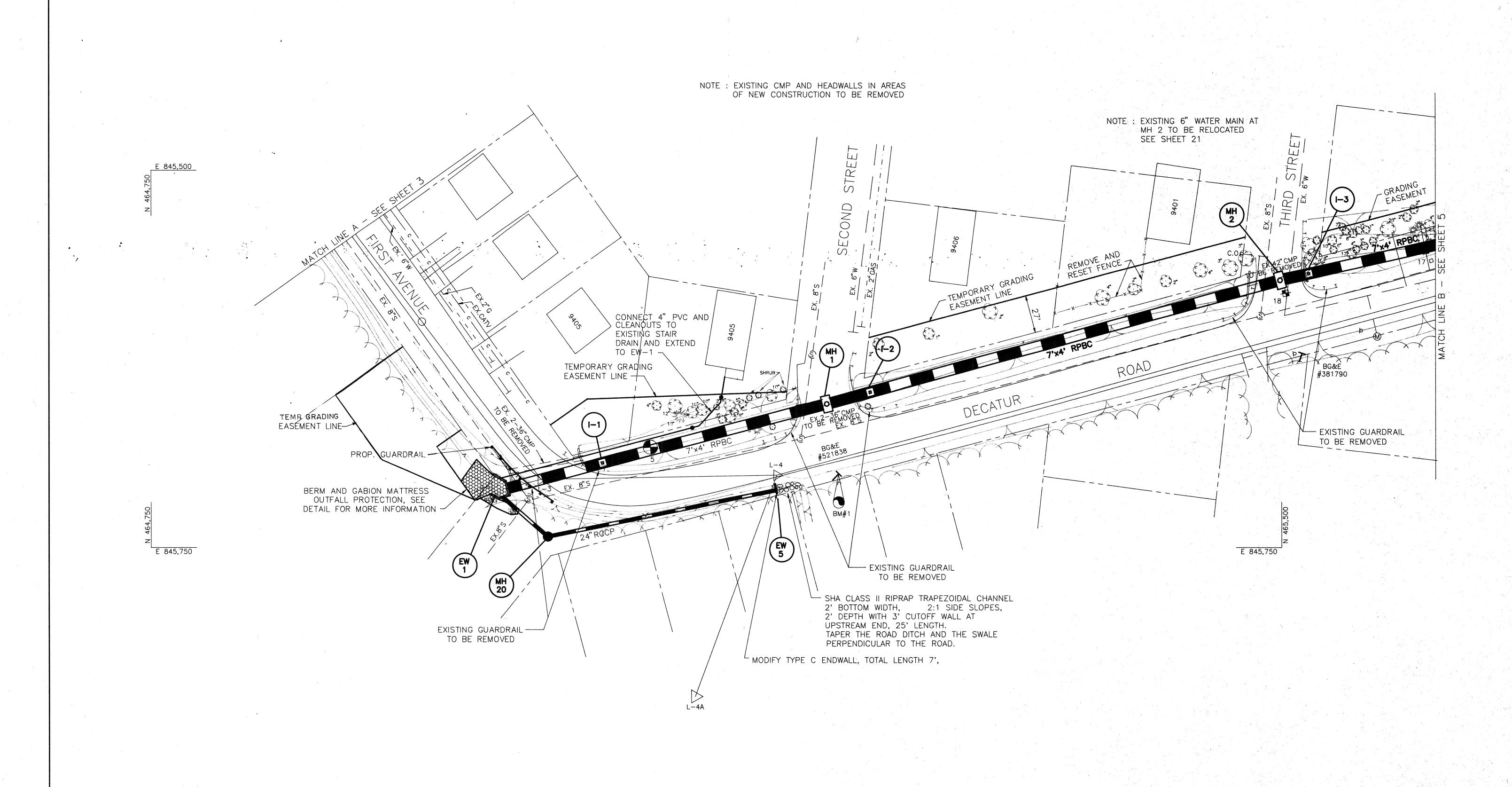
SHEET

SCALE

1" = 30 '

BLOCK NO. <u>4,10</u>

600' SCALE MAP NO. _____50___



FOR GRADING DETAIL SEE SHEET 13

DEPARTMENT OF PUBLIC WORKS
HOWARD COUNTY, MARYLAND

CHIEF, BUREAU OF HIGHWAYS DATE

CHIEF, TRANSPORTATION AND DATE WATERSHED DIVISION

CONSULTING ENGINEERS AND LAND SURVEYORS

1029 NORTH CALVERT STREET

BALTIMORE, MARYLAND 21202

TEL: (410)837-0194 FAX: (410)837-3431



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600' SCALE MAP NO. 50

BLOCK NO. 4,10

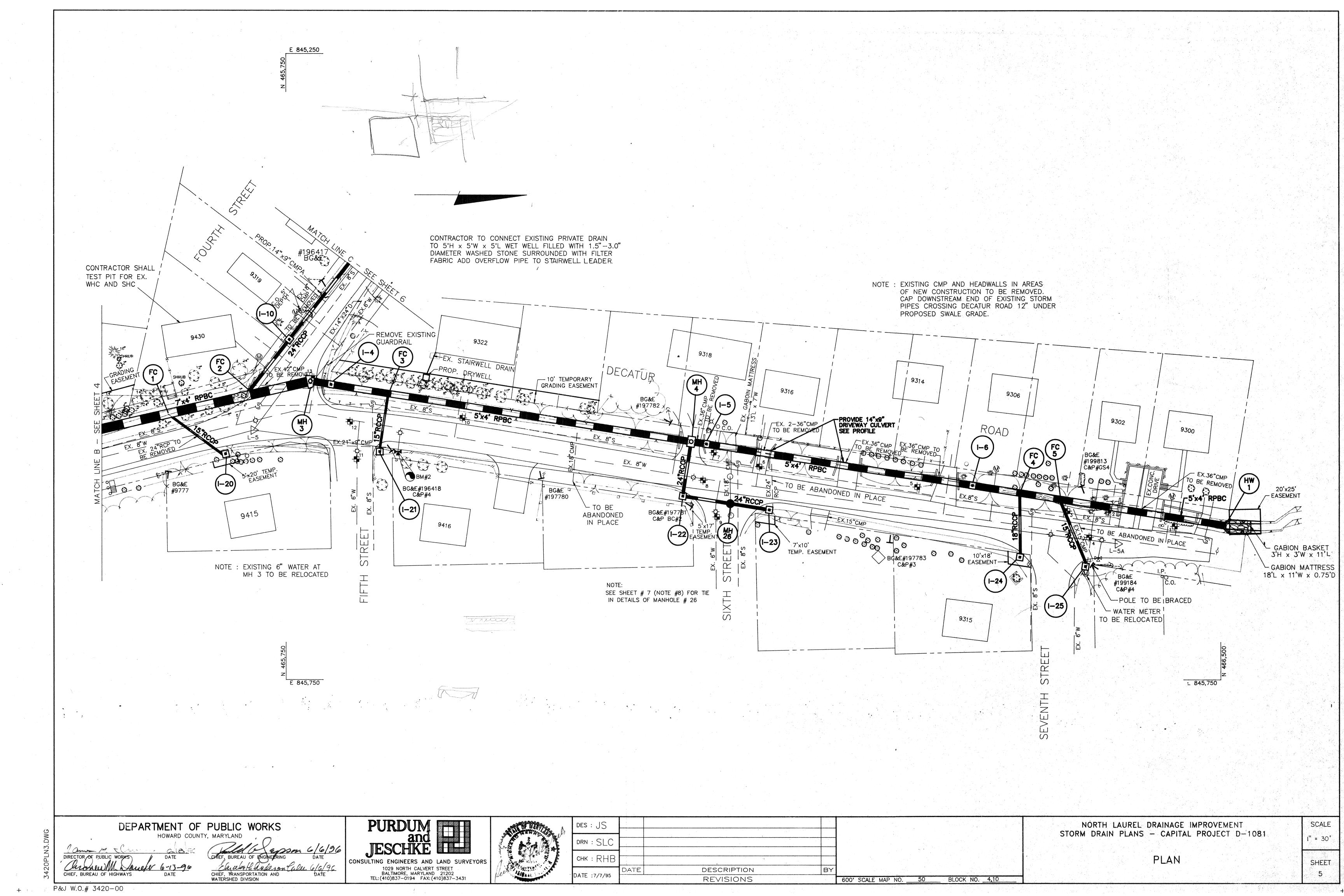
NORTH LAUREL DRAINAGE IMPROVEMENT STORM DRAIN PLANS - CAPITAL PROJECT D-1081

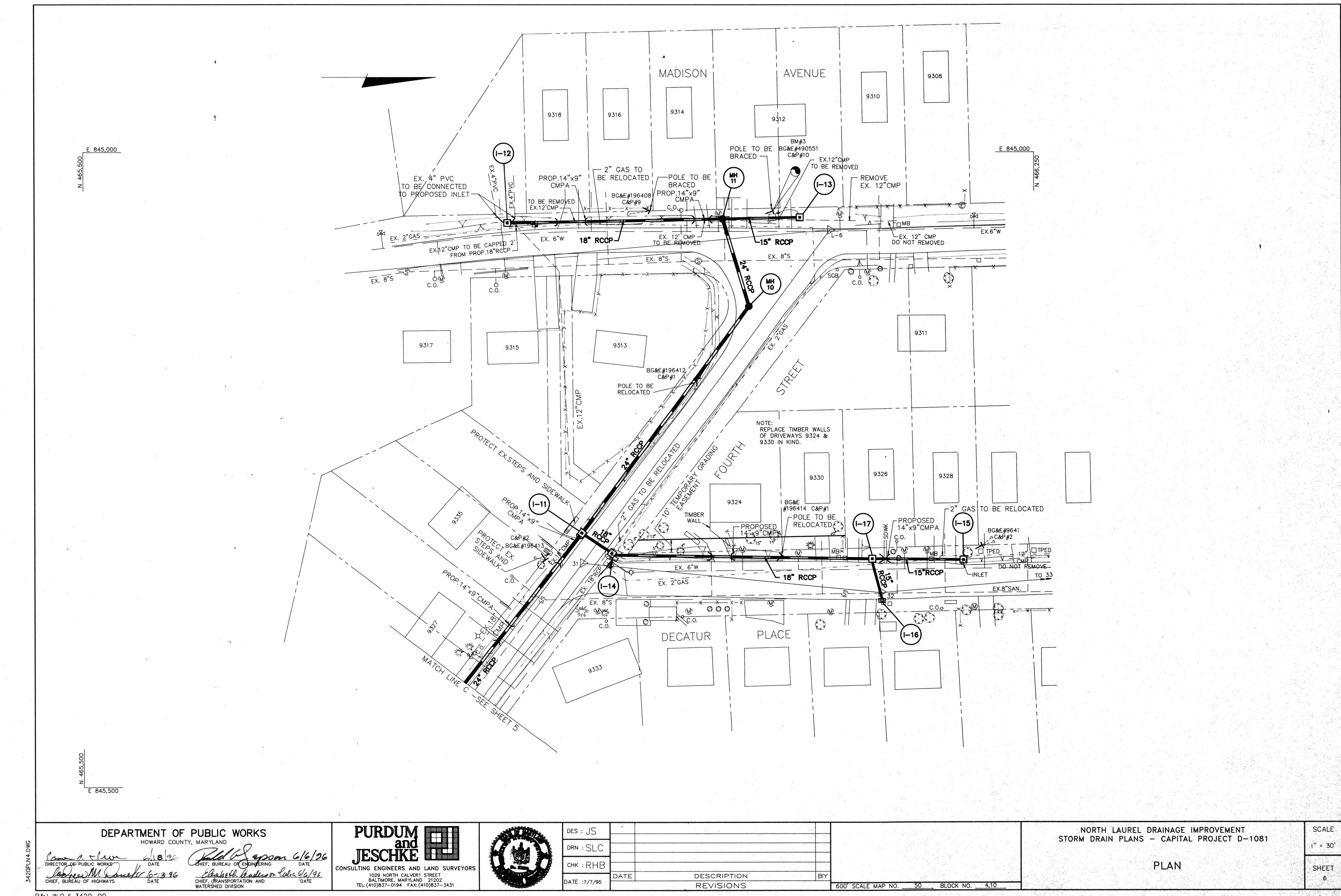
PLAN

P&J W.O.# 3420-00

ı" = 30' SHEET

SCALE





P&J W.O.# 3420-00

		STRUCTURE LOCATION	<u> </u>
NO.	TRAVERSE POINT NO.	BEARING	DISTANCE
I - 1	L-3	N 26° 59' 07" W	34.80'
1-2	L-4	N 41° 31′ 33″ W	83.20'
1-3	L-5	S 05° 05' 07" E	206.50
1-30	L-2	N 43° 16' 00" W	136.37'
I-31	L-8	S 29° 56′ 54″ E	102.71
1-32	L-8	N 36° 22' 44" W	95.68'
1-33	L-8	N 27° 57' 46" W	206.16
1-35	L-2	N 15° 38′ 44″ E	48.23'
MH-1	L-4	N 54° 54' 28" W	57.84
MH-2	L-5	S 05° 45' 15" E	225.80'
MH-30	L-2	N 47° 14' 08" W	76.77
			 158:61'
MH-31	L-8	N 31° 22′ 49″ W	
EW-1	L-3	S 15° 02' 39" E	23.54'
EW-2	L-2	S 03° 44' 07" E	21.08'
EW-3	L-2	N 74° 14' 02" E	39.67 '
ES-1	L-8	N 69° 24' 13" W	15.11'
	DRAINAGE TRAVERSE	STRUCTURE LOCATI	ONS DISTANCE
NO.	POINT NO.	BEARING	
1-4	L-5	N 30° 22′ 59″ W	72.36'
I-5 	L-5A	S 13° 39′ 53″ W	339.57'
1–6 ———	L-5A	S 21° 49′ 29″ W	125.31
I-10	L-5	N 68° 16′ 24″ W	77.94
I-11	31	S 88° 51 ′ 45″ W	23.29'
I-12	L-6	S 01' 15' 20" W	254.81
I-13	L-6	S 22° 34′ 46″ W	25.79'
1-14	31	N 18° 16′ 02″ W	24.37'
I - 15	32	N 21° 43′ 40″ W	69.80'
1-16	32	N 82° 38′ 54″ E	6.25
I - 17	32	S 75° 40' 29" W	27.62
1-20	L-5	S 40° 21′ 32″ E	29.46
1-21	L-5	N 09° 35′ 13″ E	102.93
1-22	L-5	N 08° 46′ 42″ E	348.28
1-23	L-5A	S 05° 28′ 53″ W	280.83
l-24	L-5A	S 07° 51′ 37″ E	78.97
1-25	L-5A	S 35° 13' 12" E	31.52'
MH-3	L-5	N 40° 33′ 37″ W	60.23'
MH-4	L-5	N 01° 26' 32" E	351.27
MH-10		S 43° 13' 46" E	88.06
MH-11			
	<u>.</u>	S 05° 52′ 43″ W	85.25'
HW-1	L-5A	N 09° 45′ 47″ W	87.14
1-36	L-2	N 27° 48′ 16″ W	76.46
EW-5	L-4	N 87° 40′ 36″ E	12.44'
MH-20	L-3	N 83° 04' 28" E	35.36'
FC-1	L-5	S 11° 31' 04" W	69.76
FC-2	L-5	S 82° 06′ 34″ W	33.07'
FC-3	L-5	N 16° 06′ 12″ W	114.13
FC-4	L-5A	S 27° 54′ 55″ W	88.15
FC-5	L-5A	S 35° 58' 25" W	55.65

		TEST PIT INF	ORMATION	
NO.	UTILITY	NORTH	EAST	TOP ELEVATION
1	6" SHC	466,463.0	845,627.3	176.33
2	6" SHC	466,409.7	845,614.3	175.73
	3/4" WHC	466,416.3	845,613.3	177.49
3	4" SHC	466,377.0	845,615.0	171.83
4	8" WATER	466,394.3	845,636.2	175.99
5	6" SHC	466,256.3	845,595.8	169.75
6	6" SHC	466,129.9	845,579.1	170.75
7	6" SHC	466,093.1	845,568.0	165.97
8	8" WATER	466,084.4	845,590.3	166.80
9	6" WATER	466,095.7	845,619.4	167.22
10	6" SHC	465,891.5	845,540.4	162.72
11	6" WATER	465,806.7	845,573.5	167.12
12	8" WATER	465,801.5	845,543.6	165.16
13	6" WATER	465,772.5	845,508.9	161.70
	2" GAS	465,915.4	845,324.6	182.76
14	6" WATER	465,920.5	845,317.4	180.58
15	6" WATER	466,113.3	845,061.8	217.42
1.0	6" WATER	465,859.4	845,062.9	202.53
16	2" GAS	465,856.6	845,056.3	205.37
17	6" SHC	465,595.9	845,553.3	159.70
18	6" WATER	465,502.9	845,581.5	158.96
20	2" GAS	464,828.7	845,423.6	147.37
21	6" WATER	464,823.2	845,426.5	144.82
22	6" SHC	464,958.2	845,306.3	158.70

	SURVEY TRAVERSE				
NO.	NORTH	EAST			
L-2	464,809.177	845,440.788			
L-3	465,008.506	845,704.470			
L-4	465,165.003	845,701.579			
L-4A	465,110.861	845,848.137			
L-5	465,723.828	845,550.436			
L-5A	466,417.114	845,641.337			
L-6	466,089.348	845,060.706			
L-7	465,345.432	845,127.426			
L-8	465,083.467	845,233.460			
31	465,893.705	845,322.832			
32	466,128.937	845,346.302			

	SOIL BORING	TABLE
NO.	NORTH	EAST
1	466,361.9	845,601.7
2	465,923.7	845,543.1
3	465,976.2	845,321.4
4	466,044.8	845,141.1
5	465,081.4	845,683.0
6	464,940.0	845,317.0

	BENCH MARK TABLE	
NO.	DESCRIPTION	ELEV.
1	RR SPIKE IN BG&E#521838	157.80
2	1	172.21
3	RR SPIKE IN BG&E#490551	220.02
4A	SQ. (BOX) CUT ON S.S. MH RIN	AT INT.
	MADISON & SECOND ST.	188.89

PR	OFILE !	NDEX	
PROFIL	_E	SHEET	NO.
EW-1 TO	HW-1	8	
EW-2 TO	1-33	9	
I-34 TO 1	иH-31	9	
EW-3 TO	I -3 5	9	
EW-1 TO	EW-5	8	
I-20 TO	FC-1	9	
I-21 TO	FC-3	10	
I-23 TO	MH-4	9	
I-24 TO	FC-4	9	-
I-25 TO	FC-5	9	
I-36 TO I	иH-30	9	
I-12 TO	FC-2	10	
I-11 TO	I - 15	10	
I-13 TO	MH-11	10	
I-16 TO	I-17	10	
		1	

EW-5 TO EW-1

- (1) STRUCTURE TO BE MODIFIED TO PROVIDE INCOMING AND OUTGOING PIPES PLUS CHANNELING.
- ② SEE SHEET 10, 11 FOR DETAILS.
- ③ OPTION TO USE PRECAST MANHOLE G5.12
- (4) PROPOSED ENDWALL TO BE JOINED TO EXISTING CONCRETE HEADWALL BY DOWELS SPACED EVERY 12".
- (5) INLET FRAME AND COVER TO BE SECURED TO INLET WALL WITH FRAME SPANNING OUTER DIMENSIONS AND A CONCRETE CURB IS NOT USED ATTACHED WITH 4-5/8" Ø ANCHORS PER MARYLAND STANDARD # 374.02.
- 6 SEE SPECIFICATION FOR DETAILS.
- ① USE RECT. LINEAR GRATE SD 4.94
- (8) STORM TIE-IN MANHOLE, CUT 18" CMP 24 INCHES FROM DOWNSTREAM SIDE OF MANHOLE AND CAP. CUT 18"CMP AT UPSTREAM SIDE OF TIE-IN MANHOLE TO PROVIDE FLUSH CONNECTION AT INSIDE EDGE OF MANHOLE

NOTES: EXISTING GUARDRAIL AT FIRST, SECOND, THIRD AND FOURTH STREETS AND DECATUR ROAD TO BE REMOVED. NEW GUARDRAIL TO BE INSTALLED AT FIRST AVENUE AND DECATUR ROAD.

FOR GRADING PLAN SEE SHEET 13, 14 & 15.

NO.

* NOTE: 1-34 TO BECOME A THROUGH INLET. PROVIDE CHANNELING AS NECESSARY. PLACE #4 REBARS AT 6" m VERTICAL (MOD. TYPE "D" SPACING IN INLET OPENING.

ELEVATION SD 4.21² MOD. TYPE "E" INLET MOD. TYPE "E" SD 4.21(2) 155.63 MOD. TYPE "E" 163.50 SD 4.21 ② INLET MOD. TYPE "E" 7) SD 4.21 🛈 159.60 152.00 151.80 MOD. TYPE "E" SD 4.21 ① 167.70 162.33 162.13 INLET MOD. TYPE "E" 181.50 177.68 SD 4.21 (177.48 1 - 32(181.70) TYPE "E" 1-33 186.78 SD 4.21 190.50 SD 4.11 182:05 I-34 EXISTING INLET 184.88 181.80 SEE NOTE * MOD. TYPE SD 4.37 (5) 148.50 146.08 1-35 "WR" INLET TYPE "E" 154.00 SD 4.31 149.19 1 - 36COMB. INLET SPEC. CAST-SPECIAL (2) 149.20 156.80 149.40 DESIGN IN-PLACE MH 157.98 158.18 164.70 DESIGN (2) IN-PLACE MH G 5.01 🕄 MH-30 4' BRICK MH 148.22 155.50 148.67 G 5.01 ③ 187.20 MH-31 4' BRICK MH 181.19 SPECIAL 26
DESIGN 146.70 EW-1ENDWALL TYPE "C" SD 5.21 (4) 145.80 EW-2ENDWALL SD 5.21 (4) 145.00 EW-3**ENDWALL** EX. 12" CMF TYPE SD 4.22 1-34A 187.20 182.50 INLET 183.4 151.21 EW-5SD 5.21 ENDWALL G5.01 ③ 149.77 153.50 149.97 MH-20 4" BRICK MH SD 5.61 175.03 ES-1 | END SECTION CONN. TO EX. 15" CMP MOD. TYPE "E" SD 4.21⁽²⁾ 167.60 1-4 INLET SD 4.21⁽²⁾ MOD. TYPE "E 173.15 INLET MOD. TYPE "E SD 4.21⁽²⁾ 177.77 1-6 MOD. TYPE "E 168.65 164.38 164.18 SD 4.21 INLET 178.49 SD 4.21 177.97 INLET TYPE "E" 206.30 202.70 SD 4.21 INLET 218.10 212.50 SD 4.21 1-13 INLET SD 4.21 (1) MOD. TYPE "E' 183.85 179.86 179.66 1-14 INLET MOD. TYPE SD 4.37 (5) 210.00 1-15 207.53 "WR" INLET 1 - 16208.47 204.95 SD 4.01 A-5MOD. TYPE 204.00 199.74 SD 4.37 (1) (5) 206.50 200.19 "WR" INLET SD 4.37 (5) 166.70 164.22 1-20 "WR" INLET MOD. TYPE 169.35 166.91 1 - 21SD 4.37 "WR" INLET SD 4.21① 1-22 172.80 168.27 INLET MOD. TYPE SD 4.37 (5) 172.50 169.25 "WR" INLET SD 4.37 (5) 178.40 175.69 "WR" INLET SD 4.37 (5) 179.70 176.20 "WR" INLET SPECIAL ②
DESIGN ② 169.20 161.86 161.66 IN-PLACE MH SPECIAL ②
DESIGN SPEC. CAST-167.27 174.30 MH-4167.47 IN-PLACE MH G 5.01 ③ 197.82 198.02 207.10 MH-10 4' BRICK MH 206.50 200.15 G 5.01 ③ 199.55 211.70 MH-11 4' BRICK MH SPECIAL 6 2 176.58 HW-1HEADWALL 162.00 15"RCCP TO 7'x4'RPBC 161.64 24"RCCP TO 7'x4'RPBC 164.00 15"RCCP TO 5'x4'RPBC 174.20 18"RCCP TO 5'x4'RPBC 175.08 15"RCCP TO 5'x4'RPBC CMP INVERT 171.88 168.80 168.90 174.80 TO 24"RCCP

DRAINAGE STRUCTURE SCHEDULE

INVERT

INVERT

REMARKS

DEPARTMENT OF PUBLIC WORKS

HOWARD COUNTY, MARYLAND Elevabeth anderson Cala 6/6/96 CHIEF, TRANSPORTATION AND DATE

CONSULTING ENGINEERS AND LAND SURVEYORS 1029 NORTH CALVERT STREET BALTIMORE, MARYLAND 21202

TEL: (410)837-0194 FAX: (410)837-3431



. [DES : JS	10/30/9	6 ADDITIONAL INLETS I-34 AND I-34A	AR
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) P	DRN : SLC			
			*	
F	снк : RHB			
		DATE	DESCRIPTION	B,
	DATE :7/7/95	}	REVISIONS	

600' SCALE MAP NO.

BLOCK NO.

NORTH LAUREL DRAINAGE IMPROVEMENT STORM DRAIN PLANS - CAPITAL PROJECT D-1081

CHARTS AND TABLES

SHEET

SCALE

NO. FOINT NO. BEARING DISTANCE I-1		DRAINAGE	STRUCTURE LOCATI	ONS
I-1	NO.		BEARING	DISTANCE
I-3	I-1		N 26° 59' 07" W	34.80'
I-30	1-2	L-4	N 41° 31′ 33″ W	83.20'
I-31	1–3	L-5	S 05' 05' 07" E	206.50'
I-32	1-30	L-2	N 43° 16' 00" W	136.37'
I-33	I-31	L-8	S 29° 56' 54" E	102.71
N 15' 38' 44" E	1-32	L-8	N 36° 22' 44" Ŵ	95.68'
MH-1 L-4 N 54' 54' 28" W 57.84' MH-2 L-5 S 05' 45' 15" E 225.80' MH-30 L-2 N 47' 14' 08" W 76.77' MH-31 L-8 N 31' 22' 49" W 158.61' EW-1 L-3 S 15' 02' 39" E 23.54' EW-2 L-2 S 03' 44' 07" E 21.08' EW-3 L-2 N 74' 14' 02" E 39.67' ES-1 L-8 N 69' 24' 13" W 15.11' DRAINAGE STRUCTURE LOCATIONS NO. TRAVERSE FOINT NO. BEARING DISTANCE I-4 L-5 N 30' 22' 59" W 72.36' I-5 L-5A S 13' 39' 53" W 339.57' I-6 L-5A S 21' 49' 29" W 125.31' I-10 L-5 N 68' 16' 24" W 77.94' I-11 31 S 88' 51' 45" W 23.29' I-12 L-6 S 22' 34' 46" W 25.79' I-14 31 N 18' 16' 02" W 24.37' I-15 32	1–33	L-8	N 27° 57' 46" W	206.16
MH-2 L-5 S 05' 45' 15" E 225.80' MH-30 L-2 N 47' 14' 08" W 76.77' MH-31 L-8 N 31' 22' 49" W 158.61' EW-1 L-3 S 15' 02' 39" E 23.54' EW-2 L-2 S 03' 44' 07" E 21.08' EW-3 L-2 N 74' 14' 02" E 39.67' ES-1 L-8 N 69' 24' 13" W 15.11' DRAINAGE STRUCTURE LOCATIONS NO. TRAVERSE BEARING DISTANCE I-4 L-5 N 30' 22' 59" W 72.36' I-5 L-5A S 13' 39' 53" W 339.57' I-6 L-5A S 21' 49' 29" W 125.31' I-10 L-5 N 68' 16' 24" W 77.94' I-11 31 S 88' 51' 45" W 23.29' I-12 L-6 S 01' 15' 20" W 254.81' I-13 L-6 S 22' 34' 46" W 25.79' I-14 31 N 18' 16' 02" W 24.37' I-15 32 <t< td=""><td>1-35</td><td>L-2</td><td>N 15° 38' 44" E</td><td>48.23</td></t<>	1-35	L-2	N 15° 38' 44" E	48.23
MH-30 L-2 N 47' 14' 08" W 76.77' MH-31 L-8 N 31' 22' 49" W 158.61' EW-1 L-3 S 15' 02' 39" E 23.54' EW-2 L-2 S 03' 44' 07" E 21.08' EW-3 L-2 N 74' 14' 02" E 39.67' ES-1 L-8 N 69' 24' 13" W 15.11' DRAINAGE STRUCTURE LOCATIONS NO. TRAVERSE STRUCTURE LOCATIONS NO. TRAVERSE STRUCTURE LOCATIONS NO. TRAVERSE STRUCTURE LOCATIONS I-5 L-5A S 13' 39' 53" W 339.57' I-6 L-5A S 13' 39' 53" W 339.57' I-6 L-5A S 13' 39' 53" W 339.57' I-10 L-5 N 68' 16' 24" W 77.94' I-11 31 S 88' 51' 45" W 23.29' I-14 31 N 18' 16' 24" W 25.79' I-14 31 N 18' 16' 02" W 24.37' I-15 32 N 21' 43' 40" W 69.80' I-16 <t< td=""><td>MH-1</td><td>L-4</td><td>N 54° 54' 28" W</td><td>57.84</td></t<>	MH-1	L-4	N 54° 54' 28" W	57.84
MH-31 L-8 N 31' 22' 49" W 158.61' EW-1 L-3 S 15' 02' 39" E 23.54' EW-2 L-2 S 03' 44' 07" E 21.08' EW-3 L-2 N 74' 14' 02" E 39.67' ES-1 L-8 N 69' 24' 13" W 15.11' DRAINAGE STRUCTURE LOCATIONS NO. TRAYERSE FOINT NO. BEARING DISTANCE I-4 L-5 N 30' 22' 59" W 72.36' I-5 L-5A S 13' 39' 53" W 339.57' I-6 L-5A S 21' 49' 29" W 125.31' I-10 L-5 N 68' 16' 24" W 77.94' I-11 31 S 88' 51' 45" W 23.29' I-12 L-6 S 01' 15' 20" W 25.79' I-14 31 N 18' 16' 02" W 25.79' I-14 31 N 18' 16' 02" W 24.37' I-15 32 N 21' 43' 40" W 69.80' I-16 32 N 82' 38' 54" E 6.25' I-17 32	MH-2	L-5	S 05° 45' 15" E	225.80'
EW-1	MH-30	L-2	N 47° 14' 08" W	76.77
EW-2	MH-31	L-8	N 31° 22′ 49″ W	158.61
EW-3 L-2 N 74' 14' 02" E 39.67' ES-1 L-8 N 69' 24' 13" W 15.11' DRAINAGE STRUCTURE LOCATIONS NO. TRAVERSE POINT NO. BEARING DISTANCE I-4 L-5 N 30' 22' 59" W 72.36' I-5 L-5A S 13' 39' 53" W 339.57' I-6 L-5A S 21' 49' 29" W 125.31' I-10 L-5 N 68' 16' 24" W 77.94' I-11 31 S 88' 51' 45" W 23.29' I-12 L-6 S 01' 15' 20" W 254.81' I-13 L-6 S 22' 34' 46" W 25.79' I-14 31 N 18' 16' 02" W 24.37' I-15 32 N 21' 43' 40" W 69.80' I-16 32 N 82' 38' 54" E 6.25' I-17 32 S 75' 40' 29" W 27.62' I-20 L-5 S 40' 21' 32" E 29.46' I-21 L-5 N 09' 35' 13" E 102.93' I-22 L-5	EW-1	L-3	S 15° 02' 39" E	23.54
EW-3 L-2 N 74' 14' 02" E 39.67' ES-1 L-8 N 69' 24' 13" W 15.11' DRAINAGE STRUCTURE LOCATIONS NO. TRAVERSE POINT NO. BEARING DISTANCE I-4 L-5 N 30' 22' 59" W 72.36' I-5 L-5A S 13' 39' 53" W 339.57' I-6 L-5A S 21' 49' 29" W 125.31' I-10 L-5 N 68' 16' 24" W 77.94' I-11 31 S 86' 51' 45" W 23.29' I-12 L-6 S 01' 15' 20" W 254.81' I-13 L-6 S 22' 34' 46" W 25.79' I-14 31 N 18' 16' 02" W 24.37' I-15 32 N 21' 43' 40" W 69.80' I-16 32 N 82' 38' 54" E 6.25' I-17 32 S 75' 40' 29" W 27.62' I-20 L-5 S 40' 21' 32" E 29.46' I-21 L-5 N 09' 35' 13" E 102.93' I-22 L-5	EW-2	L-2	S 03° 44' 07" E	21.08'
ES-1 L-8 N 69' 24' 13" W 15.11' DRAINAGE STRUCTURE LOCATIONS NO. TRAVERSE POINT NO. BEARING DISTANCE I-4 L-5 N 30' 22' 59" W 72.36' I-5 L-5A S 13' 39' 53" W 339.57' I-6 L-5A S 21' 49' 29" W 125.31' I-10 L-5 N 68' 16' 24" W 77.94' I-11 31 S 88' 51' 45" W 23.29' I-12 L-6 S 01' 15' 20" W 254.81' I-13 L-6 S 22' 34' 46" W 25.79' I-14 31 N 18' 16' 02" W 24.37' I-15 32 N 21' 43' 40" W 69.80' I-16 32 N 82' 38' 54" E 6.25' I-17 32 S 75' 40' 29" W 27.62' I-20 L-5 S 40' 21' 32" E 29.46' I-21 L-5 N 09' 35' 13" E 102.93' I-22 L-5 N 09' 35' 13" E 102.93' I-24 L-5A	EW-3	L-2	N 74° 14' 02" E	
DRAINAGE STRUCTURE LOCATIONS NO. TRAVERSE POINT NO. BEARING DISTANCE I-4 L-5 N 30' 22' 59" W 72.36' I-5 L-5A S 13' 39' 53" W 339.57' I-6 L-5A S 21' 49' 29" W 125.31' I-10 L-5 N 68' 16' 24" W 77.94' I-11 31 S 88' 51' 45" W 23.29' I-12 L-6 S 01' 15' 20" W 254.81' I-13 L-6 S 22' 34' 46" W 25.79' I-14 31 N 18' 16' 02" W 24.37' I-15 32 N 21' 43' 40" W 69.80' I-16 32 N 82' 38' 54" E 6.25' I-17 32 S 75' 40' 29" W 27.62' I-20 L-5 S 40' 21' 32" E 29.46' I-21 L-5 N 09' 35' 13" E 102.93' I-22 L-5 N 09' 35' 13" E 348.28' I-23 L-5A S 05' 28' 53" W 280.83' I-24 L-5A S 05' 28' 53" W 280.83' I-25 L-5A S 35' 13' 12" E 31.52' MH-3 L-5 N 40' 33' 37" W				
NO. POINT NO. BEARING DISTANCE -4				<u></u>
I-5	NO.		BEARING	DISTANCE
I-6	1-4	L-5	N 30° 22′ 59″ W	72.36'
I-10	1-5	L-5A	S 13° 39′ 53″ W	339.57
I-11 31	1-6	L-5A	S 21° 49′ 29″ W	125.31
I-12	I-10	L-5	N 68° 16′ 24″ W	77.94'
I-13	-11	31	S 88° 51′ 45″ W	23.29
I-14 31	I-12	L-6	S 01' 15' 20" W	254.81
I-15 32 N 21' 43' 40" W 69.80' I-16 32 N 82' 38' 54" E 6.25' I-17 32 S 75' 40' 29" W 27.62' I-20 L-5 S 40' 21' 32" E 29.46' I-21 L-5 N 09' 35' 13" E 102.93' I-22 L-5 N 08' 46' 42" E 348.28' I-23 L-5A S 05' 28' 53" W 280.83' I-24 L-5A S 07' 51' 37" E 78.97' I-25 L-5A S 35' 13' 12" E 31.52' MH-3 L-5 N 40' 33' 37" W 60.23' MH-4 L-5 N 01' 26' 32" E 351.27' MH-10 L-6 S 43' 13' 46" E 88.06' MH-11 L-6 S 05' 52' 43" W 85.25' HW-1 L-5A N 09' 45' 47" W 87.14' I-36 L-2 N 27' 48' 16" W 76.46' EW-5 L-4 N 87' 40' 36" E 12.44' MH-20 L-3 N 83' 04' 28" E 35.36'	1-13	L-6	S 22° 34′ 46″ W	25.79'
I-16 32 N 82' 38' 54" E 6.25' I-17 32 S 75' 40' 29" W 27.62' I-20 L-5 S 40' 21' 32" E 29.46' I-21 L-5 N 09' 35' 13" E 102.93' I-22 L-5 N 08' 46' 42" E 348.28' I-23 L-5A S 05' 28' 53" W 280.83' I-24 L-5A S 07' 51' 37" E 78.97' I-25 L-5A S 35' 13' 12" E 31.52' MH-3 L-5 N 40' 33' 37" W 60.23' MH-4 L-5 N 01' 26' 32" E 351.27' MH-10 L-6 S 43' 13' 46" E 88.06' MH-11 L-6 S 05' 52' 43" W 85.25' HW-1 L-5A N 09' 45' 47" W 87.14' I-36 L-2 N 27' 48' 16" W 76.46' EW-5 L-4 N 87' 40' 36" E 12.44' MH-20 L-3 N 83' 04' 28" E 35.36' FC-1 L-5 S 11' 31' 04" W 69.76'	1-14	31	N 18° 16′ 02″ W	24.37'
I-17 32 S 75' 40' 29" W 27.62' I-20 L-5 S 40' 21' 32" E 29.46' I-21 L-5 N 09' 35' 13" E 102.93' I-22 L-5 N 08' 46' 42" E 348.28' I-23 L-5A S 05' 28' 53" W 280.83' I-24 L-5A S 07' 51' 37" E 78.97' I-25 L-5A S 35' 13' 12" E 31.52' MH-3 L-5 N 40' 33' 37" W 60.23' MH-4 L-5 N 01' 26' 32" E 351.27' MH-10 L-6 S 43' 13' 46" E 88.06' MH-11 L-6 S 05' 52' 43" W 85.25' HW-1 L-5A N 09' 45' 47" W 87.14' I-36 L-2 N 27' 48' 16" W 76.46' EW-5 L-4 N 87' 40' 36" E 12.44' MH-20 L-3 N 83' 04' 28" E 35.36' FC-1 L-5 S 82' 06' 34" W 33.07' FC-2 L-5 S 82' 06' 34" W 33.07'	I-15	32	N 21° 43′ 40″ W	69.80'
I-20 L-5 S 40° 21′ 32″ E 29.46′ I-21 L-5 N 09° 35′ 13″ E 102.93′ I-22 L-5 N 08° 46′ 42″ E 348.28′ I-23 L-5A S 05° 28′ 53″ W 280.83′ I-24 L-5A S 07° 51′ 37″ E 78.97′ I-25 L-5A S 35° 13′ 12″ E 31.52′ MH-3 L-5 N 40° 33′ 37″ W 60.23′ MH-4 L-5 N 01° 26′ 32″ E 351.27′ MH-10 L-6 S 43° 13′ 46″ E 88.06′ MH-11 L-6 S 05° 52′ 43″ W 85.25′ HW-1 L-5A N 09° 45′ 47″ W 87.14′ I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	I-16	32	N 82° 38' 54" E	6.25'
I-21 L-5 N 09° 35′ 13″ E 102.93′ I-22 L-5 N 08° 46′ 42″ E 348.28′ I-23 L-5A S 05° 28′ 53″ W 280.83′ I-24 L-5A S 07° 51′ 37″ E 78.97′ I-25 L-5A S 35° 13′ 12″ E 31.52′ MH-3 L-5 N 40° 33′ 37″ W 60.23′ MH-4 L-5 N 01° 26′ 32″ E 351.27′ MH-10 L-6 S 43° 13′ 46″ E 88.06′ MH-11 L-6 S 05° 52′ 43″ W 85.25′ HW-1 L-5A N 09° 45′ 47″ W 87.14′ I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	1-17	32	S 75° 40′ 29″ W	27.62'
I-22 L-5 N 08° 46′ 42″ E 348.28′ I-23 L-5A S 05° 28′ 53″ W 280.83′ I-24 L-5A S 07° 51′ 37″ E 78.97′ I-25 L-5A S 35° 13′ 12″ E 31.52′ MH-3 L-5 N 40° 33′ 37″ W 60.23′ MH-4 L-5 N 01° 26′ 32″ E 351.27′ MH-10 L-6 S 43° 13′ 46″ E 88.06′ MH-11 L-6 S 05° 52′ 43″ W 85.25′ HW-1 L-5A N 09° 45′ 47″ W 87.14′ I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	1-20	L-5	S 40° 21′ 32″ E	29.46
I-23 L-5A S 05° 28′ 53″ W 280.83′ I-24 L-5A S 07° 51′ 37″ E 78.97′ I-25 L-5A S 35° 13′ 12″ E 31.52′ MH-3 L-5 N 40° 33′ 37″ W 60.23′ MH-4 L-5 N 01° 26′ 32″ E 351.27′ MH-10 L-6 S 43° 13′ 46″ E 88.06′ MH-11 L-6 S 05° 52′ 43″ W 85.25′ HW-1 L-5A N 09° 45′ 47″ W 87.14′ I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	I-21	L-5	N 09° 35' 13" E	102.93'
I-24 L-5A S 07° 51′ 37″ E 78.97′ I-25 L-5A S 35° 13′ 12″ E 31.52′ MH-3 L-5 N 40° 33′ 37″ W 60.23′ MH-4 L-5 N 01° 26′ 32″ E 351.27′ MH-10 L-6 S 43° 13′ 46″ E 88.06′ MH-11 L-6 S 05° 52′ 43″ W 85.25′ HW-1 L-5A N 09° 45′ 47″ W 87.14′ I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	1-22	L-5	N 08° 46′ 42″ E	348.28'
I-25 L-5A S 35' 13' 12" E 31.52' MH-3 L-5 N 40' 33' 37" W 60.23' MH-4 L-5 N 01' 26' 32" E 351.27' MH-10 L-6 S 43' 13' 46" E 88.06' MH-11 L-6 S 05' 52' 43" W 85.25' HW-1 L-5A N 09' 45' 47" W 87.14' I-36 L-2 N 27' 48' 16" W 76.46' EW-5 L-4 N 87' 40' 36" E 12.44' MH-20 L-3 N 83' 04' 28" E 35.36' FC-1 L-5 S 11' 31' 04" W 69.76' FC-2 L-5 S 82' 06' 34" W 33.07' FC-3 L-5 N 16' 06' 12" W 114.13'	1-23	L-5A	S 05° 28′ 53″ W	280.83'
MH-3 L-5 N 40° 33′ 37″ W 60.23′ MH-4 L-5 N 01° 26′ 32″ E 351.27′ MH-10 L-6 S 43° 13′ 46″ E 88.06′ MH-11 L-6 S 05° 52′ 43″ W 85.25′ HW-1 L-5A N 09° 45′ 47″ W 87.14′ I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	1-24	L-5A	S 07° 51′ 37″ E	78.97'
MH-4 L-5 N 01° 26′ 32″ E 351.27′ MH-10 L-6 S 43° 13′ 46″ E 88.06′ MH-11 L-6 S 05° 52′ 43″ W 85.25′ HW-1 L-5A N 09° 45′ 47″ W 87.14′ I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	1-25	L-5A	S 35° 13′ 12″ E	31.52'
MH-10 L-6 S 43° 13′ 46″ E 88.06′ MH-11 L-6 S 05° 52′ 43″ W 85.25′ HW-1 L-5A N 09° 45′ 47″ W 87.14′ I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	MH-3	L-5	N 40° 33′ 37″ W	60.23
MH-11 L-6 S 05' 52' 43" W 85.25' HW-1 L-5A N 09' 45' 47" W 87.14' I-36 L-2 N 27' 48' 16" W 76.46' EW-5 L-4 N 87' 40' 36" E 12.44' MH-20 L-3 N 83' 04' 28" E 35.36' FC-1 L-5 S 11' 31' 04" W 69.76' FC-2 L-5 S 82' 06' 34" W 33.07' FC-3 L-5 N 16' 06' 12" W 114.13'	MH-4	L-5	N 01° 26' 32" E	351.27
MH-11 L-6 S 05' 52' 43" W 85.25' HW-1 L-5A N 09' 45' 47" W 87.14' I-36 L-2 N 27' 48' 16" W 76.46' EW-5 L-4 N 87' 40' 36" E 12.44' MH-20 L-3 N 83' 04' 28" E 35.36' FC-1 L-5 S 11' 31' 04" W 69.76' FC-2 L-5 S 82' 06' 34" W 33.07' FC-3 L-5 N 16' 06' 12" W 114.13'	MH-10	L-6	S 43° 13' 46" E	
HW-1 L-5A N 09° 45′ 47″ W 87.14′ I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	MH-11	L-6		
I-36 L-2 N 27° 48′ 16″ W 76.46′ EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	HW-1	L-5A		
EW-5 L-4 N 87° 40′ 36″ E 12.44′ MH-20 L-3 N 83° 04′ 28″ E 35.36′ FC-1 L-5 S 11° 31′ 04″ W 69.76′ FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′	1-36			
MH-20 L-3 N 83° 04' 28" E 35.36' FC-1 L-5 S 11° 31' 04" W 69.76' FC-2 L-5 S 82° 06' 34" W 33.07' FC-3 L-5 N 16° 06' 12" W 114.13'				
FC-1 L-5 S 11' 31' 04" W 69.76' FC-2 L-5 S 82' 06' 34" W 33.07' FC-3 L-5 N 16' 06' 12" W 114.13'				
FC-2 L-5 S 82° 06′ 34″ W 33.07′ FC-3 L-5 N 16° 06′ 12″ W 114.13′				
FC-3 L-5 N 16° 06′ 12″ W 114.13′				
		L-5A	S 27° 54' 55" W	88.15
FC-5 L-5A S 35° 58' 25" W 55.65'	FC-5			

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TEST PIT INFORMATION				
NO.	UTILITY	NORTH	EAST	TOP ELEVATION
1	6" SHC	466,463.0	845,627.3	176.33
2	6" SHC	466,409.7	845,614.3	175.73
	3/4" WHC	466,416.3	845,613.3	177.49
3	4" SHC	466,377.0	845,615.0	171.83
4	8" WATER	466,394.3	845,636.2	175.99
5	6" SHC	466,256.3	845,595.8	169.75
6	6" SHC	466,129.9	845,579.1	170.75
7	6" SHC	466,093.1	845,568.0	165.97
8	8" WATER	466,084.4	845,590.3	166.80
9	6" WATER	466,095.7	845,619.4	167.22
10	6" SHC	465,891.5	845,540.4	162.72
11	6" WATER	465,806.7	845,573.5	167.12
12	8" WATER	465,801.5	845,543.6	165.16
13	6" WATER	465,772.5	845,508.9	161.70
1.4	2" GAS	465,915.4	845,324.6	182.76
14	6" WATER	465,920.5	845,317.4	180.58
15	6" WATER	466,113.3	845,061.8	217.42
16	6" WATER	465,859.4	845,062.9	202.53
	2" GAS	465,856.6	845,056.3	205.37
17	6" SHC	465,595.9	845,553.3	159.70
18	6" WATER	465,502.9	845,581.5	158.96
20	2" GAS	464,828.7	845,423.6	147.37
21	6" WATER	464,823.2	845,426.5	144.82
22	6" SHC	464,958.2	845,306.3	158.70

21	6 WAILK	4	64,823.2	84
22	6"SHC	4	64,958.2	84.
	SURVEY	TRAV	/ERSE	
NO.	NORTH		EAST	
L-2	464,809.1	77	845,440.7	88
L-3	465,008.5	06	845,704.4	70
L-4	465,165.0	03	845,701.5	79
L-4A	465,110.8	861	845,848.1	37
L-5	465,723.8	28	845,550.4	36
L-5A	466,417.1		845,641.3	37
L-6	466,089.3	48	845,060.7	06
L-7	465,345.4	32	845,127.4	26
L-8	465,083.4	67	845,233.4	60
31	465,893.7		845,322.8	32
32	466,128.9	37	845,346.3	02

	SOIL BORING	TABLE
NO.	NORTH	EAST
1	466,361.9	845,601.7
2	465,923.7	845,543.1
3	465,976.2	845,321.4
4	466,044.8	845,141.1
5	465,081.4	845,683.0
6	464,940.0	845,317.0

•	BENCH MARK TABLE	
NO.	DESCRIPTION	ELEV.
1	RR SPIKE IN BG&E#521838	157.80
2	RD. SIDE BOLT OF FH '	172.21
3	RR SPIKE IN BG&E#490551	220.02
4A	SQ.(BOX)CUT ON S.S.MH RIN	AT INT.
	MADISON & SECOND ST.	188.89

PROFILE	INDEX
PROFILE	SHEET NO.
EW-1 TO HW-1	8
EW-2 TO 1-33	9
I-34 TO MH-31	9
EW-3 TO I-35	9
EW-1 TO EW-5	8
I-20 TO FC-1	9
I-21 TO FC-3	10
1-23 TO MH-4	9
I-24 TO FC-4	9
I-25 TO FC-5	9
I-36 TO MH-30	9
I-12 TO FC-2	10
I-11 TO I-15	10
I-13 TO MH-11	10
I-16 TO I-17	10
EW-5 TO EW-1	8

- 1 STRUCTURE TO BE MODIFIED TO PROVIDE INCOMING AND OUTGOING PIPES PLUS CHANNELING.
- ② SEE SHEET 10, 11 FOR DETAILS.
- 3 OPTION TO USE PRECAST MANHOLE G5.12
- 4 PROPOSED ENDWALL TO BE JOINED TO EXISTING CONCRETE HEADWALL BY DOWELS SPACED EVERY 12".
- (5) INLET FRAME AND COVER TO BE SECURED TO INLET WALL WITH FRAME SPANNING OUTER DIMENSIONS AND A CONCRETE CURB IS NOT USED. ATTACHED WITH 4-5/8" Ø ANCHORS PER MARYLAND STANDARD # 374.02.
- 6 SEE SPECIFICATION FOR DETAILS.
- ① USE RECT. LINEAR GRATE SD 4.94
- (8) STORM TIE-IN MANHOLE, CUT 18" CMP 24 INCHES FROM DOWNSTREAM SIDE OF MANHOLE AND CAP.

 CUT 18"CMP AT UPSTREAM SIDE OF TIE-IN MANHOLE TO PROVIDE FLUSH CONNECTION AT INSIDE EDGE OF MANHOLE

NOTES:
EXISTING GUARDRAIL AT FIRST, SECOND, THIRD
AND FOURTH STREETS AND DECATUR ROAD TO BE
REMOVED. NEW GUARDRAIL TO BE INSTALLED AT
FIRST AVENUE AND DECATUR ROAD.

FOR GRADING PLAN SEE SHEET 13, 14 & 15.

INVERT INVERT REMARKS ELEVATION SD 4.21 2 SD 4.21 2 MOD. TYPE " MOD. TYPE ' SD 4.21@ MOD. TYPE " (7) SD 4.21 (1) 159.60 152.00 151.80 SD 4.21 ① MOD. TYPE 167.70 162.33 162.13 MOD. TYPE SD 4.21 ① 177.68 181.50 177.48 1-33 SD 4.21 190.50 186.78 I-34 EXISTING INLET 182.05 SD 4.37 (5) 1-35 148.50 146.08 TYPE "E" 154.00 1-36 149.19 SD 4.31 SPECIAL 2 149.40 149.20 LIN-PLACE MH DESIGN SPEC. CAST-SPECIAL O 157.98 164.70 158.18 IN-PLACE MH DESIGN G 5.01 ③ 155.50 MH-30 4' BRICK MH 148.67 G 5.01 ③ 181.08 MH-31 4' BRICK MH 181.19 SPECIAL 26
DESIGN EW-1146.70 ENDWALL SD 5.21 4 EW-2145.80 ENDWALL TYPE "C" SD 5.21 (4) EW-3145.00 **ENDWALL** 151.21 SD 5.21 **ENDWALL** G5.01 ③ 149.77 MH-20 4" BRICK MH 153.50 149.97 SD 5.61 ES-1 | END SECTION | 175.03 CONN. TO EX. 15" CMP MOD. TYPE "E" SD 4.212 167.60 INLET MOD. TYPE "E" SD 4.212 173.15 MOD. TYPE SD 4.21(2) 1-6 MOD. TYPE "E" SD 4.21(1) 164.18 INLET MOD. TYPE SD 4.21(1) 183.10 177.77 177.97 INLET TYPE "E" 1-12 206.30 202.70 SD 4.21 INLET 1-13 218.10 212.50 SD 4.21 INLET SD 4.21(1 179.66 INLET MOD. TYPE SD 4.37 (5) 1-15 207.53 210.00 "WR" INLET 1-16 208.47 204.95 SD 4.01 A-5MOD. TYPE SD 4.37 (1) (5) 199.74 1-17 206.50 200.19 "WR" INLET SD 4.37 (5) 1-20 166.70 164.22 "WR" INLET MOD. TYPE "WR" INLET 169.35 166.91 1 - 21SD 4.37 SD 4.211 168.17 1-22 172.80 INLET MOD. TYPE 1-23 169.25 SD 4.37 5 "WR" INLET SD 4.37 (5) 178.40 175.69 1 - 24"WR" INLET MOD. TYPE SD 4.37 (5) 1 - 25179.70 176.20 "WR" INLET SPECIAL ②
DESIGN ② SPEC. CAST-161.66 169.20 161.86 SPECIAL @ SPEC. CAST-167.27 174.30 167.47 IN-PLACE MH 198.02 197.82 MH-10 4' BRICK MH 207.10 G 5.01 ③ 206.50 200.15 199.55 G 5.01 ③ MH-11 4' BRICK MH SPECIAL 6 2 HW-1HEADWALL 162.00 15"RCCP TO 7'x4'RPBC 161.64 24" RCCP TO 7'x4'RPBC 164.00 15"RCCP TO 5'x4'RPBC 174.20 18" RCCP TO 5'x4'RPBC 175.08 15" RCCP TO 5'x4'RPBC CMP INVERT EX 18"CMP 168.80 174.80 TO 24"RCCP

DRAINAGE STRUCTURE SCHEDULE

	그리고 있다. 그러를 다른 하네는 상을 갖춰 가는 것이 얼마 하고 말했다.	4
NORTH LAURFL	DRAINAGE IMPROVEMENT	
TOWN LAUNCE		
STORM DRAIN PLANS	 CAPITAL PROJECT D-108 	ζ.

CHARTS AND TABLES

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HO	WARD COUNT	Y MARYLAND		

HOWARD COUNTY, MARYLAND

CONTROL OF PUBLIC WORKS

DATE

CHIEF, BUREAU OF ENGINEERING

LEGISLAND

CHIEF TRANSPORTATION AND

DATE

CHIEF TRANSPORTATION AND

DATE

P&J W.O.# 3420-00

PURDUM JESCI-KE

CONSULTING ENGINEERS AND LAND SURVEYORS

1029 NORTH CALVERT STREET
BALTIMORE, MARYLAND 21202

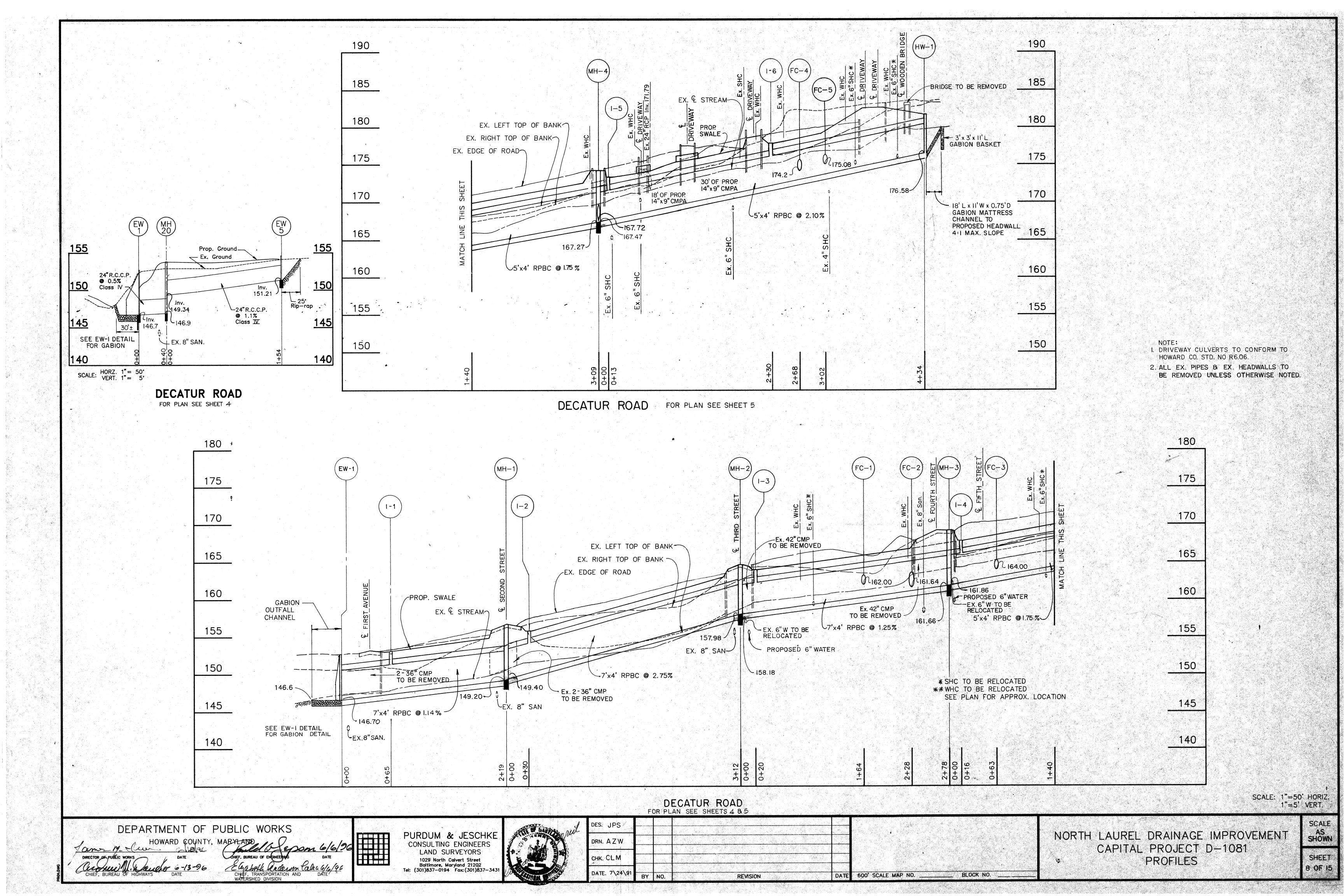
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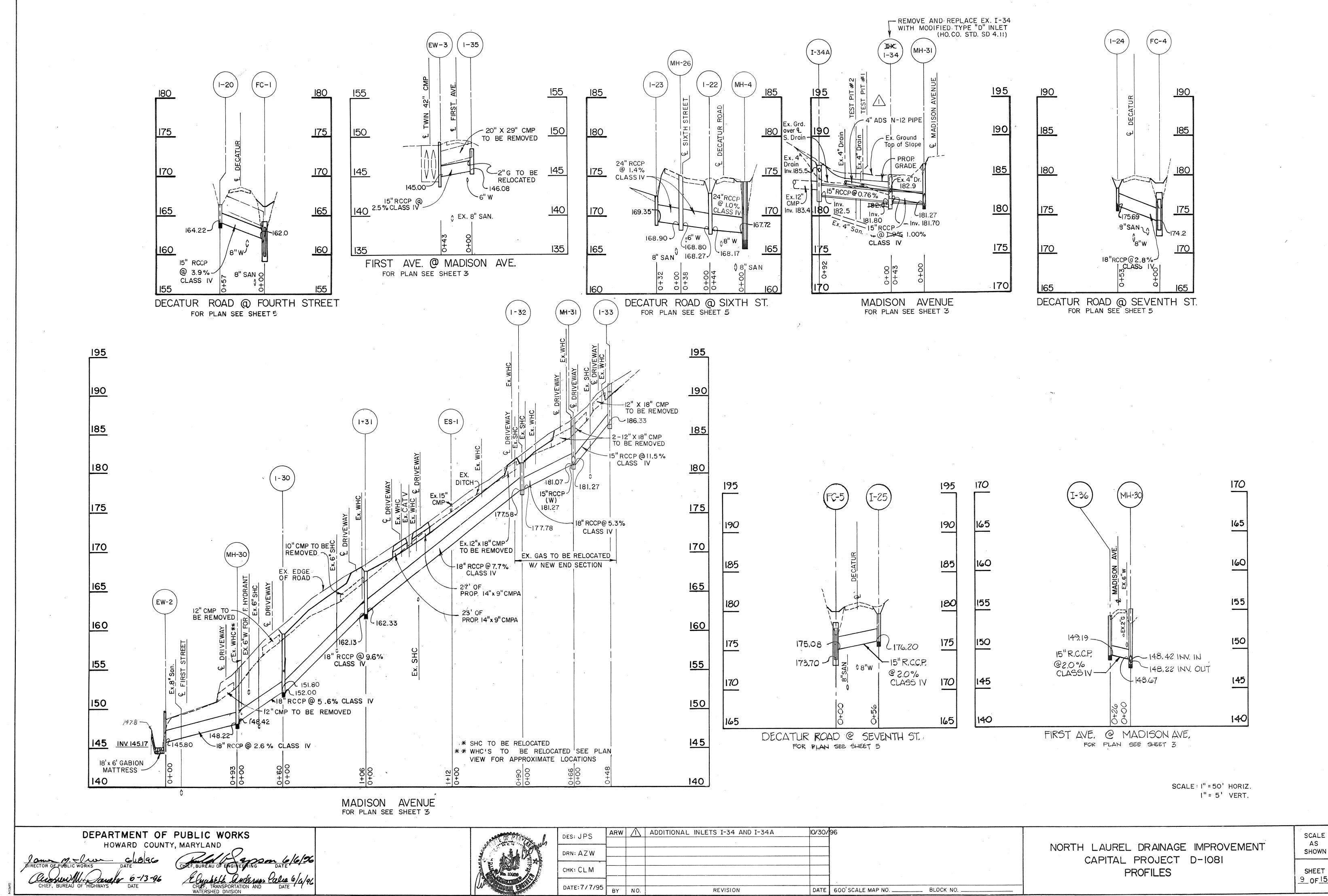


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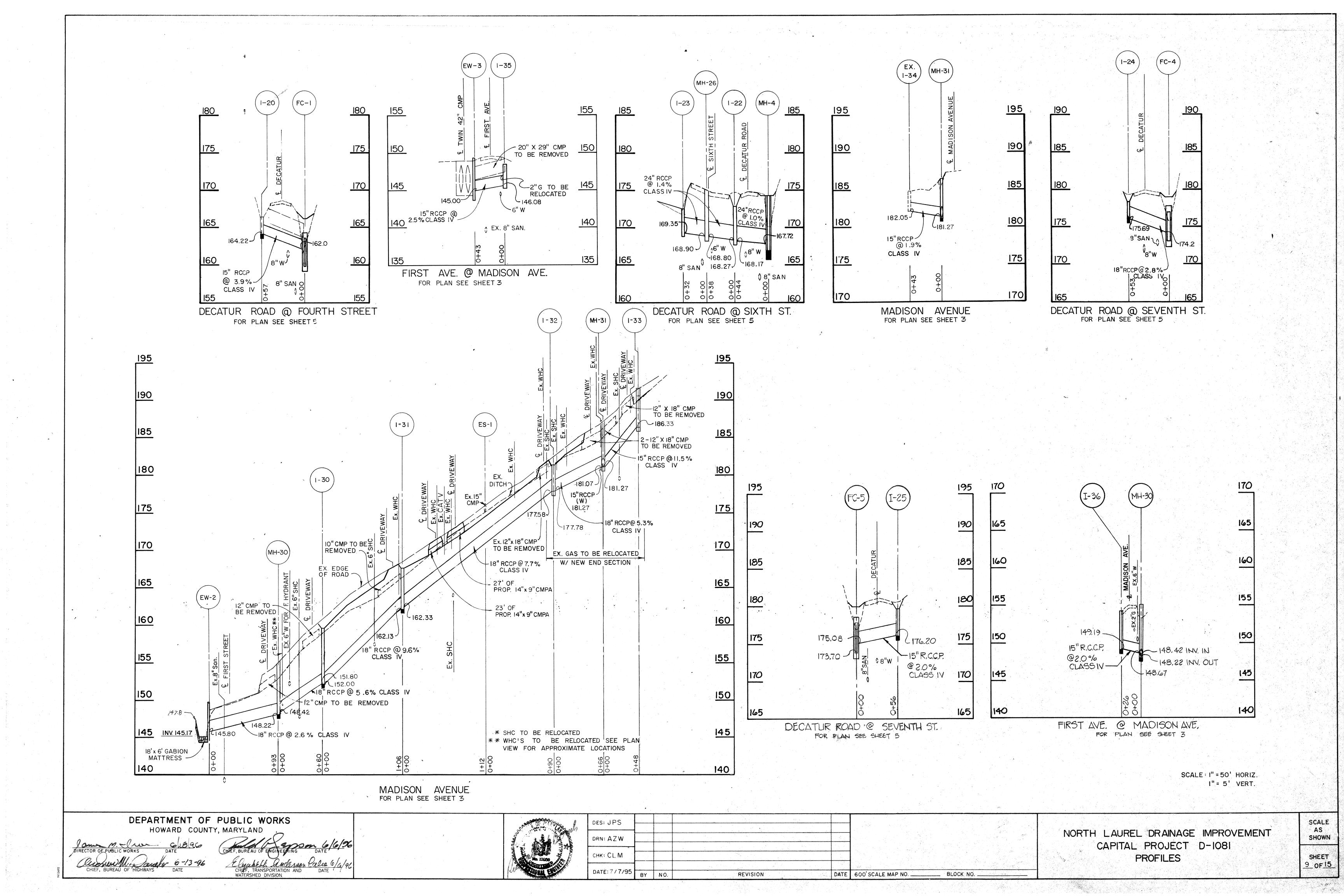
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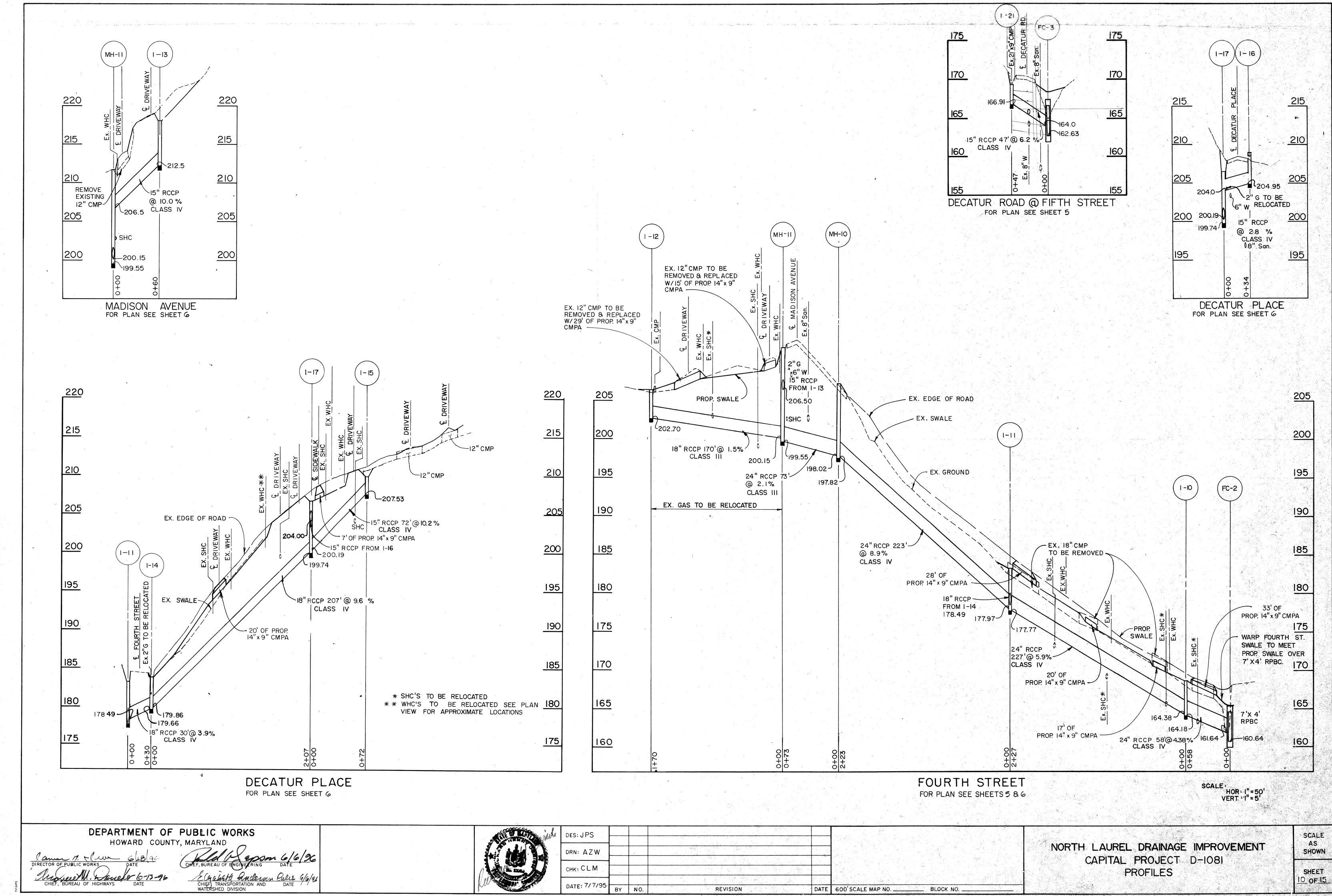
SHEET

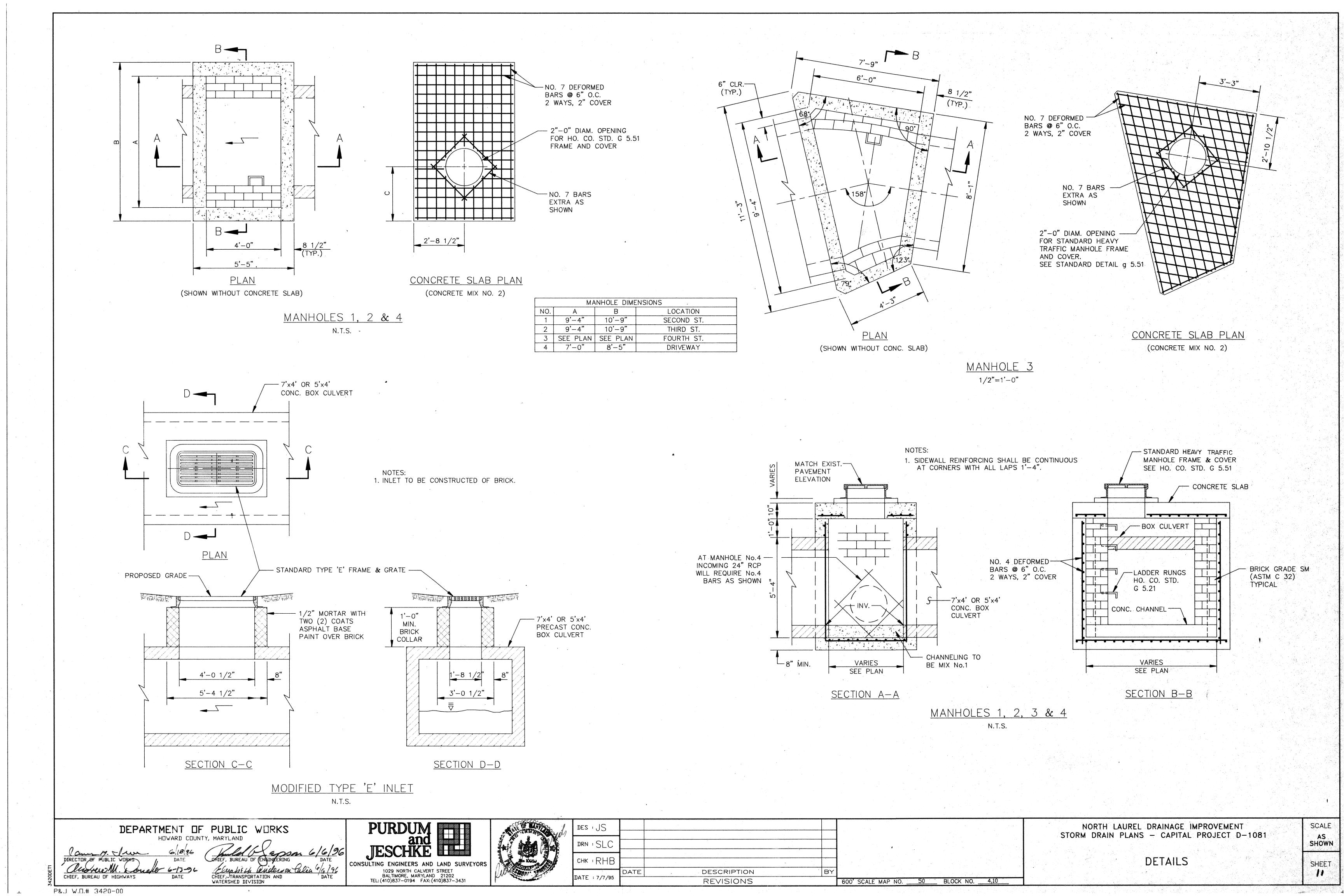


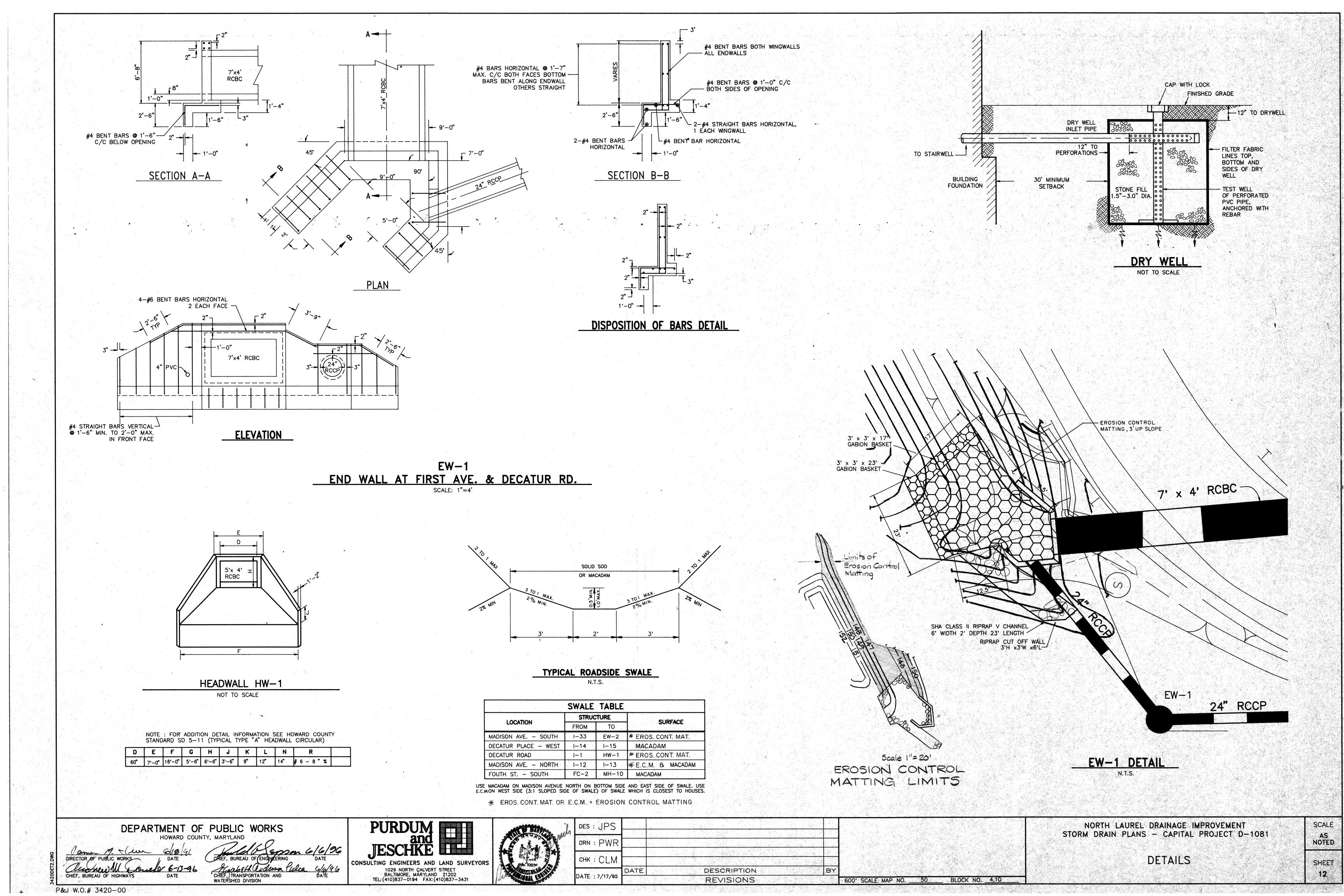


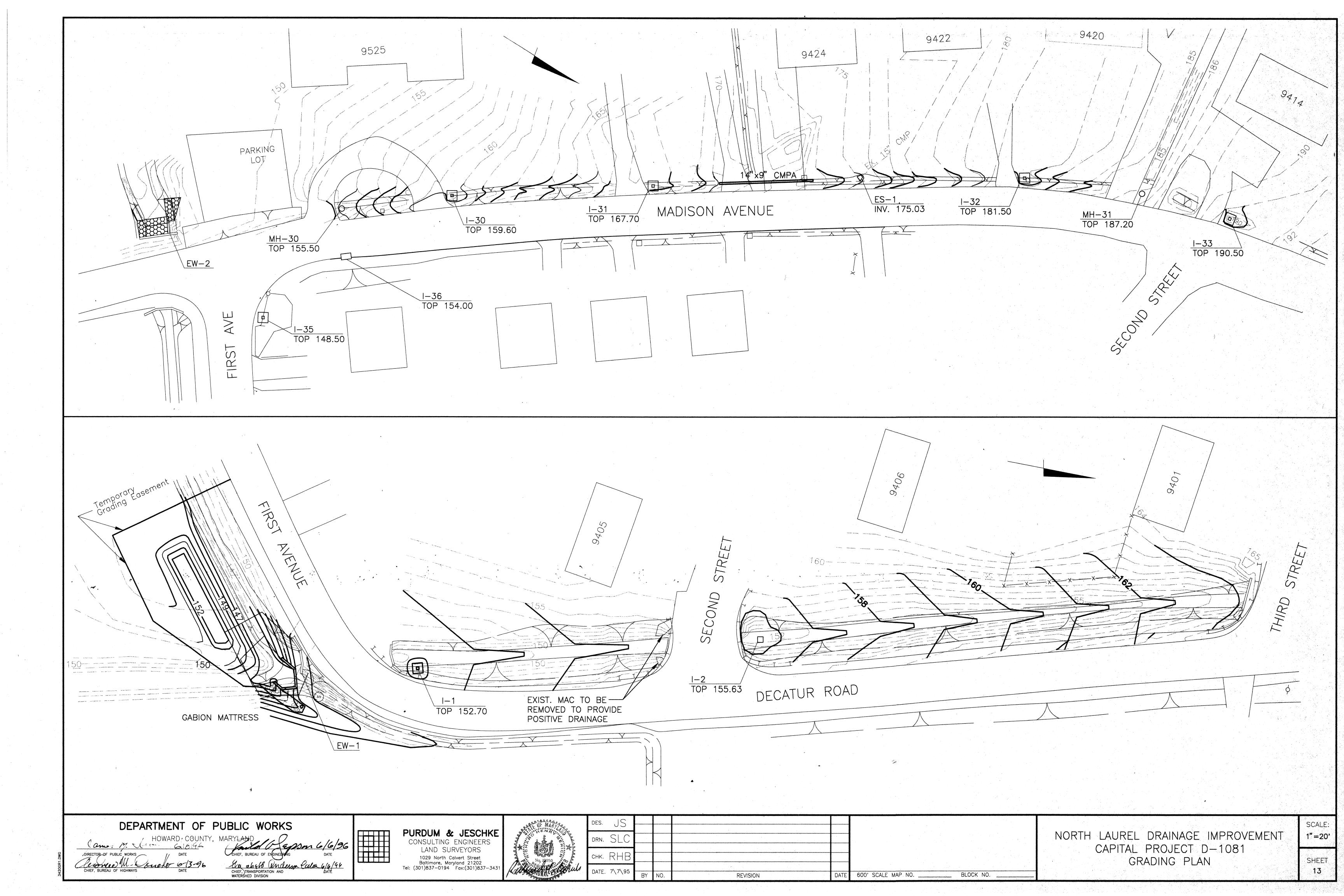
SHOWN SHEET 9 OF 15

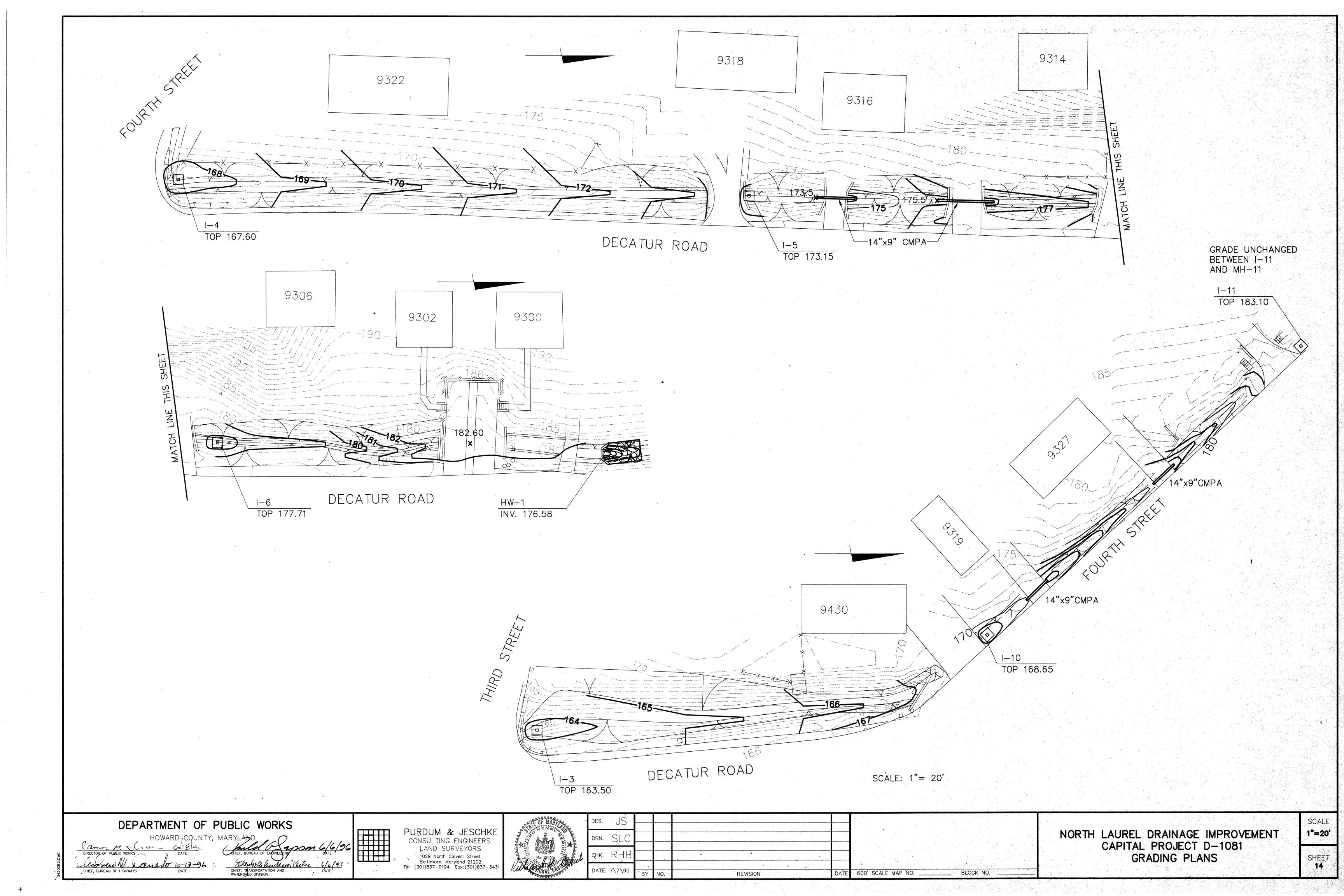


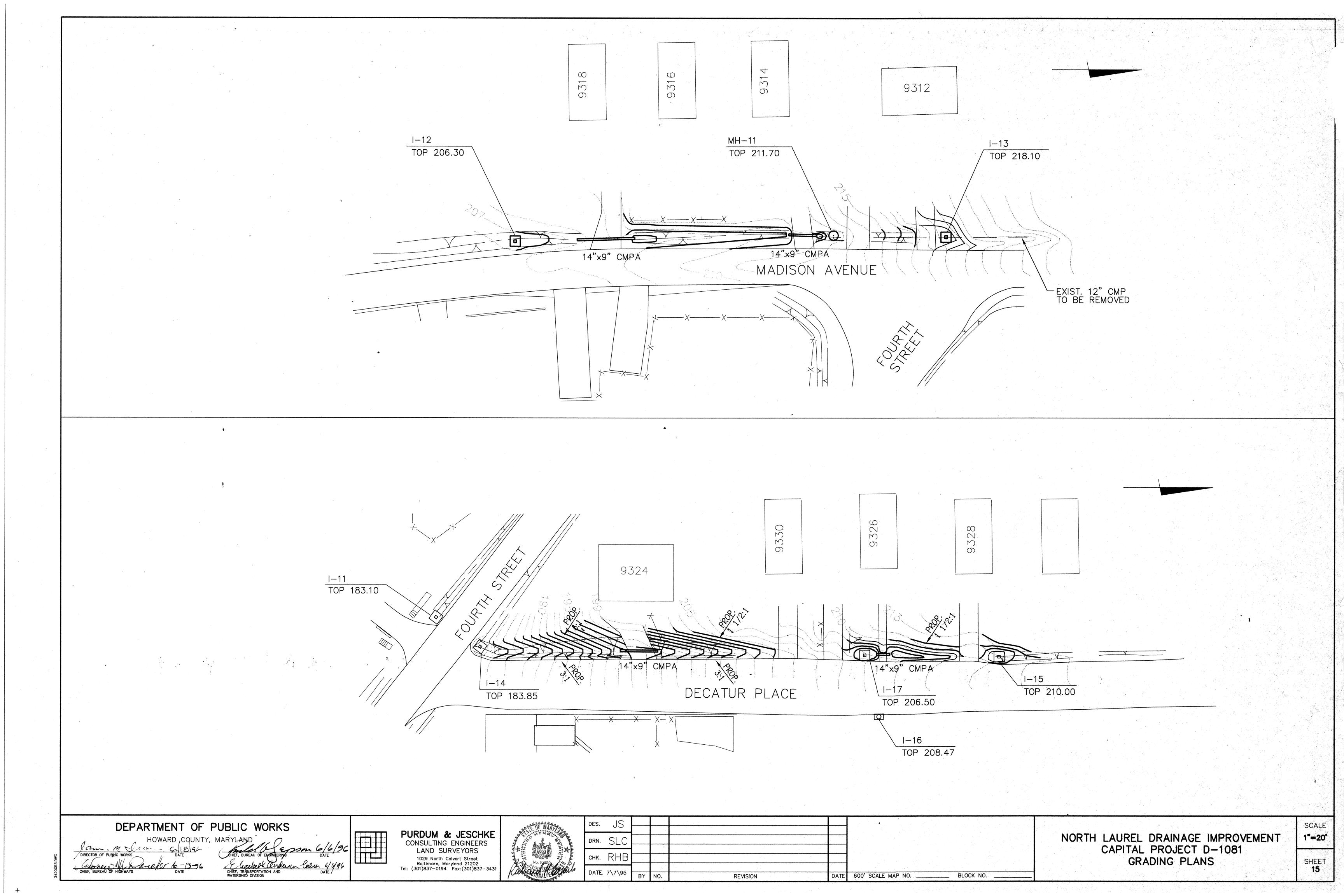












SEQUENCE OF CONSTRUCTION

1 day.

- 1. Obtain proper permits.
- 2. Notify the Howard County Bureau of Licenses, Inspections and Permits and Sediment Control Inspector at least 48 hours before any work begins.

DECATUR ROAD (WEST SIDE)

- Remove existing twin 36" CMP at First Avenue.
 Install box culvert from EW-1 to upstream of I-1, install I-1.
 Install structure EW-1.
 Install berm and gabion mattress outfall protection at EW1.
 days.
 Install berm and gabion mattress outfall protection at EW1.
- 7. Install perimeter dike between First and Second Streets directing runoff into opening at l−1 and place silt fence around l−1. Install work area and base flow area sump pits per sheet #20. 2 days

6. Provide temporary repair/stabilization of roadway at First Street.

- 8. Between First Avenue and Second Street, excavate only the maximum length of trench in which the culvert and trench backfill can be completed in the same day. Install the box culvert sections and impervious sand bag dike approximately 5 feet downstream of Second Avenue as shown on sheet #20, typical base flow diversions. Pump the base flow from the impervious sand bag dike area to the next downstream inlet during working hours. During non-working hours place twin 15" pipes from impervious sandbag dike to last section of box culvert installed for service.
- 9. At the beginning of each work day, the pump with temporary 8" discharge pipe from the base flow area shall be placed from the impervious sand bag dike to the previously installed inlet. If directed, place a second pump with temporary 4" discharge pipe from the work area to a previously installed inlet.
- 10. Continue with excavation and box culvert installation until removing twin 36" CMP's in Second Street.
- 11. Install impervious sandbag dike approximately 5 ft. downstream of Third Street. During working hours, pump base flow to I-1 until I-2 structure is installed. During non-working hours place twin 15" pipes at a minimum 2% slope from the impervious sandbag dike to last section of box culvert installed.
- 12. Rough grade, provide temporary stabilization for the area between First and Second Streets and provide temporary repair/stabilization of roadway at Second
- *13. Remove the earth dikes between First Avenue and Second Street following sod growth and approval from Sediment Control.
- 14. Install earth dikes between Second and Third Streets directing runoff into I-2 and place silt fence around I-2. Install work area and base flow area sump pits per detail on sheet #202 days.
- 15. Between Second and Third Street, excavate only the maximum length of trench in which the culvert and trench backfill can be completed in the same day. Install box culvert sections and maintain impervious sandbag dike upstream of excavation as shown on sheet #20 for typical base flow diversion. During working hours, pump the base flow from the impervious sandbag dike to the next downstream inlet. During non-working hours place twin 15" pipes at a minimum 2% slope from sandbag dike to last section of box culvert installed. 15 days.
- 16. At the beginning of each work day, the pump with temporary 8" discharge pipe from the base flow area shall be placed from the impervious sand bag dike to the previously installed inlet. If directed, place a second pump with temporary 4" discharge pipe from the work area to a previously installed inlet.
- CMP's in Third Street. Install impervious sandbag dike 5 ft. downstream of Fourth Street. During working hours, pump base flow to I-2 until I-3 structure is installed. During non-working hours place twin 15" pipes at a minimum 2% slope from sandbag dike to last section of box culvert installed.

Continue with excavation and box culvert installation until reaching twin 42*

- 18. Rough grade, provide temporary stabilization for the area between Second and Third Street and provide temporary repair/stabilization of the road at Third Street.
- 19. Remove the earth dikes between Second Avenue and Third Street following sod growth and approval from Sediment Control.
- 20. Install perimeter earth dikes between Third and Fourth Street directing runoff into I-3 and place silt fence around I-3. Install work area and base flow area sump pits per detail on sheet #20.

 2 days.
- 21. Between Third and Fourth Street, excavate only the maximum length of trench in which the culvert and trench backfill can be completed in the same day.Install box culvert sections and maintain impervious sandbag dike upstream of excavation as shown on sheet #20 for typical base flow diversions. During non-working hours place twin 15" pipes at a minimum 2% slope from impervious sandbag dike to last section of box culvert installed. Pump the base flow from the impervious sandbag dike to the next downstream inlet during working hours. 15 days.
- 22. At the beginning of each work day, the pump with temporary 8" discharge pipe from the base flow area shall be placed from the impervious sand bag dike to the previously installed inlet. If directed, place a second pump with temporary 4" discharge pipe from the work area to a previously installed inlet.
- 23. Continue with excavation and box culvert installation until reaching twin 42"

 CMP's in Fourth Street. Install impervious sand bag dike 5 ft. upstream of proposed Inlet 5. Pump base flow to I-3 until I-4 structure is installed. During non-working hours place twin 15" pipes at minimum 2% slope from the impervious sandbag dike to last section of box culvert installed.
- 24. Rough grade, provide temporary stabilization for the area between Third and Fourth Streets and provide temporary repair/stabilization of roadway at Fourth Street.
- 25. Remove the earth dikes between Third and Fourth Street following sod growth and approval from Sediment Control.

27. Between Fourth Street and I-5, excavate only the maximum length in which

- 26. Install perimeter dike between I-4 and proposed Inlet 5 directing the runoff into opening at I-4 and place silt fence around I-4. Install work area and base flow area sump pits per detail on Sheet #20 2 days.
- the culvert and trench backfill can be completed in the same day.Install box culvert sectionsand maintain impervious sand bag dike 5ft. Upstream of proposed Inlet 5. During working hours, pump the base flow from the impervious sandbag dike to the next downstream inlet. During non—working hours place twin 15" pipes from the impervious sandbag dike to last section of box culvert installed. 15 days

- 28. At the beginning of each work day, the pump with temporary 8" discharge pipe from the base flow area shall be placed from the impervious sand bag dike to the previously installed inlet. If directed, place a second pump with temporary 4" discharge pipe from the work area to a previously installed inlet.
- 29. Continue with excavation and box culvert installation until Structure I-5 is reached. Install impervious sandbag dike 5 ft. upstream of HW-1. Pump base flow to the next downstream inlet during working hours. During non-working hours place twin 15" pipes from the impervious sandbag dike to last section of box culvert installed.
- 30. Rough grade and provide stabilization for the area between Fourth Street and
- 31. Remove the earth dikes between Fourth Street and I-5 following sod growth and approval from S.C. Inspector,
- 32. Install perimeter earth dike between I—5 and HW—1 directing runoff into I—5 and place silt fence around I—5 and include pipe below driveways. Install work area and base flow area sump pits per detail on sheet #20.
- 33. Between I-5 and HW-1, excavate only the maximum length of trench in which the culvert and trench backfill can be completed in the same day and install box culvert sections. Maintain the impervious sand bag dike upstream of excavation. Pump the base flow from the impervious sandbag dike to inlet I-5 during working hours. Install twin 15" pipes from the impervious sandbag dike to last section of box culvert installed for sevice during non-working hours.
- 34. At the beginning of each work day, the pump with temporary 8" discharge pipe from the base flow area shall be placed from the impervious sand bag dike to the previously installed inlet. If directed, place a second pump with temporary 4" discharge pipe from the work area to a previously installed inlet.
- 35. Continue with excavation and box culvert installation to HW-1 and install, structure HW-1 and gabion mattress.
- 36. Rough grade and provide temporary stabilization for the area between I-5 and
- 37. Remove the earth dikes between I-5 and HW-1 following sod growth and approval from Sediment Control.

90 days total.

DECATUR ROAD (EAST SIDE)

- 38. Install an impervious stone dike approximately 75' upstream of EW-4. Pump the base flow from the impervious sandbag dike to a downstream of gabion
- 39. Install perimeter earth dike around the proposed storm drain alignment from the impervious dike, to the downstream gabion surface at EW-1.
- 40. Excavate only the maximum length of trench in which the culvert and trench backfill can be completed in the same day and install pipe system, starting culvert from downstream end.
- 41. At the completion of construction each day place a stone dike wrapped in filter cloth at the end of the storm drain pipe.
- 42. Upon completion of partial installation of pipe system and grading, provide temporary stabilization by seeding, and mulching plus provide straw bale check dams every 25'.
- 43. Continue construction sequence by repeating items # 38, 39, 40, 41 and 42 until all pipe MH#20 and EW#5 have been constructed.
- 44. Install rip—rap channel protection @ EW #5 and permanently stabilize all areas on east side of Decatur Road at 24" S.D. system.
- 45. Remove all sediment control devices such as the final section of perimeter dike and upstream impervious in-stream dike following sod growth and approval from Sediment Control 14 days

FOURTH STREET, DECATUR PLACE, AND NORTH MADISON AVENUE SYSTEM

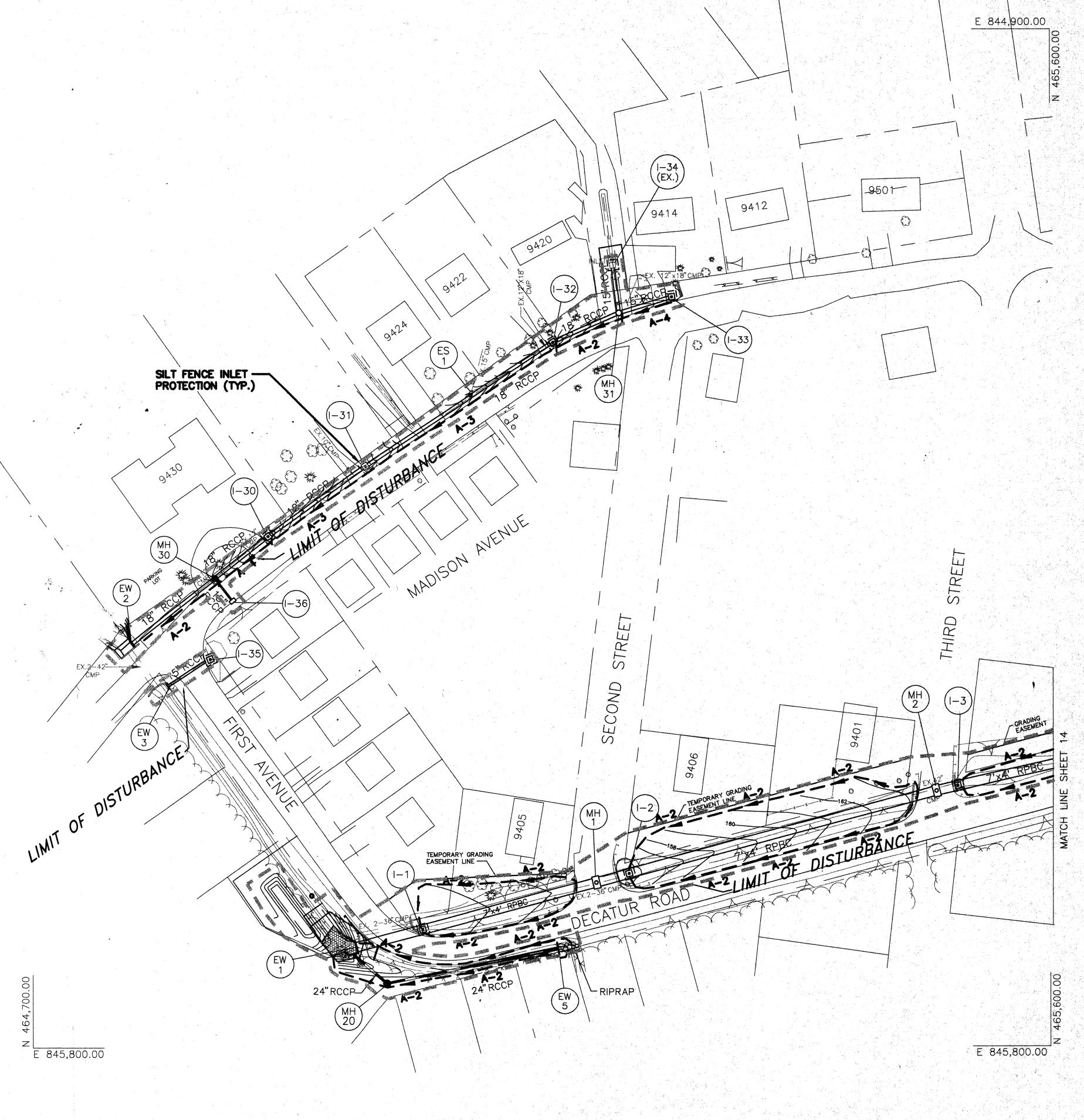
- 46. For the Fourth Street, Decatur Place and Madison Avenue north system begin pipe installation at the downstream location. (FC-2)
- 47. Excavate only the maximum length of trench in which the culvert and trench backfill can be completed in the same day and install pipe system, starting culvert from downstream end.2 days.
- 48. At the completion of construction each day place a stone dike wrapped in filter cloth at the end of the storm drain pipe.
- 49. Upon completion of partial installation of the pipe system and grading provide temporary stabilization by seeding and mulching plus providing straw bale check dams every 25'.
- 50. Place permanent stabilization on downstream areas including macadam at swale centerline per detail sheet #12.
- 51. Upon completion of S.D. systems of North Madison Avenue, solid sod and permanently stabilize (seed & mulch) disturbed areas per detail sheet #12 & #18.
- 52. Upon completion of S.D. system at Decatur Place, place solid sod per detail sheet #12 and #18 and permanently stabilized (seed & mulch) disturbed areas.
- 53. Remove sediment controls on Decatur Place following sod growth of seeded areas following sod growth and approval from S.C. Inspector.

 14 day

MADISON AVENUE / FIRST STREET

- 54. Install earth dike along edge of pavement.
- 55. Excavate only the maximum length of trench in which the culvert and trench backfill can be completed in the same day and install pipe system; starting culvert from downstream end.

 21 days.
- 56. At the completion of construction each day place a stone dike wrapped in filter cloth at the end of the storm drain pipe.
- 57. Stabilize the swale graded below the storm drain pipe by seeding and mulching and solid sodding at swale centerline per details on sheet #12 and sheet
- Upon approval of Sediment Control Inspection, remove all sediment control devices and stabilize per permanent seeding notes.
- (30 days not critical path) Fine grade areas where sediment controls are removed and stabilize as specified by the approved sediment control plan.



BLOCK NO.

600' SCALE MAP NO.

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DEPARTMENT OF PUBLIC WORKS

HOWARD COUNTY, MARYLAND LESSON 6

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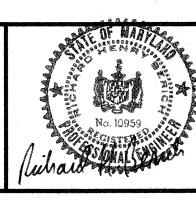
CHIEF, BUREAU OF ENGINEERING

CHIEF, TRANSPORTATION AND
WATERSHED DIVISION

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PURDUM & JESCHKE
CONSULTING ENGINEERS
LAND SURVEYORS

1029 North Calvert Street
Baltimore, Maryland 21202
Tel: (301)837-0194 Fax:(301)837-3431



DES. JS

DRN. SLC

CHK. RHB

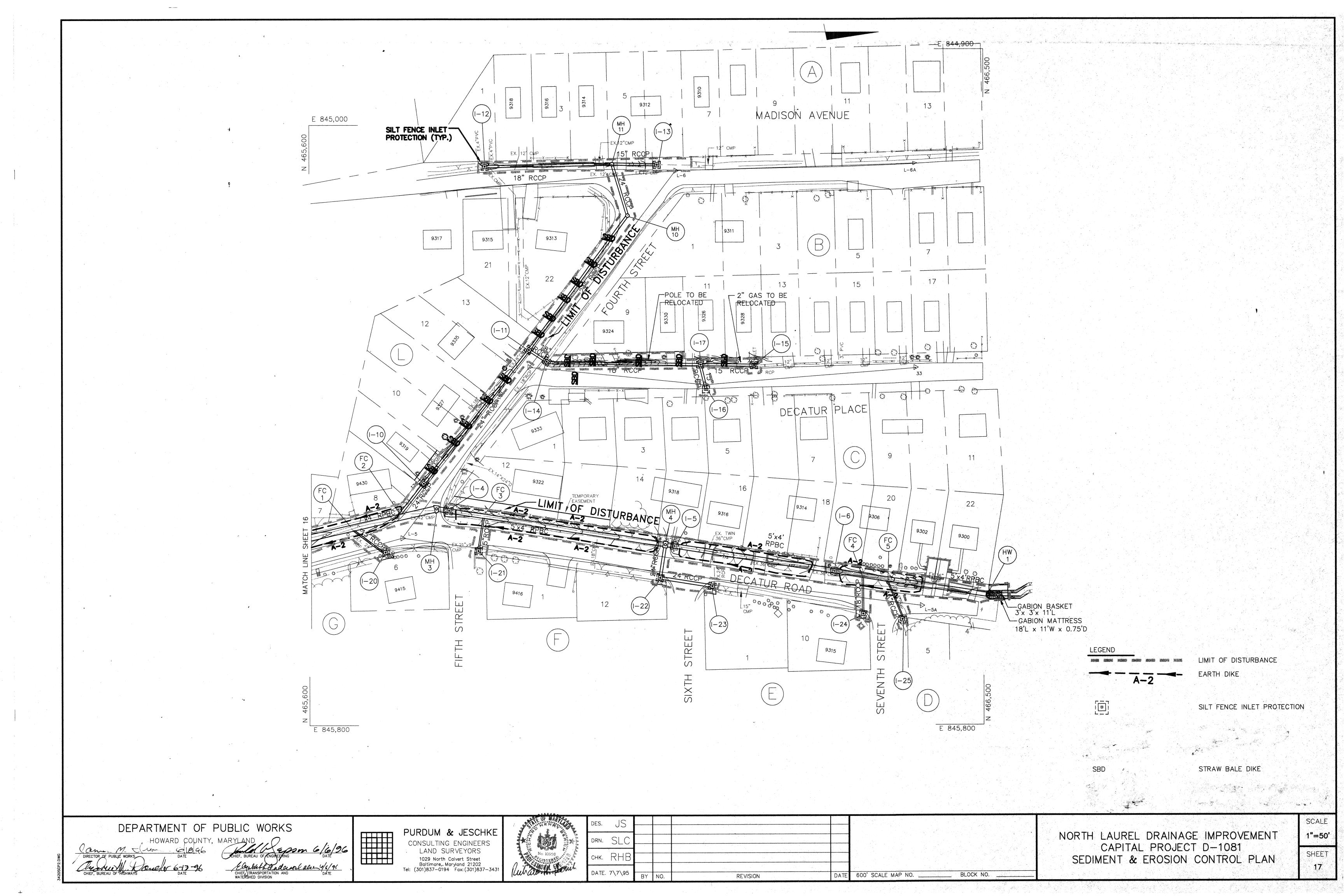
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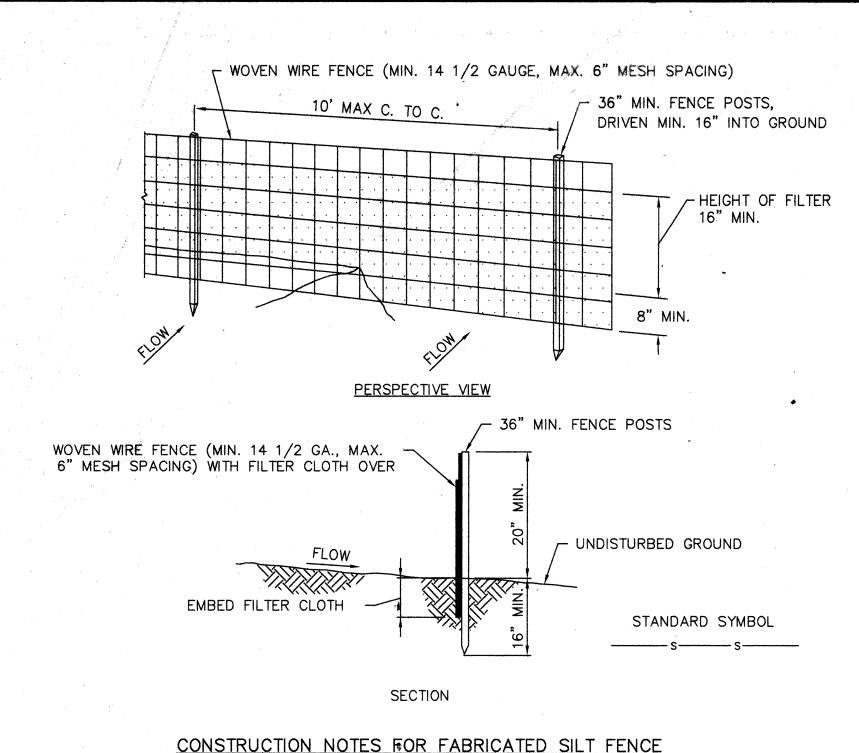
BY NO. REVISION

NORTH LAUREL DRAINAGE IMPROVEMENT CAPITAL PROJECT D-1081 SEDIMENT & EROSION CONTROL PLAN

SCALE 1"=50'

SHEET 1.6





CONSTRUCTION NOTES INON TABRICATED

- 1. WOVEN WIRE FENCE TO BE FASTENED SECURELY TO FENCE POSTS WITH WORE TIES OF STAPLES.
- 2. FILTER CLOTH TO BE FASTENED SECURELY TO WOVEN WIRE FABRIC WITH TIES SPACED EVERY 24" AT TOP AND MIDSECTION.
- 3. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER THEY SHALL BE OVER-LAPPED BY SIX INCHES AND FOLDED.
- 4. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN "BULGES" DEVELOP IN THE SILT FENCE.

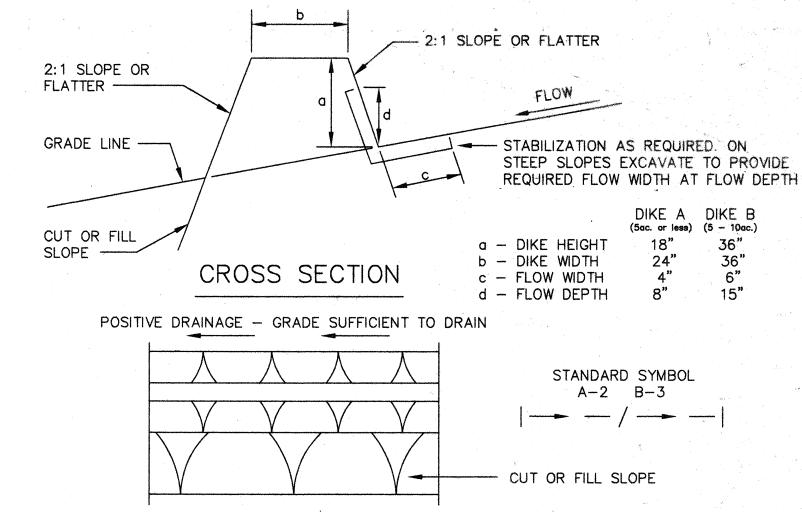
POSTS: STEEL EITHER T OR U TYPE OR 2" HARDWOOD

FENCE:
WOVEN WIRE, 14 GA. MIN.,
6" MAX. MESH OPENING
FILTER CLOTH:

MIRAFI 100X, STABI— LINKA T140N OR APPROVED EQUAL

PREFABRICATED UNIT: GEOFAB, ENVIROFENCE, OR APPROVED EQUAL

SILT FENCE



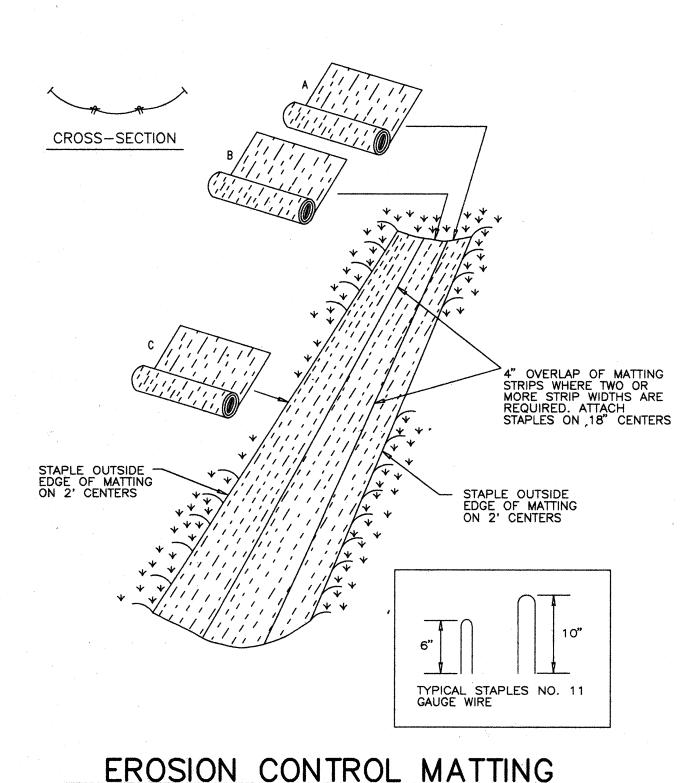
CONSTRUCTION SPECIFICATION

- 1. ALL DIKES SHALL BE COMPACTED BY EARTH-MOVING EQUIPMENT.
- 2. ALL DIKES SHALL HAVE POSITIVE DRAINAGE TO AN OUTLET.
- TOP WIDTH MAY BE WIDER AND SIDE SLOPES MAY BE FLATTER IF DESIRED TO FACILITATE CROSSING BY CONSTRUCTION TRAFFIC.
 FIELD LOCATION SHOULD BE ADJUSTED AS NEEDED TO UTILIZE A STABILIZED SAFE OUTLET.
- 5. EARTH DIKES SHALL HAVE AN OUTLET THAT FUNCTIONS WITH A MINIMUM OF EROSION. RUNOFF SHALL BE CONVEYED TO A SEDIMENT TRAPPING DEVICE SUCH AS A SEDIMENT TRAP OR SEDIMENT BASIN WHERE EITHER THE DIKE CHANNEL OR THE DRAINAGE AREA ABOVE THE DIKE ARE NOT ADEQUATELY STABILIZED.
- 6. STABILIZATION SHALL BE: (A) IN ACCORDANCE WITH STANDARD SPECIFICATION FOR SEED AND STRAW MULCH OR STRAW MULCH IF NOT IN SEEDING SEASON, (B) FLOW CHANNEL AS PER THE CHART BELOW.

	<u></u>	FLOW CHANNEL STABILIZATION	
TYPE OF TREATMENT	CHANNEL GRADE	DIKE A	DIKE B
1	.5 - 3.0%	SEED AND STRAW MULCH	SEED AND STRAW MULCH
2	3.1 - 5.0%	EROSION CONTROL MATTING W/SEED, OR SOD	SEED USING JUTE, OR EXCELSIOR; SOD; 2" STONE
3	5.1 - 8.0%	EROSION CONTROL MATTING WITH SEED, OR SOD	LINED RIPRAP 4 - 8"
4	8.1 - 20%	LINED RIPRAP 4 - 8"	ENGINEERING DESIGN

- A. STONE TO BE 2 INCH STONE, OR RECYCLE CONCRETE EQUIVALENT, IN A LAYER AT LEAST 3 INCHES IN THICKNESS AND BE PRESSED INTO THE SOIL WITH CONSTRUCTION EQUIPMENT.
- B. RIPRAP TO BE 4 8 INCHES IN A LAYER AT LEAST 8 INCHES THICKNESS AND PRESSED INTO THE SOIL.
- C. APPROVED EQUIVALENT CAN BE SUBSTITUTED FOR ANY OF THE MATERIALS.
- 7. PERIODIC INSPECTION AND REQUIRED MAINTENANCE MUST BE PROVIDED AFTER EACH RAIN EVENT.

EARTH DIKE



Construction Specifications

- 1. Key—in the matting by placing the top ends of the matting in a narrow trench, 6" in depth. Backfill the trench and tamp firmly to conform to the channel cross—section. Secure with a row of staples about 4" down slope from the trench. Spacing between staples is 6".
- 2. Staple the 4" overlap in the channel center using an 18" spacing between staples.
- 3. Before stapling the outer edges of the matting, make sure the matting is smooth and in firm contact with the soil.
- 4. Staples shall be placed 2' apart with 4 rows for each strip, 2 outer rows, and 2 alternating rows down the center.
 5. Where one roll of matting ends and another begins, the end of the top strip shall overlap the upper end of the lower strip by 4",

shiplap fashion. Reinforce the overlap with a double row of staples

- spaced 6" apart in a staggered pattern on either side.

 6. The discharge end of the matting liner should be similarly secured with 2 double rows of staples.
- Note: If flow will enter from the edge of the matting then the area effected by the flow must be keyed—in.

EROSION AND SEDIMENT CONTROL SECTION

The Contractor will comply with all requirements of Sediment and Erosion Control as set forth in the Howard County Sediment Control

All sediment controls and critical slopes must be stabilized within seven calendar days. All other disturbed areas on the project site must be stabilized within 14 calendar days.

All utilities to be constructed first, prior to any construction on

No pumping from foundation excavations will be allowed into County system unless it is filtered by way of sediment traps or filter.

All excavated material shall be placed on the high side whenever possible and confined to an area where it will not obstruct the normal flow of drainage courses.

Continuous inspection and maintenance of all sediment control devices will be required.

The Contractor shall notify in writing the Howard County Sediment Control Representative at least three working days prior to starting any work.

On all sites with disturbed areas in excess of two acres, the permittee shall request that a Howard County Erosion and Sediment Control Inspector inspect and approve the work, completed at the stages of construction specified below to ensure accordance with the approved erosion and sediment control plan, the grading or building permit, and this Manual:

- a. Upon completion of installation of perimeter erosion and sediment controls, prior to proceeding with any other earth disturbance or grading. Other building or grading inspection approvals may not be authorized until initial approval by the inspection agency is made, and
- b. Upon final stabilization be fore removal of sediment controls.

Howard County Sediment Control Section must be notified in writing of where any excess material will be disposed of or where any borrowed material will come from.

The Owner/Contractor shall not deviate from the approved sediment and erosion control plans without prior approval of the Howard County Sediment Control Representative. Variations to the plan must be submitted in writing, accompanied by a copy of the originally approved plan modified to show the requested changes, for his approval. Substantial changes will necessitate amending the building and/or grading permit if applicable.

STANDARD SEDIMENT CONTROL NOTES

- 1. A minimum of 48 hours notice must be given to the Howard County Department of Inspections, Licenses and Permits, Sediment Control Division prior to the start of any construction (313—1855).
- 2. All vegetative and structural practices are to be installed according to the provisions of this plan and are to be in conformance with the 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL and revisions thereto.
- 3. Following initial soil disturbance or re—disturbance, permanent or temporary stabilization shall be completed within: a) 7 calendar days for all perimeter sediment control structures, dikes, perimeter slopes and all slopes steeper than 3:1, b) 14 days as to all other disturbed or graded areas on the project site.
- 4. All sediment traps/basins shown must be fenced and warning signs posted around their perimeter in accordance with Vol. 1 Chapter 7, of the HOWARD COUNTY DESIGN MANUAL, Storm Drainage.
- 5. All disturbed areas must be stabilized within the time period specified above in accordance with the 1994 MARYLAND STANDARDS AND SPECIFICATIONS FOR SOIL EROSION AND SEDIMENT CONTROL for permanent seeding, sod, temporary seeding and mulching (Sec. G). Temporary stabilization with mulch alone shall only be done when recommended seeding dates do not allow for proper germination and establishment of grasses.
- 6. All sediment control structures are to remain in place and are to be maintained in operative condition until permission for their removal has been obtained from the Howard County Sediment Control Inspector.

7. Site Analysis:

Total Área of Site
Area Disturbed
Area to be roofed or paved
Area to be vegetatively stabilized
Total Cut

= 116.96 Acres = 69 Acres = 60 Acres = 9 Acres = 206120 Cu. yds.

- Total Cut = 206120 Cu. yds.

 Total Fill = 92895 Cu. yds.

 Offsite waste/borrow area location: N/A
- 8. Any sediment control practice which is disturbed by grading activity for placement of utilities must be repaired on the same day of disturbance.
- Additional sediment controls must be provided, if deemed necessary by the Howard County Sediment Control Inspector.
- 10. On all sites with disturbed areas in excess of 2 acres, approval of the inspection agency shall be requested upon completion of installation of perimeter erosion and sediment controls, but before proceeding with any other earth disturbance or grading. Other building or grading inspection approvals may not be authorized until this initial approval by the inspection agency is made.
- 11. Trenches for the construction of utilities is limited to three pipe lengths or that which shall be back—filled and stabilized within one working day, whichever is shorter.

PERMANENT SEEDING NOTES

Apply to graded or cleared areas not subject to immediate further disturbance where a permanent long—lived vegetative cover is needed.

<u>Seedbed Preparation:</u> Loosen upper three inches of soil by raking, disking or other acceptable means before seeding, if not previously loosened.

<u>Soil Amendments:</u> In lieu of soil test recommendations, use one of the following

- 1) <u>Preferred</u> Apply 2 tons per acre dolomitic limestone (92 lbs/1000 sq. ft.) and 600 lbs per acre 10—10—10 fertilizer (14 lbs/1000 sq. ft.) before seeding. Harrow or disk into upperthree inches of soil. At time of seeding, apply 400 lbs/acre 30—0—0 ureaform fertilizer (9 lbs/1000 sq. ft.)
- 2) Acceptable: Apply 2 tons per acre dolomitic limestone (92 lbs/1000 sq. ft.) and 1000 lbs/acre 10-10-10 fertilizer (23 lbs/1000 sq. ft.) before seeding. Harrow or disk into upper three inches of soil.

Seeding — For the periods March 1 thru April 30, and August 1 thru October 15, seed with 60 lbs/acre (1.4 lbs/1000 sq. ft.) of Kentucky 31 Tall Fescue. For the period May 1 thru July 31, seed with 60 lbs Kentucky 31 Tall Fescue per acre and 2 lbs per acre (.05 lbs/1000 sq. ft.) of weeping lovegrass. During the period of October 16 thru February 28, protect site by: Option (1) 2 tons per acre of well anchored straw mulch and seed as soon as possible in the Spring. Option (2) Use sod. Option (3) Seed with 60 lbs/acre Kentucky 31 Tall Fescue and mulch with 2 tons/acre well anchored straw.

Mulching — Apply 1—1/2 to 2 tons per acre (70 to 90 lbs/1000 sq. ft.) of unrotted small grain straw immediately after seeding. Anchor mulch immediately after application using mulch anchoring tool or218 gallons/acre (5 gal/1000 sq. ft.) of emulsified asphalt on flat areas. On slopes 8 feet or higher, use 348 gallons/acre (8 gal/1000 sq. ft.) for anchoring

<u>Maintenance</u> — Inspect all seeded areas and make needed repairs, replacements and redeedings.

TEMPORARY SEEDING NOTES

Apply to graded or cleared areas likely to be redisturbed where a short—term vegetative cover is needed.

<u>Seedbed Preparation:</u> Loosen upper three inches of soil by raking, disking or other acceptable means before seeding, if not previously loosened.

Soil Amendments: Apply 60 lbs/ acre 10-10-10 fertilizer (14 lbs/1000 sq. ft.) Seeding: For Periods March 1 thru April 30 and August 15 thru October 15, seed with 2-1/2 bushel/acre of annual rye (3.2 lbs/1000 sq. ft.) For the period May 1 thru August 14, seed with 3 lbs/acre of weeping lovegrass (.07 lbs/1000 sq. ft.). For the period November 16 thru February 28, protect site by applying 2 tons/acre of well anchored straw mulch and see das soon as possible in the Spring, or use sod.

BLOCK NO

Mulching: — Apply 1—1/2 to 2 tons/acre (70 to 90 lbs/1000 sq. ft.) of unrotted small grain straw immediately after seeding. Anchor mulch immediately after application using mulch anchoring tool or 218 gal/acre (5 gal/1000 sq. ft.) of emulsified asphalt on flat areas. On slopes 8 ft. or higher, use 348 gal/acre (8 gal/1000 sq. ft.) for anchoring

DEPARTMENT OF PUBLIC WORKS

DIRECTOR OF PUBLIC WORKS

DATE

CHIEF, PACERAL OF HIGHWAYS, PATE

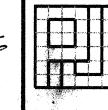
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PURDUM & JESCHKE
CONSULTING ENGINEERS
LAND SURVEYORS

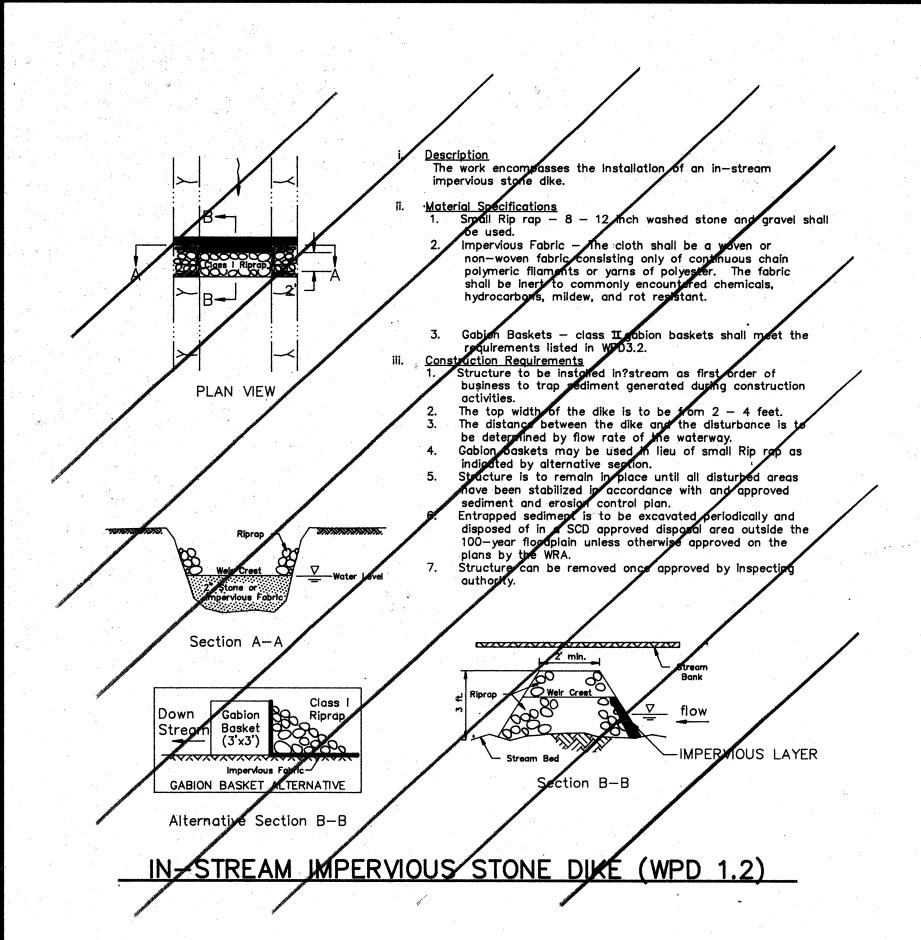
1029 North Calvert Street
Baltimore, Maryland 21202
Tel: (410)837-0194 Fax:(410)837-343

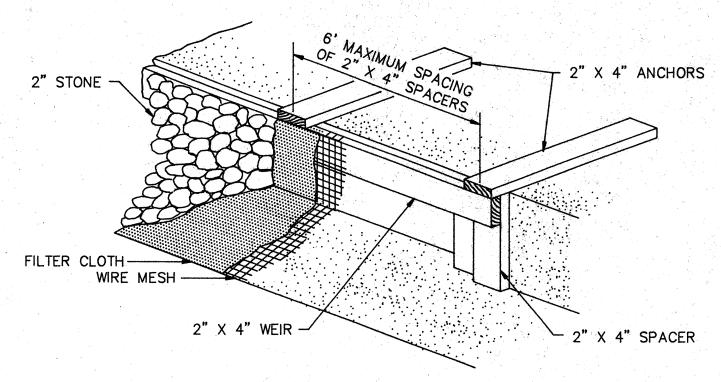


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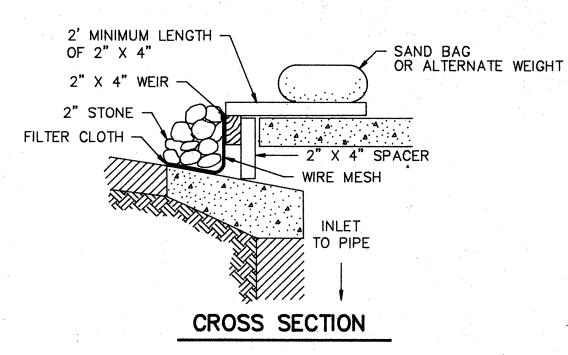
NORTH LAUREL DRAINAGE IMPROVEMENT CAPITAL PROJECT D-1081 SEDIMENT CONTROL DETAILS SCALE
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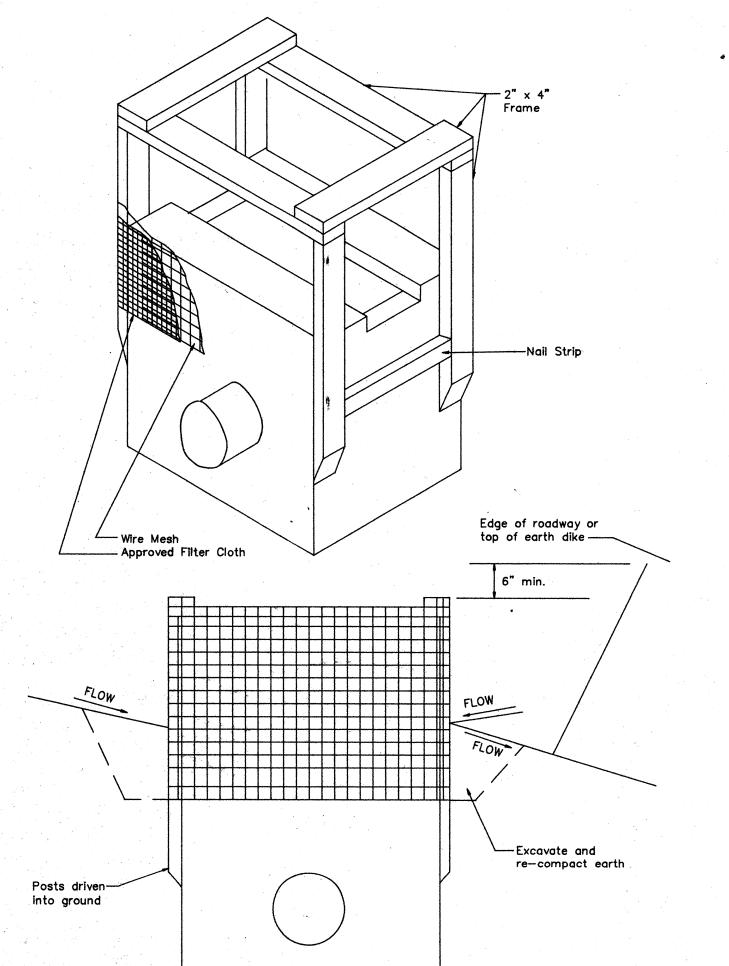




PERSPECTIVE VIEW



CURB INLET PROTECTION DETAIL



Construction Specifications

1. Excavate completely around the inlet to a depth of 18" below the

2. Drive the 2" x 4" construction grade lumber posts 1' into the ground at each corner of the inlet. Place nail strips between the posts on the ends of the inlet. Assemble the top portion of the 2" x 4" frame using the overlap joint shown on Detail 23A. The top of the frame (weir) must be 6" below adjacent roadways where flooding and safety issues may arise.

3. Stretch the 1/2' x 1/2' wire mesh tightly around the frame and fasten securely. The ends must meet and overlap at a

4. Stretch the Geotextile Class E tightly over the wire mesh with the geotixtile extending from the top of the frame to 18' below the inlet notch elevation. Fasten the geotextile firmly to the frame. The ends of the geotextile must meet at a post, be overlapped and folded, then fastened down.

5. Backfill around the inlet in compacted 6" layers until the layer of earth is level with the notch elevation on the ends and top elevation on the sides.

6. If the inlet is not in a sump, construct a compacted earth dike across the ditch line directly below it. The top of the earth dike should be at least 6" higher than the top of the frame.

7. The structure must be inspected periodically and after each rain and the geotextile replaced when it becomes clogged.

SWALE INLET PROTECTION DETAIL

DEPARTMENT OF PUBLIC WORKS

PURDUM & JESCHKE CONSULTING ENGINEERS LAND SURVEYORS 1029 North Calvert Street

Baltimore, Maryland 21202 Tel: (410)837-0194 Fax:(410)837-3431



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Construction Specifications

- 1. Attach a continuous piece of wire mesh (30" minimum width by throat length plus 4') to the 2" x 4" weir (measuring throat length plus 2') as shown on the standard
- 2. Place a continuous piece of Geotextile Class E the same dimensions as the wire mesh over the wire mesh and securely attach it to the 2" x 4" weir.
- 3. Securely nail the 2" X 4" weir to a 9" long vertical spacer to be located between the weir and the inlet face (max. 4' apart).
- 4. Place the assembly against the inlet throat and nail (minimum 2' lengths of 2" x 4" to the top of the weir at spacer locations). These 2" x 4" anchors shall extend across the inlet top and be held in place by sandbags or alternate weight.
- 5. The assembly shall be placed so that the end spacers are a minimum 1' beyond both ends of the throat opening.
- 6. Form the 1/2 " x 1/2" wire mesh and the geotextile fabric to the concrete gutter and against the face of the curb on both sides of the inlet. Place clean 3/4 " x 1 1/2" stone over the wire mesh and geotextile in such a manner to prevent water from entering the inlet under or around the geotextile.

600' SCALE MAP NO.

BLOCK NO.

- 7. This type of protection must be inspected frequently and the filter cloth and stone replaced when clogged with sediment.
- 8. Assure that storm flow does not bypass the inlet by installing a temporary earth or asphalt dike to direct the flow to the inlet.

STANDARD AND SPECIFICATIONS STORM DRAIN INLET PROTECTION

Definition

Filter cloth installed around inlets in the form of a fence or across an opening, thereby reducing sediment content of sediment laden water.

Purpose

To prevent sediment laden water from entering a storm drain system through inlets.

Conditions Where Practice Applies

This practice shall be used where the drainage area to an inlet is disturbed, it is not possible to temporarily divert the storm drain outfall into a sediment trapping device and watertight blocking of inlets is not advisable. It is not to be used in place of sediment trapping devices. This practice may be used in conjunction with storm drain diversion to help prevent siltation of pipes installed with a low slope angle.

Construction Specifications

Materials

- A. Wooden frame is to be constructed of 2" x 4" construction grade lumber.
- Wire mesh must be of sufficient strength to support filter fabric, and stone for curb inlets, with water fully impounded against it.
- C. Filter cloth must be of a type approved for this purpose; resistant to sunlight with sieve size, EOS, 40-85, to allow sufficient passage of water and removal of sediment.
- D. Stone is to be 2" in size and clean, since fines would clog the cloth.

II. Procedure

- A. A swale, ditchline or yard inlet protection.
 - 1. Excavate completely around inlet to a depth of 18" below notch
 - 2. Drive 2 x 4 post 1' into ground at four corners of inlet. Place nail strips between posts on ends of inlet. Assemble top portion of 2 x 4 frame using overlap joint shown. Top of frame (weir) must be 6" below edge of roadway adjacent to inlet.
 - 3. Stretch wire mesh tightly around frame and fasten securely. Ends must meet at post.
 - 4. Stretch filter cloth tightly over wire mesh, the cloth must extend from top of frame to 18" below inlet notch elev. Fasten securely to frame. Ends must meet at post, be overlapped and folded, then fastened
 - 5. Backfill around inlet in compacted 6" layers until layer of earth is even with notch elevation on ends and top elevation on sides.
 - 6. If the inlet is not in a low point, construct a compacted earth dike in the ditch line below it. The top of this dike is to be at least 6" higher than the top of frame (weir).
 - 7. This structure must be inspected frequently and the filter fabric replaced when clogged.

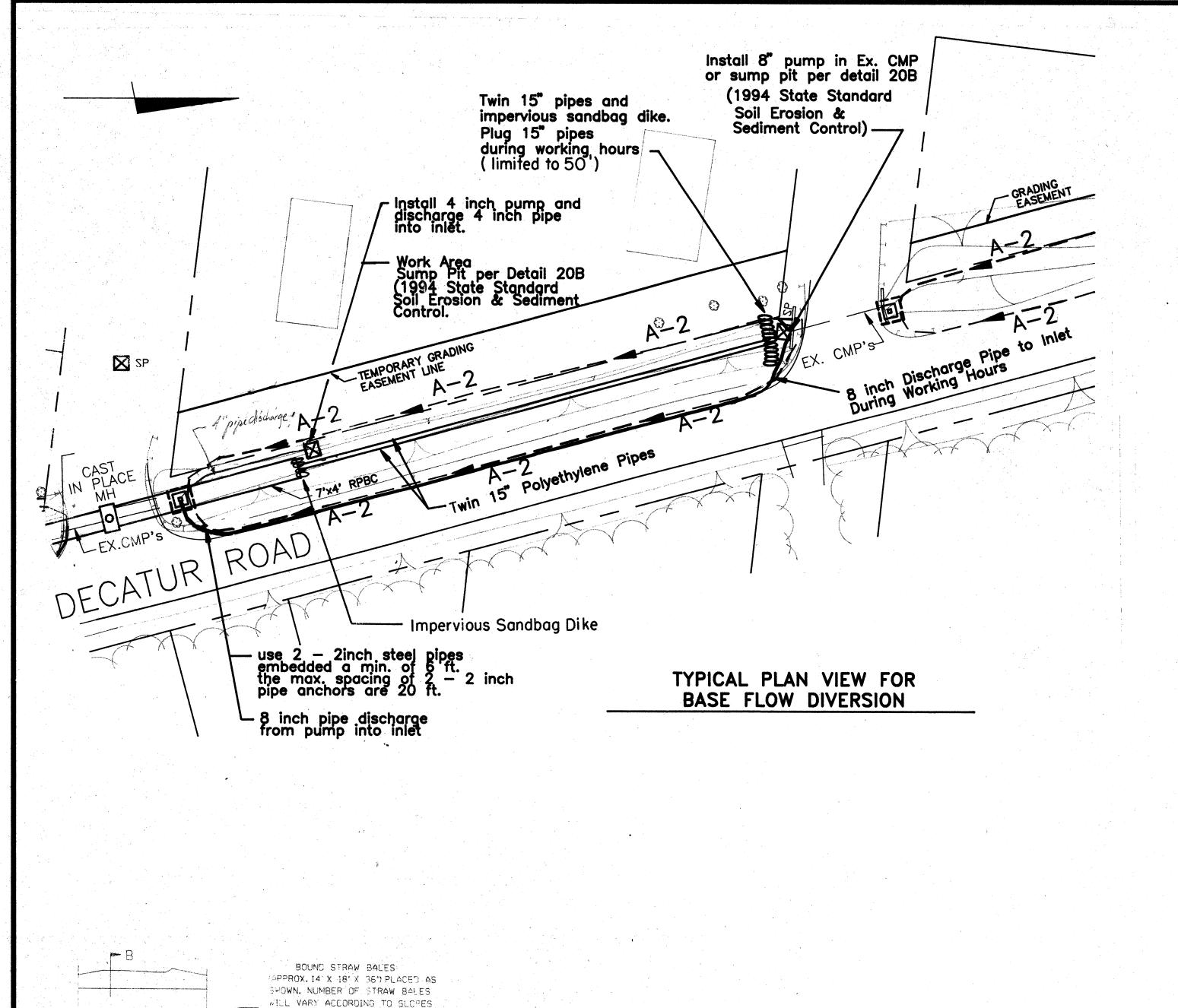
B. Curb Inlet Protection.

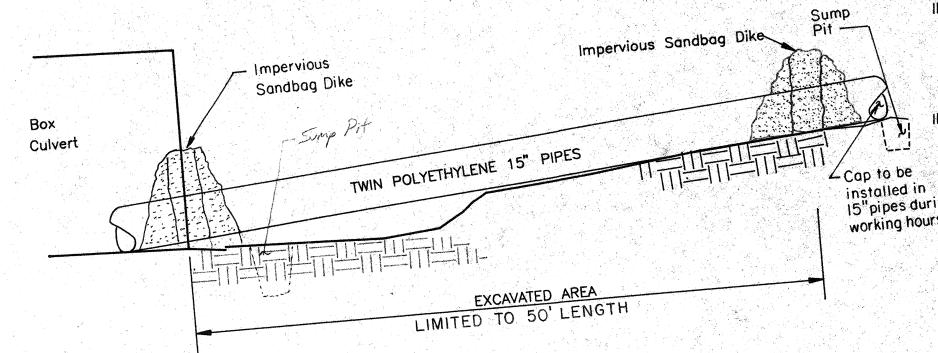
- 1. Attach a continuous piece of wire mesh (30" min. width by throat length plus 4') to the 2" x 4" weir (measuring throat length plus 2') as shown on the standard drawing.
- 2. Place a piece of approved filter cloth (40-85 sieve) of the same dimensions as the wire mesh over the wire mesh and securely attach to the 2" x 4" weir.
- 3. Securely nail the 2" x 4" weir to 9" long vertical spacers to be located between the weir and inlet face (max. 6' apart).
- 4. Place the assembly against the inlet throat and nail (minimum 2' lengths of 2" x 4" to the top of the weir at spacer locations. These 2" x 4" anchors shall extend across the inlet top and be held in place by sandbags or alternate weight.
- 5. The assembly shall be placed so that the end spacers are a minimum 1' beyond both ends of the throat opening.
- 6. Form the wire mesh and filter cloth to the concrete gutter and against the face of curb on both sides of the inlet. Place clean 2" stone over the wire mesh and filter fabric in such a manner as to prevent water from entering the inlet under or around the filter cloth.
- 7. This type of protection must be inspected frequently and the filter cloth and stone replaced when clogged with sediment.
- 8. Assure that storm flow does not bypass inlet by installing temporary earth or asphalt dikes directing flow into inlet.

ASI SHOWN SHEET

NORTH LAUREL DRAINAGE IMPROVEMENT CAPITAL PROJECT D-1081 SEDIMENT CONTROL DETAILS

19





Place twin 15" pipes in operation to gravity convey base flow during non-working hours.

TWIN 15" PIPES AND SANDBAG DIKE

15"pipes during working hours

1. Description

The work encompasses the installation of in-stream impervious sandbag dike to be used as a flow barrier for streams.

Material Specifications

Impervious Fabric - The cloth shall be impervious to stream base flow. The fabric shall be inert to commonly encountered chemicals, hydrocarbons, mildew, and rot resistant.

Construction Requirements

- The top width of the dike is to be from 2-4 feet.
- Structure is to remain in place until all disturbed areas have been stabilized in accordance with an approved sediment and erosion control plan
- Entrapped sediment is to be excavated periodically and disposed of in a SCD approved disposal area outside the 100—year floodplain unless otherwise approved on the plans by the WRA.
- 4. Structure can be removed once approved by inspecting authority.



BLOCK NO.

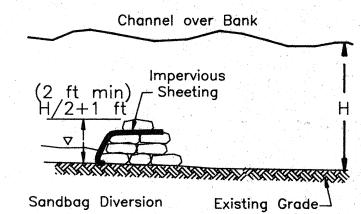
CLEAN WATER DISCHARGE SUCTION LINE TO PUMP ----THE TOP OF THE STANDPIPE - 3" MINIMUM SHOULD EXTEND AT LEAST 12' TO 18" ABOVE THE TOP OF THE PIT OR ABOVE STANDING WATER. _EXISTING GROUND LINE -STANDPIPE WRAPPED IN 1/2" HARDWARE CLOTH AND GEDTEXTILE CLASS E - SIDE SLOPE (VARIES) ₩ 12' - 36" DIAMETER PERFORATED CORRUGATED METAL OR PVC PIPE -WATERTIGHT CAP OR PLATE CLEAN GRAVEL OR AASHTO M-43 # 57 STONE BEFORE INSTALLING AGGREGATE FILL STANDPIPE. CROSS SECTION

> STANDARD SYMBOL **⊠** SP

Construction Specifications

- 1. Pit dimensions are variable, with the minimum diameter being 2 times the standpipe diameter.
- 2. The standpipe should be constructed by perforating a 12" to 24" diameter corrugated or PVC pipe. Then wrapping with 1/2" hardware cloth and Geotextile Class E. The perforations shall be 1/2" x 6" slits or 1' diameter holes.
- 3. A base of filter material consisting of clean gravel or #57 stone should be placed in the pit to a depth of 12". After installing the standpipe, the pit surrounding the standpipe should then be backfilled with the same filter material.
- 4. The standpipe should extend 12" to 18" above the lip of the pit on the riser crest elevation (basin dewatering only) and the filter material should extend 3' minimum above the anticipated standing water elevation.

SUMP PIT NOT TO SCALE



<u>Description</u>

The work shall consist of installing a flow diversion structure when construction activities take place within the stream channel such as culvert construction or storm drain construction.

Material Specifications

- 1. Sandbags: Sandbags shall consist of materials which are resistant to ultra-violet radiation, tearing and puncture and woven tightly enough to prevent leakage of fill material (i.e., sand, fine gravel, etc.).
- 3. Sheeting: Sheeting shall consist of polyethylene or other material which is impervious and resistant to puncture and tearing.

Construction Requirements

- All erosion and sediment control devices shall be installed as the first order of work.
- The diversion structure shall be installed from upstream to downstream.
- The top width of dike is to be from 2 4 feet.
- The height of the diversion structure shall be half the distance from stream bed to stream bank plus one foot, as indicated on the cross-section view. 5. All excavated materials shall be disposed of in a scd approved disposal area outside
- the 100-year floodplain unless otherwise approved on the plans by the WRA.
- 6. All dewatering of the construction area shall be pumped from a sump pit prior to
- re-entering the system. Sheeting shall be overlapped such that the upstream portion covers the downstream
- portion with at least an 18 inch overlap. 8. Sediment control devices are to remain in place until all disturbed areas are stabilized in accordance with an approved sediment and erosion control plan and the Inspecting authority approves their removal.

600' SCALE MAP NO.

SANDBAG DIVERSION (WPD 2.3)

NOT TO SCALE

STRAW BALE DITCH CHECK (S.B.D.C.)

SECTION B-1

SEDIMENT TRAP WOULD CREATE A TRAFFIC HAZARD OR WHERE DIRECTED BY THE ENGINEER.

5. FOR USE IN EMERGENCY SITUATIONS ONLY, FOR A TIME PERIOD NOT TO EXCEED 2 WEEKS.

3. INSPECTION SHALL BE FREQUENT AND REPAIR OR REPLACEMENT SHALL BE MADE PROMPTLY AS NEEDED.

4. STRAW BALES SHALL BE REMOVED WHEN THEY HAVE SERVED THEIR USEFULNESS SO AS NOT TO BLOCK OR

1. INTENDED FOR USE IN EXISTING, PROPOSED AND TEMPORARY DITCHES OF ALL TYPES WHERE THE CONSTRUCTION OF A

AND WIDTH OF DITCH.

DITCH - FOR TYPE, SIZE.

LOCATION, ETC. SEE

CONSTRUCTION PLANS.

SECTION A-A

OR ROUG- OR FINISHED 2" X 2" WOOD

STAKES, 3'-0" LENGTH DRIVEN 1'-0" TO 1'-6' INTO GROUND ANGLE FIRST SECURING

DEVICE TOWARD PREVIOUSLY LAID BALE.

NO 4 PENFORCING BARS, STEEL PICKETS

DIRECTION

- SLOPES VAR

2. FOR LOCATIONS OF STRAW BALES REFER TO CONSTRUCTION PLANS.

APPROX. THIRD'S ALONG C

IMPEDE STORM FLOW OR DRAINAGE.

EACH BALE

GENERAL NOTES:

DEPARTMENT OF PUBLIC WORKS

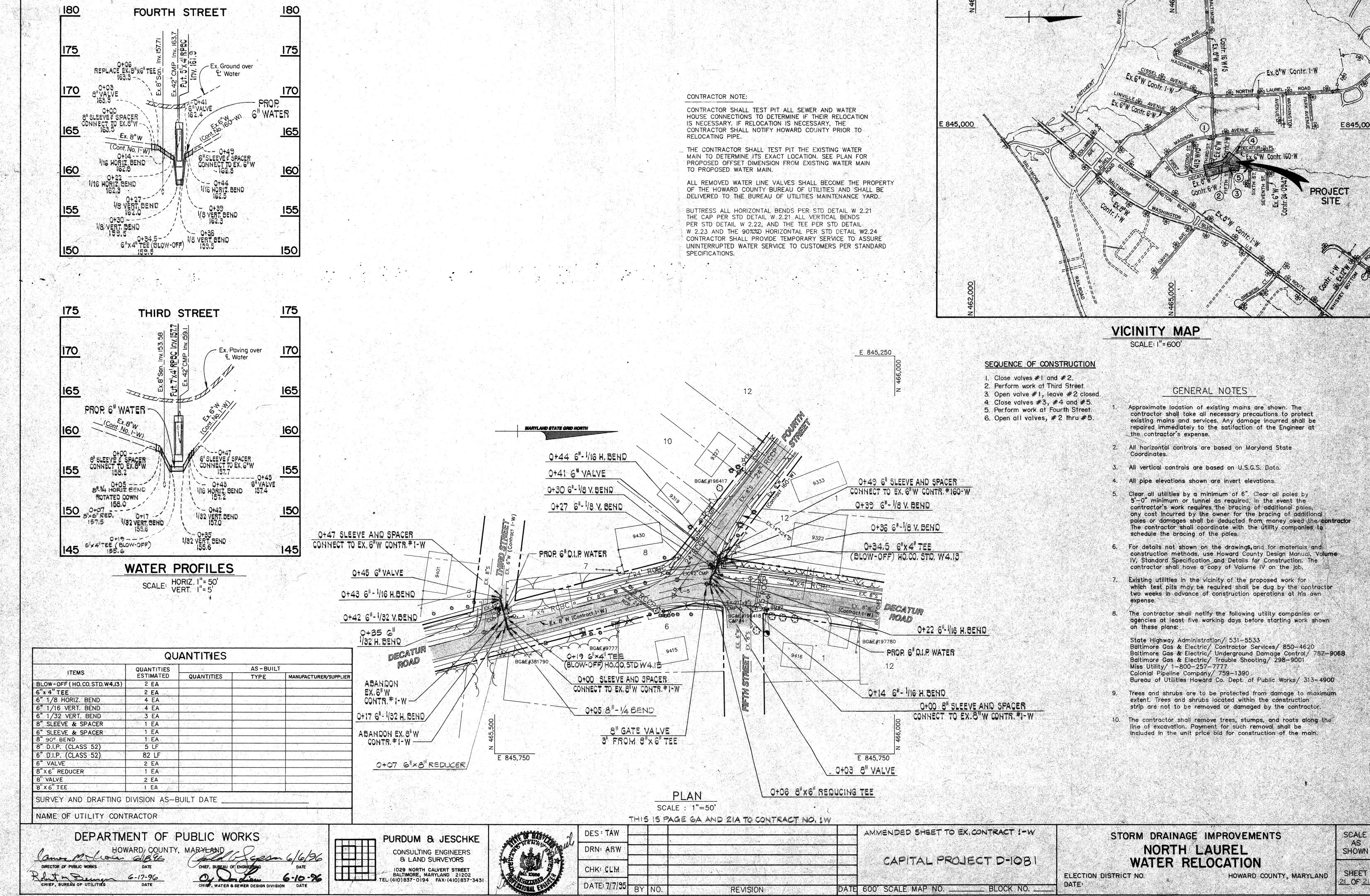




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NORTH LAUREL DRAINAGE IMPROVEMENT CAPITAL PROJECT D-1081 SEDIMENT & EROSION CONTROL PLAN

SHEET



REVISION

DATE 600' SCALE MAP NO. _

_ BLOCK NO. .

SCALE AS SHOWN